

## WARRANTY

WARRANTY to Transbay Joint Powers Authority  
(Owner Name)  
201 Mission, Suite 2100, San Francisco, CA  
(Owner Address)

We hereby warrant and guarantee that the \_\_\_\_\_  
(Description of Work)

which we have installed at Transbay Transit Center has been done in strict accordance with the plans and specifications, and that the work installed will fulfill the requirements of those specifications.

We agree to repair or replace, or cause to be repaired or replaced, any or all of the work which may prove to be defective in workmanship or materials, together with any adjacent work which required repair or replacement because of our defective work within a period of \_\_\_\_\_ year(s) from the filing of the Notice of Completion on all improvements, or acceptance by the Owner of the building, whichever is later.

If we fail to commence to comply with the above paragraph within ten (10) days after receipt of written notice, or fail to pursue such compliance with diligence, we jointly, and severally, do hereby authorize the Owner or the General Contractor to proceed to have the defects repaired and made good at our sole expense, and we will honor and pay the costs and charges for it together with interest at the maximum rate permitted by law upon demand. If we fail to fulfill the preceding obligations, and if Owner or General Contractor bring an action to enforce this Warranty, we agree to pay Owner or General Contractor reasonable attorney's fees incurred in connection therewith.

SUBCONTRACTOR:

CONTRACTOR:

\_\_\_\_\_ WEBCOR/OBAYASHI JOINT VENTURE

BY: \_\_\_\_\_ BY: \_\_\_\_\_

DATE: \_\_\_\_\_ DATE: \_\_\_\_\_

LICENSE NO. \_\_\_\_\_ LICENSE NO. 928731A, B, C-8

LOCAL REPRESENTATIVE TO BE CONTACTED FOR SERVICE:

NAME: \_\_\_\_\_

ADDRESS: \_\_\_\_\_

TELEPHONE: \_\_\_\_\_



## Exhibit C

### LIEN RELEASES

#### Form Number

#### Form Title

1034	Conditional Waiver and Release Upon Progress Payment
1035	Unconditional Waiver and Release Upon Progress Payment
1036	Conditional Waiver and Release Upon Final Payment
1037	Unconditional Waiver and Release Upon Final Payment

**CONDITIONAL WAIVER AND RELEASE ON PROGRESS PAYMENT**  
***California Civil Code Section 8132***

NOTICE: THIS DOCUMENT WAIVES THE CLAIMANT'S LIEN, STOP PAYMENT NOTICE, AND PAYMENT BOND RIGHTS EFFECTIVE ON RECEIPT OF PAYMENT. A PERSON SHOULD NOT RELY ON THIS DOCUMENT UNLESS SATISFIED THAT THE CLAIMANT HAS RECEIVED PAYMENT.

**Identifying Information**

Name of Claimant: \_\_\_\_\_  
Name of Customer: Webcor/Obayashi Joint Venture  
Job Location: Transbay Transit Center 425 Mission St. San Francisco, California  
Owner: Transbay Joint Powers Authority  
Through Date: \_\_\_\_\_

**Conditional Waiver and Release**

This document waives and releases lien, stop payment notice, and payment bond rights the claimant has for labor and service provided, and equipment and material delivered, to the customer on this job through the Through Date of this document. Rights based upon labor or service provided, or equipment or material delivered, pursuant to a written change order that has been fully executed by the parties prior to the date that this document is signed by the claimant, are waived and released by this document, unless listed as an Exception below. This document is effective only on the claimant's receipt of payment from the financial institution on which the following check is drawn:

Maker of Check: Webcor/Obayashi Joint Venture  
Amount of Check: \$ \_\_\_\_\_  
Check Payable to: \_\_\_\_\_

**Exceptions**

This document does not affect any of the following:

- (1) Retentions.
- (2) Extras for which the claimant has not received payment.
- (3) The following progress payments for which the claimant has previously given a conditional waiver and release but has not received payment:

Date(s) of waiver and release: \_\_\_\_\_

Amount(s) of unpaid progress payment(s): \$ \_\_\_\_\_

- (4) Contract rights, including (A) a right based on rescission, abandonment, or breach of contract, and (B) the right to recover compensation for work not compensated by the payment.

**Signature**

Claimant's Signature: \_\_\_\_\_  
Claimant's Title: \_\_\_\_\_  
Date of Signature: \_\_\_\_\_

**UNCONDITIONAL WAIVER AND RELEASE ON PROGRESS PAYMENT**  
***California Civil Code Section 8134***

NOTICE TO CLAIMANT: THIS DOCUMENT WAIVES AND RELEASES LIEN, STOP PAYMENT NOTICE, AND PAYMENT BOND RIGHTS UNCONDITIONALLY AND STATES THAT YOU HAVE BEEN PAID FOR GIVING UP THOSE RIGHTS. THIS DOCUMENT IS ENFORCEABLE AGAINST YOU IF YOU SIGN IT, EVEN IF YOU HAVE NOT BEEN PAID. IF YOU HAVE NOT BEEN PAID, USE A CONDITIONAL WAIVER AND RELEASE FORM.

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Claimant's Signature: \_\_\_\_\_  
Claimant's Title: \_\_\_\_\_  
Date of Signature: \_\_\_\_\_



**CONDITIONAL WAIVER AND RELEASE ON FINAL PAYMENT**  
***California Civil Code Section 8136***

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Amount of Check: \$ \_\_\_\_\_  
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**Exceptions**

This document does not affect any of the following:

Disputed claims for extras in the amount of: \$ \_\_\_\_\_

**Signature**

Claimant's Signature: \_\_\_\_\_  
Claimant's Title: \_\_\_\_\_  
Date of Signature: \_\_\_\_\_

**UNCONDITIONAL WAIVER AND RELEASE ON FINAL PAYMENT**  
***California Civil Code Section 8138***

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Claimant's Title: \_\_\_\_\_

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**Signature**

Claimant's Signature: \_\_\_\_\_

Claimant's Title: \_\_\_\_\_

Date of Signature: \_\_\_\_\_



## Exhibit D

### SAMPLE CERTIFICATE OF INSURANCE AND ADDITIONAL INSURED ENDORSEMENT

**Form Number****Form Title**

ACCORD 25

Certificate of Liability Insurance

CG 201 10 11 85

Additional Insured - Owners, Lessees or Contractors (Form B) - Commercial General Liability

WC 04 03 06

Waiver of Our Right to Recover from Others Endorsement



# CERTIFICATE OF LIABILITY INSURANCE

DATE (MM/DD/YYYY)

THIS CERTIFICATE IS ISSUED AS A MATTER OF INFORMATION ONLY AND CONFERS NO RIGHTS UPON THE CERTIFICATE HOLDER. THIS CERTIFICATE DOES NOT AFFIRMATIVELY OR NEGATIVELY AMEND, EXTEND OR ALTER THE COVERAGE AFFORDED BY THE POLICIES BELOW. THIS CERTIFICATE OF INSURANCE DOES NOT CONSTITUTE A CONTRACT BETWEEN THE ISSUING INSURER(S), AUTHORIZED REPRESENTATIVE OR PRODUCER, AND THE CERTIFICATE HOLDER.

**IMPORTANT:** If the certificate holder is an ADDITIONAL INSURED, the policy(ies) must be endorsed. If SUBROGATION IS WAIVED, subject to the terms and conditions of the policy, certain policies may require an endorsement. A statement on this certificate does not confer rights to the certificate holder in lieu of such endorsement(s).

<b>PRODUCER</b> ANY AGENT OR BROKER STREET ADDRESS CITY, STATE, ZIP PHONE/FAX		<b>CONTACT NAME:</b> <b>PHONE (A/C, No, Ext):</b> <b>E-MAIL ADDRESS:</b>		<b>FAX (A/C, No):</b>
<b>INSURED</b> ABC SUBCONTRACTOR STREET ADDRESS CITY, STATE, ZIP		<b>INSURER A:</b>		<b>XYZ INSURANCE COMPANY</b> <b>(RATED A-VII OR BETTER BY AM BEST)</b> <b>NAIC #</b>
		<b>INSURER B:</b>		
		<b>INSURER C:</b>		
		<b>INSURER D:</b>		
		<b>INSURER E:</b>		
		<b>INSURER F:</b>		

**COVERAGES****CERTIFICATE NUMBER:****REVISION NUMBER:**

THIS IS TO CERTIFY THAT THE POLICIES OF INSURANCE LISTED BELOW HAVE BEEN ISSUED TO THE INSURED NAMED ABOVE FOR THE POLICY PERIOD INDICATED. NOTWITHSTANDING ANY REQUIREMENT, TERM OR CONDITION OF ANY CONTRACT OR OTHER DOCUMENT WITH RESPECT TO WHICH THIS CERTIFICATE MAY BE ISSUED OR MAY PERTAIN, THE INSURANCE AFFORDED BY THE POLICIES DESCRIBED HEREIN IS SUBJECT TO ALL THE TERMS, EXCLUSIONS AND CONDITIONS OF SUCH POLICIES. LIMITS SHOWN MAY HAVE BEEN REDUCED BY PAID CLAIMS.

INSR LTR	TYPE OF INSURANCE	ADDL INSR	SUBR WVD	POLICY NUMBER	POLICY EFF (MM/DD/YYYY)	POLICY EXP (MM/DD/YYYY)	LIMITS	
A	<b>GENERAL LIABILITY</b> <input checked="" type="checkbox"/> <b>COMMERCIAL GENERAL LIABILITY</b> <input type="checkbox"/> <b>CLAIMS-MADE</b> <input checked="" type="checkbox"/> <b>OCCUR</b>	X	X	XYZ123456			<b>EACH OCCURRENCE</b> \$ 1,000,000 <b>DAMAGE TO RENTED PREMISES (Ea occurrence)</b> \$ TBD <b>MED EXP (Any one person)</b> \$ TBD <b>PERSONAL &amp; ADV INJURY</b> \$ 1,000,000 <b>GENERAL AGGREGATE</b> \$ 2,000,000 <b>PRODUCTS - COMP/OP AGG</b> \$ 2,000,000	
	<b>GEN'L AGGREGATE LIMIT APPLIES PER:</b> <input type="checkbox"/> <b>POLICY</b> <input checked="" type="checkbox"/> <b>PRO-JECT</b> <input type="checkbox"/> <b>LOC</b>							
	A	<b>AUTOMOBILE LIABILITY</b> <input checked="" type="checkbox"/> <b>ANY AUTO</b> <input type="checkbox"/> <b>ALL OWNED AUTOS</b> <input type="checkbox"/> <b>HIRED AUTOS</b>			XYZ654321			<b>COMBINED SINGLE LIMIT (Ea accident)</b> \$ 1,000,000 <b>BODILY INJURY (Per person)</b> \$ <b>BODILY INJURY (Per accident)</b> \$ <b>PROPERTY DAMAGE (Per accident)</b> \$
		<input type="checkbox"/> <b>SCHEDULED AUTOS</b> <input type="checkbox"/> <b>NON-OWNED AUTOS</b>						
A		<b>UMBRELLA LIAB</b> <input checked="" type="checkbox"/> <b>OCCUR</b> <b>EXCESS LIAB</b> <input type="checkbox"/> <b>CLAIMS-MADE</b>			XYZ123456			<b>EACH OCCURRENCE</b> \$ <b>AGGREGATE</b> \$
		<b>DED</b> <input type="checkbox"/> <b>RETENTION \$</b> <input type="checkbox"/>						
A	<b>WORKERS COMPENSATION AND EMPLOYERS' LIABILITY</b> ANY PROPRIETOR/PARTNER/EXECUTIVE OFFICER/MEMBER EXCLUDED? (Mandatory in NH) If yes, describe under DESCRIPTION OF OPERATIONS below	N/A	X	XYZ123456			<input checked="" type="checkbox"/> <b>WC STATU-TORY LIMITS</b> <input type="checkbox"/> <b>OTH-ER</b> <b>E.L. EACH ACCIDENT</b> \$ 1,000,000 <b>E.L. DISEASE - EA EMPLOYEE</b> \$ 1,000,000 <b>E.L. DISEASE - POLICY LIMIT</b> \$ 1,000,000	
A	<b>POLLUTION LIABILITY</b> <b>PROFESSIONAL LIABILITY</b>			XYZ123456 XYZ123456				

DESCRIPTION OF OPERATIONS / LOCATIONS / VEHICLES (Attach ACORD 101, Additional Remarks Schedule, if more space is required)

RE: Transbay Transit Center Building

**CERTIFICATE HOLDER****CANCELLATION**Webcor/Obayashi Joint Venture  
951 Mariners Island Blvd., 7th Floor  
San Mateo, CA 94404-2514

SHOULD ANY OF THE ABOVE DESCRIBED POLICIES BE CANCELLED BEFORE THE EXPIRATION DATE THEREOF, NOTICE WILL BE DELIVERED IN ACCORDANCE WITH THE POLICY PROVISIONS.

AUTHORIZED REPRESENTATIVE

Mary Jane Doe

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# **WAIVER OF OUR RIGHT TO RECOVER FROM OTHERS ENDORSEMENT**

This endorsement changes the policy to which it is attached effective on the inception date of the policy unless a different date is indicated below.

(The following "attaching clause" needs to be completed only when this endorsement is issued subsequent to preparation of the policy.)

This endorsement forms a part of Policy No. XYZ 1234567

Issued to: ABC SUBCONTRACTOR

By: XYZ INSURANCE COMPANY

Premium (if any) TBD

We have a right to recover our payments from anyone liable for an injury covered by this policy. We will not enforce our right against the person or organization named in the Schedule. (This agreement applies only to the extent that you perform work under a written contract that requires you to obtain this agreement from us).

You must maintain payroll records accurately segregating the remuneration of your employees while engaged in the work described in the Schedule.

The additional premium for this endorsement shall be 2-5% of the California workers compensation premium otherwise due on such remuneration.

## **Schedule**

Person or Organization	Job Description
WEBCOR/OBAYASHI JOINT VENTURE, Its Officers, Directors and Employees AND TRANSBAY JOINT POWERS AUTHORITY, its Board Members and Commissions, All Authorized Agents and Representatives, and Members, Directors, Officers, Trustees, Agents and Employees of Any of Them.	TRANSBAY TRANSITY CENTER BUILDING.

**WAIVER OF SUBRAGATION FOR WORKERS COMPENSATION INSURANCE TO BE INCLUDED.**

POLICY NUMBER: XYZ 1234567

COMMERCIAL GENERAL LIABILITY

**THIS ENDORSEMENT CHANGES THE POLICY. PLEASE READ IT CAREFULLY.**

**ADDITIONAL INSURED – OWNERS, LESSEES OR  
CONTRACTORS (FORM B)**

This endorsement modifies insurance provide under the following:

**COMMERCIAL GENERAL LIABILITY COVERAGE PART**

**SCHEDULE**

Name of Person or Organization:

WEBCOR/OBAYASHI JOINT VENTURE,  
Its Officers, Directors and Employees

AND

TRANSBAY JOINT POWERS AUTHORITY, its Board Members  
and Commissions, All Authorized Agents and Representatives, and  
Members, Directors, Officers, Trustees, Agents and Employees of Any  
of Them.

RE:

TRANSBAY TRANSIT CENTER BUILDING.

(If no entry appears above, information required to complete this endorsement will be shown in the Declarations as applicable to this endorsement.)

WHO IS AN INSURED (Section II) is amended to include as an insured the person or organization shown in the Schedule, but only with respect to liability arising out of "your work" for that Insured by or for you.

If required by your agreement with such insured, this insurance shall be primary insurance for such Insured. If anyone also provides similar insurance for such Insured, then that insurance will be primary, and this insurance will be excess over, or secondary to that insurance.

"The insurance afforded by this policy for the additional insured(s) is primary insurance and any other insurance maintained by or available to the additional Insured(s) is non-contributory."

**WAIVER OF SUBROGATION - WORKERS COMP**

CG 20 10 11 85

Exhibit "D"



## **TRANSBAY TRANSIT CENTER**

### **LEED Subcontractor Submission Letter and Data Sheet**

**March 13, 2014 Revision 3**

**WEBCOR/OBAYASHI JOINT VENTURE  
SAN FRANCISCO, CA**

**EXHIBIT E**

## Exhibit E – LEED Trade Subcontractor Submission Letter & Data Sheet



Transbay Transit Center  
Webcor/Obayashi Joint Venture  
175 Beale Street  
San Francisco, CA 94105  
T 415-978-5700

To Whom It May Concern:

In our efforts to complete LEED Documentation for the **Transbay Transit Center Project** we will need the following information provided on your official company letter head:

1. Company Name & Contact Information
2. Contract Value
3. Progress Report Date
4. Scope of work included in Contract with specific Division and Sections listed.
5. **List of all materials permanently installed on the project**, within the LEED boundary that were included in the contract. A total estimated weight value and total actual material cost must be provided for each material. Please provide exact Material name & manufacturer, division and specification section number (XX XX XX).
6. Recycled content (**post-consumer and pre-consumer broken out separately**) percentages for each material from CSI Masterformat 2004 Edition Divisions 3-10, 31 (Section 31 6X XX Foundations) and 32 (Sections 32 1X XX Paving, 32 3X XX Site Improvements, 32 9X XX Planting). Please provide cut sheets of each material with the recycled content values posted.
7. List the location of material extraction (city, state, country) and material manufacturing (city, state, country) for all materials from CSI Masterformat 2004 Edition Divisions 3-10, 31 (Section 31 6X XX Foundations) and 32 (Sections 32 1X XX Paving, 32 3X XX Site Improvements, 32 9X XX Planting). Specifically, we are looking for those materials that were **both extracted and manufactured** within a weighted total travel distance of 500 miles of the jobsite. Per addendum to the BD+C v2009 Reference Guide, weighted total travel distance under Option 2 of the MR Credit 5 Regional Materials should be calculated using the following formula: (Distance by rail/3) + (Distance by inland waterway/2) + (Distance by sea/15) + (Distance by all other means) ≤ 500 miles [800 kilometers]. If you are sure that your materials do not comply as Regional Material, please note that the material was extracted/manufactured "greater than 500 miles" from the jobsite.
8. If you provided any adhesives, sealants, coatings, paints, carpet systems, etc. – please be sure to include these materials on your spreadsheet with the **actual VOC content (g/L)**. Please provide proof in the form of a cutsheet, or MSDS highlighting the VOC content value.
  - a. All particleboard, MDF, Agrifiber, Veneers, and composite wood products must be **Urea-Formaldehyde free**. Please note "*Urea-Formaldehyde free*" in the VOC column for these material types. All Agrifiber/composite wood products must provide proof of being Urea-formaldehyde free in the form of MSDS, Cut Sheet, or Letter from the Manufacturer.

Exhibit E – LEED Trade Subcontractor Submission Letter & Data Sheet

9. For all materials that contain wood, please specify the FSC Wood **Chain of Custody number (COC)**. The COC Certificate and **original purchasing invoices** must be provided as proof of purchase/certification.

**LEED Submittals:**

- A. **Preliminary LEED Material Spreadsheet Submittal** - Within 30 days of Contract award, assemble and submit the “LEED Material Tracking Spreadsheet” complete with all data described in 4-8 above. Cover letter and back up documentation are not necessary for this submittal. The quantities, costs, products, and LEED metrics should be entered in the spreadsheet as the project/contract scope was bid/ estimated. Please see the sample LEED Material Tracking Spreadsheet that you must complete and submit back to Webcor/Obayashi Joint Venture within 30 days of awarded contract.
- B. **Quarterly LEED Progress Reports (Reference 01 81 13 1.5 E 2)** – Quarterly LEED Progress Reports are due by February 10 (Q1), May 10 (Q2), August 10 (Q3), and November 10 (Q4) of each year. Assemble and submit the “LEED Material Tracking Spreadsheet” in, PDF and Excel formats, complete with all data described in 4-8 above and based on the Preliminary LEED Material Spreadsheet Submittal. All changes from the previous quarter shall be indicated in bold. Update each Material Status with one of the following: Preliminary, Approved, Bought, or Installed.
  - a. Preliminary – Indicates the material has been included in its preliminary stage of planning but has yet to be approved by the design team.
  - b. Approved – Indicates the material has been approved by the Design Team as meeting all requirements specified. Include Design Team submittal approval.
  - c. Bought – Indicates the material has been bought out after approval by the Design Team.
  - d. Installed – Indicates the material has been permanently installed on the project within the LEED boundary.
- C. **Final Exhibit E Submittal** – Prior to closeout, assemble and submit all ‘actual’ LEED material information on the “LEED Material Tracking Spreadsheets” and forms provided in the Project Manual, together with all supplemental documentation as required by LEED. Please see the sample LEED cover letter and Material Tracking Spreadsheet that you must complete and submit back to Webcor/Obayashi Joint Venture prior to closeout on the project.

If you have any questions or concerns, please contact Webcor/Obayashi Joint Venture. If there is any information that you are not able to track down please let us know. We are here to support your LEED efforts.

Sincerely,

**WEBCOR/OBAYASHI JOINT VENTURE**

[Insert your company logo]  
[Type the sender address]  
Phone: [Type the sender phone number]

► Document Control  
Transbay Transit Center  
Webcor/Obayashi Joint Venture  
175 Beale Street  
San Francisco, CA 94105  
[docctrl@webcor-obayashi.com](mailto:docctrl@webcor-obayashi.com)

[Date]

To: Webcor/Obayashi Joint Venture,

Please find the following information regarding the scope of work that [subcontractor name] provided to the **Transbay Transit Center project** in San Francisco, CA.

1. Subcontractor's LEED Point of contact information:
  - a. Name: \_\_\_\_\_
  - Title: \_\_\_\_\_
  - Email: \_\_\_\_\_
  - Phone #: \_\_\_\_\_
2. The total contract value of our work is \$ \_\_\_\_\_
3. Final Status of all materials: [use LEED Material Spreadsheet]
4. Scope of work (Division/Section): [use LEED Material Spreadsheet]
5. List of Materials included in contract value (weight): [use LEED Material Spreadsheet]
6. Post-Consumer & Post-Industrial Recycled content values for each material (%): [use LEED Material Spreadsheet]
7. Location of Material Extraction & location of Material Manufacturing: [use LEED Material Spreadsheet]
8. VOC Content (g/L) for each material: [use LEED Material Spreadsheet]
  - a. VOC values only required for: adhesives, sealants, coatings, paints, carpet & flooring systems
  - b. Confirmation of "Urea-Formaldehyde Free" for Agrifiber products: [use LEED Material Spreadsheet]
9. Chain of Custody Number for all FSC Wood Products: [use LEED Material Spreadsheet]

Thank you,

---

[Insert your company logo]  
[Sender Name]  
[Sender Title]  
[Sender Company Name]  
[Date signed]

# TTC - LEED Materials Spreadsheet



Trade Group No.: \_\_\_\_\_

Subcontractor Name: \_\_\_\_\_

Total Contract Value: \_\_\_\_\_

Progress Report Date: \_\_\_\_\_

[illegible]

# Exhibit F – BIM Requirements for Subcontractors

## Transbay Transit Center



Webcor/Obayashi Joint Venture

### *I. Introduction*

Webcor/Obayashi Joint Venture is implementing a virtual building process for this project. This process will include building a digital, three-dimensional Building Information Model (BIM) linked to a project cost and labor productivity database, which will provide a platform for collaboration throughout the project's construction. In implementing this virtual building process as further outlined below, the Subcontractors will have the ability to analyze different construction sequences and methods for construction. In turn, the Subcontractor's provision of accurate virtual building data facilitates analysis and mitigation of potential costs and scheduling impacts.

The participants will adhere to the following guidelines in connection with this virtual building process. The costs of all management, administration, software, modeling, drafting, transmission, submittal, meetings, etc. for this process shall be the responsibility of the subcontractor and are included in this Subcontract.

### *II. 3D Modeling Requirements*

Subcontractors will be a part of a team that will meet at least bi-weekly, but not more than twice weekly, for coordination meetings to model the building and its systems, coordinate the work, and build the project virtually. The objectives of these meetings include the elimination of as many conflicts and clashes as possible and the development of reliable schedules that allow for efficient workflow and effective production control. The coordination meetings will occur in multiple phases and as described for Target Schedule Development (TSD) in Subsection IV, Item a, below, but shall precede the TSD at each listed phase.

Webcor/Obayashi Joint Venture will manage and lead the coordination process and assist the subcontractors in bringing the individual models together, running clash detection reports, and generally coordinating the process. The individual participants will be partners in this process, model their work, coordinate this with other trades and building components, obtain submittal approvals from the architect and engineers of record, and relocate/modify their systems as necessary when conflicts arise.

The 3D model consists of geometry control models generated and provided by Webcor/Obayashi Joint Venture and/or the Owner's design team from the 3D Database and system models generated and provided by the Subcontractors for their respective scopes of work. The system models, when integrated with the geometry control models, are referred to as the "Federated Model."

The Subcontractor's system models are the Subcontractor's sole responsibility. Prior to commencing any modeling, the Subcontractor must coordinate the initial model orientation with Webcor/Obayashi Joint Venture. All information in the system models shall be consistent with and based on the Contract Documents. The system model shall be maintained throughout the duration of the Project and updated to reflect as-built



# Exhibit F – BIM Requirements for Subcontractors

## Transbay Transit Center



Webcor/Obayashi Joint Venture

conditions. The degree of detail and accuracy of the Subcontractor's system models shall be sufficient to enable accurate and complete clash detection as well as shop drawing extraction. Subcontractors will be required to print their shop drawings directly from the Federated Model, including dimensions, elevations and location of specific trade elements, based off of the building grid and/or coordinates. The printed material shall comply with the submittal requirements noted elsewhere in the contract documents. Lastly, to allow for model quantity extraction for cost and schedule information, the Subcontractor shall coordinate breakdown and classification of systems in the systems model with Webcor/Obayashi Joint Venture.

### **B...**

The Subcontractor's system models shall be fully compatible with **Autodesk BIM 360 Glue**, Autodesk Revit, Tekla, or Graphisoft ArchiCAD in the version contemporaneously current with Subcontractor's initial submission of its system model, or the version immediately preceding the contemporaneously current version. It shall be the responsibility of the Subcontractor to maintain this compatibility at its own expense. If more trade specific software is required for a particular system model, Subcontractor must obtain Webcor/Obayashi Joint Venture's prior consent to utilize such software.

### **...B**

The Subcontractor shall transmit its system model to Webcor/Obayashi Joint Venture's BIM Coordinator who will manage the coordination process. The Subcontractor shall be required to perform clash detections and identify conflicts which shall be communicated to the Project team in a discrepancy report. Subcontractor shall review the identified conflicts as set forth in the discrepancy report and jointly develop conflict solutions and modify their system models accordingly. Trade coordination and model modification shall at all times remain a responsibility of each Subcontractor.

#### a. Modeling Schedule

Webcor/Obayashi Joint Venture will develop a Modeling Schedule showing modeling and coordination efforts required by all subcontractors in order to meet the construction and installation performance shown in the Exhibit I Project construction schedule. Subcontractor will be required to maintain its performance to meet the dates shown in the Modeling Schedule Subcontractor shall ensure that it provides adequate modeling and coordination manpower to maintain the modeling/coordination schedule.

#### b. Modeling Coordination Meetings

Subcontractor shall participate in BIM coordination and review meetings with Webcor/Obayashi Joint Venture. Subcontractors can expect these meetings to occur at least weekly or biweekly depending on the projects schedule needs. As a result of the information exchanged at such meetings, both the system model and the Work depicted

# Exhibit F – BIM Requirements for Subcontractors

## Transbay Transit Center



Webcor/Obayashi Joint Venture

in the Subcontractor's system model may be required to be changed by the Subcontractor to achieve coordination with other elements of the Project being provided by others. In accordance with General Conditions subsection 1.03.G, Subcontractor will be compensated for the associated BIM coordination efforts under the provisions for Change Orders of Article 6. Subcontractor acknowledges that BIM coordination and review meetings will require attendance of personnel that are familiar with both the data entry aspects of the BIM as well as an understanding of the Work to be performed and its relation to other elements of the Project, and the Subcontractor therefore agrees that personnel conversant in both shall attend all such meetings.

**B...**

### **III.** ~~**DELETED**~~ *Cost and Productivity Data*

~~In addition to the 3-D modeling requirements set forth above, all Subcontractors shall provide accurate cost and productivity information to be imported into a project data base in order to facilitate 4-D (time dimension) and 5-D (cost dimension) modeling. This information shall be broken down such that line items describe work activities for each building system included in Subcontractor's scope of work.~~

- ~~a. Scheduling Information. Subcontractor shall provide detailed scheduling information including, but not limited to, the following:
  - ~~i. Provide a list of tasks which identify continuous activities that can be performed with other trades.~~
  - ~~ii. Provide a list of predecessor tasks for each above defined task that needs to be complete before Subcontractor can start the subject task.~~
  - ~~iii. Provide a list of preferred minimum work areas breakdown. This breakdown shall be based on the minimum work areas that will be necessary for the Subcontractor to work efficiently.~~
  - ~~iv. Provide task and specific location based activity assignments for each item in Subcontractor Submittal Register when submitted in accordance with Exhibit F, Item III.A.12.~~
  - ~~v. All information noted within this Item 'a.' shall be provided within 15 days of Subcontract award.~~~~

**...B**

**A**

### **b.** ~~**DELETED**~~

**A**

### **IV.** *Project Scheduling and Production Control Phase Planning*

Project Schedule, is a critical path method (CPM) schedule that shows the initial plan to construct the project. This schedule sets forth certain dates for performance and a

# Exhibit F – BIM Requirements for Subcontractors

## Transbay Transit Center



Webcor/Obayashi Joint Venture

general sequence of construction that is subject to change based on project requirements and as set forth in Section G of the Instructions to Bidders. Because the BIM requirements contained in this exhibit provide an opportunity to develop a schedule that is optimized for subcontractor performance efficiencies, it is the intent of the Project Team to employ production control techniques to manage the day-to-day construction of the Project. This process will proceed generally in the following manner and is affirmatively acknowledged by Subcontractor as follows:

- a. Subcontractor agrees to participate in phased Target Schedule Development (TSD) at completion of each of the following stages of Consolidated Model Development:
  - i. TSD#1 – Foundation & Substructure
  - ii. TSD#2 – Superstructure & Exterior Envelope
  - iii. TSD#3 – Mechanical, Electrical, Plumbing, Sprinkler (Fire) [MEPS]
  - iv. TSD#4 – Interior Finishes
  - v. TSD#5 – Commissioning
- b. Phased Target Schedule Development (TSD) requirements:
  - i. Subcontractor agrees that durations for Subcontractor's tasks at each location will be calculated based on quantities at each location divided by the Subcontractor's crew production rate
  - ii. Subcontractor agrees to assist with optimization of the overall performance schedule for all trades, working from visualization(s) of labor flow using a Flowline chart (a modified Line of Balance schedule), to:
    - i. Balance the number of crews to improve flow
    - ii. Remove labor or material spikes to increase manageability and reduce site conflicts
  - iii. Use risk analysis to determine buffer placement points and durations required to minimize risk
- c. Subcontractor agrees to participate in Mid-Phase Re-optimization Development at least one (1) additional time following each of the TSD for phases of Consolidated Model Development described in Item A, above:
  - i. Mid-Phase Re-optimization Development (MRD) requirements:
    - i. Subcontractor agrees that durations for Subcontractor's tasks at each location will be calculated based on quantities at each location divided by the Subcontractor's crew production rate.
    - ii. Subcontractor agrees to assist with optimization of the overall performance schedule for all trades, working from visualization(s) of labor flow using a Flowline chart (a modified Line of Balance schedule), to enable the following:
      - i. Balance the number of crews to improve flow
      - ii. Remove labor or material spikes to increase manageability and reduce site conflicts
    - iii. Use risk analysis to determine buffer placement points and durations required to minimize risk

# Exhibit F – BIM Requirements for Subcontractors

## Transbay Transit Center



Webcor/Obayashi Joint Venture

### V. *Miscellaneous Provisions*

- a. Model Ownership: In accordance with Article 2, subsection 2.07A, BIM files, and other computer files created for the Project shall be made and remain the property of the TJPA, including all intellectual property rights to all documents or materials.
- b. Protection of Intellectual Property or Proprietary Information: Subcontractors who provide intellectual property and/or proprietary information which is incorporated into their models shall provide notification of the confidentiality of the information.
- c. Other Subcontract Requirements: Subcontractor agrees that neither the BIM nor the use of the BIM is in lieu of nor intended to relieve the Subcontractor of its responsibilities under the Subcontract, including, without limitation, to (i) coordinate its Work with the work of others involved in the Project and (ii) strictly comply with the other requirements of the Subcontract Agreement and the Contract Documents. It is expressly understood and agreed that, notwithstanding the requirement for submittals in connection with the BIM, other submissions shall be required of Subcontractor as required by the Contract Documents.
- d. BIM Liability: Subcontractor acknowledges and agrees that the TJPA and Webcor/Obayashi Joint Venture shall incur no responsibility or liability with respect to the BIM or the use thereof, including that resulting from errors, omissions or deficiencies in the BIM. In the event that Subcontractor provides deficient information or data that does not represent the Work it will ultimately be providing, or that is corrupted in that the information transmitted contains a virus, and/or that otherwise damages the BIM, Subcontractor shall bear all costs associated with reconstructing the BIM and to otherwise remediate such deficiencies and their effects. In the event Subcontractor discovers an apparent error, inconsistency or omission in its information or submissions it shall notify Webcor/Obayashi Joint Venture within 72 hours and via written correspondence. In the event Subcontractor discovers an apparent error, inconsistency or omission in the information or submissions provided by others Subcontractor shall promptly request clarification of the same from Webcor/Obayashi Joint Venture, with a written Request for Information per General Conditions Article 6.03.

### VI. *Modeling Specification*

#### **B...**

- a. The goal, through 3D coordination, is to create fully coordinated shop drawings derived from using the Models produced and coordinated by each discipline. ~~These models would then be used for scheduling (4D) and cost management (5D) purposes.~~ This section describes the **Degree of Detail (DOD)** to which

# Exhibit F – BIM Requirements for Subcontractors

## Transbay Transit Center



Webcor/Obayashi Joint Venture

each system will be modeled and whether the system should fall under the standard or high level category.

**...B**

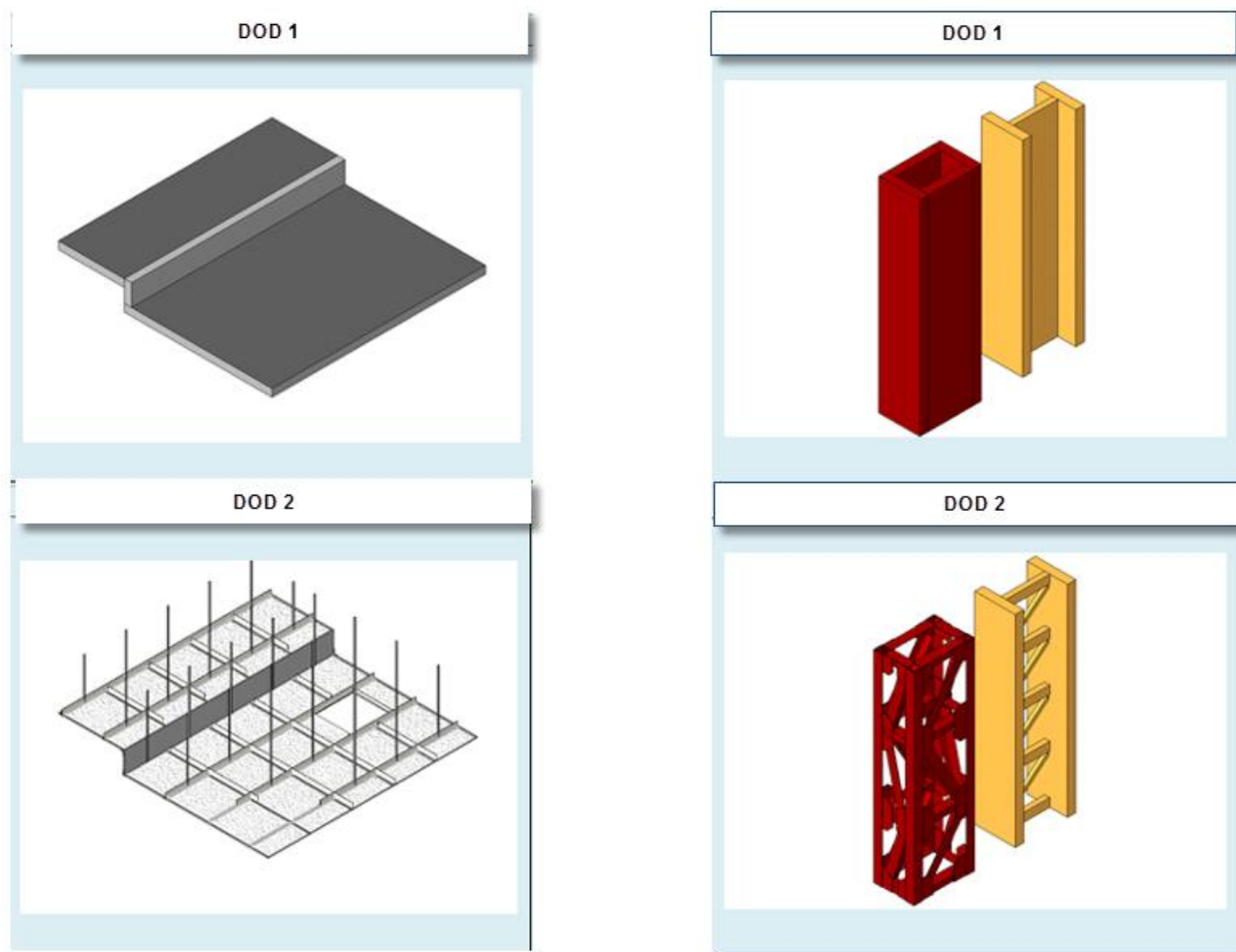
- i. DOD 1 indicates standard degree of detail where elements match the approximate space and shape the element occupies or the space required to access equipment for maintenance. Accurate geometry of components with rectangular cross sectional geometry. Components of complex cross sectional geometry are approximated with simplified cross sections and modeled with accurate enveloping geometry. Composite structures are modeled with solids. Symbolic representation of fixtures, equipment, furniture and like.
- ii. DOD 2 indicates a high degree of detail dimensionally accurate, and where applicable, manufacturer specific element (does not require manufacturing/fabrication detail – exterior envelope is required) Accurate geometry of components with rectangular and complex cross sectional geometry. The individual layers of composites are broken down to smaller components and built up piece by piece. Exact representation of fixtures, equipment, furniture and like. The model will include secondary components that may influence coordination, such as gusset plates, secondary steel members, hangers, braces etc.

# Exhibit F – BIM Requirements for Subcontractors

## Transbay Transit Center



Webcor/Obayashi Joint Venture



Please refer to the Trade Specific BIM Requirements below for the list of systems that are required to be modeled.

### *VII. Change Management*

Subcontractor will maintain its system model throughout the project duration, incorporating all changes that impact its Work. Subcontractor will update its Work as required through participation in the 3D coordination process outlined above. Subcontractor may be required to re-extract shop drawings and prepare updated submittals to incorporate changes to its Work.

After each model update for a change package (e.g. ASI), Subcontractor shall archive a copy of its model before incorporation of further changes. Using versions of its system model, Subcontractor shall publish quantity deltas per system between models. Subcontractor will apply its bought out unit rate for a particular changed system to the quantity delta to calculate

# Exhibit F – BIM Requirements for Subcontractors

## Transbay Transit Center



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### Webcor/Obayashi Joint Venture

the value of the change per the original contract. This value will serve as a baseline to enable change orders negotiations.

Exhibit F - Trade Specific BIM Requirements

Trade	System	As-Built Delivery DOD
TG16.0 Interiors/Finishes	Masonry	1
	Bracing	1
	Drywall/Framing	2
	Toilet Partitions	2
	Acoustic Ceiling Tiles, Support, and Hangers	2
	Access Flooring and Supports	2
	Support/Bracing/Embeds	1
	W-21 Terra Cotta Cladding, Framing, and Support	2
	W-18 Cement Plaster, Framing, and Support	2





## Exhibit G

### SUBCONTRACTOR PAYMENT REQUISITION

#### 1. Forms Checklist

#### 2. Forms

Form Number	Form Title
i. 1030	Subcontractor Progress Billing Invoice
ii. 1030A	Schedule of Values
iii. 1031	Subcontractor Final Retention Invoice
iv. 1031A	Schedule of Value Retention Release
v. 00 08 21/AT3-E (modified)	Progress Payment Report (With Additional SBE Columns)

## Forms Checklist

\*\*This checklist is provided as a reference, but may not be a complete list. Refer to the Contract Documents for all required submissions and their frequency.

#	FORMS	FORM	FREQ	REF
1	<b>CityBuild Workforce Projection Form 1 and 2</b> - Non-compliance results in removal from site	00 08 20/AT1 00 08 20/AT2	Initial	Div 00 08 20 1.7
2	<b>Schedule of Values</b>	1030A	Initial / Monthly	Exhibit G
3	<b>Daily Report</b> (must be CURRENT at the time of pay app submission and payment)		Daily / Monthly	Bid Manual IV. A. 4. c.
4	<b>Subcontract Progress Billing Invoice</b>	1030	Monthly	Exhibit G
5	<b>Conditional Waiver and Release Upon Progress Payment</b>	1034	Monthly	Exhibit C
6	<b>Unconditional Waiver and Release Upon Progress Payment</b>	1035	Monthly	Exhibit C
7	<b>TJPA ARRA Jobs Report Form</b>	v 1.2	Monthly	Div 00 08 13, 1.2.E & APF
8	<b>Manpower Projection</b>		Monthly	Bid Manual IV. A. 38. a.
9	<b>Billing Projection / Cashflow Projection</b>		Monthly	Bid Manual IV. A. 37. a.
10	<b>TJPA Progress Payment Report</b>	00 08 21/AT3-D	Monthly	Div 00 08 21, 1.5.B
11	<b>Subcontractor Payment Declaration</b>	00 08 21/AT3-E	Monthly	Div 00 08 21, 1.5.C
12	<b>Project Specific Insurance</b> (Must be CURRENT)		Monthly	Long Form Subcontract 16
13	<b>Certified Payroll</b> , weekly electronic submission (CURRENT at the time of pay app submission and payment) including subtiers		Weekly / Monthly	Long Form Subcontract 4.2
14	<b>Apprentice Training Fund Contributions</b> proof of payment	a) Copy of trust fund remittance report w/ copy of cancelled check OR b) DAS Form CAC 2 w/ copy of cancelled check	Monthly	Bid Manual II. F. 6. c. & Long Form Subcontract 4.2 & Div 00 08 22 1.2 D.
15	<b>Apprenticeship min/max ratio verification</b> - if under, submit a plan to satisfy requirement by the end of the project without exceeding daily max; if over, provide written explanation for each day of violation		Monthly	Bid Manual
16	<b>Apprenticeship Monthly Trade Subcontractor Affidavit</b>		Monthly	Bid Manual, Exhibit Q
17	<b>Request for Dispatch of an Apprentice (DAS 142 Form)</b> - if any	DAS 142	Monthly	Bid Manual
18	<b>Apprentice documentation</b> - documentation on employed apprentices that are current and properly registered as required by specs		Monthly	Div 00 08 13/APA, Section 23 (d) (1)
19	<b>EIC Form</b> from eligible subcontractor employees		Yearly	Div 00 08 22 1.9 C (all of 1.9)
20	<b>LEED - NC Version 3.0</b> (monthly summaries and deliverables)		Monthly	Bid Manual IV. A. 40. a. and Div 01 81 13 1.5 D.1-4
21	Reconciled Excel submittal form with Trade Package Progress Schedule ( 2 times a month) - NOTE: In Div 01 our updated schedule must be submitted in our Progress Payment Request, see 01 13 10 1.5 E.		Monthly	Bid Manual IV. A. 35. f. and C.1.J
22	<b>Weekly Safety "Tool Box" Meeting Minutes</b> (must be CURRENT at the time of pay app submission and payment)		Weekly / Monthly	Bid Manual IV. B.
23	<b>JHA Reports</b> (Job Hazard Analysis Reports) (must be CURRENT at the time of pay app submission and payment)	H4	Monthly	Bid Manual IV. B.
24	<b>Monthly Disposal and Recycling Summary Report</b> (Waste Management Requirements) (Contractor) <b>CONSTRUCTION AND DEMO DEBRIS RECOVERY MONTHLY SUMMARY REPORT</b>	00 08 15 / APA - 1 and 00 08 15 / APA - 12	Monthly	Div 00 08 15 1.5 C 1 and 2
25	monthly with Pay App		Monthly	Div 01 74 00 1.8 A. B.
26	<b>DBE Trucking Verification</b> , due at end of month, need amount paid by DBE Trucking companies to all firms, including owner-operators, for leasing of trucks - DUE TO TJPA by Contractor on the 15th of the month to TJPA	Monthly DBE Trucking Verification Form	Monthly	Div 00 08 21/AT2 5 b. i. and ii.
27	<b>Up to date As-builts drawings</b> on site at all times		Monthly	Bid Manual IV. K. 1. a.
28	<b>Stored Materials Documentation</b>		Monthly	Div 00 07 00, 1.4.I
29	<b>Daily Sign In and Out Sheet</b> (must be CURRENT at the time of pay app submission and payment)	TJPA Daily Sign-in Sheet	Daily / Monthly	Div 00 07 00 57, Article 11, 11.04
30	<b>Daily Quality Control Reports</b> (must be CURRENT at time of pay app submission and payment)		Daily	Dic 00 14 00 1.12 and Exhibit J
31	<b>Trade Package Progress Schedule</b> update in electronic format (must be CURRENT at the time of pay app submission and payment)		Monthly	
32	<b>LEED Progress Reporting</b> with each pay app		Monthly	
33	<b>Updated Bidders / Proposers Information Request Form</b> - must be submitted whenever subcontractor information is updated, regardless of SBE participation	00 08 21/AT3-B	As-needed	Div 00 08 21 1.3E
34	<b>Conditional Waiver and Release Upon Progress Payment</b> - subtiers and vendors	1034	Final	Exhibit C
35	<b>Unconditional Waiver and Release Upon Progress Payment</b> - subtiers and vendors	1035	Final	Exhibit C
36	<b>Subcontractor Final Retention Invoice</b>	1031	Final	Exhibit G
37	<b>Schedule of Values Retention Release</b>	1031A	Final	Exhibit G
38	<b>Conditional Waiver and Release Upon Final Payment</b>	1036	Final	Exhibit C
39	<b>Unconditional Waiver and Release Upon Final Payment</b>	1037	Final	Exhibit C
40	<b>Conditional Waiver and Release Upon Final Payment</b> - subtiers and vendors	1036	Final	Exhibit C
41	<b>Unconditional Waiver and Release Upon Final Payment</b> - subtiers and vendors	1037	Final	Exhibit C
42	<b>Final</b> weekly electronic submission of <b>Certified Payroll</b> (must be CURRENT at the time of pay app submission and payment) including subtiers		Final	Long Form Subcontract 4.2
43	One compact disk containing electronic files in .dwg format and pdf format and three (3) sets of accurate and complete As-built drawings - Complete As-builts are due upon completion. - prior to requesting final payment		Final	Bid Manual IV. K. 1. e and f.
44	<b>Operations and Maintenance Manuals</b> shall be submitted 12 months prior to start of commissioning and prior to requesting final payment		Final	Bid Manual IV. K. 1. f.
45	Evidence of final payment to Unions and Union Trust Funds, State Apprenticeship Programs (subs who are not signatory to unions)		Final	Long Form Subcontract 4.2

## Forms Checklist

\*\*This checklist is provided as a reference, but may not be a complete list. Refer to the Contract Documents for all required submissions and their frequency.

#	FORMS	FORM	FREQ	REF
	<b>Apprenticeship Trade Subcontractor Affidavit</b> - that the required number of apprentices were employed and/or records showing that the apprenticeship committee(s) either denied or failed to respond to a request for the dispatch of apprentices in accordance with Labor Code Section 1777.5		Final	Bid Manual, Exhibit Q
46			Final	Div 01 17 00 1.4 A 3. b.
47	<b>Warranties</b> must be submitted prior to requesting final payment		Final	Div 01 17 00 1.4 A 3. d.
48	Spare Parts and material extra stock		Final	Div 01 74 00 1.8 D.
49	<b>Final (Contractor) CONSTRUCTION AND DEMO DEBRIS RECOVERY SUMMARY REPORT</b>		Final	Bid Manual IV. A. 40. a. and Div 01 81 13 1.5 D.1-4
50	<b>Final LEED Final Reports and Documentation</b>		Final	
51	<b>Final Disposal and Recycling Summary Report</b> (Waste Management Requirements)	00 08 15 / APA - 1 and 00 08 15 / APA - 12	Final	Div 00 08 15 1.5 C 1 and 2



## Subcontractor Progress Billing Invoice

Send invoice to:

**EMAIL:** ap@webcor.com

**FAX:** (510) 748-3474

**MAIL:** 1751 Harbor Bay Parkway, Suite 200 Alameda, CA 94502

### Billing Information

Owner Pay App NO. \_\_\_\_\_

Vendor Number \_\_\_\_\_

Webcor/Obayashi Joint Venture  
Subcontract Number: \_\_\_\_\_

Webcor/Obayashi Joint Venture  
Job Number: 30100.XX

Job Name: Transbay Transit Center

**Pay App Number:** \_\_\_\_\_

**Invoice Number:** \_\_\_\_\_

**Invoice Date:** \_\_\_\_\_

**Sub Job Number:** \_\_\_\_\_

**Period From:** \_\_\_\_\_

**Period To:** \_\_\_\_\_

### Subcontractor Contact Information

Subcontractor Name: \_\_\_\_\_

Remittance Address: \_\_\_\_\_

City, State, Zip: \_\_\_\_\_

Contact Name: \_\_\_\_\_

Contact Email Address: \_\_\_\_\_

Contact Phone Number: \_\_\_\_\_

Contact Fax Number \_\_\_\_\_

Print Signer's Name and  
Title: \_\_\_\_\_

**Signature** \_\_\_\_\_

**Date Signed** \_\_\_\_\_

The following invoice covers work completed through the last day of

Original Contract Amount:	\$0.00
---------------------------	--------

Executed Change Orders (CO) though CO No:	\$0.00
---	--------

Total Revised Contract Amount:	\$0.00
--------------------------------	--------

Gross Amount Complete to Date %	\$0.00
---------------------------------	--------

Less Gross Amount Previously Invoiced:	\$0.00
--	--------

Current Gross Billing Amount:	\$0.00
-------------------------------	--------

Less Current Retention:	\$0.00
-------------------------	--------

Current Net Amount:	\$0.00
---------------------	--------

Webcor/Obayashi Joint Venture Approvals below this line

Schedule of Values

Sub:  
Sub No.:

Sub Application  
Number:  
Invoice Date:  
Webcor/Obayashi Joint Venture Job No: 30100.XX

Transbay Transit Center

Period From:  
Period To:

In tabulations below, amounts are stated to nearest dollar

Item No.	A		B	C	D	E		F	G	H	I	J
	CSI Division	Spec Section				Work Completed This Application In Place	Work Completed This Application Stored					
1												
2												
3												
4												
5												
6												
7												
8												
			Sub Total									
PCO #	CSI Division	SCO No.	Approved Change Orders									
			Total Change Orders									
			Grand Total									



## Subcontractor Final Retention Invoice

Send Invoice to:

EMAIL: ap@webcor.com

FAX: (510) 748-3474

MAIL: 1751 Harbor Bay Parkway, Suite 200 Alameda, CA 94502

### Billing Information

Vendor Number  
(W/O JV Use Only)

Invoice Number:

RETENTION:

Invoice Date:

Webcor/Obayashi JV

Subcontract Number:

Webcor/Obayashi JV

Job Number:

30100.XX

Job Name:

**Transbay Transit Center**

### Subcontractor Contact Information

Subcontractor Name:

Remittance Address:

City, State, Zip:

Contact Name:

Contact Email

Address:

Contact Phone

Number:

Contact Fax Number

Print Signer's Name

and Title:

Signature & Date

Date Signed

The following invoice covers work completed through the last date of \_\_\_\_\_ (Month), \_\_\_\_\_ (Year):

Contract Amount:

\$

-

Executed Change Orders Through Change Order NO: \_\_\_\_\_

\$

-

Total Revised Contract Amount:

\$

-

Gross Amount Complete to Date % (\_\_\_\_\_ %)

\$

-

Less: Total Net Amount Previously Billed:

\$

-

Total Amount Due:

\$

-

\*\*\*\*\*  
*For Webcor /Obayashi JV Use only*  
\*\*\*\*\*

Schedule of Values Retention Release

Sub:  
Sub No.:

Sub Application  
Number:  
Invoice Date:  
Webcor/Obayashi Joint Venture Job No: 30100.XX

Transbay Transit Center

Period From:  
Period To:

In tabulations below, amounts are stated to nearest dollar

Item No.	A		B	C	D	E		F		G	H	I	J
	CSI Division	Spec Section				Work Completed This Application In Place	Work Completed This Application Stored						
1			Description of Work	Scheduled Value	Previous Application					Total To Date (C+D+E)	% (F/B)	Balance To Finish (B-F)	Retention To Date
2													
3													
4													
5													
6													
7													
8													
			Sub Total										
PCO #	CSI Division	SCO No.	Approved Change Orders										
			Total Change Orders										
			Grand Total										

TRANSBAY JOINT POWERS AUTHORITY  
PROGRESS PAYMENT REPORT  
(WITH ADDITIONAL SBE COLUMNS)

To be completed by Trade Subcontractor and submitted to Project Manager with every monthly invoice.

PART 1: PROJECT SUMMARY

Contract Award Date:		TJPA Contract No.:		Contract Title:	
Trade Subcontractor:		Contact Person:	Contact Phone No.:	Contact Email:	
Trade Subcontractor Address		Signature:			
Invoice Date:		Invoice No.:	For the Period:		

1. Award amount of Trade Subcontract	\$	-
2. Amount of Change Orders, Amendments and Modifications to Date	\$	-
3. Total Contract Amount to Date including Change Orders, Amendments and Modifications (Line 1 + Line 2)	\$	-
4. Total Amount for this Invoice (Less Retention)	\$	-
5. Total Previously Invoiced Awaiting Payment (Less Retention)	\$	-
6. Total Amount Paid to Date (not including Lines 4 and 5)	\$	-
7. Total Invoice Amount Requested to Date (Line 4 + Line 5 + Line 6)	\$	-
8. Total Retention to Date <sup>1</sup>	\$	-
9. Percent Complete ((Line 7 + Line 8) / Line 3)		0%



**PART 2: CONSULTANT/SUBCONSULTANT PAYMENT DETAIL SUMMARY**

<sup>1</sup> As retention is requested and paid, move out of "Total Retention to Date" and into "Amount Paid to Date"

<sup>3</sup> If SBE participation is Other SBE, SBE Joint Venture Partner or SBE Trucking Company enter lump sum participation in column N in lieu of column M (Refer to TIPA Policy No. 015 Section IV)

<sup>4</sup> If SBE Firm has multiple participation types each type should be listed as separate line item



# **TRANSBAY TRANSIT CENTER**

Site Specific Safety Program  
Revision 9

**July 10, 2014**

**WEBCOR/OBAYASHI JOINT VENTURE  
SAN FRANCISCO, CA**

## **EXHIBIT H**

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## Webcor/Obayashi Joint Venture Statement on Safety

It is the policy of Webcor/Obayashi Joint Venture to provide employees a safe place to work. The personal safety and health of each employee of this company is of prime importance. The prevention of accidents and injury will be given precedence over operating productivity whenever necessary. To the greatest degree possible, management will provide facilities required for personal safety and health.

Our objective is a program that will reduce the number of injuries to a minimum and to surpass the best experience of other operations similar to ours. Our goal is zero accidents and injuries.

Our policy will be implemented as follows:

- Management will continue to develop policies and procedures that will assist in the control of personal injury, property damage and losses and fleet damage. Direct and indirect costs associated with these types of losses contribute unfavorably to operating expenses. These policies and procedures will be reviewed and updated as needed.
- Safety is the direct responsibility of all personnel. Safety is of prime importance to production and quality. Everyone has the right to stop work to address safety concerns.
- Safety on the job in all company facilities and job sites is a priority. In no instance will safety become secondary to any other considerations. Any recognized safety activity or hazard will be corrected.
- It is mandatory that all personnel engaged in work on this project comply with all federal, state and local safety codes and regulations throughout the duration of their construction on this project.
- Each site will have a Supervisor available to support the safety effort.
- Each Supervisor will be assigned various levels of safety responsibility and authority.
- All employees will be held accountable for the safety policy.
- An established system of communication, measurement, and documentation exists throughout the company.
- A Safety Committee is in place to formulate and update the company safety program and policies. This committee operates under the supervision of management.

# Health and Safety Communication

This Webcor/Obayashi Joint Venture project plan will be developed incrementally as trade packages are awarded and trade subcontractors are brought on board. Each trade subcontractors plan will become part of Webcor /Obayashi's overall project plan and will be submitted to the Transbay Joint Powers Authority (TJPA) as they are received.

## Orientation

The Webcor/Obayashi Joint Venture training will contain required elements stipulated by Webcor/Obayashi Joint Venture [Code of Safe Conduct and Work Practices](#).

Webcor/Obayashi Joint Venture and ClickSafety have partnered to create a web-based Contractor Safety orientation course for the Transbay Transit Center. All contractors requiring access to the Transbay Transit Center project must successfully complete the three (3) required sessions online through ClickSafety prior to working on site. This site-specific safety orientation will take approximately one (1) hour to complete the three (3) sessions:

- Webcor/Obayashi Safety Passport
- Webcor/Obayashi Click Green Construction Practice
- Webcor/Obayashi Transbay Transit Center Project

The three sessions' includes a discussion on site protocol, evacuation procedures, a description of the logistics of the site, safety expectations and requirements that employees are expected to understand and comply with while working on the premises. These sessions are available in both English and Spanish.

Subcontractors are required to provide other task specific orientations as needed.

## ClickSafety - Project Fees

The fee structure for ClickSafety services is a \*\$100 annual fee per user.

*\*Prorate will apply to those that begin the training after the first quarter of the current year.*

The prorate schedule is as follows:

January – June	\$100	Valid January – December
July – December	\$50	Valid July 1 – December

## ClickSafety – Account Setup

These steps are to assist Contractors in setting up their account, user registration and implementation of ClickSafety.

1. Access ClickSafety's Transbay Safety Passport home page at <http://www.clicksafety.com/safetypassport-transbay/>
2. Create a company account. Click on the *Company* tap, then on *Register Company*, follow the prompts
  - a. If your Company already have an account, your Company will still need to register your existing account for this project
3. Assign the three sessions:
  - a. Webcor/Obayashi Safety Passport
  - b. Webcor/Obayashi Click Green Construction Practice
  - c. Webcor/Obayashi Transbay Transit Center Project



4. Prepay for employee training with a credit card and create an access code
  - a. Keep this access code available as your employees will be required to enter it when they register
5. Direct all employees to ClickSafety's home page to conduct their on-line orientation
6. Employee Registration:
  - a. Click on the *User* tab
  - b. Then on *Register For Training* tab
  - c. Select *Webcor/Obayashi TransBay Terminal* from the drop down menu
  - d. Enter first name, last name, last 4 digits the employees social security number (SSN)
    - i. Employees user name will be the first letter of their first name and their full last name, their password is the last 4 of their SSN
  - e. Select preferred language to receive training in
  - f. Select your Companies name from the drop down menu
  - g. Enter access code
  - h. Continue
  - i. The three sessions will appear in the employees screen. Please ensure all employees complete each session

### **ClickSafety - Contact**

A ClickSafety representative is available to answer any of your questions about this program. For general information about this project or registration assistance, please contact ClickSafety Support at (925)855-SAFE (7233) ext. 629 or [cshep@clicksafety.com](mailto:cshep@clicksafety.com). ClickSafety's Account Manager is Christina Parkin, (925)208-2618, Email: [cparkin@clicksafety.com](mailto:cparkin@clicksafety.com).

Should you have specific questions regarding the project or safety requirements, you may contact Webcor Builders Administrative Assistance for the EHS Department Kyla Burke at (510)748-1994 or at [kburke@webcor.com](mailto:kburke@webcor.com).

### **ClickSafety - Disclaimer**

ClickSafety and Webcor/Obayashi Joint Venture make this training material available with the understanding that users exercise their own skill and care with respect to its use. It is the duty of each employer as specified in the Occupational Safety and Health Act of 1970 (P.L. 91-596)

- (a1) Shall furnish to each of his employees employment and a place of employment which are free from recognized hazards that are causing or are likely to cause death or serious physical harm to his employees;
- (a2) shall comply with occupational and health standards promulgated under this Act.
- (b) Each employee shall comply with occupational safety and health standards and all rules, regulations, and orders issued pursuant to this Act which are applicable to his own actions and conduct.

## **Emergency Response Procedures**

Webcor/Obayashi Joint Venture provides a safe and healthful work environment for all workers through progressive, proactive injury prevention planning. Job pre-planning and identification of up-coming potentially hazardous activities is supported by regularly reviewing trend analysis. Everyone on site has a

responsibility for their own safety and the safety of their work environment. If an activity is deemed unsafe workers have several ways to communicate these activities to management. Workers shall always contact their immediate supervisor and Webcor/Obayashi Joint Venture SSM if something is unsafe or an incident occurs.

Prior to starting work on this project a designated area for emergency service vehicles to enter without any delay shall be established. A current, certified First Aid/CPR/AED trained individual must be on site during work operations. All employees shall be instructed in the proper chain of command for reporting emergencies. 9-1-1 may be called at any time for an emergency by anybody on site. Each trade subcontractor and tiered subcontractor shall maintain a Cal/OSHA approved First Aid Kit on the Project at all times. An investigation will be conducted by the controlling employer's Project Management, Supervisor and SSM/DSP, under the direction of Webcor/Obayashi Joint Venture Project Management and SSM.

Reporting and documenting all accidents, incidents and near misses, is extremely important to track trends and investigate possible root causes. All on-site incidents, accidents and near misses shall be reported to Webcor/Obayashi Joint Venture Project Management and SSM immediately. All accidents resulting in industrial injuries or illnesses occurring on the jobsite will be thoroughly investigated. Completion of appropriate forms, as defined in the [Incident Reporting Instruction](#) section must be completed and submitted immediately after occurrence. Depending on the severity of the incident a [Detailed Incident Analysis \(DIA\)](#) may take place.

The scene shall be left *as is* for investigation purposes as well as safeguarded to ensure the safety of other nearby workers until Webcor/Obayashi Joint Venture Management Team releases it. Identification and review process of root causes shall be completed. Corrective actions, identification of persons responsible for corrective actions, and date of completion must be established. Follow up documentation verifying corrective action completion is required. Lessons learned from the DIA reviews will be shared with the project.

OSHA and the National Safety Council (NSC) define the following:

“Accident - The National Safety Council defines an accident as an undesired event that results in personal injury or property damage.

Incident - An incident is an unplanned, undesired event that adversely affects completion of a task.

Near Miss - Near misses describe incidents where no property was damaged and no personal injury sustained, but where, given a slight shift in time or position, damage and/or injury easily could have occurred.”

([osha.gov](http://osha.gov))

## **Near Miss**

A near miss is an unplanned event that does not result in injury or property damage.

## **First Aid**

A first aid case is one where a person is injured requiring minor first aid treatment that does not require medical attention or prescription medication.

## Minor Injuries

Minor injuries are those which require only immediate first-aid treatment and do not result in modified work or lost work days.

## Major Injuries

A significant accident is where personal injury is sustained or tangible property loss is sustained, or where the event posed a significant threat of loss or personal injury. Major injuries or illness may be those which require extended medical treatment, hospitalization resulting in loss of work time, or result in death, disfigurement, or dismemberment.

In the event of a major injury, emergency vehicles shall be directed to enter the Project at a site entrance that will be determined as conditions change on the logistic map. Upon entering the project, the emergency personnel shall be directed to the exact location of the injured person/s. While awaiting arrival of the Emergency Vehicle(s), the injured shall not be moved unless he/she is in immediate danger of additional injury in his/her current location. Equipment and material involved in or responsible for the accident shall not be disturbed unless it presents an additional danger to the injured person(s).

Immediately after the accident, Webcor/Obayashi Joint Venture Management team will meet with the responsible trade subcontractor's Superintendent and/or Foremen, review the conditions, and direct the appropriate corrective action. The trade subcontractor is responsible for ensuring the injured employee/s are escorted to and from medical facilities, reporting employee/s condition to Webcor/Obayashi Joint Venture regularly and completing and submitting a copy of all required incident reports to Webcor/Obayashi Joint Venture SSM.

Persons who have sustained head injuries, major impacts, or whose injuries are the result of a fall shall be evaluated and stabilized by a professional medical personnel and provided transportation to the medical facility. Upon return from treatment, the employee shall return to work ONLY if so released in writing by the attending physician. If required by law, injury notification to OSHA must be coordinated through the Webcor/Obayashi Joint Venture Corporate Safety Director.

Within 24 hours of a major injury, Webcor/Obayashi Joint Venture shall conduct a Safety Meeting with attendance required of all jobsite personnel.

The recommended local Emergency Medical Facilities are:

**St. Francis Health Center**  
24 Willie Mays Plaza  
San Francisco, CA 94107-2134  
(415) 972-2249

**St. Francis Memorial Hospital**  
900 Hyde St  
San Francisco, CA 94109  
(415) 353-6000

**SF General Hospital**  
1001 Potrero Ave  
San Francisco, CA 94110  
(415) 206-8000

## Incident Reporting

*This Section will conform to Specification Sections 01 13 40 (1.5 A thru C) 01 15 45 (1.9 A thru C) found in The Transbay Transit Center Contract Number 08-04-CMGC-000*

A TJPA Representative will inform Contractors of any additional hazardous condition encountered in writing. Trade subcontractor shall respond indicating their action or disposition of the matter by returning

an annotated copy of the written communication to the TJPA Representative within three (3) days. If death, serious injury, multiple injuries or serious damages occur, the accident shall be reported at once by telephone or messenger to the TJPA as well as to the proper governing authorities. In addition, trade subcontractors shall promptly report in writing to the TJPA all accidents whatsoever arising out of or in connection with the performance of the work whether on or adjacent to the site, giving full details and statements of witnesses. Within three (3) days of occurrence, the trade subcontractor shall provide the TJPA with two (2) copies of the trade subcontractor's accident and near-miss reports.

If a claim is made by anyone against the any trade subcontractor on account of any accident, the trade subcontractor shall promptly report the facts in writing to the TJPA, giving full details of the claim. Contractor shall provide the TJPA Representative copies of any laboratory test data, and medical monitoring results for record and evaluation within three (3) days of receipt of the above information or upon the request of the TJPA Representative.

All incidents, accidents and near misses shall be immediately reported to Webcor/Obayashi Joint Venture Project Management/SSM and fully investigated. Investigation shall be completed to identify the possible contributing factors and the corrective actions. A DIA will be completed for major injuries, severe property damage and as needed per Webcor/Obayashi Joint Venture Management Team. Trade subcontractors shall complete required incident packages and return them to Webcor/Obayashi Joint Venture SSM within 24-hours.

### **Accident Investigation**

The initial accident investigation is to be completed within 24 hours, with immediate notification of Webcor/Obayashi Joint Venture safety. Identification and review process of contributing factors of the accident, incident or near miss must be completed. Corrective actions, identification of persons responsible for corrective actions, and date of completion must be established. Follow up documentation verifying corrective action completion is required. Lessons learned from a DIA may be shared with the project, regionally and globally.

### **Detailed Incident Analysis (DIA)**

To identify details in incidents, accidents, near misses and at-risk behavior Webcor/Obayashi Joint Venture and trade subcontractor management will be required to, within 48 hours of the incident, conduct a Detailed Incident Analysis (DIA). The DIA will analyze any accidents, incident, near misses, environmental incident, or impact to existing facilities and operations. Accident trends will be identified and plans developed to prevent additional incidents from occurring. The DIA will be performed involving at least the Webcor/Obayashi Joint Venture Manager and SSM and trade subcontractor project teams. The mission of these meetings will be to identify problem areas, develop specific action plan(s) to address contributing factors and to immediately implement corrective actions. Webcor/Obayashi Joint Venture will periodically review implemented plans for effectiveness. Lessons learned from the DIA will be shared with the project, regionally and globally.

## **Responsibilities for Safety & Loss Control**

The objective of this Project Safety Overview (PSO) is to establish that safety and health must be addressed throughout the entire project. The prevention of accidents and protection of property are company values and are integral to our success. All safety issues shall receive active support and participation by the entire project team.

The principles of safety and loss control are intended to prevent injuries on the jobsite and to reduce the potential for damage to property and equipment. No phase of construction is of greater importance than incident and accident prevention.

Planning for safety starts with project design and continues through purchasing, fabrication and construction in all phases of the project. Practical steps will be taken to maintain an injury free environment. All trade subcontractors must accept responsibility for preventing accidents and be responsible for thorough safety and loss control training and instruction for their workers.

The primary objective of the Webcor/Obayashi Joint Venture PSO is to coordinate the elimination or reduction of risk associated with the construction of the project. Associated missions are to promote safe work practices/behaviors, prevent accidents, prevent worker injuries, prevent damage to property, and promote maximum efficiency and effect savings by reducing unplanned business interruptions.

Active participation by Webcor/Obayashi Joint Venture management, trade subcontractors, tiered subcontractors and all workers will make the program effective and successful by coordinating the participants' efforts in performing the following tasks:

- Providing a safe environment in which workers can perform high quality work.
- Using [Job Hazard Analysis \(JHA\)](#) as a tool to reduce injury to persons and property.
- Conduct jobsite safety audits to locate and abate unsafe work practices/behaviors and unsafe conditions.
- Protecting the public and property potentially affected by Webcor/Obayashi Joint Venture sites.
- Educating and training workers through new hire and site specific orientation and safety meetings.
- Task specific safety training.
- [Personal Protective Equipment \(PPE\)](#) programs.
- Immediate injury reporting and effective record keeping to maintain an up-to-date accident experience and trends analysis.
- Use of audit forms to abate deficiencies and eliminate any additional losses.

## **Webcor/Obayashi Joint Venture Responsibilities**

### **Management Team**

Webcor/Obayashi Joint Venture Management Team is responsible for construction management services for the Transbay Transit Center. The Management Team is also responsible for encouraging, reinforcing and modeling Webcor/Obayashi Joint Venture culture, including injury free environment initiatives, participating in the development and assessment of Environmental Health and Safety (EHS) leading indicators, reviewing and approving project corrective action/recovery plans. Furthermore the Management Team shall institute accountability when action plans and culture are not maintained and has the authority to stop any operations that pose a potential threat.

### **Project Manager**

The Webcor/Obayashi Joint Venture Project Manager(s) are responsible for construction management services for the Transbay Transit Center as well as determining if contract documents and specifications support the project's safety missions and objectives. The Project Manager shall also monitor trade subcontractor selection process and adherence to established guidelines, conduct periodic auditing of trade subcontractor's safety plans for compliance with the Webcor/Obayashi Joint Venture 's Environment Health & Safety Procedures (EHSP), participating in pre-task planning and trade

subcontractor pre-construction safety meetings, document weekly jobsite safety audits and support Webcor/Obayashi Joint Venture SSM for obtaining corrective actions necessary to comply with Webcor/Obayashi Joint Venture EHSP. The Project Manager must be aware of loss control and public protection requirements of the project, they must participating in fact finding, Detailed Incident Analysis (DIA), and the implementation of corrective actions. Project Manager's shall promote and support our injury free culture.

### **Superintendent**

It is the responsibility of Webcor/Obayashi Joint Venture Superintendents are to oversee safety on the jobsite. The Superintendent's EHS responsibilities include overseeing the planning and execution of all work in compliance with the Webcor/Obayashi Joint Venture EHSP and contract specifications. The Superintendent needs to be aware of loss control and public protection requirements identified in the safety specifications of the contract documents, promote and support our injury free culture and support Webcor/Obayashi Joint Venture SSM in obtaining corrective actions necessary to comply with Webcor/Obayashi Joint Venture EHSP. Furthermore, the Superintendent shall complete and review daily jobsite safety audits to ensure identified hazards are addressed in a timely manner, monitor and participate in JHA planning and shall participate in incident investigation, DIA meetings, tailgate meetings, pre-construction meetings, kick off meetings and implementation of corrective actions. Superintendents must take appropriate action to abate identified unsafe conditions and practices and document corrective actions.

### **Site Safety Manager**

The Webcor/Obayashi Joint Venture Project Site Safety Managers (SSM) has a responsibility for the safety and health on the project. The Webcor/Obayashi Joint Venture SSM is considered to be the program administrator and has the authority delegated by Webcor/Obayashi Joint Venture Corporate EHS Department to implement and promote safety as well as setting project missions and milestones goals and reporting indicators for all project personnel. Webcor/Obayashi Joint Venture SSM manager may assign all or some of these tasks to other responsible persons as appropriate.

The SSM must help ensure that the guidelines, rules and procedures in this document are followed for site work. The SSM shall be familiar with local emergency services, help ensure that the proper steps are taken in the case of emergencies when a major event resulting in a fatality, multiple injuries, or property loss occurs. The SSM is responsible for requiring that we preserve the accident scene in an "as is" condition, including any construction equipment involved, to allow for a proper investigation. The SSM must order, if necessary, the area or piece of equipment to be stabilized to preclude further injuries or loss. Furthermore, the SSM shall notify Webcor/Obayashi Joint Venture Project Manager should an OSHA inspection be required. Should citations, warnings or safety violations be issued Webcor/Obayashi Joint Venture Management Team shall receive copies within 48 hours.

The SSM will be conducting or taking the necessary steps to help ensure that tool box/tailgate safety meetings are conducted before work startup. Additional meetings may be required for specific job tasks or site activities. Webcor/Obayashi Joint Venture SSM also must help monitor the maintenance and inspection of PPE, onsite hazards, the physical condition of site personnel, and perform daily safety audits of work site activities. Furthermore the SSM shall maintain safety files, which will include training and applicable medical certifications, environmental testing and special associated training, tool box/tailgate meeting notes and rosters, safety observation/audit reports, investigation reports including near-misses, injury summaries, required safety permits, security issues, or other safety and health documentation, as applicable.

The SSM is responsible for supporting Project Management in achieving an injury, incident and impact free environment as well as reporting all accidents and incident to the Project Manager in a timely manner as well as a responsibility for overseeing development, implementation and maintenance of the project's safety program by expediting corrective action(s) to abate any observed or potential safety exposure(s) to workers. The SSM shall continuously monitor trade subcontractor's safety performance and expedite abatement action(s) report unsafe acts and conditions and notify Webcor/Obayashi Joint Venture Project Manager and Superintendent regarding advisable corrective actions.

More duties of Webcor/Obayashi Joint Venture SSM include monitoring the subcontractor's compliance with the Webcor/Obayashi Joint Venture EHSP and to help familiarize sub-contractors and trade subcontractor Project Managers, Superintendents and Supervisors with the Webcor/Obayashi Joint Venture EHSP. These individuals must be familiar with safety and health hazards to which all workers may be exposed, as well as applicable laws, regulations and safety rules and policies and how to handle emergency situations. SSM is to help assure that all workers are trained in accordance with applicable requirements and ensure that observations, inspections, recognition, evaluations and abatement of hazards are conducted on a continuous basis. If the subcontractor does not make immediate corrections after initial notification, Webcor/Obayashi Joint Venture EHS will notify the subcontractor's Project Management in writing to make prompt corrective action to help eliminate construction safety concerns, forward copies of the written notice to Webcor/Obayashi Joint Venture Project Management and develop the direction to help resolve outstanding construction safety issues and maintain documentation of corrective actions.

The SSM is responsible for ensuring a Hot Work Permit is completed prior to hot work commencing and shall keep a log of all Permits.

### **Project Engineer**

The Webcor/Obayashi Joint Venture Project Engineer assists the Webcor/Obayashi Joint Venture Project Manager with his/her responsibilities for construction management services for the project. This person will complete weekly jobsite safety audits, participate in pre-task planning, subcontractor pre-bid, pre-construction, and/or kick-off meetings, assist with jobsite safety startup, safety orientations, participate in fact finding, Detailed Incident Analysis (DIA), implementing corrective actions to prevent further occurrences on all injury/incident investigations and attend and/or participate in jobsite safety meetings.

### **Subcontractor Responsibilities**

The subcontractor has overall responsibility for accident prevention and implementation of this Webcor/Obayashi Joint Venture EHSP for anyone under their control, including their respective employees, tiered subcontractors, vendors and suppliers.

Where subcontractor is not using a Site Safety Manager (SSM) the subcontractor will assign safety responsibilities to a member of their Project Management, that person(s) will be considered a Designated Safety Person (DSP). This assignment is subject to approval by Webcor/Obayashi Joint Venture Management and Webcor/Obayashi Joint Venture SSM. The subcontractor may be responsible for providing their SSM or DSP with a reliable communication method or device in order to contact Webcor/Obayashi Joint Venture Project Management and Webcor/Obayashi Joint Venture SSM during emergency response and/or other safety related communications. Although many existing hazards may be corrected through informal communications between the trade subcontractor's and tiered subcontractor's SSM or DSP with members of Webcor/Obayashi Joint Venture Project Management, all corrective actions must be documented, with copies forwarded to Webcor/Obayashi Joint Venture Project SSM.

Subcontractors will submit a copy of their companies and their tiered subcontractors company's safety program prior to beginning work. All subcontractor workers must be orientated to their company's safety program as well as to applicable sections of this Webcor/Obayashi Joint Venture EHSP. Furthermore, subcontractors and tiered subcontractors are required to incorporate the requirements of the Webcor/Obayashi Joint Venture's EHS Plan into their safety programs and safety orientation if theirs are less protective than those of Webcor/Obayashi Joint Venture.

### **Project Manager**

The subcontractor's Project Manager is responsible for planning and monitoring all work performed in compliance with the objectives of this Webcor/Obayashi Joint Venture EHSP, trade subcontractor's safety program, federal, state and local safety and health regulations. Authorizing immediate correction of any existing construction safety-related concerns, fully supporting the SSM or DSP and cooperating with all designated project safety personnel in obtaining corrective actions necessary to comply with the Webcor/Obayashi Joint Venture EHSP. Furthermore, trade subcontractors Project Managers shall complete weekly safety audits, participate in pre-task planning and subcontractor kick-off meetings, participating in fact finding, DIA, and resolution on all injury/incident investigations as well as when requested, attend special construction safety meetings.

### **Superintendent/Supervision/Foremen**

Responsibilities of the trade's subcontractor Superintendent/Supervisor/Foremen are the same as Webcor/Obayashi Joint Venture Superintendent/Supervisor/Foremen and they shall attend weekly contractors' safety meetings.

All supervisory personnel shall have as a minimum the OSHA 30 Hour Construction Safety training within the prior four years and possess a current CPR /First Aid and AED certification. In addition supervisory personnel shall have at a minimum 5 years' experience as a superintendent in a similar type of project.

### **Site Safety Manager / Designated Safety Person**

Every trade subcontractor employing **40** or more workers, including their lower tier sub-subcontract employees, must provide a full-time SSM/DSP that has no other job duties and is present on the project anytime work is being performed. An additional DSP shall be required for each additional **60** workers thereafter. Subcontractor shall also provide EHS Administrative support personnel as necessary to implement their EHS program. Contractor reserves the right to determine appropriate qualifications for Subcontractor's SSM/DSP personnel, based on project demands and reserves the right to interview candidates to determine qualifications.

The SSM/DSP shall be current in First aid/CPR/AED and hold a Construction Health and Safety Technician (CHST) and OSHA 500 certificate and have three (3) years prior full time safety duty experience working on a similar type of project at a minimum. The SSM / DSP is responsible for ensuring a Hot Work Permit is completed prior to hot work commencing. The Fire Safety Manager shall keep a log of all Permits. Subcontractors SSM shall serve as technical advisors to their project management team on safety and health planning, training and problem resolution issues.

The SSM/DSP shall report all incidents and injuries immediately to Webcor/Obayashi Joint Venture Project Management and SSM. In the event of an accident or injury the trade subcontractors Project Manager and SSM shall complete and forward all claim forms; injury, liability, property damage, and the



like, to Webcor/Obayashi Joint Venture SSM immediately. The SSM shall participate in accident investigations and recommend proper courses of corrective action. When serious accidents occur, this task will be performed in conjunction with Webcor/Obayashi Joint Venture SSM and Webcor/Obayashi Joint Venture and the subcontractor Project Management or their representatives. Each SSM/DSP has the right and authority to stop any and all hazardous work activities being performed by his/her company or their subcontractors until necessary corrective actions are taken or if there is an immediate danger to life and/or health present.

The SSM/DSP shall perform continuous safety audits of all their respective trade subcontractors and their tiered subcontractors' work areas throughout the entire workday and take immediate action to eliminate all unsafe acts and/or conditions. These observations, along with corrective actions taken shall be reported in writing to the appropriate member of Webcor/Obayashi Joint Venture Project Management, SSM and the subcontractor's own management. The SSM/DSP shall ensure that prior to the commencement of any work activity every Supervisor/Foreman reviews each task assignment with every affected employee to ensure a comprehensive understanding of the safety requirements and precautions to be followed while performing this work. This shall be documented using a JHA. The SSM/DSP shall ensure that appropriate PPE is provided and its use enforced, ensure that all of the necessary guards are in place, safety equipment is provided, and other required steps are taken prior to starting the work.

The SSM / DSP shall attend and participate in required safety meetings. The SSM / DSP shall provide appropriate materials for those conducting weekly tool box/tailgate meetings or safety meetings, as well as, review safety meeting reports for attendance and implement required safety training programs for subcontractor employees and supervisors. The SSM / DSP shall enforce their company's safety program and disciplinary procedures, accompany Webcor/Obayashi Joint Venture's supervisory personnel as directed and perform joint inspections of work areas and activities, orient all new personnel to the site's safety program prior to work commencement and the SSM/DSP are subject to Webcor/Obayashi Joint Venture's approval and may be removed at any time with or without cause and replacement personnel shall be provided at the subcontractor's / employer's expense.

### **Everyone's Responsibilities**

Everyone has the ability to stop work for safety reasons. Everyone shall report injuries, near misses, unsafe acts and conditions immediately to supervision. Everyone shall work according to good safety practices as posted, instructed and discussed. Everyone shall comply with Webcor/Obayashi Joint Venture EHSP and subcontractor's safety program. The use of all required safety devices shall be used. Everyone shall come to work alert and free of any impairment that may affect safety. Everyone is to keep their work areas clean and orderly as well as promote and support the Injury Free Environment. Everyone agrees to be held accountable for your safety, and the safety of others. Furthermore, everyone is held accountable for their designated assignments of responsibilities as denoted in their respective definitions. Refrain from performing any work which may feel unsafe or for which proper equipment and/or training have not been provided. Everyone has the right to stop work when an unsafe condition or act occurs.

### **Weekly Safety Meetings**

Trade subcontractors and tiered subcontractors are required to hold Weekly Safety "Tool Box" Meetings with their field crews. Copies of the meeting minutes and attendees shall be submitted to Webcor/Obayashi SSM at the end of each week. Webcor/Obayashi Joint Venture may provide assistance and information to trade subcontractors and their tiered subcontractors as requested.

In addition, subcontractors and tiered subcontractors are to attend monthly or whenever determined by Webcor/Obayashi Joint Venture all hands safety meeting.

## Pre-Task Planning

Pre-planning tasks has been proven to reduce incident and accidents. All workers engaged in a specific task are required to participate in pre-planning activities. Every worker has the right to stop work and contact management if unsafe acts or conditions occur.

## Job Hazard Analysis (JHA) Guidelines

A JHA is to be conducted daily, led by the Supervisor of the crew, documented in writing and signed by all crew members prior to starting work. JHA's shall include hazards relating to the task being done and the plan of actions the crew shall take to mitigate that hazard from occurring.

The JHA shall be readily available at the work site and posted and/or placed where crew members have knowledge of its location at the work area. JHA's should be reviewed and revised whenever work conditions or crew membership change that may affect the ability to safely complete the work.

A JHA is required for the following activities (at a minimum):

• Chemicals: hazardous & irritant	• Concrete: pre-cast, tilt up, vertical, form work
• Confined Space	• Hoisting & Rigging activities
• Demolition	• Framing activities
• Excavation & Trenching	• Fall Hazards: elevated work, overhead work
• Material Handling	• Non-routine activities
• Public Exposure	• Scaffolding
• Steel Erection	• Startup/Shut down/ System testing
• Working with hazardous materials	• Introducing chemicals into systems

## Safety & Health Training/Information

*This Section will conform to Specification Section 01 15 45 (1.10A) found in The Transbay Transit Center Contract Number 08-04-CMGC-000*

Trade subcontractors and their tiered subcontractors shall maintain, on-site, all training records in accordance with federal, state, and local statutes, regulations, and policies, and provide copies of these records to Webcor/Obayashi Joint Venture Management and the TJPA upon request.

New workers will be provided with initial training and/or orientation prior to assignment or when assigned to a new task for which training has not been received. Training will include general area and specific assignment topics. Refresher training will be provided in accordance with Federal/State OSHA guidelines. Completed training records are to be submitted to Webcor/Obayashi Joint Venture SSM in a timely manner. Supervisors are expected to be knowledgeable and informed on hazards and safe work practices in their area of responsibility and to coordinate the disbursement of this information to crews.

Training may include, but not be limited to:

• Aerial / Boom Lifts	• Asbestos awareness
• Confined Space	• CPR / 1 <sup>ST</sup> aid / AED
• Electrical	• Excavation & Trenching

• Fall Protection	• Fire Watch
• Forklift	• Hazard Communication
• Hazardous Chemicals	• Ladder
• Lasers	• Lead Awareness
• Lockout / Tagout (LOTO)	• Powder Actuated Tools
• Respirator Protection	• Rigging
• Scaffolding: Use & Erection / Dismantle	• Steel Erection
• Job Hazard Analysis	• Accident Investigation (Management)

## Code of Safe Conduct and Work Practices

The following Safety Procedures will be complied with on the Transbay Transit Center project. These Safety Procedures are in accordance with Webcor/Obayashi Joint Venture Safety Program, the TJPA and the division of Industrial Safety Cal/OSHA Construction Safety Orders.

### **General**

All subcontractors must submit their Company's Project Safety Program to the Project Site Safety Manager (SSM) prior to the start of their work. As a minimum, the subcontractor's Safety Program shall meet or exceed Webcor/Obayashi Joint Venture safety requirements, the applicable parts of the Webcor/Obayashi Joint Venture Corporate Safety Manual, the contract documents and federal, state, local or other applicable regulations.

Prior to trade subcontractors arrival, measures to identify, monitor and control the workers and the general public from identified hazards shall be included in their safety plans. The Program shall be reviewed by the Webcor/Obayashi SSM who may require additional written Safety Procedures and training records as may be necessary to address the potential hazards of the operations.

### **Personal Protective Equipment (PPE)**

All persons entering the work area shall wear the proper PPE at all times.

#### **Hardhats**

All persons entering the work area on this project are required to wear ANSI Z89.1 approved hardhats. 100% hardhats use is required at all times while on this project. Any person refusing to wear a hardhat will be immediately dismissed from the project site. Metal hardhats and "Cowboy" hardhats are not allowed to be worn.

#### **Eye Protection**

The wearing of eye protection will be strictly enforced at all times. 100% safety glasses use is required at all times while on the project. ANSI approved prescription glasses with side shield are acceptable as well as ANSI approved goggles.

#### **Hearing Protection**

Each trade subcontractor shall provide and enforce the use of hearing protection for all workers exposed to noise levels exceeding 85 decibels (db). Where hearing protection is required, signs stating so shall be posted.

## **Hand Protection**

Hand protection must be worn 100% of the time in any situation where hand/finger exposure to hazards exists, unless the manufacture of the equipment/material being used states gloves should not be worn. Supervisory Positions, Visitors, and Observers of work are not required to wear hand protection 100% of the time as they are not performing work, but must have gloves readily available in case a situation where hand/finger exposure to hazards arises.

## **Clothing & Foot Protection**

All personnel shall wear safety vests, work boots or acceptable work shoes while employed on this project and keep their clothing and footwear in good condition at all times. Long pants and shirts with “T-shirt-length sleeves or longer shall be worn at all times. No sneakers, tennis shoes, soft-suede/canvas hiking boots, shorts, tank tops, tattered clothing etc., will be allowed.

Additional foot protection shall be used with jumping jack compactors and jackhammers.

## **Safety Disciplinary Policy**

Under Webcor/Obayashi Joint Venture, all employees are required to follow company safety policies and operating procedures. When needed, employees will be provided with additional training and information, or retraining to maintain their knowledge.

Although Webcor/Obayashi Joint Venture reserves the right to discharge “at will,” we believe that employees found performing work in an unsafe manner that would endanger the employee or another employee shall be subject to discipline or termination by management. Webcor/Obayashi Joint Venture strictly maintains a zero tolerance policy towards violations involving, but not restricted to: fall protection, lock-out/tag-out, and confined space violations. The Webcor/Obayashi Joint Venture Project Management and SSM shall determine the course of action best suited to the circumstances. The steps to be taken at a minimum shall include the following:

Verbal Warning – As the first step in correcting unacceptable behavior, the Supervisor shall review the pertinent facts with the employee. The Supervisor will consider the severity of the problem, and the employee’s past performance. A verbal warning will be issued to the employee, if necessary; the employee will be placed on probation.

Written Warning – If the unacceptable performance continues, the next step will be a written warning. The written warning will clearly state the safety policy that was violated. Probation will be a part of the written warning. It may also include time off without pay. At the completion of the probationary period, the Supervisor will meet with the employee to determine if the employee has achieved the required level of performance.

Termination – The employee may be terminated if said employee does not improve their performance while on probation, or has violated another company safety policy within twelve (12) months.

## **Dismissal from Project**

The following is prohibited and the individual(s) engaging in such activity(s) may be subject to dismissal from this project:

- Fighting and horseplay.
- Alcohol consumption or controlled-substance use on the site.

- Crowding or pushing while accessing work levels on ladders, scaffolds, etc.
- Throwing trash or any objects from heights.
- Using fire equipment irresponsibly.
- Destroying property or the work of other trades.
- Stealing.
- Gambling on the project site.
- Unsafe work habits.
- Persons using prescribed medication must notify his/her employer of such use prior to going to work or taking the medication.
- Working while your ability or alertness is so impaired by illness or fatigue or other causes that it might unnecessarily expose you or others to injury.
- Noncompliance of any safety rules or regulations.
- Lewd or abusive language towards jobsite personnel, Owner's personnel, or any member of the public.
- Smoking Cigarettes/E-Cigarettes in unauthorized areas

## **Job Vehicular Traffic**

Only company-owned vehicles with signage are continuously required for the pursuit of trade subcontractor's and tiered subcontractor's work, and trucks delivering materials may be allowed access to the project site. All construction vehicle traffic access will be coordinated by Webcor/Obayashi Joint Venture.

There is no trade subcontractor or tiered subcontractor onsite parking on this project. Trade subcontractors and tiered subcontractors in violation of this request will be towed at their expense without further notice.

Subcontractors are to notify Webcor/Obayashi Joint Venture 48 hours in advance for approval of material deliveries. Material storage and layout must be approved by Webcor/Obayashi Joint Venture prior to delivery. Delivery vehicles will unload and depart the project site as soon as possible with the assistance of a qualified flagger to ensure pedestrian and vehicular traffic is controlled.

Subcontractors are reminded that continuous 2-way vehicular traffic must be maintained at all times for safe public accessibility unless posted otherwise. Two-way traffic control is to be provided by trade subcontractors prior to delivery vehicles entering the property.

Due to general liability exposure created by improper traffic control, all flagging, training, lane closures, etc. shall conform to the most current edition of the Manual on Uniform Traffic Control Devices (MUTCD). Local permitting issues shall be addressed by Webcor/Obayashi Joint Venture prior to the start of work. All workers in the traffic control area must be trained according to local, state and federal requirements and wear the appropriate reflective vest or high visibility clothing. Stop/Slow paddles shall be used to control traffic flow.

## **Temporary Offices**

Temporary offices will be constructed of fire-resistant materials only and heated with approved fire-safe heating devices in accordance with manufacturers' instructions. Shall be equipped with a minimum of one 20lb ABC fire extinguisher and shall have a 40-gallon waste container adjacent to it. Temporary office locations must be approved by Webcor/Obayashi Joint Venture prior to installation.

## **Fire Protection**

The purpose of the Fire Protection is to reduce to a minimum the possibility of fire damage and associated losses incurred during the construction of the Project. The following is a guide to be used on the Project to aid in preventing the spreading of materials loosed by fires and gases associated with combustion.

Appropriate action is the key to the prevention of loss of life and property damage. Emergency phone numbers will be posted in such a manner so as to be clearly visible. If a fire occurs, notify the local fire department and Webcor/Obayashi Joint Venture Management Team immediately. Extinguish fire with a noncombustible, such as sand, or an available fire extinguisher if properly trained to do so. Remove or shut off fuel supply and combustible material if trained and safe to do so.

### **General Fire Safety**

- All temporary electric service, equipment, and wiring must be in accordance with Cal OSHA and NFPA 70, National Electric Code.
- Storage of any material within ten (10) feet of fire hydrants is strictly prohibited.
- Work areas shall be inspected on a regular basis to prevent accumulation of material.
  - All combustible waste material, dust, and debris shall be removed from the building and its immediate vicinity at the end of each work shift, or more frequently as necessary, for safe operations.
- No motors or machinery shall be left running during nonworking hours except as specifically directed by Webcor/Obayashi Joint Venture.
- All heating equipment shall have necessary Safety devices and shall be operated according to all applicable codes, rules and regulations, and manufacturers' instructions.
- All tarps and blankets shall be of fire-retardant material.
- All fuel and solvent containers shall be in approved containers and placed on drip pans.
  - Storage of these materials shall be in accordance with product Safety Data Sheet (SDS), statutory Hazardous Material requirements, and Fire Department requirements.
- No open or burning fires shall be permitted onsite.
  - Anyone doing so will be subject to immediate dismissal.
- No solid fuel shall be permitted on the site.
- Fire extinguishers shall be placed and maintained on the job in conspicuous and identified locations.
  - These fire extinguishers shall not be moved or discharged, except for fighting a fire.
- All gas bottles, such as propane, oxygen, and acetylene, shall be stored and secured in a vertical position in areas designated by Webcor/Obayashi Joint Venture.
  - All stored bottles shall be capped.
  - Oxygen and acetylene will not be stored within 20 feet of each other or must be separated by a one-half-hour-rated fire barrier.
  - At no time during construction shall propane or LPG be stored inside of a structure or building.
- All oxygen and acetylene in use shall be in proper carts with required separations and with at minimum a 10lb ABC fire extinguisher.
- During welding or cutting operations, a hot work permit and a fire watch with the proper fire extinguisher will be required and shall be the responsibility of the subcontractor or its tired subcontractor performing the work.
  - Hot work permits can be obtained from the SSM/DSP.

- Each trade is responsible for providing fire extinguishers and a fire-watch program for their work as required.

### **Hot Work Activities**

When all fire prevention measures are taken, permits shall be authorized for the work. New construction work shall require the presence of a dedicated fire extinguisher (20lb, ABC), provided by the trade subcontractor performing the work, and any other preventive measures as may be necessary for protection of life and property such as but not limited to fire blankets and water supply.

The trade subcontractor and the SSM/DSP shall ensure that the surrounding area(s) are free of combustible material. When the work is of the nature that hot material may fall to areas below, the trade subcontractor and the SSM/DSP shall ensure that those areas are free of combustible material or material that may otherwise be damaged. Work in place must be protected by the trade subcontractor performing the work.

Each trade subcontractor and tiered subcontractor shall notify Webcor/Obayashi Joint Venture of proposed Hot Work activities through a Welding/Cutting Permit. The SSM/DSP shall review the Permit form with the trade subcontractor to assure that all areas of concern are accounted for in fire protection. Hot Work shall not be performed near fuel storage areas or other areas where combustible vapors may accumulate.

In occupied building, Hot Work shall not be performed in occupied buildings without notification of the local Fire Department responding agency. The fire suppression system for the building must be in operation. The appropriate Building or Department Managers must be notified and the work coordinated with their operations. Preparation for the work and clearing of combustible materials shall be in accordance with federal and state standards. Combustible material shall be cleared from the work area by a distance of 35 feet.

### **Material Handling**

Housekeeping is an extremely important contributing factor for ensuring the safety and health in the workplace. Keeping aisles and passageways clear to provide for the free and safe movement of material handling equipment and employees is of the utmost importance. Other important contributing factors to ensure a safe working environment is as follows:

- Wear proper PPE at all times while handling material, equipment and tools.
- Post conspicuously the maximum safe load limits of floors within buildings and structures, in pounds per square foot, in all storage areas, except for floor or slab on grade.
  - Do not exceed the maximum safe loads.
- Do not store materials on scaffolds or runways in excess of supplies needed for immediate operations.
- Use ramps, blocking, or grading when a difference in road or working levels exists to ensure the safe movement of vehicles between the two levels.
- Do not place materials stored inside buildings under construction within six (6) feet of any hoist way or inside floor openings, or within ten (10) feet of an exterior wall which does not extend above the top of the material stored.
- Do not drop or throw blocks from an elevation or deliver blocks through chutes.
- Remove all nails from used lumber before stacking.
- When bending reinforcing steel on the job, use a strong bench set up on even dry ground or a floor to work on.

- Do not remove frozen material in a manner that would produce an overhang.
- Use proper lifting techniques.
- Stacking Material
  - Make sure that all materials stored in tiers are stacked, racked, blocked, interlocked, or otherwise secured to prevent sliding, falling, or collapse.
  - Stack bagged materials by stepping back the layers and cross-keying the bags at least every ten bags high.
  - When bags are removed from the pile, keep the length of the pile at an even height and maintain the necessary step backs every five bags.
  - When stacking inside a building, distribute the piles to prevent overloading the floor.
  - If not racked, stack and block structural steel, poles, pipe, bar stock, and other cylindrical materials as to prevent spreading or tilting.
  - Carefully pile structural steel to prevent danger of members rolling off or the pile toppling over.
  - Keep structural steel in low piles, giving consideration to the sequence of use of its members.
  - Stack corrugated and flat iron in flat piles, with the piles not more than 4 feet high; place spacing strips between each bundle.
  - Frequently inspect stock piles of sand, gravel, and crushed stone to prevent their becoming unsafe by continued adding to or withdrawing from the stock.
- Stacking Lumber
  - Do not stack lumber more than 20 feet high; if handling lumber manually, do not stack more than 16 feet high.
  - Stack lumber on level and solidly supported sills, and such that the stack is stable and self-supporting.
  - Stack stored lumber on timber sills to keep it off the ground. Sills must be placed level on solid supports.
  - Place cross strips in the stacks when they are stacked more than 4 feet high.
- Stacking Bricks
  - Do not stack bricks more than 7 feet high. When a loose brick stack reaches a height of 4 feet, taper it back 2 inches for every foot of height above the 4-foot level.
  - Never stack bricks, for storage purposes, on scaffolds or runways.
  - Always stack blocks; do not throw in a loose pile.
  - When stacking masonry blocks higher than 6 feet, taper back the stack one-half block per tier above the 6-foot level.
- Cement Bags
  - Carefully handle cement and lime delivered in paper bags to prevent the bags from bursting.
  - Do not pile cement and lime bags more than ten bags high except when stored in bins or enclosures built for the purpose of storage
  - When handling cement and lime bags, wear eye protection preventing any contact with the substance (such as goggles or other sealed eye protection) and wear long sleeve shirts with close fitting collar and cuffs.
  - Do not wear clothing that has become hard and stiff with cement.
  - Make sure to report any susceptibility of skin to cement and lime burns.
  - Make sure that a hand cream or Vaseline and eyewash is provided and kept ready for use to prevent burns.



- Store lime in a dry place to prevent a premature slacking action that may cause fire

## **Cleanup and Housekeeping**

Trade subcontractors and tired sub-subcontractors shall leave the site clean and free of debris and hazardous materials by the end of each working day to the satisfaction of Webcor/Obayashi Joint Venture. Each subcontractor is responsible for removal of debris created by their work. Rubbish containers will be placed at a central location for the removal of trash and debris. Accumulation of trash and debris will not be tolerated. Webcor/Obayashi Joint Venture will perform necessary cleanup of same, at trade subcontractors' expense, upon failure to comply with cleanup notice request.

Ensure compliance with local fire regulations if disposing of waste material or debris by burning. Remove all scrap lumber, waste material, and rubbish from the immediate work area as the work progresses. Keep all solvent waste, oily rags, and flammable liquids in fire-resistant covered containers until removed from the work site.

Whenever materials are dropped more than 20 feet to any point lying outside the exterior walls of the building, use an enclosed chute of wood or equivalent material. When debris is dropped without the use of chutes, make sure that the area onto which the material is dropped is completely enclosed with barricades at least 42 inches high and 20 feet back from the projected edge of the opening above. Post at each level warning signs of the hazard of falling materials. Do not remove debris in this lower area until debris handling ceases above.

## **Security Services**

Trade subcontractors and tired sub-subcontractors shall be responsible for the security of toolboxes, onsite storage materials, etc.

## **Noise Control**

*This Section will conform to Specification Section 01 35 65 (1.2E) (1.8B), (1.8C) found in The Transbay Transit Center Contract Number 08-04-CMGC-000*

Trade subcontractors shall conduct noise inspections and noise testing of equipment to ensure that all equipment on site is in good condition and effectively muffled per manufacturer's recommendation. Noise control shall be maintained by the trade subcontractors in all areas of construction, guarding against undue noise.

All motor-drive equipment shall have a proper exhaust system, which shall meet Cal/OSHA Standards on noise levels. Subcontractors are to post signage and provide proper hearing protection to employees using chipping guns, jackhammers, rock drills, or similar devices where the decibel level exceeds 85 and double hearing protection as required by state law.

Playing of radios, including headsets, is prohibited.

## **Combustible Material**

Separate storage areas for acetylene, oxygen, and gasoline will be established by Webcor/Obayashi Joint Venture. The trade subcontractor shall post proper warning signs where combustible material is being used or stored. All gasoline will be in containers that meet NFPA and Cal/OSHA requirements, and will be stored in designated areas only.

All acetylene and oxygen bottles shall be secure and in a vertical position. All carts must be equipped with a fire extinguisher. All stored oxygen and acetylene must be separated from each other, by a minimum of 20 feet or a fire-rated barrier, with bottle caps secured in place as required by Cal/OSHA.

## **Crane**

The safe operation and proper maintenance of cranes and rigging on the site shall be the overall responsibility of the trade subcontractor. Each trade subcontractor shall also be held accountable for compliance with CAL/OSHA crane regulations for all cranes or derricks on the site, whether contractor owned, leased or rented. All rigging inspection logs shall be completed and submitted to Webcor/Obayashi Joint Venture SSM monthly.

A thorough inspection by a certified independent 3<sup>rd</sup> party company shall be conducted prior to initial use and post repair of a crane or derrick. Any deficiencies found shall be corrected before the equipment is placed into service. A copy of the annual certification inspection performed by a certified independent 3<sup>rd</sup> party shall be submitted to the Webcor/Obayashi Joint Venture SSM prior to the crane being operated on site.

Each contractor shall designate a competent person who shall inspect all cranes and derricks daily as part of the trade subcontractor's job site inspection program. Such inspections shall be documented and submitted to Webcor/Obayashi Joint Venture SSM weekly. Defective equipment shall be removed from service and repaired; service/repair shall be documented and submitted to Webcor/Obayashi Joint Venture SSM.

Loads shall not be passed or suspended over persons. Routes of suspended loads shall be preplanned to ensure no workers or the public are directly below suspended loads. Lifts shall not be conducted over employees, visitors, or areas occupied by the public. Tag lines shall be used for controlling all loads. Tag lines or guide ropes shall be used to control all loads. Accessible areas within the swing radius of the rotating superstructure shall be properly barricaded to prevent employees from being struck or crushed by the crane.

## **Crane Lift Plan**

A complete, competent and Webcor/Obayashi Joint Venture approved Crane Lift Plan is required prior to any crane lift while working. The Crane Lift Plan must be submitted to Webcor/Obayashi Joint Venture 48 hours (2 business days) prior to mobilization at a minimum. Neither TJPA nor Webcor/Obayashi Joint Venture shall be held responsible for any delay allegations as a result of the trade subcontractor failing to submit Crane Lift Plans on a timely basis. The Trade Subcontractor / Crane Company / Rigging Company is responsible for the accuracy of all calculations and inspections. This planning process has been established to help ensure proper coordination between trade subcontractors and Webcor/Obayashi Joint Venture. No warranty or certification of the suitability of this plan is accepted by Webcor/Obayashi Joint Venture.

The Crane Lift Plans must be based on a “worst case” combination of load weight with chart deductions and lift radius for a specific crane configuration in a specific location. Work that is not anticipated but may arise due to site conditions (moving equipment, loading materials onto floors, etc.) must be reviewed with Webcor/Obayashi Joint Venture prior to hoisting. Changes affecting crane configuration may require the Crane Lift Plan to be amended.

Lifts exceeding 75% of the cranes stability / structural capacity chart, requiring movement of a crane carriage with the load, personnel platforms, critical loads (long lead time, cost), tripping loads, work over occupied facilities, or work involving encroachment on public rights of way, will require the preparation, submittal and review of a specific JHA (Note: These lifts are discouraged). These lifts must be reviewed in advance. The Crane Lift Plan(s) may have to be prepared and stamped by a licensed Professional Engineer to be approved by Webcor/Obayashi Joint Venture.

Attachments to the Crane Lift Plan may include but are not limited to:

- Plot plan with crane location (identify swing path, delivery truck locations, location of any overhead power lines, etc)
- Elevation plan
- Crane load charts and calculations including any notes
- Dimension illustration and specifications for crane and range chart
- Operators license, training information, USDOT medical certificate and OSHA training
- Rigging plan, lists and diagram
- Names and qualifications for designated and competent persons (crane operator, A/D Supervisor, rigger and signal person
- JHA
- Logistics and assembly / dismantle plan
- 3<sup>rd</sup> party annual inspection certification
- Weight of material
- Lighting and wind restrictions (from operators manual)

The Crane Lift Plan may be valid for more than one day, as long as the configuration, location, maximum expected load, and maximum expected radius does not change. Multiple lift plans will be required for multiple locations.

### **Responsibility**

It is the responsibility of the Trade Subcontractor and the Crane Operator to ensure that they and their employees are qualified, competent, properly equipped and properly trained to perform the activities outlined in this plan.

### **Management**

The trade subcontractor is responsible to visit the site prior to the lift date to review documentary information pertaining to the site, which is maintained by Webcor/Obayashi Joint Venture. The trade subcontractor is responsible to obtain all information that is necessary to develop a power line safety plan, if needed. Furthermore, trade subcontractors are responsible for ensuring rigging equipment is in good condition and provided with safety devices as applicable. This includes such things as safety latches on hoisting hooks, chains, wire rope and slings are free from defects and conform to standard load ratings for work being done and eye splices conform to safety standards. Trade subcontractor's employee training is current and each contractor shall ensure that all of its employees involved in crane activities receive

comprehensive training as to their responsibilities. This training shall include hand signals and those authorized to give signals. Said training shall be documented.

Each trade subcontractor shall ensure that its crane operators is not engaged in any practices that may divert their attention while engaged in crane operations, ensure the operator is physically and mentally fit for duty, responds to only clear signals and stop signals. The trade subcontractor shall ensure the operator is intimately familiar with the equipment being used and is empowered to discuss any issues with their Supervisor.

### **Operator**

Each crane operator will be specifically assigned the responsibility for safe operations and shall be given written instructions as applicable. Only designated operators who have been licensed by an approved agency or union and meet the requirements shall be in or on the crane during operations. The crane operator shall be responsible for determining the safe operation of their crane and the safety of each lift. The operator has the authority to refuse a lift due to safety concerns. For example refusing to lift any loads that are not safely rigged. Any manager, supervisor or person attempting to bypass the crane operator's authority on this issue will be immediately removed from the project. The operator shall immediately shut down the crane if the operator suspects any problems with the crane or if any part of the crane, rigging or load strikes any object. Immediately report the issue to Webcor/Obayashi Joint Venture Supervisor and SSM.

The operator is also responsible for assuring that routine maintenance is performed, as well as necessary repairs and to coordinate testing and maintenance personnel when necessary. Daily inspections shall be conducted to include but not limited to condition of brakes, functioning of safety devices and limiting devices, electric power installation, overload controls, conditions of the structural membrane and ensure a fire extinguisher is available and current.

Verification of a current annual inspection certification shall be available for the crane. Verification that manufacturer's rated load capacities, recommended operating speeds, and special warnings or instructions are posted on the crane and are visible from the operator's station. Upon request the operator may be asked to demonstrate their knowledge of the crane and the crane load chart among other items.

Responsibility for assuring that signaling and communications are adequate. This includes making sure that personnel at materials loading and receiving areas use correct hand signals. Where conditions require, radio communications will be used with a clear channel for crane operations. Making sure that adequate clearances exist between operating areas and nearby structures, especially power lines. Ensure that good housekeeping is maintained in and around the equipment. The operator shall never leave the controls while there is a load on the hook.

### **Training Requirements**

Training records must be submitted to Webcor/Obayashi Joint Venture SSM prior to the employee(s) first day on site.

Riggers shall meet the qualified rigger requirements of subpart CC – Cranes and Derricks in Construction, as specified in 29 CFR 1926.1401, 1926.1404, and 1926.1425. These provisions are effective November 8, 2010. The more stringent rule shall apply.

Operators shall meet the qualified operator requirements found in 29 CFR 1926.1427. The operator has been licensed by an approved agency or union and meet the requirements in Chapter 5, ANSI B30 and the operator has passed their physical exam conducted by a license Physician approved by the DOT.

## **Fall Protection**

Work activities that expose worker(s) to fall hazards of six (6) feet or greater measured from the work platform to the bottom of the sole of the foot are activities defined by Webcor/Obayashi Joint Venture to be High Hazard and therefore require detailed, written Job Hazard Analysis (JHA). Webcor/Obayashi Joint Venture maintains a zero tolerance policy for fall protection infractions. Anyone found violating this policy may be removed from the site immediately. All trade subcontractors shall provide appropriate fall protection at the Companies cost.

Possible conditions that may require fall protection:

- Ladders
- Aerial Lifts / Scissor Lifts
- Scaffold work
- Precast erection
- Unprotected Sides & Edges / Leading edges
- Excavations & Trenching
- Wall Openings
- Holes

Trade subcontractor are required to provide training and fall protection for their employees. This can be accomplished through the use of the following systems:

- Guardrail System
- Positioning Device System
- Warning Line System
- Personal Fall Arrest system
- Safety Net System
- Controlled Access Zone

The building perimeter cable is placed as a guardrail protection, and is not provided for tie-off protection.

Webcor/Obayashi Joint Venture does not allow the use of body belts or a Safety Monitor System.

## **Fall Protection Training**

Trade subcontractors and tiered subcontractors must provide, as a minimum, by a competent person, the following training. Documentation of training must be forwarded to Webcor/Obayashi Joint Venture upon request:

- The nature of the fall hazards in the work area.
- The correct procedure for erecting, maintaining, disassembling and inspecting the fall protection systems to be used.
- The use and operations of guardrail systems, personal fall arrest systems, safety net systems, warning line systems, controlled access zones and any other methods of protection to be used.
- The limitations on the use of mechanical equipment.
- The correct procedures for the handling and storage of equipment and materials
- The erection of overhead protection.
- The role of workers in rescue plans.

## **Rail Systems**

A standard railing should consist of a top rail, intermediate/mid-rail, toe board and posts. The top rail should be approximately 42 inches from the upper surface of the rail to the floor, platform, or ramp level. The top rail should have a smooth surface throughout its length and be made of at least 2-inch by 4-inch

stock, 3/8-inch double clamped wire rope or its equivalent. It should be secured to withstand a 200-pound, horizontal force with minimum deflection.

The midrail should be halfway between the top rail and the floor, runway, platform, or ramp. The ends of the rail should not overhang the terminal posts except when it does not constitute a projection hazard. The midrail sill should be made of at least 1-inch by 6-inch stock or its equivalent.

The toe board should have a 4-inch minimum height and should be securely fastened in place with no more than 1/4 inch clearance above the floor level.

Wooden railing posts (verticals) should be made of at least 2-inch by 4-inch stock or its equivalent, and be spaced so as not to exceed 8 feet on center.

Other types, sizes and arrangements of railing construction are acceptable, provided they meet the following requirements. Have a smooth surfaced top rail approximately 42 inches above the floor, strength to withstand the minimum of 200 pound top rail pressure with a minimum of deflection and for specific material requirements, refer to applicable regulations.

### **Guard Rail Openings**

Work that requires the opening of guardrails or the removal of hole covers shall be approved in advance by the Webcor/Obayashi Joint Venture Project Management. Particular attention shall be given to the alternate means of fall protection required to safely perform the work and protect other workers in the vicinity of the fall exposure. Those who remove the rail, are responsible for replacing it in a manner meeting or exceeding local, state, federal, or Webcor/Obayashi Joint Venture practices, whichever may be more stringent.

### **Floor & Wall Openings**

To control conditions where there is a danger of workers or materials falling through floor, roof, perimeter edges or wall openings, such openings shall be securely covered and/or protected, capable of withstand 2x the load, be secured to the floor and shall be inspected daily by the trade subcontractor competent person. Trade subcontractor's Competent Person is responsible for identifying any floor opening or hole requiring to be protected. Covers should be clearly marked "Hole Do Not Remove" in a high visible color and anchored.

For purposes of covering, a floor opening is defined as any opening from 2" up to 16 square feet. All others must be protected with top and intermediate rail and toe board. All protection systems are to be maintained at all times. Any violation that is not rectified immediately will result in removal of the responsible Supervisor. Further violations will result in termination for cause of the responsible subcontractor's contract.

The building perimeter, shafts, and floor openings shall be protected with guard rails and toe boards. Personnel working at a stationary position within 6'-0" of the building perimeter or the edge of a shaft or a floor opening will wear a full body harness and be tied off with an appropriate lifeline. Trade subcontractors and tiered subcontractors shall not remove any guard rail or fall protection device without the express consent of Webcor/Obayashi Joint Venture. Any employee removing such protection without authorization will be removed from the project without recourse. Any area where guardrails and toe boards have been removed shall not be left unattended during a shift. In no case will any guardrail or toe board be left down at the end of a shift.

In locations where temporary protection conflicts with scheduled construction, the trade subcontractor or the tiered subcontractor shall notify Webcor/Obayashi Joint Venture in advance of the work of necessary modifications. The trade subcontractor or the tiered subcontractor shall remove the temporary protection and provide other appropriate temporary measures for the performance of their work.

### **Personal Fall Arrest Systems**

Personal fall arrest systems are designed to control the fall of a worker and minimize the injury once a worker has fallen. Personal fall arrest systems consist of a full body harness, a shock absorbing lanyard or retractable, and a tie off point.

### **General Fall Protection**

- Any safety harness, lifeline or lanyard actually subjected to in-service loading must be immediately removed from service and should not be used again for worker safeguarding
- Fall arrest equipment should be removed from service when evidence of wear is detected.
- All safety harnesses, lifelines and lanyards must have a nominal breaking strength of 5,000 lbs (5,400 lbs in CA).
- All fall protection equipment shall be inspected daily/monthly and before each use, with documentation made available upon request that it is in proper working order.
- Body Harness
  -
- Lanyards
  - Retractable lifelines are preferred where direct anchorage is not available.
  - 
  - All lanyards must be equipped with locking snap hooks.
  - Appropriate shock absorbing lanyards will be used for fall protection when they do not create a greater hazard due to the length of the potential fall.
  - Shock absorbing lanyards are not to be used in combination with a retractable lanyard.
- Anchorage point
  - The anchorage (tie off point) must be capable of withstanding a minimum 5,000 lbs (5,400 lbs in CA) tensile strength per worker attached.
  - Anchorage used for attachment of personal fall arrest equipment should be secured above the point of operation whenever possible
    - Anchorage, tie off, must generally be above the worker's head.
  - Anchorage must be high enough that the worker will not strike any lower level surface or object should a fall occur.

### **Safety Nets**

The use of safety nets may be allowed only after a written fall protection plan, limited to the actual work to be performed, is reviewed and approved by Webcor/Obayashi Joint Venture. Safety nets should be provided by the trade subcontractor or tiered subcontractor when work places are more than 25 feet above the ground or other surfaces where the use of ladders, scaffolds, catch platforms, temporary floors, safety lines or safety harnesses are impractical. When safety net protection is required, operations should not be undertaken until the net is in place and has been thoroughly tested.

Safety nets should extend 8 feet beyond the edge of the work surfaces where workers are exposed and should be installed as close under the work surface as practical. In no case should the safety net be more

than 25 feet below the work surface. Nets should be hung with sufficient clearance to prevent the user's contact with surfaces or structures below. Clearances should be determined by impact load testing. The mesh size of the nets should not exceed six (6) inches by six (6) inches. All nets should meet accepted standards of 17,500 foot pounds minimum impact resistance, as determined and certified by the manufacturer, and should bear a label of proof test. Edge ropes should have a minimum breaking strength of 5,000 pounds. Forged steel safety hooks or shackles should be used to fasten the net to its supports. Connections between net panels should develop the full strength of the net.

### **Rescue Plans**

Specific plans for rescue of worker(s) should be developed and rehearsed prior to initiating work requiring the use of fall protection. Rescue plans and the basic work plan should be submitted to the Webcor/Obayashi Joint Venture Project Management and SSM for review and comment. Concerns expressed by Webcor/Obayashi Joint Venture Project Management and SSM or any other reviewing authority shall be addressed fully prior to exposing any worker to the elevated work area.

### **Falling Object Protection Systems**

Anytime a potential hazard of falling objects exists, suitable systems must be provided to protect workers. Examples of suitable fall object protection systems may include covers, toe boards, canopies and debris nets. Proper barricading shall encompass the entire possible target area.

### **Ladders**

All ladders shall be inspected prior to use and used for its intended purpose.

#### **General Ladder Safety**

- When ascending or descending a ladder, employees shall maintain three-points of contact and not carry anything that could cause them to fall. Pull ropes should be placed at all access ladders to lift tools or equipment from level to level.
- As a minimum, only type 1 or 1-A Heavy/Extra Heavy duty ladders, which carry a minimum of 275 lbs. to 300 lbs., will be allowed on Webcor/Obayashi Joint Venture projects.
- Metal ladders shall not be used on Webcor/Obayashi Joint Venture projects.
- Fall prevention shall be considered by the competent person if an employee works from a ladder 6' or more above a lower level.
- Ladders are not to be painted except for numbering purposes.
- Do not use ladders for skids, braces, workbenches, or any purpose other than climbing.
- Always face the ladder when ascending and descending.
- If you must place a ladder over a doorway, barricade the door to prevent its use and post a warning sign.
- Only one person is allowed on a ladder at a time.
- Do not jump from a ladder when descending.
- All joints between steps, rungs, and side rails must be tight.
- Safety feet must be in good working order and in place.
- Rungs must be free of grease and/or oil.
- Portable ladders must be used at such a pitch that the horizontal distance from the top support to the foot of the ladder is about one-quarter of the working length of the ladder.
- All ladders must be equipped with safety (non-skid) feet.

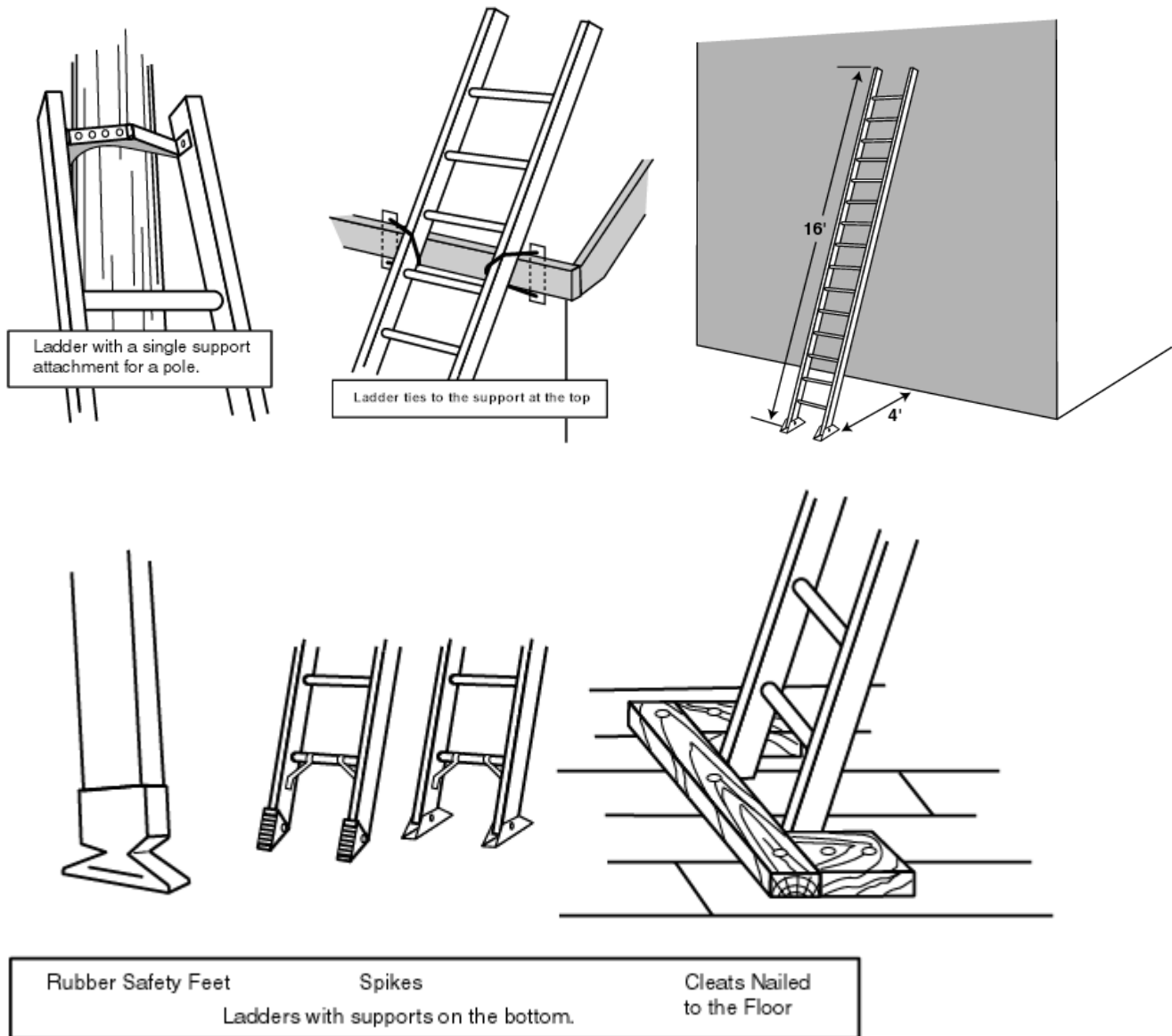


## Stepladders

- Do not place tools or materials on the steps or platform of a stepladder.
- Do not use the top two steps of a stepladder as a step or stand.
- Always level all four feet and lock spreaders in place.
- Do not use a stepladder as a straight ladder.

## Straight type or extension ladders

- All straight or extension ladders must extend at least three (3) feet beyond the supporting object when used as an access to an elevated work area.
- After raising the extension portion of a two or more stage ladder to the desired height, check to ensure that the safety dogs or latches are engaged.
- All extension or straight ladders must be secured or tied off at the top and bottom.



## **Scaffolds**

All scaffolds shall be constructed and maintained so as to meet all safety requirements of Cal/OSHA and Webcor/Obayashi Joint Venture. Failure to maintain scaffolds in good condition will result in removal by Webcor/Obayashi Joint Venture. All scaffolds must have top rails, mid rails, and toe boards at all platform levels. All scaffolds are to be built under the supervision of a competent person. The person's name and their qualifications shall be submitted in writing to Webcor/Obayashi Joint Venture prior to the start of work. Daily pre-shift inspection checklists shall be performed by a competent person, maintained by the trade subcontractor, available to all who access the scaffold and submitted to Webcor/Obayashi Joint Venture upon request.

A competent person shall determine if it is feasible to use fall protection devices while erecting /dismantling a scaffold. 100% fall protection is required at all heights above 6'. Rolling scaffold wheels shall be locked when in use. A horizontal, diagonal brace shall be in place to prevent the scaffold from "wracking". Cross bracing shall not be used as a top or mid rail.

## **Aerial Lifts**

Only authorized persons should operate an aerial lift, and must be trained on the equipment they will be operating. A spotter may be needed when there is a potential for operator injury due to physical contact with facility systems or structures or in congested areas. Spotters may also be needed when there is a potential for damage to sensitive facility systems or structures.

Lifts should be inspected each day prior to use to verify they are in safe working condition. Any lift that does not meet inspection guidelines shall be removed from service and either returned, replaced, or modified to meet requirements. Boom and basket load limits specified by the manufacture should not be exceeded. The brakes should be locked and when outriggers are used, they should be positioned on pads or a solid surface. Wheel chocks must be used before using an aerial lift on an incline provided they can be safely installed. Aerial lifts should have both upper and lower controls. Upper controls should be in or beside the platform within easy reach of the operator. Lower controls should provide for overriding the upper controls. Controls should be plainly marked as to their function. Lower level controls should not be operated unless permission has been obtained from the employee in the lift, except in case of emergency.

Always stand on the floor of the basket, do not sit or climb on the edge of the basket or use planks, ladders, or other devices for a work position. A body harness should be worn and a shock absorbing lanyard attached to the boom or basket when working from an aerial lift. Tying off only to recommended anchorage points.

An aerial lift truck should not be moved when the boom is elevated with personnel in the basket.

## **Scissor Lifts**

Lifts should be inspected each day prior to use to determine that they are in safe working condition. Only authorized persons should operate a scissor lift, and must be trained on the equipment they will be operating. Lifts should be operated in accordance with manufacturer's recommendations. Any lift that does not meet the required inspection guidelines shall immediately be removed from service and either returned, replaced, or modified to meet this requirement. A spotter may be needed when there is a potential for operator injury due to physical contact with facility systems or structures and in congested areas. Spotters may also be needed when there is a potential for damage to sensitive facility systems or structures.

## **Electrical**

Ground Fault Circuit Interrupter (GFCI) protection is required for all electrical cords and tools. Each trade subcontractor shall provide GFCI protected power strips for use on the site when permanent power has been energized and permanent outlets are placed in service. Each trade subcontractor shall be responsible for providing and maintaining temporary GFCI's for their employees if a GFCI receptacle is not available.

## **Lockout/Tag out Procedures (LOTO)**

Subcontractors shall submit their written LOTO program and documented employee training prior to beginning LOTO procedures. The program must include scope of training, pre-planning and specific LOTO procedures. All individuals who are working in or around the hazardous energy shall place their own lock and tag on the disconnect switch of the energy source. At no time will someone be allowed to remove another employee's lock unless it has been cleared through Webcor/Obayashi Joint Venture Competent Supervision.

## **Powder Actuated Tools**

Only low-velocity-type tools will be allowed on this project. Special permission from Webcor/Obayashi Joint Venture must be obtained before high-velocity types can be used, and then only if the job requires it. All personnel working with powder-actuated tools shall be properly instructed and licensed for operation of the tool and shall be in possession of current certification while using powder-actuated tools. Hearing protection signs, ear plugs and warning signs shall be posted in the work area where powder-actuated tools are in use.

## **Heat Illness Prevention**

Heat related illnesses are avoidable if the employees are trained and the right actions are taken before, during, and after working in either indoor or outdoor hot conditions. High temperatures, humidity, air velocity and radiant heat from the sun or a furnace can stress the body's ability to cool itself making heat illness a big concern during hot weather months. These would be considered environmental risk factors. Every employee whose job duties require them to work in the outdoors during summer months, are exposed to elevated heat conditions and therefore are susceptible to heat illness.

The three major forms of heat illnesses are: heat cramps, heat exhaustion, and heat stroke. Heat stroke can be a life threatening condition. This document will outline those actions as well as describing the three major forms of heat illness, how to recognize them, and what an action to take to provide first aid before medical care is provided. If an employee is experience heat related illness notify their Supervisor and Webcor/Obayashi Joint Venture SSM immediately.

## **Heat Cramps**

Heat cramps are the most common type of heat related injury and probably have been experienced by nearly everyone at one time or another. Heat cramps are muscle spasms which usually affect the arms, legs, or stomach. Frequently they do not occur until sometime later after work, at night, or when relaxing. Heat cramps are caused by heavy sweating, especially when water is not replaced quickly enough. Although heat cramps can be quite painful; they usually don't result in permanent damage.

Prevention/First Aid:

Drink electrolyte solutions such as Gatorade or plenty of water during the day and try eating more fruits such as bananas to help keep your body hydrated during hot weather. Call 911 and contact your supervisor immediately if the Person becomes ill.

### **Heat Exhaustion**

Heat exhaustion is more serious than heat cramps. It occurs when the body's internal temperature regulating system is overworked, but has not completely shut down. In heat exhaustion, the surface blood vessels and capillaries, which originally enlarged to cool the blood, collapse from loss of body fluids and necessary minerals. this happens when you do not drink enough fluids to replace what you are sweating away symptoms Include: Headache, heavy sweating, intense thirst, dizziness, fatigue, loss of coordination, nausea, impaired judgment, loss of appetite, hyperventilation, tingling in hands or feet, Anxiety, cool moist skin, weak and rapid pulse (120-200), and low to normal blood.

#### **Prevention/First Aid:**

The employee suffering these symptoms should be moved to a cool location such as a shaded area or air-conditioned building. Have them lie down with their feet slightly elevated. Loosen their clothing, apply cool, wet clothes or fan them. Have them drink water or electrolyte drinks. Try to cool them down, and have them checked by medical personnel. Victims of heat exhaustion should avoid strenuous activity for at least a day, and they should continue to drink water to replace lost body fluids. Call 911 if the person becomes non-responsive, refuses water, vomits, or loses consciousness.

### **Heat Stroke**

Heat stroke is a life threatening illness with a high death rate. It occurs when the body has depleted its supply of water and salt, and the victim's core body temperature rises to deadly levels. A heat stroke victim may first suffer heat cramps and/or heat exhaustion before progressing into the heat stroke stage, but this is not always the case. It should be noted that, on the job, heat stroke is sometimes mistaken for a heart attack. It is therefore very important to be able to recognize the signs and symptoms of heat stroke and to check for them anytime an employee collapses while working in a hot environment. Symptoms of heat stroke include: A high body temperature (103 degrees F); a distinct absence of sweating (usually); hot red or flushed dry skin; rapid pulse; difficulty breathing; constricted pupils; any/all the signs or symptoms of heat exhaustion such as dizziness, headache, nausea, vomiting, or confusion, and possibly more severe systems including; bizarre behavior; and high blood pressure. Advance symptoms may be seizure or convulsions, collapse, loss of consciousness and a body temperature of over 108 degrees F.

#### **Prevention/First Aid:**

It is vital to lower a heat stroke victim's body temperature. Quick actions can mean the difference between life and death. Pour water on them, fan them, or apply cold packs. Call 911 to get the person medical aid as soon as possible.

### **Guidelines for Preventing Heat Illness**

If an employee is coming back to work from an illness or an extended break or is just starting a job working in the heat, it is important to be aware that they are more vulnerable to heat stress until their body has time to adjust. The employee needs to let their Supervisor know they are not used to the heat. It takes about five (5) to seven (7) days for a body to adjust. Drinking plenty of water frequently is vital to workers exposed to the heat. An individual may produce as much as two (2) to three (3) gallons of sweat per day. In order to replenish that fluid the worker should drink three (3) to four (4) cups of water every hour starting at the beginning of your shift. Taking breaks in a cool shaded area and allowing time for recovery from the heat during the day are effective ways to avoid heat illness. Avoid or limit the use of

alcohol and caffeine during periods of extreme heat, both dehydrate the body. Whenever possible wear clothing that provides protection from the sun but allows airflow to the body. Protect your head and shade your eyes if working outdoors.

During the designated warmer months of the year (April through September) all jobsites are required to incorporate heat illness prevention and awareness training into the Tailgate Safety Meetings. Training documentation shall be submitted to Webcor/Obayashi Joint Venture SSM. Shade and plenty of water shall be provided in sufficient amount to each and every employee. Emergency services must be called when an employee(s) experience a heat related illness

### **Drinking Water**

Trade subcontractors shall provide potable drinking water, cups, and trash receptacles for their employees. All trash receptacles shall be properly emptied on a daily basis.

### **Use of Tools and Equipment**

Each trade subcontractor is responsible to provide proper instructions for their employee's use of all tools and equipment. When the use of portable electric or pneumatic tools is needed, proper safety guards must be in place and operational. Power tool cord "whips" must meet NEC requirements. Air compressor hoses must be "clipped" together and tools are not to be raised or lowered by their cords or air hoses.

### **Hazardous Material**

*This Section will conform to Specification Sections 01 13 50 (1.4B and C) and (1.8D) found in The Transbay Transit Center Contract Number 08-04-CMGC-000*

Currently Webcor/Obayashi Joint Venture does not anticipate, based on the scope of work, to have any excavations that will require special protection. In the event the situation does arise, The Trade Subcontractor will submit all appropriate documentation (protections, support systems, inspection process, access) preceding the activity.

### **Hazardous Communications Program**

All subcontractors are to comply with Webcor/Obayashi Joint Venture's Hazard Communication Standard Policy.

If an employee is allergic to cement, or may be susceptible to lime burns, or skin disorders ensure that employees Supervisor is aware and do not assign that employee to tasks associated with those irritants. If an employee is allergic to or cannot use any other chemicals there Supervisor must be notified.

### **Hazard Communication Standard Policy**

*This Section will conform to Specification Sections 01 15 45 (1.2A1, 1.2A2),(1.13D),(1.4A), (1.4C) found in The Transbay Transit Center Contract Number 08-04-CMGC-000*

The TJPA will not review the HASP for its content, nor will the TJPA be liable for Contractor's failure to have an adequate HASP or implement it. Receipt of the HASP by the TJPA neither constitutes the legality of the HASP nor incurs liability with Trade Sub contractor. Noncompliance with this portion of the Webcor/Obayashi Joint Venture Safety Policy will be written up as a Safety violation and may result in a

Safety fine and/or nonpayment to the subcontractor(s). Webcor/Obayashi Joint Venture is only required to train its employees to comply and observe the policy. It is the responsibility of each trade subcontractor and each tiered subcontractor to train their employees in the implementation and use of the Hazard Communication Policy.

Trade Subcontractors shall submit a Health and Safety Plan (HASP) in accordance with this Contract specification. Upon approval of the HASP, Trade Subcontractor shall provide two (2) copies on compact disc in Portable Document Format (PDF) with properly labeled cases. Safety Data Sheet (SDS) (previously known as Materials Safety Data Sheet (MSDS)) for all chemicals and other hazardous materials to be used.

Trade Subcontractors shall submit a site-specific environmental HASP in accordance with these specifications and 29 CFR 1910.120, 8 CCR 5192. The HASP shall remain in effect throughout the life of the Contract, and a copy of the HASP must be on site at all times. Trade subcontractors shall submit five (5) copies of the HASP at least ten (10) working days before any demolition or any building materials-disturbing activity, and no later than thirty (30) days after the Notice to precede for each Trade Subcontract package.

Each subcontractor must submit a copy of its written Hazard Communication Program to the Webcor/Obayashi Joint Venture SSM. An initial hazardous material/chemical listing for this specific jobsite must accompany the written Hazard Communication Program and all trade subcontractors shall maintain their SDS. A complete file of all SDS submitted is to be located at the jobsite office for review by all workers during job hours. Each trade subcontractor will discuss each new substance introduced on the jobsite at the weekly Safety meetings with their crews and the Superintendents of other trade subcontractors at the weekly Subcontractor Meeting. Each trade subcontractor must label the contents of all containers including secondary containers. The label must clearly identify the substance, hazard warnings, the name and address of the manufacturer and the location of the SDS.

Employees are required to be trained in Hazardous Communication, specifically in the dangers of working with these substances, chemicals, materials, required PPE and medical emergency training. Copies of training certificates shall kept on site and be submitted to Webcor/Obayashi Joint Venture SSM.

Bulk fuel storage is not allowed onsite.

## **Confined Space**

No person shall enter a confined space such as manholes, underground vaults, tanks, pipes, tunnels, or other similar places until it is determined that it is safe to enter the space by an approved method. The trade subcontractors Competent Person is responsible for identifying any potential confined space and shall initially determine if a permit required confined space exists. A pre-planning meeting shall be held if a confined space exists and proper procedures shall be followed to ensure worker safety.

When "Hot Work" is performed in Permit Required Confined Spaces, the applicable Standards will be followed for Permit Required Confined Space work.

## **Equipment**

Machinery and equipment shall be inspected and documented daily. Machinery and equipment shall be operated by authorized, trained personnel only. All operated equipment shall have backup alarms in working order. Operators shall inspect each work area to make sure that it is safe to operate the equipment in that area. Equipment shall not be serviced or repaired while it is in motion or running, unless there are appropriate safeguards in place to prevent injury.

Fuel-operated equipment, such as generators, air compressors, welders, etc., shall have a dedicated fire extinguisher near the equipment at all times when it is in operation. Fire extinguisher shall be rated as a minimum of 10lb ABC.

## **Excavation and Trenching**

Currently Webcor/Obayashi Joint Venture does not anticipate based on the scope of work to have any excavations that will require special protection. In the event the situation does arise, The Trade Subcontractor will submit all appropriate documentation (protections, support systems, inspection process, access) preceding the activity.

*This Section will conform to Specification Sections 00 07 00 (I), 00 08 14(1.2B), 00 08 14(1.4), 00 08 14(1.5B) and 01 35 65 (1.7C) found in The Transbay Transit Center Contract Number 08-04-CMGC-000*

Pursuant to section 6705 of the California Labor Code, excavation for trenches five (5) feet or more in depth shall not begin until Webcor/Obayashi Joint Venture has received acceptance from the TJPA of Webcor/Obayashi Joint Venture detailed plan for worker protection from the hazards of cave-in's during excavation of such trenches. Webcor/Obayashi Joint Venture shoring plan shall be submitted in accordance with the requirements of the Specifications and shall show the details and supporting calculations of the design of shoring, bracing, sloping, or other provisions to be made for worker protection during such excavation. No plan shall allow the use of shoring, sloping or other protective system less effective than that required by the Construction Safety Orders of the Division of Occupational Safety and Health. If Webcor/Obayashi Joint Venture shoring plan varies from the shoring system standards established by the Construction Safety Orders, the plan shall be prepared and sealed by an engineer retained by Webcor/Obayashi Joint Venture who is registered as a civil or structural engineer in the State of California. The TJPA acceptance of Webcor/Obayashi Joint Venture shoring plan shall not be construed to relieve Webcor/Obayashi Joint Venture of its responsibility for damage or injuries related to the excavation resulting from unsafe shoring.

The trade subcontractor will comply with all requirements of Federal OSHA, Cal/OSHA, the California Labor Code, Trade Subcontractor safety requirements, and these Contract Documents. The more stringent requirements shall apply. Prior to commence of earthwork activities the trade subcontractor shall review their safety procedures. Trade subcontractors shall submit for approval a comprehensive and site specific Health and Safety Plan (HASP) prepared by a certified Industrial Hygienist. A health and safety plan shall be certified by the trade subcontractor's Competent Hazardous Materials Supervisor and submitted to the TJPA for review and comment prior to implementation. Daily, pre-shift inspection of excavations, the adjacent areas and protective systems shall be made by the Competent Person for evidence of potential cave-ins, hazardous atmospheres or protective system failure. Daily, pre-shift inspection checklists shall be maintained by the subcontractor and submitted to Webcor/Obayashi Joint Venture weekly. No person shall enter an excavation where protection from ground movement is required until such protection is in place. 100% fall prevention and/or protection is required when working next to excavations greater than

five feet (5') in depth. Ladders or other means of approved access shall be used for all excavations. Stepladders shall not be used in a "leaning" position to enter or exit excavations.

Should trade subcontractors be notified by the TJPA of any unsafe or unhealthy condition associated with the performance of the Work and be required to take remedial action to correct such conditions, trade subcontractors shall take action immediately, if so directed, or within 48 hours after receipt of a notice of violation.

## **Respiratory Protection**

Conditions may exist which require the utilization of respiratory equipment to protect employees against exposure to the inhalation of toxic or harmful gasses, vapors, mists, fumes and dust. Each Contractor must implement and enforce a written respiratory program in accordance with CAL/OSHA standards to protect employees from these types of exposures. Trade subcontracts written Respirator Protection programs shall be submitted to Webcor/Obayashi Joint Venture prior to use of respirators.

Only respirators that are applicable and suitable for the purpose intended shall be used. Respirators and cartridges shall be selected on the basis of the hazards to which the employee may be exposed to. Respiratory protective equipment shall be inspected regularly and maintained in good condition. Cartridges shall be replaced per manufacturer's recommended or calculated filter change-out schedule so as to provide complete protection. Respiratory protective equipment, which has been previously used, shall be cleaned and disinfected before it is issued to another employee.

Dust respirators are to be replaced in accordance with manufacturer specifications.

Employee shall be medically evaluated, Fit Tested and properly trained prior to using a respirator. A copy of the employee's medical approval will be kept on site by their employer. Every employee who wears a respirator must be clean-shaven to ensure the proper fitting of the respirator

## **Concrete Code of Safe Practices**

The Concrete Code of Safe Practices is established to assist in conforming to the requirements for all construction activities involving concrete performed on Webcor/Obayashi Joint Venture projects. This includes, but is not limited to cast in place, shoring & reshoring, formwork/false work, post tensioning, placing & finishing.

### **Definitions**

*Bull float* - a tool used to spread out and smooth concrete.

*Formwork* - the total system of support for freshly placed or partially cured concrete, including the mold or sheeting (form) that is in contact with the concrete as well as all supporting members including shores, reshores, hardware, braces, and related hardware.

*Limited access zone* - an area alongside a masonry wall, which is under construction and which is clearly demarcated to limit access by employees.

*Precast concrete* - concrete members (such as walls, panels, slabs, columns, and beams) which have been formed, cast, and cured prior to final placement in a structure.



*Reshoring* - construction operation in which shoring equipment (also called reshores or reshoring equipment) is placed, as the original forms and shores are removed, in order to support partially cured concrete and construction loads.

*Shore* - a supporting member that resists a compressive force imposed by a load.

## **Fall Protection – Concrete Specific**

- Workers working more than six (6) feet above any adjacent working surface or placing reinforcing steel in walls, piers, columns, etc. should be protected by personal fall arrest system, guardrail system or equivalent device.
- Workers inside a Cunningham beam form, where the form leading edge is less than 39" in height and the worker is greater than 6' above a lower working surface, should be protected by a suitable fall protection system consisting of a catenary or similar pendant type line and personal fall arrest system.
- As soon as practical, a perimeter guardrail system should be established.
- Special attention and consideration should be given to workers on ladders within 6' of leading edge such as when working on columns or wall forms. Additional fall protection measures may be required.
- When working on vertical reinforcing steel columns or false work, fall protection should be set in advance from ladders, manually propelled elevated work platforms, or similar means so that 100% fall protection can be utilized.
- Workers on wall forms greater than six (6) feet above any adjacent working surface should be protected from falling by a personal fall arrest system or equivalent system. Ensure appropriate anchorage points are provided and utilized. Where applicable, a two (2) hook system for 100% fall protection should be utilized.
- Workers who are placing or tying reinforcing steel more than six (6) feet above any adjacent working surface should be protected from falling by personal fall arrest system or equivalent system.
- When workers are exposed to falls greater than six (6) feet above any adjacent working surface while erecting or dismantling shoring systems, they should have suitable fall protection as necessary utilize an appropriate anchorage point
- In addition to the above fall protection requirements, when erecting and dismantling shoring, a minimum of two (2) scaffold grade planks should be used or other similar means, such as mobile scaffolding, lifts, etc. Planks should rest on horizontal frame members and not on cross bracing.
- The use of positioning systems as a sole means of fall protection is not permissible.
- Unless otherwise provided by a site specific fall protection plan:
  - The placing of frames and stringers should be from below via appropriate ladders, temporary work platforms, false decks, scaffolds, or other similar work platforms.
  - The first several joists spread should be from below via appropriate ladders, temporary work platforms, false decks, scaffolds, or other similar work platforms. Once the first several joists are positioned, a work platform (e.g. 4x6 sheet of plywood or similar) should be placed on top of a placed joists and all further spreading of joists should take place from this work platform or successive sheets of plywood laid to extend this platform. Work should take place from the center of the bay, with joists spaced no greater than 24" on center. Any work within 6' of the leading edge and greater than 6' above a lower working surface should be protected by a suitable fall protection system.

## **Formwork/False work**

Formwork, false work and shoring should be designed, fabricated, erected, supported, braced and maintained so that it will be capable of supporting without failure all vertical and lateral loads that may reasonably be anticipated to be applied to the formwork. Formwork which is designed, fabricated, erected, supported, braced and maintained in conformance with ANSI A10.9-1983 Construction and Demolition Operations – concrete and masonry work, will be deemed to meet the requirements of this paragraph.

- Drawings or plans, including all revisions, for the jack layout, formwork including shoring equipment, working decks, and scaffolds, should be available at the jobsite.
- Procedures for safe installation, removal, lifting etc., should be available at the jobsite and all workers appropriately trained in these procedures as applicable.
- Work areas should be clear of all unauthorized personnel during installation, concrete placement and removal. Appropriate barricading, delineation and/or signage should be placed to limit access and alert other workers of hazards associated with the work area.
- At no time should workers place themselves underneath a live load.
- When hoisting material, the worker should be positioned to the side of the hoisted material and never into the pinch point between the hoisting equipment and the material or in the area where an operator would land material in the event of an emergency.
- Appropriate tag lines should be utilized as required and two (2) tag lines may be necessary to help align/control panels or forms.
- Safe means of access and egress should be maintained at all times.

## **Removal of Formwork**

Forms and shores (except those used for slabs on grade and slip forms) should not be removed until the employer determines that the concrete has gained sufficient strength to support its weight and superimposed loads. Such determination should be based on compliance with one of the following:

- The plans and specifications stipulate conditions for removal of forms and shores, and such conditions have been followed, or the concrete has been properly tested with an appropriate ASTM standard test method designed to indicate the concrete compressive strength, and the test results indicate that the concrete has gained sufficient strength to support its weight and superimposed loads.
- Prior to dismantling, the entire system should be inspected to determine if there are any hazards from displacement, weakening, alterations etc. of the shoring and false work.
- Shores, cross braces etc. should only be removed in the immediate work areas and as appropriate.
- All nails should be removed or bent over immediately upon stripping.
- Shoring, formwork and all other equipment being removed should be stacked, consolidated or placed in an orderly manner as soon as practicable during the removal operation and egress/access paths maintained at all times.
- Only appropriate tools should be used for removal of shoring and formwork.

## **Shoring and Reshoring**

- All shoring and reshoring operations should comply with all federal, state local and manufacturers regulations.
- All shoring equipment (including equipment used in reshoring operations) should be inspected prior to erection to determine that the equipment meets the requirements specified in the formwork drawings.

- Shoring equipment found to be damaged, severely rusted, missing locking devices etc. should not be used for shoring. Shoring equipment that is in place and is found to be damaged or weakened, should be immediately reinforced.
- Erected shoring equipment should be inspected immediately prior to, during and immediately after concrete placement.
- The sills for shoring should be sound, rigid and capable of carrying the maximum intended load.
- Base plates should be attached to a minimum of 12' square, 2" plywood or equivalent.
- All base plates, shore heads, extension devices, and adjustment screws should be in firm contact, and secured when necessary, with the foundation and the form.
- Existing ground should be level, adequately compacted and loads distributed. Consideration should be given to adverse weather conditions such as washouts, rain impact to slopes etc. Special precautions such as hardwood wedges or bracing should be utilized on sloped surfaces.
- All clamps, screws, pins and other similar components should be in a closed or engaged position.
- Eccentric loads on shore heads and similar members are prohibited unless these members have been designed for such loading.
  - Ensure stringers are centered on these members to minimize eccentric loading.
- Adequate access should be provided to all form deck surfaces.
- When horizontal shoring is required, these should be engineered and special consideration should be given to installation and conformance to the completed design.
- Ensure all stringers and joists are fully supported and centered over shoring heads/top plates and adequately secured. Further, ensure that all stringers and joists are fully upright and not rolled.
- All horizontal shoring should be installed and erected in compliance with manufacture's requirements as well as federal, state and local regulations.

### **Frame Shoring**

- The design of the shoring should be prepared by a qualified designer and the erected shoring should be inspected by an engineer qualified in structural design.
- The shoring design or layout drawing should be followed with no omissions of required components, or alteration in frame spacing's, types used, towers heights, locations or sizes.
- Shoring loads should be carried on all legs.
- All shoring frames should be plumb and level. This should be checked and corrected at a minimum of during erection and just prior to the pour.
- Adjustment of shoring frames should not be made once the pour begins.
- When shoring height exceeds a minimum of four (4) times the minimum base width, additional bracing and securing of the frames should be performed.
- Cross braces should never be climbed, workers should climb frames from the inside.

### **Screw Jacks**

Screw jacks should not exceed the manufactures recommended extension height at any time. Screw jack extension should be kept to a minimum for maximum load carrying capacity. All screw jacks should be in firm contact with the foundation and frame legs.

### **Post Shoring**

- The single post shores should be vertically aligned/plumbed.
  - This should be checked and corrected at a minimum of during erection and just prior to the pour.

- Adjustment of post shores for any reason should not be made once the pour begins.
- Refer to the manufacture's guidelines for additional stability measures and bracing requirements of each system used.
- Post shores should be adequately secured at top and bottom to prevent displacement.
- Whenever single post shores are used one on top of the other (tiered), they should comply with the following specific guidelines in addition to the general guidelines for formwork:
  - The single post shores should be spliced to prevent misalignment.
  - The single post shores should be adequately braced in two (2) mutually perpendicular directions at the splice level.
  - Each tier should also be diagonally braced in the same two (2) directions.

### **Ellis Shores**

- Ensure shores are erected with the proper length of timbers allowing a minimum of 24" overlap between shore members.
- The shore clamps should be attached 12" apart with the upper clam at a minimum of 2" from the top of the lower shore. Each clamp should be secured with the appropriate number of type of duplex nails.
- Shores should be raised to the desired height by sliding the upper shore member upwards being careful to avoid pinch points.
- Shore hand jacks should not be used to raise decks, lift formwork or elevate concrete.
- Ensure all shores, jacks and clamps are inspected prior to use and any damaged or defective materials are removed or repaired prior to use.
- Safety nails should be secured above each clamp of the upper shore member casting to prevent uplift or movement during vibration.

### **Re-shoring**

- Shores should not be removed, including cross bracing, until the concrete has gained sufficient strength to support its weight and superimposed loads. Such determination shall be based on compliance with one of the following:
- The plans and specifications stipulate conditions for removal of forms and shores, and such conditions have been followed or the concrete has been properly tested with an appropriate ASTM standard test method designed to indicate the concrete compressive strength, and test results indicate that the concrete has gained sufficient strength to support its weight and superimposed loads.
- Stripping and removal of shoring equipment should be performed in conformance to the approved stripping sequencing plan.
- Re-shoring should be erected, as the original forms and shores are removed, whenever the concrete is required to support loads in excess of its capacity.
- The design of the shoring should be prepared by a qualified designer and the erected shoring should be inspected by an engineer qualified in structural design.
- The shoring design or layout drawing should be followed with no omissions of required components, or alterations in spacing's, types used, heights, locations or sizes.
- Re-shores should be placed directly below load carrying legs to avoid punch through, stress reversals or other undesirable forces on the poured concrete.
- Slabs or beams should be allowed to take their permanent deflection before final adjustment of re-shoring equipment is made.
- Horizontal shoring should never be used as part of a re-shoring system.

## **Bracket Scaffolds**

- Bracket scaffolds should only be used through bolted walls, with at least 5/8" diameter bolts.
- Scaffolds should be solidly secured to the walls or the supporting structure.
- Scaffolds should be able to support at least four (4) times the maximum intended working load.
- Spacing of brackets should not be greater than 10' apart.
- Railings should be installed on all scaffolds 6' or greater in height.
- Platforms should consist of at least two 2"x10" planks that extend at least 6" over each bracket and no more than 18".
- Platforms should be solidly planked with no more than 7" gap under the back rail and 14" gap to the face of the form.
- Planking should be scaffold grade lumber or equivalent and should be free from damage, defects, cracks, splits etc. Damaged planks should not be used.

## **Reinforcing Steel**

All protruding reinforcing steel, onto and into which employees could fall, should be guarded to eliminate the hazard of impalement. When working at grade, impalement hazards from 4" to 6' in height, at a minimum, should be protected. Reinforcing steel for walls, piers, columns, and similar vertical structures should be adequately supported to prevent overturning and to prevent collapse. Employers should take measures to prevent unrolled wire mesh from recoiling. Such measures may include by are not limited to securing each end of the roll or turning over the roll. Reinforcing steel should be stockpiled as close as practicable to work areas. Additionally special attention should be taken towards access and egress to work areas, excavations and ensuring work areas are free from tripping hazards or other surface encumbrances.

## **Concrete Placement and Finishing**

Appropriate PPE should be utilized during concrete placement and finishing. This includes but is not limited to safety glasses, fall protection, gloves, boots, hardhat, and long sleeves. Appropriate respiratory protection should be used for all concrete cutting, grinding, sanding, and blasting, dry mixing, jack hammering etc.

The following should be observed while working with concrete:

- When discharging concrete on a slope, the wheels of ready-mix trucks should be blocked, the brakes set to prevent movement and the operator with the vehicle at all times.
- All washout activities should be completed in the designated washout area.
- All concrete cutting, finishing and cleanup should be done in such a manner that all residue or waste water will be properly contained and disposed of.
- Appropriate precautions should be taken for specialty applications (e.g. acid washing, dyes, stains etc.); in their handling, storage use and disposal.
- Powered and rotating type concrete troweling machines that are manually guided should be equipped with a control switch that will automatically shut off the power whenever the hands of the operator are removed from the equipment handles.
- Bull float handles used where they might contact energized electrical conductors, should be constructed of nonconductive material or insulated with nonconductive sheath that's electrical and mechanical characteristics provide the equivalent protection of a handle constructed of nonconductive material.

- Masonry saws should be guarded with a semicircular enclosure over the blade.
- When operation air guns for cleaning off decks, inside forms etc., these guns should have a maximum of 30 psi nozzle pressure and be equipped with a safety release valve.
- Air guns should have pressure valves, and extension tube and the hoses well maintained with appropriate whip checks.
- Employee operating air guns should have appropriate PPE, including but not limited to, chip protection (i.e. face shield, goggles etc.), ear plugs and respiratory protection as required.
- No employee should be permitted to perform maintenance or repair activity on equipment (such as compressors mixers, screens, pumps used for concrete and masonry construction activities) where the inadvertent operation of the equipment could occur and cause injury, unless all potentially hazardous energy sources have been locked out and tagged.

## **Concrete Buckets**

No employee shall be permitted to ride concrete buckets or work under concrete buckets while buckets are being elevated or lowered into position. Elevated concrete buckets shall be routed so that no employee or the fewest number of employees are exposed to the hazards associated with falling concrete or falling buckets. Concrete buckets equipped with hydraulic or pneumatic gates should have positive safety latches or similar safety devices installed to prevent premature or accidental dumping. Concrete buckets should be designed to prevent concrete from hanging up on top of the sides.

## **Pump-Crete Systems**

Concrete pumping systems using discharge pipes should be provided with pipe supports designed for 100% overload. Compressed air hoses used on concrete pumping systems should be provided with positive failsafe joint connectors to prevent separation of sections when pressurized. Movement of concrete hoses should be planned to limit the amount of manual positioning of hose as much as practicable. When necessary, the use of hooks, ropes or other similar devices should be utilized when handling the concrete hose.

## **Buggies & Wheelbarrows**

Concrete buggy handles should not extend beyond the wheels on either side of the buggy. Handles should be guarded or equipped with knuckle guards. All buggies, wheelbarrows or other similar conveyances should be properly maintained and repaired/replaced immediately if damaged, in poor repair or otherwise. Paths of access and travel should be level, free of debris and other surface encumbrances and ramps or other access ways should be appropriately built, maintained, and protected. Buggies, wheelbarrows etc. should not be overloaded.

## **Post-Tensioning Operations**

No employee (except those essential to the post-tensioning operations) should be permitted to be behind the jack during post-tensioning operations. Signs and barriers should be erected to limit employee access to the post-tensioning area during tensioning operations. Appropriate fire protection measures should be taken during burning operations, including but not limited to spark control or blankets, fire extinguishers, wetting formwork etc.

## **Permitting/Documentation**

Before a contractor is on site, the following items should be obtained in writing. A permit for excavation/trenching activities (Cal OSHA Excavation Notification Form as applicable) for all trenches/excavations that are equal to or greater than 5' in depth where an employee is required to enter as

well as a permit for any false work or scaffolding 36' in height or greater total. Excavation and trenching plan, shoring/false work design or plan needs to also be submitted to Webcor/Obayashi Joint Venture in writing. Name(s) of competent person(s), soils analysis report and a copy of the trade subcontractor's safety manual are also required prior to work.

### **General Rigging Equipment Safety:**

Inspect rigging equipment for material handling prior to use on each shift and as necessary during its use to ensure that it is safe. Remove defective rigging equipment from service.

Never load rigging equipment in excess of its recommended safe working load.

Remove rigging equipment when not in use from the immediate work area so as not to present a hazard to employees.

Mark special rigging accessories (i.e., spreader bars, grabs, hooks, clamps, etc.) or other lifting accessories with the rated capacity. Proof tests all components to 125% of the rated load prior to the first use. Maintain permanent records on the job site for all special rigging accessories.

## **Asbestos Abatement Program**

Products that contain Asbestos can be helpful, but they can also be very harmful. Asbestos is a mineral which has many positive qualities. It is fireproof, heat resistant, lightweight, resistant to most chemicals, sound-absorbing and it does not conduct electricity. Asbestos has been used to mix with plaster and wallboard for strength and support, sprayed onto wall, ceilings, and steel girders for fireproofing, wrapped around pipes, boilers and heating ducts for insulation, in floor and ceiling tiles among others. Asbestos can break down into tiny fibers, like grains of sand or rope and can float in the air for long periods of time, allowing them to be easily inhaled. A powerful microscope is needed to see the fibers since they are invisible to the human eye, they have the strength of steel, and one cannot taste or smell them. Asbestos material that a worker may encounter generally fit into two (2) categories: Friable and Non-Friable. Friable asbestos is air born, thin, easily damaged or broken asbestos and is most dangerous to human's respiratory system. Non-friable is asbestos that is not damaged, a complete piece. The three most common materials that contain asbestos are thermal system insulation, floor tiles and sprayed-on materials. Thermal system insulation is the most common type of friable asbestos material, and can be found on boilers, utility pipes, ductwork and heating systems.

This Asbestos Abatement Program is developed to inform workers who don't really work directly with asbestos, but who may have incidental exposure, must receive at least "Asbestos Awareness" training. To help address OSHA's concerns, and provide the awareness training needed by employees under the regulation, employees shall be trained, understand monitoring activities and how to protect against potential asbestos exposure. Employees should understand how long-term exposure to asbestos can harm the human body as well as understand how to avoid potentially hazardous maintenance and custodial activities that could lead to asbestos exposure since custodians, engineers and maintenance workers have the highest chance of exposure to asbestos. Employees should understand which safe work practices should be used when helping with a minor asbestos clean-up and understand why and when there is a potential for exposure to asbestos. Air monitoring and medical surveillance can be important elements in providing a safer workplace.

Exposure to asbestos fibers can lead to a disease known as "Mesothelioma." Mesothelioma is a chronic disease, occurs over time. There is rarely acute side effects when a worker is exposed to asbestos. Symptoms of asbestos exposure may include shortness of breath, enlargement of the heart, scarring of the

lungs, cancer and death. People who smoke are especially vulnerable to Asbestos. Cigarette smoke breaks down the lungs' defensive system, and leaves them vulnerable to Asbestos fibers. Smokers are over 50 times more likely to become sick after long-term exposure to Asbestos.

While working with material that has or potentially has asbestos requires safe handling and proper PPE. Even a small tear in asbestos material can cause serious harm. If an employee suspects a piece of asbestos material is damaged their supervisor shall be notified immediately and secure measures shall be taken to ensure minimal exposure. These measure may include securing the material in a plastic bag secured with duct tape and wetting down the immediate area to ensure the material does not become friable.

## **Proper PPE**

Although asbestos is not a skin contact hazard, by wearing disposable overalls helps reduce the potential of transferring asbestos from the work area to non-contaminated areas.

A respirator and designated filters shall be required to reduce the potential of introducing asbestos fibers into the lungs. A fit test and medical evaluation shall be conducted prior to an employee donning a respirator. The respirator must be the right size and securely fit a clean shaved face. Respirators shall be cleaned and stored as recommended by an Industrial Hygienist.

An Air Sampling Device may, at times, be worn by the employee to measure airborne concentrations of asbestos in the work are. The Air Sampling Device varies in design and appearance, however does include an air pump located near the employees face and a sampling cassette that is secured onto the employee. An Industrial Hygienist will instruct the employee in further details regarding the use of an Air Sampling Device.

## **Medical Surveillance**

A Medical Surveillance program is put in place to monitor employees since asbestos causes chronic illnesses. The program tests the workers lung capacity and x-rays the chest cavity and lungs for any previous damage and to record current conditions. The worker may be asked to return for continued surveying depending on their potential exposure. The Medical Surveillance costs are that of the employer, free service to the employee.

## **Clean up Methods**

The Asbestos Awareness Program is designed to make workers aware of the health hazards, locations and minor cleanup of asbestos, this program does not include Asbestos Work. Large quantities of asbestos required more detail and training than what is provided here. However, if a minor cleanup or containment is required follow these basic steps:

- Proper PPE: respirator, gloves, Tyvek body suit. Don and Done PPE properly
- Appropriate work area: the contaminated area is guarded with access available through the decontaminated area and final access to the non-contaminated area.
- Equipment: wet methods, HEPA vacuum shall be used. Low Abrasion Pad, at speeds less than 300rpm are acceptable.
  - Do not sweep or shovel material contain asbestos.
- Disposal
  - Asbestos materials must be properly bagged and labeled.
    - Use only official Asbestos Disposal Bags.
    - Use a Generator Label which lists the name and address of your facility.



- If an Asbestos Disposal Bag becomes torn, double bag and seal it immediately with tape.
- Asbestos is a regulated waste (it must be hauled to a licensed landfill).

## **Decontamination**

After any work with Asbestos materials, workers must decontaminate themselves and their equipment. This prevents the spread of Asbestos dust and debris. Always use an official decontamination area that is equipped with a HEPA vacuum, as well as a plastic drop cloth (to contain any loose fibers). Never eat, drink or smoke in these decontamination areas, or any other area where asbestos is present. Scrub hands and face with soap and water before leaving work. If possible, shower before leaving your facility as well, if not, instruct the worker to shower immediately when they get home to prevent potential exposure to others. When decontaminating clothing, never brush off dust or debris because asbestos fibers may become airborne. Use a HEPA vacuum to remove materials from clothing before taking it off. Also vacuum equipment and Asbestos Disposal Bags. Tyvek suits will need to be disposed of in an Asbestos Disposal Bag and disposed of as regulated waste.

## **Lead Abatement Program**

This program has been put in place because Webcor/Obayashi Joint Venture recognizes that some of the work we do has the potential to expose our employees to lead. We want to do as much as is practically possible to protect them from lead exposure.

Prior to the start of a project, professionals/Industrial Hygienist in lead detection and abatement will be brought in to do an Exposure Assessment to determine whether the work environments Webcor/Obayashi Joint Venture employees will be operating in has the potential to expose workers to lead. These professionals will be used to give Webcor/Obayashi Joint Venture direction as to how to proceed. It will be our goal to have lead abatement taken care of by licensed lead abatement professionals prior to the arrival of Webcor/Obayashi Joint Venture employees.

Lead can be found in a number of workplace environments. Until recently, lead was a common component in paints of all kinds (which can create exposure whenever sanding, sandblasting, scraping, or even demolition occurs).

Workplace experience and empirical studies have shown that lead is fairly easily absorbed into the body. Breathing airborne lead dust and fumes is the most common route of entry. Lead can also be absorbed if it comes into contact with the mouth or tongue.

Overexposure to lead can occur both on an acute basis, where large amounts of lead are absorbed into the body in a short period of time, or on a chronic basis where small amounts of lead are absorbed at any one time, for a long period eventually accumulating to cause significant health problems.

On May 4, 1993, OSHA published the Interim Final Rule for Lead Exposure in Construction. The Construction Standard establishes “Interim” procedures and work practices that must be followed in construction environments. The OSHA Standard and its compliance requirements are included at the end of this written program. The Lead Standards are “performance based”; the standard will tell you what you have to accomplish.

A General Requirement in the Lead Standards states employers must make sure that no employee is exposed to lead concentrations greater than 50 micrograms per cubic meter of air, averaged over an eight-hour period in any 24-hour day.

Typically, OSHA requires that you use the following methods to protect your employees through engineering controls, work-practice controls, respiratory protection, PPE, hygiene facilities and practices, housekeeping and employee information and training.

OSHA requires that every employer who is covered by these Standards provide information and training. For employers in the Construction Industry, it requires that they meet the training requirements of the Hazard Communication Standard. Information that must be given employees under the Hazard Communication Standard includes the hazards associated with lead exposure, warning signs and labels that can be found on materials containing lead, and how to find information about materials containing lead on Safety Data Sheet (SDS), and use of PPE.

## Respiratory Protection Program

The purpose of this plan is to establish a program and procedures for wearing respiratory protection at the Transbay Transit Center. This program supports compliance with the Occupational Safety and Health Administration Respiratory Protection Standard as found in 29 CFR 1910.134. This program applies to all company employees who work in areas whose exposures to airborne contaminants require the use of respirators.

### Definitions

*Dusts:* Particles released during work operations such as grinding and sawing.

*Fit Testing:* The process of making sure that an employee's respirator fits properly and will provide the necessary protection without any leaks.

*Fumes:* Vaporized, condensed metals such as lead that may be present during welding operations.

*Gases:* Examples include nitrogen, methane, and carbon monoxide.

*IDLH:* An OSHA hazard classification: Immediately Dangerous to Life & Health. An atmospheric condition that poses an immediate hazard to life or poses immediate irreversible debilitating effects on health.

*Mists:* Particles of liquid released during operations such as spray painting.

*NIOSH:* National Institute for Occupational Safety and Health; an agency that establishes minimum performance standards for respirators and tests and approves respirators for various uses.

*Vapors:* Gaseous forms of a liquid such as paint solvents.

## **Responsibilities**

### **Program Administrator**

The Program Administrator is responsible for issuing and administering this program and making sure that the program satisfies the requirements of all applicable federal, state, or local respiratory protection requirements. Providing initial and periodic training to employees on respiratory protection requirements. Conducting hazard assessments where respiratory hazards may be present. Assisting managers and supervisors in the selection of appropriate respiratory protection for use on their jobsites. Auditing the respiratory protection program to ensure its continued effectiveness.

### **Purchasing Agent**

The Purchasing Agent will be the Jobsite Superintendent and is responsible for purchasing respiratory protection equipment and assuring that all equipment purchased is approved by NIOSH/MSHA.

### **Superintendent**

Superintendents whose jobsites are required to wear respiratory equipment is responsible for knowing the hazards in their areas that require respiratory protection, knowing the types of respirators that need to be used, enforcing the wearing of respiratory protection in the areas where it is required, making sure employees are knowledgeable about the respiratory requirements for the areas in which they work and providing training on hazardous chemicals to employees.

### **Employees**

Employees who are required to wear respirator protections is responsible for wearing appropriate respiratory protection, properly maintaining their respiratory protection equipment and keeping it in a clean and operable condition and notifying their Supervisor of any additional hazards.

## **Program Activities**

Respiratory hazards will be assessed on the jobsite and appropriate protection will be provided for all affected employees. Employees are required to wear respiratory protection wherever respiratory hazards exist. Respiratory protection is stored and issued from the jobsite office. Efforts will be made to minimize the use of hazardous chemicals in the workplace. If the use of hazardous chemicals creates an imminent-danger situation, the operation will be discontinued.

## **Respirators**

Respirators will be selected according to the type of activity for which they will be used and the type of potential air contaminants associated with these activities. Only NIOSH/MSHA approved respirators will be used. All respirator protection equipment will be used in accordance with the manufacturer's recommendations. In areas in which maintenance and sanitation services are unavailable or respiratory usage is limited, disposable respirators will be used. Non-disposable respirators which are used exclusively by one person will be maintained and cared for by the wearer. All non-disposable respirators which are used by more than one person will be cleaned and sanitized between each use. Chemical cartridge respirators will be stored in airtight, labeled containers between each use. All other respirators will be stored in a clean and sanitary manner and labeled with the wearer's name. Disposable respirators will be used until the cartridge or filter media requires replacement or when the face piece is dirty.

Respirators will be inspected by the wearer prior to each use. Supervisors on jobsites where respirators are used will verify that appropriate respirator protection is being used, inspected, and maintained properly. Non disposable respirators will be inspected according to the manufacturer's instructions.

All users of respirators will be fit tested to ensure a proper face piece-to-face seal. Employees whose facial hair interferes with the face piece-to-face seal will not be allowed to wear negative-pressure air-purifying respirators.

All employees who are required to wear respirators will receive training in their use, selection and appropriate maintenance. Training will provide an opportunity for the employee to handle the respirator, have it fitted properly, test the face piece-to-face seal, wear it in normal air, and wear it in a test atmosphere.

## **Silica Exposure Program**

The purpose of this policy is to establish procedures to protect employees from the health hazards associated with exposure to airborne crystalline silica generated by various construction activities. Due to the amount of work we do with concrete and masonry on almost any project; our workers have the potential for silica exposures through abrasive blasting, chipping, hammering, sawing, grinding or demolition of concrete.

Silicosis is a lung disease marked by hardening of lung tissue and symptoms such as shortness of breath, possible fever, fatigue and eventual respiratory failure. Silicosis also renders a person more susceptible to disease of the lungs, such as tuberculosis. Where there is concrete, there is a potential silica exposure so it is essential to monitor our work activities and take the necessary corrective actions to protect our employees.

### **Responsibilities**

#### **Supervisor**

Project Supervision shall evaluate all work activities for silica exposures, institute engineering controls as a first line of protection to reduce silica exposures, institute all administrative/work practice controls to reduce silica exposures when feasible and when engineering controls have been explored and ruled out. Institute the use of respirators to reduce exposures when the above mentioned controls fail to reduce silica exposure levels, provide training identified in this policy when employees are exposed to silica hazards and provide necessary respirator protection as well as training in its proper use, when deemed necessary.

#### **Employees**

The workers shall follow all work plans that identify engineering and administrative work practice controls to reduce their exposure to crystalline silica. They will wear respiratory protection to reduce their exposure to crystalline silica when deemed necessary by their supervisor and not eat, drink, use tobacco products or apply cosmetics in areas where there is dust containing crystalline silica.

### **Program Activities**

Crystalline silica exposures must be maintained below the OSHA PEL of 10mg/m<sup>3</sup> Percentage Quartz +2. Historical data from similar operations producing silica exposure can be used as exposure monitoring when feasible. Assessment of worker exposure to respirable crystalline silica dust during various tasks associated with concrete finishing and demolition activities is performed annually by an Industrial Hygienist. Specific job tasks monitored include grinding, patching, chipping, demolition, segregation, stockpile, and loading of concrete rubble.

When it has been determined that employees will be exposed to crystalline silica in excess of the PEL, engineering controls will be used as a first line of defense. Engineering controls include, but are not limited to the use of dust collection systems which are available for many dust generating tools and equipment, using wet methods to keep dust particles down, use abrasives with a low silica or no silica content or using local exhaust ventilation to prevent dust from being released into the air. When engineering controls cannot be utilized or are not effective to sufficiently reduce exposure to the inhalation of silica, administrative controls will be used when feasible to reduce the time of exposure for the employees where work crews are of sufficient size, the pool of workers skilled in the operation of applicable tools, and job duration is sufficient to accommodate worker rotation, develop a program to reduce the exposure time of individual workers to silica.

Work tasks that must be monitored for crystalline silica exposure include by are not limited to:

- Jack hammering and chipping
- Grinding concrete
- Tunneling
- Sandblasting
- Dry sweeping or blowing concrete debris, sand or rock dust
- Demolition of concrete/masonry structures
- Crushing, loading, dumping rock or concrete
- Saw cutting concrete or rock

## **Respirators**

Respirators will be selected according to the type of activity for which they will be used and the type of potential air contaminants associated with these activities. Only NIOSH/MSHA approved respirators will be used. All respirator protection equipment will be used in accordance with the manufacturer's recommendations. In areas in which maintenance and sanitation services are unavailable or respiratory usage is limited, disposable respirators will be used. Non-disposable respirators which are used exclusively by one person will be maintained and cared for by the wearer. All non-disposable respirators which are used by more than one person will be cleaned and sanitized between each use. Chemical cartridge respirators will be stored in airtight, labeled containers between each use. All other respirators will be stored in a clean and sanitary manner and labeled with the wearer's name. Disposable respirators will be used until the cartridge or filter media requires replacement or when the face piece is dirty.

Respirators will be inspected by the wearer prior to each use. Supervisors on jobsites where respirators are used will verify that appropriate respirator protection is being used, inspected, and maintained properly. Non disposable respirators will be inspected according to the manufacturer's instructions.

All users of respirators will be fit tested to ensure a proper face piece-to-face seal. Employees whose facial hair interferes with the face piece-to-face seal will not be allowed to wear negative-pressure air-purifying respirators.

All employees who are required to wear respirators will receive training in their use, selection and appropriate maintenance. Training will provide an opportunity for the employee to handle the respirator, have it fitted properly, test the face piece-to-face seal, wear it in normal air, and wear it in a test atmosphere.

Select respirators based on the criteria identified in the respirator protection section of this manual.

## **Air Monitoring**

After the initial assessment and institution of exposure controls, follow-up air monitoring will be conducted to assess the effectiveness of the controls put in place. In the event that the follow-up monitoring reflects that instituted controls have not yet reduced employee exposures, the operations will cease, be re-evaluated and alternative controls will be explored to reduce employee exposures to silica.

## **Training**

Employees will be trained in the hazards of silica exposure, engineering and administrative/work practice controls, if any, that have been instituted to control silica exposures and PPE.

## **Appendix**

Figure 1	JHA
Figure 2	Incident Package
Figure 3	DIA
Figure 4	Notice of EHS Non-Compliance Warning Letter of EHS Non-Compliance Written Notice of Temporary Job Suspension



## TRANSBAY TRANSIT CENTER

**TG16.0 EXHIBIT I SCHEDULE**

[illegible]



## TRANSBAY TRANSIT CENTER

**TG16.0 EXHIBIT I SCHEDULE**

[illegible]

## TRANSBAY TRANSIT CENTER

**TG16.0 EXHIBIT I SCHEDULE**

[illegible]

# Exhibit J



Reviewed by Webcor/Obayashi

1751 Harbor Bay Parkway Ste. 200  
Alameda, CA 94502

Review is for general coordination and conformance with design intent only and for submittal in accordance with the contract documents. Review by Webcor Builders does not relieve the subcontractor and/or supplier of responsibility for full coordination, accurate dimensions, correct quantities and full compliance with the contract documents. In the event subcontractor and/or supplier intends to propose any substitution or deviation to the contract documents, each substitution or deviation must be submitted and approved prior to submitting it in a shop drawing or other submittal. Review by Webcor does not imply acceptance of any substitution or deviation.

Submittal Pkg. Number: WO-CQC0001.10

Submittal Number: WO0000-011400W01.10

Webcor Job No.: 30100 Transbay Transit Center

Reviewed By: Jackson Tukuafu

Date: 11/12/2013

Subcontractor: WOJV

WOJV Cycle of Submittal: 10

7/12 V1.4



## Transbay Transit Center

## Webcor/Obayashi Joint Venture

## Contractor Quality Control Plan

## for the

## Transbay Transit Center Project

## November 04, 2013

REV. 10

WO0000-011400W01.10

## REVISION LOG

- REVISION 0: SUBMITTED 10/07/2010 – REVISE AND RESUBMIT 10/29/2010
- REVISION 1: SUBMITTED 11/03/2010 – REJECTED 11/19/2010
- REVISION 2: SUBMITTED 01/04/2011 – REJECTED 01/13/2011
- REVISION 3: SUBMITTED 03/09/2011 – MAKE CORRECTIONS NOTED 12/21/2011
- REVISION 4: SUBMITTED 12/09/2011 – MAKE CORRECTIONS NOTED 2/23/2012
- REVISION 5: SUBMITTED 05/07/2012 – REVISE & RESUBMIT 06/01/2012
- REVISION 6: SUBMITTED 08/02/2012 – REVISE & RESUBMIT 08/27/2012
- REVISION 7: SUBMITTED 08/27/2012 – NO EXCEPTIONS TAKEN 02/14/2013
- REVISION 8: SUBMITTED 03/21/2013 – NO EXCEPTIONS TAKEN 04/17/2013
- REVISION 9: SUBMITTED 08/30/2013 – REVISE & RESUBMIT 10/04/2013
- REVISION 10: SUBMITTED 11/04/2013

**WEBCOR/OBIYASHI JOINT VENTURE  
CONTRACTOR QUALITY CONTROL PLAN  
TRANSBAY TRANSIT CENTER PROJECT**

- 1.0 ELEMENT 1: MANAGEMENT RESPONSIBILITY**
- 2.0 ELEMENT 2: DOCUMENTED QUALITY MANAGEMENT SYSTEM**
- 3.0 ELEMENT 3: DESIGN CONTROL**
- 4.0 ELEMENT 4: DOCUMENT CONTROL**
- 5.0 ELEMENT 5: PURCHASING**
- 6.0 ELEMENT 6: PRODUCT IDENTIFICATION AND TRACEABILITY**
- 7.0 ELEMENT 7: PROCESS CONTROL**
- 8.0 ELEMENT 8: INSPECTION AND TESTING**
- 9.0 ELEMENT 9: INSPECTION, MEASURING, AND TEST EQUIPMENT**
- 10.0 ELEMENT 10: INSPECTION, TEST & OPERATION STATUS**
- 11.0 ELEMENT 11: NONCONFORMANCE**
- 12.0 ELEMENT 12: CORRECTIVE ACTION**
- 13.0 ELEMENT 13: QUALITY RECORDS**
- 14.0 ELEMENT 14: QUALITY AUDITS**
- 15.0 ELEMENT 15: TRAINING**

This Webcor/Obayashi JV Contractor Quality Control Plan will be developed incrementally as the trade packages are awarded and trade subcontractors are brought on board. Each trade subcontractors QC plan will become part of the Webcor/Obayashi JV's overall Contractor's Quality Control Plan and will be submitted to the Transbay Joint Power Authority as they are received.

## **1.0** ELEMENT 1 **MANAGEMENT RESPONSIBILITY**

- 1.1** INTRODUCTION PLAN
- 1.2** FEDERAL TRANSIT ADMINISTRATION GUIDELINES
- 1.3** MANAGEMENT RESPONSIBILITY
- 1.4** PROJECT EXECUTIVE QUALITY RESPONSIBILITY
- 1.5** CQC ORGANIZATION CHART

## 1.0 MANAGEMENT RESPONSIBILITY

### 1.1 INTRODUCTION PLAN

Project quality is the responsibility of all members of the project team and starts at the highest level of management. This Quality Control Management Plan details the specific processes by which the Project's quality will be managed and forms the basis upon which Webcor/Obayashi JV will ensure that all quality policy requirements for the Transbay Transit Center are compliant, maintained and continually being evaluated and improved. This Plan integrates the quality management process into the Webcor/Obayashi JV organizational structure and construction management systems.

Key elements of this plan include:

- The commitment of the Webcor/Obayashi JV Senior management to delivering a project that meets the Transbay Transit Center Quality Management System Manual.
- Accepted project specific construction management policies, procedures and tools for the control of project information and the management of the construction documents, submittals and the work of the trade subcontractors.
- A Webcor/Obayashi JV project-specific quality plan that meets the TJPA and FTA quality requirements and contract requirements.
- Trade Subcontractor, site specific, quality plans that meet TJPA and FTA quality requirements and contract requirements.
- Consistent CQC staff oversight- the Webcor/Obayashi JV CQC Manager and the Trade Subcontractors CQC Managers will have a physical presence on site when work is in progress.

### 1.2 FEDERAL TRANSIT ADMINISTRATION GUIDELINES

The Webcor/Obayashi JV Contractor Quality Control Plan incorporates all 15 Essential Elements of the Federal Transit Administrations Quality Assurance and Quality Control Guidelines dated December 2012 as appropriate for Webcor/Obayashi's scope of work:

1. *Management responsibility*
2. *Documented quality management system*



3. *Design control*
4. *Document control*
5. *Purchasing*
6. *Product identification and traceability*
7. *Process control*
8. *Inspection and testing*
9. *Inspection, measuring and test equipment*
10. *Inspection, test and operating status*
11. *Nonconformance*
12. *Corrective action*
13. *Quality records*
14. *Quality audits*
15. *Training*

### **1.3** MANAGEMENT RESPONSIBILITIES

Webcor/Obayashi JV fully integrates this quality management plan into the organizational structure and performance management systems of the project.

- Maintain and follow a documented Quality System consisting of this Site Specific Quality Manual with policies and procedures.
- Establish and implement project management procedures.
- Maintain Quality System documents and records.

### **1.4** PROJECT EXECUTIVE QUALITY RESPONSIBILITIES

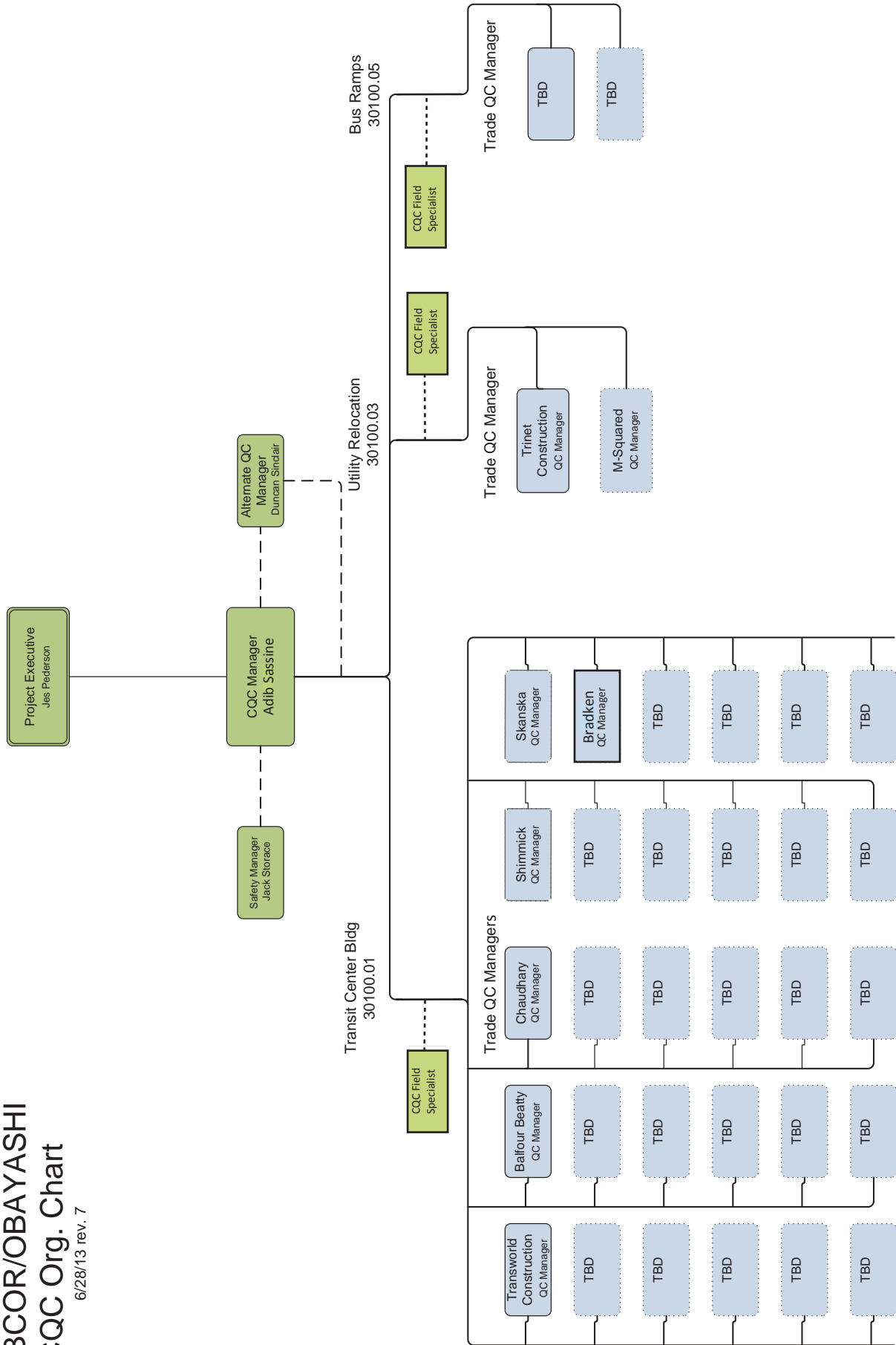
The Project Executive of Webcor/Obayashi JV is the one person in the company ultimately responsible for quality control function. Regardless of other duties, quality responsibilities of the Project Executive include:

- Empower the Webcor/Obayashi JV Transbay Transit Center CQC Manager to perform the CQC duties described in the contract documents.
- Oversee the projects quality plan and objectives.
- Ensure the availability of necessary resources and information for effective operation of the CQC System.
- Provide active oversight of the Trade Contractors Quality Control Plans



# WEBCOR/OBAYASHI CQC Org. Chart

6/28/13 rev. 7



## **2.0 ELEMENT 2 DOCUMENTED QUALITY MANAGEMENT SYSTEM**

- 2.1 INTRODUCTION**
- 2.2 CQC OVERVIEW**
- 2.3 THREE PHASES OF CONTROL**
- 2.4 TRADE SUBCONTRACTORS QUALITY CONTROL PLAN**
- 2.5 WEBCOR/OBIYASHI JV CQC MANAGER DUTIES & RESPONSIBILITIES**
- 2.6 WEBCOR/OBIYASHI JV ALTERNATE CQC MANAGER DUTIES AND RESPONSIBILITIES**
- 2.7 TRADE SUBCONTRACTOR'S QC MANAGER DUTIES AND RESPONSIBILITIES**
- 2.8 QC SPECIALISTS RESPONSIBILITIES**
- 2.9 APPOINTMENT LETTERS, RESUMES AND QUALIFICATIONS**
  - A. WEBCOR/OBIYASHI JV CQC MANAGER APPOINTMENT LETTER
  - B. WEBCOR/OBIYASHI JV ALTERNATE CQC MANAGER APPOINTMENT LETTER
  - C. CQC MANAGER RESUME
  - D. ALTERNATE CQC MANAGER RESUME
  - E. QC SPECIALIST QUALIFICATIONS
- 2.10 TRADE SUBCONTRACTORS QUALITY CONTROL MEETING**
- 2.11 DEFINITIONS**
- 2.12 LIST OF TRADE SUBCONTRACTORS DFOW'S**
- 2.13 PREPARATORY PHASE CHECK LIST FORM**
- 2.14 INITIAL PHASE CHECKLIST FORM**

## **2.0 DOCUMENT QUALITY MANAGEMENT SYSTEM**

### **2.1 INTRODUCTION**

Webcor/Obayashi JV is responsible for developing and maintaining attached written procedures and instructions regularly for activities affecting quality in design, procurement manufacturing and construction as applicable to the work performed. This will include implementing documentation of this Contractor Quality Control Plan and their assuring that Trade Subcontractors prepare, implement document trade package specific QC Plans. Webcor /Obayashi JV CQC Field Specialists will provide day to day oversight of the CQC System to assure Trade Subcontractor work conforms to the requirements of Transbay Transit Center Contract Documents and this Webcor/Obayashi JV CQC Plan.

Webcor/Obayashi JV will direct Trade Subcontractors to execute their CQC plans and maintain compliance with all project requirements as described in the Contract Documents. Contracts with Trade Subcontractors and Sub-tier Subcontractors shall include a requirement to comply with the provisions of this Plan, and to prepare and execute QC plans appropriate for their scope of work. The Trade Subcontractors, Sub-tier Subcontractors are authorized to manage their own QC Plans. All subcontractors, QC Managers, field personnel assigned to that work at the site shall conform to contract including the requirements described in this CQC Plan and their trade package specific QC Plans.

### **2.2 CQC OVERVIEW**

Quality Control Written procedures and instructions have been developed for activities affecting quality in design, procurement, manufacturing, and construction as applicable to the work performed. Procedures and instructions have been developed for control of processes including inspection, testing, nondestructive examination, disposition of nonconforming product, corrective action, maintenance of quality records, quality audits, and training.

The procedures contain a statement of the purpose and scope, and contain any references to appropriate codes, standards, or specifications. In developing the quality approved and future procedures, consideration has been given to identifying and acquiring any inspection equipment, skills, or special quality processes needed to ensure quality performance. Inspection and testing techniques shall be kept up-to-date. Where new techniques are being used for construction or manufacturing, adequate time shall be allowed to develop appropriate quality procedures for the new techniques. The procedures and instructions shall contain formats for the quality records needed to ensure that the procedures and instructions are followed and documentation requirements are understood.

By providing these guideline to Trade Subcontractors and then meeting with them, along with other key members of the project team, W/OJV will assure that each of the subcontractors, whether large or small would be able to develop a CQC Quality plan that satisfies the requirements of the FTA Guidelines, and consistent from plan to plan.

Offsite Quality Control for Bradken Steel Nodes Casting, Skanska Structural Steel Fabrications, Skylight Glass and other offsite systems fabrication and equipment will be inspected in the shop for quality in coordination with special inspections by our trade subcontractors. This will cover all offsite construction operations as required per contract. This is in addition to Quality Assurance by Turner QA team as TJPA Representative.

### 2.3 THREE PHASE QUALITY CONTROL SYSTEM

The three phase of control for the Contractor's quality control is the means by which W/OJV, including Trade Subcontractors and supplier ensure that the construction complies with the requirements of the Contract:

#### PREPARATORY PHASE:

This phase is accomplished prior to beginning work on each definable feature of work, after all required contract submittals, documents, and materials are approved and accepted and after copies are at the work site. This meeting includes:

1. A review of applicable specifications, reference codes, and standards. The Trade Subcontractor QC Manager shall make available during the preparatory inspection a copy of those sections of referenced codes and standards applicable to that portion of the Work to be accomplished in the field. The Trade Subcontractor QC Manager shall maintain and make available in the field for use by TJPA Representative until final acceptance of the Work.
2. Review of the Contract drawings and approved shop drawings (approved as noted shop drawings and record shop drawings) that incorporate all CD details.
3. Identify any submittals that have not been approved.
4. Check to assure that all materials and/or equipment have been pre-tested (if required per specification), submitted, and approved.
5. Review of provisions that have been made to provide required control inspection and testing.
6. Examination of the work area to assure that all required preliminary work has been completed and is in compliance with the Contract.

7. Examination of required materials, equipment, and sample work to assure that they are on hand, conform to approved shop drawings or submitted data, and are properly stored.
8. Review of the appropriate activity hazard analysis to assure environmental requirements are met.
9. Discussion of procedures for controlling quality of the work including repetitive deficiencies. Document construction tolerances and workmanship standards for that feature of work.
10. Check to ensure that the portion of the CQC Plan for the work to be performed has been accepted by the TJPA Representative.
11. Discussion of the initial control phase, set the date, location and scope of activities.
12. Clarification of details may be added as required after work has commenced in the form of RFI's.
13. Review Status of any outstanding RFI's

The TJPA representative shall be notified at least 48 hours in advance of beginning the preparatory control phase. Include a meeting conducted by the CQC System Manager and attended by the Trade Subcontractor's CQC Manager, other CQC personnel (as applicable), and the superintendent responsible for the definable feature of work. CQC System Manager shall document the results of the preparatory phase actions by separate minutes and attach the minutes to the weekly CQC report. CQC System Manager shall instruct applicable workers as to the acceptable level of workmanship required in order to meet Contract requirements (see the "Preparatory Phase Checklist Form" in this section; Tab/Element 7).

#### INITIAL PHASE:

This phase is accomplished at the beginning of each Definable Feature of Work (at least 1-2 days prior to start of work). This phase includes:

1. Reviewing the minutes of the preparatory meeting and ensuring any open issues have been resolved
2. Verifying the adequacy of controls to ensure full contract compliance, inspection and testing.

3. Establishing level of workmanship and verify that it meets minimum acceptable workmanship standards. Compare with required sample panels as appropriate.
4. Resolving all differences.

The CQC System Manager shall prepare separate minutes of this phase and attach the minutes to the daily CQC report. The TJPA shall be notified at least 72 hours in advance of beginning the initial phase. The initial phase shall be repeated for each new definable feature of work (see the "Initial Phase Checklist Form" in this section; Tab/Element 7).

#### FOLLOW-UP PHASE:

CQC System Manager and the Subcontractor QC manager shall perform daily checks to assure that control activities, including control testing, are providing continued compliance with contract requirements until completion of the particular feature of work. Record the checks in the CQC documentation, and file regularly in the appropriate DFOV file folder. Conduct final follow-up checks and correct all deficiencies prior to the start of additional features of work that may be affected by the deficient work. New work shall not be built upon or conceal nonconforming work. Use FCR's on BIM 360 immediately to document deficiencies with materials, installation defects or un-approved shop drawings or products.

## 2.4 TRADE SUBCONTRACTORS QUALITY CONTROL PLAN

After contract award and prior to beginning construction activities each Trade Subcontractor will submit (per specification section 01 13 00 Submittals, paragraph 1.4) to the Webcor/Obayashi Joint Venture CQC Manager their project specific quality control plan for review and approval. Each Trade Subcontractor will designate and provide a project specific Trade Subcontractor Quality Control Manager who reports to the W/OJV CQC Manager and who's primary responsibility will be to implement and manage the Trade Subcontractor's quality control plan and certify the Trade Subcontractor's compliance with the Webcor/Obayashi Joint Venture Quality Control Plan and all quality control requirements contained in the project documents including specification section 01 14 00 Quality Control. The Trade Subcontractors CQC program will be reviewed for compliance to the Contract Documents. In addition to the requirements contained in other sections of this Plan, the Trade Contractors Quality Control Program will include:

- QC Organization chart.
- Procedures for fabrication and installation.
- Procedures for planning and verifying compliance and controlling quality of the work (including checklist forms).
- Procedures for layout verification.

- Coordination with related contractors.
- List of specified tolerances and workmanship standards for each DFOW.
- Daily CQC Reports.
- Program for identifying and correcting defective work.
- Inspection, test and acceptance procedures when specified in the Technical Specifications to be part of the Trade Subcontractors scope
- A quality control Plan that addressed the Federal Transit Administration (FTA Quality Control Guidelines (ref: Transbay Transit Center Quality Management System Manual)

## 2.5 WEBCOR/OBIYASHI JV CQC MANAGER DUTIES AND RESPONSIBILITIES

The CQC Manager, or his approved alternate, oversees the overall implementation of the Webcor /Obayashi JV Quality Control Plan. The CQC manager, will be independent of the “production organization”. The CQC Manager will:

- During performance of the Work will have complete authority to take any action necessary to ensure conformance with the requirements of the Contract Documents. The Webcor/Obayashi CQC Manager or Alternate CQC Manager will have a physical presence on site when work is in progress. In the event of the CQC Managers absence, the Alternate CQC Manager must be present and will have the same authority as the CQC Manager.
- Review for conformance and completeness and approve the Trade Subcontractors QC Plans prior to submittal to the TJPA for acceptance.
- Manage the development and maintenance of the list of Definable Features of Work.
- Meet with the TJPA representative at the Coordination Meeting (Meeting of Mutual Understanding) for each Trade Work Package.
- Provide WOJV management with monthly CQC updates.
- Ensure and document Trade Subcontractor’s application of Three Phases of Control for each Definable Feature of Work.
- Conduct the Preparatory, Initial and Follow-up phase activity meetings.
- Stop and document work that does not comply with requirements of the Contract Documents, and direct removal and replacement of any defective work.
- Ensure and document that all Trade Subcontractor Work performed, on and off the construction site, conforms to requirements of the Contract Documents. Ensure and document that all materials and equipment comply with the



requirements of the Contract Documents. Report any deficiencies and corrective action planned and taken in BIM 360 Systems

- Ensure that all Trade Subcontractors CQC Plans are in conformance with the Webcor /Obayashi JV CQC plan and with the requirements of the Contract Documents.
- Review for conformance, completeness and clarity that all Trade Subcontractors certify their submittals for conformance with the requirements of the Contract Documents.
- Ensure W/O staff document review and approval of submittals prior to transmission to the CMO.
- Review and approve Webcor/Obayashi JV Daily Quality Control reports
- Prepare and submit Weekly Contractor Quality Control reports
- Ensure that all Trade Subcontractors prepare, complete and submit Daily Quality Control reports.
- Maintain copies of all quality control and quality program documents in Constructware.
- Support and facilitate the Audit Process per the QMS and FTA Element 14 (Quality Audits).
- Conduct internal audits
- Ensure that RUP Contractors use preplanning sheets and work plans for improved Quality Control, improved record keeping for M&TE (Measuring and Testing Equipment) and calibration data.
- W/OJV CQC Manager will ensure that CQC team provides a written plan and schedule for resolution of non-conforming work.
- W/OJV CQC team provides a weekly summary and review of CQC activities at the Quality Meeting.

## **2.6 WEBCOR/OBIYASHI JV ALTERNATE CQC MANAGER DUTIES AND RESPONSIBILITIES**

The Alternate CQC Manager performs all duties of the CQC Manager when the CQC Manager is not on-site. The Alternate CQC manager, when performing the duties of the CQC Manager, is independent of the “production organization”. The Alternate CQC Manager’s responsibilities are the same as the CQC Managers

## **2.7 TRADE SUBCONTRACTORS QC MANAGER DUTIES/RESPONSIBILITIES:**

The Trade Subcontractor QC Manager reports to the Webcor /Obayashi JV CQC Manager and oversees the trade specific implementation of the quality control program and whose primary responsibility will be to implement the Trade



Subcontractor's quality control plan. The Trade Subcontractor QC manager will certify that the Trade Subcontractor's work is in compliance with the Contract Documents and complies with the Webcor/Obayashi Joint Venture Quality Control Plan and all quality control requirements contained in the Contract Documents, including specification section 01 14 00 Quality Control. The Trade Subcontractor QC Manager will:

- Manage the Trade Subcontractors Quality Control Program both onsite and offsite.
- Submit a QC Plan that meets the requirements of the Webcor/Obayashi CQC Plan, Specification 01 14 00 Quality Control and the TTC Quality Management System Manual and FTA 15 Essential Elements.
- The Trade Subcontractor QC Manager or alternate QC Manager will have a physical presence on site when work is in progress.
- Designate a qualified Alternate Trade Subcontractor QC Manager to serve in the event of the Trade Subcontractor QC Manager's absence.
- During performance of the Work, will have complete authority to take any action necessary to ensure conformance with the requirements of the Contract Documents.
- Submit daily Quality Control Reports to the Webcor/Obayashi JV CQC Manager.
- Submit Preparatory and Initial Phase Checklists, along with Follow-up Phase documentation for each DFOV to the Webcor/Obayashi JV CQC Manager for review and approval.
- Establish written procedures for Trade Subcontractor document control, submittal management and material procurement.
- Maintain review for conformance and submit copies of all quality control documentation, certifications, and materials delivery receipts as required in the Contract Documents.
- Attend the Coordination meetings (Meeting of Mutual Understanding).
- Manage the Three Phases of Control process for each DFOV, including attending the Preparatory, Initial and Follow-up phase activity meetings for each of the trade subcontractors DFOV.
- Immediately stop any work, for which they are responsible, that does not comply with requirements of the Contract Documents, and direct removal and replacement of any defective work.
- Conduct daily quality inspections of Work performed prior to request for agency or special inspections to ensure compliance with requirements of the Contract Documents.

- Ensure that all Work performed, on and off the construction site, and all materials and equipment conform to requirements of the Contract Documents.

Report nonconformances and corrective action planned and taken in BIM 360 Systems.

- Remove any person from the Project that consistently fails to perform Work properly.
- Ensure that the Trade Subcontractors submittals conform to the requirements of the Contract Documents.

## 2.8 QC SPECIALIST RESPONSIBILITIES

In addition to CQC personnel specified elsewhere in the Contract, Contractor shall provide as part of the CQC organization, QC specialists that are specialized personnel to implement the CQC Plan. The QC specialist will:

- Be responsible to the CQC System Manager
- Be physically present at the construction site during work on their areas of responsibility, and have the necessary education and experience.
- These individuals may perform other duties but must be allowed sufficient time to perform their assigned quality control duties as described in the CQC plan.
- Stop and document work that does not comply with requirement of the Contract documents, and direct removal and replacement of any defective work.



## CONTRACTOR QUALITY CONTROL MANAGER APPOINTMENT LETTER

**To:** Adib Sassine  
Quality Control Manager

**From:** Jes Pederson  
President / CEO Webcor/Obayashi Joint Venture

**Date:** October 24, 2013

**Subject:** Appointment of Quality Control Manager for Transbay Project

---

Please be advised that you are hereby appointed as Quality Control Manager for the Transbay Transit Center Project. Your responsibilities include managing and implementing the Webcor/Obayashi Joint Venture Project Quality Control Plan.

You are assigned the following responsibilities:

- Implementing provisions of the Webcor/Obayashi JV Quality Control Plan as it pertains to the contract Documents.
- Assuring that the Quality Control Plan is established and implemented by persons doing work that impacts quality.
- Assuring that the Quality Control Plan complies to the FTA Guidelines, TJPA Quality Management System and Contract requirements.
- Acting as W/O JV liaison with parties outside of the company on matters relating to quality.
- Reporting to Senior Management on the performance of the Quality Control Plan, including needed improvements.
- Review for conformance, completeness and clarity of the quality control documents.
- Review for conformance, completeness and clarity of quality control records.
- Review for conformance, completeness and clarity of quality related contract submittals.
- Review for conformance, completeness and clarity of project inspection and QC activities.
- Review for conformance, completeness and clarity of subcontractors quality control programs.
- Reporting to the TJPA representative on matters pertaining to quality.
- Reviewing for conformance, completeness, clarity and distributing subcontract QC reports.

I grant you authority for carrying out the above responsibilities including:

- Stopping Work when continuing work may adversely affect quality or cover up a defect.
- To direct the removal and replacement of a nonconforming work or material by any subcontractor or supplier.

President / CEO signature and date:

W/O CQC Plan TTC Rev 1



## ALTERNATE QUALITY CONTROL MANAGER APPOINTMENT LETTER

**To:** Duncan Sinclair  
Alternate Quality Control Manager

**From:** Jes Pederson  
President / CEO Webcor/Obayashi Joint Venture

**Date:** October 24, 2013

**Subject:** Appointment of Alternate Quality Control Manager for Transbay Project

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Please be advised that you are hereby appointed as Quality Control Manager for the Transbay Transit Center Project. Your responsibilities include managing and implementing the Webcor/Obayashi Joint Venture Project Quality Control Plan.

You are assigned the following responsibilities:

- Implementing provisions of the Webcor/Obayashi JV Quality Control Plan as it pertains to the contract Documents.
- Assuring that the Quality Control Plan is established and implemented by persons doing work that impacts quality.
- Assuring that the Quality Control Plan complies to the FTA Guideline, TJPA Quality Management System and Contract requirements.
- Acting as W/O JV liaison with parties outside of the company on matters relating to quality.
- Reporting to Senior Management on the performance of the Quality Control Plan, including needed improvements.
- Review for conformance, completeness and clarity of the QC documents with contract documents and approval.
- Review for conformance, completeness and clarity of QC records with contract documents and approval.
- Review for conformance, completeness and clarity of quality related contract submittals with contract documents and approval.
- Review for conformance, completeness and clarity of project inspection and QC activities with contract documents and approval.
- Review for conformance, completeness and clarity of subcontractors quality control programs.
- Reporting to the TJPA representative on matters pertaining to quality with contract documents and approval.
- Reviewing for conformance, completeness, clarity and distributing subcontract QC reports and contract documents an approval.

I grant you authority for carrying out the above responsibilities including:

- Stopping Work when continuing work may adversely affect quality or cover up a defect.
- To direct the removal or replacement of and nonconforming work or material by any subcontractor or supplier.

President / CEO signature and date:

W/O CQC Plan TTC Rev 1



**ADIB SASSINE, AIA CA RA**  
**PRECON AND CONSTRUCTION QUALITY**  
**CONTROL MANAGER**



**Design and Construction Experience: 35 years (1978)**

Mr. Sassine is a California licensed architect and has over 35 years of strong experience in diverse large project types, including Construction Quality Control, Pre-construction and Construction Management. His extensive experience includes over 25 years of experience on new and renovated health care facilities primarily OSHPD projects; and balance of experience includes; education, schools, office buildings, public buildings, large airports, hotels and restoration of historic buildings.

**RELEVANT EXPERIENCE**

**Building Envelope Sr. Consultant and Architect – Allana Buick & Bers  
 (July 2011 to 7.2013)**

Architect and Quality Control Manager on several projects including the following:

New Stanford Hospital over \$1 billion; Performed peer review of the entire building envelope over 28 systems. (Rafael Vineolli)

9<sup>th</sup> and Broadway 17 story tower in San Diego; Design and construction quality control of the building envelope including several green roof areas. (Thornton Tomasetti)

Palo Alto Mitchell Park Library including several systems and green roof; Design of all building envelope and performed construction QC. (Group 4)

San Jose University Student Center, LA Harbor Science Building Design and construction monitoring of exterior envelope composite mock-up testing and similar other including UC Berkeley restorations. UCSF Parnassus MOB and Hospital forensic work and remediation of two major buildings.

San Mateo Medical Center MOB Exterior skin upgrade design.

And several other projects.

**Healthcare**



**Acute Care Mock-up**



### **Santa Clara Valley Medical Center SCVMC, San Jose, CA – Turner Construction Co (2007 to 2011)**

**OSHDP** – Construction Quality Control Manager on the Bed Building One project which includes the following:

1. A 6 story with Basement and Penthouse nursing tower replacement over 350,000 sf, with 168 beds primarily ICU and Acute Care Units and Rehab Center utilizing SidePlate moment frame system and phased incremental approvals.
2. A 1500 stall Parking Garage with 850 KWp Photovoltaic tracking system over the new garage and retrofit existing Garage for the added solar panel system
3. Design-Build Central Plant upgrade with Site Utilities Loop to include 2-1000 tons absorption chillers, two cooling towers and 2-2000KW generators and two boilers
4. And the Design-Build of Renova Drive intersection relocation
5. Make-ready projects to relocate all underground utilities from the site while the hospital is in operation.

As a QC Manager, Adib is responsible for the construction quality control as well as assisting Purchasing to writing scopes, for all bid packages and reviewing contracts. Some of the quality control responsibilities are to develop the quality control plan and its implementation, pre-inspection of the work before submitting inspection requests by the IOR, reviewing all RFI's, reviewing schedule, reviewing shops and certifying them for compliance with the permitted contract documents, certifying pay applications and certifying milestone completion dates. Adib was involved in providing Pre-construction services such as Sr. Project analyst to provide planning, coordination with all enabling and make-ready projects, scheduling, progress plan check, constructability reviews, report writing and evaluations, phasing plans, cost control and site logistics of the Parking Garage and Solar Power design-build projects and other related hospital projects from Cath Lab to MRI renovation on campus.

### **CHW St Joseph Women and Children Hospital Stockton, CA (\$65M) - Turner**

**OSHDP** – CM at Risk – Pre-construction

Addition of 100,000 sf of 78 beds hospital building with elevated bridge connector and underground parking Garage. Adib provided Constructability Reviews, Site Logistics and Cost Control.



### **Mills-Peninsula Medical Center Hospital, Burlingame, CA (\$400M+) - Turner**

**OSHDP** – CM at Risk – Pre-construction up to NTP

Addition of 440,000 sf six (6) level Hospital designed with base isolation and damper structural systems. Adib provided constructability reviews and purchasing services to include bidding multiple packages, writing scopes and developing bid spread sheets and reviewing all subcontracts for fast-track incremental approvals while project was being reviewed by OSHDP.

### **Historic Laguna Honda Hospital Seismic Upgrade, San Francisco, CA (\$50M) - Turner**

**OSHDP** – CM at Risk – Pre-construction PM



Adib Managed the project through bidding to include Constructability reviews, phasing, scheduling and budgeting for seismic retrofit of Wing H of the original historic Hospital project and coordination with the new Laguna Honda hospital replacement project.



#### **John Muir Medical Center Hospital Expansion, Walnut Creek, CA (\$230M) - Turner**

**OSHDP** – Pre-con services.

Addition of 429,000 sf 5-story tower and remodel of existing regional Trauma hospital including helipad and new Central Plant. Remodel consists of new Emergency Department and phased construction. Provided constructability reviews, phasing plans, cost controls and site logistics.

#### **Lucille Packard Children Hospital Expansion, Palo Alto, CA (\$70M) - Turner**

**OSHDP** – Lump Sum – Constructability review during early construction phase.

#### **CPMC Cathedral Hill Hospital Preconstruction, San Francisco, CA (\$850 M) - Turner**

**OSHDP** – Delivery Method CM at Risk – Adib provided comprehensive Constructability and Estimate Reviews in the latter part of Turner involvement on the project.

Ground up 550 beds for adults and women/children and 2,745,000 SF Women's and Children's Hospital in downtown San Francisco consisting of 19 stories above ground and 6 stories underground with base isolation. This project included a medical office building design-built with a connecting tunnel under Van Ness.

#### **Sr. PM and Healthcare Business Development – Hathaway Dinwiddie (2004-05)**

**OSHDP** – CM at Risk – during Schematics and DD

Responsible for managing small healthcare projects for Stanford ED and UC Clinical Lab. Adib was responsible to provide BD at Hathaway Dinwiddie. Also Adib managed and bid window replacement on 20 story high rise in Nob Hill in SF and performed cursory constructability review for the Millenium condo tower in SF during early design phase.

#### **Sharp Memorial Hospital, San Diego, CA (\$185M) – Gilbane (2000-03)**

**OSHDP** – Project Executive - CM at Risk – during Schematics and DD

This multi-phased project includes the construction of a new six and seven-story, 302 bed patient towers of 315,000 s.f. that include 158 Acute/IMCU beds, 24 SICU/CVICU beds, 64 AC/IMCU beds, 24 CCU/MICU beds, 32 AC/IMCU-Ortho beds and shell space for 32 beds for a total of 334 beds; 14 Operating Rooms and Surgery Suite; New Emergency Department, new Hospital Entrance and Lobby; and administrative spaces. In addition to the new hospital addition, and as part of the SB1953, the Critical Care Areas within the existing hospital will be relocated to the new HMP Addition. This project



also includes the Central Plant Expansion to accommodate new hospital replacement, Coordination with other projects on site such as an Ambulatory Care Center and OSHPD 600 stall parking Garage with Helipad.

#### **UC Davis Medical Center, Sacramento, CA (\$260M) - Gilbane**

**OSHPD** – Project Executive Agency CM - This Surgery and Emergency Services Pavilion addition at the UC Davis Medical Center. (During Schematics and DD)

This pavilion is a major addition to the Main Hospital building at UC Davis Medical Center. The project under construction will include approximately 420,000 s.f. of building construction and ten acres of site development. It includes Emergency Department, Dietary Department, Radiology, Cardiology and a 24-room Operating Room suite.

#### **Kaiser Walnut Creek Hospital, Walnut Creek, CA – BFH (1989-96)**

**OSHPD** – Design and Construction Administration - New multi-phase, three-story with full basement, 123 bed Hospital addition and replacement, 10 Operating Rooms, Surgery Suite, MRI Suite, Central Sterile, Clinical Lab, 4 C-Section Rooms, 24 LDR Rooms, ICN and other ancillary spaces. The Hospital was built while maintaining the entire existing hospital in operation on a 28-acre site with covered running creek and heritage Oak trees over 200 years old. Existing building had to be demolished in sections, and existing tower was later renovated and connected to the new Hospital.

#### **Kaiser Walnut Creek Central Plant Expansion, Walnut Creek, CA – BFH**

**OSHPD** – Design and CA - This Central Plant Expansion, Medical Gas Farm and Emergency Generator Plant. Project involved 3- 350-ton chillers, switchgear room, boiler room and 3-750KW Generators. Enclosure was adjacent to existing Parking garage with utilities running over creek lid in a high density site.

#### **Kaiser Vallejo Medical Center MOB, Vallejo, CA (\$50M) – SOM (1986-89)**

Design and CA - This two-story, 166,645 s.f. Medical Office Building with courtyards to accommodate 123 providers on a 38-acre site with on-site parking built with a connecting site utility loop to CUP.

#### **Kaiser Vallejo Medical Center Central Utility Plant, Vallejo, CA - SOM**

**OSHPD** – Design and CA - This Utility tunnel was added to connect to new Central Plant Expansion. Generator Plant

#### **Kaiser San Rafael Medical Center MOB Renovations, San Rafael, CA (\$12M) - BFH**

Design and CA - This 8,000 s.f. project, including OR, ER renovation, pharmacy and radiology renovations over 4-year plan.

#### **Coalinga Community Hospital, Coalinga, CA (LHR)**

**OSHPD** – Design and CA - This 56,000 s.f. project involving 35-bed hospital and 56-bed skilled nursing facility replacements to earthquake-damaged facility. Site is an approximately 12-acre parcel on a new development area.

### **Office Building**

#### **State Office Building at Butterfield Way, Sacramento, CA (\$171.5M) - Gilbane**

Project Executive - Agency CM - Franchise Tax Board Campus addition and renovation project for the State of California, Department of General Services, and Project Management Branch on this project.





This project involves 1 Million SF of new construction and 843,000 s.f. of renovation on 93 acre site. It is located in Sacramento, California, and consists of phased construction with separate contracts for Sitework, a Central Utility Plant (\$25M), a Warehouse, four Building Office complex, and a Town Center. This project was designed to be a LEED certified project.

#### **Wells Fargo Card Division Relocation Center, Concord, CA - BFH**

Program Manager and Construction Administrator

Fast-track, 265,000 SF Data Center, with 100% access flooring office space and high security project completed without a single change order for the tenant improvement.

### **Office and Commercial Historic/Seismic Upgrade**

#### **Oakland Rotunda Seismic Upgrade, Oakland, CA (\$32M) - AD**

Design and CA - This 265,000 s.f. historic building over 100 year old with elliptical dome and seven-story elliptical atrium sustained serious damage during Loma Prieta earthquake in 1989. The brick and steel building had to be retrofitted seismically, including replacing mechanical, plumbing, and electrical systems including provided complete tenant improvements as part of a design-build team. The building has multiple commercial tenants on the first floor and multiple office tenants on the upper floors.

### **Airports**

#### **SFO International Airport, San Francisco, CA (\$830M) – Skidmore Owings and Merrill – (1996-98)**

Sr. Technical coordinator and Construction Administration as Owner's Rep - Over 1.8 million s.f. of base isolation SFO International Terminal Addition, two five-story office buildings, and light rail, BART station additions and elevated roadway fast-track projects, including coordination with adjacent Boarding Areas A and G. This included VE implementation of over \$35 million while project being bid on a fast track delivery model. Adib was also responsible to coordinate with Boarding Areas A and G of two different architectural firms and elevated roadways for total construction cost of \$2.3 billion.

### **Hotels/Convention Centers**

#### **Marriott Hotel Tower, Santa Clara, CA (\$28M) - JYA**

Design - This 22-story tower consists of new tower with banquet facilities to accommodate 1,500 persons, a restaurant and conference center. Entire tower was designed as reinforced concrete structure with post tension slab and pre-fabricated EIFS system as the exterior skin.

#### **Original Moscone Convention Center, San Francisco, CA – JA/HOK (1980-83)**

CA assistance for the tub design by HOK/IM Pei at 40 feet below Howard and provided punch list for the entire building.

### **Other Education Facilities**

#### **Foothill and De Anza Community Colleges in Los Altos and Cupertino, CA (\$275M) - Gilbane**

Agency CM - Measure "E" Bond improvements for FHDA. This program consists of new building and



existing building renovations over 60 major projects ranging from \$1Million to \$33 Million.

**University of California at Berkeley, Berkeley, CA - JY**

Design - Renovation projects, including Julia Morgan's Hearst Gymnasium, Manville Hall, and Administration renovations.

**EDUCATION/LICENSE**

Bachelor of Science, Architecture, Cogswell College, San Clara, (formerly in SF) CA

California Licensed Architect

UC Berkeley Extension Art and architecture Courses

Construction Management Certificate - Brown University thru Gilbane

**CERTIFICATION**

Occupational Health and Safety Administration (OSHA) 30-hour training

**PROFESSIONAL AFFILIATIONS**

American Institute of Architects (AIA)

**OTHER LANGUAGES**

Arabic and French

**REFERENCES**

By Request



## Duncan J Sinclair Quality Alternate

### EDUCATION AND BACKGROUND

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As the Contractor's Alternate Quality Manager when the W/O JV Quality Manager is not on site, Mr. Sinclair will have the primary responsibility of managing the Contractors Quality Management System. His Duties include ensuring Trade Subcontractor compliance with the projects quality requirements via implementation of specified process controls and acting as the day to day interface between project production and quality management to assure the work conforms to the project requirements. He is responsible for documenting quality compliance and providing senior management with periodic quality reports.

Mr. Sinclair graduated with a BS in Mechanical Engineering from Washington State University in Pullman, Washington in 1971. Mr. Sinclair also earned a Masters in Business Administration from City University of Seattle in 1982. His 30 years of construction management and quality management experience includes implementing project-specific quality management programs for a variety of construction projects.

### RELEVANT EXPERIENCE

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<b>Transbay Transit Center San Francisco, CA</b>	Pre-Construction on Subcontractor Work Packages and analyze Commissioning Trade Specifications and correlations to Commissioning Coordinator (CxC) Specification on the Transbay Transit Center Project. Public Works; 2011- present. Total Public Works Projects is 17 years.
<b>Lawrence Livermore National Lab Livermore, CA</b>	LLNL Building HVAC Controls and Electrical Smart Meters. Construction Superintendent for Johnson Controls, Inc. (JCI) to manage field operations installing Electrical Power and HVAC DDC Controls in selective buildings at the Lawrence Livermore National Labs (LLNL) under Contract with Nuclear National Security Agency (NNSA). Duncan managed electricians and HVAC Controls Techs and field verified completeness, assured quality program compliance, Safety Program adherence & housekeeping while performing electrical power meter installations and HVAC DDC modifications and tracking. Daily Work Permits were written by JCI and approved by LLNL. Duncan verified the Work Permit was implemented and notified the JCI QC & LLNL Inspectors to witness the final installation. Public Works; 2010-2011 - 1 year.



<b>Lawrence Livermore National Lab Livermore, CA</b>	<p>Construction Manager for Jacobs Engineering Group assigned to National Ignition Facility Laser CM Team at Lawrence Livermore National Lab to manage various improvements including renovation of an adjacent 3 story office use for \$5M lab support facility. Duncan generated all the required Work Permits that includes Safety precautions, specific installation instructions, &amp; Quality management to tie-in MEP Systems to existing Configured Systems under Engineering Management Control. Duncan was responsible for Safety, Facility Access, and interfaced with project QC Inspectors to confirm compliance to Contract Drawings, &amp; Specifications. Coordinated operations with Facility personnel. Public Works; 2009-2010 - 1 year.</p>
<b>Millennium Tower (301 Mission) San Francisco, CA</b>	<p>This project is a high-end condominium/mixed-use project 60 stories tall. It also includes a 12 story condominium/amenity building connected by a 3-level Atrium/Podium. Mechanical, Electrical, Plumbing and Sprinkler (MEPS) Superintendent coordinating MEPS Subcontractors work and quality compliance, \$80M Subcontracts. Monitored, updated and planned the Project schedule for 3 week projections. Reviewed Submittals to confirm compliance with Projects Specifications. Inspect all MEPS installations to insure Quality compliance to Specifications. Managed the RFI process to resolve conflicts in drawings or obtain clarifications. Duncan Coordinated Subs to obtain Temporary Certificate of Occupancy with SFPD. Enforce OSHA, Company Safety and Quality Program requirements. \$348 million.</p>
<b>St. Regis Museum Tower San Francisco, CA</b>	<p>A five-star, 42-story mixed-use hotel and condominium project with 269 luxury hotel rooms and 102 high-end condominiums. The project also incorporates the renovation of the existing 9-story historic Williams Building, built in 1907. The renovation included a seismic upgrade and the building will house the hotel's restaurant and kitchen as well as a portion of the African American Cultural Museum. MEPS Superintendent coordinating with \$80M MEPS Subcontractors, Owners Rep's and project superintendents for Webcor Builders. Duncan monitored, updated and planned the Project schedule for 3 week projections. Reviewed Submittals and field inspected the MEPS installations for Quality compliance. Write RFI's to resolve conflicts in drawings or obtain clarifications. Duncan coordinated Subs to obtain TCO with City Officials. Enforce OSHA and Company Safety Program. \$173 million.</p>
<b>Lawrence Livermore National Lab Livermore, CA</b>	<p>Zone Manager for the Laser Bay for a \$5M contract for LLNL to install the major components used as the base equipment for the Laser Beams in the National Ignition Facility (NIF). Duncan was the Field Manager for the Subcontractor with 45 craft performing the installation. Duncan was responsible for Quality Control Management to assure exactness of tolerances and standards for welding and metal finishes, enforces Safety requirements during the installation process. Public Works; 1999-2000 - 1 year</p>
<b>Lawrence Livermore National Lab Livermore, CA</b>	<p>Field Area Manager for Jacobs' \$185M self performs activities with Union craft to install the Laser Beam Enclosures. Duncan enforced all Safety Regulations, Personal Protective Equipment, Clean Construction Protocol, Project Labor Agreement, and schedule activities. Duncan was the primary field contact with LLNL personnel for schedule coordination, engineering RFI's, Quality Control, managing non-conformance reports, and safety incidents. Conducted daily coordination with Superintendents, Subcontractors, and the Client to control installation activities in each area and avoid craft conflicts to maintain schedule objectives. Public Works; 2000-2003 - 3 years.</p>



<b>San Francisco City Hall Renovation San Francisco, CA</b>	<p>SF City Hall Seismic Retrofit &amp; TI Modification-\$200M, w/GC: Managed MEPS Subcontractors through design coordination, submittal review, sequential scheduling. Quality management, installation, and start-up. Duncan worked closely with TI Architect to incorporate new systems with existing and new architectural designs. Worked hand in hand with SF DBI by pre-inspecting installations and notifying the Inspectors when systems were ready. Public Works; 1995-1999 - 4 years.</p>
<b>Singapore US Embassy Livermore, CA</b>	<p>US Fed Government Embassy at Singapore-\$50M, w/GC; Stateside coordinator controlling mechanical and electrical vendor's submittal documentation for approval for Quality management, construction installation and systems operations. Write requisitions and submittal requirements for mechanical equipment for purchase orders. Resolve conflicts between overseas site and domestic vendors. Public Works; 1993-1995 - 2 years.</p>
<b>Sharks Hockey Arena San Jose, CA</b>	<p>San Jose Sharks Ice Hockey Arena-\$150M, w/CM; Directed mechanical &amp; plumbing subcontractors to comply with the City DPW ICBO Code requirements with project specifications involving wet and dry HVAC and plumbing including seismic bracing systems. Duncan verified all installation met Contract Specifications &amp; Drawings and equipment start-up and systems operational modes. Assisted SJ DPW on completion of ICBO Plumbing Code required pipe testing and clearances. Duncan had an active ICBO Plumbing Certification from 1988 to 1998. Public Works: 1992-1993 - 1 year.</p>
<b>US Postal Service 860 Main Street San Francisco, CA</b>	<p>US Postal Service Lost Package Facility and the US Treasury Department. US Post Offices added HVAC &amp; Fire Protection to floors that were modified from open rooms to partitioned offices. US Treasury Dept. upgraded office spaces, Computer Room and Automated check envelope wrapping machine. Duncan performed all Quality Control and code inspections for Fire Protection, plumbing, mechanical and HVAC Controls installations. Public Works: 1991-1992 - 1 year.</p>
<b>Convention Center San Jose, CA</b>	<p>The San Jose Convention Center is the main <a href="#">convention center</a> for the city of <a href="#">San Jose, California</a>. It is located in close proximity to several others of San Jose's convention and cultural structures. The San Jose McEnery Convention Center provides more than 425,000 square feet of space for conventions and events. Its flexible configuration offers 143,000 square feet of divisible, column-free prime exhibit space, a large ballroom, up to 30 meeting rooms with up to 2,400 theater-style seats and banquet facilities for up to 5,000 persons. In addition, the Convention Center has 30-foot-high finished ceilings, 12 loading bays with drive-on access to the exhibit hall floors, recessed utility boxes with electricity, water and drainage capabilities complete audio-visual, sound and lighting services, cellular, standard and ISDN telephony services and fiber optic and copper cabling throughout the facility with DS-3 high-speed Internet access. As the plumbing and mechanical inspector for O'Brien-Kreitzberg Inc., Duncan inspected all plumbing &amp; mechanical installations to insure project Quality, and code compliance in conjunction with the ICBO Plumbing City Inspector. Active in resolving RFI and Code issues with plumbing Inspector. Duncan had an active ICBO Plumbing Certification from 1988 to 1998. Public Works 1987-1990 - 3 years.</p>



CERTIFICATIONS AND PROFESSIONAL MEMBERSHIPS

US Army Corps of Engineers/NAVFAC Quality Certified, 2012

OSHA 10 & 30 Hour Certified

American Society of Mechanical Engineers; Life Member

Professional Profile for Mario B. Saladana,  
Webcor/Obayashi Quality Control Specialist

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Current Position

Mario B. Saladana serves as a Quality Control Specialist/Senior Superintendent.

Experience

Mario has 35 years of construction experience and 28 years where with Webcor.

Mario has extensive familiarity with construction codes and practices, overseeing subcontractors and with residential, hospitality, and concrete projects.

Mario is familiar with a wide variety of project types and delivery methods.

As a Quality Control Specialist/ Senior Superintendent, Mr. Saldana assumes responsibility for on-site activities including overall coordination and scheduling of subcontractors and self-performed labor, safety, and quality. He develops and manages the schedule to ensure on-time performance. Together with the project management staff, Mr. Saldana collaborates in design, estimating and constructability reviews. He manages subcontractor performance on-site.

Professional Certifications

USACE Construction Quality Management for Contractors      Certificate Awarded Oct 2012

Attachments

USACE CQM Certificate




## CERTIFICATE

Mario Saldana

SW9-02-12-00496

has completed the Corps of Engineers and Naval Facility Engineering Command Training Course

## CONSTRUCTION QUALITY MANAGEMENT FOR CONTRACTORS - #784

San Francisco, California	10/1/2012 - 10/2/2012	SW9 - NAVFAC Southwest	Michael Haliburton PMP, PE
Location	Training Date(s)	Instructional District/ NAVFAC	COM-C Manager
Kugan Panchadsaram	kugan@kugan.com	858-212-2941	
Facilitator/Instructor	Email	Telephone	Facilitator/Instructor Signature

  
Director, USACE Learning Center

**THIS CERTIFICATE EXPIRES FIVE YEARS FROM DATE OF ISSUE**



Professional Profile for Jose Verduzco  
Webcor/Obayashi Quality Control Specialist

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Current Position

Jose Verduzco serves as a Quality Control Specialist/Assistant Superintendent.

Experience

Jose has extensive familiarity with construction codes and practices.

Jose is familiar with most major construction methods.

As a Quality control Specialist/Assistant Superintendent, Mrs. Verduzco plans, schedules, coordinates, sequences, and monitors procurement and construction activities for field teams. He conducts field reviews to inspect and assure compliance to construction policies, procedures, and standards. He reviews drawings, specifications, and subcontractor submittals and ensures that field staff and subcontractors comply with required safety standards. In addition, Mrs. Verduzco prepares correspondences and reports, generates short interval schedules, and manages self-performed labor. He assumes responsibility for weekly LDR quantities and orders necessary materials and equipment.

Education

Jose holds a Bachelor of Science, Business Management in Commerce, Santa Clara University, Santa Clara, CA 2007

Professional Certifications

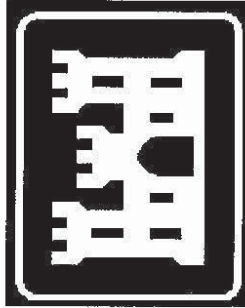
USACE Construction Quality Management for Contractors      Certificate Awarded Oct 2012

Attachments

USACE CQM Certificate



USACE LEARNING CENTER  
HUNTSVILLE, ALABAMA



NAVFAC



# CERTIFICATE

Jose Verduzco

SW9-02-12-00502

has completed the Corps of Engineers and Naval Facility Engineering Command Training Course

## CONSTRUCTION QUALITY MANAGEMENT FOR CONTRACTORS - #784

San Francisco, California	10/1/2012 -10/2/2012	SW9 - NAVFAC Southwest	Michael Haliburton PMP, PE
Location	Training Date(s)	Instructional District/ NAVFAC	COM-C Manager
Kugan Panchadsaram	kugan@kugan.com	858-212-2941	
Facilitator/Instructor	Email	Telephone	Facilitator/Instructor Signature
			
			Director, USACE Learning Center

THIS CERTIFICATE EXPIRES FIVE YEARS FROM DATE OF ISSUE

**Professional Profile for Brian Perez**  
**Webcor/Obayashi Quality Control Specialist**

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**Current Position**

Brian Perez serves as a Quality Control Specialist/Assistant Superintendent.

**Experience**

Brian has extensive San Francisco Building experience.

Brian has been involved in several of Webcor's marquis projects

Brian is familiar with construction codes and practices.

As a Quality control Specialist/Assistant Superintendent, Mr. Perez plans, schedules, coordinates, sequences, and monitors procurement and construction activities for field teams. He conducts field reviews to inspect and assure compliance to construction policies, procedures, and standards. He reviews drawings, specifications, and subcontractor submittals and ensures that field staff and subcontractors comply with required safety standards. In addition, Mr. Perez prepares correspondences and reports, generates short interval schedules, and manages self-performed labor. He assumes responsibility for weekly LDR quantities and orders necessary materials and equipment.

**Education**

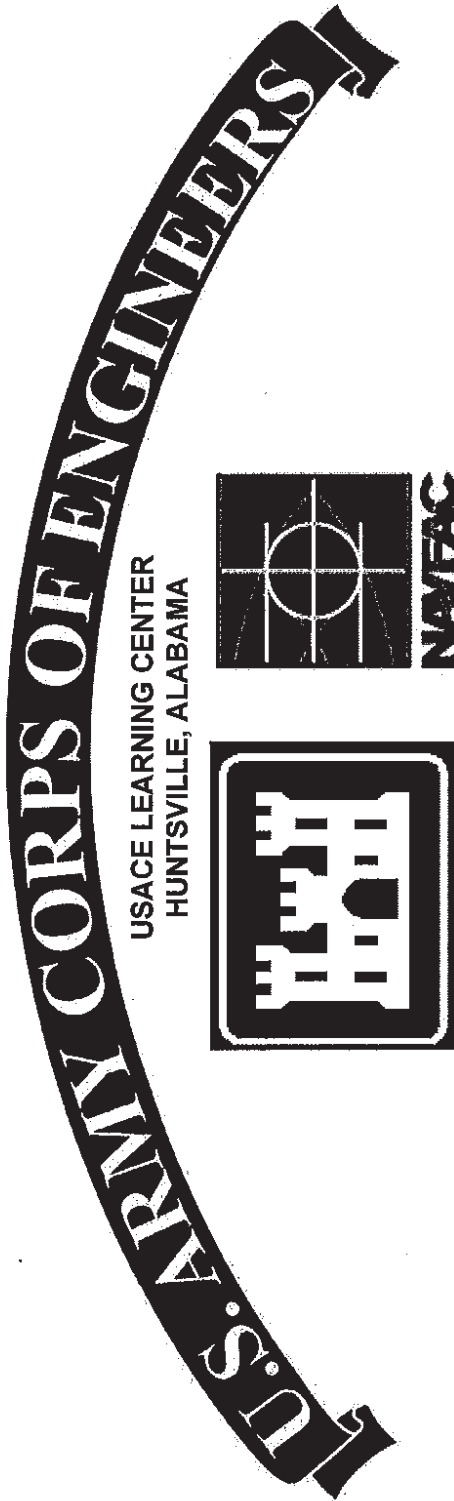
Brian holds an Associate of Science, Fire Science, Diablo Valley College, Pleasant Hill, CA 1998

**Professional Certifications**

USACE Construction Quality Management for Contractors Certificate Awarded Jan 2012

**Attachments**

USACE CQM Certificate



# CERTIFICATE

Brian Perez

SW9-02-12-00062

has completed the Corps of Engineers and Naval Facility Engineering Command Training Course

## CONSTRUCTION QUALITY MANAGEMENT FOR CONTRACTORS - #784

Concord, California	January 26-27, 2012	SW9 - NAVFAC Southwest	Michael Haliburton PMP, PE
Location	Training Date(s)	Instructional District/ NAVFAC	CQM-C Manager
Kugan Panchadsaram	kugan@kugan.com	858-212-2941	<i>[Signature]</i>
Facilitator/Instructor	Email	Telephone	Facilitator/Instructor Signature
			<i>[Signature]</i>
			Director, USACE Learning Center

THIS CERTIFICATE EXPIRES FIVE YEARS FROM DATE OF ISSUE

Professional Profile for Jordan Smith  
Webcor/Obayashi Quality Control Specialist

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Current Position

Jordan Smith serves as a Quality Control Specialist/Assistant Superintendent.

Experience

Jordan has extensive San Francisco Building experience.

Jordan has been involved in several of Webcor's marquis projects

Jordan is familiar with construction codes and practices.

As a Quality control Specialist/Assistant Superintendent, Mrs. Jordan plans, schedules, coordinates, sequences, and monitors procurement and construction activities for field teams. He conducts field reviews to inspect and assure compliance to construction policies, procedures, and standards. He reviews drawings, specifications, and subcontractor submittals and ensures that field staff and subcontractors comply with required safety standards. In addition, Mrs. Jordan prepares correspondences and reports, generates short interval schedules, and manages self-performed labor. He assumes responsibility for weekly LDR quantities and orders necessary materials and equipment.

Education

Jordan holds a Bachelors of Science, Construction Management, Cal Poly University, Los Posits, CA 2008

Professional Certifications

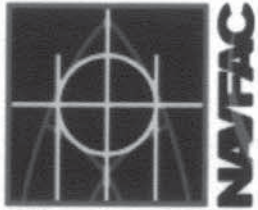
USACE Construction Quality Management for Contractors      Certificate Awarded July 2013

Attachments

USACE CQM Certificate



USACE LEARNING CENTER  
HUNTSVILLE, ALABAMA



NAVFAC

## CERTIFICATE

Jordan Smith

SW9-02-13-00319

has completed the Corps of Engineers and Naval Facility Engineering Command Training Course

## CONSTRUCTION QUALITY MANAGEMENT FOR CONTRACTORS - #784

San Francisco, CA	July 10-11, 2013	SW9 - NAVFAC Southwest	Michael Haliburton PMP, PE
Location	Training Date(s)	Instructional District/ NAVFAC	QM-C Manager
Kugan Panchadsaram	kugan@kugan.com	858-212-2941	
Facilitator/Instructor	Email	Telephone	Facilitator/Instructor Signature
			
			Chief, USACE Learning Center

**THIS CERTIFICATE EXPIRES FIVE YEARS FROM DATE OF ISSUE**  
CQM-C Recertification online course: <https://www.myuln.net>



## 2.10 TRADE SUBCONTRACTORS QUALITY CONTROL MEETINGS:

In addition to the Three Phase of Control Meetings, A Trade Subcontractor QC Meeting will be part of the Weekly Trade Subcontractors Meetings held by the Webcor/Obayashi JV Project Superintendent or Project Manager. W/OJV CQC Manager will review with the Trade Subcontractor QC Manager will review current QC issues as a segment of the weekly meeting; addressing the schedule, testing, inspection, re-work log, failed inspection status, short-term schedule of QC activities, project tests, submittal status, factory verification requirements, inspection results and any other QC issues relevant to the current activities.

## 2.11 DEFINITIONS:

- Project As-Built Drawings – All changes and modifications to the Contract work as required by site conditions and inspections in accordance with the requirements of Section 01 17 20.
- **Contractor** - Webcor/Obayashi Joint Venture (**WOJV**)
- **Coordination Meeting (Meeting of Mutual Understanding)** - A meeting held after the pre-construction conference for each Trade Work Package and before start of construction. Contractor shall meet with the TJPA Representative and TJPA QA Manager and discuss the Contractor's quality control system as it relates to the work of the trade package. Submit the CQC Plan a minimum of 15 days prior to the coordination meeting. During the meeting, a mutual understanding of the system details must be developed, including the forms for recording the CQC operations, control activities, testing, administration of the system for both onsite and offsite work, and the interrelationship of Contractor's management and control with the TJPA Representative's quality assurance. Minutes of the meeting will be prepared by the TJPA Representative, signed by both the Contractor and the TJPA Representative and will become a part of the Contract file. There may be occasions when subsequent conferences will be called by either party to confirm mutual understandings and/or address deficiencies in the CQC system or procedures that may require corrective action by the Contractor.
- **Corrective Action Plan** - A plan of action to correct nonconforming work or practices. A written document submitted by the Trade Subcontractor detailing the Trade Contractor's approach to correct an item of work that fails to conform to the project requirements.
- **Corrective Action Request** - A written request from TJPA to develop a Corrective Action Plan for non-conforming work (TJPA form QA-09-01) that establishes a method for ensuring deficiencies in process or implementation

W/O CQC Plan TTC Rev 9.1

adversely affecting quality are identified, cause determined, and an action plan to prevent recurrence is documented.

- **CQC Field Specialist** - specialized personnel to implement the CQC Plan be responsible to the CQC System Manager, be physically present at the construction site during work on their areas of responsibility, and have the necessary education or experience. These individuals may perform other duties but must be allowed sufficient time to perform their assigned quality control duties as described in the CQC Plan.
- **CQC Manager** – The Webcor/Obayashi JV Manager who is responsible for managing the Contractor's CQC System.
- **CQC Manager's Monthly CQC Report** - A section of the Contractors monthly written report prepared and submitted by the CQC Manager which reports monthly CQC activities.
- **CQC Plan** - Webcor/Obayashi JV written quality management plan that meets the requirements of the TJPA Program QMS The means by which Webcor/Obayashi JV (the Contractor/CQC) and its Trade Subcontractors (QC) ensure project quality.
- **Daily Contractor Quality Control Report** - A daily written report providing evidence that required quality control activities and tests have been performed including the work of Trade Subcontractors and Suppliers. These reports shall address deficient features and include a statement that equipment and materials incorporated in the work and workmanship comply with the Contract. These reports shall be within 5 working days after the date covered by the report. Reports shall be reviewed for completeness and accuracy, revised, signed and dated by the CQC System Manager. Reports shall be prepared by all subordinate quality control personnel and be included within the CQC System Manager's report.
- **Definable Feature of Work (DFOW)** - A definable feature of work is a task that is separate and distinct from other tasks, has separate control requirements, and may be identified by different trades or disciplines, or it may be work by the same trade in a different environment. Although each section of the Specifications may generally be considered as a definable feature of work, there are frequently more than one definable feature under a particular section. This list will be agreed upon during the coordination meeting and updated as more packages are awarded.



- **Federal Transit Administration (FTA)** - An administration within the U.S. Department of Transportation that provides stewardship to support a variety of locally planned, constructed, and operated public transportation systems throughout the United States.
- **Initial Phase Checklist** – A checklist prepared for each Definable Feature of Work (DFOW) in the Initial work Phase per 01 14 00 1.9.C.
- **Master Definable Feature of Work List** - The project list definable features of work for all trade subcontractors maintained by the Webcor/Obayashi JV CQC Manager.
- **Nonconformance Report** – A written report entered in BIM 360 Field Systems describing non-conforming Work.
- **Nonconforming Work** – Work that is unsatisfactory, faulty, defective, or deficient; Work that does not conform to the requirements of the Contract Documents; Work that does not meet the requirements of inspection, reference standards, tests, or approval referred to in the Contract Documents; or Work that has been damaged prior to Final Completion.
- **Phase 1: Preparatory Phase** – A controlled activity including a meeting conducted by the Webcor/Obayashi JV CQC Manager and with the Trade Subcontractors CQC Manager, the Subcontractor's Production Team, Trade Subcontractors Representatives, Inspectors, and TIPA representatives. This is the first of the three phases of control where all requirements of the work: drawings, specifications, submittals, RFI's, installation and coordination issues are reviewed before beginning any Definable Feature of Work (DFOW).
- **Phase 2: Initial** – A controlled activity including a meeting conducted by the Webcor/Obayashi JV CQC Manager with the Trade Subcontractors CQC Manager, the Subcontractor's Production Team, Trade Subcontractors Representatives, Inspectors, and TIPA representatives is held immediately prior to the start of the work. Using the meeting minutes from the Preparatory Phase meeting, this meeting transfers the information and requirements and agreements to the crews performing the work.
- **Phase 3: Follow-up Phase** Daily checks performed by the trade subcontractor QC an QC specialists and verified by QC System Manager to assure that control activities, including control testing, are providing compliance with contract requirements, until completion of that particular feature of work. Report the checks in the Daily QC report and upload to the DFOW records.

- **Preparatory Phase Checklist** - A checklist prepared by the CQC Manager for each Definable Feature of Work (DFOW) in the Preparatory Phase per 01 14 00 1.9.B.
- **Quality** - Conformance to the requirements established by the contract documents.
- **Quality Control Plan** - An approved written plan which includes plans, procedures, and organization necessary to produce an end product that complies with the Contract requirements. The plan covers all construction operations, both onsite and offsite, and shall be keyed to the proposed construction sequence
- **Quality Inspection** - An Inspection of the work performed as the work progresses or prior to calling for an Agency, Code or Special Inspection to confirm the work meets the requirements of the Contract Documents. Contractor shall verify all dimensions in the field and shall check all field conditions continuously during construction. Contractor shall inspect related and appurtenant work and report in writing to the TJPA Representative any conditions that will prevent proper completion of the Work in accordance with the requirements of the Contract.
- **Quality Management** — Management of Quality Control and Quality Assurance activities instituted to achieve the quality levels established by the contract documents.
- **Quality Management System Manual** - Provides specific requirements for Program implementation based upon the Program Quality Policy and the FTA Quality Assurance and Quality Control Guidelines and is the guide for all members of the Program Management Team to deliver a project that meets the highest quality standards (reference: Transbay Transit Center QMSM, Introduction, page 1).
- **Submittal Log** - A written list indicating the status of all Submittals required by the Contract Documents, maintained by the Webcor/Obayashi Joint Venture production team.
- **Technical Specifications** – Divisions 01 through 33 of the project specifications.
- **Three Phases of Control** – The three meetings or actions that bring the Trade Subcontractors CQC Managers, Contractor's Production Team, Inspectors, TJPA representatives and/or field crews together to plan and implement project

quality: The three phases of control include: The Preparatory Phase, Initial Phase and Follow-up Phase.

- **TJPA Construction Management Oversight Manager:** - Turner Construction.
- **TJPA:** - Transbay Transit Center Joint Powers Authority.
- **Trade Subcontractor QC Manager** – The Trade Subcontractor employee who is responsible for managing the Trade Subcontractor’s QC System, and reports to the Webcor/Obayashi JV CQC Manager.
- **Trade Subcontractor’s QC Plan** – The Trade Subcontractors written quality control plan that meets the requirements of the TJPA Program QMS as appropriate for the Trade Subcontractors scope of work and is the means by which the Trade Subcontractors ensure project quality.
- **Trade Subcontractor’s Definable Feature of Work List.** - The list of definable features of the work prepared by the Trade Subcontractors and submitted for review and approval to the Webcor/Obayashi JV CQC Manager
- **Trade Subcontractors Daily Quality Control Report** - The Trade Subcontractors Quality Manager’s daily report that describes: the work completed, quality measures implemented, testing and inspections performed, rework items identified, and deliveries received and as-built drawings updated. (See Tab 12 “Forms” Trade Subcontractors Daily Quality Control Report).
- **BIM 360** – Field **Web-Based Data** Management Software for construction. BIM 360 Systems combines mobile technologies and BIM at the point of construction with reporting for management. BIM 360 Field Systems field management software uses a combination of technologies including the Internet, tablets, and email-capable phones. Licensed users must have a high-speed Internet connection in the office and are responsible for procuring the necessary hardware required for field staff to use the software. All Subcontractors are required to use the BIM 360 Field Systems software, as described in Specification Section 01 31 25 (The field management system will be used to manage CM/GC and Subcontractor quality control inspection and test processes including CM/GC and Subcontractor quality control inspection reports, CM/GC and subcontractor quality control inspection request, nonconforming conditions, punch list, and incomplete items list. The field management system will also be used to manage the commission process, documenting the completion of commissioning-related tests and the resolution of any identified deficiencies). Reporting features include Field Condition Reports, Inspection Requests, Nonconformance Reports and Punch lists.

W/O JV Transbay Terminal Center  
DFOW List  
Revised 02/021/2013

DFOW Number	Baseline Schedule Activity ID	Specification Section	Required Submittals	Discription/Feature of Work	Preparatory Phase Date	Initial Phase Date	Follow Through Phase Date
BSE-001	SX-BB42160, SX-BB52100	TG03	TG0300-170 - Traffic Control TG0300-172 - Traffic Control Minna and Natoma TG0300-173 - Traffic Control Howard St. Gate TG0300-174 - Traffic Control Beale St. TG0300-177 - Traffic Control PG&E Phase II at Fremont St.	Traffic Control	5/11/2011	5/11/2011	Daily Report
BSE-002	SX-BB51900, SX-BB52000	TG03		Pre-Trench	3/30/2011	3/30/2011	Daily Report
BSE-003	SX-BB43140, SX-BB51600	TG03	TG0300-300 - Pile Removal - Trial Extraction Plan and Design Report	Test Pile Extraction	3/28/2011	3/29/2011	Daily Report
BSE-004	SX-BB51700, SX-BB51800	TG03	TG0300-310 - Pile Removal - Production Extraction Plan TG0300-311 - Existing Pile Extraction Documentation	Pile Extraction Production	4/11/2011	4/11/2011	Daily Report
BSE-005	SX-BB52400, SX-BB52500	TG03		Test CDSM Shoring Wall	5/2/2011	5/2/2011, 7/7/2011	Daily Report
W0000-011400W01.10 Contractor Quality Control Plan	SX-BB52600, SX-BB52700	TG03	TG0300-410 - Struct.I Steel - Part 1 TG0300-411 - Struct. Steel - Qualifications of Welders TG0300-412 - Struct. Steel - Mfr.'s Submittals (On-going) TG0300-413 - Struct. Steel - Contractor's QA Plan & Inspector Certs TG0300-414 - Struct. Steel - Add'l Weld Procedures TG0300-415 - Struct. Steel - Add'l Welding Wire Product Data TG0300-416 - Struct. Steel - Add'l Weld Procedure - 30 Degree Welding TG0300-580 - Shoring Wall TG0300-581 - Shoring Wall - LEED Submittal TG0300-582 - BBI - Shoring Wall - CDSM Test Section No. 2 (Zone 1) TG0300-583 - CDSM Wall Corrective Action Plans TG0300-584 - CDSM Wall Beam #859 Alignment Corrective Action Plan TG0300-585 - CDSM Corrective Action - Resilping Soldier Piles TG0300-590 - Shoring Wall - Record Doc.				WO-CQC0001 - Contractor Quality Control Plan
				CDSM Shoring Wall Production	6/1/2011	7/7/2011	Daily Report
BSE-007	SX-BB52800, SX-BB52900	TG03	TG0300-380 - Concrete - General Site Mix Design TG0300-381 - Concrete - CLSM Mix Design TG0300-382 - Concrete - CLSM Mix Designs - Buttress Shoring Wall & Pile Extraction TG0300-383 - Concrete - CLSM & Concrete Mix Designs - Buttress Shoring Work Pad TG0300-385 - Buttress Concrete - Trial Batch Program TG0300-386 - Buttress Concrete - Type 'B' Secondary Shaft Mix Design TG0300-387 - Buttress Concrete - Primary Shaft Buttress Mix Designs TG0300-388 - Buttress Concrete - Primary Shaft Buttress Mix Designs - Add'l Mixes TG0300-389 - Buttress Concrete - Sechelt Coarse Aggregate TG0300-390 - Buttress Concrete - LEED TG0300-391 - Buttress Concrete - Primary Shaft Buttress Mix Designs - Add'l Mixes II TG0300-400 - Buttress Concrete - Closeout TG0300-600 - Drilled Shafts TG0300-601 - Drilled Shafts - Installation Plan - Supplemental Submittals TG0300-610 - Drilled Shafts - Close Out				
				Install Buttress Shafts	8/30/2011	9/13/2011	Daily Report
BSE-008	SX-BB53000, SX-BB53100	TG03	TG0300-320 - Rebar - Informational Submittals and buttress Shop Dwgs.	Buttress Rebar	8/1/2011	10/26/2011, 10/31/2011	Daily Report
BSE-009	UT-203801, UT-203901	TG03	TG0300-901 - CR T-017R1 PG&E Phase II Work at First St. TG0300-903 - PG&E Phase II Work at Fremont St.	PG&E Phase 2 Infrastructure	10/18/2011	10/19/2011	Daily Report
BSE-010	SX-BB10780, SX-BB10880	TG03	Complete	Demo Basement	11/9/2011	11/28/2011, 6/11/2011	Daily Report

DFOW Number	Baseline Schedule Activity ID	Specification Section	Required Submittals	Discription/Feature of Work	Preparatory Phase Date	Initial Phase Date	Follow Through Phase Date
BSE-011	SX-BB17300, SX-BB53400	TG03	TG0300-490 - Geotechnical Instrumentation & Monitoring TG0300-491 - Internal Bracing Performance Monitoring TG0300-540 - Internal Bracing - Engineer & Peer Reviewer Information & Qualifications TG0300-541 - Internal Bracing - 50% Design Dwg's & Calculations TG0300-542 - Internal Bracing - 100% Design TG0300-543 - Internal Bracing - Installer Qualification, QC/Construction, & Inspection Plan TG0300-544 - Internal Bracing - Manufacturer's Certifications or Coupon Testing TG0300-545 - Internal Bracing - Preloading Procedures TG0300-546 - Internal Bracing - Qualifications of Welders TG0300-547 - Internal Bracing - Welding Procedures TG0300-548 - Internal Bracing - Welding Procedures (Shop Welding) TG0300-549 - Internal Bracing - Welding Procedures - Add'l TG0300-550 - Internal Bracing - Re-Bracing TG0300-551 - Internal Bracing Erection Dwg's.	Install Walers (Internal Bracing)	11/15/2011	1/13/2012	Daily Report
WO000-011	SX-BB10680, SX-BB52300	TG03	TG0300-420 - Mass Excavation - Qualified Person and Quality Plan TG0300-430 - Mass Exc. - Material Samples TG0300-440 - Mass Exc. - Material Backfill TG0300-450 - Mass Exc. - LEED TG0300-460 - Mass Exc. - Work Plan	Mass Excavation/Wood Pile Extraction	12/14/2011	1/13/2011, 6/15/2012	Daily Report
BSE-013	SX-BB17600, SX-BB53300	TG03	TG0300-280 - Access Trestle TG0300-281 - CLSM Mix for Pin Pile & Trestle Pile Installation TG0300-290 - Access Trestle - Preconstruction Photos	Install Pin Piles	1/25/2012	1/27/2012	Daily Report
BSE-014	SX-BB15200, SX-BB52200	TG03	TG0300-280 - Access Trestle TG0300-281 - CLSM Mix for Pin Pile & Trestle Pile Installation TG0300-290 - Access Trestle - Preconstruction Photos	Zone 1 Trestle (Combined with Pin Piles)	1/25/2012	2/8/2012	Daily Report
BSE-015	SX-BB10620, SX-BB53200	TG03	TG0300-520 - Dewatering TG0300-521 - Dewatering - Initial Installation Report TG0300-522 - Dewatering - System Pump Test TG0300-525 - Dewatering - System Pumping Data (Weekly) TG0300-527 - Dewatering - Pre-trenching Only	Dewatering	3/2/2012	3/7/2012	Daily Report
BSE-016		TG03		Struct Installation	3/7/2012	3/9/2012	Daily Report
BSE-017	SX-BB56312, SX-BB56412	TG03	TG0300-490 - Geotechnical Instrumentation & Monitoring TG0300-491 - Internal Bracing Performance Monitoring TG0300-540 - Internal Bracing - Engineer & Peer Reviewer Information & Qualifications TG0300-541 - Internal Bracing - 50% Design Dwg's & Calculations TG0300-542 - Internal Bracing - 100% Design TG0300-543 - Internal Bracing - Installer Qualification, QC/Construction, & Inspection Plan TG0300-544 - Internal Bracing - Manufacturer's Certifications or Coupon Testing TG0300-545 - Internal Bracing - Preloading Procedures TG0300-546 - Internal Bracing - Qualifications of Welders TG0300-547 - Internal Bracing - Welding Procedures TG0300-548 - Internal Bracing - Welding Procedures (Shop Welding) TG0300-549 - Internal Bracing - Welding Procedures - Add'l TG0300-550 - Internal Bracing - Re-Bracing TG0300-551 - Internal Bracing Erection Dwg's.	Trestle Struts / Supports (Part of Bracing)	3/15/2012	3/16/2012	Daily Report
BSE-018		TG03		Trestle Deck	4/20/2012	4/20/2012	Daily Report
BSE-018	SX-BB56912, SXBB75012	TG03	TG0300-281 - CLSM Mix for Pin Pile & Trestle Pile Installation TG0300-283 BSE Trestle Pile Material Product Data TG0300-290 - Access Trestle - Preconstruction Photos	Trestle Superstructure	4/20/2012	4/20/2012	Daily Report
BSE-019	SX-BB17100, SX-BB17700	TG03		Remove Struts			

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DFOW Number	Baseline Schedule Activity ID	Specification Section	Required Submittals	Discription/Feature of Work	Preparatory Phase Date	Initial Phase Date	Follow Through Phase Date
BSE-020	BG-BB12300, BG-BB42220	TG03	TG0300-620 - Micropiles TG0300-630 - Micropiles - Performance & Proof Test TG0300-640 - Micropiles - Grout Test	Test Micropiles	10/12/2012	12/6/2012	Daily Report
BSE-021	BG-BB42320, BG-BB42420	TG03	TG0300-620 - Micropiles - work Plan and Schedule, contractor Qualifications, Product Data, Equipment Descriptions, Installation Procedures, Working Drawings & calcs. TG0300-630 - Micropiles - Performance & Proof Test TG0300-640 - Micropiles - Grout Test	Micropile Production	10/12/2012	10/30/2012	Daily Report
BSE-022	SX-BB20800, SX-BB20900	TG03	TG0300-200 - Temp Bridges - Qualifications Data TG0300-201 - Temp Bridges - Struct. Dwgs & Calc TG0300-202 - Temp Bridges - Peer Review TG0300-203 - Temp Bridges - Utility Supports TG0300-204 - Temp Bridges - Traffic Plan - First, Fremont, Beale Streets TG0300-205 - Temp Bridges Geometrics - First, Fremont, Beale Streets TG0300-210 - Temp Bridges - Product Data TG0300-215 - Temp Bridges - Misc. Materials TG0300-220 - MUNI OCS Installation Plan Beale St. TG0300-230 - Temp Bridges - MUNI OCS Installation Plan First St. TG0300-240 - Temp Bridges - Welder AWS Cert. TG0300-244 - Temp Bridges - Steel Manufacturers Certificates or Coupon Tests TG0300-248 - Temp Bridges - Concrete Mix Designs TG0300-250 - Temp Bridges - Rebar Manufacturer Certificates TG0300-260 - Temp Bridges - Preconstruction Photos First St. TG0300-264 - Temp Bridges - Preconstruction Photos Fremont St. TG0300-268 - Temp Bridges - Preconstruction Photos Beale St.	First Street Bridge	4/4/2012	4/5/2013	Daily Report
BSE-023	SX-BB21000, SX-BB21100	TG03	TG0300-200 - Temp Bridges - Qualifications Data TG0300-201 - Temp Bridges - Struct. Dwgs & Calc TG0300-202 - Temp Bridges - Peer Review TG0300-203 - Temp Bridges - Utility Supports TG0300-204 - Temp Bridges - Traffic Plan - First, Fremont, Beale Streets TG0300-205 - Temp Bridges Geometrics - First, Fremont, Beale Streets TG0300-210 - Temp Bridges - Product Data TG0300-215 - Temp Bridges - Misc. Materials TG0300-220 - MUNI OCS Installation Plan Beale St. TG0300-230 - Temp Bridges - MUNI OCS Installation Plan First St. TG0300-240 - Temp Bridges - Welder AWS Cert. TG0300-244 - Temp Bridges - Steel Manufacturers Certificates or Coupon Tests TG0300-248 - Temp Bridges - Concrete Mix Designs TG0300-250 - Temp Bridges - Rebar Manufacturer Certificates TG0300-260 - Temp Bridges - Preconstruction Photos First St. TG0300-264 - Temp Bridges - Preconstruction Photos Fremont St. TG0300-268 - Temp Bridges - Preconstruction Photos Beale St.	First Street Bridge Utilities			Daily Report

WO0000-01100W01.10 - Contractor Quality Control Plan

W/O JV Transbay Terminal Center  
DFOW List  
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DFOW Number	Baseline Schedule Activity ID	Specification Section	Required Submittals	Discription/Feature of Work	Preparatory Phase Date	Initial Phase Date	Follow Through Phase Date
BSE-024	SX-BB48420, SX-BB48520	TG03	TG0300-200 - Temp Bridges - Qualifications Data TG0300-201 - Temp Bridges - Struct. Dwgs & Calc TG0300-202 - Temp Bridges - Peer Review TG0300-203 - Temp Bridges - Utility Supports TG0300-204 - Temp Bridges - Traffic Plan - First, Fremont, Beale Streets TG0300-205 - Temp Bridges Geometrics - First, Fremont, Beale Streets TG0300-210 - Temp Bridges - Product Data TG0300-215 - Temp Bridges - Misc. Materials TG0300-220 - MUNI OCS Installation Plan Beale St. TG0300-230 - Temp Bridges - MUNI OCS Installation Plan First St. TG0300-240 - Temp Bridges - Welder AWS Cert. TG0300-244 - Temp Bridges - Steel Manufacturers Certificates or Coupon Tests TG0300-248 - Temp Bridges - Concrete Mix Designs TG0300-250 - Temp Bridges - Rebar Manufacturer Certificates TG0300-260 - Temp Bridges - Preconstruction Photos First St. TG0300-264 - Temp Bridges - Preconstruction Photos Fremont St. TG0300-268 - Temp Bridges - Preconstruction Photos Beale St.	Fremont Street Bridge	4/4/2012	4/5/2012	Daily Report
			TG0300-200 - Temp Bridges - Qualifications Data TG0300-201 - Temp Bridges - Struct. Dwgs & Calc TG0300-202 - Temp Bridges - Peer Review TG0300-203 - Temp Bridges - Utility Supports TG0300-204 - Temp Bridges - Traffic Plan - First, Fremont, Beale Streets TG0300-205 - Temp Bridges Geometrics - First, Fremont, Beale Streets TG0300-210 - Temp Bridges - Product Data TG0300-215 - Temp Bridges - Misc. Materials TG0300-220 - MUNI OCS Installation Plan Beale St. TG0300-230 - Temp Bridges - MUNI OCS Installation Plan First St. TG0300-240 - Temp Bridges - Welder AWS Cert. TG0300-244 - Temp Bridges - Steel Manufacturers Certificates or Coupon Tests TG0300-248 - Temp Bridges - Concrete Mix Designs TG0300-250 - Temp Bridges - Rebar Manufacturer Certificates TG0300-260 - Temp Bridges - Preconstruction Photos First St. TG0300-264 - Temp Bridges - Preconstruction Photos Fremont St. TG0300-268 - Temp Bridges - Preconstruction Photos Beale St.	Fremont Street Bridge Utilities			Daily Report
BSE-025	SX-BB48620, SX-BB48720	TG03	TG0300-200 - Temp Bridges - Qualifications Data TG0300-201 - Temp Bridges - Struct. Dwgs & Calc TG0300-202 - Temp Bridges - Peer Review TG0300-203 - Temp Bridges - Utility Supports TG0300-204 - Temp Bridges - Traffic Plan - First, Fremont, Beale Streets TG0300-205 - Temp Bridges Geometrics - First, Fremont, Beale Streets TG0300-210 - Temp Bridges - Product Data TG0300-215 - Temp Bridges - Misc. Materials TG0300-220 - MUNI OCS Installation Plan Beale St. TG0300-230 - Temp Bridges - MUNI OCS Installation Plan First St. TG0300-240 - Temp Bridges - Welder AWS Cert. TG0300-244 - Temp Bridges - Steel Manufacturers Certificates or Coupon Tests TG0300-248 - Temp Bridges - Concrete Mix Designs TG0300-250 - Temp Bridges - Rebar Manufacturer Certificates TG0300-260 - Temp Bridges - Preconstruction Photos First St. TG0300-264 - Temp Bridges - Preconstruction Photos Fremont St. TG0300-268 - Temp Bridges - Preconstruction Photos Beale St.	Beale Street Bridge	4/4/2012	9/10/2013	Daily Report
			TG0300-200 - Temp Bridges - Qualifications Data TG0300-201 - Temp Bridges - Struct. Dwgs & Calc TG0300-202 - Temp Bridges - Peer Review TG0300-203 - Temp Bridges - Utility Supports TG0300-204 - Temp Bridges - Traffic Plan - First, Fremont, Beale Streets TG0300-205 - Temp Bridges Geometrics - First, Fremont, Beale Streets TG0300-210 - Temp Bridges - Product Data TG0300-215 - Temp Bridges - Misc. Materials TG0300-220 - MUNI OCS Installation Plan Beale St. TG0300-230 - Temp Bridges - MUNI OCS Installation Plan First St. TG0300-240 - Temp Bridges - Welder AWS Cert. TG0300-244 - Temp Bridges - Steel Manufacturers Certificates or Coupon Tests TG0300-248 - Temp Bridges - Concrete Mix Designs TG0300-250 - Temp Bridges - Rebar Manufacturer Certificates TG0300-260 - Temp Bridges - Preconstruction Photos First St. TG0300-264 - Temp Bridges - Preconstruction Photos Fremont St. TG0300-268 - Temp Bridges - Preconstruction Photos Beale St.	Beale Street Bridge			Daily Report

WO0000-011400W01.10 - Contractor Quality Control Plan

DFOW Number	Baseline Schedule Activity ID	Specification Section	Required Submittals	Discription/Feature of Work	Preparatory Phase Date	Initial Phase Date	Follow Through Phase Date
BSE-027	SX-BB53800, SX-BB53900	TG03	TG0300-200 - Temp Bridges - Qualifications Data TG0300-201 - Temp Bridges - Struct. Dwgs & Calc TG0300-202 - Temp Bridges - Peer Review TG0300-203 - Temp Bridges - Utility Supports TG0300-204 - Temp Bridges - Traffic Plan - First, Fremont, Beale Streets TG0300-205 - Temp Bridges Geometrics - First, Fremont, Beale Streets TG0300-210 - Temp Bridges - Product Data TG0300-215 - Temp Bridges - Misc. Materials TG0300-220 - MUNI OCS Installation Plan Beale St. TG0300-230 - Temp Bridges - MUNI OCS Installation Plan First St. TG0300-240 - Temp Bridges - Welder AWS Cert. TG0300-244 - Temp Bridges - Steel Manufacturers Certificates or Coupon Tests TG0300-248 - Temp Bridges - Concrete Mix Designs TG0300-250 - Temp Bridges - Rebar Manufacturer Certificates TG0300-260 - Temp Bridges - Preconstruction Photos First St. TG0300-264 - Temp Bridges - Preconstruction Photos Fremont St. TG0300-268 - Temp Bridges - Preconstruction Photos Beale St.	Beale Street Bridge Utilities	4/4/2012	9/10/2013	Daily Report
BSE-028	BG-BB11100, BG-BB42120	TG03	TG0300-340 - Rebar Shop Dwgs - Mud Slab TG0300-350 - Mud Slab Concrete - Submittal Schedule TG0300-355 - Mud Slab Concrete - Mix Design TG0300-360 - Mud Slab Concrete - Joint Locations TG0300-370 - Mud Slab Concrete - Hazardous Materials	FRP Concrete Mud Slab	12/20/2012	1/23/2013	Daily Report
BSE-029	BG-BB10600, BG-BB42520	TG03		Struct. Removal			

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DFOW Number	Baseline Schedule Activity ID	Specification Section/ Trade Group	Required Submittals	Discription/Feature of Work	Preparatory Phase Date	Initial Phase Date	Follow Through Phase Date
UT - 4.1-001	UT-002910, UT-003310	TG04.1		Sewer Natoma & Fremont	2/4/2011	2/4/2011	Daily Report
UT - 4.1-002	UT-002610, UT-002810	TG04.1		Water Natoma & Fremont Street	1/13/2012	1/13/2012	Daily Report
UT - 4.2-001	UT-213800, UT-214500	TG04.2	TG0402-020-Dewatering Plan TG0434-024-Proposed Method of Piholing TG0404-003-Formwork Material TG0434-002-Pipe Bedding (Crushed Rock)-Sample TG0434-003-Pipe Bedding (Crushed Rock)-Test Reports TG0434-005-Shoring Plan by Licensed CA Engineer	Trench and Excavation (AWSS)	3/26/2012	4/2/2012	Daily Report
UT - 4.2-002	UT-208000, UT-214600	TG04.2	TG0402-013-Welder Certification TG0402-008-Sample 8" pipe w/welded stops	Pipe Stop Welding (AWSS)	3/26/2012	4/2/2012	Daily Report
UT - 4.2-003	UT-208100, UT-208200	TG04.2	TG0402-016 M Squared - Cast in Place Valve Vault	CIP Concrete	6/7/2012	7/20/2012	Daily Report
UT - 4.2-004	UT-208300, UT-208400	TG04.2	TG0402-001 M Squared - Ductile Iron Pipe TG0402-006 M Squared - Pipe End Seal TG0402-008 M Squared - Sample 8" pipe w/welded stops TG0402-027 M Squared - Pipe Links and Sleeves TG0402-029 M Squared - Pipe Bedding Pea Gravel TG0406-008 M Squared - Steel Pipe Material TG0406-009 M Squared - Pipe Factory Test Results	Pipe Installation (AWSS)	3/26/2012	4/2/2012	Daily Report
UT - 4.2-005	UT-208500, UT-208600	TG04.2		Testing and Comissioning (AWSS)			
UT - 4.3-001	UT-030500, UT-030600	TG04.3		Water Howard and Beale Streets	1/13/2011	1/13/2011	Daily Report
UT - 4.4-001	UT-203700, UT-203800	TG04.4		AWSS Cap	3/3/2011	3/3/2011	Daily Report
UT - 4.4-002	UT-041000, UT-041100	TG04.4		Sewer on Natoma	2/4/2011	2/4/2011	Daily Report
UT - 4.4-003	UT-041400, UT-041500	TG04.4		Water on Natoma, First Streets	1/13/2011	1/13/2011	Daily Report
UT - 4.6-001	UT-002830, UT-002930	TG04.6		Pipe Installation Sewer/Sludge	6/21/2012	6/25/2012	Daily Report
UT - 4.6-002	UT-002830, UT-002930	TG04.6		Testing & Comissioning Sewer/Sludge	6/21/2012	6/25/2012	Daily Report
UT - 4.6-003	UT-002830, UT-002930	TG04.6		Trench and Excavation Sewer/Sludge	6/21/2012	6/25/2012	Daily Report

DFOW Number	Baseline Schedule Activity ID	Specification Section	Required Submittals	Description/Feature of Work	Preparatory Phase Date	Initial Phase Date	Follow Through Phase Date
BGP-001	TBD	02 41 02	02 41 02 - 1.6	Shoring Wall Demolition			Daily Report
BGP-002	BGS01-1140	03 xx xx	03 xx xx	Concrete-Forms/Place, Protection Slab	4/19/2013	7/31/2013	Daily Report
BGP-003	BGS01-1130	03 xx xx	03 xx xx	Concrete-Forms/Rebar/Structural Embeds/Place, Foundation Slab	7/24/2013	8/1/2013	Daily Report
BGP-004	BGS01-1160	03 30 20	03 30 20 - 1.3	Concrete-Place, Foundation Slab	8/1/2013		Daily Report
BGP-005	BGS01-5160, BGS01-5170	03 xx xx	03 xx xx	Concrete-Forms/Rebar/Structural Embeds/Place, Lower Concourse			Daily Report
BGP-006	BGS01-4220	03 15 00	03 15 00 - 1.4	Concrete-Waterstop, Install			Daily Report
BGP-007	TBD	05 50 10	05 50 10 - 1.4	Metals-Pre Fabrication	3/28/2013		Daily Report
BGP-008	TBD	05 50 10	05 50 10 - 1.4	Metals-Install			Daily Report
BGP-009	TBD	07 09 16	07 09 16 - 1.4	T&MP-Seismic Joint Assemblies, Mock up			Daily Report
BGP-010	TBD	07 09 16	07 09 16 - 1.4	T&MP-Seismic Joint Assemblies, Install			Daily Report
BGP-011	TBD	07 12 10	07 12 10 - 1.4	T&MP-Waterproofing, Mud Slab Penetrations	1/21/2013	1/22/2013	Daily Report
BGP-012	TBD	07 12 10	07 12 10 - 1.4	T&MP-Waterproofing, Below Grade Package			Daily Report
BGP-013	TBD	Sections 22 xx xx, 23 xx xx, 26 xx xx, 27 xx xx, 28 xx xx	Sections 22 xx xx, 23 xx xx, 26 xx xx, 27 xx xx, 28 xx xx	MEP - Mechanical Piping & Drainage; Electrical Raceway & Boxes; Communications Ducts & Raceways; and Fire Management System			Daily Report
BGP-014	TBD	23 57 34 Note: - includes associated work covered under Section 31 23 34, Trenching and Backfill	23 57 34 - 1.4 Note: - includes associated work covered under Section 31 23 34, Trenching and Backfill	HVAC-Ground Loop Heat Exchanger, Install / Testing / Thermal Conductivity Analysis / Water Treatment / Commissioning	2/25/2013	3/18/2013	Daily Report
BGP-015	TBD	26 05 27 - 1.4	26 05 27 - 1.4	Electrical-Grounding System, Installation and Testing	1/9/2013	1/22/2013	Daily Report

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DFOW Number	Baseline Schedule Activity ID	Specification Section	Required Submittals	Description/Feature of Work	Preparatory Phase Date	Initial Phase Date	Follow Through Phase Date
SSS-001	TBD	05 10 00	TBD	All Structural Steel			Daily Report
SSS-002	TBS	05 10 00	TBD	Elevator Guiderail Support Framing			Daily Report
SSS-003	TBS	5 10 00	TBD	Escalator Support			Daily Report
SSS-004	TBS	5 10 00	TBD	Stair Support Framing			Daily Report
SSS-005	TBS	5 10 00	TBD	Metal Decking Studs			Daily Report
SSS-006	TBS	5 10 00	TBD	Light Columns and Rings			Daily Report
SSS-007	TBD	5 10 00	TBD	OCS Attachment			Daily Report
SSS-008	TBD	5 10 00	TBD	Removal of Construction Trestle			Daily Report

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WO-CQC0001 - Contractor Quality Control Plan

<b>PREPARATORY PHASE CHECKLIST</b>		SPEC SECTION	DATE
(CONTINUED ON SECOND PAGE)		Enter Spec Section # Here	Enter Date (DD/MMM/YY)
CONTRACT NO Enter Cnt# Here	DEFINABLE FEATURE OF WORK Enter DFOW Here	SCHEDULE ACT NO. Enter Sched Act ID Here	INDEX # Enter Index# Here
<b>PERSONNEL PRESENT</b>	GOVERNMENT REP NOTIFIED _____ HOURS IN ADVANCE: YES <input type="checkbox"/> NO <input type="checkbox"/>		
	NAME	POSITION	COMPANY/GOVERNMENT
<b>SUBMITTALS</b>	REVIEW SUBMITTALS AND/OR SUBMITTAL REGISTER. HAVE ALL SUBMITTALS BEEN APPROVED? YES <input type="checkbox"/> NO <input type="checkbox"/>		
	IF NO, WHAT ITEMS HAVE NOT BEEN SUBMITTED? _____		
	ARE ALL MATERIALS ON HAND? YES <input type="checkbox"/> NO <input type="checkbox"/>		
	IF NO, WHAT ITEMS ARE MISSING? _____		
<b>MATERIAL STORAGE</b>	ARE MATERIALS STORED PROPERLY? YES <input type="checkbox"/> NO <input type="checkbox"/>		
	IF NO, WHAT ACTION IS TAKEN? _____		
<b>SPECIFICATIONS</b>	REVIEW EACH PARAGRAPH OF SPECIFICATIONS. _____		
	DISCUSS PROCEDURE FOR ACCOMPLISHING THE WORK. _____		
	CLARIFY ANY DIFFERENCES. _____		
<b>PRELIMINARY WORK &amp; PERMITS</b>	ENSURE PRELIMINARY WORK IS CORRECT AND PERMITS ARE ON FILE.		
	IF NOT, WHAT ACTION IS TAKEN? _____		

TESTING	IDENTIFY TEST TO BE PERFORMED, FREQUENCY, AND BY WHOM.	
	WHEN REQUIRED?	
	WHERE REQUIRED?	
	REVIEW TESTING PLAN.	
SAFETY	ACTIVITY HAZARD ANALYSIS APPROVED? YES <input type="checkbox"/> NO <input type="checkbox"/>	
	REVIEW APPLICABLE PORTION OF EM 385-1-1.	
MEETING COMMENTS	NAVY/ROICC COMMENTS DURING MEETING.	
OTHER ITEMS OR REMARKS	OTHER ITEMS OR REMARKS:	
		DATE _____

INITIAL PHASE CHECKLIST		SPEC SECTION	DATE
CONTRACT NO		DEFINABLE FEATURE OF WORK	SCHEDULE ACT NO.
		INDEX #	
PERSONNEL PRESENT	GOVERNMENT REP NOTIFIED _____ HOURS IN ADVANCE: YES <input type="checkbox"/> NO <input type="checkbox"/>		
	NAME	POSITION	COMPANY/GOVERNMENT
PROCEDURE COMPLIANCE	IDENTIFY FULL COMPLIANCE WITH PROCEDURES IDENTIFIED AT PREPARATORY. COORDINATE PLANS, SPECIFICATIONS, AND SUBMITTALS.		
	COMMENTS: _____		
PRELIMINARY WORK	ENSURE PRELIMINARY WORK IS COMPLETE AND CORRECT. IF NOT, WHAT ACTION IS TAKEN?		
WORKMANSHIP	ESTABLISH LEVEL OF WORKMANSHIP.		
	WHERE IS WORK LOCATED? _____		
	IS SAMPLE PANEL REQUIRED? YES <input type="checkbox"/> NO <input type="checkbox"/>		
	WILL THE INITIAL WORK BE CONSIDERED AS A SAMPLE? YES <input type="checkbox"/> NO <input type="checkbox"/>		
	(IF YES, MAINTAIN IN PRESENT CONDITION AS LONG AS POSSIBLE AND DESCRIBE LOCATION OF SAMPLE) _____		
RESOLUTION	RESOLVE ANY DIFFERENCES.		
	COMMENTS: _____		
CHECK SAFETY	REVIEW JOB CONDITIONS USING EM 385-1-1 AND JOB HAZARD ANALYSIS		
	COMMENTS: _____		
OTHER	OTHER ITEMS OR REMARKS		
<div style="text-align: right;">_____ DATE</div>			

### **3.0 ELEMENT 3 DESIGN CONTROL**

#### **3.1 INTRODUCTION**

#### **3.2 DESIGN/BUILD PACKAGES**

#### **3.3 ROLES & RESPONSIBILITIES OF THE OWNER AND THE DESIGN BUILD TRADE SUBCONTRACTOR**

#### **3.4 AS-BUILT DRAWINGS**

#### **3.5 SUBMITTAL REVIEW**



### 3.0 DESIGN CONTROL

#### 3.1 INTRODUCTION

Design control as implied in this Element is limited to Design-Build packages where applicable, as-build drawings and submittal review and coordination by Webcor/Obayashi is primarily accomplished by QC Management, Oversight and coordination design/build package, where specified and ensuring that the design requirements are understood, planning the design interfaces and design verification activities, executing the design verification activities, and controlling design changes through project completion.

The designer shall prepare a plan for design/built activities. It should also identify the various organizational interfaces required between various groups producing and commenting on the design, and specify the information to be documented, transmitted, and regularly reviewed.

Appropriate procedures shall be established for the identification, documentation, review, and approval of all changes and modifications to the design. This responsibility should extend to those responsible for construction or manufacturing to ensure compliance to design requirements and for development of "as-built" documents as part of the design documentation at the end of the project.

Each group responsible for design/built shall provide its own written QC procedures. These include peer review of drawings and check calculations. QA activities are performed to verify compliance to established QC procedures and to determine the effectiveness of the procedures in meeting quality program objectives.

Specification Section 01-14-00 Quality Control Paragraph 1.6 B. Procedures for scheduling, reviewing, certifying, and managing submittals, including those of Trade Subcontractors, offsite fabricators, Suppliers, and purchasing agents. These procedures must be in accordance with Section 01 13 00, Submittals.

#### 3.2 DESIGN BUILD PACKAGES

W/OJV Shall:

- Clearly define requirements of the QA/QC Program in the contract documents.
- Coordinate with owner agency oversight activities in order to assure effectiveness of the QA/QC Program.

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- Require additional levels of reporting and/or detail by the DB contractor team.
- Clearly define roles and responsibilities of parties involved early in the bid documents.
- Maintain a proactive and systematic quality program that encompasses all the project lifecycle stages.

### **3.3 Roles and Responsibilities of the Owner and the Design-Build Trade Subcontractor**

QC program effectiveness hinges on clear allocation of roles and responsibilities to the involved parties. QA/QC roles and responsibilities shall be defined clearly in the contract documents; and more importantly, are agreed upon by the parties at the outset. It is recommended that the owner agency conduct audits and testing at every stage of the QC process, and retain ownership of the resident database. TJPA has elected to retain the Quality Assurance (QA) role with the design-build contractor performing the Quality Control (QC) activities.

### **3.4 As-Built Drawings**

Trade Subcontractors have design-build responsibilities (such as the access trestle and traffic bridges), their quality control plans shall include design control for their scope of work.

- The Trade Subcontractors shall keep an accurately marked, up-to-date set of as-built drawings for the work actually installed, and accurately indicate on as-built drawings all site conditions, locations of utilities, work scope changes, changes in dimensions, locations, and elevations of the Work, and changes in details as specified herein and as approved by the TJPA Representative. Trade Subcontractor shall keep the as-built drawings current as the Work is performed.
- Prior to acceptance of the Work, Trade Subcontractor shall furnish to the Webcor/Obayashi JV CQC Manager the final as-built drawings, showing all changes in the Contract Drawings neatly in red ink.
- Trade Subcontractors will delegate responsibility for maintenance, coordination, and accuracy of the as-built drawings to one person on their staff.
- Accuracy of as-built drawings shall be such that future searches for items shown on the Contract Documents may rely on information obtained from the approved as-built drawings.
- Trade Subcontractors shall store as-built drawings apart from documents used for performing the work; keep in a dry, legible condition, and in good order. Label each document "AS-BUILT DRAWINGS— JOB SET" in large, neatly printed letters.
- Trade Subcontractors shall record neatly on the as-built drawings all changes made by clarifications, Change Orders, Requests for Information, and other Modifications to the Contract Documents; and changes to reflect the actual

existing conditions and utility locations references to permanent accessible features of the Work.

- Trade Subcontractors shall clearly describe changes on as-built drawings by note as required.
- Trade Subcontractors shall date all entries, calling attention to the entry by a “cloud” drawing around the area or areas affected.
- Trade Subcontractors shall record in each Specification Section the manufacturer, trade name, catalog number, and supplier of each product and equipment item incorporated into the Work.
- Trade Subcontractors shall furnish a copy of the final shop drawings which have been updated to show actual conditions. Furnish additional drawings as necessary to record deviations from the sizes, locations, and other features of the Work and to locate piping, conduit, ductwork, and similar elements of utility installations by dimensions referenced to permanent accessible features of the Work.
- Trade Subcontractors shall show on the job set of as-built drawings, by dimension accurate to within 1 inch, the centerline of each run of conduits, circuits, piping, ducts, and similar items which are shown schematically on the Contract Drawings but where the final physical arrangement is determined by Trade Subcontractor.
- Trade Subcontractors shall keep as-built drawings up to date during the entire progress of the Work, and provide access for monthly. Updates shall be accurate and current and be done at the time work is performed.
- Trade Subcontractors shall also update and include the revised or newly issued drawings as part of the as built drawings. The work of reproducing and issuing Change Order drawings and updating of as built drawings shall be done as incidental work.

### 3.5 SUBMITTAL REVIEW

Submittals will be reviewed for coordination, completeness, clarity and coordination with other trades prior to submitting to the TJPA. To obtain approval from the Architect/Engineer/Consultant for all materials, assemblies, equipment and shop drawing submittals required by the contract documents.

The purpose is to install materials, assemblies and equipment only after approval is obtained from the appropriate reviewing Architect/Engineer/Consultant responsible for the particular scope of work.

- Webcor/Obayashi and TJPA process submittals using two different types of project management software. Webcor/Obayashi uses internal system and TJPA uses ConstructWare.
- In WOJV System submittal packages contain submittals and all of the history of the submittal is tracked at the submittal level. The submittal package is simply the nest of the submittals that are attached to it.

- Submittals are transmitted to TJPA from Webcor/Obayashi via WOJV internal system and ConstructWare.
  - The naming format of the PDF submittal is crucial for the transmission to be successful.
- Submittal Actions Status:

ACTION	STATUS
Received	Open
Sent	Submitted
Returned	No Exceptions Taken, Make Corrections Noted, Revise and Resubmit, or Rejected
Forwarded	Same as Returned Status
<u>For the Record</u>	<u>Submit for record only</u>

## **4.0 ELEMENT 4 DOCUMENT CONTROL**

### **4.1 INTRODUCTION**

### **4.2 SUBMITTAL MANAGEMENT**

### **4.3 SUBMITTAL MANAGEMENT AND DOCUMENT CONTROL PROCEDURES**

#### **4.3.1 DOCUMENT CONTROL**

#### **4.3.2 SUBMITTALS**

##### **SUBMITTAL REVIEW CHECKLIST**

#### **4.3.3 TRANSMITTALS**

#### **4.3.4 DISTRIBUTION MATRICES**

#### **4.3.5 MASTER PROJECT DOCUMENT LOG**

#### **4.3.6 CQC FILE STRUCTURE**

## 4.0 DOCUMENT CONTROL

### 4.1 INTRODUCTION

Webcor/Obayashi's Document Control process is the means by which information Specified in the Contract Documents to be in Webcor/Obayashi's and the Trade Subcontractors' control are logged, filed, and updated to assure that the organization's staff is using the most current approved documents and they are following the most recently approved procedures and standards and that are compliance with contract and applicable FTA, 15 Element Guidelines.

Procedures for control of project documents and data have been established and shall be maintained. The document control measures should ensure that all relevant documents are current and available to all users who require them.

Control of project documents includes the review of documents by authorized personnel, the distribution and storage of these documents, the elimination of obsolete documents, and control of changes to the documents. Copies of the documents shall be distributed so that they will be available at all locations that need them for effective functioning of the quality management system. Obsolete documents will be promptly eliminated from each work location. Any superseded documents retained for the record will be clearly identified as such. The same authorized personnel who reviewed and approved the original documents, unless the control procedures specifically allow otherwise, should review changes to the documents and data. Changes will be promptly distributed to all locations, along with a master list enumerating the current revisions of each document.

Following are examples of the types of documents requiring control:

- Drawings
- Specifications
- Inspection procedures
- Test procedures
- Special work instructions
- Operational procedures
- QA program and procedures

## 4.2 SUBMITTAL MANAGEMENT

The Submittal process is designed to assure that all material, assemblies, equipment and shop drawings meet the Transbay Transit Center project requirements and are approved by the TJPA prior to procurement and installation. The Submittal process is the means by which the Trade Subcontractors control product purchasing. This submittal schedule will be developed incrementally and additional submittals will be added as trade packages are awarded and subcontractors are brought on board. Trade Subcontractors will submit their submittal schedules compliance with contract and FTA element guidelines for approval, as required in the Division 00, 01 and technical specifications, prior to the start of work. Element 4 guidelines state that control of project documents includes the review of documents authorized personnel, the distribution and storage of these documents, the elimination of obsolete documents and control of changes to the documents.

## 4.3 SUBMITTAL MANAGEMENT AND DOCUMENT CONTROL PROCEDURES

The Webcor/Obayashi JV Document Control and Submittal management procedures are part of Webcor/Obayashi's Transbay Transit Center Policy and Procedures Guide. The relevant sections of that guide addressing submittal management and document control are listed below and are included in this section of the Webcor/Obayashi JV CQC Manual:

- |                        |                                    |
|------------------------|------------------------------------|
| 4.3.1 Document Control | 4.3.4 Document Distribution matrix |
| 4.3.2 Submittals       | 4.3.5 Master project document log  |
| 4.3.3 Transmittals     | 4.3.6 CQC file structure           |

### 4.3.1 DOCUMENT CONTROL

The purpose of this outline is to provide guidelines for establishing the appropriate D document control system for the management of the Transbay Transit Center project. This will include the review of documents by authorized personnel. All Controlled documents will go through Document Control to be logged and tracked.



**What is a controlled document?** A controlled document is defined for this project as any contract document or correspondence which includes i) contract requirements, or ii) scope definition or requirements, including distribution of all Contract Documents (e.g. addendum, ASI's bulletins, work orders, etc.) either to/from TIPA or Trade Subcontractor. Controlled documents received will be date stamped, logged, saved electronically (in some cases hard copies filed), distributed internally, monitoring response/process time (also referred to as work flow), distribute externally, and track the distribution list.

The following is a list of **controlled document** examples:

- Project Document Distribution – Internal/External
  - Design Documents
  - Construction Document
  - ASI's
  - Sketches- to be issued with ASI's or RFI's and not on their own.
  - Reference Documents
- Submittals, including all LEED submittal requirements and substitutions.
- Design Review Questions (DRQs) - Preconstruction
- Request for Information (RFIs) - Construction
- Daily Reports and Daily Quality Control Reports
- Safety Memos – Logged and tracked
- Schedules and schedule reports
- Permit Inspections
- Payment Applications
- Cash Flow Projections
- Monthly Progress Reports
- Permits
- Original Documents - Custodianship of all original documents in a Master File until they can be boxed and transferred for long term storage.
- Formal Correspondence; including all formal incoming/outgoing correspondence
- Contract Notification Correspondence; delay notification, etc.
- Contract Modifications
- Virtual Building/Models
- Meeting Minutes
- Transmittals
- Requests for Qualification (RFQ)
- Invitation for Bid (IFB)
- Subcontracts & Change Orders
- Long Form/Short Form Purchase Orders (PO)
- SBE/DBE
- Closeout documents

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- Reimbursements

**Uncontrolled Documents:** The following are some examples of uncontrolled documents:

- Email correspondence
- Field Tags – Collected and tracked by Cost Control
- Purchase Order – Managed by Procurement/Cost Control

#### 4.3.2 SUBMITTALS

Submittals will be reviewed for coordination, completeness, clarity and coordination with other trades prior to submitting to the TJPA. To obtain approval from the Architect/Engineer/Consultant for all materials, assemblies, equipment and shop drawing submittals required by the contract documents.

The purpose is to install materials, assemblies and equipment only after approval is obtained from the appropriate reviewing Architect/Engineer/Consultant responsible for the particular scope of work.

- Webcor/Obayashi and TJPA process submittals using two different types of project management software. Webcor/Obayashi uses internal and TJPA uses ConstructWare.
- In WOJV System submittal packages contain submittals and all of the history of the submittal is tracked at the submittal level. The submittal package is simply the nest of the submittals that are attached to it.
- Submittals are transmitted to TJPA from Webcor/Obayashi via WOJV internal system and ConstructWare.
  - The naming format of the PDF submittal is crucial for the transmission to be successful.

- Submittal Actions Status:

ACTION	STATUS
Received	Open
Sent	Submitted
Returned	No Exceptions Taken, Make Corrections Noted, Revise and Resubmit, or Rejected
Forwarded	Same as Returned Status
<i>For the Record</i>	<i>Submit for record only</i>

#### Receive Submittal from Subcontractor – 0-5 days

Was it received on time? If not, have the Trade Scope PM notify the subcontractor that it was late. Is the submittal complete? If not, return the submittal to the subcontractor, transmittal shall include notification that the submittal is incomplete, give a date that the re-submittal is required, and notify them of their potential risk in missing the submittal date.

Review the submittal using the submittal process checklist once the submittal is deemed complete, stamp, (All pages of shop drawings; front page only for product data), distribute to PM, QC and Supt. to review for conformance, completeness, compliance, clarity and transmit to TJPA.

Design Team Review – 12 days Design team will review the submittal. Each layer of review (Architect and Consultants) will stamp ALL pages and return to Webcor/Obayashi's document control manger.

Returned Submittal - 5 days

Reviewed by Document Manager – Notify Author. Document Control will receive e-mail notification that the submittal has been reviewed in ConstructWare. Document Control will forward the e-mail notification along with all attachments to Author.

PM Triage – Notification Sent to Subcontractors

- Revise & Re-submit or Rejected
  - Return R&R or Rejected submittal to author subcontractor. PM will include in the transmittal a due date for re-submittal (5 days). Director will make a case-by-case determination on whether to send a preliminary submittal to other subcontractors for coordination.
- No Exceptions Taken & Make Corrections Noted
  - Email author subcontractor and all affected trade subcontractors the approved submittal. PM will include transmittal with the action required.

Is there a Cost / Schedule Impact or Scope Change?

Subcontractors have 5 days from the returned date to respond with a cost or schedule impact.

Written Notification to Owner, draft RFI to Capture Cost.

Shop drawings, product data, and samples "are not contract documents" per our contract language. Therefore, any change in scope change during submittal review by design team must be captured via ASI. Director should also send written notification to ownership of any scope change incurred from a returned Submittal.

Storing Approved Submittals

Author of submittal will file all documents and correspondence within the storage folder and post the documents electronically.

- Put approved electronic copy of submittal in the designated folder



## SUBMITTAL PROCESS CHECKLIST

Submittal Package No.: \_\_\_\_\_ Date Received: \_\_\_\_\_

Submittal Name: \_\_\_\_\_

- ☐ Review each submittal to:
- ☐ Verify that the submittal's contents match the accompanying transmittal. Did we receive everything listed on the transmittal?
  - ☐ Verify that the submittal's contents are complete per the submittal register. Important: submittal packages need to be complete and should include all information necessary for review. Partial submittals are to be rejected by W/O (if we don't the TJPA will).
  - ☐ Verify that the contents of the submittal are in conformance with the technical specifications and other appropriate contract documents.
  - ☐ Is the Submittal a Substitution?
    - ☒ No- Continue Processing Submittal
    - ☐ Yes -Reject submittals that are substitution requests- There is a separate process for substitutions.
  - ☐ Verify that the trade subcontractor has checked and coordinated all dimensions, materials, field measurements, with the requirements of the Work and the Contract Documents.
  - ☐ Verify that the submittal complies with the requirements of reference specifications –SFDPW, PG&E etc.
  - ☐ Confirm that all professional certifications (stamp) w/license number and expiration date are provided and signed if required.
  - ☐ Note any variations from the Contract requirements (if there are create an issue in CMiC)
- No questions Address all questions raised or noted in the submittals; requests to verify dimensions, etc. If there are questions with the submittal:
- ☐ Can the questions be answered by W/O?
  - ☐ Does an RFI need to be submitted?
  - ☐ Does an issue need to be created in CMiC?
  - ☐ Identify who is responsible for answering the question
- ☐ Identify all affected and adjacent trades that can be potentially impacted by submittal. Develop an action plan to coordinate submittal information with ALL affected and adjacent trades.
- ☐ If the submittal is complete, stamp the first page of each item. If it is shop drawings, all sheets must be stamped.

Trade Scope Superintendent: \_\_\_\_\_

Date: \_\_\_\_\_

Trade Scope PM: \_\_\_\_\_

Date: \_\_\_\_\_

CQC Manager: \_\_\_\_\_

Date: \_\_\_\_\_

Safety

Manager: \_\_\_\_\_

Date: \_\_\_\_\_

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#### 4.3.3 TRANSMITTALS

To ensure controlled contract documents leaving this office have a record.

Use and receipt of Transmittals is governed by the information herein.

All controlled contract document exchange with Ownership, Design Team, Subcontractor community and Agencies with Jurisdiction/Authority on the project requires a transmittal. All transmittals are created in CMiC with the reference documents listed and uploaded as attachments in CMiC. All transmittals with incoming documents are date stamped, scanned and uploaded with the documents to the pertinent folder and CMiC.

Below is a listing of all contract documents that require a transmittal to capture the exchange/submission:

- Billing
- Submittals
- Design Review reports
- Schedules & Reports
- Cost Estimates
- Drawings
- Close-out documents
- Attic Stock

Transmittal tracking numbers are auto populated in CMiC.

**Subject (RE):** The subject should be the same description used on other documents (ex. PCI's, Letters, e-mail, etc.) Subject should be descriptive and should include appropriate sub-job, TG Package # and description.

**Remarks:** In the section, the first sentence should read

RE: Transbay Transit Center [Preconstruction/TCB/Utilities/Bus Ramps select one] – 30100.[##]

#### 4.3.4 DISTRIBUTION MATRIX

To establish guidelines for who receives what documents and in what form.

All documents received by Document Control will be distributed according to the matrices.

Distribution Matrices have been established for:

1. Internal Distribution
2. External Distribution

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		General							Construction					
P = Primary cc = copy		Contract Issues	Amendment/CR/CCO	Progress Billings	Schedule	Quality	Safety	Pre Construction	Transit Center Bldg				Utility Relocation - 30100.03	
									TG03-BSE	TG05-Logistics	TG06-Below Grade	TG07.1 Superstructure		
Group                      Name		All Correspondence							Field Orders					
									Submittals					
									Inspections					
									RFI's					
									PCO's					
MANAGEMENT	Jes Pedersen	cc												
	Hidetake Taniguchi	cc	cc	cc	cc	cc	cc	cc	cc					
	Steven Humphreys	P	P		cc	cc	cc		cc	cc	cc	cc	cc	cc
	Todd Mercer	cc	cc	cc		cc	cc	cc						
	Kurt Ricci	cc	cc	cc	cc	cc	cc	cc	cc	cc	cc	cc	cc	cc
PROJECT ACCT	Jasmin Lautt		cc	P										
PROJECT ASSISTANT / ADMIN	Anne Merics			cc										
	Sarah Boyd			cc					cc	cc	cc	cc	cc	cc
	Julie O'Brien		cc											
CONTROLS/SBE	Ted Williams	cc	cc	cc					cc	cc	cc	cc	cc	cc
SAFETY	Jack Storace						P							
QUALITY CTRL	Adib Sassine					P		cc	cc	cc	cc	cc	cc	cc
	Duncan Sinclair					cc		cc	cc	cc	cc	cc	cc	cc
	Lynn Kowallis					cc		cc	cc	cc	cc	cc	cc	cc
SCHEDULING	Ryan Burke	cc	cc		P	cc	cc	cc	cc	cc	cc	cc	cc	cc
	Jose Ramirez				cc									
VIRTUAL BLDG	Mike Brown							cc						
TRANSIT CENTER BLDG 30100.01	Joanne Verrips		cc		cc	cc	cc		P		cc			
	Spencer Sayles		cc		cc	cc	cc		cc		P			
	Ryan Burke		cc		cc	cc	cc		cc		cc			
	RJ Kjome		cc		cc	cc	cc		cc		cc	cc		
	Mike Spillane		cc		cc	cc	cc		cc		cc			
	Jose Verduzco		cc		cc	cc	cc		cc		cc			
	Mario Saldana		cc		cc	cc	cc		cc	cc	cc			cc
	Jordan Smith		cc		cc	cc	cc		cc		cc	cc		
	Jeff Galoyan		cc		cc	cc	cc				cc	P		
UTILITY RELOCATION 30100.03									cc	P	cc	cc		P
	Jackson Tukuafu													
BUS RAMPS 30100.05														
	Precon													
PRECONSTRUCTION 30100P	Jeff Heath				cc			P						
	Tomoya Imai							cc						
	Sihaya Roselle							cc						
	Dennis Blatchford							cc						
	Forrest McLain							cc						
	Tim Maxwell							cc						
	Masashi Kojima							cc						
	Lewis Hampton							cc						
	JD Flaming							cc						

**TRANSBAY TRANSIT CENTER**  
**WO-CQC0001 - Contractor Quality Control Plan**  
**DISTRIBUTION MATRIX**  
**WEBCOR/OBAYASHI**  
**External**

P = Primary  
CC = Copy

P = Primary CC = Copy		General Correspondence										Trade Specific Correspondence				Precon		Engineering		
		Contract Issues	Amendments/CO	Progress Billings	Schedule Updates	NOPD/NOPC	Quality	Safety	Cost Estimating/Constructability	LEED	Field Orders/PCO	Transit Center Bldg 30100.01				Utility Relocation - 30100.03	Bus Ramps - 30100.05	Bid Packages and Correspondence	QBDs	RFI's and Submittals
												TG03 - BSE	TG05 - Logistics	TG08 - Glazing	TG19 - Mission Wall					
Group	Name																			
Turner	Steve Rule	P	CC	CC	P	P	CC	CC	CC		CC	CC	P	P	P	P	P	P		
	Jack Adams				CC	CC	P	P				CC	CC	CC	CC	CC	CC			
	Jeremy Lau		CC	CC							CC									
	Gary Krutsch	CC	P	P	CC	CC			CC		P	CC	CC	CC	CC	CC	CC	P	P	
	Judy Long															CC		CC	CC	
	Jeff Thiel																			
	Stacy Wilson											CC						CC	CC	
	Steve Cunningham				CC	CC		CC				CC				CC				
	Turner Doccontrol	CC	CC	CC	CC	CC	CC	CC	CC		CC	CC	CC	CC	CC	CC	CC	CC	CC	
PMPC	Kathleen Lasse	CC	CC		CC	CC			P		CC									
	Jim Coughlin	CC	CC		CC	CC			P		CC									
	Joyce Oishi									P										
	Mark O'Dell	CC	CC		CC				CC		CC	CC	CC	CC			CC	CC	CC	
	Dan Alvarado				CC				CC		CC	CC		CC			CC	CC	CC	
	Guy Hollins															CC	CC			
	Phil Sandri														CC		CC			
	Bill Seaver																			
	Prasad Nimmigadda								CC											
	Roger Rothenburger	CC	CC						CC		CC	CC								
	Doug Jacobson											CC								
	Larry Zarembinski																			
	Jason Partin				CC															
	PMPC DocControl	CC	CC	CC	CC	CC	CC	CC	CC		CC	CC	CC	CC	CC	CC	CC	CC	CC	
	Brian Dykes					CC						P								
	Eddie Phillips	CC	CC	CC					CC		CC							CC		
	Dennis Turchon																			
	Sara Gigliotti	CC	CC	CC					CC		CC							CC		
	*TJPA DocControl	CC	CC	CC	CC	CC	CC	CC	CC		CC	CC	CC	CC	CC	CC	CC	CC	CC	

\*All correspondence for TJPA will be sent to Doc. Control and will direct correspondence for action, information, etc.

#### 4.3.5 MASTER PROJECT DOCUMENT LOG AND LIBRARY EXHIBIT

To track and document all drawings and specifications issued throughout the life of the project and where these documents live.

The master project document log will be updated by Document Control as new drawings and specifications are issued.

1. Review master drawing log against drawing log issued with new drawings.
2. Update master drawing log when new documents are received with date, revision number and location of where documents are saved.

NOTE – Master Drawing Log has not been established; PMPC to issue master log.

#### 4.3.6 CQC FILE STRUCTURE

The CQC File Structure is outlined below and will be utilized on this project to store, organize and manage Webcor/Obayashi's CQC Plan, Daily CQC Reports and DFOWs. **This File Structure will mirror that of Constructware.**

Webcor/Obayashi will organize and store CQC documents such as the CQC Plan, Daily CQC Reports and DFOWs on the F:\ drive in a shared folder. **All required quality records** will be uploaded into Constructware as the system of record.

**CQC documents on the F:\ drive may be found at the following location.**

F:\Transbay\WEBCOR\Quality Control

CQC Plans

- CQC Plan Webcor-Obayashi JV:

Daily CQC Reports

- Transbay
  - o WEBCOR
    - Quality Control
      - Daily CQC Reports
        - o Year
          - Month
            - Day
              - o Year/Month/Day – Contractor

DFOW

- Transbay
  - o WEBCOR
    - Quality Control
      - DFOW (By Contractor)

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- DFW Number's
  - Preparatory Phase
  - Initial Phase
  - Follow up
  - DFW Record Documents

**CQC Daily Reports in Constructware may be found at the following location.**

Constructware CQC Daily Reports

140 - Transit Center Building

- File Director
  - 10 Quality
    - 12 CQC Reports
      - Year
        - Month
          - Day
            - Month/Day/Year – contractor

**CQC DFW Reports in Constructware may be found at the following location.**

Constructware CQC DFW

140 - Transit Center Building

- File Director
  - 10 Quality
    - Definable Features of Work (DFW)
      - Contractor's DFW (Ex. BSE-TG03 – BBI)
        - DFW Log
        - DFW (By Number and Title)
          - Preparatory Phase
          - Initial Phase
          - Follow up Phase
          - DFW Record Documents

DFW – Any Reference to a DFW requires filing a copy of each Sub's QC **checklists** to retrieve follow up documents in F/drive and Constructware.



## **5.0** ELEMENT 5 **PURCHASING**

### **5.1** INTRODUCTION

### **5.2** CONTROL OF PURCHASED MATERIALS, PARTS AND COMPONENTS

## 5.0 PURCHASING

### 5.1 INTRODUCTION

The contract requirements will clearly specify the expectations of WOJV, including relevant standards, drawings, specifications, process requirements, inspection instructions, and approval criteria for materials, processes, and product. The purchasing documents will be reviewed and approved by WOJV and TIPA for adequacy of specified requirements prior to release. WOJV will ensure that the supplier fully understands the contract, agrees with the contract, and has the capacity to perform the work as required.

Where construction or equipment procurement is involved, the contract between WOJV and the supplier will specify the right of WOJV or TIPA authorized representatives to carry out as required inspection and testing at the source and upon receipt to verify that the work or product meets specifications.

Where equipment procurement is involved, WOJV will define, as appropriate, the means and methods for handling, storage, packaging, and delivery of product and as required per contract documents. WOJV will establish procedures to receive, inspect, store, and maintain equipment procured. Any equipment that is damaged or is otherwise unsuited for use will be documented and reported to the supplier or Trade Subcontractor.

Purchasing requirements apply to all subcontractors and suppliers, including construction contractors, and manufacturers. The purpose of this element is to ensure that purchasing requirements are clear and complete, that the supplier or trade subcontractor understands them, and that appropriate quality elements are made part of the contract. Additional requirements, such as on-site required inspection and handling and receiving procedures, may be required for construction or equipment procurement contracts.

Specification Section 01-16-00 Material and equipment referenced in this section.

Immediately upon delivery, Contractor shall inspect shipments to assure compliance with the Contract Documents and reviewed submittals, and to verify that products are undamaged and properly protected from potential damage. Undamaged products shall be delivered to the job site in manufacturers' sealed containers or wrappings with legends and labels intact. Contractor shall maintain packaged materials with seals unbroken and labels intact until time of use. "

### 5.2 CONTROL OF PURCHASED MATERIALS, PARTS AND COMPONENTS

- As part of bid package development Webcor/Obayashi JV will prepare trade package specific subcontractor prequalification requirements. These prequalification's are submitted to, and reviewed by the TIPA.

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The pre-qualification requirements are then included in the project bidding manual.

- Prior to contract award Webcor/Obayashi JV verifies that all trade subcontractors and suppliers meet the project requirements as outlined in the project bidding manual and contract documents.
- Schedule work to be tested or inspected to allow test to be performed within reasonable time.

## **6.0** ELEMENT 6 **PRODUCT IDENTIFICATION & TRACEABILITY OF MATERIAL, PARTS & COMPONENTS**

### **6.1** OVERVIEW

### **6.2** MATERIAL IDENTIFICATION

### **6.3** PRODUCT IDENTIFICATION AND TRACEABILITY

## 6.0 PRODUCT IDENTIFICATION AND TRACEABILITY

### 6.1 OVERVIEW

W/OJV and Trade Subcontractors will identify and document material and products delivered to the site using the material checklist. Material and products will be reviewed for deficiencies. Once a deficiency is identified by using the material checklist, there is a systematic method to control the item, correct it, and ensure that project quality is not adversely impacted.

When the material or product is identified as deficient it will immediately be segregated. Segregation may occur by physical isolation and cordoning off of work/materials, or conspicuously identified by tags/markings when physical isolation is not possible. BIM 360 will be used to identify deficient materials on equipment and track resolution and closure.

### 6.2 MATERIAL IDENTIFICATION

Measures shall be established and maintained for identifying and controlling items of production (batch, materials, parts, and components) to prevent the use of incorrect or defective items and to ensure that only correct and acceptable items are used or installed.

Physical identification and control shall be used to the extent possible. Where physical identification is impractical, physical separation, procedural control, or other appropriate means may be employed. Items that fail to possess identification, or items for which record traceability has been lost, or items that do not conform to requirements shall be segregated to prevent use or installation. An item shall be able to be identified by how it is marked or where it is located.

Specification Section 01-16-00 Material and equipment; 1.6 D & E  
Immediately upon delivery, Contractor shall inspect shipments to assure compliance with the Contract Documents and reviewed submittals, and to verify that products are undamaged and properly protected from potential damage.

1. Undamaged products shall be delivered to the job site in manufacturers' sealed containers or wrappings with legends and labels intact. Contractor shall maintain packaged materials with seals unbroken and labels intact until time of use.

2. Contractor shall promptly remove damaged material and unsuitable items from the job site, and promptly replace with material meeting the specified requirements at no increase in Contract Sum without impact to construction schedule.
3. Unsuitable materials and products not removed promptly from the job site by Contractor may be removed by the TJPA. Removal costs shall be paid by Contractor.
4. Contractor shall identify materials and equipment delivered to the Site to permit checking against submittals and shop drawings.

The TJPA may reject as non-complying such material and products that do not bear identification satisfactory to the TJPA as to manufacturer, grade, quality, and other pertinent information.

### 6.3 PRODUCT IDENTIFICATION & TRACEABILITY

Product identification and traceability shall take place during all the various production phases – from receipt of raw materials, components, or subassemblies through the manufacturing process, to delivery of final products or systems. Traceability shall mean traceable to Transbay Terminal Center project, specific warranty, test report, supplier, point in time, purchase order, or through production. Raw materials shall be traceable back to a particular batch number, shipment number, packing slip, or invoice and shall be accompanied by applicable test data sheets and material certifications. Store room or inventory tracking procedures shall allow for items to be traceable back to a particular order number, batch number, date received, test lot, or other pertinent source. Assemblies in production shall be traceable to Transbay Terminal Project through the use of some form of routing documentation. Routing documentation should contain sufficient manufacturing information, including work instructions, manufacturing standards, tooling, etc. Final assemblies should be clearly marked with project numbers, model numbers, serial numbers, bar codes, etc., so that all pertinent information regarding that assembly may be retrieved.

## 7.0 ELEMENT 7 PROCESS CONTROL

## 7.0 PROCESS CONTROL

The contractor quality control process is the means by which W/OJV, Trade Subcontractors and Suppliers shall identify and plan the production and installation processes.

Suppliers and Trade Subcontractors process control shall identify and plan the production and installation processes that directly affect quality and shall ensure these processes are performed under controlled conditions. Special processes, the results of which cannot be verified by subsequent inspection and testing of the product, shall be continuously monitored. To achieve accuracy and consistency in production and installation processes, the quality program shall provide for:

- Documented work instructions where such are needed to ensure quality, use of suitable production and installation equipment, a suitable working environment, personnel qualifications, and conformance with referenced standards/codes and Quality Plans
- Monitoring and controlling of processes and product characteristics during production and installation.

Continuous monitoring and/or conformance with documented procedures is required during special processes, such as welding, nondestructive testing, and heat treatment, where the results will impact quality of the final product, but where inspection after the fact will not reveal the deficiencies.

Ensure that work is performed in the proper sequence. For example, welds should be inspected before they are painted. Earth should be compacted before concrete is poured. Documented work instructions can help with sequence control where there is complex work or when there are multi-disciplined interfaces.

Procedures or guidance to be in conformance with contract and FTA Guidelines for Control of special processes by the Trade Contractors.

Sequence of work must be identified by subcontractor prior to final fabrication on installation. Documented work inspections are required per DFOW Preparatory meeting and will be the basis for process control.



## **8.0 ELEMENT 8 INSPECTION AND TESTING**

- 8.1** QUALITY INSPECTIONS
- 8.2** INSPECTION AND TESTING LABORATORY SERVICES
- 8.3** COORDINATION MEETING
- 8.4** TESTS
- 8.5** INDEPENDENT TESTING FIRM REPORTING REQUIREMENTS
- 8.6** TJPA CODE AND AGENCY TESTING AND INSPECTION
- 8.7** TJPA SPECIAL INSPECTION AND TESTING
- 8.8** INSPECTION REQUEST PROCEDURE
- 8.9** TEST AND INSPECTION PROCEDURES BY TRADE SUBCONTRACTORS
- 8.10** CONTROL VERIFICATION AND ACCEPTANCE TESTING PROCEDURE
- 8.11** PUNCH-OUT INSPECTION
- 8.12** PRE-FINAL INSPECTION
- 8.13** FINAL ACCEPTANCE INSPECTION
- 8.14** EXAMPLES OF DFOW CHECKLISTS

## 8.0 INSPECTION AND TESTING

### 8.1 QUALITY INSPECTIONS

The Webcor/Obayashi JV Quality Control Manager or CQC Manager's alternate will verify that Trade Subcontractors are meeting the requirements outlined in the TJPA Quality Management System Manual, sections 8.5.1 Inspection and Test Planning and 8.5.2 Contractor Inspection Requirements, to provide documented evidence of inspections, lab reports and test results as required per contract. The Trade Subcontractors will also perform required inspections of all purchased items, perform source inspections, perform first article inspections and perform end process inspections and testing. Webcor & Trade Subcontractors personnel will receive training on methods to physically inspect and document critical structural DFOW components prior to ISI inspection as TJPA's 3<sup>rd</sup> Party Inspector.

Inspection and Testing- Inspection and testing procedures should be planned and executed as necessary to verify quality. Procedures should be specified, implemented, and the results documented for receiving incoming products, and for final inspection and testing.

When products are delivered to W/OJV, it is the responsibility of W/OJV and trade subcontractor QC Manager to verify they are in conformance with requirements. Verification should be in accordance with the Quality Plan or documented procedures. The extent of receiving inspection can vary with the amount of inspection at the source, the safety criticality of the product, and the confidence in the quality procedures of the supplier.

In process testing and inspection of the work to verify conformance of an item or work activity to specified requirements, should be in a conformance with the Quality Plan on documented procedure process and balance to quality. Both inspection and process monitoring methods shall be performed, as necessary, to ensure that the specified requirements for the control of work processes and the quality of the item are being achieved throughout the duration of the work.

Final inspection and testing should ensure that all specified inspections and tests, including those specified for receipt of product or in-process work, have been carried out and the resulting data meet specifications. Final inspection and testing should be carried out and properly documented to ensure conformance of the finished product to the specifications.

Records should be maintained of the various inspections and tests to provide evidence that the product has passed inspection and/or test with defined acceptance criteria.

## **8.2** INSPECTION AND TESTING LABORATORY SERVICES (SPEC. SECTION 01 14 00)

Where specified, the TJPA Representative will appoint, employ, and pay for services of an independent firm to perform inspections, testing, and other services specified in individual specification sections and as required by the TJPA Representative.

Where specified, trade subcontractors will appoint, employ, and pay for services of an independent firm to perform inspections, testing, and other services specified in individual specification sections.

Control, verification, and acceptance testing procedures for each specific test to include the test name, specification paragraph requiring test, feature of work to be tested, test frequency, and person responsible for each test. (Laboratory facilities approved by the TJPA Representative must be used.)

## **8.3** COORDINATION MEETING ( SPEC. SECTION 01 14 00 - 1.7)

After the pre-construction conference for each Trade Work Package, before start of construction, Contractor and Trade subcontractor shall meet with the TJPA Representative and TJPA QA Manager and discuss the Contractor's quality control system as it relates to the work of the trade package. Submit the CQC Plan a minimum of 15 days prior to the coordination meeting. During the meeting, a mutual understanding of the system details must be developed, including the forms for recording the CQC operations, control activities, testing, administration of the system for both onsite and offsite work, and the interrelationship of Contractor's management and control with the TJPA Representative's quality assurance. Minutes of the meeting will be prepared by the TJPA Representative, signed by both the Contractor and the TJPA Representative and will become a part of the Contract file. There may be occasions when subsequent conferences will be called by either party to confirm mutual understandings and/or address deficiencies in the CQC system or procedures that may require corrective action by the Contractor.

## **8.4** TESTS (SPEC. SECTION 01 14 00 1.10)

Trade subcontractor shall perform specified or required tests to verify that control measures are adequate to provide a product that conforms to Contract requirements. Upon request, Contractor shall furnish to the TJPA duplicate samples of test specimens for possible testing by the TJPA. Testing includes operation and/or acceptance tests when specified. Procure the services of a certified testing laboratory. Perform the following activities and record and provide the following data.

- Verify that testing procedures comply with contract requirements.

- Verify that facilities and testing equipment are available and comply with testing standards.
- Check test instrument calibration data against certified standards.
- Verify that recording forms and test identification control number system, including all of the test documentation requirements, have been prepared.
- Record results of all tests taken, both passing and failing on the CQC report for the date taken. Specify paragraph reference, location where tests were taken, and the sequential control number identifying the test. If approved by the TJPA Representative, actual test reports may be submitted later with a reference to the test number and date taken. Provide an information copy of tests performed by an offsite or commercial test facility directly to the TJPA Representative. Failure to submit timely test reports as stated may result in nonpayment for related work performed and disapproval of the test facility for this Contract.

1.2. B Trade Subcontractor's QC service responsibilities:

- "Cooperate with testing agency personnel.
- Provide access to the Work.
- Obtain and handle samples of materials and equipment as defined in Section 01 13 00, Submittals.
- Furnish storage and assistance as requested.
- Facilitate inspections and tests.
- Notify the TJPA Representative in writing a minimum of 48 hours, excluding weekends and holidays, but not more than 72 hours prior to expected time for operations requiring as needed testing or inspection services.
- Schedule work to be tested or inspected to allow tests to be performed within reasonable time period.
- Where required, deliver samples to testing agency.
- When a specified test or inspection is not performed due to Contractor's failure to notify the TJPA Representative as specified or when material, or workmanship is not ready at the time specified, the TJPA Representative will establish remedial work, and Contractor shall bear the cost of remedy.
- Take steps necessary to ensure no portion of the work requiring testing or inspection is covered prior to acceptance by authorized parties.

- Ensure that no testing or inspection is scheduled until all approvals for the work have been received. This includes welder's certifications, submittals, design/build engineering stamp, and certification".

#### 1.3. A

"Contractor shall verify all dimensions in the field and shall check all field conditions continuously during construction. Contractor shall inspect related and appurtenant work and report in writing to the TJPA Representative any conditions that will prevent proper completion of the Work in accordance with the requirements of the Contract, Trade Subcontractor's QC service responsibilities."

#### 1.3. B

"Contractor shall be responsible for any Work that is non-conforming. Any required removal, repair, or replacement caused by non-conforming work shall be done by Contractor at no cost to the TJPA. Such nonconforming work will be considered as defective and payments will be withheld in accordance with Section 00 07 00, General Conditions, paragraphs 9.05 and 9.08."

#### 1.3. C

"Contractor shall be responsible for recording all changes and modifications to the Contract work as required by site conditions and inspections in accordance with the requirements of Section 01 17 20, Project As-Built Drawings."

### 8.5 INDEPENDENT TESTING FIRM REPORTING REQUIREMENTS

#### 1.5. A

"Where specified, the TJPA Representative will appoint, employ, and pay for services of an independent firm to perform inspections, testing, and other services specified in individual specification sections and as required by the TJPA Representative, or the TJPA Representative will perform the inspection and testing services."

"Inspection reports will be submitted promptly by the independent firm in triplicate and distributed, one copy each, to the TJPA Representative, Webcor/Obayashi JV QC Manager, and the code authority having jurisdiction over the Project and will indicate observations and results of tests and compliance or noncompliance with the requirements as defined in the technical specifications."

### 8.6 TJPA CODE AND AGENCY TESTING AND INSPECTIONS

Work shall be subject to testing and inspection by representatives of the TJPA and other agencies having jurisdiction (Code and Agency Inspections) to assure compliance with all requirements of Section 00 07 00, General Conditions, and Paragraph 8.02 and as per code requirements.

#### **8.7** TJPA SPECIAL INSPECTION AND TESTING

Where specified, the TJPA Representative will appoint, employ, and pay for services of independent firms to perform inspections, testing, and other services specified in individual specification sections and as required by the TJPA Representative or the TJPA Quality Assurance Representative will perform the inspection and testing services.

#### **8.8** INSPECTION REQUEST PROCEDURE

- The Trade Subcontractors CQC Manager will verify that all prerequisites as defined by the contract specifications are completed prior to Code, Agency or Special Inspections. Inspection Request will be submitted to the Webcor/Obayashi JV CQC Manager or CQC Alternate and the TJPA Construction Management Oversight Manager 48 hours and not more than 72 hours prior to the inspection date. Inspection Requests for Code, Agency and Special Inspections require an "Inspection Request Form" to be completed in BIM 360 Systems by Webcor/Obayashi JV or the Trade Subcontractors CQC Manager. The Trade Subcontractor's CQC Manager will facilitate onsite inspections, sampling procedures, test reports, and provide notification to the Webcor/Obayashi JV CQC Manager and TJPA representative when inspections fail or test results fall below specified values. Notify Turner if 48 hour notice cannot be met. Inspections will be submitted 48 hours (by 3:00pm) prior to the inspection date.

- Day 1 3:00pm is cut off time for any inspection on Day 3
- Thursday 3:00pm is cut off time for any inspection on the weekend or following Monday:
- Friday 3:00pm is cut off time for any inspection on the following Tuesday or later.

#### **8.9** TEST AND INSPECTION PROCEDURES BY TRADE SUBCONTRACTORS

When specified, the Trade Subcontractors shall include as part of their scope all tests to verify that the Work conforms to the Contract Documents and to the Quality Control specification section 01 14 00 Rev 0 paragraph 1.10A Tests. Contractor shall perform specified or required tests to verify that control measures are adequate to provide a product that conforms to Contract requirements. Upon request, Contractor shall furnish to the TJPA Representative duplicate samples of

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test specimens for possible testing by the TJPA. Testing includes operation and/or acceptance tests when specified. Procure the services of a certified testing laboratory. Perform the following activities and record and provide the following data:

1. Verify that testing procedures comply with the contract documents-Per Code and Contract Requirements.
2. Verify that all inspection prerequisites are met prior to conducting inspections.
3. Submit a testing and inspection matrix with the design submittals showing all required inspections and the entity responsible for performing the tests or inspections, *per DFOW requirements*.
4. Track inspection and test status.
5. Verify that the facilities and testing equipment are available and comply with the testing standards. As per approved submittals.
6. Trade Contractors and Suppliers shall have documented procedures to ensure test equipment is in calibration and keep updated lists of all equipment requiring calibration. Trade Contractor shall make calibration records available for review.
7. Record results of tests taken, both passing and failing on the trade subcontractor's daily CQC report for the date taken. Specify paragraph reference, location where tests were taken. Maintain a current test results spreadsheet per each different component.
8. When the services of an independent firm are utilized, reports will be submitted promptly by the independent firm in triplicate and distributed, one copy each, for the TJPA Representative, Webcor/Obayashi JV, and the code authority having jurisdiction over the Project and will indicate observations and results of tests and compliance or noncompliance with the Contract.
9. When specified, the Trade Subcontractors shall produce test and inspection plans in accordance with the Program Quality Management System requirements. All testing and measurements specified to be performed by the Trade Subcontractors shall be performed with equipment whose calibration
10. Meets national standards and to documented standards when no national standard exists.
11. Maintain and submit a log indicating the status of the Trade Subcontractors inspections and tests.
12. Verify that facilities and testing equipment are available and comply with testing standards.
13. Check test instrument calibration data against certified standards.

14. Verify that recording forms and the test identification control number system, including all of the test documentation requirements, have been prepared. Upload test records to BIM 360.
15. Record results of all tests taken, both passing and failing, on the CQC report for the date taken. Specify paragraph reference, location where tests were taken, and the sequential control number identifying the test. If approved by the TJPA Representative, actual test reports may be submitted later with a reference to the test number and date taken. Provide directly to the TJPA Representative an information copy of tests performed by an offsite or commercial test facility. Failure to submit timely test reports as stated may result in nonpayment for related work performed and disapproval of the test facility for this Contract.
16. WOJV and Subcontractors must confirm activities are ready for inspection prior to ISI start.
17. Verify to the Webcor/Obayashi JV CQC Manager of Trade Subcontractors task completion prior to the work being inspected.
18. Verify to the Webcor/Obayashi JV CQC Manager of Trade Subcontractors task completion prior to requesting final inspections.
19. Facilitate inspections and tests.
20. Cooperate with testing agency personnel.
21. Provide access to the Work.
22. Obtain and handle samples and equipment as defined in section 01 13 00 Submittals. Furnish storage and assistance as requested.
23. Trade Subcontractor shall include within their quality control plan per Specification Section 01 16 00 Material and Equipment, article 1.3 Quality Assurance, procedures for full protection of Work and materials.
24. Where required, deliver samples to testing agency.
25. Take steps to ensure no portion of the work requiring testing or inspection is covered prior to the acceptance by authorized parties.
26. Ensure that no testing or inspection is scheduled until all approvals for the work have been received. This includes welder's certifications, submittals, design/build engineering stamp and certification.
27. Notify the TJPA Representative in writing a minimum of 48 hours. Excluding weekends and holidays, but not more than 72 hours prior to expect time for operations requiring as needed testing and inspections.
28. DFOV task checklist will be implemented to assist with inspections and comply with the required codes and contract requirements.
  - A. The frequency of checklist reviews and style of checklist will vary for each DFOV task. The DFOV initial phase process will identify which entity (TJPA, W/O, Subcontractor) is performing what type of checklist review, the



frequency for check list reviews during the initial installation and follow up phases, and the style of checklist reviews.. The base understanding is that, each entity shall maintain records.

i. Subcontractor's:

1. Procedural Review Checklist.

- a. Confirm that submittals are approved before starting work, confirm that inspections have been scheduled, confirm that inspections as-builds are being maintained, confirm that protection of material is in place.

2. Material Controls Checklist,

- a. Each sub, for each key sequence, need to identify how they maintain records such that a deficiency in the field can be tracked back to the delivery/fabrication process. A material control checklist is the sub's QC representative review and confirmation that those procedures are being followed.

3. Completed Installation Technical Verification Checklist,

- a. This is the detailed list of installation requirements that the sub confirms prior to calling for an inspection.

ii. W/O QC:

1. Procedural Review Checklist

- a. Has the sub completed their technical check list, are they protecting their materials, have they complete a material controls checklist, etc.

2. Select Installation Technical Verification Checklist

- a. Selected items within a particular W/OJV DFOV task checklist are checked by W/OJV and used to spot check/confirm that the sub's detailed checklist is accurate. Why will these vary? Because with some scopes, i.e. Welding we don't have the accreditation to make any technical evaluations – it will be a procedural review for us. On the other hand, Rebar – it's Quantity, spacing, type of bar – things that can be visually confirmed and therefore we will do some technical reviews.

iii. TIPA:

WO0000-011400WO1.10

## 1. Procedural Review Checklist

### 8.10 CONTROL VERIFICATION AND ACCEPTANCE TESTING PROCEDURES

When specified, The Trade Subcontractors CQC Managers will provide control, verification, and acceptance testing procedures for each specific test to include the test name, specification paragraph requiring test, feature of work to be tested, test frequency, and person responsible for each test. (Laboratory facilities approved by the TJPA Representative must be used.).

When specified, specific control verification and acceptance testing procedures will be provided by the Trade Subcontractors as part of the Trade Subcontractors CQC plans, and will be completed as the specification sections are defined and the Trade Subcontractors are added to the project

### 8.11 PUNCH-OUT INSPECTION

An inspection of the Work will be conducted by the Trade Subcontractor QC Manager and verified by the Webcor/Obayashi JV CQC Manager, near the end of Trade Subcontractor's work. The punch list, entered into BIM 360 Systems, will include items that do not conform to the approved Drawings and Specifications and the estimated date by which the deficiencies will be corrected. A second inspection by the Trade Subcontractor CQC Manager will ascertain that all deficiencies have been corrected. Once this is accomplished the TJPA Representative will be notified that the facility is ready for the TJPA pre-final inspection.

### 8.12 PRE-FINAL INSPECTION

The TJPA Representative will perform the pre-final inspection to verify that the facility is complete and ready to be occupied. A TJPA Representative pre-final punch list may be developed as a result of this inspection. Webcor/Obayashi JV will ensure that all items on this list have been corrected before notifying the TJPA Representative, so that a final inspection can be scheduled. Items noted on the pre-final inspection will be corrected in a timely manner. These inspections and any deficiency corrections required by this paragraph must be accomplished within the time slated for completion of the entire work or any particular increment of the Work if the Project is divided into increments by separate completion dates.

### 8.13 FINAL ACCEPTANCE INSPECTION

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The CQC System Manager, plus the Contractor's authorized representative and the TJPA Representative must be in attendance at the final acceptance inspection. Additional personnel from affected third parties may also be in attendance. The final acceptance inspection will be formally scheduled by the TJPA Representative based upon results of the pre-final inspection. The TJPA Representative will be notified at least 72 hours prior to the final acceptance inspection and include the Contractor's assurance that all punch list and nonconforming work will be complete and acceptable by the date scheduled for the final acceptance inspection.

Summary

Name	Mud Slab Checklist Details
Description	<ul style="list-style-type: none"><li>• Printable version of your QA/QC, Safety, and Commissioning Checklists with responses and comments</li><li>• Optionally include checklist attachments and details of issues generated from the checklist</li></ul>
Report run on	30 Aug 2013 11:59 AM
Number of pages	2 including this summary page

Parameters

Show attachments: Checklist Details
Include comments: Yes
Include custom fields: Yes
Include issue details: Yes
Include n/a and blank responses: Yes
Include signatures: Yes
Show cover page: Yes
Report name: Mud Slab Checklist Details
Output format: Checklist Details
Show related equipment as: Checklist Details, Equipment Name

Transbay Transit Center - P1				Mud Slab Checklist Details			
Details							
ID	000358		Company		<not set>		
Name	WO - Mud Slab Concrete Pre-Placement Form				Priority	Medium	
Description					Status	Open	
Author	lkowallis@webcor.com				Location	<Top level>	
Created On	30 Aug 2013 11:59 AM						
Tags							
Checklist Items							
Item #	Item Text	Response	Comments	# Issues			
Mud slab Concrete Pre-Placement Checklist							
MUDS-1	Enter Review Area			0			
MUDS-2	Subgrade elevation for 4" slab checked by BBII (+/- 1/2")	Yes		0			
MUDS-3	Location and count of pits per latest Drawings	Yes		0			
MUDS-4	Location of pits verified and surveyed	Yes		0			
MUDS-5	Backfill compaction acceptance testing (BY ISI)			0			
MUDS-6	Subgrade ready to inspect	Yes		0			
MUDS-7	Subgrade acceptance by Arup			0			
MUDS-8	Grounding installed and accepted			0			
MUDS-9	Geothermal piping installed, tested, backfilled, accepted			0			
MUDS-10	Subgrade elevation restored and checked	Yes		0			
MUDS-11	Waterproofing/Butyl tape on all penetrations accepted	Yes		0			
MUDS-12	Micropiles installed	Yes		0			
MUDS-13	Micripiles tested (and/or blocked out)	Yes		0			
MUDS-14	Rebar installed and accepted	Yes		0			
MUDS-15	Elevation benchmarks established for concrete finishing	Yes		0			
MUDS-16	Concrete placement area clearly delineated	Yes		0			
MUDS-17	Concrete placement area cleared of all debris	Yes		0			
MUDS-18	All micropiles grout tubes filled with grout	Yes		0			
MUDS-19	All micropile grout exposed and free of soil and ridges	Yes		0			
MUDS-20	Waterproofing protection installed	Yes		0			
MUDS-21	Subgrade screed bars, ridges and forms installed	Yes		0			

Summary

Name	Waterproofing Checklist Details
Description	<ul style="list-style-type: none"><li>• Printable version of your QA/QC, Safety, and Commissioning Checklists with responses and comments</li><li>• Optionally include checklist attachments and details of issues generated from the checklist</li></ul>
Report run on	30 Aug 2013 12:43 PM
Number of pages	3 including this summary page

Parameters

Show attachments: Checklist Details
Include comments: Yes
Include custom fields: Yes
Include issue details: Yes
Include n/a and blank responses: Yes
Include signatures: Yes
Show cover page: Yes
Report name: Waterproofing Checklist Details
Output format: Checklist Details
Show related equipment as: Checklist Details, Equipment Name

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Waterproofing Checklist Details

ID

000359

Name

QC - Grace Subgrade Waterproofing

Description

Author

lkowallis@webcor.com

Created On

30 Aug 2013 12:06 PM

Tags

Company

Priority

Status

Location

<not set>

Medium

Open

<Top level>

Checklist Items

Item #	Item Text	Response	Comments	# Issues
Substrate Sign-Off checklist				
	Enter Review Area			0
C01	Are there any voids greater than .5 inches?	No		0
C02	Is there missing grout around any penetrations?	No		0
C03	Is there loose aggregate?	No		0
C04	Are there sharp protrusions?	No		0
C05	Is there any standing water?	No		0
C06	Is there substrate more than .5" out of alignment for vertical surfaces?	No		0
Membrane Installation:				
F01	Is the temperature below 25 F (-4 C) during installation?	No		0
F02	Did installer fail to use Tape LT during installation when temperature is less than 55 F (13 C)?	No		0
Tape LT Installation on Membrane:				
G01	Was the surface dirty or have debris on it during installation?	No		0
G02	Was the surface wet during installation?	No		0
G03	Is the release liner still in place after installation?	No		0
Membrane Horizontal Applications:				
H01	Is the HDPE film side faced away from substrate?	No		0
H02	Are the end laps missing the stagger?	No		0
Membrane Horizontal Overlap Requirements				
I01	Is the overlap less than 3" along marked seldedge?	No		0
I02	Is/was the underside of succeeding sheet dirty or wet?	No		0
I03	Is the release liner remaining in the overlap?	No		0
I04	Did the overlap fail to achieve a continuous bond?	No		0

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Transbay Transit Center - P1		Waterproofing Checklist Details		
Item #	Item Text	Response	Comments	# Issues
<i>Membrane Vertical Applications:</i>				
J01	Is the HDPE film side faced away from substrate?	No		0
J02	Is/was the underside of succeeding sheet dirty or wet?	No		0
J03	Is the plastic release liner still in place?	No		0
J04	Are the fasteners different than the submittal?	No		0
J05	Are fasteners in selvage large or high profile?	No		0
<i>Vertical Roll Edges &amp; Cut Edges</i>				
K01	Is the overlap less than 3"?	No		0
K02	Are their contaminants present?	No		0
K03	Is the Tape LT application off center?	No		0
K04	Is the release liner still in place on the LT tape?	No		0
<i>Membrane Repair (Small .5" or less)</i>				
L01	Is the damaged area dirty or otherwise not prep'd for repair?	No		0
L02	Is the damaged area missing Preprufe Tape?	No		0
L03	Is Preprufe Tape installed off center from the damaged area?	No		0
L04	Was the LT Tape release liner left in place?	No		0
<i>Membrane Repair (Large &gt; .5")</i>				
M01	Is the damaged area dirty or otherwise not prep'd for repair?	No		0
M02	Is the damaged area missing a Preprufe membrane?	No		0
M03	Is edge of the repair membrane less than 6" beyond damaged area?	No		0
M04	Are the patched edges missing Preprufe tape?	No		0
M05	Is the Prepruf tape off center from the edge?	No		0
M06	Was the release liner left on the T Tape?	No		0
M07	Did the edges fail to achieve adhesion?	No		0



Summary

Name	Protection slab Pre-placement Checklist Details
Description	<ul style="list-style-type: none"><li>• Printable version of your QA/QC, Safety, and Commissioning Checklists with responses and comments</li><li>• Optionally include checklist attachments and details of issues generated from the checklist</li></ul>
Report run on	30 Aug 2013 12:09 PM
Number of pages	4 including this summary page

Parameters

Show attachments: Checklist Details
Include comments: Yes
Include custom fields: Yes
Include issue details: Yes
Include n/a and blank responses: Yes
Include signatures: Yes
Show cover page: Yes
Report name: Protection slab Pre-placement Checklist Details
Output format: Checklist Details
Show related equipment as: Checklist Details, Equipment Name

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Protection slab Pre-placement Checklist Details

Details

ID

000360

Name

WO - Protection Slab Concrete Pre-Placement Form

Description

Author

lkowallis@webcor.com

Created On

30 Aug 2013 12:08 PM

Tags

Company

Priority

Status

Location

<not set>

Medium

Open

<Top level>

Checklist Items

Item #	Item Text	Response	Comments	# Issues
Protection Slab Concrete Pre-Placement Checklist				
	Enter Review Area			0
PS-1	Horizontal Waterproofing Inst.			0
PS-2	Vertical Waterproofing Inst.			0
PS-3	Survey CJ's/Pour Area Est.			0
PS-4	Horizontal pre-prufe tape @ CJ's			0
PS-5	Vertical pre-prufe tape @ CJ's			0
PS-6	Protection of piles/penetration sleeves installed			0
PS-7	Protection of vertical waterproofing installed			0
PS-8	Protection of Horizontal waterproofing installed			0
PS-9	Pit corners surveyed and vertical line established			0
PS-10	Screeds set to elevation -40.67			0
PS-11	Edge form installed			0
PS-12	Access path for concrete placing crew installed			0
PS-13	Slick-line hose clamp protection discs installed			0
PS-14	Area clear of debris			0
PS-15	Clean soiled membrane			0
PS-16	Obtain as-build survey of mud slab elevations			0
PS-17	Sealed protection slab with foam against spill and bleeding concrete thru joint			0
PS-18	Reference Best check off list for completed items			0
PS-19	Cast Concrete within 56 days from WP membrane installation			0
PS-20	Concrete mix design approved			0
PS-21	SGH inspected WP prior to pour			0
PS-22	Sharp objects are not used in consolidating concrete			0

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Protection slab Pre-placement Checklist Details

Item #	Item Text	Response	Comments	# Issues
PS-23	Formwork remains till concrete is 1500 psi			0
Best Waterproofing Installation Checklist				
Enter Review Area				
PS-24	CDSM Substrate sign off			0
PS-25	Protection Board - Fastened with Hilti pins at 12" on center			0
PS-26	Protection Board - 4" shingle at end laps and tightly buttred at side laps			0
PS-27	Drainage Composite - Adhesive applied to Protection Board and time allowed to flash off			0
PS-28	Drainage Composite - Extends 3" into gravel bed			0
PS-29	EPS insulation - Adhesive applied to Drainage Composite and time allowed to flash off			0
PS-30	EPS Insulation - No less then 1/4" gap in but joints			0
PS-31	ESP Substrate Sign off			0
PS-32	10 mil Visqueen - Fastened with temporaty Terminatin bar above line of concrete pour			0
PS-33	Grace Preprufe 300R - Fastened with temporaty terminatin bar above line of concrete pour			0
PS-34	Grace Preprufe 300R - 6" Bituthene 3000 back-seal at all laps			0
PS-35	Grace Preprufe 300R - Remove release sheet			0
PS-36	Grace Preprufe Tape - No Fishmouths			0
PS-37	Grace Preprufe Tape - Minimum 6" Liquid Membrane at all end laps before tape installation			0
PS-38	Grace Preprufe Tape - 8" CJ tape centered over all cold joints			0
PS-39	Final Inspection for damage prior to rebar installation			0
PS-40	Final Inspection for damage form installation and concrete pour			0
Pre-Pour Concrete Requirements:				
PS-41	Is release liner remaining on any surface	No		0
PS-42	Did placement crew fail to get training on protection of waterproofing prior placement	No		0
Waterstop - ADCOR ES (General)				
PS-43	Are there 90 degree, or more, bends missing ADCOR ES	No		0
PS-44	Are there damaged sections present	No		0
PS-45	Did ADCOR get wet prior to pouring concrete	No		0
PS-46	Is ADCOR ES encapsulated w/ less than 3" of concrete cover	No		0

Item #	Item Text	Response	Comments	# Issues
PS-47	Is ADCOR ES being stored in opened packaging	No		0
PS-48	Did the disposal of ADCOR ES fail to meet environmental requirements	No		0
PS-49	Is ADCOR ES being used in a movement joint	No		0
<i>Waterstop - ADCOR ES (Control Joints)</i>				
PS-50	Is the concrete surface dirty or have contaminants	No		0
PS-51	Is there debris or loose concrete at the control joint	No		0
PS-52	Are the irregular or unformed surfaces missing a bead of ADCOR ES adhesive	No		0
PS-53	Is the bead of ADCOR ES adhesive at irregular or unformed surfaces less than 1/2"	No		0
PS-54	Is the Adcor ES missing masonry nails	No		0
PS-55	Are the masonry nails less than 1-1/2" long	No		0
PS-56	Are the masonry nails less than 3/4" in diameter	No		0
PS-57	Are the washers at the masonry nails less than 3/4"	No		0
PS-58	Are 3/4 washers missing from the nails	No		0
PS-59	Are the fasteners spaced greater than 12"	No		0
<i>Waterstop - ADCOR ES (Pipe Penetrations)</i>				
PS-60	Was/Is the substrate wet at time of application	No		0
PS-61	Is the penetration missing a bead of ADCOR ES adhesive	No		0
PS-62	Is the bead of ADCOR ES adhesive less than 1/2"	No		0
PS-63	Is the bead un-tooled w/ brush / trowel	No		0
PS-64	Was the Adcor ES applied while adhesive was wet to touch	No		0
<i>Waterstop - ADCOR ES (ES Joints)</i>				
PS-65	Is the ADCOR ES joint missing an overlap	No		0
PS-66	Is the ADCOR ES joint overlap less than 4"	No		0
PS-67	Does overlap fail to achieve full contact between pieces	No		0

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## **9.0 ELEMENT 9 INSPECTION, MEASURING AND TEST EQUIPMENT**

- 9.1** INTRODUCTION
- 9.2** INSPECTION, MEASURING AND TEST EQUIPMENT (M&TE)
- 9.3** CONTROL OF MEASURING AND TEST EQUIPMENT
- 9.4** RESOLUTION OF TESTS RESULTS FROM UN-CALIBRATED EQUIPMENT
- 9.5** TEST REPORTING

## 9.0 INSPECTION, MEASURING AND TEST EQUIPEMENT

### 9.1 INTRODUCTION

Trade Subcontractor and supplier shall comply with this Element as required per contract documents.

### 9.2 INSPECTION, MEASURING AND TEST EQUIPMENT (M&TE)

- Inspection, measuring, and testing equipment required to carry out inspection and testing shall be identified, controlled, calibrated, and maintained in order to demonstrate the conformance of work to the specified requirements. Provisions shall be made for recalibration of such equipment in a timely manner and documented.
- Inspection, measuring, and test equipment used will meet the standards of accuracy for the measurements which are required. The equipment shall be calibrated according to national standards where available, and to documented standards where no national standards exist. The equipment will be recalibrated at regular intervals, and the recalibration properly documented. A record of the equipment calibration status shall be maintained by the Contractor.
- A schedule of testing equipment that needs periodic and regulatory scheduled calibration shall be required of the contractor(s) and be checked by TJPA QA Representative.
- The equipment shall be properly maintained to ensure its fitness for use. When the equipment is in use, the user shall ensure that the environmental conditions are suitable for the use of the equipment. When inspection, measuring, or test equipment is found to be out of calibration, the validity of previous inspection and test results shall be assessed and documented.
- All calibrated gauges and calibrated testing equipment must be calibrated prior to its use on the project. Periodic calibrations must be performed in accordance with certifying agency requirements and industry practice. The equipment will be properly maintained to ensure its fitness for use. When in use, the user shall ensure that the environmental conditions are suitable for the use of the equipment. When inspection, measuring, or test equipment is found to be out of calibration, the validity of previous inspection and test results shall be assessed and documented.

### 9.3 CONTROL OF MEASURING AND TEST EQUIPMENT

Inspection, measuring, and test equipment used shall be identified, controlled, calibrated. M&TE shall be properly calibrated and currently certified.

Calibration records and procedures shall meet the following requirements:

- Measuring and test equipment will be positively identified as to its name, calibration lab, date of last calibration and calibration expiration.
- Measuring and test equipment shall be calibrated against standards that have a known, valid relationship to national standards prior to use, and periodically thereafter, if required, to provide for the accurate reporting of quality testing and inspection results. In case no national standard exists, the basis for calibration will be identified and documented.
- The tolerances used in calibration shall be in accordance with the manufacturer's recommendation or as otherwise specified.
- An independent calibration laboratory shall perform all calibration.
- Environmental conditions for calibration shall be consistent with the location where inspection and testing is performed.
- Each subcontractor must maintain a spreadsheet for all calibrated instruments and their re-calibration dates with reminders on when the next calibration is required.
- Calibration shall be performed in accordance with approved calibration procedures. These procedures shall specify the following:
  - Details of equipment type
  - Identification number
  - Location (as required)
  - Calibration method and frequency
  - Acceptance criteria
  - Action to be taken if results are unsatisfactory

### 9.4 RESOLUTION OF TESTS RESULTS FROM UN-CALIBRATED EQUIPMENT

Results from tests requiring calibrated equipment performed with equipment not currently in calibration shall be suspect. The test equipment used shall be tested and recalibrated. If the equipment is found to be within calibration limits, the test

results shall be accepted. If the equipment is not found to be within calibration limits, the tests results must be verified by other means, or the material in question replaced.

## 9.5 TEST REPORTING

Inspection and test status are documented in BIM 360 and includes the Trade Subcontractors Daily Quality Control reports.



## **10.0** ELEMENT 10 **INSPECTION, TEST AND OPERATING STATUS**

### **10.1** OVERVIEW

### **10.2** PROCEDURE

## **10.0 INSPECTION, TEST AND OPERATING STATUS**

### **10.1 OVERVIEW**

Where required by the contract documents, Trade Subcontractors shall provide means for identifying the inspection and test status of work during production and installation. The purpose of this Element is to ensure that only work that has passed the required inspections and tests are accepted.

### **10.2 PROCEDURE**

The test and inspection status shall be identified by means of markings, stamps, tags, labels, routing cards, inspections records, test software, physical location, or other suitable means.

The status identification indicates the conformance or nonconformance with regard to inspections and tests performed.

The inspection of test status of planning and design documents shall be identified by suitable means that indicate the conformance on nonconformance of product with regard to checking and review performed.

While some operations may be easily tagged in the field, in the testing lab or shop as to their inspection status, most will be recorded in the construction management BIM 360 program through status reports.

## **11.0 ELEMENT 11 NONCONFORMANCES**

- 11.1** OVERVIEW
- 11.2** NON-CONFORMANCE OBSERVATIONS AND REPORTING
- 11.3** NON-CONFORMANCE REPORT (NCR)
- 11.4** FIELD CONDITION REPORT (FCR)
- 11.5** NON-CONFORMANCE AND FIELD CONDITION REPORTS LOG
- 11.6** CONTROL THE CONTINUATION OF WORK

## 11.0 NONCONFORMANCE

### 11.1 OVERVIEW

W/OJV and Trade Subcontractors are responsible to identify and document nonconformance issues with W/OJV expected to use BIM 360 to document QA/QC issues, FCR's and Nonconforming construction. Once a nonconformance is identified by an inspection, there is a systematic method to control the item, correct it, and ensure that project quality is not adversely impacted by the event.

### 11.2 NONCONFORMANCE QA ISSUES, OBSERVATIONS, REPORTING AND FIELD CONDITION REPORTS (FCR)

A Nonconformance is an item that does not meet the requirements of the project Contract Documents. Nonconforming work will be immediately segregated. Segregation may occur by physical isolation and cordoning off of work/materials, or conspicuously identified by tags/markings when physical isolation is not possible. When Nonconforming work is discovered it is determined by the QA/QC and engineer of Record to be a Nonconformance. The Webcor/Obayashi JV CQC Manager or Trade Subcontractor QC Manager will complete a Non-Conformance Report (NCR) and enter the non-conformance issue into BIM 360 for status reporting and resolution/closure tracking.

Procedures will be established and maintained to control nonconforming work, in order to ensure that such work is not inadvertently used or installed. Nonconforming work will be identified, documented, and evaluated to determine appropriate disposition. Where practicable, nonconforming items will be segregated. Those activities affected by the nonconforming work will be notified. The responsibility for review and authority for the disposition of nonconforming work will be defined in documented procedures. Disposition of nonconforming work can include reworking it to meet requirements, accepting it with or without repair, using it for alternative applications, or scrapping it. A determination to accept nonconforming work, as is or with repair, shall have the concurrence of the engineer of record. It may be advantageous to the owner to negotiate some form of compensation for accepting nonconforming work (e.g., additional spare parts).

The TJPA Representative will notify the Contractor of any detected noncompliance. Take immediate corrective action after receipt of such notice. If the Contractor fails or refuses to comply promptly, the TJPA Representative may issue an order stopping all or part of the work until satisfactory corrective action

has been taken. No part of the time lost due to such stop orders will be made the subject of claim for extension of time or for excess costs or damages by the Contractor.

Contractor shall be responsible for any Work that is non-conforming. Any required removal, repair, or replacement caused by non-conforming work shall be done by Contractor at no cost to the TJPA. Such non-conforming work will be considered as defective and payments will be withheld in accordance with Section 00 07 00, General Conditions, paragraphs 9.05 and 9.08.

Retesting required because of non-conformance to specified requirements shall be performed by the same independent firm on instructions by the TJPA representative. Contractor shall bear all costs for such retesting at no additional cost to the TJPA.

Procedures in BIM 360 will be used for tracking construction deficiencies from identification through acceptable corrective action and there the closure of the issue. Established verification procedures that identified deficiencies have been corrected.

Follow-up Phase: CQC System Manager and Trade Subcontractor QC Managers shall perform daily checks to assure control activities, including control testing, are providing continued compliance with contract requirements, until completion of the particular feature of work. Record the checks in the CQC documentation. Conduct final follow-up checks and correct all deficiencies prior to the start of additional features of work that may be affected by the deficient work. Do not build upon or conceal non-conforming work.

### **11.3 NON-CONFORMANCE REPORT (NCR)**

When completing the Nonconformance Report, the W/OJV CQC Manager or Trade Subcontractor QC Manager shall describe the work in detail, its location, a description of the deficiency and the proposed resolution and actions taken to prevent the recurrence of the non-conformance on BIM 360. Supporting documentation shall be attached to clearly describe the issue. The report will be uploaded into BIM 360. Nonconformance Report contents are summarized as follows:

Section 1: Nonconformance identification info: Contractor, location date, etc.

Section 2: Description of Non-conformance

Section 3: Cause

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Section 4: Recommended Field Engineer Disposition (Trade Subcontractor CQC Manager)

Section 5: Project Engineering Disposition (TJPA)

Section 6: Disposition Results

Section 7: Corrective action and steps taken to prevent recurrence

Process steps when responding to the receipt of an NCR's

Step 1: QC Manager/QC Specialist notifies subcontractor, in writing (email), of NCR:

Step 1a: sub to provide in response:

- Is the NCR accurate?
- No, then what is the actual field condition (w/ supporting documentation)?
- Yes, then
  - What appears to be the root cause?
  - What remedial steps can the sub perform without the engineer's approval?

Step 1b: Project Manager/QC Manager to:

- Determine if a formal RFI or CAP (corrective action plan) needs to be submitted for prior approval?
- Trade subcontractor generates the RFI to seeking direction for remedial action.

Step 2: Webcor superintendent / QC Field Specialist – review condition in comparison w/NCR

- A. Determine if the NCR is accurate,
- B. Determine if there are any field indications for cause of the NCR,
- C. Review sub's field QC procedures and documentation of DFW task checklist associated with the subject NCR.

Step 3: Webcor pm, qc Manager, & superintendent meet w/ sub's pm, qc Manager, & foreman to review DFW preparatory meeting and initial install notes to determine:

- A. What step was missed to allow for the NCR?
- B. What lesson's learned need to be applied to avoid future NCR?
- C. Determine if changes need to be made to the frequency and type of qc reviews are done for the subject scope.

Step 4: submit the cap for the NCR based on information gathered from steps 1 - 3

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Step 5: to avoid future NCR of the same type:

- A. Schedule an initial phase review of DFOV CHECKLIST. Each DFOV process shall identify WHAT REVIEW is done by who and when. The frequency and type of reviews for the initial installation should be more intense than the follow up phases. An NCR shall reset the clock and increase the review documentation and confirm the corrective actions have been taken.
- B. Implement additional actions as determined by the cap review process.

Step 6: trade subcontractor completes the required tasks and generates an inspection request.

Step 7: populate all the pertinent blanks on the NCR form and obtain signatures for compliance...

#### 11.4 FIELD CONDITION REPORTS (FCR)

Field Condition Report (FCR) are conditions that deviate from the approved submittals, installed incorrectly or damaged work, but may be resolved without damage to permanent installation. When completing the Field Condition Report, the Trade Subcontractor CQC Manager will describe the work in detail, its location, Specification, a description of the deficiency, and the proposed resolution and actions taken to prevent the recurrence. The Subcontractor can also provide the disposition, and proposes to close the FCR. W/O JV CQC Manager will review proposed resolution on BIM 360 and either request for TJPA to close it or request for additional information from Sub QC Manager till the issue is resolved in a timely Manner.

Process Steps for writing and closing an FCR issue and the process for completing a NCR

Step 1: A FCR is identified and written by:

- a) Observation - Webcor CQC Manager, superintendent/QC Field Specialist or TJPA representative monitoring the work observes a quality issue and create a QC/QA issue in BIM 360.

- b) Task checklist - Webcor superintendent/QC Field Specialist is completing a DFOW checklist and observes an issue and creates FCR issue in BIM 360
- c) Inspection request (Tasks) – When an inspector rejects an inspection request, a FCR is generated in BIM 360 and linked to the Inspection Request.

Step 1a: When FCR escalates to an NCR:

- a. FCR's point to a systemic issue
- b. Ignored FCR's (30, 60, 90 days)
- c. Latent Issue
- d. Corrective Action Plan (CAP) or RFI is required

Step 2: A QC/QA and FCR issue is closed by:

- a) Stating the cause of the issue and proposes a corrective action plan (CAP) and submits the CAP in BIM 360.
- b) Documents the corrective action taken in BIM 360.
- c) Documents the cause and actions taken to prevent recurrence in BIM 360.

Step 2a: A NCR is closed by:

- a. Submit the Corrective Action Plan (CAP) for the NCR,

Step 3: To avoid future NCR of the same type:

- a) Schedule an Initial Phase Review of DFOW checklist. Each DFOW process shall identify what review is done by who and when. The frequency and type of reviews for the initial installation should be more intense than the follow up phases. An NCR shall reset the clock and increase the scrutiny to review documentation and confirm the corrective actions have been taken.
- b) Implement additional action as determined by the CAP review process.

Step 4: Trade Subcontractor completes the required tasks and generates an Inspection Request.

Step 5: QC Manager populate all the pertinent blanks on the NCR Form and obtain signatures for compliance.



### 11.5 NONCONFORMANCE AND FIELD CONDITION REPORT LOG

The project-wide Non-Conformance Tracking Log in Autodesk BIM 360 is maintained by the TJPA Construction Management Oversight. Webcor/Obayashi JV and the Trade Subcontractors will maintain Non-Conformance logs appropriate for their scope of work.

### 11.6 CONTROL THE CONTINUATION OF WORK

After the item of work is identified and segregated from all other active work, the W/O JV CQC Manager or Trade Subcontractor QC Manager will determine if work can continue in the affected area. When continuing work can adversely affect quality or hide the defect, work must stop in the affected area until the disposition of the item is resolved. The W/OJV CQC Manager identifies and clearly labels the limits of the affected stop work areas. Non-conforming work may be reworked to meet requirements, accepted as is, repaired, or rejected. If accepted as is or repaired, the Engineer of Record needs to approve the deviation from original specifications. Nonconforming work may require an approved Corrective Action Plan.

## **12.0** ELEMENT 12 **CORRECTIVE ACTION**

### **12.1** INTRODUCTION

### **12.1** CORRECTIVE ACTION AND CORRECTIVE ACTION PLANS

## 12.0 CORRECTIVE ACTION PLAN

### 12.1 INTRODUCTION

The following CAP procedure shall cover all construction operations, both onsite and offsite, including work by Trade Subcontractors and Suppliers. Procedures for tracking construction deficiencies from identification through acceptable corrective action. Establish verification procedures that identified deficiencies have been corrected.”

### 12.2 CORRECTIVE ACTION AND CORRECTIVE ACTION PLANS (CAP)

Corrective action procedures should be established, documented, and maintained. These include procedures for investigation of the cause of nonconforming work and the corrective action needed to prevent recurrence, and procedures for analysis to detect and eliminate potential causes of nonconforming work. This element also includes implementing and recording changes in procedures resulting from corrective action.

Once a NCR cause has been determined, a written Corrective Action Plan (CAP) will be submitted by W/OJV in order to resolve and close the NCR. The CAP will be written by the Trade Subcontractor QC Manager and submitted to W/OJV’s CQC Manager who will review and post to Constructware after sign-off. W/OJV QC Manager or Trade Subcontractor QC Manager will attach the submitted CAP to the NCR in BIM 360 Systems for tracking. Once CAP is approved, the CAP will be implemented by the Trade Subcontractor.

Corrective action procedures shall be established for:

- Investigating the cause of the nonconforming work and taking the corrective actions needed to prevent recurrence
- Analyzing the CAP processes to detect and eliminate potential causes of nonconforming products.
- Initiating preventative actions to deal with problems to a level corresponding to the risks encountered
- Ensuring that corrective actions are taken and that they are effective

- Implementing and recording changes in procedures resulting from corrective action

## **13.0** ELEMENT13 **QUALITY RECORDS**

### **13.1** INTRODUCTION

### **13.2** DOCUMENTATION

### **13.3** REPORTING

DAILY REPORTS

MONTHLY REPORTS

PERIODIC FORMS, REPORTS AND LISTS

### **13.4** DFW QC REPORTING FOLDER FILES STRUCTURE FOR CONSTRUCTWARE

W/OJV DAILY CQC REPORT FORM

NONCONFORMANCE REPORT FORM

## 13.0 QUALITY RECORDS

### 13.1 Introduction

Procedures are established and will be maintained for quality records. These procedures will identify which records shall be kept, responsibility for production and collection, and responsibility for indexing, filing, storage, maintenance, and disposition of quality records..

Quality records shall be maintained to show achievement of quality objectives and appropriate functioning of the Quality Management System. Supplier, contractor, and subcontractor quality records shall be included when pertinent, as defined by requirements agreed upon during DFOV Preparatory Meeting, based Specifications and Codes. Quality records shall be legible and specify the work involved. They shall be kept in an environment to minimize deterioration and damage. Retention times and final disposition shall be established and recorded.

The following types of Quality records requiring control:

- Inspection reports – (Code required inspection reports are uploaded by TIPA's QA team to BIM 360 and Constructware.) Trade subcontractors Reports are attached to Daily QC reports.
- Test Data – Code test uploaded by TIPA to BIM 360. Non-code tests are required per specs are included as part of Daily QC reports.
- Qualification records (BIM 360)
- Calibration Records (BIM360)
- Nonconformance (BIM 360)
- Corrective Actions (BIM 360)
- Daily QC reports with back up data and Documentation
- Material identification / batch tickets

### 13.2 Documentation

Each Subcontractor is required to produce a QC Daily Report within 3-4 days must include all sub tier documentation (Delivery tags, material traceability and heat number tags). W/O JV shall generate CQC Daily Reports that indicates interaction with Subcontractor's process in establishing Quality installation, inspection,

and documentation. DFOV checklists are used to identify items that require special attention and document any daily occurrences in QC Daily Reports

Maintain current and complete QC reports providing evidence that required quality control activities and tests have been performed. Include in these records the work of Trade Subcontractors and Suppliers on an acceptable form.

Address deficient features and include a statement that equipment and materials incorporated in the Work and workmanship comply with the Contract. Furnish these reports to the TIPA Representative daily within 5 working days after the date covered by the report. Reports must be signed and dated by the CQC System Manager. Include copies of reports prepared by all subordinate quality control personnel within the CQC System Manager's report

The W/OJV CQC will review for completeness, clarity and accuracy of W/O CQC staff or Trade Subcontractor reports.

Weekly meeting with key Trade subcontractors QC Manager will go over key QC issues to ensure timely QC reports are submitted on regular bases.

### 13.3 REPORTING

#### Daily Reports

- Webcor/Obayashi JV Daily CQC reports (see attached)
- Trade Subcontractors Daily CQC reports

#### Monthly Reports

- Webcor/Obayashi JV Construction Monthly Report
- Webcor/Obayashi JV CQC Managers Monthly Status Report (included in the Construction Monthly Report)

#### Periodic forms, reports and lists

- Definable Features of Work (DFOV) list per Trade Subcontractor (in W/OJV F: drive, Constructware and hard copies in section: Tab/Element 7).
- Non-Conformance Report (see attached)

#### 13.4 DFOW QC REPORTING FOLDER FILES STRUCTURE FOR CONSTRUCTWARE

The CQC File Structure is outlined below and will be utilized on this project to store, organize and manage W/OJV Daily CQC Reports and DFOWs. In Constructware

DFOW folder and file structure:

Each trade package has a folder and each DFOW has a subfolder with subsequent subfolders. The folders and files are managed by CM/GC Quality Control Manager and CM/GC Document Control. Files are located in File Management/File Director by Project. This arrangement puts all the records for each DFOW in one folder. It becomes the quality record for that DFOW.

- 10 Quality
  - 13 Definable Feature of Work (DFOW)
    - BSE- TG03- BBI
    - DFOW log
    - DFOW (By Number and Title)
      - Preparatory Phase  
Preparatory Phase documents are filed in this folder.
      - Initial Phase  
Initial Phase documents are filed in this folder.
      - Follow-up Phase  
Follow-up documentation is appended to Daily QC Reports and filed in this folder by number and date.
      - DFOW Record Documents  
As the work is completed but no later than after completion of the DFOW all quality records would be assembled and filed in this folder. In the event of an audit or record search this folder would contain all the records. Subfolders may be added as needed.
        - Material Records
        - Installation Records

CQC Daily Reports folder and file structure:

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Daily CQC Reports are prepared and filed in folders by date. Each folder contains the CM/GC QC Manager's Daily Report and all the Trade Contractors' QC Managers' Daily Reports. The folders and files are managed by CM/GC Quality Control Manager and CM/GC Document Control. Files are located in File Management/File Director by Project. This arrangement puts all the Daily QC reports for each day in one folder. It becomes the quality record for that day.

- 10 Quality
- 12 CQC Reports
- Year
  - Month
    - Day (By Contractor- year/month/day (i.e. BBI-13/08/29 OR 20130829)
    - CM/GC QC Daily Report  
This report is prepared by the CM/GC QC Manager
    - TCQM Daily Report (Identified by Trade Package)  
This report is prepared by each Trade Contractor QC Manager and submitted to the CM/GC Quality Control Manager for review and filing.



<b>CONTRACTOR QUALITY CONTROL REPORT</b>				DATE	
(ATTACH ADDITIONAL SHEETS IF NECESSARY)					
PHASE	TRANSBAY TRANSIT CENTER BUILDING		PROJECT NUMBER: 3100		
<b>PREPARATORY</b>	WAS A PREPARATORY MEETING HELD TODAY? YES <input type="checkbox"/> NO <input type="checkbox"/> IF YES, FILL OUT AND ATTACH SUPPLEMENTAL PREPARATORY PHASE CHECKLIST.				
	Schedule Activity No.	Definable Feature of Work			
<b>INITIAL</b>	WAS AN INITIAL PHASE MEETING HELD TODAY? YES <input type="checkbox"/> NO <input type="checkbox"/> IF YES, FILL OUT AND ATTACH SUPPLEMENTAL INITIAL PHASE CHECKLIST.				
	Schedule Activity No.				
<b>FOLLOW-UP</b>	WORK COMPLIES WITH CONTRACT AS APPROVED DURING INITIAL PHASE? YES <input type="checkbox"/> NO <input type="checkbox"/>				
	Schedule Activity No.	Description of Work, Testing Performed & By Whom, Definable Feature of Work, Specification Section, Location and List of Personnel Present,			
REWORK ITEMS IDENTIFIED TODAY (NOT CORRECTED BY CLOSE OF BUSINESS, ASSIGN REWORK ITEM TRACKING NUMBER)		REWORK ITEMS CORRECTED IN PROGRESS TODAY (FROM REWORK ITEMS LIST, IF COMPLETE RECORD CORRECTION ON TRACKING LOG)			
Issue No.	Description	Issue No.	Description		
REMARKS (Also Explain Any Follow-Up Phase Checklist Item From Above That Was Answered "NO"), Manuf. Rep On-Site, etc.					
Schedule Activity No.	Description				
On behalf of Webcor/Obayashi, I certify that this report is complete and correct and equipment and material used and work performed during this reporting period is in compliance with the contract drawings and specifications to the best of my knowledge except as noted in this report. <div style="float: right; margin-top: 10px;">             _____              WEBCOR QC REPRESENTATIVE                      DATE           </div>					
WEBCOR/OBAYASHI QUALITY CONTROL MANAGERS REMARKS AND/OR EXCEPTIONS TO THE REPORT					
Schedule Activity No.	Description				
_____ WEBCOR/OBAYASHI JV CQC MANAGER                      DATE					



CONTRACTOR QUALITY CONTROL REPORT		DATE
(CONTINUATION SHEET) (ATTACH ADDITIONAL SHEETS IF NECESSARY)		
PHASE	TRANSBAY TRANSIT CENTER BUILDING	PROJECT NUMBER: 3100
FOLLOW-UP	WORK COMPLIES WITH CONTRACT AS APPROVED DURING INITIAL PHASE? YES <input type="checkbox"/> NO <input type="checkbox"/>	
	Schedule Activity No.	Description of Work, Testing Performed & By Whom, Definable Feature of Work, Specification Section, Location and List of Personnel Present
	REMARKS (Also Explain Any Checklist Item From Above That Was Answered "NO"), Manuf. Rep. On-Site, etc.	
	Schedule Activity No.	Description

W/O # \_\_\_\_\_

Assigned by CMO QA Manager NCR # \_\_\_\_\_

Contract # \_\_\_\_\_ Contractor/Sub(s) \_\_\_\_\_

Code/Spec/Dwg \_\_\_\_\_ Location \_\_\_\_\_

Reference #s \_\_\_\_\_

Part/Lot \_\_\_\_\_ Quantity \_\_\_\_\_ Supplier \_\_\_\_\_ P.O. \_\_\_\_\_

Initiated by/Co \_\_\_\_\_ Date Issued \_\_\_\_\_

Description of Non-Conformance

Code \_\_\_\_\_  
See QMS QA-08-3, over

Cause

Code \_\_\_\_\_  
See QMS QA-08-3, over

Recommended Disposition

Contractor Field Engineering

☐ **Reject** Remove, replace, meet spec☐ **Rework** Fix to meet specifications

—Requires FE Disposition/CQC Acceptance—

☐ **Accept-As-Is** Not to spec☐ **Repair\*** Fix, but not to spec

—Requires EOR Approval/PM OK—

Resolve as Follows

☐ Proposed resolution, repair or rework plan attached (\*required)Field Engineer *Print Name, Org; Initial* \_\_\_\_\_ Date \_\_\_\_\_

Engineer of Record Disposition

Resolve as Follows

☐ **Accept-As-Is** Not to spec☐ **Repair** Fix, but not to specEngineer of Record *Print Name, Org; Initial* \_\_\_\_\_ Date \_\_\_\_\_PM Concurrence *Print Name, Org; Initial* \_\_\_\_\_ Date \_\_\_\_\_

Quality Review
TJPA QA _____
CQC _____

Disposition Results

Contractor QC Acceptance *Print Name, Org; Initial* \_\_\_\_\_ Date \_\_\_\_\_PM Verification *Print Name, Org; Initial* \_\_\_\_\_ Date \_\_\_\_\_

Corrective and Preventive Action (CAPA)

*If required*CAPA Verification *Print Name, Org; Initial* \_\_\_\_\_ Date \_\_\_\_\_

## ASSEMBLY

- 001 Interference/Improper Fit
- 002 Dis-bonding/Adhesive Defect
- 003 Incorrect Part Used
- 004 Assembly Error
- 005 Soldering Failure
- 006
- 007
- 008
- 009
- 010 Other Assembly Related Defect

## CERTIFICATION / DOCUMENTATION

- 011 Information Missing
- 012 Information Incorrect
- 013 Information Illegible
- 014 Material Incorrect
- 015 Inspection/Test Incorrect
- 016 Data Out-Of-Spec.
- 017
- 018
- 019
- 020 Other Cert./Documentation Error

## DIMENSIONAL

- 021 Thickness—Over/Under Size
- 022 Diameter – Over/Under Size
- 023 Length/Width—Over/Under Size
- 024 Depth Incorrect
- 025 Slope Incorrect
- 026 Angle Incorrect
- 027 Feature/Item Missing
- 028 Position/Location Incorrect
- 029 Radius Over/Under Size or Missing
- 030 Other Dimensional Defect

## INSTALLATION

- 031 Missing Hardware
- 032 Missing Equipment
- 033 Non-Standard Installation
- 034 Incomplete Installation
- 035 Non-Conforming Materials Used
- 036 Equipment Damaged
- 037 Incorrect Location
- 038 Incorrect Orientation
- 039
- 040 Other Installation Defect

## INSTALLATION / TEST FAILURE

- 041 Inspection/Test Equipment Failure
- 042 Equipment Not Calibrated
- 043 Procedural
- 044 Under-Test Condition
- 045 Electrical Test Failure
- 046 Leak Test Failure
- 047 Environmental Test Failure
- 048 Functional Test Failure
- 049 Mechanical Test Failure
- 050 Other Inspection/Test Failure**

## MATERIAL / SOILS

- 051 Incorrect Material Used
- 052 Material Contaminated
- 053 Gradation Test Failure
- 054 Moisture Test Failure
- 055 Density (Compaction) Test
- 056 Sand Equivalent Test Failure
- 057 Organic Content of Soils
- 058 Durability Index
- 059 Resistance (R-value)
- 060 Other Material Defect

## MATERIALS / CONCRETE & STEEL

- 061 Incorrect Materials Used
- 062 Concrete Slump Test Failure
- 063 Concrete Air Content
- 064 Concrete Compressive Strength Test Failure
- 065 Drying Shrinkage of Concrete
- 066 Concrete Honeycombing
- 067 Concrete Rock-Pocket/Voids
- 068 Mis-fabricated Reinforcing Steel Assemblies
- 069 Missing or Incorrect Reinforcing Steel
- 070 Other Material Defects

## NON-DESTRUCTIVE EXAMINATION (NDE)

- 071 Cracked Welds
- 072 Foreign Material
- 073 Component Gap/Fit-up Defect
- 074 Undercut
- 075 Porosity/Slag
- 076 Lack of Penetration/Fusion
- 077 Discontinuities
- 078 Voids
- 079 Delamination
- 080 Other NDE Indications

## SURFACE DEFECTS

- 081 Discoloration
- 082 Blisters
- 083 Sparing
- 084 Burrs/Chips/Nicks
- 085 Damaged/Bent/Torn/Twisted
- 086 Contaminated
- 087 Foreign Material
- 088 Plating/Coating Defects
- 089 Cracks
- 090 Surface Irregular/Finish

## VISUAL & OTHER DEFICIENCIES

- 091
- 092
- 093
- 094
- 095
- 096
- 097
- 098
- 099
- 100 Other Visual Anomaly**

## 14.0 ELEMENT14. QUALITY AUDITS

## **14.0** QUALITY AUDITS

### **14.1** QUALITY AUDITS

The Trade Subcontractor QC Manager reports to the Webcor /Obayashi JV CQC Manager and oversees the trade specific implementation of the quality control program and whose primary responsibility will be to implement the Trade Subcontractor's quality control plan. The Trade Subcontractor QC manager will certify that the Trade Subcontractor's work is in compliance with the Contract Documents and complies with the Webcor/Obayashi Joint Venture Quality Control Plan and all quality control requirements contained in the Contract Documents, including specification section 01 14 00 Quality Control. The Trade Subcontractor QC Manager shall:

- Support and facilitate QMS Audit process by TJPA, FTA, and Agency Audits.

## **15.0** ELEMENT 15 **TRAINING**

### **15.1** TRAINING

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## 15.0 TRAINING

### 15.1 TRAINING

Webcor/Obayashi JV will ensure that only knowledgeable capable employees carry out the planning and execution of the work.

- **The W/OJV CQC Manager will provide and document training.**  
Under the Direction of the W/OJV CQC manager the Trade Subcontractor QC Managers will provide training on the elements of the W/O JV and Trade Subcontractors site specific Contractor's Quality Control Plans to all trade subcontractor staff having CQC responsibilities.
- When specified in the Contract Documents, Trade Subcontractor CQC Managers will submit proof of tradespersons qualifications including licensing requirements, certifications or other required training qualifications for the specified task to Webcor /Obayashi JV and the TJPA.
- When specified in the Contract Documents, project or task specific training will be documented by the Trade Subcontractor. The Trade Subcontractor will provide Webcor/Obayashi JV with a copy of the training syllabus and list of attendees.
- Webcor/Obayashi JV Quality Control personnel will complete the U.S. Army Corps of Engineers/U.S. Navy Facilities Engineering Command, Construction Quality Management for Contractors
- The Trade Subcontractor QC Managers will maintain records of quality training for their personnel. The Webcor/Obayashi JV CQC Manager will maintain records of quality training for Webcor/Obayashi JV personnel.
- W/OJV continues to revise Superintendents and QC field staff procedures to improve on records and reports for field issues such as Material, installation, FCR's, and NCR's.
- As part of each DFOV's meeting process a DFOV checklist will be established and will determine the requirements for each DFOV checklists.
- W/OJV shall conduct training for Superintendent and QC staff to clarify DFOV requirements as well as what issues should be tracked and raised to the status of Field Condition Reports.
- W/OJV will conduct work sessions with TJPA QC representative and W/O Superintendents to clarify, when and who shall issue FCR's and/or NCR's.

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- Training of personal on the proper procedures to complete a DQC report.

# Construction Stormwater Pollution Control/Compliance Plan

## Transbay Transit Center Project San Francisco, California

---



Prepared for:  
Webcor /Obayashi

Prepared by:



6h 1 W 24 PM 1 49

WASTEWATER ENTERPRISE  
COLLECTION SYSTEM DIVISION

Approved as "NOT" *Qin*

February 2011

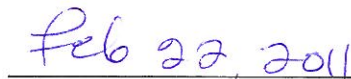
**Transbay Transit Center  
San Francisco, California**

## **Construction Stormwater Pollution Control/Compliance Plan**

Submitted to:  
Webcor /Obayashi

This report has been prepared by or under the supervision of the following Qualified Storm Water Pollution Prevention Developer and Construction General Permit Trainer of Record.

  
Debra Carey, QSD, ToR, CEG

  
Date

February 2011

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Appendix A	Inlet Locations
Appendix B	Construction Stormwater Controls Monitoring Checklist
Appendix C	SFPUC Construction Pollution Prevention Guide

# 1 REGULATORY SETTING

The Transbay Transit Center Project (Project) meets federal Clean Water Act (CWA) and State Porter – Cologne Water Quality Control requirements via connection to the combined wastewater and stormwater sewer system operated by the San Francisco Public Utilities Commission (SFPUC) under a State Water Resources Control Board-issued National Pollutant Discharge Elimination System (NPDES) permit (Order No. R2-2002-0073, NPDES Permit No CA0037664). The Project is therefore not subject to coverage under the California Construction General Stormwater Permit (Order 2009-0009-DWG), that became effective on July 1, 2010; however, the construction site must implement Best Management Practices (BMPs) to prevent pollutant discharge into the combined sewer to comply with the San Francisco municipal ordinances and codes described below. This Construction Stormwater Pollution Control/Compliance Plan provides a delegation strategy along with best management practice (BMP) categories for compliance with stormwater regulations covering construction activities at the Project.

## **San Francisco Ordinance**

San Francisco has a Stormwater Discharge Controls Ordinance requiring Pollution Prevention Procedures during any construction conducted in the City of San Francisco. In general the ordinance discusses long term BMPs such as rain gardens and green roofs particularly applicable to redevelopment areas and sections of the City serviced by small municipal separate storm sewer systems (MS4); however aspects of the ordinance apply to construction activities. For example, although coverage under the NPDES General Construction Permit (Water Board Order No. 99-08-DWQ) is not required for projects in those areas of the city that drain to the combined sewer system; all construction sites must implement BMPs to prevent illicit discharge into the combined sewer. Generally, City requirements include the development of a Storm Water Pollution Prevention Plan (SWPPP), SWPPP plan review by SFPUC, stormwater treatment measures, runoff monitoring, and frequent site inspections. The regulations also require the use of construction period (and operational period) BMPs on construction sites to keep pollutants (sediment and construction site debris), out of water conveyance systems, the treatment plants, and discharge points.

## **San Francisco Public Works Code**

The federal CWA requires that publicly-owned treatment works (POTW) regulate the discharge of industrial wastes into a sewer system subject to NPDES permit requirements, and since construction activity is regulated under the industrial category, San Francisco's department of public works (DPW) has adopted requirements for construction discharges to the combined sewer system. Under DPW regulations, discharges of construction storm water as well as any wastewater (such as dewatering from construction sites) is subject to the requirements of Article 4.1 of the San Francisco Public Works Code, which regulates the quantity and quality of discharges to the combined sewer system. Projects that conduct any dewatering activity are required to apply for a Wastewater Batch Discharge Permit from the SF PUC WWC\_CSD. Information on the Batch Discharge Permit and pre-treatment can be found online at: [http://sfwater.org/msc\\_main.cfm/MC\\_ID/14/MSC\\_ID/445](http://sfwater.org/msc_main.cfm/MC_ID/14/MSC_ID/445).

Order No. 158170 of the San Francisco DPW provides additional pre-treatment industrial waste discharge limits to augment those listed in Article 4.1. The San Francisco Municipal Code requires contractors to have a Sediment and Erosion Control Plan for projects that discharge to the Combined Sewer System.

## **RESPONSIBLE PARTIES**

The legally Responsible Party for the Project is the Transbay Joint Powers Authority (TJPA). The TJPA consists of a collaboration of Bay Area government and transportation agencies, and is managed by TJPA staff and overseen by a Board of Directors. For site-specific concerns that can be addressed by TJPA, please call **415.409.TJPA (8572)**.

Webcor /Obayashi is a joint venture contracting group hired by TJPA as general contractor for the Transbay Terminal Center Phase of the Project. Webcor /Obayashi will be subcontracting construction to Trade Subcontractors who will be responsible for preparing SWPPPs specific to their construction activity, schedule, discharge points, types of pollutants and construction boundaries. The Trade Subcontractors will be responsible for preparing and submitting for approval a SWPPP including furnishing, installing, maintaining and removing BMPs such as silt fence, filter boxes, construction entrances, sediment traps, dust control, dewatering and other erosion and sediment control measures during construction to prevent contamination of storm water from construction activities and to maintain compliance with the SF storm water ordinance and codes. For site-specific NPDES concerns that can be addressed by Webcor/Obayashi, please call **415.978.5726**.

## **2 PROJECT INFORMATION**

### **2.2 Project Description**

The Project is located generally between Second Street in the west, Beale Street in the east, Natoma Street in the south and Minna Street in the north (Figure 1). The Project is part of a larger \$4 billion transportation and housing expansion/redevelopment effort that will replace an old Transbay Terminal at First and Mission streets with a modern regional transit hub connecting eight Bay Area counties and the State of California through 11 transit systems: AC Transit, BART, Caltrain, Golden Gate Transit, Greyhound, Muni, SamTrans, WestCAT Lynx, Amtrak, Paratransit and future High Speed Rail from San Francisco to Los Angeles/Anaheim.

The entire Project consists of three broad activities as noted below. Webcor /Obayashi are the general contractors and have prepared this Construction Stormwater Pollution Control/Compliance Plan to provide for compliance with stormwater regulations covering construction activities.

- **Utility Relocation**
- **Train Box and Transit Center Building Construction**
- **Bus Ramp Construction**

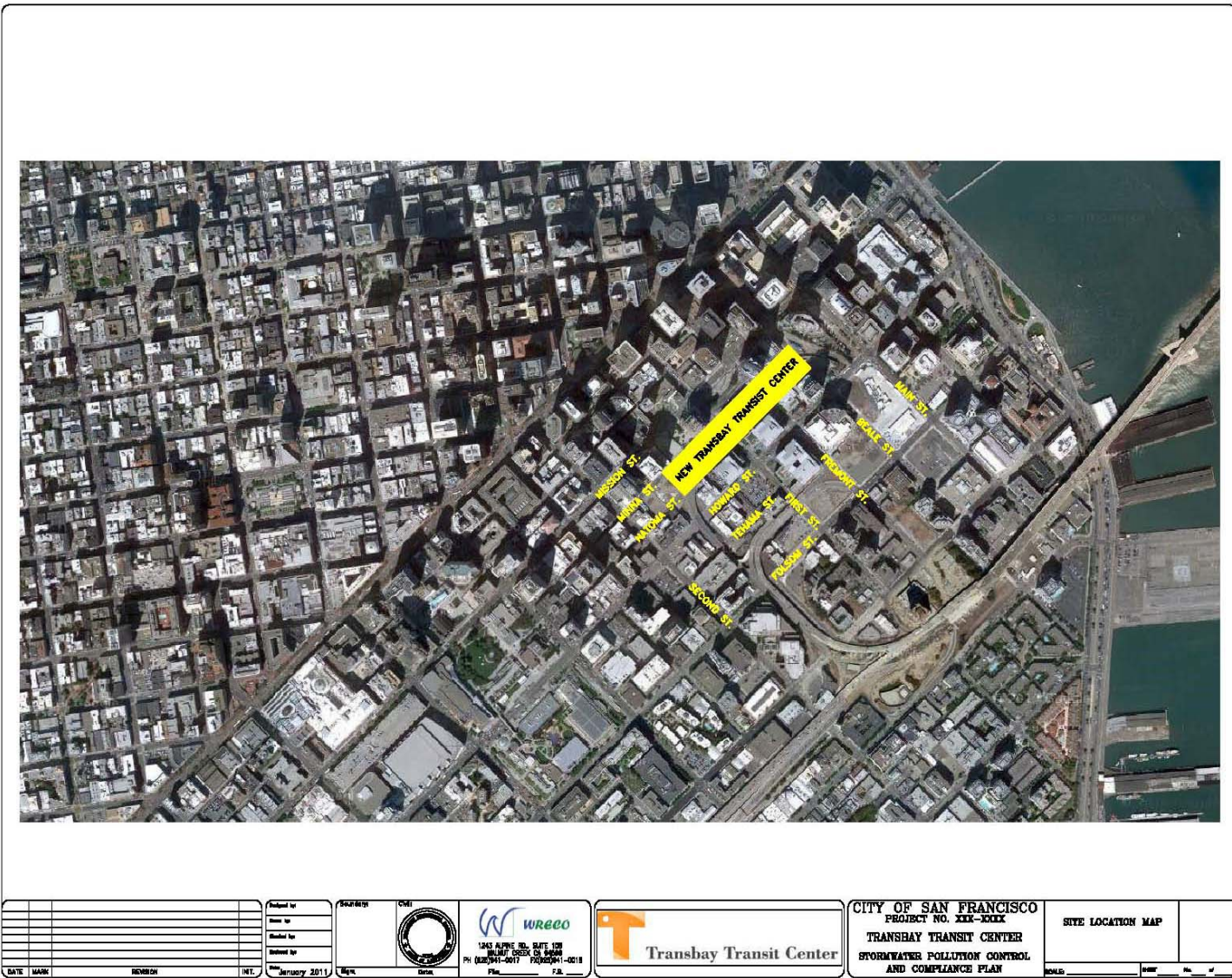


Figure 1. Project Location Map



## 2.3 Project Size and Total Disturbed Area

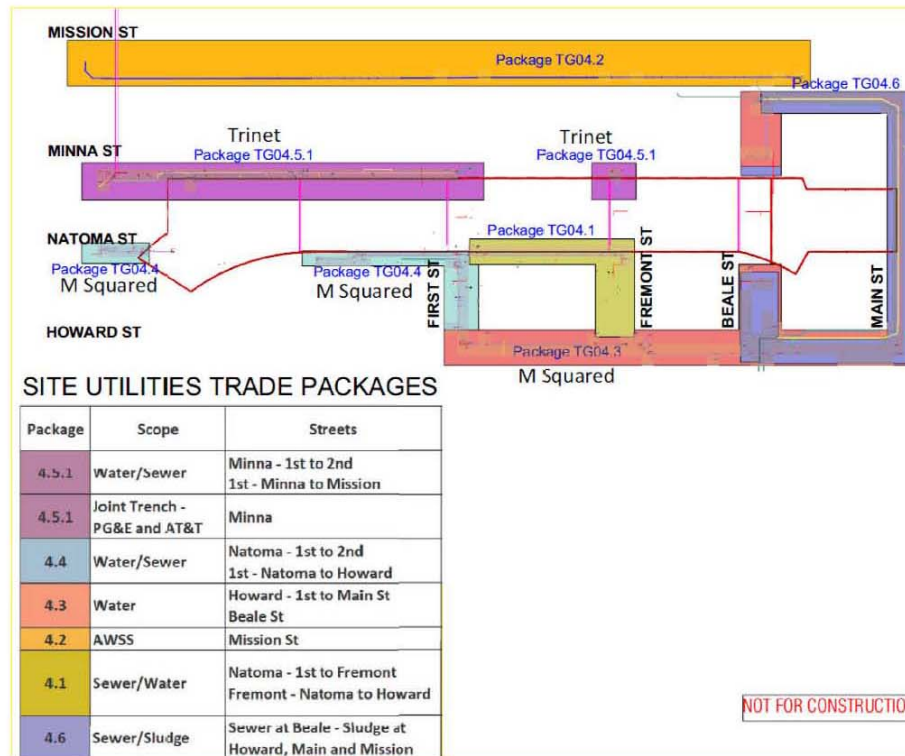
The estimated total disturbed soil area (DSA) for the Project is approximately 12.3 acres and includes the areas where the soil might be potentially disturbed by construction activities, as follows:

**Table 1. Total Land Disturbance**

<b>Area Name</b>	<b>Approximate Area Disturbed (Acres)</b>
Zone 1	2
Zone 2	1.8
Zone 3	1.5
Zone 4	4
Linear Utility Relocation	2.5
Additional Staging/Disturbance	3
<b>Total</b>	<b>12.3</b>

Figures 2 and 3 show general locations for the DSA construction zones and linear utility relocation trade packages. Several staging areas are anticipated during the life of the Project as shown in Figure 4.





DATE	MARK	REVISION	INIT.	Design by Drawn by Checked by Reviewed by Date: January 2011	Scale: 1543 ALPINE RD., SUITE 108 BERKELEY, CA 94706 PH (925) 941-5017 FAX (925) 941-4018	Transbay Transit Center	CITY OF SAN FRANCISCO PROJECT NO. XXX-XXXX TRANSBAY TRANSIT CENTER STORMWATER POLLUTION CONTROL AND COMPLIANCE PLAN	UTILITIES DSA MAP SCALE: _____ SHEET No. 4
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Figure 3. Utilities DSA Map



## **2.4 Receiving Waters and Environmentally Sensitive Areas**

The Project is located within the northeastern section of the City of San Francisco. The Project does not discharge directly to jurisdictional “receiving water.” The San Francisco combined sanitary/storm water sewer system collects all storm and waste water discharging in the Project vicinity and pipes the water to the Southeast Water Pollution Control Plant for processing and discharging under NPDES Permit No CA0037664. The SE plant currently treats runoff to secondary treatment standards established by the USEPA, meeting or exceeding water quality objectives in San Francisco Bay.

The San Francisco Bay Area has a climate characterized by wet winters and dry summers. Average annual rainfall in the area is approximately 20 inches. The majority of this rainfall generally occurs from November through April with little rainfall during the remaining months of the year. Construction for the Project will span a period of several years including both wet and dry seasons. The project does not impact any known environmentally or culturally sensitive areas. For information regarding any environmentally sensitive habitat concerns, please refer to the Biological Resource Assessment. For information on cultural or other CEQA or NEPA requirements, please refer to the appropriate State or Federal Agency.

## **2.5 Construction Activities and Schedule**

The Project activities include but are not limited to clearing, excavation and backfill, construction and finishing work within a busy city environment with established infrastructure. Several staging areas are anticipated during the life of the Project. Construction equipment and materials will be stored both onsite and at staging areas. As a result, fueling and maintenance, as well as welding and fabrication, may take place onsite. A discussion of the pollutants with potential to contact storm water as a result of these activities is included below. Since demolition of the existing ramps and terminal is currently underway by another contractor (Evans Bros Inc), the first phase of the Webcor-Obayashi Project includes utility relocation, followed by subexcavation in preparation for construction of the Transit Center Building/Train Box. Construction overseen by Webcor-Obayashi will create a new five-story Transit Center with one above-grade bus level, ground-floor, concourse, and two below-grade rail levels serving Caltrain and future California High Speed Rail, and includes new bus ramps to connect the Transit Center to a new off-site bus storage facility and the SF-Oakland Bay Bridge. Construction of the Project should be completed within or near the year 2017.

The following list generally outlines the expected Project construction schedule:

1. Utility relocation November 2010-September 2011.
2. Protection of perimeter: March 2011.
3. Trade Subcontractors awarded contracts: April 2011.
4. Activity specific SWPPPs submitted by Trade Subcontractors: April 2011.
5. Sediment control products ordered and stored on site by Trade Subcontractors: May 2011.
6. Stabilized construction entrance, equipment parking, covered storage and any concrete wash areas constructed by Trade Subcontractors: May 2011.
7. Excavation and Dewatering by Trade Subcontractors: May 2011-April 2014.

8. Transit Center Building Construction: May 2013-August 2017.
9. Bus Ramps: 4th quarter 2012-4th quarter 2014.
10. Construction of the concrete form box and train box by Trade Subcontractors: TBD.
11. Vertical Construction by Trade Subcontractors: 2013-2017.
12. Monitoring and Maintenance of BMPs: Entire construction timeline by Trade Subcontractors.
13. All BMPs functional: Entire construction timeline.

## **2.6 Potential Construction Site Pollutant Sources**

Potential materials expected from the project include, but are not limited to, excavated soil, oil products (gasoline, diesel, hydraulic oil, and kerosene), solvents, concrete and curing compounds, and other construction materials. Construction on the project site will require temporary disturbance of surface soils and removal of existing on-site pavements and subsurface structures. During the construction period, excavation and grading activities will result in exposure of soil to water runoff, and the use of haul trucks that could track material away from the construction site. Much of the excavated material will be typical of coarser sandy soil particles that do not mobilize easily. However, some of the material may consist of relatively mobile fine sediments (silt and clay). Most excavation will occur in a below-grade pit which will drain internally and contain storm water; however construction activities will impact areas outside of the excavation areas that drain toward the San Francisco combined sewer drain inlets. Water in excavation pits from rainfall and groundwater seepage would contain sediment. Removal of the pit water will probably require sediment removal before it can be discharged into the storm drains (see SF PW Code paragraph above).

Soil and debris on the haul truck tires exiting the site could be deposited on local streets and Transport in storm water into the storm drain. The majority of construction debris and materials would be loaded onto trucks within the interior of the construction boundaries, rather than from public sidewalks or streets bordering the project site. The construction debris and materials would then be hauled off site. Therefore, soil stockpiles would be minimized on site.

In addition to sediment, Table 2 lists expected construction materials that could generate pollutants, describes their chemical and physical properties, and identifies potential pollutants associated with them. This list should be updated as the project proceeds and additional phases begin.

**Table 2. Potential Stormwater Pollutants**

Source	Chemical/Physical Description	Storm Water Pollutants*
Diesel Fuel	Clear, blue-green to yellow liquid	TPH-diesel, benzene, toluene, ethylbenzene, xylenes, naphthalene
Concrete Work	Cement, fly ash, aggregate	pH
Oil and Grease	Brown oily petroleum	TPH-motor oil, oil and grease
Used Oil (oil only)	Brown oily petroleum	TPH-motor oil, oil and grease, LUFT 5 metals (cadmium, chromium, lead, nickel, and zinc)
Excavated and Stockpiled Soil	Solid particles	Soil, sediment
Gasoline	Colorless, pale brown or pink petroleum hydrocarbon	TPH-gasoline, benzene, toluene, ethylbenzene, xylenes. For “old” releases, include DIPE; ETBE; MTBE; TAME; TBA; 1,2-dibromoethane (1,2-DBA); and 1,2-dichloroethane (1,2-DCA)
Hydraulic Oil/Fluids	Brown oily petroleum hydrocarbon	TPH-hydraulic oil, benzene, toluene, ethylbenzene, xylenes, LUFT 5 metals (cadmium, chromium, lead, nickel, and zinc)
Sanitary/Septic Waste	Sewage products	Coliform, <i>E. coli</i> , viruses, solvents (i.e. volatile organic compounds such as trihalomethanes and the dichlorobenzene isomers), nitrate
Trash; Windblown and Other	Paper, pipe, electrical wires etc.	Paper, pipe, electrical wires etc.

Notes: \*TPH-gasoline = total petroleum hydrocarbons quantified as gasoline (the same pattern for TPH-diesel, TPH-motor oil, TPH-hydraulic oil)  
BTEX = benzene, toluene, ethylbenzene, and xylenes  
DIPE = di-isopropyl ether  
ETBE = ethyl tertiary butyl ether  
MTBE = methyl tertiary butyl ether  
TAME = tertiary amyl methyl ether  
TBA = tertiary butyl alcohol  
LUFT = leaking underground fuel tank  
PCBs = polychlorinated biphenyls

Pollutants of concern in the San Francisco Bay include, but are not limited to, mercury, diazinon and Polychlorinated Biphenyls (PCBs). These chemicals are not easily broken down and they tend to adhere to particles of sediment, so can be removed from stormwater in BMPs that trap sediment. For this reason, sediment trapping BMPs are highlighted in the treatment controls listed for the project. Additional pollutant categories that can be anticipated in stormwater leaving the project include oil and grease, trash, sediment, organic compounds, pesticides, nutrients and metals.

## 2.7 Identification of Non-Storm Water Discharges

Non-storm water discharges include a wide variety of sources and may contribute pollutant loads if not controlled. They can include, but are not limited to:

- discharges of process water
- saw cutting slurry
- air conditioner condensate
- non-contact cooling water
- vehicle wash water
- sanitary wastes concrete washout water
- paint wash water
- irrigation water
- pipe testing water
- natural groundwater seepage

Measures to control spills, leakage, and dumping, and to prevent illicit connections during construction must be addressed through structural as well as non-structural BMPs. Certain non-storm water discharges may be necessary for the completion of construction projects. Authorized non-storm water discharges may include those from de-chlorinated potable water sources such as: fire hydrant flushing, irrigation of vegetative erosion control measures, pipe flushing and testing, water to control dust, uncontaminated ground water dewatering, and other discharges not subject to a separate general NPDES permit adopted by a region. Authorized non-storm water dewatering discharges require a permit. Information can be found online at: [http://sfwater.org/msc\\_main.cfm/MC\\_ID/14/MSC\\_ID/445](http://sfwater.org/msc_main.cfm/MC_ID/14/MSC_ID/445).

Each Trade Subcontractor is responsible for procuring the necessary dewatering permits for construction activities undertaken. The SFPUC prohibits the discharge of storm water that causes or threatens to cause pollution, contamination, or nuisance.

Additionally, all SWPPPs prepared by Trade Subcontractors must include procedures and practices designed to minimize or eliminate the discharge offsite of pollutants from vehicle and equipment cleaning, fueling, maintenance operations and other non-storm water. Project monitoring by trade Subcontractors will include a visual check for non-storm water discharges and non-storm water discharge potential.



### **3 BEST MANAGEMENT PRACTICES (BMPS)**

BMPs shall be implemented as listed in this Plan and additionally as necessary to adequately minimize erosion on site and limit sediment transport off site to an acceptable level in accordance with the SFPUC regulations and all City Codes and Ordinances.

Erosion and sediment control measures are needed throughout the year on the Project. In particular, stormwater catch basins must be protected year round. During dry season development, BMPs will be primarily designed to mitigate the movement of sediment and pollutants off site by tracking from grading equipment and from wind. Wet season BMPs are designed to prevent soil from washing off graded areas during rainy periods, tracking of soil and pollutants off site by vehicles and any other movement of pollutants from the Project.

#### **3.2 BMP Objectives**

This Construction Stormwater Pollution Control/Compliance Plan provides the following BMP objectives:

- Provide overall guidance to Trade Subcontractors in preparing SWPPPs and dewatering plans specific to their construction activities, construction timelines and drainage areas for submittal to the SFPUC.
- Delineate typical construction pollutants and their sources, including sources of sediment associated with construction, construction site erosion and other activities associated with anticipated construction activity. Trade Subcontractors are expected to expand and amend the information provided here within to tailor their SWPPPs to their activities.
- Outline best management practice (BMP) categories that need to be included in the SWPPPs prepared, submitted and maintained by the Trade Subcontractors to a level that results in the reduction or elimination of pollutants in storm water discharges and authorized non-storm water discharges from construction activity to the standard required by the SFPUC.

BMPs categories listed in this Construction Stormwater Pollution Control/Compliance Plan should be reviewed by the Trade Subcontractors, added to their SWPPPs as applicable and additionally installed, maintained, monitored and reported as practicable to adequately minimize erosion on site and limit sediment transport off site to an acceptable level. Adjustments and modifications to the BMPs identified in this Plan need to be implemented by the Trade Subcontractors as necessary to maintain the construction site in accordance with the provisions of the SFPUC regulations and all City Codes and Ordinances.

The SFPUC identifies the following list of BMPs and pollution prevention measures that must be implemented at all construction sites:

- Identify all storm drains and catch basins near the construction site and ensure all workers are aware of their locations to prevent pollutants from entering them.
- Protect all storm drain and catch basin inlets.
- Develop an erosion control and sediment control plan for wind and rain.
- Develop spill response and containment procedures.
- Inspect site regularly to ensure that BMPs are intact.

- Conduct daily site cleanings as needed.
- Educate employees and subcontractors about BMPs.
- Regularly maintain all BMPs at project site.

### 3.2.1 Erosion Control BMPs

Erosion control practices consist of source control measures designed to prevent soil particles from becoming dislodged and transported in storm water runoff, while sediment control measures filter and otherwise recover soil particles from runoff. Erosion control BMPs protect the soil surface by covering and/or binding soil particles and in many cases, are more effective, less expensive, and require less maintenance and repair. Although they typically function by protecting the surface of exposed soil, erosion control measures cannot be effectively applied until grading activities are complete or idle.

At the Project, erosion is expected to occur primarily as a result of pavement removal, soil disturbance and subsequent wind or rain. For this reason, BMPs to limit the timing of soil disturbance and provide timely stabilization for the disturbed soil surface should be the focus of erosion control efforts for the site. Erosion control BMPs such as scheduling and non-vegetative soil stabilization (soil binders) should be considered by each Trade Subcontractor (TS) and added to their SWPPPS to control soil erosion on the construction site. Modifications to the BMPs may be necessary should construction activities or the construction schedule be altered. If modifications are needed to the BMPs, the Trade Subcontractor should work with the SFPUC to amend the SWPPP and Erosion Control BMPs to satisfactorily meet City storm water regulations.

Scheduling should be implemented throughout the project as a means of ensuring that significant earth-disturbing activities are avoided if rain is forecasted. If there are exposed areas that are not being actively worked the trade Subcontractors should consider stabilizing all areas as practical. If additional information or instructions are needed for BMP installations, the CASQA website and cutsheets can be found at: **[www.casqa.org](http://www.casqa.org)**.

### 3.2.2 Sediment Control BMPs

Sediment control is any practice that traps soil particles after they have been detached and moved by rain, flowing water, or wind. Sediment control measures are usually passive systems that rely on filtering or settling the particles. Sediment control, or capturing the sediment once it is mobilized, is considered back up or secondary to good erosion control.

Table 3 indicates the BMPs for sediment control that should be considered and included in SWPPPs by trade Subcontractors as applicable on the construction site.

**Table 3. Construction Sediment Control BMPs**

<b>BMP Name</b>
Silt Fence
Fiber Rolls
Gravel Bag Berm
Sand Bag Barrier
Storm Drain Inlet Protection
Stockpile Management

If additional information or instructions are needed for BMP installations, the CASQA website and Cutsheets can be found at: **[www.casqa.org](http://www.casqa.org)**.

### 3.2.3 Tracking Control BMPs

Tracking control consists of preventing or reducing the tracking of sediment off site by vehicles. Daily inspections will be conducted at the construction entrances and if track-out is observed, the area will be swept by the Trade Subcontractors. If additional information or instructions are needed for BMP installations, the CASQA website and cutsheets can be found at: **[www.casqa.org](http://www.casqa.org)**.

### 3.2.4 Wind Erosion Control BMPs

Wind Erosion Control is a very important BMP for the Project. All Trade Subcontractors are required to comply with the regulations specified by the local Air Quality Control District. Construction will be halted if required to do so due to high wind conditions as specified by the local Air Quality Control District, and/or common sense. Alternative forms of wind erosion control such as tackifiers and covers will be utilized as necessary to avoid and minimize windblown dust from leaving the project site. If additional information or instructions are needed for BMP installations, the CASQA website and cutsheets can be found at: **[www.casqa.org](http://www.casqa.org)**.

### 3.2.5 Non-Storm Water Control BMPs

Non-storm water management BMPs are source control BMPs that prevent pollution by limiting or reducing potential non-storm water pollutants at their source or eliminating offsite discharge. These practices involve day-to-day operations of the construction site and are also referred to as “good housekeeping practices” which involve keeping a clean, orderly construction site.

Non-storm water management BMPs includes procedures and practices designed to minimize or eliminate the discharge of pollutants from vehicle and equipment cleaning, saw cutting, pipe testing and other activities that generate liquid slurry or water based effluent. All storm/sanitary drain inlets should be located and protected during construction such that non-storm water carrying pollutants does not enter the inlets. Paving and concrete work should be undertaken during dry weather and drain inlets covered

during these activities. During wet weather construction, the drain inlets should be protected with a BMP that filters water such as sediment traps, silt bags and straw wattle.

### 3.2.6 Waste Management/Materials Control BMPs

Waste management and materials pollution control BMPs, like non-storm water management BMPs, are source control BMPs that prevent pollution by limiting or reducing potential pollutants at their source before they come in contact with storm water.

These BMPs also involve day-to-day operations of the construction site, are under the control of the Trade Subcontractors, and are additional “good housekeeping practices” which involve keeping a clean, orderly construction site. Waste management consists of implementing procedural and structural BMPs for handling, storing, and disposing of wastes generated by a construction project. The objective is to prevent the release of waste materials into storm water runoff or discharges through proper management of the following types of wastes:

- Solid
- Sanitary
- Concrete
- Hazardous
- Equipment – related wastes

Materials pollution control (also called materials handling) consists of implementing procedural and structural BMPs in the handling, storing, and the use of construction materials. The BMPs are intended to prevent the release of pollutants during storm water and non-storm water discharges. The objective is to prevent or reduce the opportunity for contamination of storm water runoff from construction materials by covering and/or providing secondary containment of storage areas, and by taking adequate precautions when handling materials. Material Safety Data Sheets, covered and secondary containment and employee training are important examples of materials pollution control. These controls must be implemented for all applicable activities, material usage, and site conditions by each Trade Subcontractor working on the Project.

The following BMP Table 4 indicates the BMPs for Trade Subcontractors to utilize to control construction site wastes and materials for the project.

**Table 4. Waste Management and Material Handling Control BMPs**

<b>BMP Name</b>
Material Delivery & Storage
Material Use
Spill Control
Solid Waste Management
Hazardous Materials/ Waste Management
Concrete Waste Management
Sanitary/Septic Waste Management
Liquid Waste Management

Fuel (gasoline/diesel), hydraulic oil, motor oil, and other liquid or hazardous waste materials used for vehicle and equipment maintenance may be used on the construction site and at the lay down areas if applicable permits are obtained and spill/response measures are adhered to. Minor amounts of lubricants and hydraulic fluid may be stored in vehicles. Spill response equipment will also be located onsite and near active construction.

Waste management BMPs includes procedures and practices designed to minimize or eliminate the discharge of pollutants from vehicle and equipment use, as well as fueling and maintenance operations to storm water drainage systems or to watercourses. Drip pans, diapers or alternative containment will be placed under equipment and vehicles (as applicable during maintenance or if leaking is suspected) while not in use, to catch and/or contain drips and leaks and prevent soil contamination. Construction crews will be educated to check parking areas visually for signs of leaking liquids; any vehicles found to be leaking onto the soil surface will be provided with temporary drip pans while at the project site. Fueling may be conducted on the job site and at the lay down area if fueling BMPs are implemented, appropriate permits are obtained and proper spill control policies and procedures are followed.

It is important that Trade Subcontractors minimize or abate the exposure of materials stored or spilled at the site. Spill Response Procedures for smaller spills are presented in BMPs. If a larger spill or discharge offsite occurs, or if the project receives a written notice or order from any regulatory agency, Trade Subcontractors will follow their Health & Safety Plan and Spill Prevention Countermeasure and Control Plan (SPCC) as well as comply with all Federal, State and local spill reporting regulations.

## 4 BMP INSPECTION, MAINTENANCE AND RECORD KEEPING

Inspection and maintenance of BMPs are an integral part of the Project and will be followed by the Trade Subcontractors. During visual inspections, if any BMP deficiencies or any storm water compliance issues are observed, the Trade Subcontractor's Construction Supervisor will be notified immediately and the deficiencies will be corrected as soon as possible. The Trade Subcontractors are responsible for maintaining and/or submitting any required monitoring records as required by regulatory agencies in accordance with current regulatory guidelines.

**Table 5. Trade Subcontractor Maintenance, Monitoring and Repair Procedures**

PRACTICE	MONITORING, MAINTENANCE AND REPAIR PROCEDURES
Erosion Control	Check all soil protection including fabric, plastic, rock, hydroseed, mulch and velocity dissipation before, during and after rain events. Repair or replace as necessary to maintain proper function.
Street Cleaning	Streets must be periodically cleaned. Large quantities of soil tracked onto the street will be picked up by a loader bucket and/or hand shoveled back onto the pad. Streets must also be swept on an as-needed basis to maintain continuous sediment and litter control. Street washing shall not be done.
Sediment Control	Check integrity and functioning of berms, straw bales, check dams, and silt fences. Repair any eroded areas and remove accumulated debris.
Inlet Protection	Monitor installation and maintenance of sediment barriers and inlet protection devices. Check periodically during storms and repair or remove sediment as necessary to maintain appropriate functioning.
Temporary Basins	Remove accumulated sediment when sediment accumulates to within one foot of the outlet elevation and restore original dimensions of the basin. Obtain dewatering discharge permit from SFPUC prior to any dewatering of stored surface or groundwater.
Materials/ Equipment Storage	<ul style="list-style-type: none"> <li>Petroleum products shall be stored out of the rain and waste materials shall be stored in secured containers. Paints, solvents, enamels, sealers, bonding agents, and other chemicals shall be stored inside a covered, secure area.</li> <li>Keep designated storage areas clean and well organized. Conduct weekly monitoring to check for damaged containers, leaks, etc.</li> <li>Keep chemicals in original containers and keep them labeled.</li> <li>Train employees and subcontractors on the use of the storage area.</li> </ul>
Fueling Practices	<ul style="list-style-type: none"> <li>If refueling of equipment is conducted on site, make sure that</li> </ul>

PRACTICE	MONITORING, MAINTENANCE AND REPAIR PROCEDURES
	<p>fueling is occurring in designated areas and that secondary containment items such as drain pan or drop cloth are nearby to catch fuels/leaks.</p> <ul style="list-style-type: none"> <li>• Inspect and maintain vehicles and equipment regularly to minimize leaks and drips.</li> <li>• Comply with Federal, State and local requirements for fuel storage tanks.</li> </ul>
Herbicide/ Pesticide Application	Provide the landscape contractor with knowledge about proper procedures for application of designated chemicals.
Waste Disposal	Provide proper disposal procedures for specific materials
Litter Control	Place trash bins in appropriate locations and are being used properly. Pets will not be allowed on the Project during construction.
Equipment Cleaning	If equipment cleaning is done on site, make sure contractors are using designated, bermed wash areas to prevent wash water from entering storm drain system.

## 5 LIST OF CONTRACTORS/SUBCONTRACTORS

The following is a partial list of Trade Subcontractors, suppliers and consultants that may be employed on the Project. Names and contact numbers for each activity on the list can be obtained from Webcor /Obayashi upon request. This list is to be updated as necessary. This plan can be utilized as part of a subcontractor notification letter to document Subcontractors notification of their obligation to uphold applicable storm water pollution control regulations.

TRADE	NAME	Signature Indicating Willingness To Provide, Maintain, and Implement SWPPP in compliance with all applicable City Ordinances and Codes
Architect		
Bricklayers		
Cabinet Makers		
Carpenters (finish)		
Carpenters (rough)		
Ceramic Tile Installers		
Civil Engineer		
Cleaning Crews		
Concrete Subcontractors Testers		
Demolition Contractors		
Door Installers		
Drywall Installers		
Electricians		
Environmental Consultants		
Fence Builders		
Fireplace Installer		
Flooring Installers		
Garage Door Installers		
Glass Workers		
Grading Contractors		
Hardware Installers		
HVAC Contractors		
Insulation Contractors		
Marble Contractors		
Masonry Contractors		



TRADE	NAME	Signature Indicating Willingness To Provide, Maintain, and Implement SWPPP in compliance with all applicable City Ordinances and Codes
Millwork Suppliers		
Landscaping Contractors		
Landscape Maintenance Crews		
Lumber and Truss Suppliers		
Mirror and Shower Door Installers		
Painting Contractors		
Paving Contractors		
Pipeline Contractors		
Plaster Contractors		
Plumbing Contractors		
Roofing Contractors		
Shelving Installers		
Striping and Signage Contractors		
Stucco Contractors		
Termite Contractors		
Underground Utility Crews	Trinet	
Waterproofing Subcontractors		
Window Installers		

## **6 INSTRUCTIONS TO FIELD PERSONNEL**

Webcor /Obayashi will be responsible for mandating that SWPPP documents be prepared by Trade Subcontractors and also for observing the site on a regular basis in keeping with the standard of care for a General Contractor. Webcor /Obayashi will coordinate day to day oversight of the Project as a whole, track compliance with their contract obligations as well as Trade Subcontractor costs, direct Trade Subcontractors to maintain the Project site in accordance with all applicable regulations, and attend to discussions with the City regarding compliance concerns. Contracts with Trade Subcontractors and Sub tier Subcontractors shall include a requirement to comply with the provisions of this Plan and to maintain compliance with all applicable City Ordinances and Codes. The Trade Subcontractors, Sub tier Subcontractors and their Project Superintendents for this project are hereby authorized to uphold, certify, and maintain their own SWPPPs and to distribute it to all field personnel responsible for monitoring the site and maintaining compliance with storm water regulations. All subcontractors, field personnel and their assigns that work at the site must conform to the requirements described in this Plan and the SWPPP developed for Trade Subcontractor activities and any alterations thereof made at the time and in the manner herein specified, and in all respects according to its intent and meaning, and shall indemnify and hold harmless Webcor /Obayashi, its officers and agents, if failure to conform results in legal action or any other action by the Regional Water Quality Control Board or City. Duties of the Trade Subcontractors include but are not limited to:

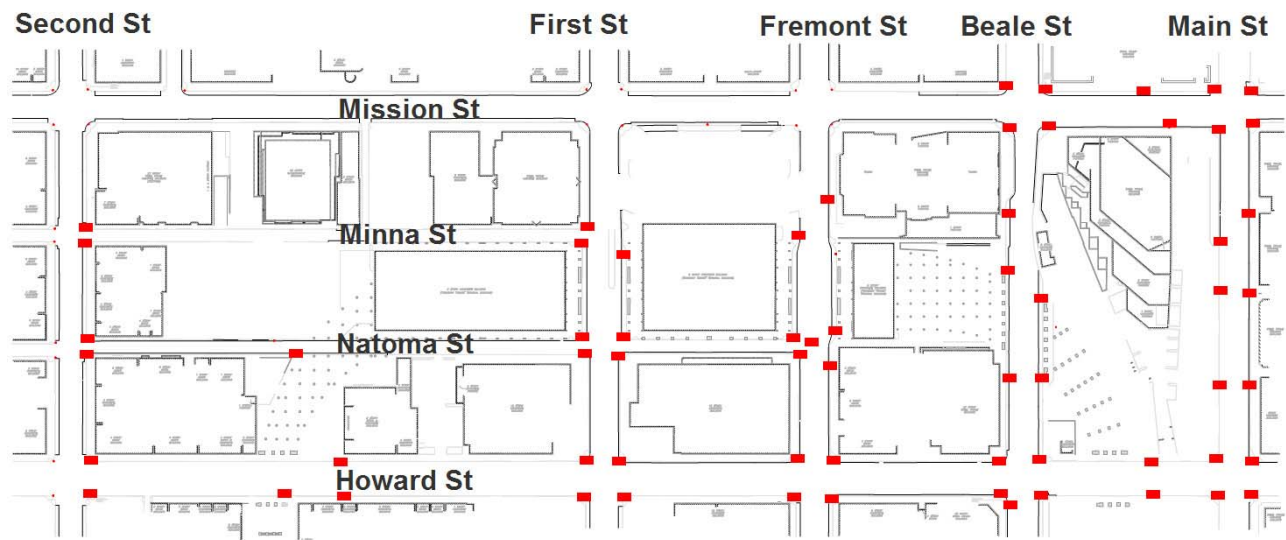
- Maintaining full compliance with their SWPPP and all City Codes and Ordinances.
- To this effect, the Trade Subcontractors shall have authority to mobilize their own crews for:
  - BMP Installation, monitoring and maintenance.
  - Obtaining dewatering and other applicable permits necessary for the satisfactory completion of their contract.
  - Providing for elimination of all unauthorized discharges.
  - Coordinating with the City such that all updates, amendments, corrections and/or repairs are made in a timely fashion.
  - Stopping any construction activity that is in violation of municipal ordinances or codes or that is inconsistent with the provisions of the Trade Subcontractors SWPPP.

## 7 CLOSING

The Project will comply with the storm water discharge regulatory framework in the site vicinity through implementation of this Construction Stormwater Pollution Control/Compliance Plan. This Plan indicates that each Trade Subcontractor is responsible for preparing, submitting for approval, installing and maintaining a SWPPP with BMPs for protecting inlets to the SF combined sewer system from construction activities. BMPs included in the SWPPPs prepared by each Trade Subcontractor should include practices from the BMP categories outlined in this Plan. The SWPPP shall be implemented concurrently with the commencement of Trade Subcontractor construction activities and maintained by the Trade Subcontractor in a form that provides the Project with full compliance throughout the construction schedule for activities undertaken by the Trade Subcontractor. Though projects such as the subject Project that are serviced by the combined sewer system in San Francisco are not subject to the terms of the State Construction General Permit, Section A of the Construction General Permit describes in detail the requirements for a SWPPP, and the City and County San Francisco specifies that it should be used as a design guide. All construction sites must prevent illicit discharge into the SF combined sewer system.

## **Appendix A     Inlet Location Map**

**TRANSBAY TRANSIT CENTER**  
**Existing Catch Basin**



## **Appendix B Construction Stormwater Controls Monitoring Checklist**

# CONSTRUCTION STORMWATER CONTROLS MONITORING CHECKLIST

WEBCOR/OBAYASHI TRANSBAY TERMINAL PROJECT

Date: \_\_\_\_\_

Inspector Name: \_\_\_\_\_ Description of Inspected Area: \_\_\_\_\_

24hr Rainfall Amount: \_\_\_\_\_ Weather Conditions: \_\_\_\_\_

Name of Trade Subcontractor Representative: \_\_\_\_\_ Contact (Cell Phone #): \_\_\_\_\_

Erosion/Sediment Controls	Repairs Needed	OK	Owner of Repair Task	Comments/Date Corrected
Check Dams/Sediment Traps	<input type="checkbox"/>	_____	_____	_____
Drainage Swales/Lined Ditches	<input type="checkbox"/>	_____	_____	_____
Entrance/Outlet/ Tire Wash	<input type="checkbox"/>	_____	_____	_____
Barrier (Sandbag/Gravel Bag)	<input type="checkbox"/>	_____	_____	_____
Fiber Rolls/Wattles/ Silt Fence	<input type="checkbox"/>	_____	_____	_____
Covers (Geotextile/Fabric/Plastic)	<input type="checkbox"/>	_____	_____	_____
Inlet Protection	<input type="checkbox"/>	_____	_____	_____
Soil Tackifiers/Dust Control Emulsions	<input type="checkbox"/>	_____	_____	_____
Street Sweeping/Vacuuming	<input type="checkbox"/>	_____	_____	_____
Other:	<input type="checkbox"/>	_____	_____	_____

Good Housekeeping Controls	Repairs Needed	OK	Owner of Repair Task	Comments/Date Corrected
Concrete Washout	<input type="checkbox"/>	_____	_____	_____
Dewatering System/Operation	<input type="checkbox"/>	_____	_____	_____
Illicit Connection Detection	<input type="checkbox"/>	_____	_____	_____
Material Delivery/Storage/Use)	<input type="checkbox"/>	_____	_____	_____
Paving and Grinding Operations	<input type="checkbox"/>	_____	_____	_____
Pile Driving Operations	<input type="checkbox"/>	_____	_____	_____
Sanitary/Septic Waste Management	<input type="checkbox"/>	_____	_____	_____
Spill Prevention and Control	<input type="checkbox"/>	_____	_____	_____
Equipment Servicing	<input type="checkbox"/>	_____	_____	_____
Waste Management	<input type="checkbox"/>	_____	_____	_____

Visual Observation of Runoff	Repairs Needed	OK	Owner of Repair Task	Comments/Date Corrected
Sediment Laden/Turbid	<input type="checkbox"/>	_____	_____	_____
Oily Sheen	<input type="checkbox"/>	_____	_____	_____
Odor	<input type="checkbox"/>	_____	_____	_____

Documentation	Repairs Needed	OK	Owner of Repair Task	Comments/Date Corrected
SWPPP on Site	<input type="checkbox"/>	_____	_____	_____
BMP materials Stockpiled	<input type="checkbox"/>	_____	_____	_____
Spill Control in Compliance	<input type="checkbox"/>	_____	_____	_____
Discharge Permit Posted	<input type="checkbox"/>	_____	_____	_____
Training Logs Available	<input type="checkbox"/>	_____	_____	_____
Inspection Logs Filled Out	<input type="checkbox"/>	_____	_____	_____
Other:	<input type="checkbox"/>	_____	_____	_____

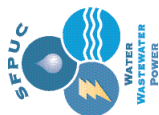
Comments: \_\_\_\_\_

\_\_\_\_\_

## **Appendix C      SFPUC Construction Pollution Prevention Guide**



*Don't Be Caught  
Unaware  
New  
Pollution  
Prevention  
Requirements  
for the  
Construction  
Industry*



**Water Pollution Prevention Program**  
San Francisco Public Utilities Commission  
City and County of San Francisco  
3801 3rd Street, Suite 600  
San Francisco CA, 94124

# Keep it on Site

## Pollution Prevention Guide

for the

### Construction Industry



## Keep it on Site

The San Francisco Public Utilities Commission (SFPUC) is pleased to announce **Keep it on Site**, as part of its new program to prevent water pollution at construction sites.

Runoff from construction sites is a major source of water pollution, and is subject to requirements such as the development of a stormwater pollution prevention plan, a plan review, stormwater treatment measures, runoff monitoring and increased site inspections.

As part of our Construction Site Water Pollution Prevention Program, this brochure will assist construction professionals understand and comply with the new State and Federal laws. Here, you will find valuable information on methods used on construction sites to keep pollution, such as dirt and construction site debris out of our sewage treatment system and sensitive local water bodies.

We hope to make your job easier while keeping our city clean by providing you with the information to create an efficient and environmentally safe construction site.

Together, we have the ability to preserve the quality of life in San Francisco.



Water Pollution Prevention Program  
San Francisco Public Utilities Commission  
City and County of San Francisco  
3801 3rd Street, Suite 600  
San Francisco CA, 94124

Construction Site Runoff: (415) 695-7310  
<http://pollutionprevention.sfwater.org>

## Water Pollution Prevention Program

The goal of the Water Pollution Program is to control pollution at its source in order to protect the Bay, ocean, creeks and lakes.

Useful links about other pollution prevention programs throughout San Francisco:

San Francisco Water Pollution Prevention Program  
<http://pollutionprevention.sfwater.org>

State Water Board  
[www.waterboards.ca.gov/sanfranciscobay](http://www.waterboards.ca.gov/sanfranciscobay)

International BMP Database  
[www.bmpdatabase.org](http://www.bmpdatabase.org)

California Stormwater Quality Association  
[www.cabmphandbooks.com](http://www.cabmphandbooks.com)

### Emergency Phone Numbers

To report illegal dumping of hazardous materials or wastes to the storm drain or sewer system, call San Francisco Water Pollution Prevention Program hotline: (415) 695-2020

### Hazardous Spills: 911

### Inspection and Enforcement Program

The Construction Site Inspection and Enforcement Program was established to ensure that all businesses operate in compliance with all appropriate stormwater laws and other City requirements. Contractors, site supervisors and property owners can be held responsible for violations, which may lead to a civil penalty of up to \$25,000 per day and reimbursing the City for all expenses associated with clean up<sup>1</sup>.

Construction materials such as paint, dirt, and trash often find their way into our storm drains,

<sup>1</sup> San Francisco Sewer Use Ordinance Article 4.1, Public Works Codes

## Best Management Practices

jeopardizing San Francisco's sewer system, and polluting surrounding local water bodies.

Contractors are now required to implement what are known as Best Management Practices (BMPs) on all construction sites. BMPs are methods used to keep pollution out of our storm drains and catch basins and off of City property such as sidewalks, streets, and alleys. Installing and maintaining these BMPs on the construction site is critical to protecting our sensitive water bodies.

If your project is greater than 1 acre, you are required to prepare a formal Stormwater Pollution Prevention Plan (SWPPP). Please contact SFPUC's Environmental Regulation and Management for more information at (415) 695-7310.

The following is a list of BMPs and pollution prevention measures that must be implemented at all construction sites.

- Identify all storm drains and catch basins near the construction site and ensure all workers are aware of their locations to prevent pollutants from entering them.
- Protect all storm drain and catch basin inlets.
- Develop an erosion control and sediment control plan for wind and rain.
- Develop spill response and containment procedures.
- Inspect site regularly to ensure that BMPs are intact.
- Conduct daily site cleanings as needed.
- Educate employees and subcontractors about BMPs.
- Regularly maintain all BMPs at project site.



# BEST MANAGEMENT PRACTICES

## Site Overview

This drawing illustrates Best Management Practices (BMPs) that must be followed at all construction sites in San Francisco.

## Preserve existing vegetation

Preserving existing trees and vegetation where possible will prevent erosion.

## Paint and Stucco

All paint and stucco materials stored on the site must be contained and covered. It is illegal for contractors to wash out paintbrushes in the street or dump any residues in the sewer or the storm drain. Paintbrushes and spray guns shall be washed/cleaned out into a hazardous materials barrel or put back into its original container and disposed of properly. Latex paint should be dried in its container and placed in the garbage. Oil paint and thinners need to be recycled as hazardous wastes.

## Perimeter Controls

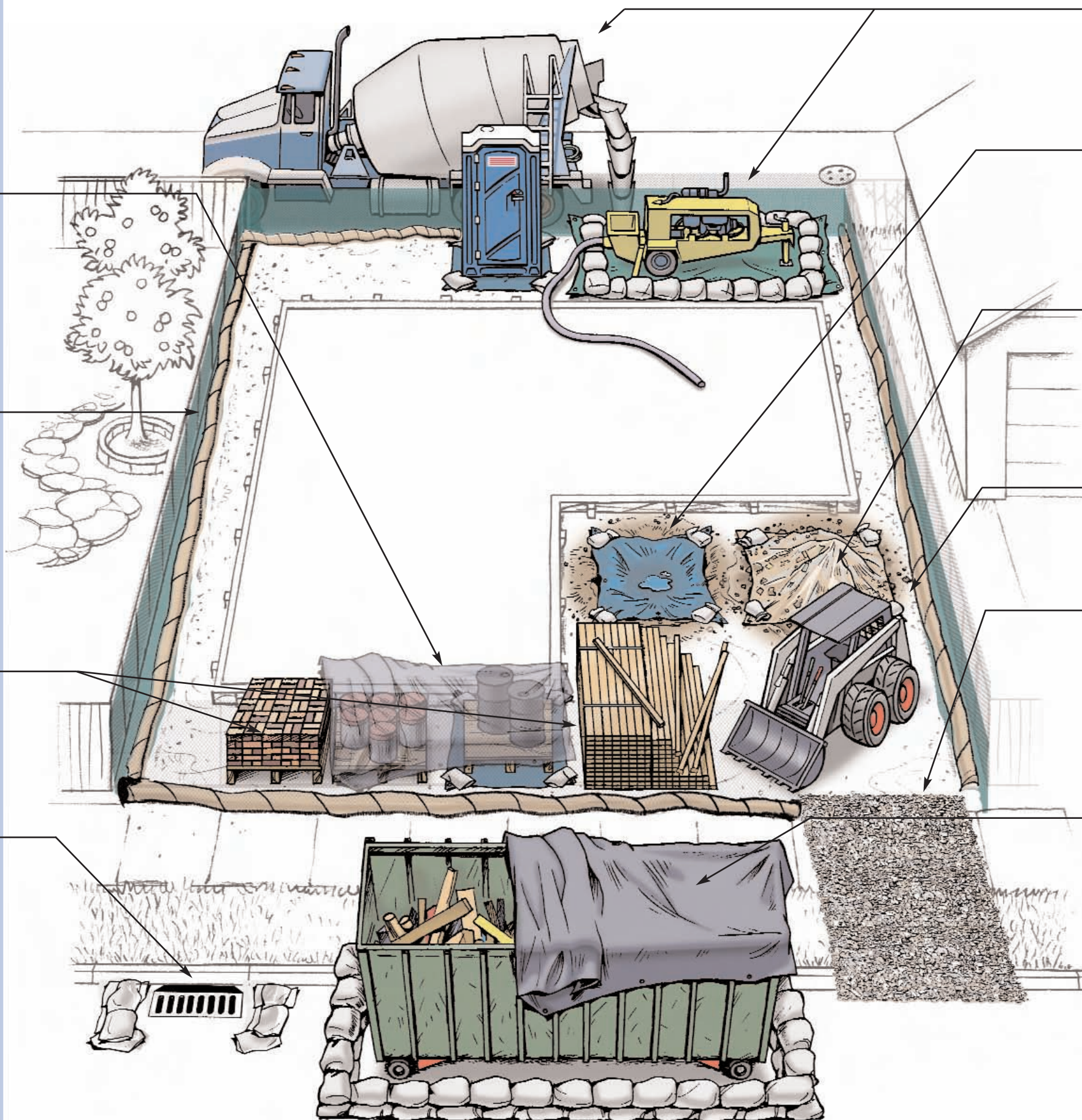
Gravel bags, silt fences, and fiber rolls are acceptable perimeter controls, and shall be used to surround the entire site. Upstream perimeter controls prevent water from running into your site and downstream controls prevent sediment from leaving your site. Avoid running over perimeter controls with vehicles or heavy equipment, as they can damage the materials. Replace any damaged perimeter controls immediately. Keep extra absorbent materials and/or a wet/dry vacuum on site to quickly pick up unintended spills. Sites must also be checked and maintained daily.

## Building Materials / Staging areas

Construction materials must be stored onsite at all times. The only exception is if you have a right-way-permit. Building materials should always be covered when not in use to prevent runoff caused by wind or rain. To apply for a right-of-way permit, contact the Bureau of Streets Use and Mapping at (415) 554-5810.

## Storm Drains and Catch Basins

Storm drains must be protected at all times with perimeter controls, such as fiber rolls or gravel bags.



## Concrete Trucks / Pumpers

Any concrete pumpers parked in public streets or alleys must be surrounded by perimeter controls, such as berms, gravel bags or fiber rolls. Tarps also must be placed beneath concrete pumpers at all times. Residual materials must be cleaned up as well.

## Washout Area

The disposal of "wet" construction materials should be handled in the washout area. This includes paint, stucco, and concrete. Use a gravel bag or fiber roll and tarp to collect evaporation and prevent run-off in nearby areas. The washout area must be checked and maintained daily to ensure compliance.

## Dirt and Grading

Mounds of dirt or gravel should be stored on site and covered each day with a tarp. When in use, all exposed dirt piles should be sprayed with water to prevent excessive dust. Tarps must be available and onsite to cover 125% of exposed areas during the rainy season (October-April).

## Earthmoving Equipment

All earthmoving equipment should be stored onsite. Maintenance and repair should never be conducted on the site. All tracks and trails left by equipment leading to and from the site should be cleaned up immediately.

## Construction site stone or rock access drives

Stone or rock access drives at any construction site should be made of 3-4 inch fractured stone aggregate with a geo-textile liner below the grade of the road. This is to be used by all vehicles to limit tracks of mud onto the streets.

## Dewatering Activities

A batch discharge permit is required before releasing any construction site wastewater. Call 415-695-7310 for more information.

## Dumpsters

Keep dumpsters covered. Areas around dumpsters should be swept daily.



## Water Pollution Prevention Program

San Francisco Public Utilities Commission  
City and County of San Francisco  
3801 3rd Street, Suite 600  
San Francisco CA, 94124  
(415) 695-7310

[siterunoff@sfgov.org](mailto:siterunoff@sfgov.org)  
<http://pollutionprevention.sfgov.org>

Original artwork and concepts developed by the City of Coronado, CA  
revised by SFPUC Graphics staff personnel.



## **Exhibit L**



### **TRANSBAY TRANSIT CENTER**

### **Hazardous Materials Management Plan**

Revision 1

**March 11, 2011**

**WEBCOR/OBAYASHI JOINT VENTURE  
SAN FRANCISCO, CA**

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**Hazardous Materials Management Plan  
TRANSBAY TRANSIT CENTER  
San Francisco, California**

Webcor/Obayashi Joint Venture will be responsible for mandating that Hazardous Materials Procedures documents shall be prepared by Trade Subcontractors and also for observing the Trans Bay Transit Center site on a regular basis in keeping with the standard of care for a General Contractor. Webcor/Obayashi Joint Venture will also coordinate the day to day oversight of the Project as a whole, compliance with their contract obligations, the tracking of Trade Subcontractor costs, directing Trade Subcontractors to maintain the Project site in accordance with all applicable regulations, and for discussions with the City regarding compliance concerns. Contracts with Trade Subcontractors and Sub tier Subcontractors shall include a requirement to comply with the provisions of this Plan and to maintain compliance with all applicable City Ordinances and Codes. The Trade Subcontractors, Sub tier Subcontractors and their Project Superintendents for this project are hereby authorized to uphold, certify, and maintain their own Hazardous Materials Procedures Plans and to distribute it to all field personnel responsible for monitoring the site and maintaining compliance with Federal State and local regulations. All subcontractors, field personnel and their assigns that work at the site must conform to the requirements described in this Hazardous Materials Procedures developed for Trade Subcontractor activities and any alterations thereof made at the time and in the manner herein specified, and in all respects according to its intent and meaning, and shall indemnify and hold harmless Webcor Builders-Obayashi, its officers and agents, if failure to conform results in legal action or any other action. Duties of the Trade Subcontractors include but are not limited to:

- Maintaining full compliance with their Hazardous Materials Procedures plan and all City Codes and Ordinances.
- To this effect, the Trade Subcontractors shall have authority to mobilize their own crews for: monitoring and maintenance.
- Obtaining dewatering and other applicable permits necessary for the satisfactory completion of their contract.
- Stopping any construction activity that is in violation of municipal ordinances or codes or that is inconsistent with the provisions of the Trade Subcontractors Hazardous Materials Procedures plan.

The Transbay existing Terminal Building has been demolished and replaced with a multimodal Transit Center that includes an underground rail station. The depth of the excavation will be approximately 65 feet. A soil-cement shoring wall extending approximately 120 feet below ground surface (bgs) will form the perimeter of the Transit Center. A concrete buttress will be placed under the Transit Center adjacent to 301 Mission Street extending down to bedrock, approximately 240 feet.

This HMMP includes the requirement to mitigate potential health and safety (H&S) risks to the environment, workers, and site-user associated with the presence of certain constituents in the soil at the Site.

## **ENVIRONMENTAL REPORTS**

Webcor /Obayashi Joint Venture have reviewed environmental reports prepared for the site. The following is a summary of the previous reports:

### **Phase I Environmental Site Assessment**

The eastern portion of the Site is located in an area historically known as the Tar Flat which was a former industrial area developed during the Gold Rush Era of the 1850's. The Site has been occupied by numerous buildings involved in metal work facilities, foundries, and a coal yard. Also, the San Francisco Gas Light Company was located on the south central and south eastern edge Site. Coal tar waste is believed to have been discharged into the surrounding tidelands which include the eastern portion of the Site. The Transbay Terminal Building was constructed between the years of 1936 ad 1938 and was used as a passenger rail station. In 1958, the train tracks were removed and/or paved over and the Site has been used by buses since. In the 1950's, elevated concrete roadways were built on the Site as part of the Transbay Terminal and the Embarcadero Freeway. The Embarcadero Freeway was damaged during the 1989 Loma Prieta earthquake and was subsequently demolished. Since the 1990's, the Site has remained largely unchanged.

Significant findings included:

- The subsurface fill material at the Site may contain elevated concentrations of heavy metals and other residual petroleum hydrocarbons. These concentrations are likely associated with the presence of 1906 earthquake fill material located below the ground surface. Special soil handling and/or sampling will likely be required during any construction activities.

- Due to the proximity of the former San Francisco Gas and Light Plant (bounded by First, Fremont, Howard, and Natoma Streets) and the presence of manufactured gas by-product waste found on nearby properties, hazardous materials may exist in the subsurface beneath the Site. Special soil handling and/or sampling will likely be required during any construction activity.
- The soil and groundwater near the West section of the Transbay Terminal Building may contain petroleum hydrocarbons and VOCs associated with the former USTs release. Special soil and groundwater handling and/or sampling will likely be required during any construction activities.

### **Site Investigations**

Limited soil and groundwater sampling has been performed beneath the ramps and near the Transbay Terminal building in 1999 and 2008 by Treadwell & Rollo. Also, they performed an Environmental Site Characterization (ESC) in 2009 at the Transbay Terminal which included collecting soil samples of the fill material and underlying sand from 23 exploratory borings, chemical testing of selected samples, and evaluating the results. Treadwell & Rollo collected groundwater grab samples from four of the exploratory borings for chemical analysis. The objective of the ESC was to assess the presence of petroleum hydrocarbon and metal contamination in the soil and groundwater beneath the Site that will be removed and disposed during the proposed construction activities. Concentrations of chemical compounds and metals detected in the soil and groundwater samples were compared to state and federal criteria for hazardous waste and disposal options.

The results of our environmental site characterization and other available subsurface information in the vicinity indicate the Site is generally underlain by approximately 5 to 16 feet of fill material, composed of loose to medium dense silty sand with varying amounts of brick, wood, tar, and glass fragments. The presence of fill material underlying the Site is likely associated with the 1906 earthquake and fire. A sand layer consisting of medium dense to very dense sand with variable amounts of silt approximately 15 to 18 feet thick underlies the fill material. Bay Mud is present beneath the sand layer.

### **Soil Results**

TPHg was detected above the method reporting limit (0.1 mg/kg) in 3 of the 88 samples analyzed at concentrations ranging from 0.29 mg/kg to 26 mg/kg. TPHd was detected above the method reporting limit (2 mg/kg) in 9 of the 87 samples analyzed at concentrations ranging from 2.01 mg/kg to 54.8 mg/kg. TPHmo was detected above the method reporting limit (4 mg/kg) in 49 of the 88 samples



analyzed at concentrations ranging from 4.09 mg/kg to 137 mg/kg. Methylene chloride was detected in 3 of the 14 samples analyzed at concentrations ranging from 0.056 mg/kg to 0.24 mg/kg. No other VOCs were detected at or above methods reporting limits.

Total cyanide was not detected above the method reporting limit (1 mg/kg) in any of the 5 samples analyzed. No SVOCs, Pesticides, PCBs, Sulfide, or Cyanide were detected at or above method reporting limits in the samples analyzed. The pH measured in five samples ranged from 6.70 standard units (S.U.) to 8.66 S.U.

Total lead was detected in each of the samples analyzed at concentrations ranging from 1.2 mg/kg to 1,000 mg/kg (Table 2). Total lead was detected at concentrations at or above 50 mg/kg but below 1,000 mg/kg in 33 soil samples. Each of these soil samples was subsequently run for STLC and TCLP lead to determine soluble lead levels. One soil sample (TR-21-5) matched the State of California hazardous waste criteria of 1,000 mg/kg for total lead and subsequently run for TCLP lead to determine if this soil represents a federal RCRA hazardous waste. The TCLP result was 0.83 milligrams per liter (mg/L) so less than the federal RCRA hazardous waste criteria of 5 mg/L.

STLC lead was detected at or above the method reporting limits in 33 of the 34 samples analyzed at concentrations ranging from 0.13 mg/L to 52.1 mg/L. A total of 19 soil samples exceeded the State of California hazardous waste criteria of 5 mg/L. TCLP lead was detected at or above the method reporting limits in 22 of the 36 samples analyzed at concentrations ranging from 0.13 milligrams per liter (mg/L) to 14.5 mg/L. A total of one soil sample (TR-21-5) exceeded the Federal hazardous waste criteria of 5 mg/L.

The remaining metal concentrations were within normal<sup>1</sup> background ranges found in the western United States with the exception of zinc in sample TR-2-1.5 which was detected at a concentration of 5,600 mg/kg.

## **Groundwater Results**

No oil and grease, TRPH, or SVOCs were detected above method reporting limits in any of the four samples. TSS was detected in all the samples with concentrations ranging from 110 mg/L to 160,000 mg/L. COD was detected in TR-19-GW, TR-20-GW, and TR-24-GW with concentrations of 24 mg/L, 20

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<sup>1</sup> "U.S.G.S. Professional Paper 1270, Element Concentrations in Soils and Other Surficial Materials of the Conterminous United States," 1984.

mg/L, and 64 mg/L, respectively. Phenolics were detected in TR-24-GW at a concentration of 0.074 mg/L. TR-19-GW, TR-20-GW, and TR-24-GW were tested for pH with concentrations of 7.41 S.U., 7.07 S.U., and 7.45 S.U., respectively.

Trichloroethylene was detected in TR-8-GW at a concentration of 1.58 mg/L. 1,2,4-trimethylbenzene, 1,3,5-trimethylbenzene, benzene, ethylbenzene, isopropyl benzene, n-propylbenzene, styrene, toluene, and total xylenes were detected in TR-19-GW with concentrations of 0.0223 mg/L, 0.00568 mg/L, 0.0251 mg/L, 0.011 mg/L, 0.00561 mg/L, 0.00138 mg/L, 0.00143 mg/L, 0.0171 mg/L, and 0.0591 mg/L, respectively. Methyl tert-butyl ether (MTBE) was detected in TR-20-GW at a concentration of 0.00078 mg/L. Naphthalene was detected in TR-19-GW, TR-20-GW, and TR-24-GW at concentrations of 0.417 mg/L, 0.00371 mg/L, and 0.0548 mg/L, respectively. No other VOCs were detected in any of the samples.

Antimony was detected in TR-20-GW at a concentration of 0.012 mg/L. Arsenic was detected in TR-24-GW at a concentration of 0.024 mg/L. Barium was detected in TR-8-GW, TR-19-GW, TR-20-GW, and TR-24-GW at concentrations of 0.066 mg/L, 0.052 mg/L, 0.085 mg/L, and 0.022 mg/L, respectively. Chromium was detected in TR-8-GW and TR-20-GW at concentrations of 0.032 mg/L and 0.008 mg/L, respectively. Cobalt was detected in TR-8-GW and TR-20-GW at concentrations of 0.011 mg/L and 0.006 mg/L, respectively. Molybdenum was detected in TR-8-GW, TR-20-GW, and TR-24-GW at concentrations of 0.01 mg/L, 0.024 mg/L, and 0.009 mg/L, respectively. Nickel was detected in TR-8-GW, TR-20-GW, and TR-24-GW at concentrations of 0.054 mg/L, 0.052 mg/L, and 0.013 mg/L, respectively. Vanadium was detected in TR-8-GW, TR-19-GW, TR-20-GW, and TR-24-GW at concentrations of 0.032 mg/L, 0.012 mg/L, 0.012 mg/L, and 0.021 mg/L, respectively. Zinc was detected in TR-8-GW, TR-20-GW, and TR-24-GW at concentrations of 1.1 mg/L, 0.013 mg/L, and 0.011 mg/L, respectively. No other metals were detected in any of the samples.

## **SUBSURFACE CONDITIONS**

The results of previous site investigations and other available subsurface information in the vicinity indicate the Site is generally underlain by approximately 5 to 16 feet of fill material, composed of loose to medium dense silty sand with varying amounts of brick, wood, tar, and glass fragments. The presence of fill material underlying the Site is likely associated with the 1906 earthquake and fire. A sand layer consisting of medium dense to very dense sand with variable amounts of silt approximately 15 to 18 feet thick underlies the fill material. Bay Mud is present beneath the sand layer.

Groundwater was encountered at the time of the investigation at depths ranging from 13 to 20 feet bgs. Groundwater levels may fluctuate depending on the season. The groundwater flow direction is likely to the northeast towards San Francisco Bay.

## **DISCUSSION**

Based on the analytical results from the Site subsurface investigation and previous analytical results, some of the fill material contains elevated total and soluble lead levels at concentrations exceeding Federal and State of California hazardous waste criteria. The remaining fill material will most likely be accepted at a regulated Class II and/or Class III landfill. Based on previous environmental investigations at the Site and vicinity, the sand underlying the fill would likely be disposed of as unrestricted waste.

The area of fill material containing soluble lead concentrations exceeding the Federal hazardous waste criteria are near boring TR-21 at a depth of 5 feet bgs. The areas of fill material containing total and soluble lead concentrations exceeding the State of California waste criteria are located near borings TR-1 at depths of 1.5 and 5 feet bgs, TR-2 at depths of 1.5, 3 and 5 feet bgs, TR-4 at depths of 3 and 5 feet bgs, TR-8 at depths of 1.5 and 3 feet bgs, TR-14 at a depth of 3 feet bgs, TR-15 at a depth of 3 feet bgs, TR-16 at a depth of 5 feet bgs and 10 bgs, TR-17 at depths of 1.5, 3 and 5 feet bgs, TR-19 at a depth of 7.5 feet bgs, TR-20 at a depth of 7.5 feet bgs, and TR-21 at a depth of 3 feet bgs. The remaining fill material will be disposed as Class II non-hazardous waste.

Groundwater is encountered at depths ranging from approximately 13 to 20 feet bgs across the Site. The proposed construction activities most likely will encounter groundwater in quantities that will require its removal from the subsurface. Prior to discharge into the sanitary sewer system, the dewatering contractor will obtain a batch groundwater discharge permit from the San Francisco Public Utilities Commission (SFPUC).

Because hazardous materials were detected at the Site, a SMP and a HASP will be required prior to construction. The Subcontractor HASP will outline proper soil handling procedures and H&S requirements to minimize worker and public exposure to hazardous materials during construction.

## **RECOMMENDATIONS FOR MITIGATIVE ACTIONS**

The results of previous environmental investigations at and near the Site indicate the fill material beneath the Site contains elevated concentrations of heavy metals and petroleum hydrocarbons. The presence of these compounds poses soil management and potential H&S issues to be addressed as part of the Site

development activities. The soil management objectives for the Site are to minimize exposure to construction workers at the Site, nearby residents and/or pedestrians, and future users of the Site to constituents in the soil.

### **Health and Safety Issues**

There may be a potential H&S risks associated with the heavy metals and petroleum hydrocarbons detected at the Site. There also may be a potential for this soil to affect construction workers at the Site, nearby residents and/or pedestrians, and future users of the Site. The routes of potential exposure to the petroleum hydrocarbons and metals could be through three pathways: 1) dermal (skin) contact with the soil, 2) inhalation of dusts, and 3) ingestion of the soil.

The most likely potential for human exposure to the petroleum hydrocarbons and metals in the soil will be during soil excavation operations. Because on-site materials contain concentrations of petroleum hydrocarbons and lead in excess of the Proposition 65 guidelines, there is a requirement that appropriate health and safety procedures, as well as warning requirements, be implemented during construction. The trade sub contractor will be responsible for establishing and maintaining proper H&S procedures to minimize worker and public exposure to Site contaminants during construction. Webcor/Obayashi Joint Venture will oversee this process and require the development and implementation of a comprehensive HASP, which should be prepared by a certified industrial hygienist that represents each subcontractor or its sub tier contractor.

The H&S training requirements, i.e. trained in accordance with Section 1910.120 of 29 Code of Federal Regulations (HazWoper training), specific personal hygiene, and monitoring equipment that will be used during construction to protect and verify the H&S of the construction workers and the general public from exposure to constituents in the soil. Air monitoring to evaluate the amount of airborne particles during excavation will be required by the tub trade contractors. All reports will be kept in a central location managed by Webcor/Obayashi Joint Venture.

A representative of Webcor/Obayashi Joint Venture and the Site health and safety officer (HASO) representing the trade subcontractor will be on site at all times during excavation activities to ensure that all health and safety measures are maintained. The Webcor/Obayashi Joint Venture representative or HASO will have authority to direct and stop (if necessary) all construction activities in order to ensure compliance with the HASP.

The purpose of the HASP is to provide field personnel with an understanding of the potential chemical and physical hazards, protection of any off-site receptors, procedures for entering the project Site, H&S procedures, and emergency response to hazards should they occur. All project personnel shall read and adhere to the procedures established in this HASP. A copy of all plans will be kept on site during field activities and will be reviewed and updated as necessary.

The general public will be protected through the following measures maintained by trade subcontractors and monitored by Webcor/Obayashi Joint Venture:

- the Site will be fenced;
- exposed soil at the construction Site will be watered as necessary to prevent visible dust from migrating off-site;
- soil stockpiles will be covered;
- water will be misted or sprayed during the loading of soil onto trucks for off haul;
- trucks transporting contaminated soil will be covered with a tarpaulin or other cover;
- the wheels of the trucks exiting the Site will be cleaned prior to entering public streets;
- public streets will be swept daily if soil is visible; and
- Excavation and loading activities will be suspended if winds exceed 20 miles per hour.

### **Soil Management**

The proposed construction activities will disturb soil during the excavation activities including: soil handling during archeological investigations, shoring wall installation, construction of a buttress for the adjoining 301 Mission Street property, timber pile removal and disposal, utility relocation and the mass excavation for the new Transbay Transit Center. During all excavation activities, dust control measures will be implemented to reduce potential exposure. These measures shall include moisture-conditioning the soil using dust suppressants and covering the exposed soil and stockpiles with weighed down plastic sheeting to prevent exposure of the soil.

Since all the contaminated fill material will be excavated and disposed of off-site, there will be no risk of direct contact with the underlying fill material by future Site users.

The Site's HASP (prepared by the trade sub contractor) will contain additional dust monitoring, action levels, dust control measures, and work stoppage provisions that will be followed during construction activities.

### **Soil Segregation and Disposal**

Before any excavation activities begin at the Site, a TJPA representative shall be provided documentation from the excavation contractor that the accepting landfill facility for the soil from Transbay Terminal project has been provided with and has reviewed all analytical data collected from the Site. TJPA shall approve all off-site disposal facilities and soil transportation contractors, including, without limitation, available insurable coverage, and prior to the shipment of any soil or other waste materials. The TJPA representative will provide testing and schedule the intervals that testing shall occur.

The results of previous soil analytical testing indicate that some of the soil located at the Site will be disposed off-site at a Class I landfill, however additional chemical testing of the soil may be required by the landfill prior to disposal. The excavation contractor shall be responsible for tracking the disposition of soil removed from the Site. Any excavated soil characterized as a hazardous waste shall be tracked using the Uniform Hazardous Waste Manifest System (USEPA Form 8700-22), as applicable. Soil not characterized as a hazardous waste shall be tracked using non-hazardous bills of lading. All documentation will be provided to TJPA during the excavation activities.

If soil stockpiling of suspected contaminated soil is to be performed, the excavation contractor shall establish appropriate soil stockpile locations on the Site to properly segregate, cover, control dust, profile, and manage the excavated soil. Stockpiled soils are to be placed on top of one layer of 10-mil polyethylene sheeting (or equivalent), such as Visqueen. When stockpiled soil is not actively being handled, top sheeting will be adequately secured so that all surface areas are covered.

### **Soil Disposition**

The Trade Sub contractor will establish appropriate off-site soil disposal locations and direct truck loading scheduling and/or soil stockpile locations on the Site to properly segregate, cover, moisture control, and profile the excavated soil. Soil profiling criteria will ultimately depend on the acceptance criteria of the landfills receiving the soil. These procedures will be established by the excavation contractor and coordinated with the proposed landfills prior to initiating soil excavation. It is not anticipated that soil will be reused at the Site for construction-related activities.

The Webcor Obayashi JV will, on behalf of TJPA, will be responsible for tracking final soil dispositions and turn that information to the TJPA representative. Any excavated soil considered hazardous waste will be tracked using the Uniform Hazardous Waste Manifest System (USEPA Form 8700-22), as applicable. Soil not considered hazardous waste will be tracked using non-hazardous bills of lading. These two systems will be used to comply with appropriate state and local requirements.

The contractor will arrange for transportation of all wastes off-site. Hazardous and non-hazardous waste will be transported to the appropriate disposal facility using a permitted, licensed, and insured transportation company. Transporters of hazardous waste must meet the requirements of 40 CFR 263 and 22 CCR 66263. All trucks transporting bulk hazardous waste will be properly lined and covered with compatible materials. Trucks will be decontaminated prior to any use other than hauling contaminated materials unless the contaminated material was already double-contained. The contractor will be responsible for preparing and submitting traffic control plans for trucks entering and leaving the Site. A decontamination pad location plan and decontamination procedures will be prepared. A route plan will also be prepared showing the expected route each truck will use to reach each landfill.

For soil that is to be exported off-site that is characterized as a hazardous waste, an appropriate USEPA Generator Identification Number will be recorded on the hazardous waste manifests used to document transport of hazardous waste off-site. The hazardous waste transporter, disposal facility, and U.S. Department of Transportation (DOT) waste description required for each manifest will be determined on a case-by-case basis. A description of the number of containers being shipped, the type of container, and the total quantity of waste being shipped will also be included on each manifest.

Webcor/Obayashi Joint Venture representative will be responsible for overseeing the sub trade provides accurate completion of the hazardous waste manifests and nonhazardous bills of lading. Records of all wastes shipped off-site will be maintained by TJPA and will be made available for inspection on request. The final destination of wastes transported off-site will be documented in the Site Closure Report that will be prepared by others.

### **Soil Sampling**

If needed, chemical testing of the stockpiled soil will be performed to profile the soil for disposal. Soil profiling criteria depends on the proposed landfill location or off-site receiving facility. These procedures shall be established by the excavation contractor and coordinated with the proposed landfills prior to initiating soil excavation. Typical soil profiling requirements are one four-point composite sample per 500 to 750 cubic yards to be disposed.

If soil samples are required for analysis, the samples shall be collected by the TJPA representative and tracked.

### **Timber Pile Removal and Disposal**

Part of the foundation system for the Transbay Terminal building includes timber piles beneath the basement slab. During the excavation activities these timber piles will be removed and disposed of. The timber piles will be extracted from the subsurface and as much as possible removal of all the soil which is attached to the timber pile will need to be performed. The extracted timber piles will be segregated, tested by the TJPA representative and transported. If disposed of as a Treated Wood at a Class II non-hazardous waste with copies of the Bill of Ladings will be submitted to TJPA representative.

### **Underground Storage Tank Removal and Disposal**

If a underground storage tank (UST) and/or and associated product lines are found, arrange for a licensed tank removal contractor to properly remove and dispose of the UST. Proper permits and notifications should be in place prior to removing the UST. If soil staining is observed, place the affected soil into a stockpile onto plastic sheets and cover with plastic sheets. The Environmental Consultant will complete soil sampling and analysis tasks for UST closure in accordance with San Francisco Fire Department (SFFD) and SFDPH.

### **Coal Gasification Residual Material**

The former San Francisco Gas Light Company was located on the south central and south eastern edge of the Site. Coal tar waste is believed to have been discharged into the surrounding tidelands which include the eastern portion of the Site. Excavation in this area of the Site will most likely encounter residual coal tar waste. Some of the coal gasification residual material encountered may be former piping, coal tar, phenols, heavy metals, and polynuclear aromatic hydrocarbons. If any coal gasification residual material is encountered during the excavation, the material will be stockpiled onto plastic sheeting and covered with plastic sheeting. The TJPA representative will collect soil samples and analyzed the material to determine proper disposal of the material.

### **Groundwater Management**

Groundwater is encountered at depths ranging from approximately 13 to 20 feet bgs across the Site. The proposed construction activities most likely will encounter groundwater in quantities that will require its removal from the subsurface. Prior to discharge into the sanitary sewer system, the dewatering Trade Subcontractors will obtain a batch groundwater discharge permit from the San Francisco Public Utilities



Commission (SFPUC). Based on analytical results of the groundwater samples analyzed during previous Site investigations, approval of the groundwater discharge from the dewatering system would be granted by SFPUC.

### **Dust Control**

Prior to initiating construction activities, a dust control plan (prepared by Trade Subcontractor and specific to this project) will be implemented to reduce potential exposure during excavation and loading operations. This document will contain measures to protect construction workers and the public including: dust monitoring, action levels, dust control measures, and work stoppage provisions that will be followed during construction activities.

Dust control will be accomplished through implementation of engineering controls, including light water spraying or misting of stockpiled soil, truck loading areas and work areas. Misting or spraying will be performed to sufficiently reduce fugitive dust emissions, but limited to prevent water runoff. Efforts will also be made to minimize the soil drop height from an excavator's bucket onto soil piles or into transport trucks. The site-specific dust control plan will as needed, include some or all of the following procedures: site fencing; wetting soil; analysis of wind direction; dust monitors at the work zone and at the Site perimeter and appropriate record keeping, visible inspection; establishing a hotline for community response; limiting excavation area; soil storage regulations (e.g. covering stockpiles); windbreaks; paving; truck loading requirements (e.g. covering vehicles or excavator bucket drop heights); Site vehicle speed limits; wheel washing; street sweeping; termination of excavation if winds exceed 20 mph; and/or addition of soil stabilizers; or other responses as needed.

### **Contingency Procedures**

Hazardous materials including; sumps and/or vaults, asbestos piping, former monitoring wells, and soil with petroleum hydrocarbon odors and/or stains may be encountered during excavation activities. If unanticipated hazardous materials are encountered, the following procedures will be maintained by trade subcontractors and monitored by Webcor/Obayashi Joint Venture:

- stop work in the area where the suspect material was encountered and cover it with plastic sheets;
- notify the Webcor/Obayashi Joint Venture representative, the TJPA Environmental Consultant for Site a inspection and appropriate action in the suspect area; and
- review the existing H&S plan and make revisions, if necessary; and

- Have appropriately trained personnel on Site to work with the affected materials, once directed by Webcor/Obayashi Joint Venture.

If a sump and/or vaults are encountered during excavation activities, contact the TJPA Environmental Consultant for inspection and appropriate action. If no liquid, obvious staining or odors are observed, sump and/or vaults will likely be destroyed and disposed of. If liquid is present within the sump and/or vault and/or obvious staining and odors are observed, the TJPA, Environmental Consultant will collect samples for analyses to determine how to properly disposal of the material.

If stained soil or odors are observed, plastic sheeting will be placed over the affected area and the TJPA Environmental Consultant will be contacted for inspection and appropriate action. If the material is to be excavated, the material will be stockpiled onto plastic sheeting and covered with plastic sheeting. Soil samples will be collected and analyzed to determine proper disposal of the material.

## REFERENCES

*Site Mitigation Plan Transbay Transit Center:* Treadwell & Rollo, Inc. dated March 2010.



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0001	<b>SCS - Transfer Girder Clarification</b>  From: Webcor Construction LP                      Claude Titcher  <b>REQUEST:</b>  Please clarify the designer's intent:  Sheet SI-2303 indicates that the south end of Transfer Girder TR9 extends beyond the B87 and B88 beams southern edge and partially into the intersecting MFB1 beam which is angular to the B87 beam. Section 8/S 1-3701 indicates that there are welded rebar couplers at the top flange of the TR9 girder to match the B78 beam reinforcing, but the B78 beam ends at the B87/B88 intersection prior to the southern end of the TR9 girder.	Void	01	05/15/2014	05/25/2014	
	<b>ANSWER:</b>  Please clarify the designer's intent:  Sheet SI-2303 indicates that the south end of Transfer Girder TR9 extends beyond the B87 and B88 beams southern edge and partially into the intersecting MFB1 beam which is angular to the B87 beam. Section 8/S 1-3701 indicates that there are welded rebar couplers at the top flange of the TR9 girder to match the B78 beam reinforcing, but the B78 beam ends at the B87/B88 intersection prior to the southern end of the TR9 girder.					
0002	<b>Wedge Barriers at Beale Street</b>  From: Webcor Construction LP                      Andrew Kitchen  <b>REQUEST:</b>  Reference: 28 16 44/APA 1.2.E  Shows only two wedge barriers at Beale Street (Bus Plaza). The drawings show four wedge barriers at Beale Street (Bus Plaza) at GL 33. Revise and coordinate documents accordingly.	Void	01	06/09/2014	06/19/2014	
	<b>ANSWER:</b>  Reference: 28 16 44/APA 1.2.E  Shows only two wedge barriers at Beale Street (Bus Plaza). The drawings show four wedge barriers at Beale Street (Bus Plaza) at GL 33. Revise and coordinate documents accordingly.					
0003	<b>Coating for Metal Surfaces</b>  From: Webcor Construction LP                      Andrew Kitchen  <b>REQUEST:</b>  Per 05 15 21 Steel Castings (see attached), steel castings (most notably the cast nodes) are to be furnished as bare metal. This was confirmed in the construction issues meeting held on 04/17/14 (see attached), with the indication that coating of the bare metal is to be included in TG16.5 Painting. 09 97 16 High Performance Coatings & Superstructure Package (see attached) identifies a coating system for exterior exposed factory-primed metal surfaces, and a coating system for galvanized steel, but not a coating system for exterior exposed unprimed/non-galvanized metal surfaces. Please provide the coating system for all unprimed/non-galvanized metal surfaces.	Void	01	06/09/2014	06/19/2014	
	<b>ANSWER:</b>  Per 05 15 21 Steel Castings (see attached), steel castings (most notably the cast nodes) are to be furnished as bare metal. This was confirmed in the construction issues meeting held on 04/17/14 (see attached), with the indication that coating of the bare metal is to be included in TG16.5 Painting. 09 97 16 High Performance Coatings & Superstructure Package (see attached) identifies a coating system for exterior exposed factory-primed metal surfaces, and a coating system for galvanized steel, but not a coating system for exterior exposed unprimed/non-galvanized metal surfaces. Please provide the coating system for all unprimed/non-galvanized metal surfaces.					



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0004	Watertight Condition 07 14 13	Void	0P	06/11/2014	06/21/2014	
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference: 07 14 13, 1.8.B		Reference: 07 14 13, 1.8.B				
See P1-0116. Same requirement for completely watertight building during new construction -waterproofing installation is not feasible.		See P1-0116. Same requirement for completely watertight building during new construction - waterproofing installation is not feasible.				
0005	Seismic Joint Coordination Between A Drawings	Void	0P	06/11/2014	06/21/2014	
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference: A1-8880, A1-2302, A1-7001		Reference: A1-8880, A1-2302, A1-7001				
Per A1-8880, WJC8 and RJC1 are located between Stair 201 and the adjacent existing building. Per A1-2302 and A1-7001, there does not appear to be a seismic joint at this location. Please revise so that drawings match each other.		Per A1-8880, WJC8 and RJC1 are located between Stair 201 and the adjacent existing building. Per A1-2302 and A1-7001, there does not appear to be a seismic joint at this location. Please revise so that drawings match each other.				
0006	BGP - Bracing removal sequence on the East end of Zone 4	Void	01	07/25/2014	08/04/2014	
From: Webcor Construction LP                      Claude Titche						
REQUEST:		ANSWER:				
Bracing removal sequence on the East end of Zone 4 WOJV is proposing the following sequence for the bracing removal for the east side of Zone 4. See sketch SK1, attached.		Bracing removal sequence on the East end of Zone 4 WOJV is proposing the following sequence for the bracing removal for the east side of Zone 4. See sketch SK1, attached.				
Sequence 1. Remove level D struts and walers from within the green clouded area up to GL- 32.2 once the mat slab beneath has reached adequate strength. 2. Remove level D struts within the Blue clouded area STD-65 to 74, 82 & 83 and all corresponding walers once the mat slab beneath has reached adequate strength, the sequence for de-stressing will be the diagonals struts should be all de-stressing prior to the 4 remaining cross lot struts (STD-65,66,67 & 68).		Sequence 1. Remove level D struts and walers from within the green clouded area up to GL- 32.2 once the mat slab beneath has reached adequate strength. 2. Remove level D struts within the Blue clouded area STD-65 to 74, 82 & 83 and all corresponding walers once the mat slab beneath has reached adequate strength, the sequence for de-stressing will be the diagonals struts should be all de-stressing prior to the 4 remaining cross lot struts (STD-65,66,67 & 68).				
For the remaining levels A, B and C WOJV is proposing to follow a similar removal sequence as Level D		For the remaining levels A, B and C WOJV is proposing to follow a similar removal sequence as Level D				



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<div>Please confirm if this sequence would be acceptable.</div> <div>Please confirm if this sequence would be acceptable.</div>						
0007	Void	Void	0P	07/25/2014	08/04/2014	
From: Webcor Construction LP      Zachary Moore						
REQUEST: Void		ANSWER: Void				
0007.1	BRP - Contaminated Materials Location Meeting 6/20/14 - Action Items	Closed	01	06/23/2014	07/03/2014	06/25/2014
From: Webcor Construction LP      Claude Titcher						
REQUEST: <p>Per the Contaminated Materials Meeting held at 1:30PM in BRAVO conference room at the WOJV office on 6/20/2014 between SCCI, WOJV, Turner, TJPA, PMPC, and Treadwell &amp; Rollo, please address the following post-meeting action items (meeting minutes to be distributed):</p> <p>1) Provide site map characterizing locations of Class I and Class II contaminated materials on site and the depths of contamination at these locations (Map to be provided by Treadwell &amp; Rollo to TJP A for distribution to SCCI). SCCI will use this map to plan safe excavation and handling of excavated materials.</p> <p>2) Confirm that Federal RCRA classified waste is not expected to be encountered on the site at this time per soil boring logs and testing. Further stockpile testing will be performed by Treadwell &amp; Rollo at the direction of the TJPA as necessary to determine final soil classification for disposal.</p>		ANSWER: <p>Per the Contaminated Materials Meeting held at 1:30PM in BRAVO conference room at the WOJV office on 6/20/2014 between SCCI, WOJV, Turner, TJPA, PMPC, and Treadwell &amp; Rollo, please address the following post-meeting action items (meeting minutes to be distributed):</p> <p>1) Provide site map characterizing locations of Class I and Class II contaminated materials on site and the depths of contamination at these locations (Map to be provided by Treadwell &amp; Rollo to TJP A for distribution to SCCI). SCCI will use this map to plan safe excavation and handling of excavated materials.</p> <p>2) Confirm that Federal RCRA classified waste is not expected to be encountered on the site at this time per soil boring logs and testing. Further stockpile testing will be performed by Treadwell &amp; Rollo at the direction of the TJPA as necessary to determine final soil classification for disposal.</p>				



**ANSWER:**

RFI T-1591 response noted two requirements to waive the testing requirements.

1. Provide specifications for the product that is intended to be used in the project that meets the design requirements outlined in the contract documents.  
See attached preliminary Pendulum Bearing product information and drawing.
2. Provide test data for a bearing that is comparable in size and performance to those intended to be used in this project tested with conditions that are comparable to the design requirements.  
Please see attached additional performance test results from other bearings of comparable size and performance. Also reference T-1591 for previously submitted test results.

Please confirm that the mentioned test reports could serve as evidence of the proper functioning of mageba pendulum bearings, as well as the fulfillment for the testing requirements in the specifications.



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0009	Sloping to Drain on Bathroom Floor Plans	Void	0P	09/18/2014	09/28/2014	
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
Reference: 2/A1-9001, 2/A1-9002, 2/A1-9004, 2/A1-9012, 2/A1-9015, 2/A1-9017, 2/A1-9021, 2/A1-9023, 2/A1-9024, 3/A1-9033, 3/A1-9034, 5/A1-9041, 10/A1-9042 (IFC Main Set 03/31/2014)						Reference: 2/A1-9001, 2/A1-9002, 2/A1-9004, 2/A1-9012, 2/A1-9015, 2/A1-9017, 2/A1-9021, 2/A1-9023, 2/A1-9024, 3/A1-9033, 3/A1-9034, 5/A1-9041, 10/A1-9042 (IFC Main Set 03/31/2014)
Bathroom floor plans show sloping adjacent to the drain, but flat at the remainder of the field tile (see 2/A1-9001, 2/A1-9002, 2/A1-9004, 2/A1-9012, 2/A1-9015, 2/A1-9017, 2/A1-9021, 2/A1-9023, 2/A1-9024, 3/A1-9033, 3/A1-9034). 5/A1-9041 and 10/A1-9042 call out for a setting bed on topping slab at the restroom floors. Please confirm only the areas around the floor drains, as shown on the floor plans, are to be sloped, and that a setting bed is not required where tile is not sloped (i.e. the tile is thin set on the topping slab).						Bathroom floor plans show sloping adjacent to the drain, but flat at the remainder of the field tile (see 2/A1-9001, 2/A1-9002, 2/A1-9004, 2/A1-9012, 2/A1-9015, 2/A1-9017, 2/A1-9021, 2/A1-9023, 2/A1-9024, 3/A1-9033, 3/A1-9034). 5/A1-9041 and 10/A1-9042 call out for a setting bed on topping slab at the restroom floors. Please confirm only the areas around the floor drains, as shown on the floor plans, are to be sloped, and that a setting bed is not required where tile is not sloped (i.e. the tile is thin set on the topping slab).
0010	BRP - Number of Barrette Pile Tremies	Void	01	09/18/2014	09/28/2014	
From: Webcor Construction LP                      Claude Titché						
REQUEST:						ANSWER:
Contract drawing S-3191 indicates two openings (for two tremies) in each Barrette's rebar cage. This drawing is in conflict with Contract Specification 31 63 32 3.4 D 3 that indicates that three tremies are needed for each Barrette (one tremie for each 7 feet of wall length). Please note that the Response to RFI-0050 allowed for the length of the barrette piles to be increased from 20'-0" to 21 '-0".						Contract drawing S-3191 indicates two openings (for two tremies) in each Barrette's rebar cage. This drawing is in conflict with Contract Specification 31 63 32 3.4 D 3 that indicates that three tremies are needed for each Barrette (one tremie for each 7 feet of wall length). Please note that the Response to RFI-0050 allowed for the length of the barrette piles to be increased from 20'-0" to 21 '-0".
The European Standard EN 1538 indicates two tremies (properly spaced so that the concrete does not have to travel more than 8 feet) is sufficient for each Barrette. Additionally, in reference to RFI BRP-0055 and, in an effort to reduce rebar congestion, 2 tremies are preferred.						The European Standard EN 1538 indicates two tremies (properly spaced so that the concrete does not have to travel more than 8 feet) is sufficient for each Barrette. Additionally, in reference to RFI BRP-0055 and, in an effort to reduce rebar congestion, 2 tremies are preferred.
Please confirm that two tremie pipes per Barrette are acceptable.						Please confirm that two tremie pipes per Barrette are acceptable.
0011	Unit Pricing	Void	0P	09/23/2014	10/03/2014	





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	<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference Specification Section 32 91 30 3.6.A.1 IFC Main Set (03/31/2014)</p> <p>This section states to "Provide allowance for 15 eight-hour days of adjustment grading work with a work crew of six using rakes and shovels to smooth and shape the planting area surfaces. Provide unit cost per day." If a unit price is desired, it needs to be part of the Unit Price Specification. Unit Prices should not be in the technical specifications. Please remove and place in the Unit Price Specification.</p>					
						<p><b>ANSWER:</b></p> <p>Reference Specification Section 32 91 30 3.6.A.1 IFC Main Set (03/31/2014)</p> <p>This section states to "Provide allowance for 15 eight-hour days of adjustment grading work with a work crew of six using rakes and shovels to smooth and shape the planting area surfaces. Provide unit cost per day." If a unit price is desired, it needs to be part of the Unit Price Specification. Unit Prices should not be in the technical specifications. Please remove and place in the Unit Price Specification.</p>
<b>0012</b>	<b>SCS - Electrical Conduit in Foundation Wall</b>	<b>Void</b>	<b>01</b>	<b>09/29/2014</b>	<b>10/09/2014</b>	
	<p><b>From:</b> Webcor Construction LP                      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>Reference: ES-2107(Dated: 04/24/14) , A1-3010(Dated: 07/18/14)</p> <p>Detail A &amp; B on ES-2107 provides the section view for PG&amp;E 12 KV Main Service coming into North Electrical Room (B1289) but A1-3010 doesn't refer to an Elevation view identifying these sleeves in the walls. Please confirm the section views and the elevation of the bottom of the conduits in Detail A&amp;B/ES-2107 are correct.</p> <p>Reference : A1-9244(Dated: 07/18/14), ES-2107 to ES-2111(Dated: 04/24/14)</p> <p>Detail D/A1-9244 specifies center to center vertical spacing between 2" and 6" electrical conduits as 1'-5"(See attached). Please confirm this spacing is applicable to all the section details in ES-2107 to ES-2111 entering the building, if not please provide the center to center vertical spacing between 2' and 6" conduits called out in ES-2107 to ES-2111.</p> <p>Reference: ES-2107 to ES-2111(Dated: 04/24/14)</p> <p>The section view on ES-2107 to ES-2111 calls out the bottom of the conduit elevations for 2" and 6" conduits. Please confirm these elevations are accurate for all the</p>					<p><b>ANSWER:</b></p> <p>Reference: ES-2107(Dated: 04/24/14) , A1-3010(Dated: 07/18/14)</p> <p>Detail A &amp; B on ES-2107 provides the section view for PG&amp;E 12 KV Main Service coming into North Electrical Room (B1289) but A1-3010 doesn't refer to an Elevation view identifying these sleeves in the walls. Please confirm the section views and the elevation of the bottom of the conduits in Detail A&amp;B/ES-2107 are correct.</p> <p>Reference : A1-9244(Dated: 07/18/14), ES-2107 to ES-2111(Dated: 04/24/14)</p> <p>Detail D/A1-9244 specifies center to center vertical spacing between 2" and 6" electrical conduits as 1'-5"(See attached). Please confirm this spacing is applicable to all the section details in ES-2107 to ES-2111 entering the building, if not please provide the center to center vertical spacing between 2' and 6" conduits called out in ES-2107 to ES-2111.</p> <p>Reference: ES-2107 to ES-2111(Dated: 04/24/14)</p> <p>The section view on ES-2107 to ES-2111 calls out the bottom of the conduit elevations for 2" and 6"</p>



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	ES drawings.					conduits. Please confirm these elevations are accurate for all the ES drawings.
<b>0013</b>	<b>TTC - Civil Station to Architectural Gridline Correlation</b>	<b>Void</b>	<b>01</b>	<b>10/08/2014</b>	<b>10/18/2014</b>	
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b> The Architectural and Civil drawings contain no correlation between Stations and gridlines. Please provide a correlation so that points can be referenced between different sets of drawings.						<b>ANSWER:</b> The Architectural and Civil drawings contain no correlation between Stations and gridlines. Please provide a correlation so that points can be referenced between different sets of drawings.
<b>0014</b>	<b>BRP - Fremont Off Ramp Limits of Demolition</b>	<b>Void</b>	<b>01</b>	<b>11/05/2014</b>	<b>11/15/2014</b>	
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b> Contract Drawings D-1003 and D-1007 show the limits of removal of the existing Fremont Street Bus Off Ramp. SCCI would like to confirm the extent of the demolition off the off ramps concrete barrier at the limit of the demolition. SCCI believes that the concrete barrier should be removed to the limits shown on the attached picture as existing cracks in the concrete could lead to spalling if this section is not completely removed.  Please confirm.						<b>ANSWER:</b> Contract Drawings D-1003 and D-1007 show the limits of removal of the existing Fremont Street Bus Off Ramp. SCCI would like to confirm the extent of the demolition off the off ramps concrete barrier at the limit of the demolition. SCCI believes that the concrete barrier should be removed to the limits shown on the attached picture as existing cracks in the concrete could lead to spalling if this section is not completely removed.  Please confirm.







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	explanation of how these bid item quantities were determined and a map indicating where contaminated materials should be encountered on the project.					an explanation of how these bid item quantities were determined and a map indicating where contaminated materials should be encountered on the project.
<b>B-0003</b>	<b>BRP - Project Control Points</b>	<b>Closed</b>	<b>01</b>	<b>05/29/2014</b>	<b>06/08/2014</b>	<b>06/06/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> Notes 53, 54 and 56 on Contract Drawing C-0005 reference control points utilized by Martin Ron Associates for the job topographic survey Shown on Contract Drawing Sheets 386-397. Only one Survey Control Point, DPW Survey Control Point No. 54, is shown on these drawings. SCCI requests that at least three additional job control point coordinates be supplied along with a description of those points.						<b>ANSWER:</b> Notes 53, 54 and 56 on Contract Drawing C-0005 reference control points utilized by Martin Ron Associates for the job topographic survey Shown on Contract Drawing Sheets 386-397. Only one Survey Control Point, DPW Survey Control Point No. 54, is shown on these drawings. SCCI requests that at least three additional job control point coordinates be supplied along with a description of those points.
<b>B-0003.1</b>	<b>BRP - Project Control Points</b>	<b>Closed</b>	<b>01</b>	<b>06/13/2014</b>	<b>06/23/2014</b>	<b>06/17/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> SCCI has received and reviewed the response to RFI #0003 which directed attention to the reference survey drawings. SCCI has reviewed these drawings and found control points far from the Bus Ramps project. SCCI would like to confirm that no additional control points have been established within a reasonable distance of the work area. If additional survey control points are available, please provide  Please see attached map for current control points given by the contract and reference survey drawings.						<b>ANSWER:</b> SCCI has received and reviewed the response to RFI #0003 which directed attention to the reference survey drawings. SCCI has reviewed these drawings and found control points far from the Bus Ramps project. SCCI would like to confirm that no additional control points have been established within a reasonable distance of the work area. If additional survey control points are available, please provide  Please see attached map for current control points given by the contract and reference survey drawings.



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
<b>B-0004</b>	<b>BRP - Bent 8 CIDH Pile Construction Joints</b>	<b>Closed</b>	<b>01</b>	<b>06/05/2014</b>	<b>06/15/2014</b>	<b>06/11/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>  As shown in the CIDH elevation detail on Contract Drawing S1-3190, there is an optional construction joint at the elevation of the bottom of column rebar cage, 18 feet below the cut off line. However, the Bent8 CIDH Elevation detail does not allow for an optional construction joint at the elevation of the bottom of column rebar cage. In order to facilitate the proper installation of the column rebar cages in CIDH Piles B8-1 and B8-2, it is necessary to have an optional construction joint at this location (see attachment for illustration). Therefore, SCCI proposes to revise the drawings to include this joint. Is this acceptable?						<b>ANSWER:</b>  As shown in the CIDH elevation detail on Contract Drawing S1-3190, there is an optional construction joint at the elevation of the bottom of column rebar cage, 18 feet below the cut off line. However, the Bent8 CIDH Elevation detail does not allow for an optional construction joint at the elevation of the bottom of column rebar cage. In order to facilitate the proper installation of the column rebar cages in CIDH Piles B8-1 and B8-2, it is necessary to have an optional construction joint at this location (see attachment for illustration). Therefore, SCCI proposes to revise the drawings to include this joint. Is this acceptable?
<b>B-0005</b>	<b>BRP - Fremont Off Ramp Bent Weakened Plane Joints</b>	<b>Closed</b>	<b>01</b>	<b>06/06/2014</b>	<b>06/16/2014</b>	<b>06/09/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>  The elevation view of the bents on sheet S-3100 show 5 weakened plane joints at the base of the columns. These can be seen in further detail from sheet S-3102, details H and 1. SCCI proposes that these weakened joints be removed so that bents match the condition of the existing Fremont street offramp bents.						<b>ANSWER:</b>  The elevation view of the bents on sheet S-3100 show 5 weakened plane joints at the base of the columns. These can be seen in further detail from sheet S-3102, details H and 1. SCCI proposes that these weakened joints be removed so that bents match the condition of the existing Fremont street offramp bents.
<b>B-0006</b>	<b>BRP - Utility Demolition Plan - Tehama Street</b>	<b>Closed</b>	<b>01</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>  Sheet U-1005 shows utility demolition work in the area surrounding Clementina Street, and Sheet U- 1006 shows utility demolition work in the area surrounding Howard Street. There is no sheet showing any utility demolition work (or lack thereof) in the area surrounding Tehama Street (between Clementina and Howard). Please provide plan sheet showing utility demolition work surrounding Tehama Street.						<b>ANSWER:</b>  Sheet U-1005 shows utility demolition work in the area surrounding Clementina Street, and Sheet U- 1006 shows utility demolition work in the area surrounding Howard Street. There is no sheet showing any utility demolition work (or lack thereof) in the area surrounding Tehama Street (between Clementina and Howard). Please provide plan sheet showing utility demolition work surrounding Tehama Street.



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
<b>B-0006.1</b>	<b>BRP - Utility Demolition Plan - Tehama Street</b>	<b>Closed</b>	<b>01</b>	<b>06/25/2014</b>	<b>07/05/2014</b>	<b>07/18/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
In response to "RFI B-0006 BRP-Response-Utility Demolition Plan - Tehama Street", Shimmick requests a utility demolition/relocation plan for Tehama Street be provided (between Sheets U-1005 and U-1006). There is an existing CCSF Street Light in the path of the proposed new bus ramp, as well as an overhead utility line (Comcast fiber optic) and associated poles (PG&E) which will interfere with the proposed bus ramp construction. Please confirm CCSF streetlight and Comcast fiber optic line and poles on Tehama to be moved by others.			In response to "RFI B-0006 BRP-Response-Utility Demolition Plan - Tehama Street", Shimmick requests a utility demolition/relocation plan for Tehama Street be provided (between Sheets U-1005 and U-1006). There is an existing CCSF Street Light in the path of the proposed new bus ramp, as well as an overhead utility line (Comcast fiber optic) and associated poles (PG&E) which will interfere with the proposed bus ramp construction. Please confirm CCSF streetlight and Comcast fiber optic line and poles on Tehama to be moved by others.			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>B-0007</b>	<b>BRP - Hazardous and Contaminated Materials Location</b>	<b>Closed</b>	<b>01</b>	<b>06/12/2014</b>	<b>06/22/2014</b>	<b>06/13/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Follow-Up To RFI B-0002 ERP-RESPONSE- Hazardous and Contaminated Materials Location:			Follow-Up To RFI B-0002 ERP-RESPONSE- Hazardous and Contaminated Materials Location:			
RFI B-0002 ERP-RESPONSE- Hazardous and Contaminated Materials Location listed "Transbay Transit Center Program Limited Phase II Soil and Groundwater Investigation Report Transbay Terminal West Loop Bus Ramps and Future Transit Center Site East of Beale St." (ERM-West, December 2008) and "Transbay Program Site Management Plan Addendum Transbay Transit Center Bus Ramps" (Treadwell & Rollo, February 2013) as the basis of bid item estimates for bid items 40, 41, and 42.			RFI B-0002 ERP-RESPONSE- Hazardous and Contaminated Materials Location listed "Transbay Transit Center Program Limited Phase II Soil and Groundwater Investigation Report Transbay Terminal West Loop Bus Ramps and Future Transit Center Site East of Beale St." (ERM-West, December 2008) and "Transbay Program Site Management Plan Addendum Transbay Transit Center Bus Ramps" (Treadwell & Rollo, February 2013) as the basis of bid item estimates for bid items 40, 41, and 42.			
These documents do not provide for specific quantities and locations of Class I (Bid Item 40), Class II (Bid Item 41) and Federal RCRA (Bid Item 42) contaminated materials on site.			These documents do not provide for specific quantities and locations of Class I (Bid Item 40), Class II (Bid Item 41) and Federal RCRA (Bid Item 42) contaminated materials on site.			
Please provide a detailed map and/or plan indicating the locations of Class I, Class II, and Federal RCRA contaminated materials similar to the site plan sheets titled "Site Plan With Boring Locations and Map Extents" (Figures 2-7, Treadwell & Rollo) and provided in Spec 0 I 13 50/ AP A - Site Mitigation Plan of the project documents (provided as an attachment to this RFI).			Please provide a detailed map and/or plan indicating the locations of Class I, Class II, and Federal RCRA contaminated materials similar to the site plan sheets titled "Site Plan With Boring Locations and Map Extents" (Figures 2-7, Treadwell & Rollo) and provided in Spec 0 I 13 50/ AP A - Site Mitigation Plan of the project documents (provided as an attachment to this RFI).			
Please confirm that all contaminated soils identification, testing, and analytics shall be provided by the TJPA and/or the prime contractor.			Please confirm that all contaminated soils identification, testing, and analytics shall be provided by the TJPA and/or the prime contractor.			





<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
B-0007.2	BRP - Hazardous Materials - Class I/II Locations	Closed	01	07/03/2014	07/13/2014	07/07/2014
From: Webcor Construction LP                      Claude Titcher						
REQUEST:			ANSWER:			
In Figure 1 of "Site Plan of Preliminary Limits of State of California Class I Non-RCRA and Class II Non-Hazardous Fill Material", provided by Treadwell Rollo, the locations of the Class I and Class II materials are labeled incorrectly. The data provided in "Transbay Transit Center Program Limited Phase II Soil and Groundwater Investigation Report Transbay Terminal West Loop Bus Ramps and Future Transit Center Site East of Beale St." (ERM-West, December 2008), "Transbay Program Site Management Plan Addendum Transbay Transit Center Bus Ramps" (Treadwell & Rollo, February 2013) and per our meeting on 6/20/14, indicate that the Class I material was found in borings SB-04 through SB-09 as well as S-3 through S-5. These borings are identified as Class II material on the map provided. Borings SB-10 through SB-12 and S-1 /2 contained Class II material. It appears the colors in the legend are reversed. Please provide map with a revised legend.			In Figure 1 of "Site Plan of Preliminary Limits of State of California Class I Non-RCRA and Class II Non-Hazardous Fill Material", provided by Treadwell Rollo, the locations of the Class I and Class II materials are labeled incorrectly. The data provided in "Transbay Transit Center Program Limited Phase II Soil and Groundwater Investigation Report Transbay Terminal West Loop Bus Ramps and Future Transit Center Site East of Beale St." (ERM-West, December 2008), "Transbay Program Site Management Plan Addendum Transbay Transit Center Bus Ramps" (Treadwell & Rollo, February 2013) and per our meeting on 6/20/14, indicate that the Class I material was found in borings SB-04 through SB-09 as well as S-3 through S-5. These borings are identified as Class II material on the map provided. Borings SB-10 through SB-12 and S-1 /2 contained Class II material. It appears the colors in the legend are reversed. Please provide map with a revised legend.			



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<b>B-0007.3</b>	<b>BRP - Hazardous Materials Location Follow Up</b>	<b>Closed</b>	<b>01</b>	<b>07/22/2014</b>	<b>08/01/2014</b>	
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Per RFI-0007.2 Response, a map of locations of Class I and Class II contaminated soils located on the Bus Ramps site was provided by Treadwell &amp; Rollo for use by SCCI. During the meeting held 6/20/2014 at the Webcor Offices to discuss the hazardous materials on site and development of a hazardous materials location map, it was indicated that there would be no federal RCRA material encountered on the site and that Class I and Class II contamination depths would vary between 1' and 2' below grade.</p> <p>During waste profiling with our disposal site, SCCI has encountered the following discrepancies:</p> <p>Per the T &amp;R map, the area around sample SB-05 should be Class I material. Per the soil test analytics provided in the Bus Ramps Site Mitigation Plan and the ERM-West Phase II Soils Report, samples SB- 5-3 and SB-5-6 test over the Class I limit for Lead and Barium, respectively. Without the STLC (Soluble Threshold Limit Concentration) and TCLP (Toxicity Characteristic Leaching Procedure) test results to confirm otherwise, material with these TTLC (Total Threshold Limit Concentration) results would have to be classified and disposed of as a federal RCRA material which is in conflict with the direction provided by the T &amp;R map and the 6/20 meeting discussion.</p> <p>Per the T&amp;R map and meeting discussion, areas approximately north of Folsom Street would be contaminated with Class I material to a depth of 2' below grade, and areas approximately south of Folsom Street would be contaminated with Class II material to a depth of 1' below grade. A small portion of the area east of the Sterling substation would be contaminated with Class I material to a depth of 1' . Per the soil test analytics provided in the Bus Ramps Site Mitigation Plan and the ERM-West Phase II Soils Report, samples SB-5-3 and SB-5-6 are both potentially contaminated to a Class I or federal RCRA classification level at depths below 2', which is in conflict with the direction provided by the T &amp;R map and the 6/20 meeting discussion.</p> <p>The following is a list of possible soil samples provided which may be in conflict with the T &amp;R map and 6/20 meeting discussion direction and may require additional testing to confirm soil classification:</p>			<p>Per RFI-0007.2 Response, a map of locations of Class I and Class II contaminated soils located on the Bus Ramps site was provided by Treadwell &amp; Rollo for use by SCCI. During the meeting held 6/20/2014 at the Webcor Offices to discuss the hazardous materials on site and development of a hazardous materials location map, it was indicated that there would be no federal RCRA material encountered on the site and that Class I and Class II contamination depths would vary between 1' and 2' below grade.</p> <p>During waste profiling with our disposal site, SCCI has encountered the following discrepancies:</p> <p>Per the T &amp;R map, the area around sample SB-05 should be Class I material. Per the soil test analytics provided in the Bus Ramps Site Mitigation Plan and the ERM-West Phase II Soils Report, samples SB- 5-3 and SB-5-6 test over the Class I limit for Lead and Barium, respectively. Without the STLC (Soluble Threshold Limit Concentration) and TCLP (Toxicity Characteristic Leaching Procedure) test results to confirm otherwise, material with these TTLC (Total Threshold Limit Concentration) results would have to be classified and disposed of as a federal RCRA material which is in conflict with the direction provided by the T &amp;R map and the 6/20 meeting discussion.</p> <p>Per the T&amp;R map and meeting discussion, areas approximately north of Folsom Street would be contaminated with Class I material to a depth of 2' below grade, and areas approximately south of Folsom Street would be contaminated with Class II material to a depth of 1' below grade. A small portion of the area east of the Sterling substation would be contaminated with Class I material to a depth of 1' . Per the soil test analytics provided in the Bus Ramps Site Mitigation Plan and the ERM-West Phase II Soils Report, samples SB-5-3 and SB-5-6 are both potentially contaminated to a Class I or federal RCRA classification level at depths below 2', which is in conflict with the direction provided by the T &amp;R map and the 6/20 meeting discussion.</p> <p>The following is a list of possible soil samples provided which may be in conflict with the T &amp;R map</p>			



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>SB-4-3 Need STLC and TCLP for Lead SB-4-28 Need STLC for Chromium SB-5-3 Need STLC and TCLP For Lead SB-5-6 Need STLC and TCLP for Barium SB-7-3 Need STLC and TCLP for Lead SB-7-5 Need STLC for Chromium, Need STLC for Copper, Need STLC and TCLP for Lead SB-8-3 Need STLC and TCLP for Lead SB-8-21 Need STLC for Chromium SB-9-3 Need STLC for Chromium, Need STLC and TCLP for Lead SB-9-6 Need STLC for Chromium, Need STLC for Nickel SB-10-12.5 Need STLC for Chromium</p> <p>Please advise on discrepancies and additional test results and provide additional detail on location and depth of contamination on the site map.</p> <p>Also, all soil samples begin at 3' or greater depth below grade - please advise how contamination classification was determined for 1-2' depths below grade as the landfill will not accept the information as currently provided.</p>				<p>and 6/20 meeting discussion direction and may require additional testing to confirm soil classification:</p> <p>SB-4-3 Need STLC and TCLP for Lead SB-4-28 Need STLC for Chromium SB-5-3 Need STLC and TCLP For Lead SB-5-6 Need STLC and TCLP for Barium SB-7-3 Need STLC and TCLP for Lead SB-7-5 Need STLC for Chromium, Need STLC for Copper, Need STLC and TCLP for Lead SB-8-3 Need STLC and TCLP for Lead SB-8-21 Need STLC for Chromium SB-9-3 Need STLC for Chromium, Need STLC and TCLP for Lead SB-9-6 Need STLC for Chromium, Need STLC for Nickel SB-10-12.5 Need STLC for Chromium</p> <p>Please advise on discrepancies and additional test results and provide additional detail on location and depth of contamination on the site map.</p> <p>Also, all soil samples begin at 3' or greater depth below grade - please advise how contamination classification was determined for 1-2' depths below grade as the landfill will not accept the information as currently provided.</p>	
B-0008	BRP - Bus Ramp to Transit Center Elevation Confirmation	Closed	01	06/12/2014	06/22/2014	06/18/2014
From: Webcor Construction LP                      Claude Titcher						
REQUEST:						
Reference attached Contract Documents.						
Elevation discrepancy's exist between the Transbay Transit Center drawings and the Bus Ramp Bridge drawings, see below.						
TTC building drawing A1-2502: 57' - 11 1/4" (57.938') [HP of Bus Deck Level] A1-6102: 57' - 10" (57.833') [Bus Deck Level]						
Bus Ramp drawing						
ANSWER:						
Reference attached Contract Documents.						
Elevation discrepancy's exist between the Transbay Transit Center drawings and the Bus Ramp Bridge drawings, see below.						
TTC building drawing A1-2502: 57' - 11 1/4" (57.938') [HP of Bus Deck Level] A1-6102: 57' - 10" (57.833') [Bus Deck Level]						



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	<div>S-2063: 56' - 1 3/16" (56.10') ["BBTT" STA 29+04.81 FB] C-2202: 57' - 11 3/4 (57.98') [Calculated at STA 29+04.81 per vertical curve information]  Please confirm which elevation governs.</div>			<div>Bus Ramp drawing S-2063: 56' - 1 3/16" (56.10') ["BBTT" STA 29+04.81 FB] C-2202: 57' - 11 3/4 (57.98') [Calculated at STA 29+04.81 per vertical curve information]  Please confirm which elevation governs.</div>		
B-0009	BRP - Exist Bent 20 Demolition	Closed	01	06/18/2014	06/18/2014	06/25/2014
From: Webcor Construction LP		Claude Titcher				
REQUEST:		ANSWER:				
<p>As shown in Contract Drawing D-1005, the entire existing Bent 20 foundation is to be removed. However, USA North ticket #219688 located two ATT duct banks 3' away from, and running parallel to, the property line on the south sidewalk of Clementina St. This utility is not shown on the Contract Drawings. The utility is located directly on top of the Bent 20 foundation as shown on the attached sketch.</p> <p>Per SCCI's understanding, the reason to remove Bent 20 is to facilitate the installation of CIDH B4-3 and the drainage system shown on Contract Drawings U-1005 and C-4102. Shimmick proposes to remove the Bent 20 foundation only to the extent needed to enable the installation of CIDH B4-3 and the drainage system (the catch basin and sewer manhole will still be protected and supported in place). The extent of demolition should end roughly 4'-8" north of the CIDH pile to allow sufficient room for shoring yet not intrude on the utility. See attached drawing for proposed limits of demolition. Is this acceptable?</p>		<p>As shown in Contract Drawing D-1005, the entire existing Bent 20 foundation is to be removed. However, USA North ticket #219688 located two ATT duct banks 3' away from, and running parallel to, the property line on the south sidewalk of Clementina St. This utility is not shown on the Contract Drawings. The utility is located directly on top of the Bent 20 foundation as shown on the attached sketch.</p> <p>Per SCCI's understanding, the reason to remove Bent 20 is to facilitate the installation of CIDH B4-3 and the drainage system shown on Contract Drawings U-1005 and C-4102. Shimmick proposes to remove the Bent 20 foundation only to the extent needed to enable the installation of CIDH B4-3 and the drainage system (the catch basin and sewer manhole will still be protected and supported in place). The extent of demolition should end roughly 4'-8" north of the CIDH pile to allow sufficient room for shoring yet not intrude on the utility. See attached drawing for proposed limits of demolition. Is this acceptable?</p>				



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<b>B-0010</b>	<b>BRP - Discrepancy Between Contract Drawing and As-Built</b>	<b>Closed</b>	<b>01</b>	<b>06/20/2014</b>	<b>06/30/2014</b>	<b>06/25/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
SCCI has reviewed Contract Drawing D-1006 and compared it to the Tieback As-built Document SH- 2600 per the Tieback Final Package from Balfour Beatty. The tieback angles are all different, most varying by roughly 8 degrees. The angle change affects the locations of the tiebacks as well. The precise location and angle of the tiebacks is paramount to the installation of shoring for pylon 9. SCCI would like to confirm that the As-builts are correct for layout use.			SCCI has reviewed Contract Drawing D-1006 and compared it to the Tieback As-built Document SH- 2600 per the Tieback Final Package from Balfour Beatty. The tieback angles are all different, most varying by roughly 8 degrees. The angle change affects the locations of the tiebacks as well. The precise location and angle of the tiebacks is paramount to the installation of shoring for pylon 9. SCCI would like to confirm that the As-builts are correct for layout use.			
<b>B-0011</b>	<b>BRP - Discrepancy In Girder layout</b>	<b>Closed</b>	<b>01</b>	<b>06/25/2014</b>	<b>07/05/2014</b>	<b>07/02/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
On contract drawing S-6066 "Girder Layout- Frame No. 3", the sum of the dimensions for girders 3A through 3F is 51 '-0" while the plans scale to 52'-0". It appears that the error is in the dimension "3 EQ Spaces = 29'-0"" which is dimensioned as 29' -0", but scales to 30' -0". Please advise.			On contract drawing S-6066 "Girder Layout- Frame No. 3", the sum of the dimensions for girders 3A through 3F is 51 '-0" while the plans scale to 52'-0". It appears that the error is in the dimension "3 EQ Spaces = 29'-0"" which is dimensioned as 29' -0", but scales to 30' -0". Please advise.			
<b>B-0012</b>	<b>BRP - Requirements for Shoring Design</b>	<b>Closed</b>	<b>01</b>	<b>06/26/2014</b>	<b>07/06/2014</b>	<b>07/02/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Section 1.6H1 of Contract Specification Section 02 41 06 - Selective Site Demolition - Bus Ramps states that the "removal plan shall be prepared, wet-signed, and stamped by an engineer who is registered as a Structural Engineer in the State of California". This specification also applies to the design of temporary shoring for use in the demolition of existing foundations.			Section 1.6H1 of Contract Specification Section 02 41 06 - Selective Site Demolition - Bus Ramps states that the "removal plan shall be prepared, wet-signed, and stamped by an engineer who is registered as a Structural Engineer in the State of California". This specification also applies to the design of temporary shoring for use in the demolition of existing foundations.			
Per California Business and Professions Code, licensed Professional Civil Engineers may design structures except for public schools and hospitals. Therefore, SCCI proposes to allow the design of temporary shoring for demolition of existing foundations to be prepared, wet-			Per California Business and Professions Code, licensed Professional Civil Engineers may design structures except for public schools and hospitals. Therefore, SCCI proposes to allow the design of			



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	<p>signed and stamped by a licensed Professional Civil Engineer deemed by SCCI to have the qualifications and experience necessary for this work.</p> <p>Is this acceptable?</p>					<p>temporary shoring for demolition of existing foundations to be prepared, wet-signed and stamped by a licensed Professional Civil Engineer deemed by SCCI to have the qualifications and experience necessary for this work.</p> <p>Is this acceptable?</p>
<b>B-0013</b>	<b>BRP - Existing Conditions As-Built Utility Drawings</b>	<b>Closed</b>	<b>01</b>	<b>06/26/2014</b>	<b>07/06/2014</b>	<b>07/18/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						<b>ANSWER:</b>
SCCI requests existing condition as-built utility drawings for TG 18.1. Existing utilities have been provided for Howard St in Contract Drawing U-1006, however, U-1003, U-1004 and U-1005 do not include any utilities (Harrison St, Folsom St, Clementina St and Tehama St).						SCCI requests existing condition as-built utility drawings for TG 18.1. Existing utilities have been provided for Howard St in Contract Drawing U-1006, however, U-1003, U-1004 and U-1005 do not include any utilities (Harrison St, Folsom St, Clementina St and Tehama St).
SCCI requests that Contract Drawings U-1003, U-1004 and U-1005 be updated, or new drawings issued, to reflect all existing utility locations similar those shown in Contract Drawing U-1006. As-built utility drawings are needed for Harrison St, Folsom St, Clementina St, Tehama St and Howard St in between 1st St and 2nd St. This includes, but is not limited to, the roadway and adjacent sidewalks, the area surrounding the Caltrans Sterling Substation adjacent to Interstate 80, as well as all areas and lots in between the above mentioned streets.						SCCI requests that Contract Drawings U-1003, U-1004 and U-1005 be updated, or new drawings issued, to reflect all existing utility locations similar those shown in Contract Drawing U-1006. As-built utility drawings are needed for Harrison St, Folsom St, Clementina St, Tehama St and Howard St in between 1st St and 2nd St. This includes, but is not limited to, the roadway and adjacent sidewalks, the area surrounding the Caltrans Sterling Substation adjacent to Interstate 80, as well as all areas and lots in between the above mentioned streets.



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<b>B-0014</b>	<b>Construction of Bent 20A, 21, 22, 23</b>	<b>Closed</b>	<b>01</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/10/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						
Contract drawings S-3100 and S-3102 show the bent details for Bent 20A, 21, 22 and 23. SCCI proposes to pour Section D of the main column using the following steps -						
Step 1: Pour the inner section (shown in green on the attached drawing) using a round, steel column form						
Step 2: Lateral reinforcement drilled and bonded to inner round core using an approved adhesive, from the Cal trans list of approved chemical adhesives (please see document attached).						
Step 3: Pour the outer square (shown in blue) in 4 lifts (with the weakened joint installed after each lift) to maintain the concrete finish, as per Spec 03 30 06.						
5 lifts total: 1 center round core 4 outer separated by weakened joints						
Is this proposal acceptable?						
<b>ANSWER:</b>						
Contract drawings S-3100 and S-3102 show the bent details for Bent 20A, 21, 22 and 23. SCCI proposes to pour Section D of the main column using the following steps -						
Step 1: Pour the inner section (shown in green on the attached drawing) using a round, steel column form						
Step 2: Lateral reinforcement drilled and bonded to inner round core using an approved adhesive, from the Cal trans list of approved chemical adhesives (please see document attached).						
Step 3: Pour the outer square (shown in blue) in 4 lifts (with the weakened joint installed after each lift) to maintain the concrete finish, as per Spec 03 30 06.						
5 lifts total: 1 center round core 4 outer separated by weakened joints						
Is this proposal acceptable?						
<b>B-0015</b>	<b>BRP - AS-Builts of 303-2nd st</b>	<b>Closed</b>	<b>01</b>	<b>07/08/2014</b>	<b>07/18/2014</b>	<b>07/15/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						
Per Contract Drawing D-1003 and Section I on D-1103, there is an existing retaining wall to be removed adjacent to the building 303 2nd St. In order to properly design our temporary shoring system at this location, SCCI needs to know of any possible conflicts in the immediate proximity of the excavation. Please provide the foundation as-builts of 303 2nd St.						
<b>ANSWER:</b>						
Per Contract Drawing D-1003 and Section I on D-1103, there is an existing retaining wall to be removed adjacent to the building 303 2nd St. In order to properly design our temporary shoring system at this location, SCCI needs to know of any possible conflicts in the immediate proximity of the excavation. Please provide the foundation as-builts of 303 2nd St.						
<b>B-0016</b>	<b>BRP - Design Calculations for Connection to TTC</b>	<b>Closed</b>	<b>01</b>	<b>07/08/2014</b>	<b>07/18/2014</b>	<b>07/15/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						
Can the TJPA provide SCCI with ARUP's Frame 5 design calculations? This will allow OPAC to ensure that Frame 5						
<b>ANSWER:</b>						
Can the TJPA provide SCCI with ARUP's Frame 5 design calculations? This will allow OPAC to ensure						





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	redesign will provide the same response characteristics, at the cable-stayed bridge and Transbay Terminal building interfaces, as were intended.					that Frame 5 redesign will provide the same response characteristics, at the cable-stayed bridge and Transbay Terminal building interfaces, as were intended.
<b>B-0017</b>	<b>BRP - Frame 5 Dimension Discrepancy</b>	<b>Closed</b>	<b>01</b>	<b>07/11/2014</b>	<b>07/21/2014</b>	<b>07/24/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>						<b>ANSWER:</b>
1) Distance from RDS line to centerline bearing is shown to be 1'-3". Distance from centerline bearing to "TTBB" STA 29+07.235 is shown to be 3'-0". So the distance from RD5 to "TTBB" STA 29+07.235 should be 4'-3". However RD5 is shown to be at "TTBB" STA 29+03.000 which is only 4.235' or 4'-2 13/16" from "TTBB" STA 29+07.235. Please explain why there is a discrepancy of 0.015' or 3/16".						1) Distance from RDS line to centerline bearing is shown to be 1'-3". Distance from centerline bearing to "TTBB" STA 29+07.235 is shown to be 3'-0". So the distance from RD5 to "TTBB" STA 29+07.235 should be 4'-3". However RD5 is shown to be at "TTBB" STA 29+03.000 which is only 4.235' or 4'-2 13/16" from "TTBB" STA 29+07.235. Please explain why there is a discrepancy of 0.015' or 3/16".
2) LD5 is shown to intersect the left edge of deck at "BBTT" STA 29+02.21, 10.51' left. LD5 is shown to intersect the right edge of deck at "BBTT" STA 29+02.67, 13.00' right. SCCI took the "BBTT" and "TTBB" alignments shown on sheet C-2101 and drew in the RD5 line at "TTBB" STA 29+03.00. SCCI then extended the RD5 line to create the LD5 line. When doing so, SCCI found that the LD5 line intersects the left edge of deck at "BBTT" STA 29+03.135, 10.51' left and intersects the right edge of deck at "BBTT" STA 29+02.594, 13.00' right. Please clarify whether the LD5 stations shown at edge of deck are correct.						2) LD5 is shown to intersect the left edge of deck at "BBTT" STA 29+02.21, 10.51' left. LD5 is shown to intersect the right edge of deck at "BBTT" STA 29+02.67, 13.00' right. SCCI took the "BBTT" and "TTBB" alignments shown on sheet C-2101 and drew in the RD5 line at "TTBB" STA 29+03.00. SCCI then extended the RD5 line to create the LD5 line. When doing so, SCCI found that the LD5 line intersects the left edge of deck at "BBTT" STA 29+03.135, 10.51' left and intersects the right edge of deck at "BBTT" STA 29+02.594, 13.00' right. Please clarify whether the LD5 stations shown at edge of deck are correct.





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B-0018	BRP - Bottom of Existing Footing Elevations	Closed	01	07/17/2014	07/27/2014	07/24/2014
From: Webcor Construction LP Claude Titcher						
REQUEST:			ANSWER:			
<p>SCCI bid and planned the shoring necessary for demolition of the existing footings per the response to Question TG18.1-037 of IFB Questions &amp; Answers TG18.1 Set #3 which included the statement that the bents north of Folsom Street were similar to Section P on Contract Drawing D-1106. This section shows a footing that is immediately below existing grade and is eight feet thick.</p> <p>Additionally, SCCI reviewed the reference documents to confirm the bottom of footing elevations for these footings. Supplemental Drawing Nos. 7A and 8A of the San Francisco-Oakland Bay Bridge Railway Facilities drawings show the elevations of the bottom of footings. The first note on the Supplemental Drawing No. F-112 of the SFO Bay Bridge Railway Facilities drawings states to subtract 11 . 71' from these elevations to correct them to the city datum. SCCI has done this and compiled these elevations below.</p>			<p>SCCI bid and planned the shoring necessary for demolition of the existing footings per the response to Question TG18.1-037 of IFB Questions &amp; Answers TG18.1 Set #3 which included the statement that the bents north of Folsom Street were similar to Section P on Contract Drawing D-1106. This section shows a footing that is immediately below existing grade and is eight feet thick.</p> <p>Additionally, SCCI reviewed the reference documents to confirm the bottom of footing elevations for these footings. Supplemental Drawing Nos. 7A and 8A of the San Francisco-Oakland Bay Bridge Railway Facilities drawings show the elevations of the bottom of footings. The first note on the Supplemental Drawing No. F-112 of the SFO Bay Bridge Railway Facilities drawings states to subtract 11 . 71' from these elevations to correct them to the city datum. SCCI has done this and compiled these elevations below.</p>			
<p>This RFI will use the Bent 5 footing for example to show why SCCI does not believe that this is the correct datum correction to use. Section P on Contract Drawing D-1106 shows the Bent 5 footing immediately below grade and eight feet thick with a bottom of footing elevation of 53'-8". As shown in the table above, the elevation shown for this bottom of footing elevation on Supplemental Drawing 8A and correcting it to the city datum would provide an elevation of 29.29'. The same issue is easily seen using the section views on Bent 2 and 3 on the same Contract Drawing.</p>			<p>This RFI will use the Bent 5 footing for example to show why SCCI does not believe that this is the correct datum correction to use. Section P on Contract Drawing D-1106 shows the Bent 5 footing immediately below grade and eight feet thick with a bottom of footing elevation of 53'-8". As shown in the table above, the elevation shown for this bottom of footing elevation on Supplemental Drawing 8A and correcting it to the city datum would provide an elevation of 29.29'. The same issue is easily seen using the section views on Bent 2 and 3 on the same Contract</p>			



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B-0019	BRP - Selective Demolition of Foundations for CIDH Piles	Closed	01	07/17/2014	07/27/2014	07/24/2014
<p><b>From:</b> Webcor Construction LP                      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>On July 11, SCCI performed potholing for the existing foundations to be demolished and discovered that the depths of these foundations are significantly deeper than expected due to contradictory information provided in the responses to Bidder Questions and Reference Documents at bid time. The results of the potholing has allowed SCCI to eliminate certain information from these documents and is now proceeding under the assumption that the elevations provided in the San Francisco-Oakland Bay Bridge Railway Facilities and SFO Bay Bridge Railway Facilities are correct. These drawings indicate that these foundations are at depths as great as 34.5' from grade. SCCI requested that these elevations be confirmed in RFI B-0018 - Bottom of Existing Footing Elevations, submitted July 14.</p> <p>The foundations to be removed are located adjacent to major San Francisco thoroughfares and buildings of up to ten stories. SCCI believes that large excavations next to these structures could potentially undermine these streets and the foundations of these buildings. SCCI also believes that it is possible to reduce the number of these large excavations by reducing the volume of concrete to be removed from these foundations.</p> <p>SCCI has reviewed Contract Drawings D 1000 through D 1106 and has determined that the existing foundations Bent 5, 7 and 11 need only to be partially removed to facilitate the installation of CIDH piles. Additionally, SCCI is in receipt of Bus Ramps Change Order Request B-002 in which foundations similar to those in the base contract and in the same Bus Ramp alignment are only demolished</p>						<p>Drawing.</p> <p>Please confirm that the Datum Correction shown on Supplemental Drawing F-112 does not apply to Supplemental Drawings 7 A and 8A and provide the correct datum correction if necessary.</p> <p><b>ANSWER:</b></p> <p>On July 11, SCCI performed potholing for the existing foundations to be demolished and discovered that the depths of these foundations are significantly deeper than expected due to contradictory information provided in the responses to Bidder Questions and Reference Documents at bid time. The results of the potholing has allowed SCCI to eliminate certain information from these documents and is now proceeding under the assumption that the elevations provided in the San Francisco-Oakland Bay Bridge Railway Facilities and SFO Bay Bridge Railway Facilities are correct. These drawings indicate that these foundations are at depths as great as 34.5' from grade. SCCI requested that these elevations be confirmed in RFI B-0018 - Bottom of Existing Footing Elevations, submitted July 14.</p> <p>The foundations to be removed are located adjacent to major San Francisco thoroughfares and buildings of up to ten stories. SCCI believes that large excavations next to these structures could potentially undermine these streets and the foundations of these buildings. SCCI also believes that it is possible to reduce the number of these large excavations by reducing the volume of concrete to be removed from these foundations.</p> <p>SCCI has reviewed Contract Drawings D 1000 through D 1106 and has determined that the existing foundations Bent 5, 7 and 11 need only to be partially removed to facilitate the installation of CIDH piles. Additionally, SCCI is in receipt of Bus Ramps Change</p>



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	<p>to depths of five feet below grade.</p> <p>Therefore, SCCI proposes to demolish Bents 5, 7 and 11 to a depth of five feet below grade and, below that, only to the extent necessary to allow for CIDH installation. Please see the attached drawing for reference.</p> <p>Is this acceptable?</p>					<p>Order Request B-002 in which foundations similar to those in the base contract and in the same Bus Ramp alignment are only demolished to depths of five feet below grade.</p> <p>Therefore, SCCI proposes to demolish Bents 5, 7 and 11 to a depth of five feet below grade and, below that, only to the extent necessary to allow for CIDH installation. Please see the attached drawing for reference.</p> <p>Is this acceptable?</p>
<b>B-0019.1</b>	<b>BRP - Selective Demolition - Additional Foundations</b>	<b>Closed</b>	<b>01</b>	<b>08/01/2014</b>	<b>08/11/2014</b>	<b>08/04/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>						<b>ANSWER:</b>
SCCI has received the response to RFI #B-0019 which allows for limited and selective demolition of existing foundations for Bent 5, 7 and 11. SCCI proposes to also demolish existing foundations for Bent 9 and 16 in a technique pursuant with the response to RFI #0018. Please see attached RFI #0018 for reference. Is this acceptable?						SCCI has received the response to RFI #B-0019 which allows for limited and selective demolition of existing foundations for Bent 5, 7 and 11. SCCI proposes to also demolish existing foundations for Bent 9 and 16 in a technique pursuant with the response to RFI #0018. Please see attached RFI #0018 for reference. Is this acceptable?







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<b>B-0022</b>	<b>BRP - Surcharge Loading For Equipment Walking Off the Trestle</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/24/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
As discussed in Tuesday's (7/15/2014) Movement Review Committee, please confirm whether the TTC shoring system is capable of resisting the surcharge loads from the following equipment walking off the trestle to the east of the Pylon 9:			As discussed in Tuesday's (7/15/2014) Movement Review Committee, please confirm whether the TTC shoring system is capable of resisting the surcharge loads from the following equipment walking off the trestle to the east of the Pylon 9:			
1. Manitowoc 888 crawler crane: Maximum total load when walking off trestle = 565± kips. The "footprint" of the crane (defined as a rectangle with a length equal to the bearing length of the tracks and a width equal to the out-to-out track dimension) is 24.6'x23.2', which has a total area of 571 sqft. The average vertical surcharge load acting over the machine footprint is $565,000/571 = 990$ psf.			1. Manitowoc 888 crawler crane: Maximum total load when walking off trestle = 565± kips. The "footprint" of the crane (defined as a rectangle with a length equal to the bearing length of the tracks and a width equal to the out-to-out track dimension) is 24.6'x23.2', which has a total area of 571 sqft. The average vertical surcharge load acting over the machine footprint is $565,000/571 = 990$ psf.			
2. Bauer MC128 Foundation Crane with a BC40 Trench Cutter (Hydromill): Maximum total load when walking off trestle = 460± kips. The "footprint" of the hydromill is 23.0'x20.5' = 472 sqft. The average vertical surcharge load acting over the machine footprint is $460,000/472 = 976$ psf.			2. Bauer MC128 Foundation Crane with a BC40 Trench Cutter (Hydromill): Maximum total load when walking off trestle = 460± kips. The "footprint" of the hydromill is 23.0'x20.5' = 472 sqft. The average vertical surcharge load acting over the machine footprint is $460,000/472 = 976$ psf.			
3. Liebherr HS855HD crawler crane with clam. Maximum total load when walking off trestle = 240± kips. The "footprint" of the clam is 17.6'x16.2' = 285 sqft. The average vertical surcharge load acting over the machine footprint is $240,000/285 = 842$ psf.			3. Liebherr HS855HD crawler crane with clam. Maximum total load when walking off trestle = 240± kips. The "footprint" of the clam is 17.6'x16.2' = 285 sqft. The average vertical surcharge load acting over the machine footprint is $240,000/285 = 842$ psf.			



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<b>B-0023</b>	<b>BRP - Restrainer Pipe Clarification</b>	<b>Closed</b>	<b>01</b>	<b>07/21/2014</b>	<b>07/31/2014</b>	<b>07/24/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  1. Detail C on sheet S-6217 shows an API 5L80X Schedule 120 pipe. At the right, an end plate is shown welded to the pipe with a ¼" fillet weld. Please clarify the size and thickness of the end plate. Also please clarify the extent of the pipe since the length is not shown. It is not clear how far the pipe extends beyond the stiffener plate which is 1'-0" behind the end plate. The pipe is also shown in plan view of sheet S-6216 but the length is not given there either.  2. Please clarify the vertical location from the profile grade of the API 5L80X Schedule 120 pipe shown on Diaphragm LD1/S-6104, Diaphragm RD1/S-6107, and detail C-S6217.						<b>ANSWER:</b>  1. Detail C on sheet S-6217 shows an API 5L80X Schedule 120 pipe. At the right, an end plate is shown welded to the pipe with a ¼" fillet weld. Please clarify the size and thickness of the end plate. Also please clarify the extent of the pipe since the length is not shown. It is not clear how far the pipe extends beyond the stiffener plate which is 1'-0" behind the end plate. The pipe is also shown in plan view of sheet S-6216 but the length is not given there either.  2. Please clarify the vertical location from the profile grade of the API 5L80X Schedule 120 pipe shown on Diaphragm LD1/S-6104, Diaphragm RD1/S-6107, and detail C-S6217.
<b>B-0024</b>	<b>BRP - Shear Key Clarifications</b>	<b>Closed</b>	<b>01</b>	<b>07/21/2014</b>	<b>07/31/2014</b>	<b>07/23/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  On section A/S-6216, please clarify the locations of the centerline of shear key and the centerlines of bearings.						<b>ANSWER:</b>  On section A/S-6216, please clarify the locations of the centerline of shear key and the centerlines of bearings.
<b>B-0025</b>	<b>BRP - Diaphragm Dimension Clarifications</b>	<b>Closed</b>	<b>01</b>	<b>07/21/2014</b>	<b>07/31/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  1. On Diaphragm LD5/S-6106, Diaphragm RD5/S-6109, Section H/S-6113, Section J/S-6113, and Section K/S-6113, please clarify the vertical dimension from the profile grade to the bearing surface at the top of the sole plate.  2. On Diaphragm RD5/S-6109, the centerline of tie to TTC column is how at the centerline of the web plate. Please clarify the location of the web plate.						<b>ANSWER:</b>  1. On Diaphragm LD5/S-6106, Diaphragm RD5/S-6109, Section H/S-6113, Section J/S-6113, and Section K/S-6113, please clarify the vertical dimension from the profile grade to the bearing surface at the top of the sole plate.  2. On Diaphragm RD5/S-6109, the centerline of tie to TTC column is how at the centerline of the web plate. Please clarify the location of the web plate.





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<b>B-0026</b>	<b>BRP - Steel Box Girder Detail Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>07/21/2014</b>	<b>07/31/2014</b>	<b>08/24/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titche						
<b>REQUEST:</b>  On section K/S-6113, please clarify the vertical dimension from the profile grade to the center of the pin.						<b>ANSWER:</b>  On section K/S-6113, please clarify the vertical dimension from the profile grade to the center of the pin.
<b>B-0027</b>	<b>BRP - Restrainer Pipe Clarification 2</b>	<b>Closed</b>	<b>01</b>	<b>07/21/2014</b>	<b>07/31/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titche						
<b>REQUEST:</b>  Reference CD sheets S-6217. Would it be acceptable to substitute a member of equal axial tension capacity for the 12" API 5L80X Schedule 120 Pipe? It is SCCI's understanding that this pipe acts as a seismic longitudinal catch and will only experience axial tension, is this the designer's intent?						<b>ANSWER:</b>  Reference CD sheets S-6217. Would it be acceptable to substitute a member of equal axial tension capacity for the 12" API 5L80X Schedule 120 Pipe? It is SCCI's understanding that this pipe acts as a seismic longitudinal catch and will only experience axial tension, is this the designer's intent?
<b>B-0028</b>	<b>BRP - Bus Deck Level Slab Zone 1</b>	<b>Closed</b>	<b>CR</b>	<b>07/21/2014</b>	<b>07/21/2014</b>	<b>07/23/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titche						
<b>REQUEST:</b>  Please confirm that the bus deck level slab at Zone 1 does not need to be placed prior to installation of the Steel box girders.						<b>ANSWER:</b>  Please confirm that the bus deck level slab at Zone 1 does not need to be placed prior to installation of the Steel box girders.
<b>B-0029</b>	<b>BRP - Clementina Pedestrian Access</b>	<b>Closed</b>	<b>01</b>	<b>07/23/2014</b>	<b>08/02/2014</b>	<b>07/24/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titche						
<b>REQUEST:</b>  SCCI has found a conflict between demolition drawing D-1005 and specification section 011570 3.2G.  According to drawing D-1005, existing Bent 22 extends underneath the entire south Clementina sidewalk width and 3' into the street. The shoring required to remove this footing along with the water barriers will extend another 5' into the street totaling to 8' intrusion into the street. The street is 18' wide at this bent location leaving a 10' for vehicle traffic as noted in approved traffic control plan 002 (TG 1801-036.1 ).						<b>ANSWER:</b>  SCCI has found a conflict between demolition drawing D-1005 and specification section 011570 3.2G.  According to drawing D-1005, existing Bent 22 extends underneath the entire south Clementina sidewalk width and 3' into the street. The shoring required to remove this footing along with the water barriers will extend another 5' into the street totaling to 8' intrusion into the street. The street is 18' wide at this bent location leaving a 10' for vehicle traffic as noted in approved traffic control plan 002 (TG 1801-036.1 ).





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<b>B-0030</b>	<b>BRP - Construction of Pylon 9 and Link Beam</b>	<b>Closed</b>	<b>01</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>07/29/2014</b>
	<b>From:</b> Webcor Construction LP      Claude Titché					
<b>REQUEST:</b>						
Contract drawings S-3174 through S-3182 show the details for Pylon 9. As per S-3175 and S-3176, the link beam has reinforcement through the Pylon 9. SCCI proposes to pour Pylon 9 using perimeter steel forms and double ended form savers to avoid running rebar through the steel form and compromising the structural integrity of the steel Pylon forms.						
Is it acceptable to use double ended form savers for the horizontal link beam rebar that extends beyond the width of Pylon 9?						
<b>ANSWER:</b>						
Contract drawings S-3174 through S-3182 show the details for Pylon 9. As per S-3175 and S-3176, the link beam has reinforcement through the Pylon 9. SCCI proposes to pour Pylon 9 using perimeter steel forms and double ended form savers to avoid running rebar through the steel form and compromising the structural integrity of the steel Pylon forms.						
Is it acceptable to use double ended form savers for the horizontal link beam rebar that extends beyond the width of Pylon 9?						



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<b>B-0030.1</b>	<b>BRP - Construction of Pylon 9 and Link Beam</b>	<b>Closed</b>	<b>01</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Further to RFI 0030, SCCI is proposing to pour Pylon 9 and the link beam in 3 separate pours and have 2 construction joints at the locations shown on the attached drawings. All horizontal rebar will remain as per the contract drawings and will be continuous at the link beam & Pylon 9 connection.			Further to RFI 0030, SCCI is proposing to pour Pylon 9 and the link beam in 3 separate pours and have 2 construction joints at the locations shown on the attached drawings. All horizontal rebar will remain as per the contract drawings and will be continuous at the link beam & Pylon 9 connection.			
Pour # 1 will be Pylon 9 and the center section of link beam, to the extent of Section B and Pylon 9 (see attached drawings).			Pour # 1 will be Pylon 9 and the center section of link beam, to the extent of Section B and Pylon 9 (see attached drawings).			
Pour #2 and #3 will be the left and right sections of link beam (see attached drawings).			Pour #2 and #3 will be the left and right sections of link beam (see attached drawings).			
Is this acceptable?			Is this acceptable?			









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<b>B-0034</b>	<b>BRP - Folsom Falsework Vertical Clearance</b>	<b>Closed</b>	<b>01</b>	<b>08/06/2014</b>	<b>08/16/2014</b>	<b>08/14/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>  Per the traffic coordination meeting between SCCI, Turner and SFMT A on 8/4/14, a traffic island for the falsework on Folsom Street will be avoided. The lane widths and striping will be changed in order to meet the falsework width requirements.  The falsework doesn't meet the 15 foot vertical clearance requirements per specification section 01 15 70 3.2H. Reference RFI B-0031 and attached drawings for further detail. SFMTA has proposed to allow a vertical clearance of 14'2" or greater with proper signage. Please confirm and provide any signage requirements and restriping details to shorten lane widths.						<b>ANSWER:</b>  Per the traffic coordination meeting between SCCI, Turner and SFMT A on 8/4/14, a traffic island for the falsework on Folsom Street will be avoided. The lane widths and striping will be changed in order to meet the falsework width requirements.  The falsework doesn't meet the 15 foot vertical clearance requirements per specification section 01 15 70 3.2H. Reference RFI B-0031 and attached drawings for further detail. SFMTA has proposed to allow a vertical clearance of 14'2" or greater with proper signage. Please confirm and provide any signage requirements and restriping details to shorten lane widths.
<b>B-0035</b>	<b>BRP - Tie Down for Cable Stayed Bridge (CSB)</b>	<b>Closed</b>	<b>01</b>	<b>08/12/2014</b>	<b>08/22/2014</b>	<b>08/14/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>  Please reference Drawing S-1201.  Cable Stay Construction Sequence on Drawings No. 2 (S-1201) shows Frame 5 being installed after all of the stays are fully stressed. Does this sequence require a tie down at Hinge 9 prior to installation of Frame 5?						<b>ANSWER:</b>  Please reference Drawing S-1201.  Cable Stay Construction Sequence on Drawings No. 2 (S-1201) shows Frame 5 being installed after all of the stays are fully stressed. Does this sequence require a tie down at Hinge 9 prior to installation of Frame 5?
<b>B-0036</b>	<b>BRP - Bent 8 Link Beam Dimensions Clarification</b>	<b>Closed</b>	<b>01</b>	<b>08/12/2014</b>	<b>08/22/2014</b>	<b>08/14/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>  Reference attached sheet S-3172.  S-3172 shows two different dimension from the centerline of bent 8 to edges of the link beam for details B and C. Which dimensions are correct?						<b>ANSWER:</b>  Reference attached sheet S-3172.  S-3172 shows two different dimension from the centerline of bent 8 to edges of the link beam for details B and C. Which dimensions are correct?



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B-0037	BRP - Monitoring of CDSM Wall Tieback Detensioning	Closed	01	08/12/2014	08/22/2014	08/14/2014
From: Webcor Construction LP Claude Titcher						
REQUEST:			ANSWER:			
Note 4 on Contract Drawing D-1006 states:			Note 4 on Contract Drawing D-1006 states:			
"De-tension and remove existing tiebacks which interfere with construction of the pylon foundation. Do not de-tension the tiebacks until the TTC mat slab in Zone 1 has been placed and has reached its 28 day strength."			"De-tension and remove existing tiebacks which interfere with construction of the pylon foundation. Do not de-tension the tiebacks until the TTC mat slab in Zone 1 has been placed and has reached its 28 day strength."			
The mat slab in Zone 1 has been placed and has reached its 28 day design strength.			The mat slab in Zone 1 has been placed and has reached its 28 day design strength.			
SCCI submitted Submittal TG 1801-208.1 Selective Site Demolition - Pylon 9 Temporary Shoring which included the memo De-tensioning of Existing Tiebacks at TTC South CDSM Wall for Pylon 9 Foundation Construction. This memo provided a procedure for de-tensioning these tiebacks. The response to this submittal stated that the tiebacks were to be de-tensioned and removed one at a time and that "instrumentation placed at the adjacent TTC excavation shall be reviewed for movements".			SCCI submitted Submittal TG 1801-208.1 Selective Site Demolition - Pylon 9 Temporary Shoring which included the memo De-tensioning of Existing Tiebacks at TTC South CDSM Wall for Pylon 9 Foundation Construction. This memo provided a procedure for de-tensioning these tiebacks. The response to this submittal stated that the tiebacks were to be de-tensioned and removed one at a time and that "instrumentation placed at the adjacent TTC excavation shall be reviewed for movements".			
In response to the submittal comments received, SCCI plans to establish a baseline survey of the CDSM wall at the tieback locations and to monitor these benchmarks after each tieback is de-tensioned. All surveying/monitoring will be done with a Total Station. If no movement is detected at the time of monitoring, SCCI will proceed with de-tensioning of the next tieback. Is this acceptable?			In response to the submittal comments received, SCCI plans to establish a baseline survey of the CDSM wall at the tieback locations and to monitor these benchmarks after each tieback is de-tensioned. All surveying/monitoring will be done with a Total Station. If no movement is detected at the time of monitoring, SCCI will proceed with de-tensioning of the next tieback. Is this acceptable?			



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<b>B-0038</b>	<b>BRP - Removal of Pylon 9 Soldier Piles</b>	<b>Closed</b>	<b>CR</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Submittal TG 1801 -208. I - Selective Site Demolition - Pylon 9 Temporary Shoring detailed the procedure for the installation and removal of the beam and plate shoring system at Pylon 9. Step 15 of the Construction Sequence on Sheet SH-I stated that the final step would be to "extract" soldier piles. One of the comments received on this submittal was that the soldier piles "may not be removed by pulling as this will reduce the strength of the soil and its ability to resist later load-resisting capacity of the soil at the top of the barrettes". SCCI plans to remove these piles using a vibratory hammer as this will increase consolidation of the soil as the pile is removed.			Submittal TG 1801 -208. I - Selective Site Demolition - Pylon 9 Temporary Shoring detailed the procedure for the installation and removal of the beam and plate shoring system at Pylon 9. Step 15 of the Construction Sequence on Sheet SH-I stated that the final step would be to "extract" soldier piles. One of the comments received on this submittal was that the soldier piles "may not be removed by pulling as this will reduce the strength of the soil and its ability to resist later load-resisting capacity of the soil at the top of the barrettes". SCCI plans to remove these piles using a vibratory hammer as this will increase consolidation of the soil as the pile is removed.			
Is this acceptable?			Is this acceptable?			
<b>B-0039</b>	<b>BRP - Removal of Unexpected Structure in Lot G</b>	<b>Closed</b>	<b>01</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/14/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>			<b>ANSWER:</b>			
While excavating around existing Bents 16 and Bent 17 in Lot G, SCCI encountered an unexpected structure comprised of concrete and brick at a depth of approximately six feet. The extent and dimensions of this structure have yet to be detennined. SCCI has determined that this structure will conflict with the Bent 8 pile cap to be constructed as well as the shoring system to be used for the demolition of existing Bent 16 and 17 and the construction of new Bent 8.			While excavating around existing Bents 16 and Bent 17 in Lot G, SCCI encountered an unexpected structure comprised of concrete and brick at a depth of approximately six feet. The extent and dimensions of this structure have yet to be detennined. SCCI has determined that this structure will conflict with the Bent 8 pile cap to be constructed as well as the shoring system to be used for the demolition of existing Bent 16 and 17 and the construction of new Bent 8.			
Please advise on how to proceed.			Please advise on how to proceed.			
<b>B-0040</b>	<b>BRP - Existing Column Foundation Removal for CIDH C1</b>	<b>Closed</b>	<b>01</b>	<b>08/15/2014</b>	<b>08/25/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Drawing D-1005 calls for the removal of an existing column foundation at the location of new CIDH C1. The detail on this drawing shows a column coming up			Contract Drawing D-1005 calls for the removal of an existing column foundation at the location of new CIDH C1. The detail on this drawing shows a column			





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	<p>from a spread footing. Therefore, SCCI expected this bottom of this spread footing to be at a reasonable depth similar to other foundation to be removed. No as-builts of this footing have been provided. Potholing performed by SCCI on August 14 determined that no spread footing exists within 40 feet from grade. Without additional drawings or information about this footing, SCCI believes it is possible that this foundation is actually a CIDH installed in a retrofit contract.</p> <p>If this foundation is indeed a CIDH pile and extends as deep as to bedrock, its removal would be both cost and time intensive if it is possible at all. As shown on the attached picture, the edge of CIDH C1 is in conflict with the existing foundation at grade. This condition will prevent the installation of CIDH C1. Possible solutions include decreasing the diameter of the CIDH or moving the CIDH and column away from the existing foundation.</p> <p>Please advise on how to proceed.</p>					
<b>B-0040.1</b>	<b>BRP - Additional Foundation Adjacent to Existing Bent 5 CIDH</b>	<b>Closed</b>	<b>01</b>	<b>08/27/2014</b>	<b>09/06/2014</b>	<b>09/09/2014</b>
	<p><b>From:</b> Webcor Construction LP      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>SCCI previously submitted Rfl B- 040 which stated that the footing to be demolished adjacent to new CIDH C1 appeared to be a CIDH pile. SCCI has since received Reference Drawings SFOBB - San Francisco Approach Replace which appears to confirm that this footing is actually the CIDH at Bent 5 installed in this 2004 retrofit.</p> <p>While doing hazardous material removal for new CIDH C1 on August 27, SCCI encountered an additional footing adjacent to the Bent 5 CIDH. Pictures of this footing are attached. Also attached are sketches for clarification that show the footing as currently exposed.</p> <p>Please provide as-builts of this footing and advise on how to proceed.</p>					
						<p><b>ANSWER:</b></p> <p>SCCI previously submitted Rfl B- 040 which stated that the footing to be demolished adjacent to new CIDH C1 appeared to be a CIDH pile. SCCI has since received Reference Drawings SFOBB - San Francisco Approach Replace which appears to confirm that this footing is actually the CIDH at Bent 5 installed in this 2004 retrofit.</p> <p>While doing hazardous material removal for new CIDH C1 on August 27, SCCI encountered an additional footing adjacent to the Bent 5 CIDH. Pictures of this footing are attached. Also attached are sketches for clarification that show the footing as currently exposed.</p> <p>Please provide as-builts of this footing and advise on</p>



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						how to proceed.
<b>B-0040.2</b>	<b>BRP - CIDH C1 Location</b>	<b>Closed</b>	<b>01</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>09/26/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titche						
<b>REQUEST:</b> In accordance with RFI B-0040 response, new C1 location will be issued in an upcoming ASI. Since CIDH work will start in early October 2014 at Lot H, please provide the detail of new C1 location.						<b>ANSWER:</b> In accordance with RFI B-0040 response, new C1 location will be issued in an upcoming ASI. Since CIDH work will start in early October 2014 at Lot H, please provide the detail of new C1 location.
<b>B-0041</b>	<b>BRP - Design Reaction at Hinge H</b>	<b>Closed</b>	<b>01</b>	<b>08/15/2014</b>	<b>08/25/2014</b>	<b>08/29/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titche						
<b>REQUEST:</b> Please provide the design reaction, that were submitted to the TTC building designer, for the Hinge H bearing.  This was discussed at the weekly Structural Issues Review Meeting on 8/14/2014. Information requested would help coordinate the interface work between the two trade packages at hinge H (TG7.1 and TG18.1), and progress on the Box Girder V.E.						<b>ANSWER:</b> Please provide the design reaction, that were submitted to the TTC building designer, for the Hinge H bearing.  This was discussed at the weekly Structural Issues Review Meeting on 8/14/2014. Information requested would help coordinate the interface work between the two trade packages at hinge H (TG7.1 and TG18.1), and progress on the Box Girder V.E.

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<b>B-0043</b>	<b>BRP - CIDH Permanent Casing</b>	<b>Closed</b>	<b>01</b>	<b>08/21/2014</b>	<b>08/31/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>SCCI proposes to use corrugated metal pipe (CMP) as permanent steel casing for the CIDHs when using the optional construction joint, is this acceptable? The specifications are confusing on this subject.</p> <p>Per Specification Section 31 63 30 3.5-B, " ... The permanent casing must:" then in section 3.5-B-5 " ... must comply with article 3.3." Per 31 63 30 3.3-C,"Temporary casings must be:" then in section 3.3- C-2; "noncorrugated with smooth surfaces." Since we are discussing a permanent casing and this section applies to temporary casings, does the noncorrugated rule apply?</p> <p>Project Specification 31 63 30 appears to be based on the Caltrans Standard Specification. Using the 2010 version of the CTSS, 49-3.02C(6)-5 is similar to the project specification 31 63 30 3.5, except it refers the "must comply" to a section on "Permanent Steel Casing Installation" {49-3.02C(5)}. As SCCI's Project Specification does not include a section on permanent steel casing materials, SCCI believes the reference to the Temporary Steel Casing to be inadvertent.</p> <p>If safety and/or quality is a concern, SCCI can provide stamped engineering to stating that the CMP is capable of withstanding loads from installation, lateral concrete pressures and earth pressures and will support personnel working inside the casing after the CIDH installation is complete.</p>			<p>SCCI proposes to use corrugated metal pipe (CMP) as permanent steel casing for the CIDHs when using the optional construction joint, is this acceptable? The specifications are confusing on this subject.</p> <p>Per Specification Section 31 63 30 3.5-B, " ... The permanent casing must:" then in section 3.5-B-5 " ... must comply with article 3.3." Per 31 63 30 3.3-C,"Temporary casings must be:" then in section 3.3- C-2; "noncorrugated with smooth surfaces." Since we are discussing a permanent casing and this section applies to temporary casings, does the noncorrugated rule apply?</p> <p>Project Specification 31 63 30 appears to be based on the Caltrans Standard Specification. Using the 2010 version of the CTSS, 49-3.02C(6)-5 is similar to the project specification 31 63 30 3.5, except it refers the "must comply" to a section on "Permanent Steel Casing Installation" {49-3.02C(5)}. As SCCI's Project Specification does not include a section on permanent steel casing materials, SCCI believes the reference to the Temporary Steel Casing to be inadvertent.</p> <p>If safety and/or quality is a concern, SCCI can provide stamped engineering to stating that the CMP is capable of withstanding loads from installation, lateral concrete pressures and earth pressures and will support personnel working inside the casing after the CIDH installation is complete.</p>			



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<b>B-0044</b>	<b>BRP - CIDH Pile Splice Detail</b>	<b>Closed</b>	<b>CR</b>	<b>08/29/2014</b>	<b>08/29/2014</b>	<b>09/02/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Note 8 on Contract Drawing S-3190 states that the vertical reinforcement bars, CI , are to be fabricated ten feet longer and that ten feet of additional hoops, D, are to be provided. To avoid fabricating all CIDH cages ten feet longer than may be necessary, SCCI and Case Pacific preproposing to fabricate all CIDH cages to the lengths shown on Contract Drawing S-3190 and to splice the additional ten feet of rebar cage when conditions require it. The proposed splice detail is shown on the attached sketch. The additional reinforcement for these splice lengthened cages will be kept on site.			Note 8 on Contract Drawing S-3190 states that the vertical reinforcement bars, CI , are to be fabricated ten feet longer and that ten feet of additional hoops, D, are to be provided. To avoid fabricating all CIDH cages ten feet longer than may be necessary, SCCI and Case Pacific preproposing to fabricate all CIDH cages to the lengths shown on Contract Drawing S-3190 and to splice the additional ten feet of rebar cage when conditions require it. The proposed splice detail is shown on the attached sketch. The additional reinforcement for these splice lengthened cages will be kept on site.			
<b>B-0045</b>	<b>BRP - Seismic Displacement of Sliding Bearings at Hinge 9</b>	<b>Closed</b>	<b>CR</b>	<b>08/29/2014</b>	<b>09/08/2014</b>	<b>09/05/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
For the design of the sliding bearings at Hinge 9 as part the post bid VE for Frame 5, please provide the maximum seismic displacement demands, at Hinge 9, for the cable-stayed bridge.			For the design of the sliding bearings at Hinge 9 as part the post bid VE for Frame 5, please provide the maximum seismic displacement demands, at Hinge 9, for the cable-stayed bridge.			
<b>B-0046</b>	<b>BRP - Erection Sequence and Diagrams</b>	<b>Closed</b>	<b>01</b>	<b>09/04/2014</b>	<b>09/14/2014</b>	<b>09/16/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Are the camber diagrams on drawing S-6069 and the cable loads shown on column 5 of Stay Cable data table, drawing S-6079, based on the erection sequence analysis shown on drawings S-1200 and S-1201?			Are the camber diagrams on drawing S-6069 and the cable loads shown on column 5 of Stay Cable data table, drawing S-6079, based on the erection sequence analysis shown on drawings S-1200 and S-1201?			
SCCI intends to adhere to the design engineer's specified erection sequence shown on the contract plans. Therefore, for our erection analysis and plan, it is important to understand if the design engineer calculated these girder cambers and stay cable loads based on the shown sequence of installing the dropin span after frame 4 has its cables stressed, barriers and rails installed and the falsework removed.			SCCI intends to adhere to the design engineer's specified erection sequence shown on the contract plans. Therefore, for our erection analysis and plan, it is important to understand if the design engineer calculated these girder cambers and stay cable loads based on the shown sequence of installing the dropin span after frame 4 has its cables stressed, barriers and rails installed and the falsework removed.			





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B-0050	<div>BRP - Length of Barrette Piles</div> <div>From: Webcor Construction LP</div> <div>Stephanie Azzolino</div> <div>REQUEST:</div> <div>Contract Drawing S-3191 indicates a Barrette pile length of 20 feet. The standard construction method to build the barrettes involves two full primary bites being dug first followed by a middle bite in between the previously dug primary bites. With a Barrette measuring 20 feet in length, the middle bite will only be 1.6' in length. Such small middle bite may collapse on itself while digging a primary bite which could affect the verticality of that primary bite. Nicholson proposes to increase the length of the Barrettes to 21' to decrease the chance of the middle bite to collapse.</div> <div>Please confirm Barrettes 21 feet in length is acceptable.</div>	Closed	CR	09/08/2014	09/18/2014	09/09/2014
	<div></div> <div></div> <div></div> <div>ANSWER:</div> <div>Contract Drawing S-3191 indicates a Barrette pile length of 20 feet. The standard construction method to build the barrettes involves two full primary bites being dug first followed by a middle bite in between the previously dug primary bites. With a Barrette measuring 20 feet in length, the middle bite will only be 1.6' in length. Such small middle bite may collapse on itself while digging a primary bite which could affect the verticality of that primary bite. Nicholson proposes to increase the length of the Barrettes to 21' to decrease the chance of the middle bite to collapse.</div> <div>Please confirm Barrettes 21 feet in length is acceptable.</div>					





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<b>B-0051</b>	<b>BRP - Intumescent For Frame 5</b>	<b>Closed</b>	<b>CR</b>	<b>09/08/2014</b>	<b>09/18/2014</b>	<b>09/11/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						
<p>SCCI is actively pricing and re-designing the frame 5, drop-in span as part of CR B-005 and needs to better understand the original design intent for this bridge section as it relates to the intumescent paint.</p> <p>1. The only drawings SCCI see reference to intumescent are S-5068, S-6111 Detail B, S-6112 Detail D, S-6113 Detail H and K, and S-6114 Detail N. Per these it appears the design intent is to install intumescent on the underside of the "tub" (S-5068), the additional "Tub" piece (N/S-6114), and all of the exposed steel on the frame 4 side, except the lowest horizontal steel (B/S-6111 and C/S6-112). At the building side, it is confusing as only H/S-6113 shows intumescent similar to the frame 5 side. K/S-6113 only shows the intumescent on the bottom of the restraining rod steel box. J/S-6113 and L/S-6114 do not show intumescent. Also of note, the steel link beam on details D and B of S-6115 do not show intumescent. Please confirm design intent of intumescent coverage.</p> <p>2. Per specification 09 96 46 Intumescent Paint ¿ Bus Ramps, SCCI is to "provide the specified fire resistance classification approved by Authorities having Jurisdiction" Who are the authorities having jurisdiction? What is the fire resistance classification for frame 5? SCCI assumed during bid time and currently a 1- hour rating.</p>						
<b>ANSWER:</b>						
<p>SCCI is actively pricing and re-designing the frame 5, drop-in span as part of CR B-005 and needs to better understand the original design intent for this bridge section as it relates to the intumescent paint.</p> <p>1. The only drawings SCCI see reference to intumescent are S-5068, S-6111 Detail B, S-6112 Detail D, S-6113 Detail H and K, and S-6114 Detail N. Per these it appears the design intent is to install intumescent on the underside of the "tub" (S-5068), the additional "Tub" piece (N/S-6114), and all of the exposed steel on the frame 4 side, except the lowest horizontal steel (B/S-6111 and C/S6-112). At the building side, it is confusing as only H/S-6113 shows intumescent similar to the frame 5 side. K/S-6113 only shows the intumescent on the bottom of the restraining rod steel box. J/S-6113 and L/S-6114 do not show intumescent. Also of note, the steel link beam on details D and B of S-6115 do not show intumescent. Please confirm design intent of intumescent coverage.</p> <p>2. Per specification 09 96 46 Intumescent Paint ¿ Bus Ramps, SCCI is to "provide the specified fire resistance classification approved by Authorities having Jurisdiction" Who are the authorities having jurisdiction? What is the fire resistance classification for frame 5? SCCI assumed during bid time and currently a 1- hour rating.</p>						







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	<div>REQUEST:<div>Section A and B of Contract Drawings S-3191 call for 6" concrete cover around the reinforcing cage in the barrette piles. Per Nicholson Construction Company's experience, given the rebar congestion of the reinforcement cage, the 6" concrete cover allows for a path of least resistance for the concrete to rise faster on the outside of the cage and increase the risks of having bentonite being trapped inside the cage during the concreteting operation. Nicholson believes that reducing the concrete cover to 3" will decrease the risk of this happening. SCCI's proposal will not change the width of the barrette pile, it will increase the width of the rebar cage by 6". Please note that the response to RFI B-0050 allowed for the barrette pile to be on foot longer in length. These changes will be shown on the shop drawings, to be submitted in a forthcoming submittal.</div></div>					
						<div>ANSWER:<div>Section A and B of Contract Drawings S-3191 call for 6" concrete cover around the reinforcing cage in the barrette piles. Per Nicholson Construction Company's experience, given the rebar congestion of the reinforcement cage, the 6" concrete cover allows for a path of least resistance for the concrete to rise faster on the outside of the cage and increase the risks of having bentonite being trapped inside the cage during the concreteting operation. Nicholson believes that reducing the concrete cover to 3" will decrease the risk of this happening. SCCI's proposal will not change the width of the barrette pile, it will increase the width of the rebar cage by 6". Please note that the response to RFI B-0050 allowed for the barrette pile to be on foot longer in length. These changes will be shown on the shop drawings, to be submitted in a forthcoming submittal.</div></div>
	Is this acceptable?					Is this acceptable?
B-0055	BRP - Barrette Pile Reinforcement Cage	Closed	01	09/15/2014	09/25/2014	09/25/2014
	From: Webcor Construction LP					Claude Titcher
	<div>REQUEST:<div>Contract Drawing S-3191 details the reinforcement needed for the Barrettes. The current reinforcement details indicate a clear space between rebar as low as 3.75" in some areas. Per Nicholson's experience and several codes (such as EN 1534 and US FHW A), a larger clear space is recommended as small clear spaces may impede the flow of concrete inside the cage and increase the risks of concrete anomalies.</div></div>					<div>ANSWER:<div>Contract Drawing S-3191 details the reinforcement needed for the Barrettes. The current reinforcement details indicate a clear space between rebar as low as 3.75" in some areas. Per Nicholson's experience and several codes (such as EN 1534 and US FHW A), a larger clear space is recommended as small clear spaces may impede the flow of concrete inside the cage and increase the risks of concrete anomalies.</div></div>
	As a result, Nicholson proposes to increase the bar sizes and use anchor heads (in lieu of hooks) in order					As a result, Nicholson proposes to increase the bar sizes and use anchor heads (in lieu of hooks) in order



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	<p>to increase the clear space between rebar. Please see attached sketches SD-02 and SD-03. Note that the section of steel remains similar in any direction in order to avoid impacting the structural capacity of the Barrette.</p> <p>Furthermore, due to space constraints at the site the cage will be built in two (2) 1 0ft wide by three (3) 60ft long sections approximately (the length will vary depending on the reinforcement requirement by elevation). Each 10ft x 60ft section will be prefabricated and transported to the site. Two 1 0ft sections will be joined together to form a 20ft section. Our sketches include the required horizontal reinforcement to join the sections. The three 20ft by 60ft sections will be lifted and installed in the trench.</p> <p>Please confirm the reinforcement cage proposed is acceptable.</p>					
B-0056	BRP - Stay Cable Strand Size	Closed	01	09/15/2014	09/25/2014	09/18/2014
	<p>From: Webcor Construction LP</p> <p>Claude Titcher</p> <p>REQUEST:</p> <p>SCCI has received the following RFI from VStrutural LLC (VSL):</p> <p>"The contract documents for the Transbay Transit Center Bus Ramp project (ASI 150-003) specify that the steel strand for the stay cables be 0.6" diameter, 270 ksi, weldless low-relaxation seven-wire strand with fy=0.9fs, conforming to the requirements of ASTM A416. The majority of VSL's recent and ongoing stay cable projects have used or are using strand with the same overall properties, but with a slightly larger diameter of 0.62". Some of the projects using the 0.62" diameter strand include:</p> <p>St. Croix River Crossing project in Stillwater, Minnesota (ongoing)</p> <p>Tappan Zee Bridge Replacement project in Tarrytown,</p>					
	<p>ANSWER:</p> <p>SCCI has received the following RFI from VStrutural LLC (VSL):</p> <p>"The contract documents for the Transbay Transit Center Bus Ramp project (ASI 150-003) specify that the steel strand for the stay cables be 0.6" diameter, 270 ksi, weldless low-relaxation seven-wire strand with fy=0.9fs, conforming to the requirements of ASTM A416. The majority of VSL's recent and ongoing stay cable projects have used or are using strand with the same overall properties, but with a slightly larger diameter of 0.62". Some of the projects using the 0.62" diameter strand include:</p> <p>St. Croix River Crossing project in Stillwater, Minnesota (ongoing)</p> <p>Tappan Zee Bridge Replacement project in Tarrytown,</p>					

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	<p>New York (ongoing) Gerald Desmond Bridge Replacement project in Long Beach, California (ongoing) Ironton-Russell Bridge project in Ironton, Ohio (ongoing) Audubon Bridge project in St. Francisville, Louisiana (recent) A25 Bridge project near Montreal, Quebec, Canada (recent)</p> <p>In addition to being more widely utilized on recent stay cable projects than its 0.6" diameter counterpart, the 0.62" diameter strand has a slightly larger cross-sectional steel area (0.2325 sq in vs. 0.217 sq in for the 0.6" diameter strand). Using the larger individual strand steel area allows for the overall steel area in a particular stay cable to be achieved using fewer strands. Using fewer strands in a stay cable allows for extra unused holes in the cable anchorages, which allows for future expansion of load capacity within the stay. VSL's stay cable anchorages are designed to use either 0.6" or 0.62" diameter strand.</p> <p>To illustrate the principle described above on the Transbay project, the currently project parameters specify that VSL's 109-strand anchorages be utilized with 109 EA, 0.6" diameter strands. In this configuration, the anchorages are completely filled with strand and thus cannot allow for future expansion of load capacity within the stays. The total required steel area for each cable is: 0.217 sq in x 109 = 23.653 sq in. By using 0.62" diameter strands, the strand count for each anchorage could be reduced to 102 EA (0.2325 sq in x 102 = 23.715 sq in &gt; 23.653 sq in). Using 102 EA strands in each cable leaves seven open holes in each anchorage, allowing for a significant expansion of load capacity should the need arise in the future. As noted above, VSL's stay cable anchorages are designed to be used with either strand size, and therefore the anchorages could sustain the extra loading that would come with any future addition of 0.62" diameter strands.</p> <p>As one can see from the list above, 0.62" diameter strand has also become the norm for current and recent projects that VSL has worked on here in the USA. As such, it is more readily available from domestic strand suppliers.</p> <p>VSL therefore proposes to use 0.62" diameter strands on</p>					
	<p>New York (ongoing) Gerald Desmond Bridge Replacement project in Long Beach, California (ongoing) Ironton-Russell Bridge project in Ironton, Ohio (ongoing) Audubon Bridge project in St. Francisville, Louisiana (recent) A25 Bridge project near Montreal, Quebec, Canada (recent)</p> <p>In addition to being more widely utilized on recent stay cable projects than its 0.6" diameter counterpart, the 0.62" diameter strand has a slightly larger cross-sectional steel area (0.2325 sq in vs. 0.217 sq in for the 0.6" diameter strand). Using the larger individual strand steel area allows for the overall steel area in a particular stay cable to be achieved using fewer strands. Using fewer strands in a stay cable allows for extra unused holes in the cable anchorages, which allows for future expansion of load capacity within the stay. VSL's stay cable anchorages are designed to use either 0.6" or 0.62" diameter strand.</p> <p>To illustrate the principle described above on the Transbay project, the currently project parameters specify that VSL's 109-strand anchorages be utilized with 109 EA, 0.6" diameter strands. In this configuration, the anchorages are completely filled with strand and thus cannot allow for future expansion of load capacity within the stays. The total required steel area for each cable is: 0.217 sq in x 109 = 23.653 sq in. By using 0.62" diameter strands, the strand count for each anchorage could be reduced to 102 EA (0.2325 sq in x 102 = 23.715 sq in &gt; 23.653 sq in). Using 102 EA strands in each cable leaves seven open holes in each anchorage, allowing for a significant expansion of load capacity should the need arise in the future. As noted above, VSL's stay cable anchorages are designed to be used with either strand size, and therefore the anchorages could sustain the extra loading that would come with any future addition of 0.62" diameter strands.</p> <p>As one can see from the list above, 0.62" diameter strand has also become the norm for current and recent projects that VSL has worked on here in the</p>					



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	<p>the Transbay Transit Center Bus Ramp project stay cables in lieu of 0.6" diameter strands. The 0.62" diameter strand will meet all of the other criteria specified within the contract documents and the current edition of the PTI Stay Cable Recommendations."</p> <p>Is it acceptable to replace the 0.6" diameter steel stay cable strands with 0.62" diameter strands?</p>					<p>USA. As such, it is more readily available from domestic strand suppliers.</p> <p>VSL therefore proposes to use 0.62" diameter strands on the Transbay Transit Center Bus Ramp project stay cables in lieu of 0.6" diameter strands. The 0.62" diameter strand will meet all of the other criteria specified within the contract documents and the current edition of the PTI Stay Cable Recommendations."</p> <p>Is it acceptable to replace the 0.6" diameter steel stay cable strands with 0.62" diameter strands?</p>
B-0057	<p><b>BRP - Stay Cable Geometry and Loading</b></p> <p><b>From:</b> Webcor Construction LP                      Claude Titché</p> <p><b>REQUEST:</b></p> <p>SCCI has received the following RFI from VStructural LLC.</p> <p>After a review of the contract drawings and specifications, the following items are requested for the design of the saddle body/system:</p> <ol style="list-style-type: none"><li>1. Change the tangent points of the saddle to the locations shown in the drawing and provide coordinates to VSL for final design</li><li>2. Review the Stay Cable Data on Sheet No. S-6079 and update as necessary. In particular, it appears that Note 3 may need to be revised to reflect the revised conditions.</li><li>3. The stay cable strands are continuous through the saddles. Update stay cable lengths "Ls" and nominal stay steel weights "Ms" in the table on Sheet No S-6079 to reflect the cable lengths from bearing plate to bearing plate, inclusive of the saddle in the pylon</li><li>4. Provide work points at the bearing plate location on the deck</li><li>5. Provide cable rotation in ULS and FLS (characteristics and fatigue)</li></ol>	Closed	01	09/15/2014	09/25/2014	10/13/2014
						<p><b>ANSWER:</b></p> <p>SCCI has received the following RFI from VStructural LLC.</p> <p>After a review of the contract drawings and specifications, the following items are requested for the design of the saddle body/system:</p> <ol style="list-style-type: none"><li>1. Change the tangent points of the saddle to the locations shown in the drawing and provide coordinates to VSL for final design</li><li>2. Review the Stay Cable Data on Sheet No. S-6079 and update as necessary. In particular, it appears that Note 3 may need to be revised to reflect the revised conditions.</li><li>3. The stay cable strands are continuous through the saddles. Update stay cable lengths "Ls" and nominal stay steel weights "Ms" in the table on Sheet No S-6079 to reflect the cable lengths from bearing plate to bearing plate, inclusive of the saddle in the pylon</li><li>4. Provide work points at the bearing plate location on the deck</li><li>5. Provide cable rotation in ULS and FLS</li></ol>



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6. Provide angular rotation of each anchorage under ULS, SLS, FLS						(characteristics and fatigue)
7. Provide tension variation under SLS, ULS, FLS						6. Provide angular rotation of each anchorage under ULS, SLS, FLS
Please advise on the above points.						7. Provide tension variation under SLS, ULS, FLS
						Please advise on the above points.
<b>B-0058</b>	<b>BRP - Stay Cable Design Temperatures</b>	<b>Closed</b>	<b>01</b>	<b>09/15/2014</b>	<b>09/25/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>						<b>ANSWER:</b>
SCCI has received the following RFI from VStructural LLC -						SCCI has received the following RFI from VStructural LLC -
After a review of the contract drawings and specifications, the following item is requested for the design of the stay cable system:						After a review of the contract drawings and specifications, the following item is requested for the design of the stay cable system:
Please provide the design temperature range (maximum and minimum temperatures) for the stay cables in order to determine the correct HDPE stay pipe lengths and the lengths of connecting elements. Expansion and contraction of the HDPE pipes must be taken into account when specifying the lengths of these components.						Please provide the design temperature range (maximum and minimum temperatures) for the stay cables in order to determine the correct HDPE stay pipe lengths and the lengths of connecting elements. Expansion and contraction of the HDPE pipes must be taken into account when specifying the lengths of these components.

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B-0059	BRP - Hangers for Slurry Lines off Of Fremont St. Off-Ramp	Closed	01	09/18/2014	09/28/2014	09/30/2014
<div> <div> <b>From:</b> Webcor Construction LP           Claude Titche         </div> <div> <b>REQUEST:</b> <p>As shown on the Site Logistics Plan, Submittal TG 1801-81.1 , Case Pacific's mud plant and baker tank will be staged in Lot H, North of Clementina St. In order to provide slurry and water to the CIDH locations in Lots G and I, SCCI has installed trenches across Clementina St. and Tehama St. for piping. However, due to the number and type of utilities (shown on the attached excerpt from SCCI Transmittal WOJV-0001 - Utility Locates per USA North) below Folsom Street, SCCI would like to avoid trenching at this location by hanging two 4" slurry pipes from the Fremont Street Off-Ramp. Based on this, SCCI has two questions:</p> <ol style="list-style-type: none"> <li>1. Will Cal trans allow SCCI to install hangers of any sort from the Fremont Street Off-Ramp to facilitate this work?</li> <li>2. If yes, is the attached detail provided by Case Pacific (SK-I) acceptable?</li> </ol> <p>SCCI will work to prevent damage to the existing structure and would be responsible for any repairs caused by these hangers.</p> </div> <div> <b>ANSWER:</b> <p>As shown on the Site Logistics Plan, Submittal TG 1801-81.1 , Case Pacific's mud plant and baker tank will be staged in Lot H, North of Clementina St. In order to provide slurry and water to the CIDH locations in Lots G and I, SCCI has installed trenches across Clementina St. and Tehama St. for piping. However, due to the number and type of utilities (shown on the attached excerpt from SCCI Transmittal WOJV-0001 - Utility Locates per USA North) below Folsom Street, SCCI would like to avoid trenching at this location by hanging two 4" slurry pipes from the Fremont Street Off-Ramp. Based on this, SCCI has two questions:</p> <ol style="list-style-type: none"> <li>1. Will Cal trans allow SCCI to install hangers of any sort from the Fremont Street Off-Ramp to facilitate this work?</li> <li>2. If yes, is the attached detail provided by Case Pacific (SK-I) acceptable?</li> </ol> <p>SCCI will work to prevent damage to the existing structure and would be responsible for any repairs caused by these hangers.</p> </div> </div>						





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B-0060	BRP - Number of Barrette Pile Tremies	Closed	01	09/18/2014	09/28/2014	09/26/2014
From: Webcor Construction LP Claude Titché						
REQUEST: Contract drawing S-3191 indicates two openings (for two tremies) in each Barrette's rebar cage. This drawing is in conflict with Contract Specification 31 63 32 3.4 D 3 that indicates that three tremies are needed for each Barrette (one tremie for each 7 feet of wall length). Please note that the Response to RFI-0050 allowed for the length of the barrette piles to be increased from 20'-0" to 21 '-0".  The European Standard EN 1538 indicates two tremies (properly spaced so that the concrete does not have to travel more than 8 feet) is sufficient for each Barrette. Additionally, in reference to RFI BRP-0055 and, in an effort to reduce rebar congestion, 2 tremies are preferred.  Please confirm that two tremie pipes per Barrette are acceptable.						
					ANSWER: Contract drawing S-3191 indicates two openings (for two tremies) in each Barrette's rebar cage. This drawing is in conflict with Contract Specification 31 63 32 3.4 D 3 that indicates that three tremies are needed for each Barrette (one tremie for each 7 feet of wall length). Please note that the Response to RFI-0050 allowed for the length of the barrette piles to be increased from 20'-0" to 21 '-0".  The European Standard EN 1538 indicates two tremies (properly spaced so that the concrete does not have to travel more than 8 feet) is sufficient for each Barrette. Additionally, in reference to RFI BRP-0055 and, in an effort to reduce rebar congestion, 2 tremies are preferred.  Please confirm that two tremie pipes per Barrette are acceptable.	
<hr/>						
B-0061	BRP - Roof Scallop Wall Construction Joint Relocation	Void	CR	09/19/2014	09/29/2014	
From: Webcor Construction LP Stephanie Azzolino						
REQUEST: Per drawing SI-3282 detail 4 and 5 the base of the scallop wall requires a chamfered starter wall that transitions from a 1 to 1 angle, 3 to 2 angle, and a 4 to 1 angle throughout the geometry of the scallops.Shimmick requests approval to relocate the construction joint from the wall to the deck and pour the full height of the wall including the deck beneath. See attachment "A" for clarification. This change would only occur at the scallops and detail 2 "Typical walls at the roof north & south edges" would not change.  Please confirm the joint location is acceptable.						
					ANSWER: Per drawing SI-3282 detail 4 and 5 the base of the scallop wall requires a chamfered starter wall that transitions from a 1 to 1 angle, 3 to 2 angle, and a 4 to 1 angle throughout the geometry of the scallops.Shimmick requests approval to relocate the construction joint from the wall to the deck and pour the full height of the wall including the deck beneath. See attachment "A" for clarification. This change would only occur at the scallops and detail 2 "Typical walls at the roof north & south edges" would not change.  Please confirm the joint location is acceptable.	
<hr/>						
B-0062	BRP - Expansion and Fixed Bearing Clarifications	Closed	CR	09/24/2014	10/04/2014	10/24/2014
From: Webcor Construction LP Claude Titché						
REQUEST:					ANSWER:	





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	<p>Please see questions below regarding the PTFE Bearings -</p> <p>Drawing S-6250 "Fremont Street Off Ramp (Widen) PTFE Expansion Bearing Details" (revision date 5/21114):</p> <p>1. Dimensions in the "Expansion Bearing Table" show a " * " for the Concave, Convex and Masonry plates at "Hinge 23 Bearing A" and "Hinge 23 Bearing B" with a note that says " * =use existing assembly parts". Does this imply a refurbishment using pre-existing components? or does this imply that the dimensions are the same as what is shown for Hinge 23 Bearing C?</p> <p>2. Top view of the Sole Plate shows a call-out for a total of (16) 114" DIA x 3/4" Stainless Steel Countersunk Cap Screws. Is there a requirement for drilling and tapping the Sole Plates to provide for CTSK cap screws? If so, what does the installation consist of?</p> <p>3. Plans are missing anchorage details showing the complete stud layout for the Masonry Plate. Please provide.</p> <p>4. We plan to use ASTM A108 Headed Studs and Threaded Studs for the welded anchorage shown. Please advise if this is acceptable.</p> <p>Drawing S-6251 "Fremont Street Off Ramp (Widen) PTFE Fixed Bearing Detail" (revision date 5/21/14):</p> <p>1. Section A detail shows the call-out for "H ACT" from the bottom of the Masonry Plate to the bottom of the recess. If this detail is incorrect, please confirm the maximum thickness of the Convex Plate is 1.45" and the Assembly Height is 4.50" as shown in the "Fixed Bearing Table".</p> <p>2. Note 1 says "anchor bolts shall conform with ASTM F1554 Grade 105.". We plan to use ASTM A 108 Headed Studs. Please advise if this is acceptable.</p> <p>Drawing S-6252 "Bus Ramp Viaduct PTFE Expansion</p>					
	<p>Please see questions below regarding the PTFE Bearings -</p> <p>Drawing S-6250 "Fremont Street Off Ramp (Widen) PTFE Expansion Bearing Details" (revision date 5/21114):</p> <p>1. Dimensions in the "Expansion Bearing Table" show a " * " for the Concave, Convex and Masonry plates at "Hinge 23 Bearing A" and "Hinge 23 Bearing B" with a note that says " * =use existing assembly parts". Does this imply a refurbishment using pre-existing components? or does this imply that the dimensions are the same as what is shown for Hinge 23 Bearing C?</p> <p>2. Top view of the Sole Plate shows a call-out for a total of (16) 114" DIA x 3/4" Stainless Steel Countersunk Cap Screws. Is there a requirement for drilling and tapping the Sole Plates to provide for CTSK cap screws? If so, what does the installation consist of?</p> <p>3. Plans are missing anchorage details showing the complete stud layout for the Masonry Plate. Please provide.</p> <p>4. We plan to use ASTM A108 Headed Studs and Threaded Studs for the welded anchorage shown. Please advise if this is acceptable.</p> <p>Drawing S-6251 "Fremont Street Off Ramp (Widen) PTFE Fixed Bearing Detail" (revision date 5/21/14):</p> <p>1. Section A detail shows the call-out for "H ACT" from the bottom of the Masonry Plate to the bottom of the recess. If this detail is incorrect, please confirm the maximum thickness of the Convex Plate is 1.45" and the Assembly Height is 4.50" as shown in the "Fixed Bearing Table".</p> <p>2. Note 1 says "anchor bolts shall conform with ASTM F1554 Grade 105.". We plan to use ASTM A 108 Headed Studs. Please advise if this is acceptable.</p>					



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Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
	<p>Bearing Details" (revision date 4/21/14):</p> <p>1. Sole Plate shows a call out for (20) ASTM A325SC H.S. 3/4" DIA x 8" Stud Bolts. Will we need to drill and tap holes in the Sole Plate to accommodate this type of assembly? If so, please send a detailed drawing showing anchorage requirements for the Sole Plate.</p> <p>2. Plans are missing anchorage details showing the complete stud layout for the Masonry Plate. Please provide.</p> <p>3. We plan to use ASTM A 108 Headed Studs for the welded anchorage shown. Please advise if this is acceptable.</p> <p>Drawing S-6253 "Bus Ramp Viaduct PTFE Fixed Bearing Details" (revision date 4/21/14):</p> <p>1. Sole Plate shows a call out for (20) ASTM A325SC H.S. 3/4" DIA x 8" Stud Bolts. Will we need to drill and tap holes in the Sole Plate to accommodate this type of assembly? If so, please send a detailed drawing showing anchorage requirements for the Sole Plate.</p> <p>2. Section "Hinge H Bearing Details" and section "1 Detail" show different Masonry Plate anchorage details. Please advise correct anchorage details and complete stud layout for the Masonry Plates.</p> <p>3. We plan to use ASTM A108 Headed Studs for any welded anchorage. Please advise if this is acceptable.</p>					
	<p>Drawing S-6252 "Bus Ramp Viaduct PTFE Expansion Bearing Details" (revision date 4/21/14):</p> <p>1. Sole Plate shows a call out for (20) ASTM A325SC H.S. 3/4" DIA x 8" Stud Bolts. Will we need to drill and tap holes in the Sole Plate to accommodate this type of assembly? If so, please send a detailed drawing showing anchorage requirements for the Sole Plate.</p> <p>2. Plans are missing anchorage details showing the complete stud layout for the Masonry Plate. Please provide.</p> <p>3. We plan to use ASTM A 108 Headed Studs for the welded anchorage shown. Please advise if this is acceptable.</p> <p>Drawing S-6253 "Bus Ramp Viaduct PTFE Fixed Bearing Details" (revision date 4/21/14):</p> <p>1. Sole Plate shows a call out for (20) ASTM A325SC H.S. 3/4" DIA x 8" Stud Bolts. Will we need to drill and tap holes in the Sole Plate to accommodate this type of assembly? If so, please send a detailed drawing showing anchorage requirements for the Sole Plate.</p> <p>2. Section "Hinge H Bearing Details" and section "1 Detail" show different Masonry Plate anchorage details. Please advise correct anchorage details and complete stud layout for the Masonry Plates.</p> <p>3. We plan to use ASTM A108 Headed Studs for any welded anchorage. Please advise if this is acceptable.</p>					
B-0063	BRP - Pylon and Barrette Reinforcing Conflict	Closed	01	09/26/2014	10/06/2014	10/13/2014
From: Webcor Construction LP		Claude Titcher				
REQUEST:		ANSWER:				
Please review the attached sketch as it pertains to the				Please review the attached sketch as it pertains to the		



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B-0063.1	<p>Pylon 9 vertical reinforcing and the Barrette vertical reinforcing. Please confirm the length of the horizontal hook on the Pylon vertical bars, normally if the hook length is not specifically called out it is considered to be a standard hook (in this case 1'-10").</p> <p>Additionally, to avoid conflict between the Pylon and Barrette vertical reinforcing and to facilitate the setting of a pre-built Pylon cage we propose to use an HRC-555 on the outside face bundled vertical bar and turn the inside face bundled bar inward as shown in the attachment. Is this acceptable?</p>	Closed	01	11/05/2014	11/15/2014	11/12/2014
	<p><b>BRP - Pylon and Barrette Reinforcing Conflict</b></p> <p><b>From:</b> Webcor Construction LP      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>Please review the attached sketch as it pertains to the Pylon 9 reinforcing and the barrette vertical reinforcing. Please confirm the length of the horizontal hook on the pylon vertical bars. Normally if the hook length is not specifically called out, it is considered to be a standard hook (in the case 1'-10").</p> <p>RFI B-0063 - In the original RFI CMC Rebar asked to use HRC-555 on the vertical bars. In fact, CMC's intent was to request the use of an HRC-555 on the top #11 footing bars that will pass through the Pylon vertical reinforcing. This will facilitate the installation of the top footing bars.</p> <p>Is the acceptable?</p>					
	<p>Pylon 9 vertical reinforcing and the Barrette vertical reinforcing. Please confirm the length of the horizontal hook on the Pylon vertical bars, normally if the hook length is not specifically called out it is considered to be a standard hook (in this case 1'-10").</p> <p>Additionally, to avoid conflict between the Pylon and Barrette vertical reinforcing and to facilitate the setting of a pre-built Pylon cage we propose to use an HRC-555 on the outside face bundled vertical bar and turn the inside face bundled bar inward as shown in the attachment. Is this acceptable?</p>					
	<p><b>ANSWER:</b></p> <p>Please review the attached sketch as it pertains to the Pylon 9 reinforcing and the barrette vertical reinforcing. Please confirm the length of the horizontal hook on the pylon vertical bars. Normally if the hook length is not specifically called out, it is considered to be a standard hook (in the case 1'-10").</p> <p>RFI B-0063 - In the original RFI CMC Rebar asked to use HRC-555 on the vertical bars. In fact, CMC's intent was to request the use of an HRC-555 on the top #11 footing bars that will pass through the Pylon vertical reinforcing. This will facilitate the installation of the top footing bars.</p> <p>Is the acceptable?</p>					



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
<b>B-0064</b>	<b>BRP - Linkbeam Stressing Requirement</b>	<b>Closed</b>	<b>01</b>	<b>09/26/2014</b>	<b>10/06/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference CD S-6070. On sheet S-6070 Pjack is given as 1095 Kips. Is this per tendon?			Please reference CD S-6070. On sheet S-6070 Pjack is given as 1095 Kips. Is this per tendon?			
Please clarfiy			Please clarfiy			





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>B-0066</b>	<b>BRP - Vertical Alignment of CIDH Piles</b>	<b>Closed</b>	<b>01</b>	<b>10/02/2014</b>	<b>10/12/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>						
Section 3.2 A of contract specification section 31 63 30 states "The axis of the drilled hole must not deviate from plumb more than 6 inches per 00feet of length".						
ACI 336 specification for the construction of Drilled Piers states in section 3.1.1, "Out of plumbness of piers shall not exceed 1.5%".						
ACI 117-8 section 3 "Foundations", 3.1 "Vertical Alignment", States in 3.1.1.3, "Category C " for reinforced concrete shafts " not more than 2.0 percent of the shaft length".						
Caltrans 2010 Standard Specification Section 49-3.02C(2) "Drilled Holes" states, "The axis of the drilled hole mist not deviate from plumb more than 1-1/2" per 10 feet of length.						
Will 15" out of plumb vertical alignment over 100 feet of length be acceptable? The Sonic caliber device will be used to verify this requirement.						
<b>ANSWER:</b>						
Section 3.2 A of contract specification section 31 63 30 states "The axis of the drilled hole must not deviate from plumb more than 6 inches per 00feet of length".						
ACI 336 specification for the construction of Drilled Piers states in section 3.1.1, "Out of plumbness of piers shall not exceed 1.5%".						
ACI 117-8 section 3 "Foundations", 3.1 "Vertical Alignment", States in 3.1.1.3, "Category C " for reinforced concrete shafts " not more than 2.0 percent of the shaft length".						
Caltrans 2010 Standard Specification Section 49-3.02C(2) "Drilled Holes" states, "The axis of the drilled hole mist not deviate from plumb more than 1-1/2" per 10 feet of length.						
Will 15" out of plumb vertical alignment over 100 feet of length be acceptable? The Sonic caliber device will be used to verify this requirement.						
<b>B-0067</b>	<b>BRP - CIDH Pile Gamma Gamma Inspection Tubes</b>	<b>Closed</b>	<b>01</b>	<b>10/02/2014</b>	<b>10/12/2014</b>	<b>10/07/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>						
Caltran's Foundation Testing Branch has calibrated their gamma gamma testing probes for perforning testing with water in the inspection pipes.						
Will the PVC inspection pipes need to be dewatered at the time of testing for the selected testing agency?						
<b>ANSWER:</b>						
Caltran's Foundation Testing Branch has calibrated their gamma gamma testing probes for perforning testing with water in the inspection pipes.						
Will the PVC inspection pipes need to be dewatered at the time of testing for the selected testing agency?						
<b>B-0068</b>	<b>BRP 732 MOD Concrete Barrier Form Liner</b>	<b>Closed</b>	<b>01</b>	<b>10/02/2014</b>	<b>10/12/2014</b>	<b>10/09/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>						
Based on Section A of Contract Drawing S-6400, the 732 Mod Concrete Barrier has indentations in the concrete,						
<b>ANSWER:</b>						
Based on Section A of Contract Drawing S-6400, the 732 Mod Concrete Barrier has indentations in the						



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
	<p>with the bottom line of the indentation showing to have a ½" draft. The same detail shows the other indentation lines being square which would not be able to be stripped. Attached is a drawing for a proposed form liner for the Barrier that is to be installed showing the same ½" draft, which is on the bottom of the indentation, all the way around the recess opening. This 1/2" draft around will allow the form liner to be stripped.</p> <p>Is this acceptable?</p>					
<b>B-0069</b>	<b>BRP - OCS Pole Relocation</b>	<b>Closed</b>	<b>01</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>11/13/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>See the attached sketch of the proposed OCS pole relocation per SCCI's field walk with Turner, MUNI, and WOJV on 10/2/2014. Please provide direction on scope with design documents and specifications for construction.</p>					
						<p><b>ANSWER:</b></p> <p>See the attached sketch of the proposed OCS pole relocation per SCCI's field walk with Turner, MUNI, and WOJV on 10/2/2014. Please provide direction on scope with design documents and specifications for construction.</p>
<b>B-0070</b>	<b>BRP - Clarification of Section Detail on S-6705</b>	<b>Closed</b>	<b>01</b>	<b>10/09/2014</b>	<b>10/19/2014</b>	<b>10/15/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>Drawing S-2043, Retaining Wall 3 - Plan and Elevation, calls out the new slope paving section detail on drawing S-6705. Please clarify how this section detail relates to the paving slope on Retaining Wall 3, drawing S-2043.</p> <p>If the section detail shown on S-6705 is not intended for Retaining Wall 3, please clarify where the section is applicable (as no other retaining walls call out the section detail on S-6705).</p>					
						<p><b>ANSWER:</b></p> <p>Drawing S-2043, Retaining Wall 3 - Plan and Elevation, calls out the new slope paving section detail on drawing S-6705. Please clarify how this section detail relates to the paving slope on Retaining Wall 3, drawing S-2043.</p> <p>If the section detail shown on S-6705 is not intended for Retaining Wall 3, please clarify where the section is applicable (as no other retaining walls call out the section detail on S-6705).</p>
<b>B-0071</b>	<b>BRP - Clarification Striping Detail C-7002</b>	<b>Closed</b>	<b>01</b>	<b>10/13/2014</b>	<b>10/13/2014</b>	<b>10/15/2014</b>



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<hr/>						
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Refer to attached drawings C-7002. Please advise where striping callout #40 ends and striping callout #9 begins and ends.			Refer to attached drawings C-7002. Please advise where striping callout #40 ends and striping callout #9 begins and ends.			
<b>B-0072</b>	<b>BRP - Clarification of Specification Section 31 63 32</b>	<b>Closed</b>	<b>01</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/15/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Section 3.4-B-1 of Contract Specification Section 31 63 32, States:			Section 3.4-B-1 of Contract Specification Section 31 63 32, States:			
"Do not begin excavation of the barrette piles until geotechnical instrumentation for monitoring ground movements, other than that to be installed within the barrette pile itself, is installed and stabilized baseline reading sets have been taken."			"Do not begin excavation of the barrette piles until geotechnical instrumentation for monitoring ground movements, other than that to be installed within the barrette pile itself, is installed and stabilized baseline reading sets have been taken."			
Please provide clarification as to what geotechnical instrumentation outside of the barrette pile has been installed and who is monitoring it. If baseline readings have been established, please provide them.			Please provide clarification as to what geotechnical instrumentation outside of the barrette pile has been installed and who is monitoring it. If baseline readings have been established, please provide them.			
Additionally, please provide clarification as to what geotechnical instrumentation is to be installed within the barrette pile to monitor ground movement.			Additionally, please provide clarification as to what geotechnical instrumentation is to be installed within the barrette pile to monitor ground movement.			
<hr/>						
<b>B-0073</b>	<b>BRP - HDPE Pipe Specification</b>	<b>Closed</b>	<b>01</b>	<b>10/15/2014</b>	<b>10/25/2014</b>	<b>10/21/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
SCCI has been unable to find any specifications for the HDPE pipe in drawings C4300-4302. Please see submittal #TG1801-621 regarding SCCI's proposed HDPE pipe. Is this acceptable?			SCCI has been unable to find any specifications for the HDPE pipe in drawings C4300-4302. Please see submittal #TG1801-621 regarding SCCI's proposed HDPE pipe. Is this acceptable?			





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<b>B-0074</b>	<b>BRP - Cable Stay Bridge Jacking Forces</b>	<b>Closed</b>	<b>01</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>11/04/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
SCCI has received the following RFI from OPAC Engineers:		SCCI has received the following RFI from OPAC Engineers:				
This bridge, like all cable-stayed bridges, is a highly redundant structure and there is an infinite number of combinations of dead load superstructure moments, superstructure axial load, cable forces, etc. It is understood that the designer has selected an optimal dead load/cable jacking force combination for the design of the bridge based on the designer's suggested construction sequence. Since OPAC will follow the designer's suggested construction sequence in general, it will be helpful for the designer to provide the jacking forces at each stage that will produce the final cable forces.		This bridge, like all cable-stayed bridges, is a highly redundant structure and there is an infinite number of combinations of dead load superstructure moments, superstructure axial load, cable forces, etc. It is understood that the designer has selected an optimal dead load/cable jacking force combination for the design of the bridge based on the designer's suggested construction sequence. Since OPAC will follow the designer's suggested construction sequence in general, it will be helpful for the designer to provide the jacking forces at each stage that will produce the final cable forces.				
Please advise.		Please advise.				
<b>B-0075</b>	<b>BRP - Fremont Off-Ramp Overhead Sign Removal</b>	<b>Closed</b>	<b>01</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/29/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
SCCI will be removing the sign shown contract drawing D-1007, C-7000, and C-7004. SCCI does not intend on using any temporary signage in the interim while the new sign is being constructed since there is already sufficient signage for the off-ramp (see attached photo). Is this acceptable?		SCCI will be removing the sign shown contract drawing D-1007, C-7000, and C-7004. SCCI does not intend on using any temporary signage in the interim while the new sign is being constructed since there is already sufficient signage for the off-ramp (see attached photo). Is this acceptable?				
<b>B-0076</b>	<b>BRP - Conduit Type In Roadway Cells</b>	<b>Closed</b>	<b>01</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/29/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Sheet E-5005 Detail 1 shows EMT conduit inside the roadway deck while the spec section 26 05 36 Part 3.3C calls out for galvanized rigid steel conduit. Please confirm that it is acceptable to use EMT in roadway cells.		Sheet E-5005 Detail 1 shows EMT conduit inside the roadway deck while the spec section 26 05 36 Part 3.3C calls out for galvanized rigid steel conduit. Please confirm that it is acceptable to use EMT in roadway cells.				



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<b>B-0077</b>	<b>BRP - Catch Basin #3 Invert Elevation Discrepancy</b>	<b>Closed</b>	<b>01</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>11/05/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> <p>There is a discrepancy between the slope of the pipe and the invert elevation of CB#2 in drawing C- 4300. CB#2 invert elevation is 89.52 which is downstream of CB#1, yet the invert elevation is higher than CB#1. SCCI intends to use an invert elevation of 88.83' for CB#2 so that this slope is maintained.</p> <p>Is this acceptable?</p>						
<b>ANSWER:</b> <p>There is a discrepancy between the slope of the pipe and the invert elevation of CB#2 in drawing C- 4300. CB#2 invert elevation is 89.52 which is downstream of CB#1, yet the invert elevation is higher than CB#1. SCCI intends to use an invert elevation of 88.83' for CB#2 so that this slope is maintained.</p> <p>Is this acceptable?</p>						
<b>B-0078</b>	<b>BRP - Correct PTI Edition</b>	<b>Closed</b>	<b>01</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>11/04/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> <p>VSL is requesting clarification on the correct edition of the "Recommendations for Stay Cable Design, Testing, and Installation" published by the Post Tensioning Institute (PTI) to be used on this project.</p> <p>On drawings S-0005 (71 of 470) dated October 1, 2013, the contract drawing refers to the "Recommendations for Stay Cable Design, Testing, and Installation" Fifth Edition (2007) by the Post Tensioning Institute (PTI) as the basis for design of the bridge.</p> <p>On page 05-16-33-1, section 05-16-1.2-C-1, of the project specification, dated October 1, 2013, the specifications specify the "Latest edition issued" be the standard for the project. The latest edition of the "Recommendations for Stay Cable Design, Testing, and Installation" published by PTI is the Sixth Edition dated May 2012.</p> <p>Please clarify which edition of the "Recommendations for Stay Cable Design, Testing, and Installation" as published by PTI should be used on this project.</p>						
<b>ANSWER:</b> <p>VSL is requesting clarification on the correct edition of the "Recommendations for Stay Cable Design, Testing, and Installation" published by the Post Tensioning Institute (PTI) to be used on this project.</p> <p>On drawings S-0005 (71 of 470) dated October 1, 2013, the contract drawing refers to the "Recommendations for Stay Cable Design, Testing, and Installation" Fifth Edition (2007) by the Post Tensioning Institute (PTI) as the basis for design of the bridge.</p> <p>On page 05-16-33-1, section 05-16-1.2-C-1, of the project specification, dated October 1, 2013, the specifications specify the "Latest edition issued" be the standard for the project. The latest edition of the "Recommendations for Stay Cable Design, Testing, and Installation" published by PTI is the Sixth Edition dated May 2012.</p> <p>Please clarify which edition of the "Recommendations for Stay Cable Design, Testing, and Installation" as published by PTI should be used on this project.</p>						



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<b>B-0079</b>	<b>BRP - AWSS Submittals</b>	<b>Closed</b>	<b>01</b>	<b>10/24/2014</b>	<b>11/03/2014</b>	<b>10/27/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
As confirmed by SFDPW at the AWSS meeting, on 10/23/14, with Synergy Project Management (AWSS subcontractor), SCCI, WOJV, TCCO and SFDPW, submittals are not required for the following items:			As confirmed by SFDPW at the AWSS meeting, on 10/23/14, with Synergy Project Management (AWSS subcontractor), SCCI, WOJV, TCCO and SFDPW, submittals are not required for the following items:			
AWSS Dewatering Plan AWSS Valves and Fittings (also see attached email)			AWSS Dewatering Plan AWSS Valves and Fittings (also see attached email)			
Please confirm.			Please confirm.			



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<b>B-0080</b>	<b>BRP - Anti Ram Barrier Specification</b>	<b>Closed</b>	<b>01</b>	<b>10/28/2014</b>	<b>11/07/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
SENSITIVE SECURITY INFORMATION		SENSITIVE SECURITY INFORMATION				
Please see below questions from Shimmick's Anti-Ram Barrier subcontractor, FutureNet.		Please see below questions from Shimmick's Anti-Ram Barrier subcontractor, FutureNet.				
Question 1 Spec Section 28 16 34, 2.1 A 3 states "the design and structural materials of the vehicle barrier shall be the same as those used in the crash test."		Question 1 Spec Section 28 16 34, 2.1 A 3 states "the design and structural materials of the vehicle barrier shall be the same as those used in the crash test."				
Discrepancy: The original crash tested barriers are installed in dirt for a permanent in-ground installation. This project requires the barrier be installed into a bridge section. The barrier foundation will need to be modified to fit into and react with the bridge section. Dirt reacts differently than a concrete bridge section during impact and the bridge section has space restrictions. Therefore we need to provide a foundation design that varies from the original crash tested design for both fit and function.		Discrepancy: The original crash tested barriers are installed in dirt for a permanent in-ground installation. This project requires the barrier be installed into a bridge section. The barrier foundation will need to be modified to fit into and react with the bridge section. Dirt reacts differently than a concrete bridge section during impact and the bridge section has space restrictions. Therefore we need to provide a foundation design that varies from the original crash tested design for both fit and function.				
Proposed Solution: Allow a modified foundation design, proven to absorb the minimum force via engineering calculations.		Proposed Solution: Allow a modified foundation design, proven to absorb the minimum force via engineering calculations.				
Is this acceptable?		Is this acceptable?				
Question 2 Spec Section 28 16 34, 2.1 B. I. states "the barrier shall fail when subjected to the maximum input force."		Question 2 Spec Section 28 16 34, 2.1 B. I. states "the barrier shall fail when subjected to the maximum input force."				
Discrepancy: Anti-ram barriers are not designed to fail. The industry designs to protect against a defined force and to meet or exceed that rating. No barrier company has defined a systems to fail completely. If the barrier absolutely must fail and allow a vehicle access, then we are back to discrepancy #1 which causes a variation from the barrier design as originally crash tested.		Discrepancy: Anti-ram barriers are not designed to fail. The industry designs to protect against a defined force and to meet or exceed that rating. No barrier company has defined a systems to fail completely. If the barrier absolutely must fail and allow a vehicle access, then we are back to discrepancy #1 which causes a variation from the barrier design as originally crash tested.				
Proposed Solution: Remove reference to barrier failure (allowing vehicle access above x weight and speed). The GRAB-300 system currently uses shear pins and energy absorption devises which may mitigate the requirement for barrier failure.		Proposed Solution: Remove reference to barrier failure (allowing vehicle access above x weight and speed). The GRAB-300 system currently uses shear pins and energy absorption devises which may mitigate the requirement for barrier failure.				
Is this acceptable?		Is this acceptable?				



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	<p>For reference, please see attached cut sheet and sample drawing of a 48' GRAB-300 with standard foundation. TG 18.1 requires a 52' unit (drawings for this size are not readily available for this RFI), the foundations are identical except wider in the middle. The GRAB-300 foundation needs to be trimmed on the outside to fit the bridge section.</p> <p>WARNING: This record contains Sensitive Security Information that is controlled under 49 C.F.R. parts 15 and 1520. No part of this record may be disclosed to persons without a "need to know", as defined in 49 C.F.R. parts 15 and 1520, except with the written permission of the Administrator of the Transportation Security Administration or the Secretary of Transportation. Unauthorized release may result in civil penalty or other action. For U.S. government agencies, public disclosure is governed by 5 U.S.C. Section 552 and 49 C.F.R. parts 15 and 1520.</p>					
	<p>Is this acceptable?</p> <p>For reference, please see attached cut sheet and sample drawing of a 48' GRAB-300 with standard foundation. TG 18.1 requires a 52' unit (drawings for this size are not readily available for this RFI), the foundations are identical except wider in the middle. The GRAB-300 foundation needs to be trimmed on the outside to fit the bridge section.</p> <p>WARNING: This record contains Sensitive Security Information that is controlled under 49 C.F.R. parts 15 and 1520. No part of this record may be disclosed to persons without a "need to know", as defined in 49 C.F.R. parts 15 and 1520, except with the written permission of the Administrator of the Transportation Security Administration or the Secretary of Transportation. Unauthorized release may result in civil penalty or other action. For U.S. government agencies, public disclosure is governed by 5 U.S.C. Section 552 and 49 C.F.R. parts 15 and 1520.</p>					
<b>B-0081</b>	<b>BRP - Contract Drawing S-2053 Not Provided</b>	<b>Closed</b>	<b>01</b>	<b>10/28/2014</b>	<b>11/07/2014</b>	<b>11/06/2014</b>
<div><div>From: Webcor Construction LP</div><div>Claude Titche</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Drawing S-2040 provides reference to drawing S-2053 for Retaining Wall 10 details but SCCI is not in receipt of drawing S-2053. Please provide this drawing if it exists.			Contract Drawing S-2040 provides reference to drawing S-2053 for Retaining Wall 10 details but SCCI is not in receipt of drawing S-2053. Please provide this drawing if it exists.			



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<b>B-0082</b>	<b>BRP - Removal of Concrete Structure in Lot A</b>	<b>Closed</b>	<b>01</b>	<b>10/29/2014</b>	<b>11/08/2014</b>	<b>11/04/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  Offhaul of hazardous material and excavation around existing foundations in Lot A have exposed what appears to be a spread footing for a previously demolished concrete wall. The concrete is 2'-0" wide by 1'-6" tall and 85'-0" long with rebar extending vertically out of it. SCCI has checked all reference drawings and was unable to determine what this structure was and when it was built. This concrete will interfere with the future installation of Retaining Wall 10. SCCI proposes to demolish this concrete structure and bill towards Bid Item 25 Demolition/Removal of Concrete Foundations.  Is this acceptable?						
						<b>ANSWER:</b>  Offhaul of hazardous material and excavation around existing foundations in Lot A have exposed what appears to be a spread footing for a previously demolished concrete wall. The concrete is 2'-0" wide by 1'-6" tall and 85'-0" long with rebar extending vertically out of it. SCCI has checked all reference drawings and was unable to determine what this structure was and when it was built. This concrete will interfere with the future installation of Retaining Wall 10. SCCI proposes to demolish this concrete structure and bill towards Bid Item 25 Demolition/Removal of Concrete Foundations.  Is this acceptable?
<b>B-0083</b>	<b>BRP - Overhead Sign Foundation Demo</b>	<b>Closed</b>	<b>01</b>	<b>10/29/2014</b>	<b>11/08/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  Per the note on D-1007, the overhead sign square pedestal post foundation is to be removed. It is SC CI's understanding that the grout pad and bolts are to be removed and the concrete corbel is to remain intact.  Please confirm.  Note: The West side will be removed as part of the Off Ramp Demo, the East side is in question.						
						<b>ANSWER:</b>  Per the note on D-1007, the overhead sign square pedestal post foundation is to be removed. It is SC CI's understanding that the grout pad and bolts are to be removed and the concrete corbel is to remain intact.  Please confirm.  Note: The West side will be removed as part of the Off Ramp Demo, the East side is in question.
<b>B-0084</b>	<b>BRP - Barrette Pile Concrete Allowable Delivery Time</b>	<b>Closed</b>	<b>01</b>	<b>10/31/2014</b>	<b>11/10/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  Section 2. 7 C 6 c. of Specification Section 03 05 15 Portland Cement Concrete - Bus Ramps states that concrete that uses an admixture to retard the set time has a time limit from batch to placement of two hours. The mix design for the concrete to be used in the						
						<b>ANSWER:</b>  Section 2. 7 C 6 c. of Specification Section 03 05 15 Portland Cement Concrete - Bus Ramps states that concrete that uses an admixture to retard the set time has a time limit from batch to placement of two hours. The mix design for the concrete to be used in the



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	<p>Barrette Piles uses admixtures that ensure that the slump is greater than seven inches up to eight hours from batch time. Therefore, SCCI proposes to revise the allowed time from batch to placement to three hours. This would apply to approved Barrette Pile mix designs only. Is this acceptable?</p>					
	<p>the Barrette Piles uses admixtures that ensure that the slump is greater than seven inches up to eight hours from batch time. Therefore, SCCI proposes to revise the allowed time from batch to placement to three hours. This would apply to approved Barrette Pile mix designs only. Is this acceptable?</p>					
<b>B-0085</b>	<b>BRP - CIDH Pile Concrete Allowable Delivery Time</b>	<b>Closed</b>	<b>01</b>	<b>10/31/2014</b>	<b>11/10/2014</b>	<b>11/17/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Claude Titche</p> <p><b>REQUEST:</b></p> <p>Section 2. 7 C 6 c. of Specification Section 03 05 15 Portland Cement Concrete - Bus Ramps states that concrete that uses an admixture to retard the set time has a time limit from batch to placement of two hours. The mix design for the concrete to be used in the Cast-In-Drilled-Hole Piles uses admixtures that ensure that the slump is greater than seven inches up to eight hours from batch time. Therefore, SCCI proposes to revise the allowed time from batch to placement to three hours. This would apply to currently approved CIDH mix designs only. Is this acceptable?</p>					
	<p><b>ANSWER:</b></p> <p>Section 2. 7 C 6 c. of Specification Section 03 05 15 Portland Cement Concrete - Bus Ramps states that concrete that uses an admixture to retard the set time has a time limit from batch to placement of two hours. The mix design for the concrete to be used in the Cast-In-Drilled-Hole Piles uses admixtures that ensure that the slump is greater than seven inches up to eight hours from batch time. Therefore, SCCI proposes to revise the allowed time from batch to placement to three hours. This would apply to currently approved CIDH mix designs only. Is this acceptable?</p>					



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<b>B-0086</b>	<b>BRP - AWSS Pipe Sample Submittal</b>	<b>Closed</b>	<b>01</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/12/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						
Spec Section 02723, 3.28, A States:						
"The Contractor's Independent Testing Laboratory shall inspect and approve in writing the welds on two (2) test welded collar stops".						
Following the AWSS meeting on 10/23/14, with Synergy Project Management (AWSS subcontractor), SCCI, WOJV, TCCO and SFDPW, it was confirmed that the only inspection required by SFDPW on the Sample Collar Stops (Submittal TG 1801-103) was the SF Water Department (SFWD) inspection. It was also confirmed that all SFWD inspections were to be requested through the BIM 360 IR process.						
Please confirm no further inspection is required on Submittal TG 1801-103 and required SFWD testing will be arranged through the TJPA representative.						
<b>ANSWER:</b>						
Spec Section 02723, 3.28, A States:						
"The Contractor's Independent Testing Laboratory shall inspect and approve in writing the welds on two (2) test welded collar stops".						
Following the AWSS meeting on 10/23/14, with Synergy Project Management (AWSS subcontractor), SCCI, WOJV, TCCO and SFDPW, it was confirmed that the only inspection required by SFDPW on the Sample Collar Stops (Submittal TG 1801-103) was the SF Water Department (SFWD) inspection. It was also confirmed that all SFWD inspections were to be requested through the BIM 360 IR process.						
Please confirm no further inspection is required on Submittal TG 1801-103 and required SFWD testing will be arranged through the TJPA representative.						
<b>B-0087</b>	<b>BRP - Fremont Off-Ramp Closure TCP Clarification</b>	<b>Closed</b>	<b>01</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						
As discussed, between CalTrans and CMC Traffic, on 10/31 /14 regarding TCP B1 (Fremont Off-Ramp Closure) of the Traffic Management Plan, the 84" wide SC7 sign will be relocated inside the lane closure as it is too wide to be installed prior to the cone taper. 48"x48" roll up signs will be used on the 2' shoulder of the bay bridge and only 2 legs of the sign stand will be deployed. Tie-wire will be used to attach the stand to the vertical cables of the bridge.						
Please confirm this is acceptable.						
<b>ANSWER:</b>						
As discussed, between CalTrans and CMC Traffic, on 10/31 /14 regarding TCP B1 (Fremont Off-Ramp Closure) of the Traffic Management Plan, the 84" wide SC7 sign will be relocated inside the lane closure as it is too wide to be installed prior to the cone taper. 48"x48" roll up signs will be used on the 2' shoulder of the bay bridge and only 2 legs of the sign stand will be deployed. Tie-wire will be used to attach the stand to the vertical cables of the bridge.						
Please confirm this is acceptable.						
<b>B-0088</b>	<b>BRP - Unknown Foundation in Lot A</b>	<b>Closed</b>	<b>01</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/11/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						
Please see the attached photograph and sketch of an						
<b>ANSWER:</b>						
Please see the attached photograph and sketch of an						





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	<p>unknown concrete foundation in Lot A. It is between existing Bent G and 303 Second. SCCI has checked all reference drawings and was unable to determine what this structure is and when it was built. This concrete will interfere with the future installation of Retaining Wall 9. SCCI proposes to demolish this concrete structure and bill towards Bid Item 25 Demolition/Removal of Concrete Foundations.</p> <p>Is this acceptable?</p>					
<b>B-0089</b>	<b>BRP - Barrette Pile Initial Slump Range</b>	<b>Closed</b>	<b>01</b>	<b>11/05/2014</b>	<b>11/15/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Approved Submittal TG 1801-302.1 indicates the structural mix design planned to be used for the barrettes. The initial slump indicated is 9" ( + 1"). Findings on several similar jobsites indicated that an initial slump 8" ( + 1 ") improves concrete stability (less bleed and decreased risks of segregation) without significantly impacting concrete placement or slump retention.  Please confirm an initial slump of 8" ( + 1 ") is acceptable.  Note that the mix design will remain the same, only the initial slump requirement will be adjusted (see attached).						
<b>ANSWER:</b>  Approved Submittal TG 1801-302.1 indicates the structural mix design planned to be used for the barrettes. The initial slump indicated is 9" ( + 1"). Findings on several similar jobsites indicated that an initial slump 8" ( + 1 ") improves concrete stability (less bleed and decreased risks of segregation) without significantly impacting concrete placement or slump retention.  Please confirm an initial slump of 8" ( + 1 ") is acceptable.  Note that the mix design will remain the same, only the initial slump requirement will be adjusted (see attached).						



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<b>B-0090</b>	<b>BRP - Fremont Off Ramp Overhead Sign Pedestal Removal</b>	<b>Closed</b>	<b>01</b>	<b>11/05/2014</b>	<b>11/15/2014</b>	<b>11/12/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Drawing D-1007 states that the removal of the overhead sign truss on the Fremont Street Off Ramp is to be removed, including the "square pedestal post foundation". SCCI interprets the square pedestal post foundation to be the square block of concrete on top of the larger beam that extends from the bridge deck, as shown on the attached drawing. SCCI believes that this beam is intended to remain as Evans Brothers' experience has shown that these beams are tied into the bridge structure. Is this correct?			Contract Drawing D-1007 states that the removal of the overhead sign truss on the Fremont Street Off Ramp is to be removed, including the "square pedestal post foundation". SCCI interprets the square pedestal post foundation to be the square block of concrete on top of the larger beam that extends from the bridge deck, as shown on the attached drawing. SCCI believes that this beam is intended to remain as Evans Brothers' experience has shown that these beams are tied into the bridge structure. Is this correct?			
<b>B-0091</b>	<b>BRP - Fremont Off Ramp Limits of Demolition</b>	<b>Closed</b>	<b>01</b>	<b>11/05/2014</b>	<b>11/15/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Drawings D-1003 and D-1007 show the limits of removal of the existing Fremont Street Bus Off Ramp. SCCI would like to confirm the extent of the demolition off the off ramps concrete barrier at the limit of the demolition. SCCI believes that the concrete barrier should be removed to the limits shown on the attached picture as existing cracks in the concrete could lead to spalling if this section is not completely removed.  Please confirm.			Contract Drawings D-1003 and D-1007 show the limits of removal of the existing Fremont Street Bus Off Ramp. SCCI would like to confirm the extent of the demolition off the off ramps concrete barrier at the limit of the demolition. SCCI believes that the concrete barrier should be removed to the limits shown on the attached picture as existing cracks in the concrete could lead to spalling if this section is not completely removed.  Please confirm.			
<b>B-0092</b>	<b>BRP - Proposed Modification to Welded Barrette Pile Reinforcement</b>	<b>Closed</b>	<b>01</b>	<b>11/06/2014</b>	<b>11/16/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
A Quality Control inspection was performed by Nicholson on the reinforcement cage of Barrette PI. During the inspection, it was observed that 2 ea. 3"x3"x0.25" angles (which are part of the assembly to pick the cage) are welded to a total of 4 ea. #10 structural bars (2 bars on each angle).  After a discussion with ARUP, Nicholson proposes to			A Quality Control inspection was performed by Nicholson on the reinforcement cage of Barrette PI. During the inspection, it was observed that 2 ea. 3"x3"x0.25" angles (which are part of the assembly to pick the cage) are welded to a total of 4 ea. #10 structural bars (2 bars on each angle).  After a discussion with ARUP, Nicholson proposes to			

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	<p>install 4 ea. additional #10 bars 13' long to lap splice the structural bars affected by the weld. Those additional #10 bars will be centered on the weld location and installed before the Barrette is poured (see attached sketch).</p> <p>Alternatively, if the additional #10 bars cannot be supplied in time, Nicholson proposes to install Lenton mechanical couplers at the weld location.</p> <p>Please confirm this modification is acceptable.</p>					
<b>B-0093</b>	<b>BRP - Pylon 9 Coordinates</b>	<b>Closed</b>	<b>01</b>	<b>11/06/2014</b>	<b>11/16/2014</b>	<b>11/13/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Claude Titche</p> <p><b>REQUEST:</b></p> <p>In the response to RFI B-0057, the Northing and Easting of the pylon anchors were changed from 21 14752.046, 6013323.549 to 2114752.130, 6013323.580. Has the pylon centerline also moved to 2114752.130, 6013323.580, or are the saddles offset from pylon centerline?</p>					
	<p>install 4 ea. additional #10 bars 13' long to lap splice the structural bars affected by the weld. Those additional #10 bars will be centered on the weld location and installed before the Barrette is poured (see attached sketch).</p> <p>Alternatively, if the additional #10 bars cannot be supplied in time, Nicholson proposes to install Lenton mechanical couplers at the weld location.</p> <p>Please confirm this modification is acceptable.</p>					
	<p><b>ANSWER:</b></p> <p>In the response to RFI B-0057, the Northing and Easting of the pylon anchors were changed from 21 14752.046, 6013323.549 to 2114752.130, 6013323.580. Has the pylon centerline also moved to 2114752.130, 6013323.580, or are the saddles offset from pylon centerline?</p>					



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>B-0094</b>	<b>BRP - Circular Hoops for Pylon 9</b>	<b>Closed</b>	<b>01</b>	<b>11/10/2014</b>	<b>11/20/2014</b>	<b>11/19/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The configuration of the #8 "ultimate spliced" hoops shown in the referenced sections as well as the varying dimensions of the Pylon present a challenge in both the fabrication as well as the splicing of these bars.			The configuration of the #8 "ultimate spliced" hoops shown in the referenced sections as well as the varying dimensions of the Pylon present a challenge in both the fabrication as well as the splicing of these bars.			
SCCI suggest the use of circular "ultimate spliced" hoop, interlocking when section permits and single above that, at the required spacing.			SCCI suggest the use of circular "ultimate spliced" hoop, interlocking when section permits and single above that, at the required spacing.			
Attached is preliminary drawings which provides sections at various vertical locations.			Attached is preliminary drawings which provides sections at various vertical locations.			
Additionally, #5@12 skin reinforcing would be added to accommodate the increased clearance created by the circular hoops.			Additionally, #5@12 skin reinforcing would be added to accommodate the increased clearance created by the circular hoops.			
Is this acceptable?			Is this acceptable?			
<b>B-0095</b>	<b>BRP - Pylon 9 - Vertical Reinforcing Clarification</b>	<b>Closed</b>	<b>01</b>	<b>11/10/2014</b>	<b>11/20/2014</b>	<b>12/04/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Sections A thru D/S-3178 call for #10 vertical bars. Sections E, F/S-3179 call for #11 vertical bars. Detail G/S-3180 calls for #11 vertical bars. Detail 1/S-3180 calls for #10 and #11 vertical bars. Please clarify the vertical reinforcing as we cannot define the limits of the difference size bars called out.			Sections A thru D/S-3178 call for #10 vertical bars. Sections E, F/S-3179 call for #11 vertical bars. Detail G/S-3180 calls for #11 vertical bars. Detail 1/S-3180 calls for #10 and #11 vertical bars. Please clarify the vertical reinforcing as we cannot define the limits of the difference size bars called out.			
<b>B-0096</b>	<b>BRP - Viaduct Bent Rebar - Section Limits</b>	<b>Closed</b>	<b>01</b>	<b>11/10/2014</b>	<b>11/20/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference CD S-3170 and the attached RFI From CMC Rebar. Please confirm that there are 5 sets of cross ties at the footing as well as at the bent cap.			Please reference CD S-3170 and the attached RFI From CMC Rebar. Please confirm that there are 5 sets of cross ties at the footing as well as at the bent cap.			



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<b>B-0097</b>	<b>BRP - Signal Control Box Conflict</b>	<b>Closed</b>	<b>01</b>	<b>11/11/2014</b>	<b>11/21/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Please see the attached sketches. Per the contract drawings, a signal control box located at the NW corner of Harrison and Essex is 2' away from the retaining wall and is noted "to remain." The actual location is only 1' away which is in conflict with the footing of the retaining wall to be demolished.  Please provide direction on how to proceed.						<b>ANSWER:</b>  Please see the attached sketches. Per the contract drawings, a signal control box located at the NW corner of Harrison and Essex is 2' away from the retaining wall and is noted "to remain." The actual location is only 1' away which is in conflict with the footing of the retaining wall to be demolished.  Please provide direction on how to proceed.
<b>B-0098</b>	<b>BRP - Traffic Light Relocation</b>	<b>Closed</b>	<b>01</b>	<b>11/11/2014</b>	<b>11/21/2014</b>	<b>11/13/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Please see attached schematic showing the locations of street lights and poles that will be in the way of the new structure and falsework. Per contract drawings U-1003 through U-1006, SCCI is instructed to relocate one street light and deenergize and demolish two street lights. The rest of are not shown on the contract drawings or include a note stating "street light removed." Please provide direction on how to proceed with the removal/relocation of these street lights not shown on the contract drawings and those that SCCI assumed were already removed.						<b>ANSWER:</b>  Please see attached schematic showing the locations of street lights and poles that will be in the way of the new structure and falsework. Per contract drawings U-1003 through U-1006, SCCI is instructed to relocate one street light and deenergize and demolish two street lights. The rest of are not shown on the contract drawings or include a note stating "street light removed." Please provide direction on how to proceed with the removal/relocation of these street lights not shown on the contract drawings and those that SCCI assumed were already removed.
<b>B-0099</b>	<b>BRP - Additional Pylon Dimensions</b>	<b>Closed</b>	<b>01</b>	<b>11/12/2014</b>	<b>11/22/2014</b>	<b>11/25/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Please see attached preliminary drawings of cable saddles from VSL. VSL needs confirmation on the blister dimensions as well as the work points. Also, any definition of pylon dimensions would be helpful. currently, the only pylon dimensions that we have is shown on Drawing S-3173.						<b>ANSWER:</b>  Please see attached preliminary drawings of cable saddles from VSL. VSL needs confirmation on the blister dimensions as well as the work points. Also, any definition of pylon dimensions would be helpful. currently, the only pylon dimensions that we have is shown on Drawing S-3173.
<b>B-0100</b>	<b>BRP - Additional Pylon Geometry</b>	<b>Closed</b>	<b>01</b>	<b>11/12/2014</b>	<b>11/22/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						



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<b>B-0103</b>	<b>BRP - Stainless Steel Convex Plates</b>	<b>Closed</b>	<b>01</b>	<b>11/13/2014</b>	<b>11/23/2014</b>	<b>12/01/2014</b>
<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Per section 03 15 16, the Convex Plates for the PTFE Bearings must comply with ASTM A240, Type 304 stainless steel. The maximum thickness of convex plates at Hinge 9 and Hinge H are 3" and 3.5" respectively. This thickness is not available in Domestic ASTM A240 Type 304 stainless steel plates. Please advise if another specification is acceptable for the convex plates at Hinge 9 and Hinge H (ie ASTM A276 stainless steel). Please note: If ASTM A240 is required for the convex plates at Hinge 9 and Hinge H, WOJV will issue a 'Buy America' substitution request.</p>			<p>Per section 03 15 16, the Convex Plates for the PTFE Bearings must comply with ASTM A240, Type 304 stainless steel. The maximum thickness of convex plates at Hinge 9 and Hinge H are 3" and 3.5" respectively. This thickness is not available in Domestic ASTM A240 Type 304 stainless steel plates. Please advise if another specification is acceptable for the convex plates at Hinge 9 and Hinge H (ie ASTM A276 stainless steel). Please note: If ASTM A240 is required for the convex plates at Hinge 9 and Hinge H, WOJV will issue a 'Buy America' substitution request.</p>			
<b>B-0104</b>	<b>BRP - Stay Cable Forces</b>	<b>Closed</b>	<b>01</b>	<b>11/17/2014</b>	<b>11/27/2014</b>	<b>12/01/2014</b>
<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Per the conference call between OPAC and Arup on 11/13/2014 to discuss OPAC's current stage-bystage time dependent erection analysis, OPAC finds slightly higher final stay cable forces than shown on drawing S-6097, about 10% to 15% higher. Please advise on which of the following cable tuning options is preferable?</p> <ol style="list-style-type: none"> <li>1. The cables tuned such that, although the forces are higher, they result in a similar pattern as S-6097.</li> <li>2. The cables tuned such that they all have approximately the same force, say approximately 12% of the ultimate strength.</li> </ol>			<p>Per the conference call between OPAC and Arup on 11/13/2014 to discuss OPAC's current stage-bystage time dependent erection analysis, OPAC finds slightly higher final stay cable forces than shown on drawing S-6097, about 10% to 15% higher. Please advise on which of the following cable tuning options is preferable?</p> <ol style="list-style-type: none"> <li>1. The cables tuned such that, although the forces are higher, they result in a similar pattern as S-6097.</li> <li>2. The cables tuned such that they all have approximately the same force, say approximately 12% of the ultimate strength.</li> </ol>			
<b>B-0105</b>	<b>BRP-Pylon 9 Footing</b>	<b>Closed</b>	<b>01</b>	<b>11/18/2014</b>	<b>11/28/2014</b>	<b>11/25/2014</b>
<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>See attached marked up drawings of the Pylon 9 footing shoring. SCCI is proposing to pour the (N) Pylon 9 footing against the temporary shoring wall, as shown on the attached drawings. A plastic sheet (visqueen) will be attached to the temporary shoring wall to provide a bond breaker for easier extraction of the shoring system.</p>			<p>See attached marked up drawings of the Pylon 9 footing shoring. SCCI is proposing to pour the (N) Pylon 9 footing against the temporary shoring wall, as shown on the attached drawings. A plastic sheet (visqueen) will be attached to the temporary shoring wall to provide a bond breaker for easier extraction of the shoring system.</p>			



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	Is this acceptable?					Is this acceptable?
<b>B-0106</b>	<b>BRP - CCTV Pole</b>	<b>Closed</b>	<b>01</b>	<b>11/19/2014</b>	<b>11/29/2014</b>	<b>11/20/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> At the meeting with CalTrans Maintenance on 11 /5/ 14, CalTrans advised that the CCTV pole shown on drawing D-1007 does not contain CCTV equipment, only a battery operated radar.  1. Please confirm the radar is the only equipment on the pole. 2. Please confirm the radar is battery operated and the pole has no wiring into the barrier wall.						<b>ANSWER:</b> At the meeting with CalTrans Maintenance on 11 /5/ 14, CalTrans advised that the CCTV pole shown on drawing D-1007 does not contain CCTV equipment, only a battery operated radar.  1. Please confirm the radar is the only equipment on the pole. 2. Please confirm the radar is battery operated and the pole has no wiring into the barrier wall.
<b>B-0107</b>	<b>BRP - Pile Tip elevation of CIDH 5-2</b>	<b>Closed</b>	<b>01</b>	<b>11/19/2014</b>	<b>11/29/2014</b>	<b>11/21/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> While drilling CIDH 5-2, Case Pacific hit refusal at an approximate elevation of -66.9' due to extremely hard rock. The CIDH Table on Contract Drawing S-3190 calls for a pile tip elevation of -69. I' but SCCI is proposing to cease drilling and revise the pile tip elevation to -66.9'. The cut-off elevation of 29. 7' would not change.  Is this acceptable?						<b>ANSWER:</b> While drilling CIDH 5-2, Case Pacific hit refusal at an approximate elevation of -66.9' due to extremely hard rock. The CIDH Table on Contract Drawing S-3190 calls for a pile tip elevation of -69. I' but SCCI is proposing to cease drilling and revise the pile tip elevation to -66.9'. The cut-off elevation of 29. 7' would not change.  Is this acceptable?





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B-0108	BRP - Rebar Couplers	Closed	01	11/19/2014	11/29/2014	12/01/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
As approved onsite by the ARUP representative, please confirm that in the case of the Barrette pile, if a rebar coupler for the #10 vertical steel cannot be coupled, it can be replaced with a splice bar that overlaps the end of each uncoupled bar by 6'-6".						As approved onsite by the ARUP representative, please confirm that in the case of the Barrette pile, if a rebar coupler for the #10 vertical steel cannot be coupled, it can be replaced with a splice bar that overlaps the end of each uncoupled bar by 6'-6".
B-0109	BRP - Lighting Under Overpass	Closed	01	11/19/2014	11/29/2014	12/01/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
Per discussions with Turner/WOJV, there has been discussion on what the final street lighting configuration on Natoma, Howard, Tehama, Clementina, Folsom and Harrison streets shall be. Per the contract electrical drawings, there are no lights installed in the bridge soffit, only lights to illuminate the top surface of the bridge. The utility demolition plans and the response to RFI 098 directs SCCI to remove and salvage numerous street lights. There is no item within our scope of work to re-illuminate the city streets after we remove the temporary lights and bridge falsework. Please verify if this is the contract's intent, or provide a change order and direction to install permanent lighting on the streets or bridge structure.						Per discussions with Turner/WOJV, there has been discussion on what the final street lighting configuration on Natoma, Howard, Tehama, Clementina, Folsom and Harrison streets shall be. Per the contract electrical drawings, there are no lights installed in the bridge soffit, only lights to illuminate the top surface of the bridge. The utility demolition plans and the response to RFI 098 directs SCCI to remove and salvage numerous street lights. There is no item within our scope of work to re-illuminate the city streets after we remove the temporary lights and bridge falsework. Please verify if this is the contract's intent, or provide a change order and direction to install permanent lighting on the streets or bridge structure.
B-0110	BRP - Mass Concrete - Pylon 9	Closed	01	11/20/2014	11/30/2014	12/09/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
Section 03 70 06 of the Contract Specifications calls out Pylon 9 from top of pile cap to top of bridge as mass concrete. Per approved as noted submittal TG 1801-257, SCCI is planning to pour this section in 2 stages -  -1st stage (2 pours): From top of pile cap to 1" above bottom of link beam -2nd stage (1 pour): Link beam and Pylon 9 from 1" above bottom of link beam to top of link beam  Is it acceptable to consider the Pylon 9 mass concrete						Section 03 70 06 of the Contract Specifications calls out Pylon 9 from top of pile cap to top of bridge as mass concrete. Per approved as noted submittal TG 1801-257, SCCI is planning to pour this section in 2 stages -  -1st stage (2 pours): From top of pile cap to 1" above bottom of link beam -2nd stage (1 pour): Link beam and Pylon 9 from 1" above bottom of link beam to top of link beam



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	upper limit at the top of the 1st stage (i.e. at the construction joint 1" above the bottom of the link beam)? Please reference attached sketch.					Is it acceptable to consider the Pylon 9 mass concrete upper limit at the top of the 1st stage (i.e. at the construction joint 1" above the bottom of the link beam)? Please reference attached sketch.
<b>B-0111</b>	<b>BRP - Frame 4 Soffit and Stems Construction Joints</b>	<b>Closed</b>	<b>01</b>	<b>11/21/2014</b>	<b>12/01/2014</b>	<b>01/05/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> SCCI is proposing construction joints in Frame 4 between the soffit and stems, in the locations shown in the attached drawing.  Is this acceptable?						<b>ANSWER:</b> SCCI is proposing construction joints in Frame 4 between the soffit and stems, in the locations shown in the attached drawing.  Is this acceptable?
<b>B-0112</b>	<b>BRP - Maximum Force Allowable Through Anti-Ram Barrier Foundation</b>	<b>Closed</b>	<b>01</b>	<b>11/21/2014</b>	<b>12/01/2014</b>	<b>11/26/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> What is the maximum force allowable as applied to the bridge section through the Anti-Ram barrier foundation over a period of 0.25 seconds?						<b>ANSWER:</b> What is the maximum force allowable as applied to the bridge section through the Anti-Ram barrier foundation over a period of 0.25 seconds?
<b>B-0112.1</b>	<b>BRP - Maximum force Allowable Through Anti-Ram Barrier Foundation</b>	<b>Open</b>	<b>01</b>	<b>12/09/2014</b>	<b>12/19/2014</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Following RFI B-0112 response, SCCI has provided the requested anti-ram barrier information in submittal TG 1801-162 - Anti-Ram Barrier Test Data (Minimum Stopping Capacity), dated 12/2/14. What is the maximum force allowable, as applied to the bridge section through the anti-ram barrier foundation over a period of 0.25 seconds?						<b>ANSWER:</b> Following RFI B-0112 response, SCCI has provided the requested anti-ram barrier information in submittal TG 1801-162 - Anti-Ram Barrier Test Data (Minimum Stopping Capacity), dated 12/2/14. What is the maximum force allowable, as applied to the bridge section through the anti-ram barrier foundation over a period of 0.25 seconds?
<b>B-0113</b>	<b>BRP - Deck Drain Pipe Material</b>	<b>Closed</b>	<b>01</b>	<b>11/24/2014</b>	<b>12/04/2014</b>	<b>12/12/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						

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<b>B-0116</b>	<b>BRP - Bridge Removal Hammer Striking Energy</b>	<b>Closed</b>	<b>01</b>	<b>11/24/2014</b>	<b>12/04/2014</b>	<b>12/04/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Section 3.3-A-8-a of Specification Section 02 4116 states that the Contractor shall not use a tool with a striking energy greater than 1,200 ft-lb per blow for breaking or removing concrete attached to the bridge. SCCI's demolition subcontractor, Evans Brothers, proposes an exception to this requirement in order to limit the number of Fremont Off Ramp closures by using a 11,000 ft-lb hammer attached to a 349 Excavator, or equivalent, staged at the below grade elevation in the Caltrans Substation work area. The 11,000 ft-lb hammer's use would be limited to horizontal striking on the vertical concrete barrier, as shown on attached SK-1.			Section 3.3-A-8-a of Specification Section 02 4116 states that the Contractor shall not use a tool with a striking energy greater than 1,200 ft-lb per blow for breaking or removing concrete attached to the bridge. SCCI's demolition subcontractor, Evans Brothers, proposes an exception to this requirement in order to limit the number of Fremont Off Ramp closures by using a 11,000 ft-lb hammer attached to a 349 Excavator, or equivalent, staged at the below grade elevation in the Caltrans Substation work area. The 11,000 ft-lb hammer's use would be limited to horizontal striking on the vertical concrete barrier, as shown on attached SK-1.			
Is this acceptable?			Is this acceptable?			
<b>B-0117</b>	<b>BRP - Fremont Off Ramp As-Built</b>	<b>Closed</b>	<b>01</b>	<b>11/24/2014</b>	<b>12/04/2014</b>	<b>11/25/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please provide any available as-built drawings of the existing Fremont Off Ramp bridge structure. These drawings are required by the civil engineer to prove the stability of the bridge structure during bridge removal as required by Section 1.3-C of Contract Specifications Section 02 41 16.			Please provide any available as-built drawings of the existing Fremont Off Ramp bridge structure. These drawings are required by the civil engineer to prove the stability of the bridge structure during bridge removal as required by Section 1.3-C of Contract Specifications Section 02 41 16.			
<b>B-0118</b>	<b>BRP - CIDH 20A Utility Drawings</b>	<b>Closed</b>	<b>01</b>	<b>11/24/2014</b>	<b>12/04/2014</b>	<b>12/02/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
SCCI has made an overlay of the CIDH piles with the utility demolition drawings. It appears that there is a conflict with the CIDH at Bent 21 and a street light conduit. Please confirm. There are also no utility drawings for the area around CIDH 20A. Please investigate whether there is a utility conflict and provide utility drawings.			SCCI has made an overlay of the CIDH piles with the utility demolition drawings. It appears that there is a conflict with the CIDH at Bent 21 and a street light conduit. Please confirm. There are also no utility drawings for the area around CIDH 20A. Please investigate whether there is a utility conflict and provide utility drawings.			



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B-0119	BRP - Protection of Transverse Rebar in Fremont Off Ramp	Closed	01	11/24/2014	12/04/2014	12/01/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:			ANSWER:			
Per Contract Drawing D-1007, a 3'-0" section of the Fremont Street Off-Ramp is to be demolished. Per Contract Drawing S-5000, the transverse reinforcement in the 3'-0" section is to remain with no damage. SCCI and its demolition subcontractor, Evans Brothers, plan to take measures to satisfy this requirement and protect this reinforcement in place. However, it is possible that reinforcement is damaged during the demolition process. In the case of a transverse rebar being damaged, SCCI proposes to install a #6 dowel with 1 O" embedment with Hilti RE-500 epoxy adjacent to the damaged rebar ensuring that the necessary 2'-0" lap splice is provided.			Per Contract Drawing D-1007, a 3'-0" section of the Fremont Street Off-Ramp is to be demolished. Per Contract Drawing S-5000, the transverse reinforcement in the 3'-0" section is to remain with no damage. SCCI and its demolition subcontractor, Evans Brothers, plan to take measures to satisfy this requirement and protect this reinforcement in place. However, it is possible that reinforcement is damaged during the demolition process. In the case of a transverse rebar being damaged, SCCI proposes to install a #6 dowel with 1 O" embedment with Hilti RE-500 epoxy adjacent to the damaged rebar ensuring that the necessary 2'-0" lap splice is provided.			
Is this acceptable?			Is this acceptable?			



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<b>B-0121</b>	<b>BRP - Guard Booth Equipment Cabinet</b>	<b>Closed</b>	<b>01</b>	<b>12/01/2014</b>	<b>12/11/2014</b>	<b>12/09/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please see below RFI from WPCS, SCCI's security and communication subcontractor.			Please see below RFI from WPCS, SCCI's security and communication subcontractor.			
Specification 27 1117 3.1.C. I , requests a Cabinet to be a minimum of I 8RU with a usable depth of 32" and sized to fit within the footprint of the guard booth. Upon looking into the B-line wall mount cabinet			Specification 27 1117 3.1.C. I , requests a Cabinet to be a minimum of I 8RU with a usable depth of 32" and sized to fit within the footprint of the guard booth. Upon looking into the B-line wall mount cabinet			
WPCS submitting on as an "or equal ", WPCS have looked into all the other manufactures listed as well as additional manufactures, and have found that no manufacture offers a half size wall mount cabinet with 32" of usable depth. The only way the specification can be met is if a full size 7' minimum cabinet were to be installed, however taking into account that this cabinet is being located in a fairly small guard			WPCS submitting on as an "or equal ", WPCS have looked into all the other manufactures listed as well as additional manufactures, and have found that no manufacture offers a half size wall mount cabinet with 32" of usable depth. The only way the specification can be met is if a full size 7' minimum cabinet were to be installed, however taking into account that this cabinet is being located in a fairly small guard			
shack, a full size cabinet will require a larger foot print and may not be the designer's intent.			shack, a full size cabinet will require a larger foot print and may not be the designer's intent.			
1. Please confirm the specification requirements are correct. If not, please provide new cabinet dimension criteria.			1. Please confirm the specification requirements are correct. If not, please provide new cabinet dimension criteria.			
2. If specification is confirmed, please confirm a full size cabinet is what is desired to be installed in the guard shack.			2. If specification is confirmed, please confirm a full size cabinet is what is desired to be installed in the guard shack.			



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<b>B-0122</b>	<b>BRP - Seat Extender at Hinge 9</b>	<b>Closed</b>	<b>01</b>	<b>12/09/2014</b>	<b>12/19/2014</b>	<b>01/06/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>1. During the December 1, 2014 conference call to discuss the seat extender/vertical restrainer, Arup stated that the seat extender/vertical restrainer was initially intended to perform as only a vertical restrainer. As shown on the bid drawings and incorporated into OPAC's redesign, if the bridge falls off of its bearing, any deflection of the seat extender/vertical restrainer pipe would prevent the bridge from reseating on the bearing. An alternative to the seat extender would be to follow Caltrans MTD 20-22, "Seismic Design of Bridges with Isolation Bearings", which recommends that isolation bearings, such as PTFE sliding spherical bearings, have adequate sliding surface to permit 1.25 times the MCE displacement and that unseating be prevented through the use of catcher blocks at displacements higher than 1.25 time the MCE displacement. Is it acceptable to replace the seat extender component of the seat extender/vertical restrainer with bearings designed according to Caltrans MTD 20-22?</p> <p>2. As discussed and agreed upon, during the December 1, 2014 conference call, the Hinge 9 pipe vertical restrainer shown in the bid drawings and incorporated into OPAC's redesign prevents the Hinge 9 bearings from rotating as intended. Arup proposed providing a gap between the restrainer pipe and vertical restrainer assembly to allow the Hinge 9 bearings to rotate as intended. OP AC commented that a gap would allow for some uplift at Hinge 9. Arup agreed and said that some uplift maybe acceptable. We find that the vertical restrainer assembly requires a 2.5" gap top and bottom for the Hinge 9 bearings to be able to rotate the 2 degrees as specified. Is a 2.5" gap acceptable for the vertical restrainer assembly to perform as intended?</p>			<p>1. During the December 1, 2014 conference call to discuss the seat extender/vertical restrainer, Arup stated that the seat extender/vertical restrainer was initially intended to perform as only a vertical restrainer. As shown on the bid drawings and incorporated into OPAC's redesign, if the bridge falls off of its bearing, any deflection of the seat extender/vertical restrainer pipe would prevent the bridge from reseating on the bearing. An alternative to the seat extender would be to follow Caltrans MTD 20-22, "Seismic Design of Bridges with Isolation Bearings", which recommends that isolation bearings, such as PTFE sliding spherical bearings, have adequate sliding surface to permit 1.25 times the MCE displacement and that unseating be prevented through the use of catcher blocks at displacements higher than 1.25 time the MCE displacement. Is it acceptable to replace the seat extender component of the seat extender/vertical restrainer with bearings designed according to Caltrans MTD 20-22?</p> <p>2. As discussed and agreed upon, during the December 1, 2014 conference call, the Hinge 9 pipe vertical restrainer shown in the bid drawings and incorporated into OPAC's redesign prevents the Hinge 9 bearings from rotating as intended. Arup proposed providing a gap between the restrainer pipe and vertical restrainer assembly to allow the Hinge 9 bearings to rotate as intended. OP AC commented that a gap would allow for some uplift at Hinge 9. Arup agreed and said that some uplift maybe acceptable. We find that the vertical restrainer assembly requires a 2.5" gap top and bottom for the Hinge 9 bearings to be able to rotate the 2 degrees as specified. Is a 2.5" gap acceptable for the vertical restrainer assembly to perform as intended?</p>			



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<b>B-0123</b>	<b>BRP - Hinge 9 Bearing Sole Plate</b>	<b>Closed</b>	<b>01</b>	<b>12/02/2014</b>	<b>12/12/2014</b>	<b>01/06/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Please confirm that the Hinge 9 Seismic displacement provided in RFI B-0045 should be used to design the Hinge 9 bearing sole plate.						<b>ANSWER:</b>  Please confirm that the Hinge 9 Seismic displacement provided in RFI B-0045 should be used to design the Hinge 9 bearing sole plate.
<b>B-0124</b>	<b>BRP - Removal of Trees in Lot A"</b>	<b>Closed</b>	<b>01</b>	<b>12/04/2014</b>	<b>12/14/2014</b>	<b>12/12/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  As shown on the attached survey of the existing grade of future Retaining Walls 8 and 9, there are five trees that will be affected by the excavation required to safely slope to the elevation of the foundations of these retaining walls. In order to be able to safely excavate to this elevation, SCCI proposes to remove these trees prior to the start of this work. Please note that this requires work beyond the TJPA property line.  Is this acceptable?						<b>ANSWER:</b>  As shown on the attached survey of the existing grade of future Retaining Walls 8 and 9, there are five trees that will be affected by the excavation required to safely slope to the elevation of the foundations of these retaining walls. In order to be able to safely excavate to this elevation, SCCI proposes to remove these trees prior to the start of this work. Please note that this requires work beyond the TJPA property line.  Is this acceptable?
<b>B-0125</b>	<b>BRP - Lap Splice of CIDH 4-1 Reinforcement Cage</b>	<b>Closed</b>	<b>01</b>	<b>12/03/2014</b>	<b>12/13/2014</b>	<b>12/04/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  The rebar cage for CIDH 4-1 was fabricated with "C2" bars 80'-0" rather than the 84'-0" length required by Contract Drawing S-3190 and the approved shop drawings, Submittal TG1801-345.1. SCCI proposes to install a lap splice with a lap length of 8'-9" above the bottom of the installed C2 bar.  Is this acceptable?						<b>ANSWER:</b>  The rebar cage for CIDH 4-1 was fabricated with "C2" bars 80'-0" rather than the 84'-0" length required by Contract Drawing S-3190 and the approved shop drawings, Submittal TG1801-345.1. SCCI proposes to install a lap splice with a lap length of 8'-9" above the bottom of the installed C2 bar.  Is this acceptable?
<b>B-0126</b>	<b>BRP - Longitudinal Bars in CIDH 4-1</b>	<b>Closed</b>	<b>01</b>	<b>12/03/2014</b>	<b>12/13/2014</b>	<b>12/04/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Per approved CIDH shop drawings in Submittal TO 1801-345.1, the "C1" bars were shown to be two bars						<b>ANSWER:</b>  Per approved CIDH shop drawings in Submittal TO 1801-345.1, the "C1" bars were shown to be two bars





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	<p>mechanically coupled together to reach the design length of 84'-0". However, the cages were fabricated with the "C1" bars as a single bar that is 84'-0" long. Please confirm that this is acceptable.</p>					<p>mechanically coupled together to reach the design length of 84'-0". However, the cages were fabricated with the "C1" bars as a single bar that is 84'-0" long. Please confirm that this is acceptable.</p>
<b>B-0126.1</b>	<b>BRP - Longitudinal C1 Bars in CIDH 4-2 and 4-3</b>	<b>Closed</b>	<b>01</b>	<b>12/12/2014</b>	<b>12/22/2014</b>	<b>12/16/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> <p>Per approved CIDH shop drawings in Submittal TG 1801-345.1, the "C1" bars in the Bent 4 CIDH's were shown to be two bars mechanically coupled together to reach the design length of 84'-0". RFI 0126 allowed these bars to be substituted with a single bar that is 84'-0" long in the CIDH 4-1 reinforcement cage. Please confirm that this is also acceptable for the "C1" bars in CIDH 4-2 and 4-3.</p>						<b>ANSWER:</b> <p>Per approved CIDH shop drawings in Submittal TG 1801-345.1, the "C1" bars in the Bent 4 CIDH's were shown to be two bars mechanically coupled together to reach the design length of 84'-0". RFI 0126 allowed these bars to be substituted with a single bar that is 84'-0" long in the CIDH 4-1 reinforcement cage. Please confirm that this is also acceptable for the "C1" bars in CIDH 4-2 and 4-3.</p>
<b>B-0127</b>	<b>BRP - Traffic Signal Conflict at Essex and Harrison</b>	<b>Closed</b>	<b>01</b>	<b>12/04/2014</b>	<b>12/14/2014</b>	<b>12/12/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> <p>Please see the attached sktech. The traffic signal at the intersection of Harrison and Essex may potentially conflict with the Harrison right bridge overcrossing falsework. In addition, the falsework supporting the new Fremont Off-ramp structure will obstruct the view of the signal. SCCI is still designing the falsework and the attached sketch is conceptual, but illustrates the issue. This traffic signal is not shown on the contract utility drawings and SCCI cannot erect falsework without creating a public safety hazard in the present condition. Survey information is attached for your reference. Please provide direction.</p>						<b>ANSWER:</b> <p>Please see the attached sktech. The traffic signal at the intersection of Harrison and Essex may potentially conflict with the Harrison right bridge overcrossing falsework. In addition, the falsework supporting the new Fremont Off-ramp structure will obstruct the view of the signal. SCCI is still designing the falsework and the attached sketch is conceptual, but illustrates the issue. This traffic signal is not shown on the contract utility drawings and SCCI cannot erect falsework without creating a public safety hazard in the present condition. Survey information is attached for your reference. Please provide direction.</p>



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<b>B-0128</b>	<b>BRP - MSE Wall Ripped Texture Detail</b>	<b>Closed</b>	<b>01</b>	<b>12/04/2014</b>	<b>12/14/2014</b>	<b>12/17/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Drawing S-3212 shows the MSE wall ripped texture detail. SCCI's supplier, Tensar Corp, is proposing to replace this ripped texture with the attached ripped texture from Spec Formliners. This proposed texture is more readily available to Tensar and SCCI believes it meets the specifications and intended architectural design, as it is similar to the current texture detail. Is this ripped texture acceptable?						<b>ANSWER:</b>  Drawing S-3212 shows the MSE wall ripped texture detail. SCCI's supplier, Tensar Corp, is proposing to replace this ripped texture with the attached ripped texture from Spec Formliners. This proposed texture is more readily available to Tensar and SCCI believes it meets the specifications and intended architectural design, as it is similar to the current texture detail. Is this ripped texture acceptable?
<b>B-0129</b>	<b>BRP - Frame 4 Bent Plate Coating</b>	<b>Closed</b>	<b>01</b>	<b>12/05/2014</b>	<b>12/15/2014</b>	<b>12/09/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Contract drawing S-5067, showing the Frame 4 section, calls for galvanized bent plate bolts. There is no coating listed for the actual bent plates. Per specification 05 12 06 there is no requirement for coating structural steel.  Please confirm the bent plates do not need to be galvanized or have any other coating.						<b>ANSWER:</b>  Contract drawing S-5067, showing the Frame 4 section, calls for galvanized bent plate bolts. There is no coating listed for the actual bent plates. Per specification 05 12 06 there is no requirement for coating structural steel.  Please confirm the bent plates do not need to be galvanized or have any other coating.
<b>B-0130</b>	<b>BRP - Frame 4 Bent Plate Lengths</b>	<b>Closed</b>	<b>01</b>	<b>12/05/2014</b>	<b>12/15/2014</b>	<b>12/09/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Per contract drawing S-6068, the longest bent plate is over 29'. SCCI is proposing to use multiple steel bent plates with a maximum length of 10', for all sections longer than 10'. The plates would be installed end-to-end without welding, there would be a maximum gap between the plates of 1/2".  Is this acceptable?						<b>ANSWER:</b>  Per contract drawing S-6068, the longest bent plate is over 29'. SCCI is proposing to use multiple steel bent plates with a maximum length of 10', for all sections longer than 10'. The plates would be installed end-to-end without welding, there would be a maximum gap between the plates of 1/2".  Is this acceptable?
<b>B-0131</b>	<b>BRP - Frame 4 Curved Bent Plate</b>	<b>Closed</b>	<b>01</b>	<b>12/05/2014</b>	<b>12/15/2014</b>	<b>12/09/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	<p>Per contract drawing S-6068, several of the Frame 4 bent plates have a slight curve (see attached sketch for some examples). SCCI is proposing to install only straight bent plates at Frame 4, including all curved locations.</p> <p>Is this acceptable?</p>					<p>Per contract drawing S-6068, several of the Frame 4 bent plates have a slight curve (see attached sketch for some examples). SCCI is proposing to install only straight bent plates at Frame 4, including all curved locations.</p> <p>Is this acceptable?</p>
<b>B-0132</b>	<b>BRP - PG&amp;E Line Relocation at Harrison</b>	<b>Closed</b>	<b>01</b>	<b>12/06/2014</b>	<b>12/16/2014</b>	<b>12/08/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Per contract drawing U-1003, note 5, the existing electrical line is to be relocated by PG&E. Per Turner's direction, a survey was performed to see if removal was necessary (survey attached). SCCI believes this duct bank will be above the sidewalk grade if the line is to remain. Please confirm this line is to be relocated.						<b>ANSWER:</b> Per contract drawing U-1003, note 5, the existing electrical line is to be relocated by PG&E. Per Turner's direction, a survey was performed to see if removal was necessary (survey attached). SCCI believes this duct bank will be above the sidewalk grade if the line is to remain. Please confirm this line is to be relocated.
<b>B-0132.1</b>	<b>BRP - PG&amp;E Line Relocation at Harrison</b>	<b>Closed</b>	<b>01</b>	<b>12/09/2014</b>	<b>12/19/2014</b>	<b>12/10/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Please see attached visual interpretation of the survey notes from RFI#132 of the (E) electrical duct bank at (N) Abutment 1. As noted in previously submitted SCCI's RFI # 132, per contract drawing U-1003, note 5, the existing electrical line is to be relocated by PG&E. Per Turner's direction, a survey was performed to see if removal was necessary (survey attached). SCCI believes this duct bank is above the sidewalk grade if the structure is to remain. Please confirm this line is to be relocated.						<b>ANSWER:</b> Please see attached visual interpretation of the survey notes from RFI#132 of the (E) electrical duct bank at (N) Abutment 1. As noted in previously submitted SCCI's RFI # 132, per contract drawing U-1003, note 5, the existing electrical line is to be relocated by PG&E. Per Turner's direction, a survey was performed to see if removal was necessary (survey attached). SCCI believes this duct bank is above the sidewalk grade if the structure is to remain. Please confirm this line is to be relocated.

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<b>B-0133</b>	<b>BRP - Potential Tree Conflict at Folsom</b>	<b>Closed</b>	<b>01</b>	<b>12/08/2014</b>	<b>12/18/2014</b>	<b>12/12/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> SCCI has noticed a potential conflict with five large trees and the new bridge structure south of Folsom Street. These trees are not shown on the contract drawings. Survey information and a photograph are attached. Please advise.		<b>ANSWER:</b> SCCI has noticed a potential conflict with five large trees and the new bridge structure south of Folsom Street. These trees are not shown on the contract drawings. Survey information and a photograph are attached. Please advise.				
<b>B-0133.1</b>	<b>BRP - Potential Tree Conflict at Folsom</b>	<b>Closed</b>	<b>01</b>	<b>01/05/2015</b>	<b>01/15/2015</b>	<b>01/30/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> Add'l Doc Ref: B-0133 Location: Bus Ramp  Per RFI Response B-0133, SCCI is not allowed to remove or trim trees on private property. SCCI has looked into available options and had certified arborists from The Professional Tree Care Company and Bartlett Tree Experts visit the site. They both refused to trim the trees at the property line due to stability concerns. They stated that these are Lombardy Poplars with soft wood, a short life span, and a tendency to drop limbs and have trunk or branch failures if the weight of the tree is shifted by significantly trimming them. The bridge cannot be built with these trees remaining in place as significant amount of branches directly conflict with the bridge structure. Please see the attached schematic and photo for reference. Please advise if the trees can be removed.		<b>ANSWER:</b> Add'l Doc Ref: B-0133 Location: Bus Ramp  Per RFI Response B-0133, SCCI is not allowed to remove or trim trees on private property. SCCI has looked into available options and had certified arborists from The Professional Tree Care Company and Bartlett Tree Experts visit the site. They both refused to trim the trees at the property line due to stability concerns. They stated that these are Lombardy Poplars with soft wood, a short life span, and a tendency to drop limbs and have trunk or branch failures if the weight of the tree is shifted by significantly trimming them. The bridge cannot be built with these trees remaining in place as significant amount of branches directly conflict with the bridge structure. Please see the attached schematic and photo for reference. Please advise if the trees can be removed.				
<b>B-0135</b>	<b>BRP - Deck Drain Column Pipe Outlet</b>	<b>Closed</b>	<b>01</b>	<b>12/15/2014</b>	<b>12/25/2014</b>	<b>01/06/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> Per Caltrans Detail B7-6, the deck drain pipe exits the columns 6" above finished grade. This is inconsistent with the attached civil and structural sheets showing slots in the catch basins for the pipes to enter.  Please clarify which detail is to be used for the connection between the deck drainage and the catch basins.		<b>ANSWER:</b> Per Caltrans Detail B7-6, the deck drain pipe exits the columns 6" above finished grade. This is inconsistent with the attached civil and structural sheets showing slots in the catch basins for the pipes to enter.  Please clarify which detail is to be used for the connection between the deck drainage and the catch				



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			basins.			
<b>B-0136</b>	<b>BRP - Utility Conduit Clarification at Pylon 9</b>	<b>Closed</b>	<b>01</b>	<b>12/15/2014</b>	<b>12/25/2014</b>	<b>12/22/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Per contract drawing S-3173, the utility conduits leave Pylon 9 from the base of the column. Per contract drawing E-5006, the utility conduits leave Pylon 9 from the pile cap. SCCI proposes to have the utility conduits leave Pylon 9 from the base of the column as shown in contract drawing S-3173. Is this acceptable?			<b>ANSWER:</b> Per contract drawing S-3173, the utility conduits leave Pylon 9 from the base of the column. Per contract drawing E-5006, the utility conduits leave Pylon 9 from the pile cap. SCCI proposes to have the utility conduits leave Pylon 9 from the base of the column as shown in contract drawing S-3173. Is this acceptable?			
<b>B-0137</b>	<b>BRP - Barrier Gate at Harrison St. Off Ramp</b>	<b>Closed</b>	<b>01</b>	<b>12/15/2014</b>	<b>12/25/2014</b>	<b>12/19/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Contract drawing E-3000 calls out a semaphore arm at the Harrison St off ramp, which is in a different location to the barrier gate shown in the civil drawings (see attached).  1. Please confirm the barrier gate is a 20' arm barrier gate, not a semaphore arm. 2. Please confirm the location of the barrier gate is per the civil drawings, not E-3000.			<b>ANSWER:</b> Contract drawing E-3000 calls out a semaphore arm at the Harrison St off ramp, which is in a different location to the barrier gate shown in the civil drawings (see attached).  1. Please confirm the barrier gate is a 20' arm barrier gate, not a semaphore arm. 2. Please confirm the location of the barrier gate is per the civil drawings, not E-3000.			
<b>B-0138</b>	<b>BRP - Recycled Material for Backfill</b>	<b>Closed</b>	<b>01</b>	<b>12/16/2014</b>	<b>12/26/2014</b>	<b>12/22/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> SCCI proposes to use Class II recycled aggregate that meets the requirements of Contract Specifications Section 31 00 06 - Earthwork, Section 2.1 for non-structural backfill. Please confirm that this is acceptable.			<b>ANSWER:</b> SCCI proposes to use Class II recycled aggregate that meets the requirements of Contract Specifications Section 31 00 06 - Earthwork, Section 2.1 for non-structural backfill. Please confirm that this is acceptable.			





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Please advise if the above sequence of work is acceptable.						
<b>B-0140</b>	<b>BRP - Overhead Sign Structures Signal Face Dimension</b>	<b>Closed</b>	<b>01</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>01/27/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please provide the location of the signal face from the top of the SP-2 & SP-3 truss (see attached).			Please provide the location of the signal face from the top of the SP-2 & SP-3 truss (see attached).			
<b>B-0140.1</b>	<b>BRP - Overhead Sign Signal Face Dimension SP-2 &amp; SP-3</b>	<b>Open</b>	<b>01</b>	<b>01/29/2015</b>	<b>02/08/2015</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: RFI B-0140 Location: Bus Ramp			Contract Doc Ref: RFI B-0140 Location: Bus Ramp			
The RFI B-0140 response directs us to the Cal trans Standard Plans ES-14C. The details on ES-14C pertain to a light weight sign structure with a 4'-0" tall EMS panel. The drawings for ES-14C appear to have the signal in the center of the EMS panel. The sign structure we are fabricating (SP-2 & SP-3) will have an aluminum sign panel that is 6'-8" tall. It appears from the project plans that the signals are located near the top of the panel, which is contradictory to Caltrans ES-14C. Based on the project plans and our experiences we would assume the centerline of the signals would be located approximately 12" to 24" below the top of the sign panel.			The RFI B-0140 response directs us to the Cal trans Standard Plans ES-14C. The details on ES-14C pertain to a light weight sign structure with a 4'-0" tall EMS panel. The drawings for ES-14C appear to have the signal in the center of the EMS panel. The sign structure we are fabricating (SP-2 & SP-3) will have an aluminum sign panel that is 6'-8" tall. It appears from the project plans that the signals are located near the top of the panel, which is contradictory to Caltrans ES-14C. Based on the project plans and our experiences we would assume the centerline of the signals would be located approximately 12" to 24" below the top of the sign panel.			
Please provide the location of the signal face from the top of truss or from the top of the sign panel.			Please provide the location of the signal face from the top of truss or from the top of the sign panel.			





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<b>B-0141</b>	<b>BRP - CIDH Pile Slurry Testing Requirements</b>	<b>Closed</b>	<b>01</b>	<b>12/22/2014</b>	<b>01/01/2015</b>	<b>01/16/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: 31 63 30 Location: Bus Ramp		Contract Doc Ref: 31 63 30 Location: Bus Ramp				
Contract Specification Section 31 63 30 - Cast-In-Drilled-Hole Piles details the material and testing requirements for the bentonite slurry used in the CIDH piles. This specification states that slurry testing should be performed every two hours. Per Case Pacific's experience, the slurry in the shaft during the drilling operation will not meet the specifications per the Mineral Slurry Requirement table in Section 2.4-A-6 because suspended material created by drilling will make the slurry thicker. Therefore, SCCI and Case Pacific propose to only test the bentonite slurry in drilled CIDH shafts once the slurry has been recirculated before the concrete placement. Per Arup's previous request, slurry testing will be possible after periods of 48 hours of no drilling operation.		Contract Specification Section 31 63 30 - Cast-In-Drilled-Hole Piles details the material and testing requirements for the bentonite slurry used in the CIDH piles. This specification states that slurry testing should be performed every two hours. Per Case Pacific's experience, the slurry in the shaft during the drilling operation will not meet the specifications per the Mineral Slurry Requirement table in Section 2.4-A-6 because suspended material created by drilling will make the slurry thicker. Therefore, SCCI and Case Pacific propose to only test the bentonite slurry in drilled CIDH shafts once the slurry has been recirculated before the concrete placement. Per Arup's previous request, slurry testing will be possible after periods of 48 hours of no drilling operation.				
Please confirm that this is acceptable.		Please confirm that this is acceptable.				
<b>B-0142</b>	<b>BRP - Overhead Sign Structure Post Height</b>	<b>Closed</b>	<b>01</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>12/23/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract drawing C-7004 states the overhead sign posts must be a minimum of 18' 6" from top of deck to bottom of sign frame. SCCI's subcontractor, Midstate Barrier, is proposing a 19' post (from top of deck to bottom of sign frame). Please confirm this height is acceptable for all 4 overhead sign posts.		Contract drawing C-7004 states the overhead sign posts must be a minimum of 18' 6" from top of deck to bottom of sign frame. SCCI's subcontractor, Midstate Barrier, is proposing a 19' post (from top of deck to bottom of sign frame). Please confirm this height is acceptable for all 4 overhead sign posts.				
<b>B-0143</b>	<b>BRP - Overhead Sign Structure SP-1 Post and Mast Dimension</b>	<b>Open</b>	<b>01</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
The contract drawings do not provide a post and mass size for SP-1, the lightweight sign structure. SCCI's subcontractor, Midstate Barrier, have used the 2006 CalTrans design reference sheets for these dimensions.		The contract drawings do not provide a post and mass size for SP-1, the lightweight sign structure. SCCI's subcontractor, Midstate Barrier, have used the 2006 CalTrans design reference sheets for these dimensions.				





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<p>Please confirm these dimensions are correct or please provide the correct dimensions.</p>						
<b>B-0144</b>	<b>BRP - Rebar Splices in Pylon 9 Vertical Reinforcement</b>	<b>Closed</b>	<b>01</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>01/06/2015</b>
<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference S-3177, S-3180, attached sketch of the pylon 9 reinforcement and draft drawing of the cable stayed bridge saddle assembly. In order to efficiently utilize the splices in the pylon 9 vertical reinforcement, SCCI proposes to shift the splice locations as shown on the attached sketch: Lower splice (near EL 64.80') will be shifted up approximately 6', and the upper splice (near EL 102.80) would be shifted up approximately 12".			Please reference S-3177, S-3180, attached sketch of the pylon 9 reinforcement and draft drawing of the cable stayed bridge saddle assembly. In order to efficiently utilize the splices in the pylon 9 vertical reinforcement, SCCI proposes to shift the splice locations as shown on the attached sketch: Lower splice (near EL 64.80') will be shifted up approximately 6', and the upper splice (near EL 102.80) would be shifted up approximately 12".			
Is this acceptable?			Is this acceptable?			
<b>B-0145</b>	<b>BRP - Bent 8 CIDH Pile Reinforcement Cage C1 and C2 Quantities</b>	<b>Closed</b>	<b>01</b>	<b>12/22/2014</b>	<b>01/01/2015</b>	<b>01/13/2015</b>
<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: 31 63 30, S-3190 Location: Bus Ramp			Contract Doc Ref: 31 63 30, S-3190 Location: Bus Ramp			
The CIDH Table on Contract Drawing S-3190 calls for 29 ea # 11 bars for the C1 bars and 26 ea # 10 bars for the C2 bars. Please confirm that this is correct.			The CIDH Table on Contract Drawing S-3190 calls for 29 ea # 11 bars for the C1 bars and 26 ea # 10 bars for the C2 bars. Please confirm that this is correct.			



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B-0146	BRP - Type 60 Series Concrete Barrier Mix Design	Closed	01	12/22/2014	01/01/2015	01/12/2015
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Contract Doc Ref: 34 71 06 Location: Bus Ramp		Contract Doc Ref: 34 71 06 Location: Bus Ramp				
Per contract specification section 03 30 06 2.1 A, barriers require 3,600 psi with minimum 590 lb/cu yd concrete. However, per the 2010 CalTrans Standard Specifications Type 60 series barriers are considered minor concrete. SCCI is proposing to use the attached approved minor concrete mix design (from TG1801-367 Minor Concrete Mix Design, Agg Grading and Cert of Compliance) for Type 60 and Type 60C concrete barriers. Is this acceptable?		Per contract specification section 03 30 06 2.1 A, barriers require 3,600 psi with minimum 590 lb/cu yd concrete. However, per the 2010 CalTrans Standard Specifications Type 60 series barriers are considered minor concrete. SCCI is proposing to use the attached approved minor concrete mix design (from TG1801-367 Minor Concrete Mix Design, Agg Grading and Cert of Compliance) for Type 60 and Type 60C concrete barriers. Is this acceptable?				
Please note: The Type 60 and Type 60C barriers fall under the CalTrans ROW.		Please note: The Type 60 and Type 60C barriers fall under the CalTrans ROW.				
B-0147	BRP - Type 732 and 732MOD Concrete Barrier Mix Design	Closed	01	12/22/2014	01/01/2015	01/09/2015
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Contract Doc Ref: 03 30 06 Location: Bus Ramp		Contract Doc Ref: 03 30 06 Location: Bus Ramp				
SCCI is proposing to use the approved mix design from submittal TG1801-303 - Pile Cap Concrete Mix Design, Gradation and Certificates, for the Type 732 and Type 732MOD concrete barriers, as it meets the concrete barrier mix design specifications in 03 30 06. Please confirm this mix design can be used for Type 732 and Type 732MOD concrete barriers.		SCCI is proposing to use the approved mix design from submittal TG1801-303 - Pile Cap Concrete Mix Design, Gradation and Certificates, for the Type 732 and Type 732MOD concrete barriers, as it meets the concrete barrier mix design specifications in 03 30 06. Please confirm this mix design can be used for Type 732 and Type 732MOD concrete barriers.				
B-0148	BRP - Bent 8 CIDH Reinforcement Cage Headed Rebar	Closed	01	12/22/2014	01/01/2015	01/09/2015
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Contract Doc Ref: 03 20 56, TG1801-345.1 Location: Bus Ramp		Contract Doc Ref: 03 20 56, TG1801-345.1 Location: Bus Ramp				
Drawing P2R1 in Submittal TG1801-345.1 - CIDH Reinforcement Cage Shop Drawing shows that the headed rebar on the Bent 8 CIDH reinforcement cages are		Drawing P2R1 in Submittal TG1801-345.1 - CIDH Reinforcement Cage Shop Drawing shows that the headed rebar on the Bent 8 CIDH reinforcement				



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	<p>to be HRC-555 T-Heads. Upon further review of the Contract Specifications, CMC Rebar noted that Contract Specification 03 20 45 - Headed Bar Reinforcement, Section 1.4 B states that "headed bar reinforcement must have full size heads and must be on the Caltrans Authorized Material List". HRC-555 T-Heads are on the Caltrans Authorized Material List but under the reduced size heads not the full size heads. Please confirm that the HRC-555 T-Heads are acceptable. An alternative to the HRC-555 is the HRC-120 which is on the Caltrans list and has full size heads.</p>					<p>cages are to be HRC-555 T-Heads. Upon further review of the Contract Specifications, CMC Rebar noted that Contract Specification 03 20 45 - Headed Bar Reinforcement, Section 1.4 B states that "headed bar reinforcement must have full size heads and must be on the Caltrans Authorized Material List". HRC-555 T-Heads are on the Caltrans Authorized Material List but under the reduced size heads not the full size heads. Please confirm that the HRC-555 T-Heads are acceptable. An alternative to the HRC-555 is the HRC-120 which is on the Caltrans list and has full size heads.</p>
<b>B-0149</b>	<b>BRP - Bent 8 CIDH Reinforcement Cage C2 Splices</b>	<b>Closed</b>	<b>01</b>	<b>12/23/2014</b>	<b>01/01/2015</b>	<b>01/20/2015</b>
	<p><b>From:</b> Webcor Construction LP      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: 31 63 30, S-3190 Location: Bus Ramp</p> <p>Note 3 on Contract Drawing S-3 190 states that there is a "No splice zone at top 60'-0" of shaft cage for main reinforcement. Ultimate splice coupler shall be used for reinforcement CI". CMC Rebar assumes that the C2 bars are also considered main reinforcement but no splice requirement is provided in this note. If splices are needed in the C2 bars, is it acceptable to use a 6-3 stagger lap splice and can it encroach the 60'-0" no splice zone.</p>					<p><b>ANSWER:</b></p> <p>Contract Doc Ref: 31 63 30, S-3190 Location: Bus Ramp</p> <p>Note 3 on Contract Drawing S-3 190 states that there is a "No splice zone at top 60'-0" of shaft cage for main reinforcement. Ultimate splice coupler shall be used for reinforcement CI". CMC Rebar assumes that the C2 bars are also considered main reinforcement but no splice requirement is provided in this note. If splices are needed in the C2 bars, is it acceptable to use a 6-3 stagger lap splice and can it encroach the 60'-0" no splice zone.</p>
<b>B-0150</b>	<b>BRP - Grout For Inspection Tubes</b>	<b>Closed</b>	<b>01</b>	<b>12/19/2014</b>	<b>12/29/2014</b>	<b>01/06/2015</b>
	<p><b>From:</b> Webcor Construction LP      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Per communications with ARUP, grout consisting of Portland cement mixed at a ratio of no more than 5 gallons of water per 94 lb. bag can be used to fill the inspection tubes in the CIDH and Barrette piles, provided the mix is pumped from the bottom up. Please confirm this is acceptable.</p>					<p><b>ANSWER:</b></p> <p>Per communications with ARUP, grout consisting of Portland cement mixed at a ratio of no more than 5 gallons of water per 94 lb. bag can be used to fill the inspection tubes in the CIDH and Barrette piles, provided the mix is pumped from the bottom up. Please confirm this is acceptable.</p>



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<b>B-0151</b>	<b>BRP - Painting of Stay Cable Armor</b>	<b>Closed</b>	<b>01</b>	<b>12/22/2014</b>	<b>01/01/2015</b>	<b>01/09/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: 05 16 34 Location: Bus Ramp Add'l Doc Ref's: VSL - Trade Subcontractor RFI (Attached)  VSL is requesting clarification from designer on the following: Will the stay cable armor need to be painted for aesthetic purposes? If so, please specify RAL #, Product manufacturer to be used for this work, and applicable specifications to be used.						<b>ANSWER:</b>  Contract Doc Ref: 05 16 34 Location: Bus Ramp Add'l Doc Ref's: VSL - Trade Subcontractor RFI (Attached)  VSL is requesting clarification from designer on the following: Will the stay cable armor need to be painted for aesthetic purposes? If so, please specify RAL #, Product manufacturer to be used for this work, and applicable specifications to be used.
<b>B-0151.1</b>	<b>BRP - Paint of Stay Cable Armor</b>	<b>Open</b>	<b>01</b>	<b>01/20/2015</b>	<b>01/30/2015</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  This question contains Sensitive Security Information and is available only to individuals who have been granted access to the document that is the basis for the question.						<b>ANSWER:</b>  This question contains Sensitive Security Information and is available only to individuals who have been granted access to the document that is the basis for the question.
<b>B-0152</b>	<b>BRP - Stay Armor Thermal Cutting Device</b>	<b>Closed</b>	<b>01</b>	<b>12/22/2014</b>	<b>01/01/2015</b>	<b>01/09/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: 05 16 34 Location: Bus Ramp Add'l Doc Ref's: VSL - Trade Subcontractor RFI (Attached)  When testing the armor for thermal cutting, please confirm the thermal cutting device is an exothermic torch of 1/4 inch diameter rods, 18 inch long with up to 40 ft3 of oxygen (2 ea x 20 ft3 oxygen tanks).						<b>ANSWER:</b>  Contract Doc Ref: 05 16 34 Location: Bus Ramp Add'l Doc Ref's: VSL - Trade Subcontractor RFI (Attached)  When testing the armor for thermal cutting, please confirm the thermal cutting device is an exothermic torch of 1/4 inch diameter rods, 18 inch long with up to 40 ft3 of oxygen (2 ea x 20 ft3 oxygen tanks).
<b>B-0153</b>	<b>BRP - Stay Cable Armor Testing</b>	<b>Closed</b>	<b>01</b>	<b>12/22/2014</b>	<b>01/01/2015</b>	<b>01/22/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	<p>Contract Doc Ref: 05 16 34 Location: Bus Ramp</p> <p>The following questions pertain to the Bus Ramp Stay Cable Bridge:</p> <p>#1 - Please confirm that unstressed strands are acceptable when performing physical testing of the armor.</p> <p>#2 - Please confirm that 1 test is acceptable for each threat category when performing physical testing for the armor.</p> <p>#3 - Please confirm that 2 months advance notice is acceptable prior to performing physical testing of the armor. Physical testing will be performed at VSL's facility in Pocomoke, MD.</p>					
	<p>Contract Doc Ref: 05 16 34 Location: Bus Ramp</p> <p>The following questions pertain to the Bus Ramp Stay Cable Bridge:</p> <p>#1 - Please confirm that unstressed strands are acceptable when performing physical testing of the armor.</p> <p>#2 - Please confirm that 1 test is acceptable for each threat category when performing physical testing for the armor.</p> <p>#3 - Please confirm that 2 months advance notice is acceptable prior to performing physical testing of the armor. Physical testing will be performed at VSL's facility in Pocomoke, MD.</p>					
<b>B-0154</b>	<b>BRP - Bollard Quantity Clarification</b>	<b>Closed</b>	<b>01</b>	<b>12/22/2014</b>	<b>01/01/2015</b>	<b>01/12/2015</b>
	<p><b>From:</b> Webcor Construction LP      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: MA-4 (Sheet 343) Location: Bus Ramp</p> <p>Per contract Drawing MA-4 (Sheet 343) there are 7 bollards to install. SCCI is only able to locate 4 bollards on this sheet.</p> <p>Please confirm that there are only 4 bollards to furnish and install per contract drawing MA-4.</p>					
	<p><b>ANSWER:</b></p> <p>Contract Doc Ref: MA-4 (Sheet 343) Location: Bus Ramp</p> <p>Per contract Drawing MA-4 (Sheet 343) there are 7 bollards to install. SCCI is only able to locate 4 bollards on this sheet.</p> <p>Please confirm that there are only 4 bollards to furnish and install per contract drawing MA-4.</p>					



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<b>B-0156</b>	<b>BRP - Stay Cable Weight Included in Stay Cable Catenary</b>	<b>Open</b>	<b>01</b>	<b>12/22/2014</b>	<b>01/01/2015</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: 05 16 34 Location: Bus Ramp Add'l Doc Ref's: VSL - Trade Subcontractor RFI (Attached)			Contract Doc Ref: 05 16 34 Location: Bus Ramp Add'l Doc Ref's: VSL - Trade Subcontractor RFI (Attached)			
The amount of load transferred into the stay cable by the armor is 2,350 lbs. The stay pipe armor is supported by a collar that clamps on to the strand bundle. The stay pipe armor is approximately 6.6 ft long and weighs 1,630 lbs. The fire pipe sits on top of the stay pipe armor. The fire pipe is approximately 16 ft long and weighs 720 lbs.			The amount of load transferred into the stay cable by the armor is 2,350 lbs. The stay pipe armor is supported by a collar that clamps on to the strand bundle. The stay pipe armor is approximately 6.6 ft long and weighs 1,630 lbs. The fire pipe sits on top of the stay pipe armor. The fire pipe is approximately 16 ft long and weighs 720 lbs.			
VSL is requesting clarification from the designer on the following: Please confirm the weight of the armor has been included in the stay cable catenary.			VSL is requesting clarification from the designer on the following: Please confirm the weight of the armor has been included in the stay cable catenary.			
Please note that this has been coordinated with the OPAC.			Please note that this has been coordinated with the OPAC.			
<b>B-0157</b>	<b>BRP - Proposed Telecommunication Line Relocation Location</b>	<b>Open</b>	<b>01</b>	<b>12/22/2014</b>	<b>01/01/2015</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: U-3001 & U-3003 Location: Bus Ramp Add'l Doc Ref's: VSL - Attached Sketch			Contract Doc Ref: U-3001 & U-3003 Location: Bus Ramp Add'l Doc Ref's: VSL - Attached Sketch			
Per SCCI field walk with TCCO and AT&T on 12/1 8/ 14, a location has been selected for the new telecommunication line south of Harrison. The location was approved by AT&T in the field and differs slightly from contract drawings U-3001 and U-3003. Please reference the attached sketch for the proposed location.			Per SCCI field walk with TCCO and AT&T on 12/1 8/ 14, a location has been selected for the new telecommunication line south of Harrison. The location was approved by AT&T in the field and differs slightly from contract drawings U-3001 and U-3003. Please reference the attached sketch for the proposed location.			
Is the proposed telecommunication line relocation location acceptable?			Is the proposed telecommunication line relocation location acceptable?			



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<b>B-0158</b>	<b>BRP - No Parking Pavement Detail Clarification</b>	<b>Closed</b>	<b>01</b>	<b>12/22/2014</b>	<b>01/01/2015</b>	<b>01/09/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: 32 17 28, C-7000 Location: Bus Ramp  Contract Drawing C-7000 calls for painted lines outside of the CalTrans substation in the no parking area but the actual "NO PARKING" pavement marking is not called out (see attached). Please confirm the "NO PARKING" pavement marking is to be paint, not thermoplastic.		<b>ANSWER:</b>  Contract Doc Ref: 32 17 28, C-7000 Location: Bus Ramp  Contract Drawing C-7000 calls for painted lines outside of the CalTrans substation in the no parking area but the actual "NO PARKING" pavement marking is not called out (see attached). Please confirm the "NO PARKING" pavement marking is to be paint, not thermoplastic.				
<b>B-0159</b>	<b>BRP - Striping Detail 40 on C-7002</b>	<b>Closed</b>	<b>01</b>	<b>12/23/2014</b>	<b>01/02/2015</b>	<b>01/26/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: C-7002 Specification Section 32 17 28  Location: Bus Ramp  Add'l Doc Ref's: N/A  Please confirm the striping highlighted on the attached contract drawing, C-7002, is striping detail #40.		<b>ANSWER:</b>  Contract Doc Ref: C-7002 Specification Section 32 17 28  Location: Bus Ramp  Add'l Doc Ref's: N/A  Please confirm the striping highlighted on the attached contract drawing, C-7002, is striping detail #40.				
<b>B-0160</b>	<b>BRP - CR B-002 Additional Footing clarification-1</b>	<b>Closed</b>	<b>01</b>	<b>12/29/2014</b>	<b>12/29/2014</b>	<b>01/09/2015</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  Contract Doc Ref: D-1004 Locatoion: Bus Ramp, Bent 8 Gridline: N/A Add'l Doc Ref's: CR B-002 TG18.1 Additional Footing, PCI#310293  RFI is for reviewing of scope and quantities for CR B-002 TG18.1 Additional Footing, PCI#310293  Per D-1004 Rev X, SCCI is to demolish the top 5' of		<b>ANSWER:</b>  Contract Doc Ref: D-1004 Locatoion: Bus Ramp, Bent 8 Gridline: N/A Add'l Doc Ref's: CR B-002 TG18.1 Additional Footing, PCI#310293  RFI is for reviewing of scope and quantities for CR B-002 TG18.1 Additional Footing, PCI#310293  Per D-1004 Rev X, SCCI is to demolish the top 5' of				



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	<div>existing Bent 8. Per reference document San Francisco-Oakland Bridge Railway Facilities (aka 1937 drawings), the details of this Bent are on sheet 24A, which is missing.</div> <div>Provide the missing sheet.</div>					<div>existing Bent 8. Per reference document San Francisco-Oakland Bridge Railway Facilities (aka 1937 drawings), the details of this Bent are on sheet 24A, which is missing.</div> <div>Provide the missing sheet.</div>
B-0161	BRP - CR B-002 Additional Footing clarification-2	Closed	CR	12/29/2014	01/08/2015	01/09/2015
<div>From: Webcor Construction LP</div> <div>Stephanie Azzolino</div>						
REQUEST:			ANSWER:			
<div>Contract Doc Ref: D-1004</div> <div>Locatoion: Bus Ramp, Bent 7</div> <div>Gridline: N/A</div> <div>Add'l Doc Ref's: CR B-002 TG18.1 Additional Footing, PCI#310293</div> <div>RFI is for reviewing of scope and quantities for CR B-002 TG18.1 Additional Footing, PCI#310293</div> <div>Per D-1004 Rev X, SCCI is to demolish completely an unnamed existing foundation just east of existing Bent 7. Per reference document D1 San Francisco-Oakland Bridge Railway Facilities (aka 1937 drawings), this foundation appears to be called 224 and appears to pre-date these as-built.</div> <div>a. Please provide as-built information.</div> <div>b. Confirm that only the top 5' similar to the surrounding foundations is to be demolished.</div>			<div>Contract Doc Ref: D-1004</div> <div>Locatoion: Bus Ramp, Bent 7</div> <div>Gridline: N/A</div> <div>Add'l Doc Ref's: CR B-002 TG18.1 Additional Footing, PCI#310293</div> <div>RFI is for reviewing of scope and quantities for CR B-002 TG18.1 Additional Footing, PCI#310293</div> <div>Per D-1004 Rev X, SCCI is to demolish completely an unnamed existing foundation just east of existing Bent 7. Per reference document D1 San Francisco-Oakland Bridge Railway Facilities (aka 1937 drawings), this foundation appears to be called 224 and appears to pre-date these as-built.</div> <div>a. Please provide as-built information.</div> <div>b. Confirm that only the top 5' similar to the surrounding foundations is to be demolished.</div>			





**ANSWER:**

Contract Doc Ref: D-1005, D-1034, Q/D-1107  
Locatoion: Bus Ramp, Bent 22  
Gridline: N/A  
Add'l Doc Ref's: CR B-002 TG18.1 Additional Footing, PCI#310293

RFI is for reviewing of scope and quantities for CR B-002 TG18.1 Additional Footing, PCI#310293

Per D-1005 Rev X, SCCI is to demo the top 5' of existing Bent 22. It is shown as only a footing, no columns, but per Note 2 that refers you to Q/D1107, we should expect only column demolition. Per reference document Transbay Terminal & Ramps Demolition As-Built (aka 2011 drawings) D-1034, the demo limits should have been 3' below grade, leaving the footing and some column remaining.

Confirm only column demolition will be required.



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<b>B-0163</b>	<b>BRP - CR B-002 Additional Footing clarification-4</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2014</b>	<b>12/30/2014</b>	<b>01/09/2015</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: D-1008 Locatoion: Bus Ramp Gridline: N/A Add'l Doc Ref's: CR B-002 TG18.1 Additional Footing, PCI#310293  The purpose of this RFI is for reviewing of scope and quantities for CR B-002 TG18.1 Additional Footing, PCI#310293  Per D-1008 Rev X, SCCI is to demolish the top 6' of the existing wall footing along Clementina. Reviewing all of the reference documents, there does not appear to be an as- built of this drawing or anything scalable.  Does any information on the Provide as-built information for the wall footing exist? This is important information as the wall footing is very close and in between the columns of the in-service Fremont Street off-ramp and Clementina Street.			Contract Doc Ref: D-1008 Locatoion: Bus Ramp Gridline: N/A Add'l Doc Ref's: CR B-002 TG18.1 Additional Footing, PCI#310293  The purpose of this RFI is for reviewing of scope and quantities for CR B-002 TG18.1 Additional Footing, PCI#310293  Per D-1008 Rev X, SCCI is to demolish the top 6' of the existing wall footing along Clementina. Reviewing all of the reference documents, there does not appear to be an as-built of this drawing or anything scalable.  Does any information on the Provide as-built information for the wall footing exist? This is important information as the wall footing is very close and in between the columns of the in-service Fremont Street off-ramp and Clementina Street.			
<b>B-0164</b>	<b>BRP - CR B-002 Additional Footing clarification-5</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2014</b>	<b>01/09/2015</b>	<b>01/09/2015</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: D-1005, D-1008 Locatoion: Bus Ramp Gridline: N/A Add'l Doc Ref's: for CR B-002 TG18.1 Additional Footing, PCI#310293  RFI is for reviewing of scope and quantities for CR B-002 TG18.1 Additional Footing, PCI#310293  Per Note 3 on D-1005 Rev X & D-1008 Rev X, SCCI is to "Remove AC paving within entire under-ramp park boundary."  Provide the boundaries of the under-ramp park.			Contract Doc Ref: D-1005, D-1008 Locatoion: Bus Ramp Gridline: N/A Add'l Doc Ref's: for CR B-002 TG18.1 Additional Footing, PCI#310293  RFI is for reviewing of scope and quantities for CR B- 002 TG18.1 Additional Footing, PCI#310293  Per Note 3 on D-1005 Rev X & D-1008 Rev X, SCCI is to "Remove AC paving within entire under-ramp park boundary."  Provide the boundaries of the under-ramp park.			





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<b>B-0166</b>	<b>BRP - Mass Concrete Temperature Monitoring Equipment Installation</b>	<b>Closed</b>	<b>01</b>	<b>01/05/2015</b>	<b>01/15/2015</b>	<b>01/26/2015</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: Specification 037006-1.3B Location: N/A Grid Line: N/A Add'l Doc Ref's: Attached sketch						Contract Doc Ref: Specification 037006-1.3B Location: N/A Grid Line: N/A Add'l Doc Ref's: Attached sketch
Per Specification 03 70 06-1 .3B, SCCI will install temperature monitoring devices at specified locations and depths to be selected in the thermal control plan. These instruments use RFID Tag technology for communication with the data logger. The RFID transmitter, which is wired to the temperature monitoring device will be elevated out of the concrete. SCCI will tie a 1 /4" diameter fiberglass, or similar non-corrosive, rod to the reinforcing mat, the temperature monitoring RFID transmitter will then be elevated clear of the concrete. Once thermal monitoring activities are complete, this non-corrosive rod and cable will be cut flush with the concrete (same procedure as below grade concrete package). Reference attached brochure and SCCI sketch. Is this method acceptable?						Per Specification 03 70 06-1 .3B, SCCI will install temperature monitoring devices at specified locations and depths to be selected in the thermal control plan. These instruments use RFID Tag technology for communication with the data logger. The RFID transmitter, which is wired to the temperature monitoring device will be elevated out of the concrete. SCCI will tie a 1 /4" diameter fiberglass, or similar non-corrosive, rod to the reinforcing mat, the temperature monitoring RFID transmitter will then be elevated clear of the concrete. Once thermal monitoring activities are complete, this non-corrosive rod and cable will be cut flush with the concrete (same procedure as below grade concrete package). Reference attached brochure and SCCI sketch. Is this method acceptable?
<b>B-0167</b>	<b>BRP - Pylon 9 #6 Cross Tie Spacing</b>	<b>Closed</b>	<b>01</b>	<b>01/05/2015</b>	<b>01/15/2015</b>	<b>01/09/2015</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: S-3178, S-3179 Location: Pylon 9 Grid Line: N/A Add'l Doc Ref's: See attached						Contract Doc Ref: S-3178, S-3179 Location: Pylon 9 Grid Line: N/A Add'l Doc Ref's: See attached
Reference attached sections B-D on S-3178 and F on S-3179. SCCI and CMC have reviewed the drawings and were not able to identify the spacing of the #6 cross ties.  Please provide spacing for the #6 cross ties shown in the referenced sections.						Reference attached sections B-D on S-3178 and F on S-3179. SCCI and CMC have reviewed the drawings and were not able to identify the spacing of the #6 cross ties.  Please provide spacing for the #6 cross ties shown in the referenced sections.
<b>B-0168</b>	<b>BRP - Pylon 9 Limits of Sections</b>	<b>Closed</b>	<b>01</b>	<b>01/05/2015</b>	<b>01/15/2015</b>	<b>01/08/2015</b>



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<p><b>From:</b> Webcor Construction LP</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: S-3177, S-3179  Location: Pylon 9  Grid Line: N/A  Add'l Doc Ref's: See attached</p> <p>Reference S-3177 thru S-3179.</p> <p>Please advise if it is acceptable to use Section B/S-3178 in lieu of A/S-3178 for the bottom 24'-0" if Pylon 9.</p> <p>This will facilitate the assembly of the cage.</p>	<p>Claude Titche</p>					<p><b>ANSWER:</b></p> <p>Contract Doc Ref: S-3177, S-3179  Location: Pylon 9  Grid Line: N/A  Add'l Doc Ref's: See attached</p> <p>Reference S-3177 thru S-3179.</p> <p>Please advise if it is acceptable to use Section B/S-3178 in lieu of A/S-3178 for the bottom 24'-0" if Pylon 9.</p> <p>This will facilitate the assembly of the cage.</p>
<b>B-0169</b>	<b>BRP - CalTrans Lot Landscaping and Irrigation As Builts with WOJV Comments</b>	<b>Open</b>	<b>01</b>	<b>01/06/2015</b>	<b>01/16/2015</b>	
<p><b>From:</b> Webcor Construction LP</p> <p><b>REQUEST:</b></p> <p>Location: Bus Ramp  Add'l Doc Ref's: WOJV email with attachments (Attached)</p> <p>WOJV has provided three plan sheets for the landscaping and irrigation of the Caltrans lot. SCCI requires a full set of the as-builts and specifications for this work in order to replace any disturbed landscape in kind. The sheets currently provided are mostly noted in abbreviations and diagrams, of which no key or legend is provided.</p> <p>Please provide a full set of as-builts and specifications for this work.</p> <p>WOJV Comment: The three plan sheets were provided by TJPA on 8/22/14. See attached email and drawings.</p>	<p>Andrew Kitchen</p>					<p><b>ANSWER:</b></p> <p>Location: Bus Ramp  Add'l Doc Ref's: WOJV email with attachments (Attached)</p> <p>WOJV has provided three plan sheets for the landscaping and irrigation of the Caltrans lot. SCCI requires a full set of the as-builts and specifications for this work in order to replace any disturbed landscape in kind. The sheets currently provided are mostly noted in abbreviations and diagrams, of which no key or legend is provided.</p> <p>Please provide a full set of as-builts and specifications for this work.</p> <p>WOJV Comment: The three plan sheets were provided by TJPA on 8/22/14. See attached email and drawings.</p>



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B-0170	BRP - Bent 8 Reinforcement Footing Vertical Ties	Closed	01	01/08/2015	01/18/2015	02/03/2015
From: Webcor Construction LP      Andrew Kitchen						
REQUEST:		ANSWER:				
Contract Doc Ref: 31 63 32 Location: Bus Ramp		Contract Doc Ref: 31 63 32 Location: Bus Ramp				
The footing section calls for #6 @6 each way vertical ties with a 90 degree at bottom. With the spacing of the typical (main) reinforcement at 6" OC and the hook length being 1'-0" long this presents a congestion and constructability issue. One solution would be to use an HRC-555 head at the bottom of these vertical ties. Is this acceptable?		The footing section calls for #6 @6 each way vertical ties with a 90 degree at bottom. With the spacing of the typical (main) reinforcement at 6" OC and the hook length being 1'-0" long this presents a congestion and constructability issue. One solution would be to use an HRC-555 head at the bottom of these vertical ties. Is this acceptable?				
Please advise.		Please advise.				
B-0172	BRP - Bent 8 CIDH Non-Stagger Due to Odd Number of Bars	Closed	01	01/09/2015	01/19/2015	01/26/2015
From: Webcor Construction LP      Andrew Kitchen						
REQUEST:		ANSWER:				
Contract Doc Ref: 31 63 32, S-3190 Location: Bus Ramp Add'l doc ref: Attached sketch and RFI B-0145		Contract Doc Ref: 31 63 32, S-3190 Location: Bus Ramp Add'l doc ref: Attached sketch and RFI B-0145				
Please verify that in CIDH cages B8-1 thru B8-4, it is acceptable, due to the odd number of vertical bars (29), that (2) splices will occur adjacent to each other and not staggered. The attached drawing depicts the bars and the staggering.		Please verify that in CIDH cages B8-1 thru B8-4, it is acceptable, due to the odd number of vertical bars (29), that (2) splices will occur adjacent to each other and not staggered. The attached drawing depicts the bars and the staggering.				
B-0173	BRP - Fremont St. Off Ramp CIDH Permanent Casing Diameter	Open	01	01/12/2015	01/22/2015	
From: Webcor Construction LP      Andrew Kitchen						
REQUEST:		ANSWER:				
Contract Doc Ref: 31 63 30 Location: Bus Ramp		Contract Doc Ref: 31 63 30 Location: Bus Ramp				
The Elevation View on Contract Drawing S-3100 shows that 12'-0" permanent casings are to be installed below the cut-off elevation. Section F in Contract Drawing S-3102 shows that this permanent steel casing shall have an inner diameter of 7'-0" which matches the diameter of the CIDH pile. Per Case Pacific, the steel casing will be damaged by drilling tools if it is not oversized. Is it acceptable to use an		The Elevation View on Contract Drawing S-3100 shows that 12'-0" permanent casings are to be installed below the cut-off elevation. Section F in Contract Drawing S-3102 shows that this permanent steel casing shall have an inner diameter of 7'-0" which matches the diameter of the CIDH pile. Per Case Pacific, the steel casing will be damaged by				



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B-0174	8'-0" diameter permanent casing to facilitate drilling operations?					drilling tools if it is not oversized. Is it acceptable to use an 8'-0" diameter permanent casing to facilitate drilling operations?
	<b>BRP - Fremont St. Off Ramp CIDH Permanent Casing Material</b>  From: Webcor Construction LP      Andrew Kitchen  <b>REQUEST:</b> Contract Doc Ref: 31 63 30 Location: Bus Ramp  The Elevation View on Contract Drawing S-3100 shows that 12'-0" permanent casings are to be installed below the cut-off elevation. Section F on Contract Drawing S-3102 calls for this to be a "permanent steel casing". Case Pacific is proposing to use 12 gage corrugated metal pipe as this permanent steel casing. CMP meets the requirements for Permanent Steel Casing in the Caltrans Standard Specifications Section 49-3.02C(5).  Please confirm if this is acceptable.	Open	01	01/12/2015	01/22/2015	<b>ANSWER:</b> Contract Doc Ref: 31 63 30 Location: Bus Ramp  The Elevation View on Contract Drawing S-3100 shows that 12'-0" permanent casings are to be installed below the cut-off elevation. Section F on Contract Drawing S-3102 calls for this to be a "permanent steel casing". Case Pacific is proposing to use 12 gage corrugated metal pipe as this permanent steel casing. CMP meets the requirements for Permanent Steel Casing in the Caltrans Standard Specifications Section 49-3.02C(5).  Please confirm if this is acceptable.
B-0175	BRP - Tire Spike Location					
	From: Webcor Construction LP      Andrew Kitchen  <b>REQUEST:</b> Contract Doc Ref: C-2100, 11 12 06 Location: Bus Ramp  Drawing C-2100 calls out two locations of tire spikes. It is SCCI's understanding that the tire spikes are located at the barrier gate entrance under the arm only, and not at the second location (please see attached drawings). Please confirm this is correct or please provide a detail of the second tire spike location.	Closed	01	01/13/2015	01/23/2015	01/26/2015  <b>ANSWER:</b> Contract Doc Ref: C-2100, 11 12 06 Location: Bus Ramp  Drawing C-2100 calls out two locations of tire spikes. It is SCCI's understanding that the tire spikes are located at the barrier gate entrance under the arm only, and not at the second location (please see attached drawings). Please confirm this is correct or please provide a detail of the second tire spike location.



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<b>B-0176</b>	<b>BRP - AWSS Reinforced Concrete Pad</b>	<b>Closed</b>	<b>01</b>	<b>01/14/2015</b>	<b>01/24/2015</b>	<b>01/20/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: MA-4 Note 6, AWSS 03200 Location: Bus Ramp Other Doc Ref: Attached SFDWP Standard reinforced concrete bus pad detail  Per contract drawing MA-4, Note 6, a Civil Engineer Licensed in the State of California is to provide rebar shop drawings of the reinforced concrete pad located at the new AWSS hydrant. SCCI's AWSS Subcontractor, Synergy Project Management, is proposing to use the SFDPW standard reinforced concrete bus pad detail for the new 10' x 10' reinforced pad (please see attached). As this is a standard detail it does not require a Licensed Engineer's stamp. Please advise if this is acceptable.  Please note - This RFI pertains to the SFDPW contract drawings.						<b>ANSWER:</b>  Contract Doc Ref: MA-4 Note 6, AWSS 03200 Location: Bus Ramp Other Doc Ref: Attached SFDWP Standard reinforced concrete bus pad detail  Per contract drawing MA-4, Note 6, a Civil Engineer Licensed in the State of California is to provide rebar shop drawings of the reinforced concrete pad located at the new AWSS hydrant. SCCI's AWSS Subcontractor, Synergy Project Management, is proposing to use the SFDPW standard reinforced concrete bus pad detail for the new 10' x 10' reinforced pad (please see attached). As this is a standard detail it does not require a Licensed Engineer's stamp. Please advise if this is acceptable.  Please note - This RFI pertains to the SFDPW contract drawings.
<b>B-0176.1</b>	<b>BRP - AWSS Reinforced Concrete Pad</b>	<b>Closed</b>	<b>01</b>	<b>01/20/2015</b>	<b>01/30/2015</b>	<b>01/27/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: AWSS 03200, MA-4, B-0176.1 Location: Bus Ramp  Further to the response to RFI B-0176, the AWSS contract drawings do not specify a design criteria for the 10' x 10' by 1' thick pad; an engineer will need to know what type of loading will be encountered in order to design the pad. Is this pad to be traffic rated for H-20 loading or just for pedestrian usage (seeing as there are x4 bollards surrounding the concrete pad).  Please provide the design criteria for the concrete pad and what type of loading will be placed on the concrete pad that is surrounded by the x4 bollards OR approve the city standard detail for bus pads as these pads are designed for traffic rating and are reinforced.						<b>ANSWER:</b>  Contract Doc Ref: AWSS 03200, MA-4, B-0176.1 Location: Bus Ramp  Further to the response to RFI B-0176, the AWSS contract drawings do not specify a design criteria for the 10' x 10' by 1' thick pad; an engineer will need to know what type of loading will be encountered in order to design the pad. Is this pad to be traffic rated for H-20 loading or just for pedestrian usage (seeing as there are x4 bollards surrounding the concrete pad).  Please provide the design criteria for the concrete pad and what type of loading will be placed on the concrete pad that is surrounded by the x4 bollards OR approve the city standard detail for bus pads as these pads are designed for traffic rating and are reinforced.





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<b>B-0177</b>	<b>BRP - Bent 8 Tiedown Clarification</b>	<b>Closed</b>	<b>01</b>	<b>01/15/2015</b>	<b>01/25/2015</b>	<b>02/03/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: S-6067, 03 30 06 Location: Bus Ramp		Contract Doc Ref: S-6067, 03 30 06 Location: Bus Ramp				
Data table on referenced drawings depicts a temporary tiedown at hinge 8 on the ¿seat¿ portion of the hinge. Based on the construction sequence shown in the contract drawings and current TG18.1 schedule, SCCI believes that this tiedown at Hinge 8 is not needed. It's SCCI interpretation that temporary tiedown at hinge 8 would be needed only if the frame 4 is constructed independently without frame 3 and hinge 8 to counter the "curling" effect of the frame 4 on bent 8, once falsework on frame 4 is removed. If frame 3/hinge 8 is constructed prior to completion of frame 4, as currently shown on the schedule, the tiedown at hinge 8 would not be required. Please confirm.		Data table on referenced drawings depicts a temporary tiedown at hinge 8 on the ¿seat¿ portion of the hinge. Based on the construction sequence shown in the contract drawings and current TG18.1 schedule, SCCI believes that this tiedown at Hinge 8 is not needed. It's SCCI interpretation that temporary tiedown at hinge 8 would be needed only if the frame 4 is constructed independently without frame 3 and hinge 8 to counter the "curling" effect of the frame 4 on bent 8, once falsework on frame 4 is removed. If frame 3/hinge 8 is constructed prior to completion of frame 4, as currently shown on the schedule, the tiedown at hinge 8 would not be required. Please confirm.				
<b>B-0178</b>	<b>BRP - Pylon 9 Rebar Cage Support</b>	<b>Closed</b>	<b>01</b>	<b>01/21/2015</b>	<b>01/31/2015</b>	<b>02/10/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: 03 30 11 Section 1.2.D.8 Location: Bus Ramp Add'l Doc Ref: Attached schematic, WOJV Email, ACI 318 Section 5.7		Contract Doc Ref: 03 30 11 Section 1.2.D.8 Location: Bus Ramp Add'l Doc Ref: Attached schematic, WOJV Email, ACI 318 Section 5.7				
The construction of Pylon 9 will entail placing the pile cap and pylon rebar on top of the existing sub grade using standard concrete dobies, SCCI expects unwanted settlement if only concrete dobies are used. To prevent this SCCI proposes to place 8" x 8" x 3/4" plywood sheets between the dobies and the subgrade. See attached schematic for reference.		The construction of Pylon 9 will entail placing the pile cap and pylon rebar on top of the existing sub grade using standard concrete dobies, SCCI expects unwanted settlement if only concrete dobies are used. To prevent this SCCI proposes to place 8" x 8" x 3/4" plywood sheets between the dobies and the subgrade. See attached schematic for reference.				
Is this acceptable?		Is this acceptable?				
WOJV Comment: See attached email thread between WOJV and SCCI		WOJV Comment: See attached email thread between WOJV and SCCI				



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<b>B-0179</b>	<b>BRP - Columns 3-2 and 5-1 Drain Outlets</b>	<b>Closed</b>	<b>01</b>	<b>01/19/2015</b>	<b>01/29/2015</b>	<b>02/10/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: S-2160, C-4102 Location: Bus Ramp			Contract Doc Ref: S-2160, C-4102 Location: Bus Ramp			
Per contract drawing S-2160, the deck drain pipe flows to the top of columns 3-1, 3-2, 5-1, and 5-2. Per contract drawing C-4102 the deck drain pipe only exits from columns 3-1 and 5-2.			Per contract drawing S-2160, the deck drain pipe flows to the top of columns 3-1, 3-2, 5-1, and 5-2. Per contract drawing C-4102 the deck drain pipe only exits from columns 3-1 and 5-2.			
Please clarify where the deck drain pipes flow to from the tops of columns 3-2 and 5-1.			Please clarify where the deck drain pipes flow to from the tops of columns 3-2 and 5-1.			





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<b>B-0182</b>	<b>BRP - CB #12 Down Drain Pipe Clarification</b>	<b>Closed</b>	<b>01</b>	<b>01/21/2015</b>	<b>01/31/2015</b>	<b>01/30/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: S-3215, C-4302, 33 46 06 Location: Bus Ramp  Contract drawing C-4302 says that the down drain pipe beneath CB #12 is per MSE wall drainage detail. Per MSE wall drainage inlet at RW10 detail on sheet S-3215, no pipe type is called out. SCCI intends to use a 3" to 4" PVC connection as is shown on sheet S-3214.  Is this acceptable?		<b>ANSWER:</b>  Contract Doc Ref: S-3215, C-4302, 33 46 06 Location: Bus Ramp  Contract drawing C-4302 says that the down drain pipe beneath CB #12 is per MSE wall drainage detail. Per MSE wall drainage inlet at RW10 detail on sheet S-3215, no pipe type is called out. SCCI intends to use a 3" to 4" PVC connection as is shown on sheet S-3214.  Is this acceptable?				
<b>B-0183</b>	<b>BRP - Headed Rebar in Pylon 9 Barrette Pile Cage</b>	<b>Closed</b>	<b>01</b>	<b>01/26/2015</b>	<b>02/05/2015</b>	<b>02/02/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: S-3191, TG1801-346.1, B-0055 Location: Bus Ramp  Per RFI B-0055 ERP-RESPONSE- Barrette Pile Reinforcement Cage, headed rebar are an acceptable equivalent to the standard hooks detailed at the top of the longitudinal bars of drawing S-3191. Submittal TG 1801-346.1 - Barrette Pile Reinforcement Shop Drawings approved shop drawing of barrette piles indicate the use of the HRC-555 anchor head. The HRC-555 proposed and installed is an equivalent of standard hooks that meets Section 12.6 of ACI 318-08 (please refer to the technical documentation attached).  Please confirm the HRC-555s installed at the top of the Barrettes are acceptable.		<b>ANSWER:</b>  Contract Doc Ref: S-3191, TG1801-346.1, B-0055 Location: Bus Ramp  Per RFI B-0055 ERP-RESPONSE- Barrette Pile Reinforcement Cage, headed rebar are an acceptable equivalent to the standard hooks detailed at the top of the longitudinal bars of drawing S-3191. Submittal TG 1801-346.1 - Barrette Pile Reinforcement Shop Drawings approved shop drawing of barrette piles indicate the use of the HRC-555 anchor head. The HRC-555 proposed and installed is an equivalent of standard hooks that meets Section 12.6 of ACI 318-08 (please refer to the technical documentation attached).  Please confirm the HRC-555s installed at the top of the Barrettes are acceptable.				
<b>B-0184</b>	<b>BRP - Hinge 9 Seismic Expansion Joint</b>	<b>Closed</b>	<b>01</b>	<b>01/23/2015</b>	<b>02/02/2015</b>	<b>02/02/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: S-6283, 03 15 06 Location: Bus Ramp  See attached sheet for section view and movement rating		<b>ANSWER:</b>  Contract Doc Ref: S-6283, 03 15 06 Location: Bus Ramp  See attached sheet for section view and movement				



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	for proposed Watson Bowman Acme seismic expansion joint. Please confirm the attached joint will be acceptable for the hinge 9 expansion joint.					rating for proposed Watson Bowman Acme seismic expansion joint. Please confirm the attached joint will be acceptable for the hinge 9 expansion joint.
B-0185	BRP - TTC Foundation Stiffness for Cable Stayed Bridge	Open	01	01/26/2015	02/05/2015	
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
Contract Doc Ref: 05 16 33, TG1801-521 Location: Bus Ramp						Contract Doc Ref: 05 16 33, TG1801-521 Location: Bus Ramp
In Comments to submittal TG1801-521, Designers asked that foundation stiffness be incorporated into the stage-by-stage erection model. Can Design team please provide SCCI/OPAC with the cable-stayed bridge geotechnical report with the assumed foundation stiffness?						In Comments to submittal TG1801-521, Designers asked that foundation stiffness be incorporated into the stage-by-stage erection model. Can Design team please provide SCCI/OPAC with the cable-stayed bridge geotechnical report with the assumed foundation stiffness?
B-0186	BRP - Viaduct Prestressing Force Coefficient	Open	01	01/29/2015	02/08/2015	
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
Contract Doc Ref: S-6065 Prestressing notes Location: Bus Ramp						Contract Doc Ref: S-6065 Prestressing notes Location: Bus Ramp
The force coefficient at the point of no movement is given as 0.850 for all girders. This value is directly dependent on the length of the tendons, number of spans, and tendon profile, and cannot be the same for all the tendons.						The force coefficient at the point of no movement is given as 0.850 for all girders. This value is directly dependent on the length of the tendons, number of spans, and tendon profile, and cannot be the same for all the tendons.
Please provide a correct value for the force coefficient for individual tendons.						Please provide a correct value for the force coefficient for individual tendons.



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B-0187	BRP - Viaduct Box Girder Dimension	Open	01	01/28/2015	02/07/2015	
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:			ANSWER:			
Contract Doc Ref: S-2061 and S-2062 Location: Bus Ramp Frame 3			Contract Doc Ref: S-2061 and S-2062 Location: Bus Ramp Frame 3			
Contract Drawing S-2061 shows sections of the viaduct box girder. Section C on this drawing contains a dimension which appears to show that the distance from the top of deck to the bottom of soffit is 5'-6". Section D and E on this drawing are less clear as to where this dimension is applied to. Note 2 on Contract Drawing S-2062, states that height of this dimension varies between Bent 7 and Hinge 8.			Contract Drawing S-2061 shows sections of the viaduct box girder. Section C on this drawing contains a dimension which appears to show that the distance from the top of deck to the bottom of soffit is 5'-6". Section D and E on this drawing are less clear as to where this dimension is applied to. Note 2 on Contract Drawing S-2062, states that height of this dimension varies between Bent 7 and Hinge 8.			
Please confirm that this dimension is intended to show the distance from top of deck to bottom of soffit.			Please confirm that this dimension is intended to show the distance from top of deck to bottom of soffit.			





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<b>B-0190</b>	<b>BRP - Conduits at Pylon 9 Base</b>	<b>Open</b>	<b>01</b>	<b>02/02/2015</b>	<b>02/12/2015</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: E-5007 Detail A Location: Bus Ramp						Contract Doc Ref: E-5007 Detail A Location: Bus Ramp
CD A/E-5007 calls out for 6 ea (5x4" and 1x2" dia.), and 5 ea. (3x4" and 2x2" dia.) conduits coming out of West and East side of the pylon column, respectively. These conduits are coming from the racks that runs down the bridge girders, which implies that conduits will have 3 ea. 90 degree bends in them between bridge girders and exit point at the base of pylon 9. Due to the code requirements and coordination with the other trade packages, SCCI believes that these conduits need to be stubbed out at a specific elevation and orientation. Reference documents provided to SCCI do not have necessary information to establish these parameters.						CD A/E-5007 calls out for 6 ea (5x4" and 1x2" dia.), and 5 ea. (3x4" and 2x2" dia.) conduits coming out of West and East side of the pylon column, respectively. These conduits are coming from the racks that runs down the bridge girders, which implies that conduits will have 3 ea. 90 degree bends in them between bridge girders and exit point at the base of pylon 9. Due to the code requirements and coordination with the other trade packages, SCCI believes that these conduits need to be stubbed out at a specific elevation and orientation. Reference documents provided to SCCI do not have necessary information to establish these parameters.
Please provide details with specific elevation and orientation for each conduit coming out of the pylon 9. These conduits are depicted on noted A/E-5007.						Please provide details with specific elevation and orientation for each conduit coming out of the pylon 9. These conduits are depicted on noted A/E-5007.
<b>B-0191</b>	<b>BRP - Bent 8 Reinforcement Cages</b>	<b>Closed</b>	<b>01</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	<b>02/06/2015</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract doc Ref: TG1801-345.1 Location: Lot G						Contract doc Ref: TG1801-345.1 Location: Lot G
Per Arup's comments in regards to the length of the Bent 8 CIDH reinforcement cages shown in Submittal TGI801-345.I - CIDH Reinforcement Cage Shop Drawings, the overall length of the cages should be 153'-9 1/2". The reinforcement cage for CIDH 8-1 was delivered to the site with an overall length of 150'-0". Upon review with CMC Rebar, it was confirmed that all four Bent 8 CIDH reinforcement cages were fabricated to this length. SCCI proposes to correct this issue by installing a lap splice per the attached drawing.						Per Arup's comments in regards to the length of the Bent 8 CIDH reinforcement cages shown in Submittal TGI801-345.I - CIDH Reinforcement Cage Shop Drawings, the overall length of the cages should be 153'-9 1/2". The reinforcement cage for CIDH 8-1 was delivered to the site with an overall length of 150'-0". Upon review with CMC Rebar, it was confirmed that all four Bent 8 CIDH reinforcement cages were fabricated to this length. SCCI proposes to correct this issue by installing a lap splice per the attached drawing.
Is this acceptable?						Is this acceptable?





<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
BALFO900-0001	BSE Natoma Street Trestle Access	Closed	01	04/18/2011	05/02/2011	04/20/2011
<b>From:</b> Balfour Beatty Infrastructure, Inc. Ural Yal						
<b>REQUEST:</b> Reference Project Bidding Manual (Exhibit A)  Per the requirements outlined in the project bidding manual (Exhibit A), BBII has developed our trestle design to provide access for Natoma street extending from gridline 11.5 at the center of the excavation (grid line E) to gridline 10 at the centerline of the shoring wall. After staking out this point on the shoring wall, it is apparent that the 530 Howard St. building is in conflict with the access point. See the attached sketch and photos indicating the approximate location of 530 Howard in relation to the trestle access. Please advise if the Natoma St. access point should be changed to a more suitable location.		<b>ANSWER:</b> Reference Project Bidding Manual (Exhibit A)  Per the requirements outlined in the project bidding manual (Exhibit A), BBII has developed our trestle design to provide access for Natoma street extending from gridline 11.5 at the center of the excavation (grid line E) to gridline 10 at the centerline of the shoring wall. After staking out this point on the shoring wall, it is apparent that the 530 Howard St. building is in conflict with the access point. See the attached sketch and photos indicating the approximate location of 530 Howard in relation to the trestle access. Please advise if the Natoma St. access point should be changed to a more suitable location.				
BALFO900-0001.1	BSE - Natoma Street Trestle Access	Closed	01	05/05/2011	05/15/2011	05/09/2011
<b>From:</b> Balfour Beatty Infrastructure, Inc. Ural Yal						
<b>REQUEST:</b> Reference Project Bidding Manual (Exhibit A)  Per our discussion at our meeting on 4/26/11, the response to BBI RFI 076 indicated that BBII should relocate the access trestle but was not specific enough. Please provide an exact location for the Natoma St. offshoot that will satisfy the access requirements of future trade subcontractors. BBII requests a meeting to discuss any impacts of the relocation.		<b>ANSWER:</b> Reference Project Bidding Manual (Exhibit A)  Per our discussion at our meeting on 4/26/11, the response to BBI RFI 076 indicated that BBII should relocate the access trestle but was not specific enough. Please provide an exact location for the Natoma St. offshoot that will satisfy the access requirements of future trade subcontractors. BBII requests a meeting to discuss any impacts of the relocation.				
BALFO900-0002	BSE - Scaffolding For Interim Screen Wall	Closed	01	03/21/2011	03/31/2011	03/22/2011
<b>From:</b> Balfour Beatty Infrastructure, Inc. Ural Yal						
<b>REQUEST:</b> Reference attached photo  Scaffolding is currently being erected for the interim screen wall within Zone 4. It appears that the scaffolding lies in the path of the CDSM wall and will conflict with our work (See attached photo). When is the scaffolding scheduled to be completely dismantled and removed from		<b>ANSWER:</b> Reference attached photo  Scaffolding is currently being erected for the interim screen wall within Zone 4. It appears that the scaffolding lies in the path of the CDSM wall and will conflict with our work (See attached photo). When is the scaffolding scheduled to be completely dismantled				



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	the area?					and removed from the area?
<b>BALFO900-0003</b>	<b>BSE - Additional Project Control</b>	<b>Closed</b>	<b>01</b>	<b>04/19/2011</b>	<b>04/26/2011</b>	<b>04/25/2011</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification 01 10 50 and Drawing GT-0100			Reference Specification 01 10 50 and Drawing GT-0100			
Drawing GT-0100 indicates four points established for control. Our surveyors, KCA Engineers, are concerned about maintaining consistent control between various contractors on the project with such extensive distance between the provided control points. It is suggested that additional control points with horizontal and vertical coordinates be provided at the following locations:			Drawing GT-0100 indicates four points established for control. Our surveyors, KCA Engineers, are concerned about maintaining consistent control between various contractors on the project with such extensive distance between the provided control points. It is suggested that additional control points with horizontal and vertical coordinates be provided at the following locations:			
- Howard St. at Fremont St.			- Howard St. at Fremont St.			
- Howard St. at First St.			- Howard St. at First St.			
- Howard St. halfway between First and Second St.			- Howard St. halfway between First and Second St.			
- Mission St. at Fremont St.			- Mission St. at Fremont St.			
- Mission St. at First St.			- Mission St. at First St.			
- Mission Street at Shaw Alley.			- Mission Street at Shaw Alley.			
KCA RFI 001 has been attached for reference.			KCA RFI 001 has been attached for reference.			



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<b>BALFO900-0004</b>	<b>BSE - CDSM Pile Tolerance</b>	<b>Closed</b>	<b>01</b>	<b>06/06/2011</b>	<b>06/16/2011</b>	<b>06/13/2011</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification Section 31 56 13			Reference Specification Section 31 56 13			
In reference to the CDSM Shoring Wall DFOV QC meeting held in BBII's office on June 1, 2011, please find below the following RFI submitted by BBII's sub-contractor DND Construction:			In reference to the CDSM Shoring Wall DFOV QC meeting held in BBII's office on June 1, 2011, please find below the following RFI submitted by BBII's sub-contractor DND Construction:			
"The reference specifications for tolerance relative to centerline of wall for both the CDSM and steel soldier beams are extremely strict compared to what is common for this nature of work, particularly given the depth of the work. It is also more strict than if the verticality tolerance (1:150 CDSM/1:200 pile) is applied at a conservative excavation depth of 60 feet. Can the tolerance be changed from 0" in/2" out (CDSM) & 0" in/3" out (piles) to a uniform 0" in/4" out"?			"The reference specifications for tolerance relative to centerline of wall for both the CDSM and steel soldier beams are extremely strict compared to what is common for this nature of work, particularly given the depth of the work. It is also more strict than if the verticality tolerance (1:150 CDSM/1:200 pile) is applied at a conservative excavation depth of 60 feet. Can the tolerance be changed from 0" in/2" out (CDSM) & 0" in/3" out (piles) to a uniform 0" in/4" out"?			
<b>BALFO900-0005</b>	<b>BSE - Temporary Power For Construction</b>	<b>Closed</b>	<b>01</b>	<b>06/21/2011</b>	<b>07/01/2011</b>	<b>07/05/2011</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Temporary Power Package TG05.2 was awarded to Bass Electric on 5/12/2011. Drawing SL-003 shows locations for Temporary Power Skids that will be used to facilitate construction. Please provide dates of when the following Temporary Power Skids are going to be made available to BBII:			Temporary Power Package TG05.2 was awarded to Bass Electric on 5/12/2011. Drawing SL-003 shows locations for Temporary Power Skids that will be used to facilitate construction. Please provide dates of when the following Temporary Power Skids are going to be made available to BBII:			
Skid 1 by Natoma St. Skid 2 by Minna St. Skid 3 by First St. Skid 4 by Fremont St. Skid 5 by Beale St.			Skid 1 by Natoma St. Skid 2 by Minna St. Skid 3 by First St. Skid 4 by Fremont St. Skid 5 by Beale St.			
<b>BALFO900-0006</b>	<b>BSE - Discharge Point for Buttress Operation</b>	<b>Closed</b>	<b>01</b>	<b>06/23/2011</b>	<b>07/05/2011</b>	<b>07/05/2011</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference attached sketch.			Please reference attached sketch.			



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	<p>BBII is planning to discharge water generated by the Buttress operation into the sewer manholes shown in the sketch. Please confirm that this is acceptable. Note that location of sewer manholes is approximate and will be per As-Built. Temporary piping layout shown in the attached sketch is diagrammatic.</p>					<p>BBII is planning to discharge water generated by the Buttress operation into the sewer manholes shown in the sketch. Please confirm that this is acceptable. Note that location of sewer manholes is approximate and will be per As-Built. Temporary piping layout shown in the attached sketch is diagrammatic.</p>
<b>BALFO900-0007</b>	<b>BSE - Archeological Dig Site D-3 Information</b>	<b>Closed</b>	<b>01</b>	<b>10/13/2011</b>	<b>10/23/2011</b>	<b>10/13/2011</b>
<p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Reference Specification Section 01 13 50 and Sheet D-1002</p> <p>Due to the recent Archeological Investigation at dig site D-3, at the depth of 10-25 feet, BBII request confirmation that the excavation, observation, and all the investigations at that depth have been completed.</p> <p>Please Confirm.</p>			<p>Reference Specification Section 01 13 50 and Sheet D-1002</p> <p>Due to the recent Archeological Investigation at dig site D-3, at the depth of 10-25 feet, BBII request confirmation that the excavation, observation, and all the investigations at that depth have been completed.</p> <p>Please Confirm.</p>			
<b>BALFO900-0008</b>	<b>BSE - PG&amp;E Dimensions at Tie-in Points - VOID</b>	<b>Closed</b>	<b>01</b>	<b>10/12/2011</b>	<b>10/12/2011</b>	<b>10/13/2011</b>
<p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Reference CR T-017 and attached drawings</p> <p>The drawings provided for the installation of the PG&amp;E phases 2 utilities do not provide dimensions for the tie ins between the existing utilities and the phase 2 utility installation. Please see attached modified sketch indicating areas of concern.</p> <p>Please provide updated drawings, with dimensions from existing property lines to the tie in locations for the existing utilities and phase 2 utilities.</p> <p>Confirm MH/Vault number for the tie north west of A line (see attached drawing)</p>			<p>Reference CR T-017 and attached drawings</p> <p>The drawings provided for the installation of the PG&amp;E phases 2 utilities do not provide dimensions for the tie ins between the existing utilities and the phase 2 utility installation. Please see attached modified sketch indicating areas of concern.</p> <p>Please provide updated drawings, with dimensions from existing property lines to the tie in locations for the existing utilities and phase 2 utilities.</p> <p>Confirm MH/Vault number for the tie north west of A line (see attached drawing)</p>			



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<b>BALFO900-0009</b>	<b>BSE - D.I. Installation on First Street</b>	<b>Closed</b>	<b>01</b>	<b>10/27/2011</b>	<b>11/06/2011</b>	<b>10/31/2011</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Sheet U-3021 and D-2230			Reference Sheet U-3021 and D-2230			
The attached drawing shows a new Catch Basin #501 RUP drawing U-3021 BSE drawing D-2230 to be installed on First Street.			The attached drawing shows a new Catch Basin #501 RUP drawing U-3021 BSE drawing D-2230 to be installed on First Street.			
Currently this CB does not exist. Please confirm it will be installed.			Currently this CB does not exist. Please confirm it will be installed.			
<b>BALFO900-0010</b>	<b>BSE - Conflicts between revised trainbox columns and internal bracing</b>	<b>Closed</b>	<b>01</b>	<b>10/31/2011</b>	<b>11/10/2011</b>	<b>11/03/2011</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification Section 31 55 00			Reference Specification Section 31 55 00			
BBII received additional comments on the internal bracing from Thornton Tomasetti on 10/17/11, after the 100% submittal had already been reviewed and approved by DBI. The comments provided include revised column locations and sizes that differ from our BSE drawings.			BBII received additional comments on the internal bracing from Thornton Tomasetti on 10/17/11, after the 100% submittal had already been reviewed and approved by DBI. The comments provided include revised column locations and sizes that differ from our BSE drawings.			
The attached drawings highlight conflicts and reduced clearances presented by these revisions to the trainbox columns. As trainbox drawings are not available to BBII, please provide direction on where to locate bracing elements to resolve these conflicts.			The attached drawings highlight conflicts and reduced clearances presented by these revisions to the trainbox columns. As trainbox drawings are not available to BBII, please provide direction on where to locate bracing elements to resolve these conflicts.			
<b>BALFO900-0011</b>	<b>BSE - CR T-018 Gate Requirements</b>	<b>Closed</b>	<b>01</b>	<b>11/02/2011</b>	<b>11/12/2011</b>	<b>11/03/2011</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference CR T-018			Reference CR T-018			
CR T-018 issued to BBII indicates that the gates need to be installed at the fire lane access of 540-580 Howard. The gates will prevent access to the rear of the building from Howard and Natoma Street.			CR T-018 issued to BBII indicates that the gates need to be installed at the fire lane access of 540-580 Howard. The gates will prevent access to the rear of the building from Howard and Natoma Street.			
Please advise if the gates specified in CR T-018 are due to be installed by BBII.			Please advise if the gates specified in CR T-018 are due to be installed by BBII.			

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	<p>If BBII is requested to install the gates under CR T-018, please provide a specification and detail for the gate system that will be in meet fire regulation and standards.</p>					<p>If BBII is requested to install the gates under CR T-018, please provide a specification and detail for the gate system that will be in meet fire regulation and standards.</p>
<b>BALFO900-0012</b>	<b>BSE - Natoma Street Trestle Access - VOID</b>	<b>Closed</b>	<b>01</b>	<b>11/01/2011</b>	<b>11/11/2011</b>	<b>12/02/2011</b>
	<p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal</p> <p><b>REQUEST:</b></p> <p>Reference CR T-018, Specification Section 01 53 13, BBI Letter #4225-000-0145 (attached), and attached sketch</p> <p>CR T-018 included drawings for access to the side and rear of 540 Howard St. BBII issued letter 4225-000-0145 in response and included a sketch highlighting a conflict between the proposed building access and the Natoma St. trestle offshoot.</p> <p>The Natoma St. trestle offshoot was originally specified to span from Grid 11.5 at the center of the excavation to Grid 10 at the edge of excavation. The offshoot was moved further west per [W/O] response to the conflict with 530 Howard St.</p> <p>The 540 Howard St. building access arrangement as proposed in CR T-018 does not provide sufficient access to the Natoma offshoot (see attached sketch). Please provide direction if the offshoot is to be relocated or eliminated.</p>					<p><b>ANSWER:</b></p> <p>Reference CR T-018, Specification Section 01 53 13, BBI Letter #4225-000-0145 (attached), and attached sketch</p> <p>CR T-018 included drawings for access to the side and rear of 540 Howard St. BBII issued letter 4225-000-0145 in response and included a sketch highlighting a conflict between the proposed building access and the Natoma St. trestle offshoot.</p> <p>The Natoma St. trestle offshoot was originally specified to span from Grid 11.5 at the center of the excavation to Grid 10 at the edge of excavation. The offshoot was moved further west per [W/O] response to the conflict with 530 Howard St.</p> <p>The 540 Howard St. building access arrangement as proposed in CR T-018 does not provide sufficient access to the Natoma offshoot (see attached sketch). Please provide direction if the offshoot is to be relocated or eliminated.</p>



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
BALFO900-0012.1	BSE - Natoma Street Trestle Access	Closed	CR	12/06/2011	12/16/2011	12/06/2011
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference CR T-018, Specification Section 01 53 13, BBI Letter #4225-000-0145 (attached), and attached sketch			Reference CR T-018, Specification Section 01 53 13, BBI Letter #4225-000-0145 (attached), and attached sketch			
CR T-018 included drawings for access to the side and rear of 540 Howard St. BBII issued letter 4225-000-0145 in response and included a sketch highlighting a conflict between the proposed building access and the Natoma St. trestle offshoot.			CR T-018 included drawings for access to the side and rear of 540 Howard St. BBII issued letter 4225-000-0145 in response and included a sketch highlighting a conflict between the proposed building access and the Natoma St. trestle offshoot.			
The Natoma St. trestle offshoot was originally specified to span from Grid 11.5 at the center of the excavation to Grid 10 at the edge of excavation. The offshoot was moved further west per [W/O] response to the conflict with 530 Howard St.			The Natoma St. trestle offshoot was originally specified to span from Grid 11.5 at the center of the excavation to Grid 10 at the edge of excavation. The offshoot was moved further west per [W/O] response to the conflict with 530 Howard St.			
The 540 Howard St. building access arrangement as proposed in CR T-018 does not provide sufficient access to the Natoma offshoot (see attached sketch). Please provide direction if the offshoot is to be relocated or eliminated.			The 540 Howard St. building access arrangement as proposed in CR T-018 does not provide sufficient access to the Natoma offshoot (see attached sketch). Please provide direction if the offshoot is to be relocated or eliminated.			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>BALFO900-0013</b>	<b>BSE - Access Trestle at Gridline 3 - VOID</b>	<b>Closed</b>	<b>01</b>	<b>11/21/2011</b>	<b>12/01/2011</b>	<b>12/02/2011</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI #T-0251.1 and Specification Section 01 53 13			Reference RFI #T-0251.1 and Specification Section 01 53 13			
In order to avoid conflicts with both the Thornton Tomasetti "pile exclusion zones" provided in response to RFI T-0251.1, the first trestle pier near gridline 3 must be relocated. BBII Proposes two options:			In order to avoid conflicts with both the Thornton Tomasetti "pile exclusion zones" provided in response to RFI T-0251.1, the first trestle pier near gridline 3 must be relocated. BBII Proposes two options:			
Option A - Move the last pier East to clear the pile exclusion zones and adjacent bracing struts, resulting in a trestle deck that ends approximately 15' East of gridline 3. The capacity of this end span would be increased to allow for the additional reach.			Option A - Move the last pier East to clear the pile exclusion zones and adjacent bracing struts, resulting in a trestle deck that ends approximately 15' East of gridline 3. The capacity of this end span would be increased to allow for the additional reach.			
Option B - Move the last pier West and extend the end span to clear the pile exclusion zones and adjacent bracing struts, resulting in a trestle deck that ends approximately 20' West of gridline 3.			Option B - Move the last pier West and extend the end span to clear the pile exclusion zones and adjacent bracing struts, resulting in a trestle deck that ends approximately 20' West of gridline 3.			
Please advise how BBII should proceed.			Please advise how BBII should proceed.			





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<b>BALFO900-0013.1</b>	<b>BSE - Access Trestle at Gridline 3 Revised W/O Response to BALFO900-0013</b>	<b>Closed</b>	<b>CR</b>	<b>12/06/2011</b>	<b>12/16/2011</b>	<b>12/06/2011</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI #T-0251.1 and Specification Section 01 53 13			Reference RFI #T-0251.1 and Specification Section 01 53 13			
In order to avoid conflicts with both the Thornton Tomasetti "pile exclusion zones" provided in response to RFI T-0251.1, the first trestle pier near gridline 3 must be relocated. BBII Proposes two options:			In order to avoid conflicts with both the Thornton Tomasetti "pile exclusion zones" provided in response to RFI T-0251.1, the first trestle pier near gridline 3 must be relocated. BBII Proposes two options:			
Option A - Move the last pier East to clear the pile exclusion zones and adjacent bracing struts, resulting in a trestle deck that ends approximately 15' East of gridline 3. The capacity of this end span would be increased to allow for the additional reach.			Option A - Move the last pier East to clear the pile exclusion zones and adjacent bracing struts, resulting in a trestle deck that ends approximately 15' East of gridline 3. The capacity of this end span would be increased to allow for the additional reach.			
Option B - Move the last pier West and extend the end span to clear the pile exclusion zones and adjacent bracing struts, resulting in a trestle deck that ends approximately 20' West of gridline 3.			Option B - Move the last pier West and extend the end span to clear the pile exclusion zones and adjacent bracing struts, resulting in a trestle deck that ends approximately 20' West of gridline 3.			
Please advise how BBII should proceed.			Please advise how BBII should proceed.			
<b>BALFO900-0014</b>	<b>BSE - Location of Security Cameras</b>	<b>Closed</b>	<b>CR</b>	<b>01/16/2012</b>	<b>01/26/2012</b>	<b>01/16/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
According to Exhibit A - Rev H of the trade subcontractors bid manual. "Temporary poles shall include conduit for security cameras, power at the pole tops for security cameras, and mounting hardware for security cameras." Please advise on quantity and the location of these temporary poles.			According to Exhibit A - Rev H of the trade subcontractors bid manual. "Temporary poles shall include conduit for security cameras, power at the pole tops for security cameras, and mounting hardware for security cameras." Please advise on quantity and the location of these temporary poles.			
<b>BALFO900-0015</b>	<b>BSE - Beale St. Trestle Pile Conflict Follow-Up</b>	<b>Closed</b>	<b>CR</b>	<b>02/08/2012</b>	<b>02/18/2012</b>	<b>02/08/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Shad Gardner						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The response to RFI T-264.1 requested BBII provide the loading that would placed onto the CDSM wall. This response leads us to believe that the option to leave			The response to RFI T-264.1 requested BBII provide the loading that would placed onto the CDSM wall. This response leads us to believe that the option to			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>the pile in the current location was unacceptable. Please confirm that the pile must be moved and provide a detailed location of where the pile placement would be accepted. Upon receipt of this information BBII can accurately determine the load to placed on the Wall for Arup's review.</p>					
	<p>leave the pile in the current location was unacceptable. Please confirm that the pile must be moved and provide a detailed location of where the pile placement would be accepted. Upon receipt of this information BBII can accurately determine the load to placed on the Wall for Arup's review.</p>					
<b>BALFO900-0016</b>	<b>BSE- Stabilization of CDSM Wall</b>	<b>Closed</b>	<b>CR</b>	<b>04/10/2012</b>	<b>04/20/2012</b>	<b>04/10/2012</b>
<p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>BBII is requesting direction for a method to stabilize the unimproved soil conditions along the interior face of the CDSM wall. This request was generated after a field review of the wall conditions revealed a potential safety issue regarding workers working on the mass excavation, bracing and dewatering activities</p> <p>The current condition of the CDSM wall includes unimproved soil conditions that have the potential to become detached from the wall and create a falling safety hazard to workers as the mass excavation and bracing reach lower depths. Please reference attached photo for visual details.</p> <p>Based on our records, the CDSM wall met all the specification requirements for uniformity and improved soil as per section 31 56 13 of the contract specifications.</p>			<p>BBII is requesting direction for a method to stabilize the unimproved soil conditions along the interior face of the CDSM wall. This request was generated after a field review of the wall conditions revealed a potential safety issue regarding workers working on the mass excavation, bracing and dewatering activities</p> <p>The current condition of the CDSM wall includes unimproved soil conditions that have the potential to become detached from the wall and create a falling safety hazard to workers as the mass excavation and bracing reach lower depths. Please reference attached photo for visual details.</p> <p>Based on our records, the CDSM wall met all the specification requirements for uniformity and improved soil as per section 31 56 13 of the contract specifications.</p>			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>BALFO900-0017</b>	<b>BSE - Beale Street Bridge Pile Location Conflicts</b>	<b>Closed</b>	<b>01</b>	<b>09/19/2012</b>	<b>09/29/2012</b>	<b>09/19/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Diarmuid Cregg						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The response to submittals TG0300-206.1 and TG0300-261.1 states that BBII's Beale St Bridge fails to comply with specification section 01-53-13.1.3D with regard to coordination and constructability, but does not elaborate. BBII assumes this is related to future work not included in the BSE contract documents. BBII had previously coordinated pile locations, and cleared future concrete structures shown in drawings that were available to us, however please advise us what clashes you have detected or what specific clearances revisions are necessary for future work, so BBII can properly incorporate into our design.			The response to submittals TG0300-206.1 and TG0300-261.1 states that BBII's Beale St Bridge fails to comply with specification section 01-53-13.1.3D with regard to coordination and constructability, but does not elaborate. BBII assumes this is related to future work not included in the BSE contract documents. BBII had previously coordinated pile locations, and cleared future concrete structures shown in drawings that were available to us, however please advise us what clashes you have detected or what specific clearances revisions are necessary for future work, so BBII can properly incorporate into our design.			
<b>BALFO900-0018</b>	<b>BSE - Beale Street Bridge Pile Location Conflicts</b>	<b>Closed</b>	<b>01</b>	<b>09/24/2012</b>	<b>10/04/2012</b>	<b>09/24/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Based on the discussions at today's BSE meeting, BBII understands that the W/O's intention is to relocate the Beale Bridge from the location depicted in BBII's current submittal in order to accommodate work of future trade packages. Please provide detailed information regarding where to place the bridge, and what horizontal and vertical clearances are required. Time is of the essence for BBII to receive this additional, previously unavailable information, so the re-design process can be started as soon as possible.			Based on the discussions at today's BSE meeting, BBII understands that the W/O's intention is to relocate the Beale Bridge from the location depicted in BBII's current submittal in order to accommodate work of future trade packages. Please provide detailed information regarding where to place the bridge, and what horizontal and vertical clearances are required. Time is of the essence for BBII to receive this additional, previously unavailable information, so the re-design process can be started as soon as possible.			
<b>BALFO900-0019</b>	<b>BSE - Removal of Over Head Power Lines In Lot N</b>	<b>Closed</b>	<b>CR</b>	<b>10/08/2012</b>	<b>10/19/2012</b>	<b>10/09/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
In order to construct the Beale Street Bridge per submittal: TZ1030-015313A38, it must be pre constructed in Lot N. In order to do this the overhead power lines located on the east side of Lot N must be taken down throughout the bridge deck fabrication phase and during the final installation of the deck on Beale Street. The attached drawing illustrates the fabrication area in Lot N and the location of the overhead power lines			In order to construct the Beale Street Bridge per submittal: TZ1030-015313A38, it must be pre constructed in Lot N. In order to do this the overhead power lines located on the east side of Lot N must be taken down throughout the bridge deck fabrication phase and during the final installation of the deck on Beale Street.			



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	<p>through this area. BBII will also need to acquire a section of the W/O lot to complete the bridge deck fabrication. Please confirm that these items will be resolved before the Beale Street Bridge deck fabrication commences.</p>					<p>The attached drawing illustrates the fabrication area in Lot N and the location of the overhead power lines through this area. BBII will also need to acquire a section of the W/O lot to complete the bridge deck fabrication. Please confirm that these items will be resolved before the Beale Street Bridge deck fabrication commences.</p>
<b>BALFO900-0020</b>	<b>BSE - Rebracing Supports above the Lower Concourse Level</b>	<b>Closed</b>	<b>01</b>	<b>11/06/2012</b>	<b>11/16/2012</b>	<b>11/06/2012</b>
<p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>In futher review of W/O letter COM 00479, dated November 2, 2011, regarding rebracing of internal bracing above concourse level, BBII requests the following clarification.</p> <p>The letter states "internal bracing cannot be rebraced to a pin pile above the concourse level." Are trestle piles considered pin piles in this statement? Also, please clarify why rebracing above the concourse level cannot be supported to pin piles and/or trestle piles.</p>			<p>In futher review of W/O letter COM 00479, dated November 2, 2011, regarding rebracing of internal bracing above concourse level, BBII requests the following clarification.</p> <p>The letter states "internal bracing cannot be rebraced to a pin pile above the concourse level." Are trestle piles considered pin piles in this statement? Also, please clarify why rebracing above the concourse level cannot be supported to pin piles and/or trestle piles.</p>			
<b>BALFO900-0021</b>	<b>BSE - Sump Pit Location and Dimension</b>	<b>Closed</b>	<b>CR</b>	<b>12/05/2012</b>	<b>12/15/2012</b>	<b>12/05/2012</b>
<p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Joe Chapman</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>In Drawing S1-2022 the Sump Pit on the North Side of Zone 1 between GL 4 and GL 5, does not have all necessary dimensions to properly excavate. Please provide the dimensions drawn in blue on Drawing GT-2101, and the dimensions of the bottom footprint of the pit (See G-3004).</p>			<p>In Drawing S1-2022 the Sump Pit on the North Side of Zone 1 between GL 4 and GL 5, does not have all necessary dimensions to properly excavate. Please provide the dimensions drawn in blue on Drawing GT-2101, and the dimensions of the bottom footprint of the pit (See G-3004).</p>			



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<b>BALFO900-0022</b>	<b>BBII RFI # 342: Minna Street Manhole Sewer As-built Video</b>	<b>Closed</b>	<b>01</b>	<b>01/21/2013</b>	<b>01/31/2013</b>	<b>01/22/2013</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Dean Wallahan						
<b>REQUEST:</b>  Please provide BBII a copy of the as-built CCTV (video recording) of Minna Street sewer from SSMH#203 to SSMH#501.			<b>ANSWER:</b>  Please provide BBII a copy of the as-built CCTV (video recording) of Minna Street sewer from SSMH#203 to SSMH#501.			
<b>BALFO900-0023</b>	<b>BSE - Chain Link Fence Locations on Beale Street Temporary Bridge</b>	<b>Closed</b>	<b>CR</b>	<b>02/19/2013</b>	<b>03/01/2013</b>	<b>02/19/2013</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Brandon Miller						
<b>REQUEST:</b>  Ref: CR T-043A  Please refer to CRT -043A Scope of Work regarding installation of chain link fence on temporary bridges in lieu of contract specified plywood. CR T-043A references blind spots for "199 Fremont Street and 301 Mission Street onto Beale Street." Please see the attached sketch of Beale Street Temporary Bridge with location for chain link fence to be installed per CR T-043A.  Please confirm locations for chain link fence on Beale Street Temporary Bridge.			<b>ANSWER:</b>  Ref: CR T-043A  Please refer to CRT -043A Scope of Work regarding installation of chain link fence on temporary bridges in lieu of contract specified plywood. CR T-043A references blind spots for "199 Fremont Street and 301 Mission Street onto Beale Street." Please see the attached sketch of Beale Street Temporary Bridge with location for chain link fence to be installed per CR T-043A.  Please confirm locations for chain link fence on Beale Street Temporary Bridge.			
<b>BALFO900-0024</b>	<b>BSE - Relocate Zone 3 Dewatering and Electrical Equipment</b>	<b>Closed</b>	<b>CR</b>	<b>02/27/2013</b>	<b>03/09/2013</b>	<b>02/27/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>  BBII will be relocating equipment along the North perimeter wall in Zone 3 per W/O and TCCO direction. Items to be relocated include but are not limited to dewatering header pipe, dewatering control boxes, site electrical, monitoring equipment, etc. Please see the attached photos and sketches and for approval to proceed with relocation of said equipment.  Please confirm the utility locations shown herein do not conflict with other trade subcontractors and can remain for the duration of the dewatering system.			<b>ANSWER:</b>  BBII will be relocating equipment along the North perimeter wall in Zone 3 per W/O and TCCO direction. Items to be relocated include but are not limited to dewatering header pipe, dewatering control boxes, site electrical, monitoring equipment, etc. Please see the attached photos and sketches and for approval to proceed with relocation of said equipment.  Please confirm the utility locations shown herein do not conflict with other trade subcontractors and can remain for the duration of the dewatering system.			



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<b>BALFO900-0025</b>	<b>BSE - As-built Minna Street Manhole Rim Elevations</b>	<b>Closed</b>	<b>01</b>	<b>03/04/2013</b>	<b>03/14/2013</b>	<b>03/05/2013</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Brandon Miller						
<b>REQUEST:</b> Please provide BBII with as-built elevations of Minna Street sewer manholes: MH#201 , 202, 203, 204, 205, 206, 207.					<b>ANSWER:</b> Please provide BBII with as-built elevations of Minna Street sewer manholes: MH#201 , 202, 203, 204, 205, 206, 207.	
<b>BALFO900-0026</b>	<b>Project Milestones and Substantial Completion</b>	<b>Closed</b>	<b>01</b>	<b>08/08/2013</b>	<b>08/18/2013</b>	<b>08/08/2013</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Rodney Gordon						
<b>REQUEST:</b> Based on conversation in today's progress meeting, please confirm that substantial completion is not a prerequisite of project milestones and is therefore not required to meet any milestone obligations.					<b>ANSWER:</b> Based on conversation in today's progress meeting, please confirm that substantial completion is not a prerequisite of project milestones and is therefore not required to meet any milestone obligations.	
<b>BALFO900-0027</b>	<b>BSE - Waterproofing Damage at Area 2</b>	<b>Closed</b>	<b>01</b>	<b>11/12/2013</b>	<b>11/22/2013</b>	<b>11/18/2013</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Diarmuid Cregg						
<b>REQUEST:</b> During bracing removal at area 2, a section of waterproofing was damaged. This damage is consistent to the top of the concrete.  Please confirm the minimum waterproofing material lap needed by the WP subcontractor to repair this Section.					<b>ANSWER:</b> During bracing removal at area 2, a section of waterproofing was damaged. This damage is consistent to the top of the concrete.  Please confirm the minimum waterproofing material lap needed by the WP subcontractor to repair this Section.	
<b>P-0013</b>	<b>Pre-Qualification Questions</b>	<b>Closed</b>	<b>06</b>	<b>01/05/2010</b>	<b>01/19/2010</b>	<b>03/24/2010</b>
<b>From:</b> Webcor Construction LP      Ryan Cerri						
<b>REQUEST:</b> Please see the attached questions regarding the pre-qualification process. Please verify if the answers are correct. If they are not, please provide the correct answer. Thanks.					<b>ANSWER:</b> Please see the attached questions regarding the pre-qualification process. Please verify if the answers are correct. If they are not, please provide the correct answer. Thanks.	
<b>P-0014</b>	<b>Caltrans Spec for Temp Road Design Criteria</b>	<b>Closed</b>	<b>06</b>	<b>01/13/2010</b>	<b>01/27/2010</b>	<b>01/14/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Ryan Cerri						



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<b>P-0018</b>	<b>TC1 Transmittal for Buttress Package received 1-14-2010</b>	<b>Closed</b>	<b>06</b>	<b>01/15/2010</b>	<b>01/29/2010</b>	<b>01/21/2010</b>
<b>From:</b> Webcor Construction LP      Ryan Cerri						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: "Buttress Package - Construction Documents Issued for Review" transmittal dated 1/14/10			Ref: "Buttress Package - Construction Documents Issued for Review" transmittal dated 1/14/10			
The transmittal sent with the "Buttress Package - Construction Documents Issued for Review" is not complete. Please include the following information in the transmittal and reissue so we can verify all documents have been received:			The transmittal sent with the "Buttress Package - Construction Documents Issued for Review" is not complete. Please include the following information in the transmittal and reissue so we can verify all documents have been received:			
- Listing of all drawings transmitted			- Listing of all drawings transmitted			
- Listing of all specifications transmitted			- Listing of all specifications transmitted			
- Title and date of CD, including a list of all documents included on the CD			- Title and date of CD, including a list of all documents included on the CD			
- Review Comments Responses (which were found on the CD, but no hard copy)			- Review Comments Responses (which were found on the CD, but no hard copy)			
Please apply this protocol to all future transmittals so W/O knows exactly what is included in the packages.			Please apply this protocol to all future transmittals so W/O knows exactly what is included in the packages.			
<b>P-0019</b>	<b>TC1 Construction Documents Issuance Schedule</b>	<b>Closed</b>	<b>06</b>	<b>01/19/2010</b>	<b>02/02/2010</b>	<b>03/03/2010</b>
<b>From:</b> Webcor Construction LP      Ryan Cerri						
<b>REQUEST:</b>			<b>ANSWER:</b>			
A part of our preconstruction scope of services, we are to provide cost estimates at 100%DD, 50%CD, 85%CD, and 100%CD, however there are currently no publish dates for 50%CD and 85%CD. Please provide publish dates for 50%CD and 85%CD for incorporation into the project schedule.			A part of our preconstruction scope of services, we are to provide cost estimates at 100%DD, 50%CD, 85%CD, and 100%CD, however there are currently no publish dates for 50%CD and 85%CD. Please provide publish dates for 50%CD and 85%CD for incorporation into the project schedule.			
<b>P-0020</b>	<b>301 Mission Wall - Survey Info, Dim. From A-Line</b>	<b>Closed</b>	<b>06</b>	<b>03/04/2010</b>	<b>03/18/2010</b>	<b>04/14/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: email "301 Mission Wall - Survey Info, dated 3/3/10", C-2003 - A Line, A-2306 - A line. Please provide the dimension from the "x" marked on the sidewalk (adjacent to the 301 Mission wall) to gridline A in the 100% Design Development drawings.			Ref: email "301 Mission Wall - Survey Info, dated 3/3/10", C-2003 - A Line, A-2306 - A line. Please provide the dimension from the "x" marked on the sidewalk (adjacent to the 301 Mission wall) to gridline A in the 100% Design Development drawings.			







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	<p>stations 4+00 and 4+50 to accommodate the shoring wall.</p> <p>After review of the demolition drawings as provided by TJPA to Webcor/Obayashi for reference only, and a field review taking into account the extent of the footings we see a conflict with the (E) water line based on the location of the water line provided thru U.S.A. The footings are extensive and demolition will require shoring that will be very close to the existing water line if not on top of the line.</p> <p>Please review and provide a solution. Webcor/Obayashi JV believes that a temporary relocation of that water line from station 2+50 and 4+50 is a potential solution.</p>					
	<p>waterline between stations 4+00 and 4+50 to accommodate the shoring wall.</p> <p>After review of the demolition drawings as provided by TJPA to Webcor/Obayashi for reference only, and a field review taking into account the extent of the footings we see a conflict with the (E) water line based on the location of the water line provided thru U.S.A. The footings are extensive and demolition will require shoring that will be very close to the existing water line if not on top of the line.</p> <p>Please review and provide a solution. Webcor/Obayashi JV believes that a temporary relocation of that water line from station 2+50 and 4+50 is a potential solution.</p>					
<b>P-0024</b>	<b>DTX 650' HSR Tracks And Platform Extension Study Drawings</b>	<b>Closed</b>	<b>06</b>	<b>03/19/2010</b>	<b>04/02/2010</b>	<b>04/15/2010</b>
<p><b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Webcor / Obayashi received drawings regarding the "Updated Figures for DTX 650' HSR Tracks and Platform Extension Study", please provide a transmittal and direction on how to proceed with the following attached files:</p> <p>1) Sheet-DTX Modifications for HSR FIG1- 03-09-10 2) Sheet-DTX Modifications for HSR FIG2-BLOWUP 03-09-10 3) Sheet-DTX Modifications for HSR FIG3-201 Mission 03-09-10</p>			<p>Webcor / Obayashi received drawings regarding the "Updated Figures for DTX 650' HSR Tracks and Platform Extension Study", please provide a transmittal and direction on how to proceed with the following attached files:</p> <p>1) Sheet-DTX Modifications for HSR FIG1- 03-09-10 2) Sheet-DTX Modifications for HSR FIG2-BLOWUP 03-09-10 3) Sheet-DTX Modifications for HSR FIG3-201 Mission 03-09-10</p>			





In Volume 1 of the 100%DD specifications, section 00 30 00 - Desktop Cladding And Secondary Structure Wind Load Review (12/14/09) is marked as issued in the Table of Contents, but is not included in the package. Please provide specification section 00 30 00 - Desktop Cladding And Secondary Structure Wind Load Review (12/14/09) or confirm it has not been issued and update contract documents accordingly.



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<b>P-0029</b>	<b>Length Of Concrete Mat Slab Pour</b>	<b>Closed</b>	<b>06</b>	<b>04/01/2010</b>	<b>04/15/2010</b>	<b>04/05/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
As discussed in previous meetings, please confirm it is acceptable to pour the concrete mat slab in the full width of the project and in up to 400' in length.			As discussed in previous meetings, please confirm it is acceptable to pour the concrete mat slab in the full width of the project and in up to 400' in length.			
(Note: Webcor / Obayashi needs this response for 100%DD estimating purposes.)			(Note: Webcor / Obayashi needs this response for 100%DD estimating purposes.)			
<b>P-0030</b>	<b>TC1 100% DD Train Platform Mechanical Room Door Sizes</b>	<b>Closed</b>	<b>CR</b>	<b>05/12/2010</b>	<b>05/19/2010</b>	<b>05/20/2010</b>
<b>From:</b> Webcor Construction LP      Ryan Cerri						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: A-3001, grids 3-6 and B-D (dated 2/16/10) Per our scheduling exercises, we observed these conditions in the 100% DD drawings: The mechanical rooms in the Train Platform BOH call for CMU walls. We are scheduling to install the mechanical equipment prior to CMU because most equipment in these rooms will not fit through a 3' wide door. Please confirm the door sizes for the following rooms are 3' wide: B2222, B2223, B2230, and B2228.			Ref: A-3001, grids 3-6 and B-D (dated 2/16/10) Per our scheduling exercises, we observed these conditions in the 100% DD drawings: The mechanical rooms in the Train Platform BOH call for CMU walls. We are scheduling to install the mechanical equipment prior to CMU because most equipment in these rooms will not fit through a 3' wide door. Please confirm the door sizes for the following rooms are 3' wide: B2222, B2223, B2230, and B2228.			
<b>P-0031</b>	<b>TC1 100% DD PE301 &amp; PE603 Phase 1/Phase 2 Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>05/12/2010</b>	<b>05/26/2010</b>	<b>05/20/2010</b>
<b>From:</b> Webcor Construction LP      Ryan Cerri						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: Adamson Associates Transbay Transit Center Phasing, February 26, 2010; A-2106 and A-2103 (dated 2/16/10) Per the referenced phasing document, PE301 and PE603 are identified on the Train Platform level as the core being built in Phase 1 (color red), and the elevator being built in Phase 2 (color green). We have scheduled the CMU installation for Phase 1 and the elevator installation for Phase 2 at both elevator locations. Please confirm this is in compliance with Phase 1 and Phase 2 construction. This is important for the 100% DD schedule development.			Ref: Adamson Associates Transbay Transit Center Phasing, February 26, 2010; A-2106 and A-2103 (dated 2/16/10) Per the referenced phasing document, PE301 and PE603 are identified on the Train Platform level as the core being built in Phase 1 (color red), and the elevator being built in Phase 2 (color green). We have scheduled the CMU installation for Phase 1 and the elevator installation for Phase 2 at both elevator locations. Please confirm this is in compliance with Phase 1 and Phase 2 construction. This is important for the 100% DD schedule development.			









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	3. Provide a typical length that takes into account the tolerance of the basket columns.				glazing sub frame connection.	3. Provide a typical length that takes into account the tolerance of the basket columns.
<b>P1-0001</b>	<b>Perimeter Protection Bollard Cladding</b>  <b>From:</b> Webcor Construction LP      Andrew Kitchen	<b>Closed</b>	<b>0P</b>	<b>06/05/2014</b>	<b>06/05/2014</b>	<b>06/23/2014</b>
	<b>REQUEST:</b>  Provide documents for secondary mitigation to be incorporated for TG07.9 - Eliminate decorative SS cladding from the perimeter protection bollards and provide painted steel jacket in lieu of SS.				<b>ANSWER:</b>  Provide documents for secondary mitigation to be incorporated for TG07.9 - Eliminate decorative SS cladding from the perimeter protection bollards and provide painted steel jacket in lieu of SS.	
<b>P1-0002</b>	<b>Operable /Retractable Bollards and Wedge Barrier Traffic Lights</b>  <b>From:</b> Webcor Construction LP      Andrew Kitchen	<b>Closed</b>	<b>0P</b>	<b>06/05/2014</b>	<b>06/15/2014</b>	<b>06/23/2014</b>
	<b>REQUEST:</b>  Reference: A1-2302, A1-2303, A1-2304, A1-2310  Do operable/retractable bollards and wedge barrier traffic lights interface with street light signals? If so provide specifications for this interaction.				<b>ANSWER:</b>  Reference: A1-2302, A1-2303, A1-2304, A1-2310  Do operable/retractable bollards and wedge barrier traffic lights interface with street light signals? If so provide specifications for this interaction.	





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<b>P1-0002.1</b>	<b>Additional Information for Operable Wedge Barriers</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b>  Reference: 28 16 44/APA, RFI P1-0002, A1-2302, A1-2303, A1-2304, and A1-2310  RFI response P1-0002: "Reference to traffic light interface is specified in section 28 16 44/APA - 3.5.I.1 included with VE Round 2 documentation delivered on 06/18/2014"  Original comment remains. 28 16 44/APA, 3.5.I. indicates that wedge barriers provide an output to the traffic control system. Provide details and output requirements.						<b>ANSWER:</b>  Reference: 28 16 44/APA, RFI P1-0002, A1-2302, A1-2303, A1-2304, and A1-2310  RFI response P1-0002: "Reference to traffic light interface is specified in section 28 16 44/APA - 3.5.I.1 included with VE Round 2 documentation delivered on 06/18/2014"  Original comment remains. 28 16 44/APA, 3.5.I. indicates that wedge barriers provide an output to the traffic control system. Provide details and output requirements.
<b>P1-0003</b>	<b>HPU Bollard Detail</b>	<b>Closed</b>	<b>0P</b>	<b>06/05/2014</b>	<b>06/15/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b>  Reference: A1-2302, A1-2304, A1-2203, A1-2210  Provide detail of HPU for retractable bollards.						<b>ANSWER:</b>  Reference: A1-2302, A1-2304, A1-2203, A1-2210  Provide detail of HPU for retractable bollards.
<b>P1-0004</b>	<b>Retractable Bollard Hydraulic Connections and Lines</b>	<b>Closed</b>	<b>0P</b>	<b>06/05/2014</b>	<b>06/15/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b>  Reference: A1-2302, A1-2303, A1-2304, A1-2310  Provide detail and routing for retractable bollard hydraulic connections and lines.						<b>ANSWER:</b>  Reference: A1-2302, A1-2303, A1-2304, A1-2310  Provide detail and routing for retractable bollard hydraulic connections and lines.
<b>P1-0005</b>	<b>Hydraulic Hose Conduit</b>	<b>Closed</b>	<b>0P</b>	<b>06/05/2014</b>	<b>06/15/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b>  Reference - 28 16 44 Section 3.3.I  Specify type of containment infrastructure/conduit required						<b>ANSWER:</b>  Reference - 28 16 44 Section 3.3.I  Specify type of containment infrastructure/conduit



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	for hydraulic hoses.					required for hydraulic hoses.
P1-0006	HPU for Wedge Barriers	Closed	0P	06/05/2014	06/15/2014	06/23/2014
From: Webcor Construction LP Andrew Kitchen						
REQUEST: Reference: A1-2306, A1-2307, A1-2310, A1-2206, A1-2207, A1-2210  Provide detail of HPU for wedge barriers.						ANSWER: Reference: A1-2306, A1-2307, A1-2310, A1-2206, A1-2207, A1-2210  Provide detail of HPU for wedge barriers.
P1-0007	Wedge Barrier Hydraulic Connections and Lines	Closed	0P	06/05/2014	06/15/2014	06/23/2014
From: Webcor Construction LP Andrew Kitchen						
REQUEST: Reference: A1-2306, A1-2307, A1-2310  Provide detail and routing for wedge barrier hydraulic connections and lines.						ANSWER: Reference: A1-2306, A1-2307, A1-2310  Provide detail and routing for wedge barrier hydraulic connections and lines.
P1-0008	HPU Containment Pans	Closed	0P	06/05/2014	06/15/2014	06/23/2014
From: Webcor Construction LP Andrew Kitchen						
REQUEST: Reference: A1-2306, A1-2307, A1-2310  Confirm that HPU containment pans are not required for leakage of hydraulic fluid.						ANSWER: Reference: A1-2306, A1-2307, A1-2310  Confirm that HPU containment pans are not required for leakage of hydraulic fluid.
P1-0009	Details and Cuts for Wedge Barriers at Beale and First Streets	Closed	0P	06/05/2014	06/15/2014	06/23/2014
From: Webcor Construction LP Andrew Kitchen						
REQUEST: Reference: A1-2306, A1-2307  Provide details and cuts for wedge barriers at Beale and First Streets. Coordinate with Structural drawings.						ANSWER: Reference: A1-2306, A1-2307  Provide details and cuts for wedge barriers at Beale and First Streets. Coordinate with Structural





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	Provide traffic light details and locations for all wedge barriers.					Provide traffic light details and locations for all wedge barriers.
<b>P1-0013.1</b>	<b>Traffic Light Details for Wedge Barriers</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference: A1-7402, A1-2306, A1-2307, A1-2310, 28 16 44, P1-0013  RFI response P1-0013: "No indicator lights are required at the wedge barriers in the Bus Plaza (sheets A1-2306 and A1-2307). Indicator lights for the wedge barriers at the Vehicle Ramp are now shown on A1-7402 and A1-7418 delivered with MEPTSc Addendum # 4 to be issued on 06/20/2014."  Original comment remains. 28 16 44, 3.5.I. indicates that wedge barriers provide an output to the traffic control system. Provide details and output requirements. A1-7402 shows one indicator light for two wedge barriers, while 28 16 44 2.3.H. states that each wedge barrier shall have their own light. Clarify what is required.						<b>ANSWER:</b> Reference: A1-7402, A1-2306, A1-2307, A1-2310, 28 16 44, P1-0013  RFI response P1-0013: "No indicator lights are required at the wedge barriers in the Bus Plaza (sheets A1-2306 and A1-2307). Indicator lights for the wedge barriers at the Vehicle Ramp are now shown on A1-7402 and A1-7418 delivered with MEPTSc Addendum # 4 to be issued on 06/20/2014."  Original comment remains. 28 16 44, 3.5.I. indicates that wedge barriers provide an output to the traffic control system. Provide details and output requirements. A1-7402 shows one indicator light for two wedge barriers, while 28 16 44 2.3.H. states that each wedge barrier shall have their own light. Clarify what is required.
<b>P1-0014</b>	<b>Impact of Attenuator</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference: A1-2502  What is the Impact Attenuator located at GL F & 2? Provide specifications and details.						<b>ANSWER:</b> Reference: A1-2502  What is the Impact Attenuator located at GL F & 2? Provide specifications and details.



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P1-0015	Wedge Barrier and Retractable Bollard Details	Closed	0P	06/09/2014	06/19/2014	06/23/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
Reference: Secondary Mitigation						Reference: Secondary Mitigation
Provide updated details for wedge barriers and retractable bollards showing removal of topping slab with proper elevations, etc., as they are impacted by the following secondary mitigation measure: Delete 87K sf topping slab in the west end of the Lower Concourse at the SOC, Engineering and Vehicle & Bike Ramps.						Provide updated details for wedge barriers and retractable bollards showing removal of topping slab with proper elevations, etc., as they are impacted by the following secondary mitigation measure: Delete 87K sf topping slab in the west end of the Lower Concourse at the SOC, Engineering and Vehicle & Bike Ramps.
P1-0016	Retractable Bollards on Howard	Closed	0P	06/09/2014	06/19/2014	06/30/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:						ANSWER:
Reference: A1-2310						Reference: A1-2310
Provide locations and dimension retractable bollard traffic lights at the Vehicle/Bike Ramp on Howard Street.						Provide locations and dimension retractable bollard traffic lights at the Vehicle/Bike Ramp on Howard Street.



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<b>P1-0016.1</b>	<b>Additional Information for Retractable Bollards</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: A1-2310, ASI 118 A1-7418, and ASI 119 A1-7418 (SKA-3526),  RFI response P1-0016: "Locations of indicator light pylons and other devices shown on updated sheets A1-7402 and A1-7418 delivered with MEPTSc Addendum # 4, to be issued 06/20/2014."  A1-7402 is not related to retractable bollards, just to wedge barriers. Sheet A1-7418 from ASI 118 has been superseded by sheet A1-7418 (SKA-3526) issued in ASI 119 which does not show any indicator lights. Clarify which sheet is meant to be the most current version.						<b>ANSWER:</b>  Reference: A1-2310, ASI 118 A1-7418, and ASI 119 A1-7418 (SKA-3526),  RFI response P1-0016: "Locations of indicator light pylons and other devices shown on updated sheets A1-7402 and A1-7418 delivered with MEPTSc Addendum # 4, to be issued 06/20/2014."  A1-7402 is not related to retractable bollards, just to wedge barriers. Sheet A1-7418 from ASI 118 has been superseded by sheet A1-7418 (SKA-3526) issued in ASI 119 which does not show any indicator lights. Clarify which sheet is meant to be the most current version.
<b>P1-0017</b>	<b>Incorrect Details on A1-8168</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/09/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference: A1-8168  Detail 3 - Bollard placement is missing on 3/A1-8168. Section cut B/A1-8168 needs to be verified. Section B/A1-8168 and plan view on A1-2502 don't match. Confirm which placement is correct and update details accordingly.						<b>ANSWER:</b>  Reference: A1-8168  Detail 3 - Bollard placement is missing on 3/A1-8168. Section cut B/A1-8168 needs to be verified. Section B/A1-8168 and plan view on A1-2502 don't match. Confirm which placement is correct and update details accordingly.
<b>P1-0018</b>	<b>Design Build Bollards</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/09/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: 05 50 00  2.3.E through H - Bollards are indicated as being design build in this specification. Confirm that they are design build.						<b>ANSWER:</b>  Reference: 05 50 00  2.3.E through H - Bollards are indicated as being design build in this specification. Confirm that they are design build.



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<b>P1-0019</b>	<b>Structural Drawings for Bollards</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference: A1-3190  Detail 2 - Says "Refer to Structural Dwgs", but there are no bollard details in the structural drawings. Provide structural details. Confirm that BOL-3 as shown in Detail 2 and 4 is sturdy enough to withstand impact from a vehicle at required forces.						<b>ANSWER:</b> Reference: A1-3190  Detail 2 - Says "Refer to Structural Dwgs", but there are no bollard details in the structural drawings. Provide structural details. Confirm that BOL-3 as shown in Detail 2 and 4 is sturdy enough to withstand impact from a vehicle at required forces.
<b>P1-0020</b>	<b>Bollards Detail</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Reference: 05 50 00  2.5.E. - The specification for BOL-1 and BOL-2 do not indicate that they are concrete filled, however the details on A1-8676 show that they are concrete filled. Coordinate drawings and specifications accordingly.						<b>ANSWER:</b> Reference: 05 50 00  2.5.E. - The specification for BOL-1 and BOL-2 do not indicate that they are concrete filled, however the details on A1-8676 show that they are concrete filled. Coordinate drawings and specifications accordingly.
<b>P1-0021</b>	<b>WPM Detail for Retractable Bollard Drain</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference: A1-8721  Provide waterproofing details for retractable bollard drain and all penetrations.						<b>ANSWER:</b> Reference: A1-8721  Provide waterproofing details for retractable bollard drain and all penetrations.
<b>P1-0021.1</b>	<b>Slab penetration for Bollard</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Reference: A1-8721  RFI response P1-0021: "Slab penetrations required for retractable bollards are located on plans. For the retractable bollard waterproofing details, refer to Details 3,4, and 5 of A1-8721"						<b>ANSWER:</b> Reference: A1-8721  RFI response P1-0021: "Slab penetrations required for retractable bollards are located on plans. For the retractable bollard waterproofing details, refer to Details 3,4, and 5 of A1-8721"



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	<p>Provide waterproofing details for all penetrations, including hydraulic and electrical lines which will need to penetrate the waterproofing at the side wall of the pit.</p>					<p>Provide waterproofing details for all penetrations, including hydraulic and electrical lines which will need to penetrate the waterproofing at the side wall of the pit.</p>
<b>P1-0022</b>	<b>Design Build Bollards - Quality Assurance</b>  <b>From:</b> Webcor Construction LP      Zachary Moore	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
	<b>REQUEST:</b>  Reference: 12 93 30/APA (SSI)  1.5 - Quality Assurance - This states that the site bollards are design build. Confirm that they are design build.					<b>ANSWER:</b>  Reference: 12 93 30/APA (SSI)  1.5 - Quality Assurance - This states that the site bollards are design build. Confirm that they are design build.
<b>P1-0023</b>	<b>Operable Bollards at Natoma Pedestrian Area West, Location 12</b>  <b>From:</b> Webcor Construction LP      Andrew Kitchen	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
	<b>REQUEST:</b>  Reference: 28 16 44/APA (SSI) 1.2.E  Shows that there are operable bollards at Natoma Pedestrian Area West, location 12, which is at Natoma and 2nd Street. Provide location and details of HPU for this set of operable bollards. Confirm this is TJPA's property. There are no details for operable bollards at this location. Clarify that this is a requirement and if so provide details and confirm that the TJPA is allowed to install operable bollards at this location and that other businesses/buildings are not adversely impacted.					<b>ANSWER:</b>  Reference: 28 16 44/APA (SSI) 1.2.E  Shows that there are operable bollards at Natoma Pedestrian Area West, location 12, which is at Natoma and 2nd Street. Provide location and details of HPU for this set of operable bollards. Confirm this is TJPA's property. There are no details for operable bollards at this location. Clarify that this is a requirement and if so provide details and confirm that the TJPA is allowed to install operable bollards at this location and that other businesses/buildings are not adversely impacted.





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<b>P1-0024</b>	<b>Number of Wedge Barriers at Fremont</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/09/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: 28 16 44/APA (SSI)  1.2.E - Shows only five wedge barriers at Fremont Street (Bus Plaza). The drawings show eight wedge barriers at Fremont Street (Bus Plaza) at GL 27. Revise and coordinate documents accordingly.		<b>ANSWER:</b>  Reference: 28 16 44/APA (SSI)  1.2.E - Shows only five wedge barriers at Fremont Street (Bus Plaza). The drawings show eight wedge barriers at Fremont Street (Bus Plaza) at GL 27. Revise and coordinate documents accordingly.				
<b>P1-0025</b>	<b>Wedge Barriers at Beale Street</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference: 28 16 44/APA 1.2.E - Shows only two wedge barriers at Beale Street (Bus Plaza). The drawings show four wedge barriers at Beale Street (Bus Plaza) at GL 33. Revise and coordinate documents accordingly.		<b>ANSWER:</b>  Reference: 28 16 44/APA 1.2.E - Shows only two wedge barriers at Beale Street (Bus Plaza). The drawings show four wedge barriers at Beale Street (Bus Plaza) at GL 33. Revise and coordinate documents accordingly.				
<b>P1-0026</b>	<b>Removable Bollards</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/27/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: 28 16 44/APA (SSI)  1.2.E - This does not show all Manually Removable Bollard locations that are indicated in the drawings, for example on L1-2302 GL 2-4 @ GL A. Remove from specification or provide all locations.		<b>ANSWER:</b>  Reference: 28 16 44/APA (SSI)  1.2.E - This does not show all Manually Removable Bollard locations that are indicated in the drawings, for example on L1-2302 GL 2-4 @ GL A. Remove from specification or provide all locations.				
<b>P1-0027</b>	<b>Utility Vault Bollards</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference: L1-2305  Confirm that B3 and not B3A is required at utility vaults between GL 20-22 at GL J. All other utility vaults have B3A bollards.		<b>ANSWER:</b>  Reference: L1-2305  Confirm that B3 and not B3A is required at utility vaults between GL 20-22 at GL J. All other utility vaults have B3A bollards.				



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P1-0028	Secondary Controllers	Closed	0P	06/09/2014	06/19/2014	06/23/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:  Reference: 28 16 44/APA (SSI)  1.3.A.2 - Locate all Secondary Controllers for all operable bollards and wedge barriers. They are not indicated in the drawings and quantities are not provided.						ANSWER:  Reference: 28 16 44/APA (SSI)  1.3.A.2 - Locate all Secondary Controllers for all operable bollards and wedge barriers. They are not indicated in the drawings and quantities are not provided.
P1-0029	Card Readers for Operable Bollards and Wedge Barriers	Closed	0P	06/09/2014	06/19/2014	06/23/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:  Reference: 28 16 44/APA 3.5.G:  Locate all Card Readers for all operable bollards and wedge barriers. They are not indicated in the drawings and quantities are not provided.						ANSWER:  Reference: 28 16 44/APA 3.5.G:  Locate all Card Readers for all operable bollards and wedge barriers. They are not indicated in the drawings and quantities are not provided.
P1-0029.1	Card Readers for Operable Bollards and Wedge Barriers	Closed	0P	07/25/2014	08/04/2014	08/20/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:  Reference: 28 16 44/APA 3.5.G:  RFI Response P1-0029.1: Locations for all required card readers are identified in updated sheets A1-2302, A1-2303, A1-7402 and A1-7418 included with MEPTSc Addendum # 4, to be issued 06/20/2014.  Original comment remains. Card Reader locations are not indicated.						ANSWER:  Reference: 28 16 44/APA 3.5.G:  RFI Response P1-0029.1: Locations for all required card readers are identified in updated sheets A1-2302, A1-2303, A1-7402 and A1-7418 included with MEPTSc Addendum # 4, to be issued 06/20/2014.  Original comment remains. Card Reader locations are not indicated.
P1-00291	VOID	Void	CR	07/18/2014	07/28/2014	
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:						ANSWER:



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<b>P1-0030</b>	<b>Touchscreen Controllers</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/09/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference: 28 16 44/APA (SSI)  1.3.A.3 - Locate all Touchscreen Controlers for all operable bollards and wedge barriers. They are not indicated in the drawings and quantities are not provided.						<b>ANSWER:</b> Reference: 28 16 44/APA (SSI)  1.3.A.3 - Locate all Touchscreen Controlers for all operable bollards and wedge barriers. They are not indicated in the drawings and quantities are not provided.
<b>P1-0030.1</b>	<b>Additional Information for Touchscreen Controllers</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference: 28 16 44/APA (SSI) RFI response: P1-0030: "See response to RFI P1-0028"  Confirm that only one touch screen is required and that location is in the Security Command Center.						<b>ANSWER:</b> Reference: 28 16 44/APA (SSI) RFI response: P1-0030: "See response to RFI P1-0028"  Confirm that only one touch screen is required and that location is in the Security Command Center.
<b>P1-0031</b>	<b>Design Build Perimeter Security System</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b> Reference: 28 16 44 1.2.E:  This states that the perimeter security system (operable bollards and wedge barriers) are design build. Confirm they are design build.						<b>ANSWER:</b> Reference: 28 16 44 1.2.E:  This states that the perimeter security system (operable bollards and wedge barriers) are design build. Confirm they are design build.
<b>P1-0032</b>	<b>Photoelectric Beam for Operable Barriers</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference: 28 16 44 (SSI)  1.2.1 - Provide detail for the photoelectric beam required to detect people or vehicles on or near the operable barriers. Locate and dimension on drawings. They are not indicated in the drawings and quantities are not provided.						<b>ANSWER:</b> Reference: 28 16 44 (SSI)  1.2.1 - Provide detail for the photoelectric beam required to detect people or vehicles on or near the operable barriers. Locate and dimension on drawings. They are not indicated in the drawings and quantities



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are not provided.						
<b>P1-0032.1</b>	<b>Additional Information Photoelectric Beam for Operable Barriers</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference:28 16 44 (SSI), A1-2304, A1-2306, A1-2307, A1-2302, A1-2303, A1-7402, A1-7418 from ASI 118, A1-7418 (SKA-3526) issued in ASI 119			Reference:28 16 44 (SSI), A1-2304, A1-2306, A1-2307, A1-2302, A1-2303, A1-7402, A1-7418 from ASI 118, A1-7418 (SKA-3526) issued in ASI 119			
RFI response P1-0032: "Equipment and detail to be provided by the Operable Barrier manufacturer/supplier. Locations for photoelectric beam pylons identified on updated sheets A1-2302, A1-2303, A1-7402 and A1-7418 included with MEPTSc Addendum # 4, to be issued 06/20/2014."			RFI response P1-0032: "Equipment and detail to be provided by the Operable Barrier manufacturer/supplier. Locations for photoelectric beam pylons identified on updated sheets A1-2302, A1-2303, A1-7402 and A1-7418 included with MEPTSc Addendum # 4, to be issued 06/20/2014."			
- Locations of photoelectric beams are not called out on sheet A1-2302, A1-2303. Provide updated details. Also provide details for sheets A1-2304, A1-2306, A1-2307. - Confirm that photoelectric beam on A1-7402 is wide enough to cover the entire wedge barrier if someone is standing on it.			- Locations of photoelectric beams are not called out on sheet A1-2302, A1-2303. Provide updated details. Also provide details for sheets A1-2304, A1-2306, A1-2307. - Confirm that photoelectric beam on A1-7402 is wide enough to cover the entire wedge barrier if someone is standing on it.			



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<b>P1-0033</b>	<b>HPU Relocations and Cosntruction Scheduling</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference: 28 16 44 1.2.J:  "The contractor will be required to install the anti-ram barrier systems in phases to match the construction schedule of the project. The phasing includes the relocation of the hydraulic power units at a later date and re-installation of hydraulic hoses, fittings and valves based on the revised distance between the hydraulic power unit and the barriers it is controlling." Why are the HPU's being relocated? What is the phasing in the construction schedule that is being referred to here? How does is the relocation of the hydraulic lines, piping and drainage being accounted for in the drawings? Routing and all accommodations for temporary and permanent locations needs to be provided. Coordinate all drawings and specifications. Revise accordingly.						
						<b>ANSWER:</b>  Reference: 28 16 44 1.2.J:  "The contractor will be required to install the anti-ram barrier systems in phases to match the construction schedule of the project. The phasing includes the relocation of the hydraulic power units at a later date and re-installation of hydraulic hoses, fittings and valves based on the revised distance between the hydraulic power unit and the barriers it is controlling." Why are the HPU's being relocated? What is the phasing in the construction schedule that is being referred to here? How does is the relocation of the hydraulic lines, piping and drainage being accounted for in the drawings? Routing and all accommodations for temporary and permanent locations needs to be provided. Coordinate all drawings and specifications. Revise accordingly.
<b>P1-0034</b>	<b>Vehicle Barrier Controller locations</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  reference: 28 16 44 (SSI)  2.1 - Locate and provide detail for all vehicle barrier controllers. They are not indicated in the drawings and quantities are not provided.						
						<b>ANSWER:</b>  reference: 28 16 44 (SSI)  2.1 - Locate and provide detail for all vehicle barrier controllers. They are not indicated in the drawings and quantities are not provided.
<b>P1-0035</b>	<b>Wedge Barrier CIP Locations</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference: 28 16 44/APA 2.2  Coordinate and detail wedge barrier cast in place locations at the Bus Muni Plaza on Beale and Fremont Streets. These are not in the Structural drawings.						
						<b>ANSWER:</b>  Reference: 28 16 44/APA 2.2  Coordinate and detail wedge barrier cast in place locations at the Bus Muni Plaza on Beale and Fremont Streets. These are not in the Structural drawings.



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<b>P1-0036</b>	<b>Road Loop Details</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Reference: 28 16 44 (SSI)  2.2.H - Provide details and location of road loops. They are not indicated in the drawings. Provide quantities.					<b>ANSWER:</b> Reference: 28 16 44 (SSI)  2.2.H - Provide details and location of road loops. They are not indicated in the drawings. Provide quantities.	
<b>P1-0036.1</b>	<b>Road loop Location</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Reference: 28 16 44/APA (section 2.6.B),  RFI response P1-0036: "Final location of road loops provided by the Barrier System manufacturer/fabricator. Locations of road loops currently shown on C1-7xxx series. Road loop locations for Howard Street Vehicle Ramp and West Natoma Street barrier to be provided in a future Civil package. Contractor shall obtain quantities from Contract Documents."  This states that loop detectors are located on both the secure and non-secure side of an operable barrier. The current Civil drawings do not show loops on both sides of the operable barriers, for example on sheet C1-7001. Clarify this requirement.					<b>ANSWER:</b> Reference: 28 16 44/APA (section 2.6.B),  RFI response P1-0036: "Final location of road loops provided by the Barrier System manufacturer/fabricator. Locations of road loops currently shown on C1-7xxx series. Road loop locations for Howard Street Vehicle Ramp and West Natoma Street barrier to be provided in a future Civil package. Contractor shall obtain quantities from Contract Documents."  This states that loop detectors are located on both the secure and non-secure side of an operable barrier. The current Civil drawings do not show loops on both sides of the operable barriers, for example on sheet C1-7001. Clarify this requirement.	
<b>P1-0037</b>	<b>Final Sequence of Operations</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>07/28/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference: 28 16 44/APA 3.5.A:  Confirm that "The final sequence of operations will be identified at a later time and will require coordination with the Owner's security concept of operations." does not require any work from the bidders.					<b>ANSWER:</b> Reference: 28 16 44/APA 3.5.A:  Confirm that "The final sequence of operations will be identified at a later time and will require coordination with the Owner's security concept of operations." does not require any work from the bidders.	



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<b>P1-0038</b>	<b>Operable Bollards Finish Details</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference 28 16 44 (SSI)  2.3.C.2 - Provide finish and material for operable bollard decorative sleeve.						<b>ANSWER:</b> reference 28 16 44 (SSI)  2.3.C.2 - Provide finish and material for operable bollard decorative sleeve.
<b>P1-0039</b>	<b>Decorative Bollard Detail</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b> Reference: 28 16 44 2.4.E  Provide details for decorative bollard for traffic light.						<b>ANSWER:</b> Reference: 28 16 44 2.4.E  Provide details for decorative bollard for traffic light.
<b>P1-0040</b>	<b>Bollard Base Plate Detail</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference: A1-8676 A1-2502  Detail 1 - Based on the BOL-1 bollard steel base plate 4'-10" width and 4'-6" spacing indicated on A1-2502, the base plates are overlapping by 4". Confirm there is enough space in the slab to accommodate an overlap. Corner bollards are also setting on top of the neighboring bollard plate. Revise accordingly.						<b>ANSWER:</b> Reference: A1-8676 A1-2502  Detail 1 - Based on the BOL-1 bollard steel base plate 4'-10" width and 4'-6" spacing indicated on A1-2502, the base plates are overlapping by 4". Confirm there is enough space in the slab to accommodate an overlap. Corner bollards are also setting on top of the neighboring bollard plate. Revise accordingly.
<b>P1-0041</b>	<b>Bollard Plate and Rebar Cover</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b> Reference: A1-8676  Confirm there is enough cover for the BOL-1 plate and rebar in topping slab. 4/S1-5003 Note #6 states that top and bottom rebar are required while 5/S1-5000 requires 1" clear. If there is not enough cover, revise accordingly.						<b>ANSWER:</b> Reference: A1-8676  Confirm there is enough cover for the BOL-1 plate and rebar in topping slab. 4/S1-5003 Note #6 states that top and bottom rebar are required while 5/S1-5000 requires 1" clear. If there is not enough cover, revise accordingly.



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<b>P1-0042</b>	<b>Topping slab Detail</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: A1-8676  Detail 1 shows 6" for the topping slab, including waterproof membrane. Calculations from elevations and slab thicknesses for the Bus Deck equal 5.5". Correct this discrepancy.					<b>ANSWER:</b>  Reference: A1-8676  Detail 1 shows 6" for the topping slab, including waterproof membrane. Calculations from elevations and slab thicknesses for the Bus Deck equal 5.5". Correct this discrepancy.	
<b>P1-0043</b>	<b>Bollard Coordination Between L and A Drawings</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference: A1-2302 through A1-2310, L1-2302 through L1-2310  Bollard locations and number are not coordinated in Landscape and Architectural drawings. For example B2 bollards on GL 5. Coordinate and revise accordingly.					<b>ANSWER:</b>  Reference: A1-2302 through A1-2310, L1-2302 through L1-2310  Bollard locations and number are not coordinated in Landscape and Architectural drawings. For example B2 bollards on GL 5. Coordinate and revise accordingly.	
<b>P1-0044</b>	<b>Bollard 3</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  reference: 12 93 30 A1-3190  Bollard 3 is not showing as removable on A1-3190 or in spec 05 50 00, where it is completely filled with concrete. Provide details for a removable concrete filled bollard or revise accordingly.					<b>ANSWER:</b>  reference: 12 93 30 A1-3190  Bollard 3 is not showing as removable on A1-3190 or in spec 05 50 00, where it is completely filled with concrete. Provide details for a removable concrete filled bollard or revise accordingly.	
<b>P1-0044.1</b>	<b>Conflicting Details Bollard 3</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: 12 93 30, A1-3190, 28 16 44  RFI P1-0044 response: "Section 12 93 30 has been consolidated into Section 28 16 44. Refer to 06/18/2014					<b>ANSWER:</b>  Reference: 12 93 30, A1-3190, 28 16 44  RFI P1-0044 response: "Section 12 93 30 has been consolidated into Section 28 16 44. Refer to	





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	Secondary Mitigation VE Round 2 package."  A1-3190 still refers to 05 50 00 in detail 7. Detail 2 and 4 are also in conflict with specification 28 16 44 regarding BOL-3 details. Detail 2 also refers to structural drawings, yet there are no structural drawings for any bollards. Clarify the conflicting details and requirements. Provide what the correct details and requirements are for BOL-3.			06/18/2014	Secondary Mitigation VE Round 2 package."	
P1-0045	Slab Plate at Bollard  From: Webcor Construction LP      Andrew Kitchen  <b>REQUEST:</b> Reference: 05 50 00 2.5.F.1  Through slab plate is not detailed in the drawings, but the specifications say that this is part of the bollard. Coordinate documents and revise accordingly.	Closed	0P	06/09/2014	06/19/2014	07/11/2014
					<b>ANSWER:</b> Reference: 05 50 00 2.5.F.1  Through slab plate is not detailed in the drawings, but the specifications say that this is part of the bollard. Coordinate documents and revise accordingly.	
P1-0046	Bus Deck Bollard  From: Webcor Construction LP      Zachary Moore  <b>REQUEST:</b> Reference: A1-2507, A1-2502, 05 50 00  Confirm that BOL-3 on the Bus Deck is a crash bollard. Is it strong enough, as designed, to withstand ramming from a vehicle?	Closed	0P	06/09/2014	06/19/2014	06/24/2014
					<b>ANSWER:</b> Reference: A1-2507, A1-2502, 05 50 00  Confirm that BOL-3 on the Bus Deck is a crash bollard. Is it strong enough, as designed, to withstand ramming from a vehicle?	



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<b>P1-0047</b>	<b>Bollard Coordination Between Specifications</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference: 12 93 30, 05 50 00  Bollards in spec 12 93 30 and 05 50 00 do not match in regards to their parameters, ratings and requirements. Clarify why there are two incongruous lists of bollards that use three of the same numbers. Correct and revise accordingly.						<b>ANSWER:</b>  Reference: 12 93 30, 05 50 00  Bollards in spec 12 93 30 and 05 50 00 do not match in regards to their parameters, ratings and requirements. Clarify why there are two incongruous lists of bollards that use three of the same numbers. Correct and revise accordingly.
<b>P1-0048</b>	<b>Bollard Detail type</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>  Reference: L1-7360  Landscape drawings do not detail each bollard type. Clarify which bollard type is on this sheet. Coordinate with Architectural and Structural drawings. Clarify if bus deck level BOL-1 and BOL-2, and loading dock BOL-3 are to be used for all areas? Provide details for all bollard types required.						<b>ANSWER:</b>  Reference: L1-7360  Landscape drawings do not detail each bollard type. Clarify which bollard type is on this sheet. Coordinate with Architectural and Structural drawings. Clarify if bus deck level BOL-1 and BOL-2, and loading dock BOL-3 are to be used for all areas? Provide details for all bollard types required.
<b>P1-0049</b>	<b>Bollard Details</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference: 12 93 30 2.2  Provide details for bollards listed under section 2.2. Including but not limited to, details for removable bollards, stationary bollards, footings, special footings, anchorages, concrete fill level, concrete type, sleeves, etc.						<b>ANSWER:</b>  Reference: 12 93 30 2.2  Provide details for bollards listed under section 2.2. Including but not limited to, details for removable bollards, stationary bollards, footings, special footings, anchorages, concrete fill level, concrete type, sleeves, etc.
<b>P1-0050</b>	<b>Stationary Bollards</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>  Reference: L1-2304, L1-2306						<b>ANSWER:</b>  Reference: L1-2304, L1-2306



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	<div>B1B is shown on L1-2304 and L1-2306 but is not specified in 12 93 30, for example at GL C between 17 and 18. Revise accordingly.</div>					<div>B1B is shown on L1-2304 and L1-2306 but is not specified in 12 93 30, for example at GL C between 17 and 18. Revise accordingly.</div>
P1-0050.1	Stationary Bollards	Closed	0P	07/28/2014	08/07/2014	08/27/2014
From: Webcor Construction LP Zachary Moore						
REQUEST:						ANSWER:
Reference: L1-2304, L1-2306, 28 16 44						Reference: L1-2304, L1-2306, 28 16 44
RFI P1-0050 response: "See L-0002 for description of bollard. Rating to match Bollard Type 1. Refer to updated consolidated specification section 28 16 44."						RFI P1-0050 response: "See L-0002 for description of bollard. Rating to match Bollard Type 1. Refer to updated consolidated specification section 28 16 44."
RFI original Question P1-0050: "B1B is shown on L1-2304 and L1-2306 but is not specified in 12 93 30, for example at GL C between 17 and 18. Revise accordingly."						RFI original Question P1-0050: "B1B is shown on L1-2304 and L1-2306 but is not specified in 12 93 30, for example at GL C between 17 and 18. Revise accordingly."
Original question remains. There is no listing for Bollard Type B1B in the consolidated 28 16 44.						Original question remains. There is no listing for Bollard Type B1B in the consolidated 28 16 44.
P1-0051	Retractable Bollard Drain Detail	Closed	0P	06/09/2014	06/19/2014	06/24/2014
From: Webcor Construction LP Andrew Kitchen						
REQUEST:						ANSWER:
Reference: A1-8720 and A1-8721						Reference: A1-8720 and A1-8721
Provide detail for Retractable Bollard Drain, piping route and termination. Does the effluent need to be collected and treated as hazardous waste? Coordinate with Plumbing Drawings.						Provide detail for Retractable Bollard Drain, piping route and termination. Does the effluent need to be collected and treated as hazardous waste? Coordinate with Plumbing Drawings.





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<b>P1-0054</b>	<b>Details for 1-A pole</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/09/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: 1/C1-7001  Detail 1 / C1-7001 calls out a "INST. TYPE 1-A POLE WITH 8" DIA. 2 SECTION RED WITH TV-2-T MOUNTING" Provide more detail and description of type 1-A pole, TV2-2-T Mounting, 2 section RED.						<b>ANSWER:</b> reference: 1/C1-7001  Detail 1 / C1-7001 calls out a "INST. TYPE 1-A POLE WITH 8" DIA. 2 SECTION RED WITH TV-2-T MOUNTING" Provide more detail and description of type 1-A pole, TV2-2-T Mounting, 2 section RED.
<b>P1-0054.1</b>	<b>Details for Indicator Light Pylon</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: 28 16 44  RFI P1-0054 response: "Mounting Details attached for specified poles"  Confirm that these details are to be used for the indicator light pylon. They do not match the criteria in 28 16 44.						<b>ANSWER:</b> reference: 28 16 44  RFI P1-0054 response: "Mounting Details attached for specified poles"  Confirm that these details are to be used for the indicator light pylon. They do not match the criteria in 28 16 44.
<b>P1-0055</b>	<b>Missing Note</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/23/2014</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b> Reference: 3/L1-9636  Missing note. Please provide.						<b>ANSWER:</b> Reference: 3/L1-9636  Missing note. Please provide.
<b>P1-0056</b>	<b>Wedge Barriers on Civil Drawing</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: C1-2009  Civil Drawings do not indicate wedge barriers, but civil contractor will need to know where wedge barriers are located. Show wedge barriers on civil drawings.						<b>ANSWER:</b> reference: C1-2009  Civil Drawings do not indicate wedge barriers, but civil contractor will need to know where wedge barriers are located. Show wedge barriers on civil drawings.



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P1-0057	Missing Bollard Locations	Closed	0P	06/09/2014	06/19/2014	06/24/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST: Reference: L1-2312A  Missing bollard locations. Clarify what the alternate is on this sheet.						ANSWER: Reference: L1-2312A  Missing bollard locations. Clarify what the alternate is on this sheet.
P1-0058	Water tank Liners spec	Void	0P	06/09/2014	06/19/2014	
From: Webcor Construction LP                      Zachary Moore						
REQUEST: reference 07 13 55 2.1  07 13 55 2.1 does not provide a specific manufacturer for thermoplastic water tank liners. Webcor intends to require all manufactures be approved via QBD prior to bid. If this is not acceptable, please provide specific manufacturers and products which are acceptable to use.						ANSWER: reference 07 13 55 2.1  07 13 55 2.1 does not provide a specific manufacturer for thermoplastic water tank liners. Webcor intends to require all manufactures be approved via QBD prior to bid. If this is not acceptable, please provide specific manufacturers and products which are acceptable to use.
P1-0059	Structural Details for 24" Curb at GL 1.4	Closed	0P	06/09/2014	06/19/2014	06/24/2014
From: Webcor Construction LP                      Scott Shope						
REQUEST: Reference: 2/A1-8157  Please provide structural details for 24" wide curb at GL 1.4, Second Level.						ANSWER: Reference: 2/A1-8157  Please provide structural details for 24" wide curb at GL 1.4, Second Level.
P1-0060	2/A1-8151	Closed	0P	06/09/2014	06/19/2014	06/24/2014
From: Webcor Construction LP                      Scott Shope						
REQUEST: Reference: 2/A1-8151  Please confirm that the future finish floors are not part of Phase 1 work.						ANSWER: Reference: 2/A1-8151  Please confirm that the future finish floors are not part of Phase 1 work.
P1-0061	Sheet Note on A1-3001 - Vertical Joints	Closed	0P	06/09/2014	06/19/2014	06/24/2014





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<b>P1-0063</b>	<b>Detail 2/A1-9228 Similar Condition</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP                      Scott Shope						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: 2/A1-9228		Reference: 2/A1-9228				
4/A-0026 is called out to be similar to the detail shown. 4/A-0026 does not appear to be a similar condition. Please provide detail reference for the condition shown.		4/A-0026 is called out to be similar to the detail shown. 4/A-0026 does not appear to be a similar condition. Please provide detail reference for the condition shown.				
<b>P1-0063.1</b>	<b>Details for Embedded Plate per Detail 2/A1-9228</b>	<b>Void</b>	<b>0P</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI P1-0063 Response, Detail 2/A1-9228, Sheet SKA-3667 (IFC Drawings for Main Package dated 3/31/2014)		REFERENCE: RFI P1-0063 Response, Detail 2/A1- 9228, Sheet SKA-3667 (IFC Drawings for Main Package dated 3/31/2014)				
Per RFI P1-0063 response, "Detail 2/A1-9228 has been updated. Detail reference 4/A-0026 SIM has been removed. Refer to the attached SKA-3667."		Per RFI P1-0063 response, "Detail 2/A1-9228 has been updated. Detail reference 4/A-0026 SIM has been removed. Refer to the attached SKA-3667."				
The revised detail shows an embedded plate at the B.O. Lower Concourse Deck. No information is provided for the embedded plate. Please provide information on embedded plate.		The revised detail shows an embedded plate at the B.O. Lower Concourse Deck. No information is provided for the embedded plate. Please provide information on embedded plate.				
<b>P1-0064</b>	<b>Continuous Seal at Interior Side of Exterior Concrete Wall</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP                      Scott Shope						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: 4/A1-9211		Reference: 4/A1-9211				
Detail 4/A1-9211 shows a continuous seal between the structural steel, and the interior side of the exterior concrete wall. Is cont. seal required at the int. side of ext. wall? If so, please provide detail.		Detail 4/A1-9211 shows a continuous seal between the structural steel, and the interior side of the exterior concrete wall. Is cont. seal required at the int. side of ext. wall? If so, please provide detail.				
<b>P1-0065</b>	<b>Steel Plate Supporting Concrete Topping Slab</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP                      Scott Shope						
<b>REQUEST:</b>		<b>ANSWER:</b>				





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	<p>Reference: 1/A1-3190</p> <p>The contract documents show a metal plate supporting the topping slab, but does not provide structural details. Please provide detailing (size, thickness, attachment, waterproofing, etc) for steel plate supporting concrete topping.</p>					<p>Reference: 1/A1-3190</p> <p>The contract documents show a metal plate supporting the topping slab, but does not provide structural details. Please provide detailing (size, thickness, attachment, waterproofing, etc) for steel plate supporting concrete topping.</p>
<b>P1-0066</b>	<b>Escalator Pit at Shaw Alley</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: 4/A1-7550						Reference: 4/A1-7550
The detail shows a metal plate at the edge of the escalator pit, while S1-7301 and 4/S1-7660 shows a conc. wall on all sides of the pit. Please coordinate the drawings.						The detail shows a metal plate at the edge of the escalator pit, while S1-7301 and 4/S1-7660 shows a conc. wall on all sides of the pit. Please coordinate the drawings.
<b>P1-0067</b>	<b>Column Base Detail at Loading Dock</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>07/01/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: 1/A1-3190						Reference: 1/A1-3190
Curb is shown on Plan Detail 1, but not on the section detail (5/A1-3190). Please coordinate the drawings, and provide structural information.						Curb is shown on Plan Detail 1, but not on the section detail (5/A1-3190). Please coordinate the drawings, and provide structural information.
<b>P1-0068</b>	<b>Insulation at Detail 2/A1-8181</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: 2/A1-8181						Reference: 2/A1-8181
A note calls out for 2" rigid insulation, but points to concrete. Is rigid insulation required? If so, please revise drawing and specify which insulation is to be used.						A note calls out for 2" rigid insulation, but points to concrete. Is rigid insulation required? If so, please revise drawing and specify which insulation is to be used.



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<b>P1-0068.1</b>	<b>Documentation for Concrete Platform at Bus Deck Superintendent Station</b>	<b>Void</b>	<b>0P</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI P1-0068 Response, Detail 2/A1-8181 (IFC Drawings for Main Package dated 3/31/2014)  As stated in the RFI P1-0068 Response, "The note is incorrect; no rigid insulation is required. In a future Addendum the Bus Deck Superintendent's Station will be revised to a pre-fabricated booth per VE mitigation meetings. The referenced sheet has been omitted in TG08.10. Documentation for pre-fabricated booths to be issued in a future package."  The concrete platform is to be furnished and installed as part of topping slab scope of work. Please provide documentation for concrete platform.						<b>ANSWER:</b>  REFERENCE: RFI P1-0068 Response, Detail 2/A1-8181 (IFC Drawings for Main Package dated 3/31/2014)  As stated in the RFI P1-0068 Response, "The note is incorrect; no rigid insulation is required. In a future Addendum the Bus Deck Superintendent's Station will be revised to a pre-fabricated booth per VE mitigation meetings. The referenced sheet has been omitted in TG08.10. Documentation for pre-fabricated booths to be issued in a future package."  The concrete platform is to be furnished and installed as part of topping slab scope of work. Please provide documentation for concrete platform.
<b>P1-0069</b>	<b>Grout at Steel Beam</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>  Reference: 4/A1-9228  Detail 4/A1-9228 shows a fully grouted beam pocket, but does not show details on securing grout to the steel beam, or specify a grout to be used. Please provide details and specs.						<b>ANSWER:</b>  Reference: 4/A1-9228  Detail 4/A1-9228 shows a fully grouted beam pocket, but does not show details on securing grout to the steel beam, or specify a grout to be used. Please provide details and specs.
<b>P1-0070</b>	<b>Steel Flashing at Column</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/09/2014</b>	<b>06/09/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>  Reference: 3/A1-3190  Detail calls out for a 12 ga. steel flashing closure plate within the web of interior cruciform columns, but does not provide details for attachment, or where material is to flow from the flashing (there is a checker plate cap at the top of the column to deflect material from entering the enclosure). Is the 12 ga steel flashing required? If so, please provide details on attachment and draining. If not, please revise detail.						<b>ANSWER:</b>  Reference: 3/A1-3190  Detail calls out for a 12 ga. steel flashing closure plate within the web of interior cruciform columns, but does not provide details for attachment, or where material is to flow from the flashing (there is a checker plate cap at the top of the column to deflect material from entering the enclosure). Is the 12 ga steel flashing required? If so, please provide details on attachment and draining. If not, please revise detail.



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<b>P1-0071</b>	<b>Layout for Guardrail</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b> Reference: 1/L1-9665  Provide layout for guardrail and associated stone header.					<b>ANSWER:</b> Reference: 1/L1-9665  Provide layout for guardrail and associated stone header.	
<b>P1-0072</b>	<b>C-Channel Support in Mech Shaft 01244</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b> Reference: 1/A1-7575  This C-channel support is not shown in plan view in the Architectural or Structural drawings. Please provide size and layout.					<b>ANSWER:</b> Reference: 1/A1-7575  This C-channel support is not shown in plan view in the Architectural or Structural drawings. Please provide size and layout.	
<b>P1-0073</b>	<b>C-Channel Support in Mech Shaft 01242</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b> Reference: 7/A1-7579  This C-channel support is not shown in plan view in the Architectural or Structural drawings. Please provide size and layout.					<b>ANSWER:</b> Reference: 7/A1-7579  This C-channel support is not shown in plan view in the Architectural or Structural drawings. Please provide size and layout.	
<b>P1-0074</b>	<b>Davit Arm Sockets</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b> Reference: 2/A1-3001  Detail calls out for davit arm sockets, but does not provide a detail or specification for the davit arm sockets. Please provide details and specs for the davit arms sockets.					<b>ANSWER:</b> Reference: 2/A1-3001  Detail calls out for davit arm sockets, but does not provide a detail or specification for the davit arm sockets. Please provide details and specs for the davit arms sockets.	
<b>P1-0075</b>	<b>Angle at Shaw Alley Bridge Topping Slab</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/11/2014</b>	<b>06/24/2014</b>



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	<p><b>From:</b> Webcor Construction LP                      Scott Shope</p> <p><b>REQUEST:</b></p> <p>Reference: 1/A1-8179</p> <p>Detail shows an angle behind the deck closure plate (screenshot attached), but does not identify the function of the angle. Please provide the function, size and location of the angle.</p>					
	<p><b>ANSWER:</b></p> <p>Reference: 1/A1-8179</p> <p>Detail shows an angle behind the deck closure plate (screenshot attached), but does not identify the function of the angle. Please provide the function, size and location of the angle.</p>					
<b>P1-0076</b>	<b>CMU Lateral Ties</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Scott Shope</p> <p><b>REQUEST:</b></p> <p>Reference: 1/A1-9207</p> <p>Details call out for lateral ties to support CMU and directs to "ref. to structural." Structural does not appear to provide details for this work. Please provide structural detail for this work.</p>					
	<p><b>ANSWER:</b></p> <p>Reference: 1/A1-9207</p> <p>Details call out for lateral ties to support CMU and directs to "ref. to structural." Structural does not appear to provide details for this work. Please provide structural detail for this work.</p>					
<b>P1-0076.1</b>	<b>Structural Details for Lateral Ties per Detail 1/A1-9207</b>	<b>Void</b>	<b>0P</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: RFI Response P1-0076, Detail 1/A1-9207</p> <p>Per RFI Response P1-0076, "Lateral ties have been eliminated. Detail shall be updated in future ASI."</p> <p>RFI Response P1-0076 refers to a revised detail to be issued in a future bid package. The detail is required to accurately bid TG07.4 - Concrete Masonry Unit. Please provide the revised detail referenced in RFI Response P1-0076.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: RFI Response P1-0076, Detail 1/A1-9207</p> <p>Per RFI Response P1-0076, "Lateral ties have been eliminated. Detail shall be updated in future ASI."</p> <p>RFI Response P1-0076 refers to a revised detail to be issued in a future bid package. The detail is required to accurately bid TG07.4 - Concrete Masonry Unit. Please provide the revised detail referenced in RFI Response P1-0076.</p>					



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<b>P1-0077</b>	<b>CMU Lateral Ties</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>  Reference: 5/A1-9206  Details call out for lateral ties to support CMU and directs to "ref. to structural." Structural does not appear to provide details for this work. Please provide structural detail for this work.					<b>ANSWER:</b>  Reference: 5/A1-9206  Details call out for lateral ties to support CMU and directs to "ref. to structural." Structural does not appear to provide details for this work. Please provide structural detail for this work.	
<b>P1-0077.1</b>	<b>Structural Details for Lateral Ties per Detail 5/A1-9206</b>	<b>Void</b>	<b>0P</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0077, Detail 5/A1-9206  Per RFI Response P1-0077, "Lateral ties have been eliminated. Detail shall be updated in future ASI."  RFI Response P1-0077 refers to a revised detail to be issued in a future bid package. The detail is required to accurately bid TG07.4 - Concrete Masonry Unit. Please provide the revised detail referenced in RFI Response P1-0077.					<b>ANSWER:</b>  REFERENCE: RFI Response P1-0077, Detail 5/A1-9206  Per RFI Response P1-0077, "Lateral ties have been eliminated. Detail shall be updated in future ASI."  RFI Response P1-0077 refers to a revised detail to be issued in a future bid package. The detail is required to accurately bid TG07.4 - Concrete Masonry Unit. Please provide the revised detail referenced in RFI Response P1-0077.	
<b>P1-0078</b>	<b>Flood Coat of Asphalt</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/21/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>  Reference: 3/A1-3191  Detail calls out for a "flood coat of asphalt" over foam glass insulation. Please provide the specification of the asphalt flood coat.					<b>ANSWER:</b>  Reference: 3/A1-3191  Detail calls out for a "flood coat of asphalt" over foam glass insulation. Please provide the specification of the asphalt flood coat.	
<b>P1-0079</b>	<b>Topping Slab in Loading Dock Area</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>  Reference: 3/A1-3192					<b>ANSWER:</b>  Reference: 3/A1-3192	



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	<p>Detail calls out for a traffic topping at the loading dock area, but A1-9523 does not call out the topping slab as a traffic topping. Please confirm the topping slab requirement at this location.</p>					
<b>P1-0079.1</b>	<b>Topping Slab and Reinforcement Requirements for Loading Dock 01222</b>	<b>Closed</b>	<b>0P</b>	<b>07/16/2014</b>	<b>07/26/2014</b>	<b>07/24/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Sheet A1-9526 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Please confirm Loading Dock 01222 is to receive non-vehicular rated concrete topping slab and reinforcement as called out in Item 4.0 of the A1-9526 Notes.</p>					



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<b>P1-0080.1</b>	<b>Concrete Block Placement for Gantry Crane Support at Transformer Vaults</b>	<b>Closed</b>	<b>0P</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI Response P1-0080, Sketch SKA-0360, Sketch SKA-3694		REFERENCE: RFI Response P1-0080, Sketch SKA-0360, Sketch SKA-3694				
Per RFI Response P1-0080, Sketch SKS-0360 shows the reinforced concrete block is to be placed directly on top of the structural MFB Beam.		Per RFI Response P1-0080, Sketch SKS-0360 shows the reinforced concrete block is to be placed directly on top of the structural MFB Beam.				
Per RFI Response P1-0080, Sketch SKA-3694, the reinforced concrete block is placed directly on top of all the following components: the drainage composite and protection board, WPM-1A flashing, and structural MFB.		Per RFI Response P1-0080, Sketch SKA-3694, the reinforced concrete block is placed directly on top of all the following components: the drainage composite and protection board, WPM-1A flashing, and structural MFB.				
Please clarify if the reinforced concrete block is to be placed directly over the structural MFB, or on top of drainage composite and protection board. Please revise details to match.		Please clarify if the reinforced concrete block is to be placed directly over the structural MFB, or on top of drainage composite and protection board. Please revise details to match.				
<b>P1-0080.2</b>	<b>Concrete Mix for Concrete Block at Gantry Crane Support</b>	<b>Closed</b>	<b>0P</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI Response P1-0080, Sketch SKS-0360		REFERENCE: RFI Response P1-0080, Sketch SKS-0360				
Per RFI Response P1-0080, the gantry equipment is to be supported by a reinforced concrete block as detailed on Sketch SKS-0360, but does not call out the type of concrete mix to use.		Per RFI Response P1-0080, the gantry equipment is to be supported by a reinforced concrete block as detailed on Sketch SKS-0360, but does not call out the type of concrete mix to use.				
Specification Section 03 30 02 2.1 does not specifically note a mix for "Concrete Blocks".		Specification Section 03 30 02 2.1 does not specifically note a mix for "Concrete Blocks".				
Please confirm the "All other concrete" mix called out in Specification Section 03 30 02 2.1 is to be used to furnish and install the concrete block referenced in RFI Response P1-0080.		Please confirm the "All other concrete" mix called out in Specification Section 03 30 02 2.1 is to be used to furnish and install the concrete block referenced in RFI Response P1-0080.				



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<b>P1-0081</b>	<b>Pour Stop at Column</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b> Reference: 1/A1-9211  Detail appears to show a break metal pour stop around the column. Please provide information (material, attachment, size, etc.) of the pour stop.		<b>ANSWER:</b> Reference: 1/A1-9211  Detail appears to show a break metal pour stop around the column. Please provide information (material, attachment, size, etc.) of the pour stop.				
<b>P1-0082</b>	<b>Dimension of Seismic Joint</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b> Reference: 3/L1-7633  Dimension conflicts with Architectural - Conflicts with 2/A1-8897. Please coordinate landscape and architectural.		<b>ANSWER:</b> Reference: 3/L1-7633  Dimension conflicts with Architectural - Conflicts with 2/A1-8897. Please coordinate landscape and architectural.				
<b>P1-0082.1</b>	<b>Clarification of Siesmic Joints and Curb Dimensions on the Roof Park Level</b>	<b>Closed</b>	<b>0P</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b> REFERENCE: RFI Response P1-0082, Detail 3/L1-7633, Detail 2/A1-8897  RFI Response P1-0082 states, "Architectural Dimensions generally will govern. Please ask specific questions on conflicts."  Per Detail 3/L1-7633, the park level seismic joints are 1'-10" and are flanked by 1'-6" curbs. Per Detail 2/A1-8897, the park level seismic joints at GL 10 and GL 20 are 2'-0" and are flanked by 8" curbs. Please confirm the roof park level seismic joint and associated curb dimensions, and coordinate the details.		<b>ANSWER:</b> REFERENCE: RFI Response P1-0082, Detail 3/L1-7633, Detail 2/A1-8897  RFI Response P1-0082 states, "Architectural Dimensions generally will govern. Please ask specific questions on conflicts."  Per Detail 3/L1-7633, the park level seismic joints are 1'-10" and are flanked by 1'-6" curbs. Per Detail 2/A1-8897, the park level seismic joints at GL 10 and GL 20 are 2'-0" and are flanked by 8" curbs. Please confirm the roof park level seismic joint and associated curb dimensions, and coordinate the details.				
<b>P1-0083</b>	<b>PVC Roofing Substrate Requirement</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b> Reference: Specification Section 07 54 19 3.2A		<b>ANSWER:</b> Reference: Specification Section 07 54 19 3.2A				





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	<p>07 54 19 3.2A requires "the general contractor shall examine substrates, areas, and conditions" with the installer for compliance with the contract documents. Please remove the requirement for the general contractor to verify existing conditions with subcontractor. Subcontractor is solely responsible to verify and accept existing conditions prior to commencing work.</p>					<p>07 54 19 3.2A requires "the general contractor shall examine substrates, areas, and conditions" with the installer for compliance with the contract documents. Please remove the requirement for the general contractor to verify existing conditions with subcontractor. Subcontractor is solely responsible to verify and accept existing conditions prior to commencing work.</p>
P1-0083.1	Additional Review of PVC Substrate Requirement	Closed	0P	07/01/2014	07/11/2014	09/01/2014
From: Webcor Construction LP      Tram Nguyen						
REQUEST:						ANSWER:
REFERENCE: RFI Response P1-0083, Specification Section 07 54 19 3.2A						REFERENCE: RFI Response P1-0083, Specification Section 07 54 19 3.2A
As stated in RFI Response P1-0083, "The Contract Documents refer to the Contractor defined as the General Contractor/Construction Manager (CMGC) in the TJPA/CMGC Agreement for responsibility to complete the Work and for coordination of the Work, not to individual subcontractors that are contracted to the CMGC. The specification is correct."						As stated in RFI Response P1-0083, "The Contract Documents refer to the Contractor defined as the General Contractor/Construction Manager (CMGC) in the TJPA/CMGC Agreement for responsibility to complete the Work and for coordination of the Work, not to individual subcontractors that are contracted to the CMGC. The specification is correct."
Per the contract, the CMGC verifies compliance of the work with the contract documents via the QA/QC process at the time of installation. Additional review of the work to reverify conformance with the contract documents as set forth in Specification Section 07 54 19 3.2A is an added service. Please confirm TJPA intends to have this added service to CM/GC's contract incorporated into the work.						Per the contract, the CMGC verifies compliance of the work with the contract documents via the QA/QC process at the time of installation. Additional review of the work to reverify conformance with the contract documents as set forth in Specification Section 07 54 19 3.2A is an added service. Please confirm TJPA intends to have this added service to CM/GC's contract incorporated into the work.



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P1-0084	Attachment Details for Linear Supply Air Diffuser	Closed	0P	06/11/2014	06/21/2014	07/01/2014
From: Webcor Construction LP Scott Shope						
REQUEST: Reference: 5/A1-9040.  Please provide attachment details for 2" linear supply air diffuser		ANSWER:  Reference: 5/A1-9040.  Please provide attachment details for 2" linear supply air diffuser				
P1-0085	Trench Drain Type 1	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP Scott Shope						
REQUEST: Reference: 1/L1-7318  There does not appear to be a specification for the trench drain body, grate, or grate support. Please provide spec.		ANSWER:  Reference: 1/L1-7318  There does not appear to be a specification for the trench drain body, grate, or grate support. Please provide spec.				
P1-0085.1	Specification for Continuous Metal Liner in Detail 1/L1-7318	Closed	0P	07/01/2014	07/11/2014	07/11/2014
From: Webcor Construction LP Tram Nguyen						
REQUEST: REFERENCE: RFI Response 0085, Specification Section 05 60 00 2.3D & 2.3E, Detail 1/L1-7318  As stated in RFI Response 0085, "Refer to specification section 05 60 00, 2.3 D and E."  Detail 1/L1-7318 calls out for a continuous metal liner. Specification Section 05 60 00 does not appear to provide a specification for this metal liner. Please provide a specification for the metal liner called out in Detail 1/L1-7318.		ANSWER:  REFERENCE: RFI Response 0085, Specification Section 05 60 00 2.3D & 2.3E, Detail 1/L1-7318  As stated in RFI Response 0085, "Refer to specification section 05 60 00, 2.3 D and E."  Detail 1/L1-7318 calls out for a continuous metal liner. Specification Section 05 60 00 does not appear to provide a specification for this metal liner. Please provide a specification for the metal liner called out in Detail 1/L1-7318.				
P1-0086	Trench Drain Type 2	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP Scott Shope						
REQUEST: Reference: 2/L1-7318  There does not appear to be a specification for the trench		ANSWER:  Reference: 2/L1-7318  There does not appear to be a specification for the				



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	drain body, grate, or grate support. Please provide spec.					trench drain body, grate, or grate support. Please provide spec.
<b>P1-0086.1</b>	<b>Specification for Continuous Metal Liner in Detail 2/L1-7318</b>	<b>Closed</b>	<b>0P</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b> REFERENCE: RFI Response P1-0086, Specification Section 05 60 00, 2.3D & 2.3E, Detail 2/L1-7318  As stated in RFI Response P1-0086, "Refer to specification section 05 60 00, 2.3 D and E."  Detail 2/L1-7318 calls out for a continuous metal liner. Specification Section 05 60 00 does not appear to provide a specification for this metal liner. Please provide a specification for the continuous metal liner called out in Detail 1/L1-7318.						<b>ANSWER:</b> REFERENCE: RFI Response P1-0086, Specification Section 05 60 00, 2.3D & 2.3E, Detail 2/L1-7318  As stated in RFI Response P1-0086, "Refer to specification section 05 60 00, 2.3 D and E."  Detail 2/L1-7318 calls out for a continuous metal liner. Specification Section 05 60 00 does not appear to provide a specification for this metal liner. Please provide a specification for the continuous metal liner called out in Detail 1/L1-7318.
<b>P1-0087</b>	<b>Trench Drain at Property Line</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b> Reference: 3/L1-7318  There does not appear to be a specification for the trench drain body, grate, or grate support. Please provide spec.						<b>ANSWER:</b> Reference: 3/L1-7318  There does not appear to be a specification for the trench drain body, grate, or grate support. Please provide spec.



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<b>P1-0087.1</b>	<b>Specification for Continuous Metal Liner in Detail 3/L1-7318</b>	<b>Closed</b>	<b>0P</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0087, Specification Section 05 60 00, 2.3D & 2.3E, Detail 3/L1-7318  As stated in RFI Response P1-0087, "Refer to specification section 05 60 00, 2.3 D and E."  Detail 3/L1-7318 calls out for a continuous metal liner. Specification Section 05 60 00 does not appear to provide a specification for this metal liner. Please provide a specification for the continuous metal liner called out in Detail 3/L1-7318.						<b>ANSWER:</b>  REFERENCE: RFI Response P1-0087, Specification Section 05 60 00, 2.3D & 2.3E, Detail 3/L1-7318  As stated in RFI Response P1-0087, "Refer to specification section 05 60 00, 2.3 D and E."  Detail 3/L1-7318 calls out for a continuous metal liner. Specification Section 05 60 00 does not appear to provide a specification for this metal liner. Please provide a specification for the continuous metal liner called out in Detail 3/L1-7318.
<b>P1-0088</b>	<b>CMU Support Wall at Seismic Joint</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/11/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP                      Scott Shope						
<b>REQUEST:</b>  Reference: 1/L1-7613  Detail does not agree with what is on 1/A1-8898. Please coordinate drawings (waterproofing, adj. wall/curb, topping slab, etc.)						<b>ANSWER:</b>  Reference: 1/L1-7613  Detail does not agree with what is on 1/A1-8898. Please coordinate drawings (waterproofing, adj. wall/curb, topping slab, etc.)
<b>P1-0089</b>	<b>WPM-6</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP                      Scott Shope						
<b>REQUEST:</b>  Reference: 4/A1-7511.  Detail calls out WPM-6. No specification for WPM-6 has been provided. Please provide a specification for WPM-6.						<b>ANSWER:</b>  Reference: 4/A1-7511.  Detail calls out WPM-6. No specification for WPM-6 has been provided. Please provide a specification for WPM-6.
<b>P1-0090</b>	<b>Waterproofing soffit</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>  reference: 4/A1-7511  Detail shows waterproofing at the top of soffit below the						<b>ANSWER:</b>  reference: 4/A1-7511  Detail shows waterproofing at the top of soffit below



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P1-0091	Plywood Installed with Soil	Closed	0P	06/11/2014	06/21/2014	06/24/2014
	From: Webcor Construction LP      Zachary Moore					
REQUEST:			ANSWER:			
reference: 2/L1-9665			reference: 2/L1-9665			
Detail calls out for 3/4" plywood to be installed directly in contact with soil. Please provide specification for plywood.			Detail calls out for 3/4" plywood to be installed directly in contact with soil. Please provide specification for plywood.			
P1-0091.1	Specification for Plywood Installed with Soil per Detail 2/L1-9665	Closed	0P	07/02/2014	07/12/2014	07/11/2014
	From: Webcor Construction LP      Tram Nguyen					
REQUEST:			ANSWER:			
REFERENCE: RFI Response P1-0091, Detail 2/L1-9665			REFERENCE: RFI Response P1-0091, Detail 2/L1-9665			
As stated in RFI Response P1-0091, "3/4" plywood is temporary protection. It is not necessary to be treated or finished surfaced."			As stated in RFI Response P1-0091, "3/4" plywood is temporary protection. It is not necessary to be treated or finished surfaced."			
The response to RFI P1-0091 does not appear to provide the requested specification. Please provide a specification for the 3/4" plywood called out in Detail 2/L1-9665.			The response to RFI P1-0091 does not appear to provide the requested specification. Please provide a specification for the 3/4" plywood called out in Detail 2/L1-9665.			



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P1-0092	Rated Soffit	Closed	0P	06/11/2014	06/21/2014	07/11/2014
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b> reference: 4/A1-7511  Detail calls out for a rated soffit below the stairs, but the top end of the soffit does not appear to be a rated condition. Please confirm the end of the soffit does not need to be rated.		<b>ANSWER:</b> reference: 4/A1-7511  Detail calls out for a rated soffit below the stairs, but the top end of the soffit does not appear to be a rated condition. Please confirm the end of the soffit does not need to be rated.				
P1-0093	Fire rated assembly	Closed	0P	06/11/2014	06/21/2014	06/24/2014
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b> reference: 1/A1-7575  Is this a fire rated assembly? Code calls out for stl. to be independently rated, does TS meet this requirement?		<b>ANSWER:</b> reference: 1/A1-7575  Is this a fire rated assembly? Code calls out for stl. to be independently rated, does TS meet this requirement?				
P1-0094	Elevator Deferral	Closed	0P	06/11/2014	06/21/2014	06/24/2014
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b> reference: 1/A1-7589A  The elevator in this detail has been deferred. Is the elevator call lantern on this deleted elevator to be roughed in as part of Phase 1? If so, please provide rough-in information. If not, please revise details.		<b>ANSWER:</b> reference: 1/A1-7589A  The elevator in this detail has been deferred. Is the elevator call lantern on this deleted elevator to be roughed in as part of Phase 1? If so, please provide rough-in information. If not, please revise details.				
P1-0094.1	Additional Information For Elevator Deferral	Closed	0P	07/02/2014	07/12/2014	07/11/2014
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b> Reference: P1-0094, 1/A1-7589A  P1-0094:The elevator will be roughed-in as part of Phase 1. Refer to sheet A1-7576 & A1-7577 for rough-in information.  RFI P1-0094 provides direction to rough-in the elevator		<b>ANSWER:</b> Reference: P1-0094, 1/A1-7589A  P1-0094:The elevator will be roughed-in as part of Phase 1. Refer to sheet A1-7576 & A1-7577 for rough-in information.  RFI P1-0094 provides direction to rough-in the				





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<b>P1-0096</b>	<b>Framing Material</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/01/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>  Reference: 4/-A1-7823  Specs indicate that drawings will provide size and spacing of framing material, dwg does not show size or max spacing. Please provide size and spacing of Z girts.						<b>ANSWER:</b>  Reference: 4/-A1-7823  Specs indicate that drawings will provide size and spacing of framing material, dwg does not show size or max spacing. Please provide size and spacing of Z girts.
<b>P1-0096.1</b>	<b>Information for Vertical Z Girts</b>	<b>Closed</b>	<b>0P</b>	<b>07/09/2014</b>	<b>07/19/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0096  As stated in RFI Response P1-0096, "Size of horizontal girts is shown on 4/A1-7823 as 2" depth. Vertical spacing of Z girts to match panelization pattern of W-5 cladding."  Please provide the information requested and referenced in RFI P1-0096 regarding vertical Z girt adjacent to door openings.						<b>ANSWER:</b>  REFERENCE: RFI Response P1-0096  As stated in RFI Response P1-0096, "Size of horizontal girts is shown on 4/A1-7823 as 2" depth. Vertical spacing of Z girts to match panelization pattern of W-5 cladding."  Please provide the information requested and referenced in RFI P1-0096 regarding vertical Z girt adjacent to door openings.
<b>P1-0097</b>	<b>Bench Details</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/01/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>  reference: 1/A1-9061 Detail calls out for benches, but does not provide installation details. Please provide bench details.						<b>ANSWER:</b>  reference: 1/A1-9061 Detail calls out for benches, but does not provide installation details. Please provide bench details.
<b>P1-0097.1</b>	<b>Clarification for Choice in Bench Anchors</b>	<b>Closed</b>	<b>0P</b>	<b>07/09/2014</b>	<b>07/19/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0097, Specification Section 10 51 13  As stated in RFI Response P1-0097, "As per specification section 10 51 13, benches are manufactured product and						<b>ANSWER:</b>  REFERENCE: RFI Response P1-0097, Specification Section 10 51 13  As stated in RFI Response P1-0097, "As per specification section 10 51 13, benches are









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P1-0099.3	VOID	Void	CR	08/14/2014	08/24/2014	
From: Webcor Construction LP      Tram Nguyen						
REQUEST:			ANSWER:			
P1-0100	Floor Mislabeled	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP      Zachary Moore						
REQUEST: 3/A1-7513  Floor is mislabeled as "galv steel pour stop," Please revise note.			ANSWER: 3/A1-7513  Floor is mislabeled as "galv steel pour stop," Please revise note.			
P1-0101	Access Door	Void	0P	06/11/2014	06/21/2014	
From: Webcor Construction LP      Zachary Moore						
REQUEST: reference: A1-7427  Details 1 & 2 call out for an access door, but no door number, door material, or hardware information is provided. Please provide information on this access door.			ANSWER: reference: A1-7427  Details 1 & 2 call out for an access door, but no door number, door material, or hardware information is provided. Please provide information on this access door.			
P1-0102	Door Callout	Void	0P	06/11/2014	06/21/2014	
From: Webcor Construction LP      Zachary Moore						
REQUEST: reference:1/A1-3001 Note calls out for a door, however no Door is shown should this just opening? Sim E/A1-9235			ANSWER: reference:1/A1-3001 Note calls out for a door, however no Door is shown should this just opening? Sim E/A1-9235			



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P1-0103	Decking Support Architectural vs Structural Coordination	Closed	0P	06/11/2014	06/21/2014	07/14/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: 2/A1-9229  Detail shows decking supported on a C-channel wedge anchored into the adjacent CMU. Structural shows decking penetrating CMU, please coordinate arch vs. structural.		ANSWER: reference: 2/A1-9229  Detail shows decking supported on a C-channel wedge anchored into the adjacent CMU. Structural shows decking penetrating CMU, please coordinate arch vs. structural.				
P1-0104	Wide Flange assembly Details	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: 1/A1-7550 Code requires structural steel members to be fire proofed independently of adj. walls. Is the top of wide flange in this detail a rated UL assembly?		ANSWER: 1/A1-7550 Code requires structural steel members to be fire proofed independently of adj. walls. Is the top of wide flange in this detail a rated UL assembly?				
P1-0105	Plate over Wide Flange	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: 1/A1-7550  Detail indicates the plate over the wide flange is identified on structural. Structural does not appear to call out this plate. Please provide information (size, attachment, etc.) for steel plate.		ANSWER: reference: 1/A1-7550  Detail indicates the plate over the wide flange is identified on structural. Structural does not appear to call out this plate. Please provide information (size, attachment, etc.) for steel plate.				
P1-0106	Conflicting Details	Closed	0P	06/11/2014	06/21/2014	07/14/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: 3/A1-9229  Detail appears to conflict with assembly shown on S1-2252, please coordinate arch and struct.		ANSWER: reference: 3/A1-9229  Detail appears to conflict with assembly shown on S1-2252, please coordinate arch and struct.				
P1-0106R	Composite Deck Type S1 Support	Closed	0P	07/16/2014	07/26/2014	07/28/2014





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P1-0108	Waterproofing System Spec	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:						ANSWER:
reference: 07 54 19   2.2A						reference: 07 54 19   2.2A
07 54 19 2.2A specifies only Sika Sarafil Waterproofing systems, or substitute products of another manufacturer for RF-1. Webcor intends to require all manufactures be approved via QBD prior to bid. If this is not acceptable, please provide specific manufacturers and products which are acceptable to use.						07 54 19 2.2A specifies only Sika Sarafil Waterproofing systems, or substitute products of another manufacturer for RF-1. Webcor intends to require all manufactures be approved via QBD prior to bid. If this is not acceptable, please provide specific manufacturers and products which are acceptable to use.
P1-0108.1	Product/Manufacturer Substitution for PVC Roofing	Closed	0P	07/31/2014	08/10/2014	08/05/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:						ANSWER:
REFERENCE: RFI Response P1-0108, Specification Section 00 04 40						REFERENCE: RFI Response P1-0108, Specification Section 00 04 40
RFI Response P1-0108 states, "The Request for Substitution process established by TJPA and WOJV as outlined in the General Conditions specification section 00 04 40 is the process to be utilized for Bidder proposed substitutions to the Basis of Design."						RFI Response P1-0108 states, "The Request for Substitution process established by TJPA and WOJV as outlined in the General Conditions specification section 00 04 40 is the process to be utilized for Bidder proposed substitutions to the Basis of Design."
Specification Section 00 04 40 states, "Contractor will be provided a period of 10 days after the date of each Trade Package award for submission of data substantiating a request for a substitution with an "or equal" item."						Specification Section 00 04 40 states, "Contractor will be provided a period of 10 days after the date of each Trade Package award for submission of data substantiating a request for a substitution with an "or equal" item."
The specification section referenced in RFI Response P1-0108 indicates that substitutions are addressed post-bid.						The specification section referenced in RFI Response P1-0108 indicates that substitutions are addressed post-bid.
Per the meeting between TJPA, PCPA, Turner, and Webcor-Obayashi on 7/17/14, the response to RFI P1-0108 is to be revised to state that the QBD process is to be used for suggested alternate materials/manufacturers pre-bid.						Per the meeting between TJPA, PCPA, Turner, and Webcor-Obayashi on 7/17/14, the response to RFI P1-0108 is to be revised to state that the QBD process is to be used for suggested alternate materials/manufacturers pre-bid.
RFI Response P1-0108 has not been revised.						RFI Response P1-0108 has not been revised.
Please confirm the QBD process is to be used to suggestion of alternate materials/manufacturers.						Please confirm the QBD process is to be used to suggestion of alternate materials/manufacturers.



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<b>P1-0109</b>	<b>Dampproofing spec</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/01/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>			<b>ANSWER:</b>			
reference: 07 11 16 2.3			reference: 07 11 16 2.3			
07 11 16 2.3 specifies Degussa Building Systems (BASF Company), or equal for dampproofing. In an attempt to prevent single sourcing, and assure bidders only bid approved materials, Webcor intends to require any alternate manufactures be approved via QBD prior to bid. If this is not acceptable, please provide alternate manufacturers/products which are acceptable to use.			07 11 16 2.3 specifies Degussa Building Systems (BASF Company), or equal for dampproofing. In an attempt to prevent single sourcing, and assure bidders only bid approved materials, Webcor intends to require any alternate manufactures be approved via QBD prior to bid. If this is not acceptable, please provide alternate manufacturers/products which are acceptable to use.			



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<b>P1-0109.1</b>	<b>Product/Manufacturer Substitution for Site Dampproofing</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI Response P1-0109, Specification Section 00 04 40		REFERENCE: RFI Response P1-0109, Specification Section 00 04 40				
RFI Response P1-0109 states, "The Request for Substitution process established by TJPA and WOJV as outlined in the General Conditions specification section 00 04 40 is the process to be utilized for Bidder proposed substitutions to the Basis of Design."		RFI Response P1-0109 states, "The Request for Substitution process established by TJPA and WOJV as outlined in the General Conditions specification section 00 04 40 is the process to be utilized for Bidder proposed substitutions to the Basis of Design."				
Specification Section 00 04 40 states, "Contractor will be provided a period of 10 days after the date of each Trade Package award for submission of data substantiating a request for a substitution with an "or equal" item."		Specification Section 00 04 40 states, "Contractor will be provided a period of 10 days after the date of each Trade Package award for submission of data substantiating a request for a substitution with an "or equal" item."				
The specification section referenced in RFI Response P1-0109 indicates that substitutions are addressed post-bid.		The specification section referenced in RFI Response P1-0109 indicates that substitutions are addressed post-bid.				
Per the meeting between TJPA, PCPA, Turner, and Webcor-Obayashi on 7/17/14, the response to RFI P1-0109 is to be revised to state that the QBD process is to be used for suggested alternate materials/manufacturers pre-bid.		Per the meeting between TJPA, PCPA, Turner, and Webcor-Obayashi on 7/17/14, the response to RFI P1-0109 is to be revised to state that the QBD process is to be used for suggested alternate materials/manufacturers pre-bid.				
RFI Response P1-0109 has not been revised.		RFI Response P1-0109 has not been revised.				
Please confirm the QBD process is to be used to suggestion of alternate materials/manufacturers.		Please confirm the QBD process is to be used to suggestion of alternate materials/manufacturers.				
<b>P1-0110</b>	<b>Anchoring Details for Bicycle Sign</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: 1/SG1-6202		Reference: 1/SG1-6202				
Please provide anchoring details for the post mounted bicycle directional sign shown on detail 1 / SG1-6202		Please provide anchoring details for the post mounted bicycle directional sign shown on detail 1 / SG1-6202				
<b>P1-0110.1</b>	<b>Signage Mounting Design Requirements</b>	<b>Closed</b>	<b>0P</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						





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<div><div><b>REQUEST:</b><p>REFERENCE: RFI Response P1-0110, SG1 Drawings, Division 10 Specifications</p><p>Please refer to SG1-6202, "General Sheet Notes, Item 'D' - "SIGN SUB-CONTRACTOR TO PROVIDE SIGN MOUNTING LOCATIONS/SURFACE CONDITION REQUIREMENTS AND METHOD OF ATTACHMENTS TO TJPA REPRESENTATIVE". Sub-Contractor's proposed anchoring details (proposed 'methods of attachment') are to be included as a component of the required signage submittal/shop drawing package."</p><p>SG1 Drawings and Division 10 Specifications do not appear to provide mounting design requirements (i.e. allowable point loads, mounting into concrete over geosynthetic fill, mounting into waterproofing, etc.). Please provide design requirements for mounting all signage.</p></div><div><b>ANSWER:</b><p>REFERENCE: RFI Response P1-0110, SG1 Drawings, Division 10 Specifications</p><p>Please refer to SG1-6202, "General Sheet Notes, Item 'D' - "SIGN SUB-CONTRACTOR TO PROVIDE SIGN MOUNTING LOCATIONS/SURFACE CONDITION REQUIREMENTS AND METHOD OF ATTACHMENTS TO TJPA REPRESENTATIVE". Sub-Contractor's proposed anchoring details (proposed 'methods of attachment') are to be included as a component of the required signage submittal/shop drawing package."</p><p>SG1 Drawings and Division 10 Specifications do not appear to provide mounting design requirements (i.e. allowable point loads, mounting into concrete over geosynthetic fill, mounting into waterproofing, etc.). Please provide design requirements for mounting all signage.</p></div></div>						
<b>P1-0111</b>	<b>Callout on L1-9612</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/01/2014</b>
<div><div><b>From:</b> Webcor Construction LP      Andrew Kitchen</div><div><b>REQUEST:</b><p>Reference: 3/L1-9612</p><p>"DECK BOARD TYP" is called out twice, one of which is pointing to a boulder. Please provide correct callout.</p></div><div><b>ANSWER:</b><p>Reference: 3/L1-9612</p><p>"DECK BOARD TYP" is called out twice, one of which is pointing to a boulder. Please provide correct callout.</p></div></div>						
<b>P1-0112</b>	<b>Concrete Cure</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/01/2014</b>
<div><div><b>From:</b> Webcor Construction LP      Andrew Kitchen</div><div><b>REQUEST:</b><p>Reference: 32 34 10 3.3 B3</p><p>Requires that concrete cure for 28 days above 70 degrees Fahrenheit, which will require heating of the concrete for a month. Is this what TJPA wants to do?</p></div><div><b>ANSWER:</b><p>Reference: 32 34 10 3.3 B3</p><p>Requires that concrete cure for 28 days above 70 degrees Fahrenheit, which will require heating of the concrete for a month. Is this what TJPA wants to do?</p></div></div>						
<b>P1-0113</b>	<b>Incorrect Detail on A1-6014 and A1-6016</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/11/2014</b>	<b>06/24/2014</b>

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<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference: A1-6014 &amp; A1-6016</p> <p>A1-6014 and A1-6016 call out 7/A1-8717 at the detail at center of street. This appears to be the incorrect detail, and should be 5/A1-8717. Please revise.</p>						<p><b>ANSWER:</b></p> <p>Reference: A1-6014 &amp; A1-6016</p> <p>A1-6014 and A1-6016 call out 7/A1-8717 at the detail at center of street. This appears to be the incorrect detail, and should be 5/A1-8717. Please revise.</p>
<b>P1-0114</b>	<b>Steel Plate Coordination on Architectural Drawings</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/01/2014</b>
<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference: A1-8881, A1-8717 &amp; A1-8710</p> <p>2/A1-8710, 5/A1-8717 and 3/A1-8881 requires 1" steel plate under roadways and curbs below roadways. A1-2922 through A1-2927 omit a significant amount of plate shown on Beale, Fremont, First, Minna, and Natoma Streets by the details. Please revise the details or plan sheets to agree with each other.</p>						<p><b>ANSWER:</b></p> <p>Reference: A1-8881, A1-8717 &amp; A1-8710</p> <p>2/A1-8710, 5/A1-8717 and 3/A1-8881 requires 1" steel plate under roadways and curbs below roadways. A1-2922 through A1-2927 omit a significant amount of plate shown on Beale, Fremont, First, Minna, and Natoma Streets by the details. Please revise the details or plan sheets to agree with each other.</p>



**ANSWER:**

REFERENCE:

RFI Response P1-0114

Sketch SKA-3693

Sketch SKA-3695

Sketch SKA-3696

Sketch SKA-3697

Sketch SKA-3698

Detail 2/SKA-3538 (ASI 119 dated 6/20/14)

Per RFI Response P1-0114:

1. Sketch SKA-3693 does not show metal plates at the following Gridlines: C/2 and G/4.
2. Sketch SKA-3695 does not show metal plates at the following Gridlines: G/9.9, G/10.1, G/12, and F/9.
3. Sketch SKA-3696 does not show a metal plate at Gridline G/16
4. Sketch SKA-3697 does not show metal plates at the following Gridlines: D/19.9, D/20.1, D/21, D22, D24, F/19.9, F/20.1, F/21, F/22, and F/24
5. Sketch SKA-3698 does not show metal plates at any location.

However, Sketches SKA-3693, SKA-3695, SKA-3696, and SKA-3697 show metal plates are to be installed at significant amounts of similar conditions.

Please note Detail 2/SKA-3538 of ASI 119 shows metal plates at GL G/10.1 but steel plates are omitted on Sketch-3695 at this location

Please confirm that only locations identified as having metal plates on Sketches SKA-3693, SKA-3695, SKA-3696, and SKA-3697 require metal plates.



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<b>P1-0115</b>	<b>Electrical Continuity and Grounding</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference: 05 50 00  05 50 00 1.1 A4 indicates that electrical continuity and grounding for metal fabrications is included in 05 50 00, but give no further information on what is required. Please confirm no grounding of metal fabrications is required, or provide the grounding requirements.						<b>ANSWER:</b>  Reference: 05 50 00  05 50 00 1.1 A4 indicates that electrical continuity and grounding for metal fabrications is included in 05 50 00, but give no further information on what is required. Please confirm no grounding of metal fabrications is required, or provide the grounding requirements.
<b>P1-0116</b>	<b>Perm-A-Barrier VPS Contractors</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/01/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference: 07 13 14 2.3 B1  07 13 14 2.3 B1 specifies Grace "Perm-A-Barrier VPS" or approved equal for WPM-10A. In an attempt to prevent single sourcing, and assure bidders only bid approved materials, Webcor intends to require any alternate manufacture be approved via QBD prior to bid. If this is not acceptable, please provide alternate manufacturers/products which are acceptable to use.						<b>ANSWER:</b>  Reference: 07 13 14 2.3 B1  07 13 14 2.3 B1 specifies Grace "Perm-A-Barrier VPS" or approved equal for WPM-10A. In an attempt to prevent single sourcing, and assure bidders only bid approved materials, Webcor intends to require any alternate manufacture be approved via QBD prior to bid. If this is not acceptable, please provide alternate manufacturers/products which are acceptable to use.
<b>P1-0116.1</b>	<b>Product/Manufacturer Substitution for Self-Adhered Sheet Waterproofing</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0116, Specification Section 00 04 40  RFI Response P1-0116 states, "The Request for Substitution process established by TJPA and WOJV as outlined in the General Conditions specification section 00 04 40 is the process to be utilized for Bidder proposed substitutions to the Basis of Design."  Specification Section 00 04 40 states, "Contractor will be provided a period of 10 days after the date of each Trade Package award for submission of data substantiating a request for a substitution with an "or equal" item."  The specification section referenced in RFI Response P1-						<b>ANSWER:</b>  REFERENCE: RFI Response P1-0116, Specification Section 00 04 40  RFI Response P1-0116 states, "The Request for Substitution process established by TJPA and WOJV as outlined in the General Conditions specification section 00 04 40 is the process to be utilized for Bidder proposed substitutions to the Basis of Design."  Specification Section 00 04 40 states, "Contractor will be provided a period of 10 days after the date of each Trade Package award for submission of data substantiating a request for a substitution with an "or equal" item."



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	<p>0116 indicates that substitutions are addressed post-bid.</p> <p>Per the meeting between TJPA, PCPA, Turner, and Webcor-Obayashi on 7/17/14, the response to RFI P1-0116 is to be revised to state that the QBD process is to be used for suggested alternate materials/manufacturers pre-bid.</p> <p>RFI Response P1-0116 has not been revised.</p> <p>Please confirm the QBD process is to be used to suggestion of alternate materials/manufacturers.</p>					<p>The specification section referenced in RFI Response P1-0116 indicates that substitutions are addressed post-bid.</p> <p>Per the meeting between TJPA, PCPA, Turner, and Webcor-Obayashi on 7/17/14, the response to RFI P1-0116 is to be revised to state that the QBD process is to be used for suggested alternate materials/manufacturers pre-bid.</p> <p>RFI Response P1-0116 has not been revised.</p> <p>Please confirm the QBD process is to be used to suggestion of alternate materials/manufacturers.</p>
<b>P1-0117</b>	<b>Waterproofing Systems</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/01/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: 07 13 54 2.1 A						Reference: 07 13 54 2.1 A
07 13 54 2.1 A specifies Sika Sarnafil Waterproofing Systems, or other manufacturers that meet or exceed their physical property specifications for WPM-3. In an attempt to prevent single sourcing, and assure bidders only bid approved materials, Webcor intends to require any alternate manufactures be approved via QBD prior to bid. If this is not acceptable, please provide alternate manufacturers/products which are acceptable to use.						07 13 54 2.1 A specifies Sika Sarnafil Waterproofing Systems, or other manufacturers that meet or exceed their physical property specifications for WPM-3. In an attempt to prevent single sourcing, and assure bidders only bid approved materials, Webcor intends to require any alternate manufactures be approved via QBD prior to bid. If this is not acceptable, please provide alternate manufacturers/products which are acceptable to use.



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<b>P1-0117.1</b>	<b>Product/Manufacturer for PVC Waterproofing</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/06/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI Response P1-0117, Specification Section 00 04 40		REFERENCE: RFI Response P1-0117, Specification Section 00 04 40				
RFI Response P1-0117 states, "The Request for Substitution process established by TJPA and WOJV as outlined in the General Conditions specification section 00 04 40 is the process to be utilized for Bidder proposed substitutions to the Basis of Design."		RFI Response P1-0117 states, "The Request for Substitution process established by TJPA and WOJV as outlined in the General Conditions specification section 00 04 40 is the process to be utilized for Bidder proposed substitutions to the Basis of Design."				
Specification Section 00 04 40 states, "Contractor will be provided a period of 10 days after the date of each Trade Package award for submission of data substantiating a request for a substitution with an "or equal" item."		Specification Section 00 04 40 states, "Contractor will be provided a period of 10 days after the date of each Trade Package award for submission of data substantiating a request for a substitution with an "or equal" item."				
The specification section referenced in RFI Response P1-0117 indicates that substitutions are addressed post-bid.		The specification section referenced in RFI Response P1-0117 indicates that substitutions are addressed post-bid.				
Per the meeting between TJPA, PCPA, Turner, and Webcor-Obayashi on 7/17/14, the response to RFI P1-0117 is to be revised to state that the QBD process is to be used for suggested alternate materials/manufacturers pre-bid.		Per the meeting between TJPA, PCPA, Turner, and Webcor-Obayashi on 7/17/14, the response to RFI P1-0117 is to be revised to state that the QBD process is to be used for suggested alternate materials/manufacturers pre-bid.				
RFI Response P1-0117 has not been revised.		RFI Response P1-0117 has not been revised.				
Please confirm the QBD process is to be used to suggestion of alternate materials/manufacturers.		Please confirm the QBD process is to be used to suggestion of alternate materials/manufacturers.				
<b>P1-0118</b>	<b>Thermoplastic Water Tank Liner Manufacturer</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/01/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: 07 13 55 2.1		Reference: 07 13 55 2.1				
07 13 55 2.1 does not provide a specific manufacturer for thermoplastic water tank liners. Webcor intends to require all manufactures be approved via QBD prior to bid. If this is not acceptable, please provide specific manufacturers and products which are acceptable to use.		07 13 55 2.1 does not provide a specific manufacturer for thermoplastic water tank liners. Webcor intends to require all manufactures be approved via QBD prior to bid. If this is not acceptable, please provide specific manufacturers and products which are acceptable to use.				



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<b>P1-0118.1</b>	<b>Product/Manufacturer for Thermoplastic Water Tank Liners</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: RFI Response P1-0118, Specification Section 00 04 40						
RFI Response P1-0118 states, "The Request for Substitution process established by TJPA and WOJV as outlined in the General Conditions specification section 00 04 40 is the process to be utilized for Bidder proposed substitutions to the Basis of Design."						
Specification Section 00 04 40 states, "Contractor will be provided a period of 10 days after the date of each Trade Package award for submission of data substantiating a request for a substitution with an "or equal" item."						
The specification section referenced in RFI Response P1-0118 indicates that substitutions are addressed post-bid.						
Per the meeting between TJPA, PCPA, Turner, and Webcor-Obayashi on 7/17/14, the response to RFI P1-0118 is to be revised to state that the QBD process is to be used for suggested alternate materials/manufacturers pre-bid.						
RFI Response P1-0118 has not been revised.						
Please confirm the QBD process is to be used to suggestion of alternate materials/manufacturers.						
<b>ANSWER:</b>						
REFERENCE: RFI Response P1-0118, Specification Section 00 04 40						
RFI Response P1-0118 states, "The Request for Substitution process established by TJPA and WOJV as outlined in the General Conditions specification section 00 04 40 is the process to be utilized for Bidder proposed substitutions to the Basis of Design."						
Specification Section 00 04 40 states, "Contractor will be provided a period of 10 days after the date of each Trade Package award for submission of data substantiating a request for a substitution with an "or equal" item."						
The specification section referenced in RFI Response P1-0118 indicates that substitutions are addressed post-bid.						
Per the meeting between TJPA, PCPA, Turner, and Webcor-Obayashi on 7/17/14, the response to RFI P1-0118 is to be revised to state that the QBD process is to be used for suggested alternate materials/manufacturers pre-bid.						
RFI Response P1-0118 has not been revised.						
Please confirm the QBD process is to be used to suggestion of alternate materials/manufacturers.						
<b>P1-0119</b>	<b>GFRC Shown at W-13</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						
Reference: 1/A1-8479						
Is the future GFRC shown in 1/A1-8479 provided as part of phase 1? Will it remain GFRC or be changed to a different material?						
<b>ANSWER:</b>						
Reference: 1/A1-8479						
Is the future GFRC shown in 1/A1-8479 provided as part of phase 1? Will it remain GFRC or be changed to a different material?						



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P1-0120	Light Sculpture and Laminated Glass	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
Reference: A1-9375						Reference: A1-9375
The W-13 glass light sculpture is shown on detail 1/A1-9375. Is this light sculpture to be deleted from the drawings or will it be part of our scope of work?						The W-13 glass light sculpture is shown on detail 1/A1-9375. Is this light sculpture to be deleted from the drawings or will it be part of our scope of work?
Laminated glass is shown on detail 2/A1-9375. Is this laminated glass to be deleted from the drawings or will it be part of our scope of work?						Laminated glass is shown on detail 2/A1-9375. Is this laminated glass to be deleted from the drawings or will it be part of our scope of work?
P1-0121	Mock-up Locations in A Drawings	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
Reference: 08 88 53						Reference: 08 88 53
Provide location in Architectural drawings for extent of mock-ups. Section 1.9.O does not list a valid location for extent. Section 1.9.O shows 084426A references for mock-up extent.						Provide location in Architectural drawings for extent of mock-ups. Section 1.9.O does not list a valid location for extent. Section 1.9.O shows 084426A references for mock-up extent.
P1-0122	Contractor's Proprietary System Language for Glass Floors	Closed	0P	06/11/2014	06/21/2014	07/01/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
Reference: 08 88 53 and 08 44 33 (TG08.2/TG08.10 - Design/Build Glazing Systems dated 5/16/2014)						Reference: 08 88 53 and 08 44 33 (TG08.2/TG08.10 - Design/Build Glazing Systems dated 5/16/2014)
Add design build language similar to that of 08 44 33 Section 1.1.A.25 to allow for contractor's proprietary system.						Add design build language similar to that of 08 44 33 Section 1.1.A.25 to allow for contractor's proprietary system.
P1-0123	Location of Bearing Supports	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:						ANSWER:
Reference: 8/S1-6010, 1 and 2/S1-6020						Reference: 8/S1-6010, 1 and 2/S1-6020
Confirm location of the bearing supports. The table on						Confirm location of the bearing supports. The table on





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	8/S1-6010 and details 1 and 2/S1-6020 do not match. Clarify.					8/S1-6010 and details 1 and 2/S1-6020 do not match. Clarify.
P1-0123.1	Updated Detail for Bearing Plate	Closed	0P	07/03/2014	07/13/2014	07/11/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: Reference: RFI P1-0123, 8/S1-6010, 1, and 2/S1-6020  As stated in RFI Response P1-0123: "The location of the bearing supports is clearly indicated on the drawings. We have updated the details 1 and 2 to add missing information. If there is any specific information missing, please let us know."  Please provide updated details to clearly show the size and dimensions of the bearing plate on 1 & 2/S1-6020.						ANSWER: Reference: RFI P1-0123, 8/S1-6010, 1, and 2/S1-6020  As stated in RFI Response P1-0123: "The location of the bearing supports is clearly indicated on the drawings. We have updated the details 1 and 2 to add missing information. If there is any specific information missing, please let us know."  Please provide updated details to clearly show the size and dimensions of the bearing plate on 1 & 2/S1-6020.
P1-0124	Column Contact Point 7/S1-6010	Closed	0P	06/11/2014	06/11/2014	06/24/2014
From: Webcor Construction LP Andrew Kitchen						
REQUEST: Reference: 7/S1-6010  Detail 7/S1-6010 shows 1 point of contact per column. Confirm the 1 contact point is sufficient to keep the collar framing around the light column from rotating.						ANSWER: Reference: 7/S1-6010  Detail 7/S1-6010 shows 1 point of contact per column. Confirm the 1 contact point is sufficient to keep the collar framing around the light column from rotating.



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<b>P1-0124.1</b>	<b>Column Contact Detail</b>	<b>Closed</b>	<b>0P</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: RFI P1-0124, 7/S1-601  RFI P1-0124 Response: "Two contact points are required at each column in order to keep the collar framing around the light column from rotating. These details will be updated in a future Addendum."  Response to P1-0124 refers to a revised detail to be issued in a future bid package. The detail is required to accurately bid the associated trade package. Please provide the revised detail referenced in the RFI response.		<b>ANSWER:</b>  Reference: RFI P1-0124, 7/S1-601  RFI P1-0124 Response: "Two contact points are required at each column in order to keep the collar framing around the light column from rotating. These details will be updated in a future Addendum."  Response to P1-0124 refers to a revised detail to be issued in a future bid package. The detail is required to accurately bid the associated trade package. Please provide the revised detail referenced in the RFI response.				
<b>P1-0125</b>	<b>W-12 Anchoring</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference: 4 and 6/S1-6034 and 4/A1-8454  Detail 4 and 6/S1-6034 show the W-12 bolting to the box beam while detail 4/A1-8454 show the glass floor attached to the box beam with an angle. Clarify W-12 connection to box beam.		<b>ANSWER:</b>  Reference: 4 and 6/S1-6034 and 4/A1-8454  Detail 4 and 6/S1-6034 show the W-12 bolting to the box beam while detail 4/A1-8454 show the glass floor attached to the box beam with an angle. Clarify W-12 connection to box beam.				
<b>P1-0125.1</b>	<b>W-12 Anchoring Detail</b>	<b>Closed</b>	<b>0P</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: RFI P1-0125, 4 and 6/S1-6034, and 4/A1-8454  RFI P1-0125 Response: "Details 4 and 6 on S1-6034 are correct. Architectural details will be revised in a future package."  Response to P1-0124 refers to a revised detail to be issued in a future bid package. The detail is required to accurately bid the associated trade package. Please provide the revised detail referenced in the RFI response.		<b>ANSWER:</b>  Reference: RFI P1-0125, 4 and 6/S1-6034, and 4/A1-8454  RFI P1-0125 Response: "Details 4 and 6 on S1-6034 are correct. Architectural details will be revised in a future package."  Response to P1-0124 refers to a revised detail to be issued in a future bid package. The detail is required to accurately bid the associated trade package. Please provide the revised detail referenced in the RFI response.				



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<b>P1-0126</b>	<b>Horizontal Life Line Stanchion</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: 1 and 2/WW1-5001 and 4/A1-8860						Reference: 1 and 2/WW1-5001 and 4/A1-8860
Details 1 and 2/WW1-5001 shows typical details for a prefabricated horizontal life line support however Architectural floor plans refer to 4/A1-8860 for typical horizontal life line stanchion detail.						Details 1 and 2/WW1-5001 shows typical details for a prefabricated horizontal life line support however Architectural floor plans refer to 4/A1-8860 for typical horizontal life line stanchion detail.
1. Clarify which stanchion is to be used to support the horizontal life line system						1. Clarify which stanchion is to be used to support the horizontal life line system
2. Clarify the means of connection to the WF beam. Detail 4/A1-8860 shows a bolted connection and 1/WW1-5001 calls for a welded connection.						2. Clarify the means of connection to the WF beam. Detail 4/A1-8860 shows a bolted connection and 1/WW1-5001 calls for a welded connection.
3. Provide waterproofing and flashing details for the stanchion.						3. Provide waterproofing and flashing details for the stanchion.
<b>P1-0126.1</b>	<b>Fall Arrest Stanchions.</b>	<b>Closed</b>	<b>0P</b>	<b>08/05/2014</b>	<b>08/15/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference 2/WW1-5001 and 4/A1-8860						Reference 2/WW1-5001 and 4/A1-8860
Per response to RFI P1-0126 detail 4/A1-8860 is to be used for typical waterproofing. Detail 4 shows the fall arrest stanchion waterproofed on top and both sides with flashing. Should the fall arrest stanchion be completely flashed it may get damaged due to use of lanyard creating an entry point for water intrusion. Please confirm the fall arrest stanchions should have flashing on all sides						Per response to RFI P1-0126 detail 4/A1-8860 is to be used for typical waterproofing. Detail 4 shows the fall arrest stanchion waterproofed on top and both sides with flashing. Should the fall arrest stanchion be completely flashed it may get damaged due to use of lanyard creating an entry point for water intrusion. Please confirm the fall arrest stanchions should have flashing on all sides
<b>P1-0127</b>	<b>Davit Base Plate Connection</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: 5/A1-8860 and 12/S1-7600						Reference: 5/A1-8860 and 12/S1-7600
Detail 5/A1-8860 shows the davit base plate bolted to the WF however 12/S1-7600 shows the davit base plate						Detail 5/A1-8860 shows the davit base plate bolted to the WF however 12/S1-7600 shows the davit base



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	welded to the WF. Please clarify method of attaching the davit base plate to the WF.					
P1-0128	Spec Section 11 24 23	Closed	0P	06/11/2014	06/21/2014	06/24/2014
	From: Webcor Construction LP Andrew Kitchen					
	REQUEST: Reference: 11 24 23  For the referenced Specification Section, what does subsection 3.8 cover that subsection 3.6 does not?					ANSWER: Reference: 11 24 23  For the referenced Specification Section, what does subsection 3.8 cover that subsection 3.6 does not?
P1-0129	Seismic Joint Coordination Between A Drawings	Closed	0P	06/11/2014	06/21/2014	06/24/2014
	From: Webcor Construction LP Andrew Kitchen					
	REQUEST: Reference: A1-8880, A1-2302, A1-7001  Per A1-8880, WJC8 and RJC1 are located between Stair 201 and the adjacent existing building. Per A1-2302 and A1-7001, there does not appear to be a seismic joint at this location. Please revise so that drawings match each other.					ANSWER: Reference: A1-8880, A1-2302, A1-7001  Per A1-8880, WJC8 and RJC1 are located between Stair 201 and the adjacent existing building. Per A1-2302 and A1-7001, there does not appear to be a seismic joint at this location. Please revise so that drawings match each other.



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P1-0130	Carboline Protective Coatings Primer	Closed	0P	06/11/2014	06/21/2014	07/01/2014
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: 07 81 00 3.1.B, A1-8662, A1-8663		Reference: 07 81 00 3.1.B, A1-8662, A1-8663				
Per the attached letter, Skanska will furnish and install Carboline Protective Coatings Primer on all steel to receive Intumescent Fireproofing. Bidders will be directed to assume the steel will be received with this coating, and they will use compatible coatings, or perform all necessary surface prep to use a different system.		Per the attached letter, Skanska will furnish and install Carboline Protective Coatings Primer on all steel to receive Intumescent Fireproofing. Bidders will be directed to assume the steel will be received with this coating, and they will use compatible coatings, or perform all necessary surface prep to use a different system.				
Per 07 81 00 3.1 B fireproofing cannot begin prior to completion of roofing. This will not work with the construction schedule. Please confirm that FP-1 and RFLE do not exist, or provide spec and location (see A1-8662 & A1-8663)		Per 07 81 00 3.1 B fireproofing cannot begin prior to completion of roofing. This will not work with the construction schedule. Please confirm that FP-1 and RFLE do not exist, or provide spec and location (see A1-8662 & A1-8663)				
P1-0131	Fire Blankets Shown at Ceiling Locations	Closed	0P	06/11/2014	06/21/2014	06/30/2014
<b>From:</b> Webcor Construction LP                      Scott Shope						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: A1-8890, 07 09 13		Reference: A1-8890, 07 09 13				
The contract documents show fire blankets within the ceiling space at several locations. Are these part of the rated seismic joint assemblies? The seismic joint assemblies note that the assembly must have a "fire barrier", but does not identify the barrier as a "fire blanket." If the "fire blanket" is the "fire barrier" identified in 07 09 13, please remove/revise the language on the drawings to match the specification. If this is an added material to be furnished and installed separately, please provide a spec., extent, and details for the fire blanket.		The contract documents show fire blankets within the ceiling space at several locations. Are these part of the rated seismic joint assemblies? The seismic joint assemblies note that the assembly must have a "fire barrier", but does not identify the barrier as a "fire blanket." If the "fire blanket" is the "fire barrier" identified in 07 09 13, please remove/revise the language on the drawings to match the specification. If this is an added material to be furnished and installed separately, please provide a spec., extent, and details for the fire blanket.				
This affects TG16.1 Framing/Drywall and/or TG07.8 Expansion Joints. Please provide the requested information so as not to impact the bidding schedule.		This affects TG16.1 Framing/Drywall and/or TG07.8 Expansion Joints. Please provide the requested information so as not to impact the bidding schedule.				



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<b>P1-0132</b>	<b>Installation Requirements for Insulation</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/02/2014</b>
<b>From:</b> Webcor Construction LP                      Scott Shope						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: 09 80 00						Reference: 09 80 00
1.9 A requires that all materials requiring "wetness, moisture or dampness" be completed prior to installation of insulation. This would mean that all drywall finish, thin set tile, and any other "wet applied" finishes be installed prior to acoustic insulation/sealant being installed within the wall cavities, or penetrations. Please rewrite this requirement to follow the manufacturer's written recommendations.						1.9 A requires that all materials requiring "wetness, moisture or dampness" be completed prior to installation of insulation. This would mean that all drywall finish, thin set tile, and any other "wet applied" finishes be installed prior to acoustic insulation/sealant being installed within the wall cavities, or penetrations. Please rewrite this requirement to follow the manufacturer's written recommendations.
<b>P1-0132.1</b>	<b>Language Revision for Material Installation per Specification Section 09 80 00</b>	<b>Closed</b>	<b>0P</b>	<b>07/09/2014</b>	<b>07/19/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: RFI Response P1-0132, Specification Section 09 80 00						REFERENCE: RFI Response P1-0132, Specification Section 09 80 00
As stated in RFI Response P1-0132, "This is a means and methods requirement that the CM/GC shall manage as stated in Division 01. Specification 09 80 00 sets forth the industry standard requirements for installation of the insulation materials an systems. Specification language does not change."						As stated in RFI Response P1-0132, "This is a means and methods requirement that the CM/GC shall manage as stated in Division 01. Specification 09 80 00 sets forth the industry standard requirements for installation of the insulation materials an systems. Specification language does not change."
RFI Response 0132 requires that insulation not be installed unless no further installation of materials requiring wetness, moisture or dampness is contemplated. Drywall mud, thin set for tile, terrazzo, paint and other finish materials create wetness, moisture, and damp condition within the building. Please revise the language to mandate material specified within Specification Section 09 80 00 shall be installed per the manufactuer's written installation instructions.						RFI Response 0132 requires that insulation not be installed unless no further installation of materials requiring wetness, moisture or dampness is contemplated. Drywall mud, thin set for tile, terrazzo, paint and other finish materials create wetness, moisture, and damp condition within the building. Please revise the language to mandate material specified within Specification Section 09 80 00 shall be installed per the manufactuer's written installation instructions.



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<b>P1-0133</b>	<b>Relative Humidity Requirement</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/02/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b> Reference: 09 80 00  1.9 B requires that relative humidity in the area of work does not exceed 55% humidity for the duration of the project. The average relative humidity for San Francisco is above 55% for +80% of the year (and those days below 55% are intermixed between days above 55%). By this standard, we would need to scaffold, shrink wrap, rent air conditioners/dehumidifiers and run them for several years until substantial completion. Please rewrite this requirement to follow the manufacturer's written recommendations.						<b>ANSWER:</b> Reference: 09 80 00  1.9 B requires that relative humidity in the area of work does not exceed 55% humidity for the duration of the project. The average relative humidity for San Francisco is above 55% for +80% of the year (and those days below 55% are intermixed between days above 55%). By this standard, we would need to scaffold, shrink wrap, rent air conditioners/dehumidifiers and run them for several years until substantial completion. Please rewrite this requirement to follow the manufacturer's written recommendations.
<b>P1-0134</b>	<b>Embeds for Superintendent Station</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b> Reference: A1-8181  Per A1-8181, there are embeds within the curbs at the Superintendent Station at the bus deck level. The design of these embeds is not indicated. Please provide the size, location (i.e. continuous, etc.), and method of embedment (1/2" welded studs at 24" o.c.). This will impact TG07.3. Please provide the requested information so as not to impact the bidding schedule.						<b>ANSWER:</b> Reference: A1-8181  Per A1-8181, there are embeds within the curbs at the Superintendent Station at the bus deck level. The design of these embeds is not indicated. Please provide the size, location (i.e. continuous, etc.), and method of embedment (1/2" welded studs at 24" o.c.). This will impact TG07.3. Please provide the requested information so as not to impact the bidding schedule.
<b>P1-0134.1</b>	<b>Required Detailing for Superintendent's Station Concrete Platform</b>	<b>Closed</b>	<b>0P</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b> REFERENCE: RFI Response P1-0134  As stated in RFI Response P1-0134, "The embeds for the Bus Deck Superintendent's Station shown on sheet A1-8181 are not required. Sheet A1-8181 has been omitted in TG08.10. The Bus Deck Superintendent's Station will be a pre-fabricated booth per VE mitigation meetings. Documentation for pre-fabricated booths to be issued in a future package."						<b>ANSWER:</b> REFERENCE: RFI Response P1-0134  As stated in RFI Response P1-0134, "The embeds for the Bus Deck Superintendent's Station shown on sheet A1-8181 are not required. Sheet A1-8181 has been omitted in TG08.10. The Bus Deck Superintendent's Station will be a pre-fabricated booth per VE mitigation meetings. Documentation for pre-fabricated booths to be issued in a future package."



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P1-0135	<b>Below Grade Waterproofing</b>  From: Webcor Construction LP                      Zachary Moore  <b>REQUEST:</b> reference: 07 13 54  Per the contract documents, WPM-3 is to be used at the roof park level below grade. Per 07 13 54, WPM-3 is Sika Sarnafil Waterproofing System PVC G476-20. Per the product data for Sarnafil PVC G476 (see attached), WPM-3 can be exposed for a maximum of 3 months. Based upon this limitation Webcor will be requiring the waterproofing subcontractor to protect the waterproofing until permanent overburden (foam fill, soil, rock, etc.) can be installed. This may lead to substantial added cost to the project. Please confirm that this is acceptable, or revise the contract documents to a product which can be exposed for an extended duration.	Closed	0P	06/11/2014	06/21/2014	06/24/2014
						<b>ANSWER:</b> reference: 07 13 54  Per the contract documents, WPM-3 is to be used at the roof park level below grade. Per 07 13 54, WPM-3 is Sika Sarnafil Waterproofing System PVC G476-20. Per the product data for Sarnafil PVC G476 (see attached), WPM-3 can be exposed for a maximum of 3 months. Based upon this limitation Webcor will be requiring the waterproofing subcontractor to protect the waterproofing until permanent overburden (foam fill, soil, rock, etc.) can be installed. This may lead to substantial added cost to the project. Please confirm that this is acceptable, or revise the contract documents to a product which can be exposed for an extended duration.





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P1-0136	Roofing Spec	Closed	0P	06/11/2014	06/21/2014	07/02/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: 07 13 54 1.8 A 2  Per 07 13 54 1.8 A 2, the roofing subcontractor must be prepared to cover unfinished work with temporary covers in the event of an unexpected rainfall. Per 07 13 54 1.8 B, the roofing subcontractor must leave the building in a completely watertight condition at the end of each day, and make unfinished work watertight. Based upon these requirements, Webcor will be requiring the roofing subcontractor to protect the entire area of the roof until it is completely watertight. This may add substantial added costs to the project. Please confirm this is the desired requirements, or revise the contract documents.		ANSWER: reference: 07 13 54 1.8 A 2  Per 07 13 54 1.8 A 2, the roofing subcontractor must be prepared to cover unfinished work with temporary covers in the event of an unexpected rainfall. Per 07 13 54 1.8 B, the roofing subcontractor must leave the building in a completely watertight condition at the end of each day, and make unfinished work watertight. Based upon these requirements, Webcor will be requiring the roofing subcontractor to protect the entire area of the roof until it is completely watertight. This may add substantial added costs to the project. Please confirm this is the desired requirements, or revise the contract documents.				
P1-0137	Waterproofing Membrane Curing time	Closed	0P	06/11/2014	06/21/2014	07/02/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: REFERENCE: 07 13 00 3.5 A  Spec Section 07 13 00 3.5 A 1 requires the waterproofing membrane to cure a minimum of 7 days at 70 degrees Fahrenheit. In San Francisco, the average daily high temperature does not get up to 70 degrees for 8 months out of the year, and the average low temperature is below 70 degrees year round. As such, acclimatization (scaffold, tenting, and heaters) will be required to maintain the 70 degrees the entire duration of the W/P activity. This may add substantial added costs to the project. Please confirm this is the desired requirements, or revise the contract documents.		ANSWER: REFERENCE: 07 13 00 3.5 A  Spec Section 07 13 00 3.5 A 1 requires the waterproofing membrane to cure a minimum of 7 days at 70 degrees Fahrenheit. In San Francisco, the average daily high temperature does not get up to 70 degrees for 8 months out of the year, and the average low temperature is below 70 degrees year round. As such, acclimatization (scaffold, tenting, and heaters) will be required to maintain the 70 degrees the entire duration of the W/P activity. This may add substantial added costs to the project. Please confirm this is the desired requirements, or revise the contract documents.				
P1-0138	Unfinished work watertight spec	Closed	0P	06/11/2014	06/21/2014	07/02/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: 07 14 13 1.8 B  Spec Section 07 14 13 1.8 B requires "at the end of each		ANSWER: 07 14 13 1.8 B  Spec Section 07 14 13 1.8 B requires "at the end of				



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P1-0139	<b>Unfinished work watertight spec</b>  <b>From:</b> Webcor Construction LP      Zachary Moore  <b>REQUEST:</b> reference: 07 54 19 1.9 A 5  Spec Section 07 54 19 1.9 A 5 requires "at the end of each day, leave the building in a completely watertight condition. Make unfinished work watertight." This requirement will require the tenting of the entire building in order to make the building and unfinished work "watertight." This may add substantial added costs to the project. Please confirm this is the desired requirements, or revise the contract documents.	Closed	0P	06/11/2014	06/21/2014	06/24/2014
	<b>ANSWER:</b> reference: 07 54 19 1.9 A 5  Spec Section 07 54 19 1.9 A 5 requires "at the end of each day, leave the building in a completely watertight condition. Make unfinished work watertight." This requirement will require the tenting of the entire building in order to make the building and unfinished work "watertight." This may add substantial added costs to the project. Please confirm this is the desired requirements, or revise the contract documents.					



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<b>P1-0139.1</b>	<b>Direction For Revision of Specification Section 07 54 19 1.9 A 5</b>	<b>Void</b>	<b>0P</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: RFI Response P1-00139						REFERENCE: RFI Response P1-00139
As stated in RFI Response P1-00139, "The following sentence; "...at the end of each day, leave the building in a completely watertight condition. Make unfinished work watertight." Will be deleted from Spec Section 07 54 19 1.9 A 5. You are correct, construction phase weather protection is a means and methods issue.						As stated in RFI Response P1-00139, "The following sentence; "...at the end of each day, leave the building in a completely watertight condition. Make unfinished work watertight." Will be deleted from Spec Section 07 54 19 1.9 A 5. You are correct, construction phase weather protection is a means and methods issue.
WOJV shall review this changed specification in relation to the Bid Manual and adjust the Bid Manual as required. The Bid Manual previously stated: "It shall be the responsibility of the Trade Subcontractor to take all measures to protect Work from inclement weather, including that determined by WOJV, until completion of the work and acceptance by the Owner." and "In the event that the building requires weather protection and rain water control, Trade Subcontractor shall furnish and install all weather protection as required for Work to continue unabated and will be liable for damage to other Work or costs for weather protection installation due to Trade Subcontractor's failure to timely or properly install weather protection." We also don't believe your text is intended to require each individual trade subcontractor to fully tent the building each day or even on a rainy day."						WOJV shall review this changed specification in relation to the Bid Manual and adjust the Bid Manual as required. The Bid Manual previously stated: "It shall be the responsibility of the Trade Subcontractor to take all measures to protect Work from inclement weather, including that determined by WOJV, until completion of the work and acceptance by the Owner." and "In the event that the building requires weather protection and rain water control, Trade Subcontractor shall furnish and install all weather protection as required for Work to continue unabated and will be liable for damage to other Work or costs for weather protection installation due to Trade Subcontractor's failure to timely or properly install weather protection." We also don't believe your text is intended to require each individual trade subcontractor to fully tent the building each day or even on a rainy day."
RFI Response P1-00139 indicates that Specification Section 07 54 19 1.9 A 5 will be changed. No specification or specific direction to delete the language was provided. Please provide the revised language or direct the language to be stricken for Specification Section 07 54 19 1.9 A 5.						RFI Response P1-00139 indicates that Specification Section 07 54 19 1.9 A 5 will be changed. No specification or specific direction to delete the language was provided. Please provide the revised language or direct the language to be stricken for Specification Section 07 54 19 1.9 A 5.



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<b>P1-0140</b>	<b>Floor Coating Temperature Spec</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/22/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b> reference: 07 18 14 1.8 B  Spec Section 07 18 14 1.8 B indicates ¿ambient and surface temperatures shall be at least 60 degrees F for a minimum period of 48 hours before, during and after coating system application.¿ I would suggest language requiring environmental conditions be established as required by the product manufacturer. Otherwise, this may add substantial added costs to the project. Please confirm this is the desired requirements, or revise the contract documents.						<b>ANSWER:</b> reference: 07 18 14 1.8 B  Spec Section 07 18 14 1.8 B indicates ¿ambient and surface temperatures shall be at least 60 degrees F for a minimum period of 48 hours before, during and after coating system application.¿ I would suggest language requiring environmental conditions be established as required by the product manufacturer. Otherwise, this may add substantial added costs to the project. Please confirm this is the desired requirements, or revise the contract documents.
<b>P1-0141</b>	<b>Coating Requirements</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/02/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> Reference: 07 18 14 3.6 A  Spec Section 07 18 14 3.6 A indicates "Ambient and surface temperatures shall be at least 50 degrees F for a minimum period of 48 hours before, during and after coating system application." This conflicts with 07 18 14 1.8 B. In addition, I would suggest language requiring environmental conditions be established as required by the product manufacturer. This may reduce acclimatization, and conflicts with the manufacturer's installation instructions. Otherwise, this may add substantial added costs to the project. Please confirm this is the desired requirements, or revise the contract documents.						<b>ANSWER:</b> Reference: 07 18 14 3.6 A  Spec Section 07 18 14 3.6 A indicates "Ambient and surface temperatures shall be at least 50 degrees F for a minimum period of 48 hours before, during and after coating system application." This conflicts with 07 18 14 1.8 B. In addition, I would suggest language requiring environmental conditions be established as required by the product manufacturer. This may reduce acclimatization, and conflicts with the manufacturer's installation instructions. Otherwise, this may add substantial added costs to the project. Please confirm this is the desired requirements, or revise the contract documents.
<b>P1-0142</b>	<b>Thermoplastic Water Tank Liners</b>	<b>Void</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> Reference: 07 13 55 2.1  07 13 55 2.1 does not provide a specific manufacturer for thermoplastic water tank liners. Webcor intends to require all manufactures be approved via QBD prior to bid. If this is not acceptable, please provide specific manufacturers						<b>ANSWER:</b> Reference: 07 13 55 2.1  07 13 55 2.1 does not provide a specific manufacturer for thermoplastic water tank liners. Webcor intends to require all manufactures be approved via QBD prior to bid. If this is not acceptable, please provide specific



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	and products which are acceptable to use.					manufacturers and products which are acceptable to use.
<b>P1-0143</b>	<b>TG08.7 Glass Floor Corrosion Expert</b>	<b>Closed</b>	<b>0P</b>	<b>06/12/2014</b>	<b>06/22/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: 08 88 53						Reference: 08 88 53
Specification section 1.9 G.1 requires that the design build contractor engage a California licensed Corrosion Engineer who is an expert in corrosion.						Specification section 1.9 G.1 requires that the design build contractor engage a California licensed Corrosion Engineer who is an expert in corrosion.
It is further required that the design build contractor get a confirmation letter from the corrosion engineer as stated in section 2.11 B and that it must comply with the recommendations of a corrosion engineer approved by TJPA Representative.						It is further required that the design build contractor get a confirmation letter from the corrosion engineer as stated in section 2.11 B and that it must comply with the recommendations of a corrosion engineer approved by TJPA Representative.
These requirements will increase bid substantially and may limit the bidding pool.						These requirements will increase bid substantially and may limit the bidding pool.
Please confirm it is TJPA intent to leave these requirements in thereby increasing costs and potentially limiting the bidding pool.						Please confirm it is TJPA intent to leave these requirements in thereby increasing costs and potentially limiting the bidding pool.
<b>P1-0144</b>	<b>Maximum Pour Height for 10" Blocks</b>	<b>Closed</b>	<b>0P</b>	<b>06/12/2014</b>	<b>06/22/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: 04 20 00 3.5 L 9						Reference: 04 20 00 3.5 L 9
04 20 00 3.5 L 9 Specifies maximum pour height for 8" and 12" blocks, but does not identify one for 10" (CMU Wall Types 9 and 11). Please provide information.						04 20 00 3.5 L 9 Specifies maximum pour height for 8" and 12" blocks, but does not identify one for 10" (CMU Wall Types 9 and 11). Please provide information.
<b>P1-0144.1</b>	<b>Revision to Specification Section 04 20 00 3.5 L 9</b>	<b>Void</b>	<b>0P</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	
<b>From:</b> Webcor Construction LP      Tram Nguyen						





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<b>P1-0147</b>	<b>Concrete Curbs at CMU Walls</b>	<b>Closed</b>	<b>0P</b>	<b>06/13/2014</b>	<b>06/23/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP                      Scott Shope						
<b>REQUEST:</b>  Reference: 4/A1-0022  4/A1-0022 indicates that a "CC" accompanies the CMU wall symbol if a concrete curb is to be erected below the wall. 1/A-0025 indicates that typical CMU walls do not have curbs. 2/A-0025 indicates that if a CMU wall is on a sloped slab, a curb is typically required below the CMU wall. Enlarged floor plans in architectural plans do not typically show a concrete curb below CMU walls (i.e. no "CC" adj. to the symbol). Typical structural details (1 & 2/S1-9001) show concrete curbs at all CMU walls. Are all CMU walls supposed to receive concrete curbs or only the ones identified on architectural plans?						<b>ANSWER:</b>  Reference: 4/A1-0022  4/A1-0022 indicates that a "CC" accompanies the CMU wall symbol if a concrete curb is to be erected below the wall. 1/A-0025 indicates that typical CMU walls do not have curbs. 2/A-0025 indicates that if a CMU wall is on a sloped slab, a curb is typically required below the CMU wall. Enlarged floor plans in architectural plans do not typically show a concrete curb below CMU walls (i.e. no "CC" adj. to the symbol). Typical structural details (1 & 2/S1-9001) show concrete curbs at all CMU walls. Are all CMU walls supposed to receive concrete curbs or only the ones identified on architectural plans?
<b>P1-0148</b>	<b>CMU Rated Walls</b>	<b>Closed</b>	<b>0P</b>	<b>06/13/2014</b>	<b>06/23/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP                      Scott Shope						
<b>REQUEST:</b>  Reference: 04 20 00  Per 04 20 00 2.2 F, preformed rubber is to be used in masonry expansion joints. Per 3 & 4/A-0024, mineral wool, backer rod, and fire/smoke stopping is to be used at rated walls. Per A-0022, all CMU walls are rated. Please revise spec or drawing to agree.						<b>ANSWER:</b>  Reference: 04 20 00  Per 04 20 00 2.2 F, preformed rubber is to be used in masonry expansion joints. Per 3 & 4/A-0024, mineral wool, backer rod, and fire/smoke stopping is to be used at rated walls. Per A-0022, all CMU walls are rated. Please revise spec or drawing to agree.
<b>P1-0148.1</b>	<b>Revision for Specification Section 04 20 00</b>	<b>Void</b>	<b>0P</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0148, Specification Section 04 20 00 2.2 F  RFI Response P1-0148 states, "For fire rated CMU walls, use mineral wool, backer rod and fire / smoke stop, as has been illustrated on detail 3/ A1-0024. Specification section 04 20 00 2.2 F shall be revised to conform and shall be issued with a future ASI.  RFI Response P1-0148 refers to a future ASI to revise						<b>ANSWER:</b>  REFERENCE: RFI Response P1-0148, Specification Section 04 20 00 2.2 F  RFI Response P1-0148 states, "For fire rated CMU walls, use mineral wool, backer rod and fire / smoke stop, as has been illustrated on detail 3/ A1-0024. Specification section 04 20 00 2.2 F shall be revised to conform and shall be issued with a future ASI.  RFI Response P1-0148 refers to a future ASI to revise



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	Specification Section 04 20 00. No revised specification has been received. Please provide the revision for Specification Section 04 20 00.					Specification Section 04 20 00. No revised specification has been received. Please provide the revision for Specification Section 04 20 00.
P1-0149	Train Box Construction Joints	Closed	0P	06/13/2014	06/23/2014	06/24/2014
From: Webcor Construction LP	Scott Shope					
REQUEST:						ANSWER:
Reference: 3 & 4/A-0024						Reference: 3 & 4/A-0024
Per 3 & 4/A-0024 within the train box there are 2 different wall joint details depending if the wall is running north and south, or east and west. The details go on to reference structural for wall control joint information in each direction. Per S1-9000 - S1-9003 (Typical CMU Details) there is one standard detail for all construction joints. Please confirm there is only one construction joint detail desired for walls running in any direction, and coordinate arch. drawings to match.						Per 3 & 4/A-0024 within the train box there are 2 different wall joint details depending if the wall is running north and south, or east and west. The details go on to reference structural for wall control joint information in each direction. Per S1-9000 - S1-9003 (Typical CMU Details) there is one standard detail for all construction joints. Please confirm there is only one construction joint detail desired for walls running in any direction, and coordinate arch. drawings to match.





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<b>P1-0149.1</b>	<b>Widths for Below Grade/Train Box CMU Control Joints</b>	<b>Closed</b>	<b>0P</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI Response P1-0149, Details 3 & 4/A-0024, Details 7 & 10/S1-9000		REFERENCE: RFI Response P1-0149, Details 3 & 4/A-0024, Details 7 & 10/S1-9000				
As stated in RFI Response P1-0149, "This question is vague and cannot be answered. Joint widths vary along the E/W length of the building as well as vertically by level. Contractor to provide information on the joints in question by both gridlines (N/S & E/W) as well as building level (Platform, Lower Concourse etc.)"		As stated in RFI Response P1-0149, "This question is vague and cannot be answered. Joint widths vary along the E/W length of the building as well as vertically by level. Contractor to provide information on the joints in question by both gridlines (N/S & E/W) as well as building level (Platform, Lower Concourse etc.)"				
<ul style="list-style-type: none"><li>- Per Detail 3/A-0024 the train box CMU control joints running in an east to west direction appears to be the width of a standard grout joint.</li><li>- Per Detail 4/A-0024 the train box CMU control joints running in a north to south direction are larger than the width of a standard grout joint.</li><li>- Details 3 &amp; 4/A-0024 indicate that structural drawings are to be referenced for control joint information.</li><li>- Per Details 7 &amp; 10/S1-9000 standard below grade CMU wall control joints are 3/8" regardless of orientation.</li></ul>		<ul style="list-style-type: none"><li>- Per Detail 3/A-0024 the train box CMU control joints running in an east to west direction appears to be the width of a standard grout joint.</li><li>- Per Detail 4/A-0024 the train box CMU control joints running in a north to south direction are larger than the width of a standard grout joint.</li><li>- Details 3 &amp; 4/A-0024 indicate that structural drawings are to be referenced for control joint information.</li><li>- Per Details 7 &amp; 10/S1-9000 standard below grade CMU wall control joints are 3/8" regardless of orientation.</li></ul>				
Please confirm that all below grade/train box CMU wall control joints are to be 3/8" wide as required by structural, rather than differing widths based upon orientation as indicated on the architectural drawings.		Please confirm that all below grade/train box CMU wall control joints are to be 3/8" wide as required by structural, rather than differing widths based upon orientation as indicated on the architectural drawings.				
<b>P1-0149.2</b>	<b>CMU Wall Layout Requirements</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>09/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI Response P1-0149.1		REFERENCE: RFI Response P1-0149.1				
RFI Response P1-0149.1 does not provide specific sizes and layout requirements for CMU walls.		RFI Response P1-0149.1 does not provide specific sizes and layout requirements for CMU walls.				
Please provide specific sizes and layout requirements for CMU walls as Trade Package TG07.4 CMU is not design-build.		Please provide specific sizes and layout requirements for CMU walls as Trade Package TG07.4 CMU is not design-build.				



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<b>P1-0150</b>	<b>CMU Wall Intersections at Loading Docks</b>	<b>Closed</b>	<b>0P</b>	<b>06/13/2014</b>	<b>06/23/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>  Reference: 1 - 4/A1-3192  Per 1 - 4/A1-3192 UL Design 2079 is to be used at CMU wall intersections at the Loading Dock. The UL listed assembly may not work with the opening shown (see attached Hilti listing with UL 2079). Please confirm the fire rated listing to be used at these locations.						<b>ANSWER:</b>  Reference: 1 - 4/A1-3192  Per 1 - 4/A1-3192 UL Design 2079 is to be used at CMU wall intersections at the Loading Dock. The UL listed assembly may not work with the opening shown (see attached Hilti listing with UL 2079). Please confirm the fire rated listing to be used at these locations.
<b>P1-0150.1</b>	<b>Penetration Firestopping Testing Clarification</b>	<b>Closed</b>	<b>0P</b>	<b>07/07/2014</b>	<b>07/17/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0150, Specification Section 07 84 13 (IFC Drawings for Main Package dated 3/31/14)  As stated in RFI Response P1-0150, "For applications, exceeding listed 3 ½ inches joint width, engineering judgment from the manufacturer (in this case Hilti) is required."  Per RFI Response P1-150 an engineering judgement from Hilti will be accepted. There are several other specified manufacturers for penetration fireproofing, 07 84 13 1.7 C requires that penetration firestopping be UL/Intertek/FM Global tested assemblies, and tested by a qualified testing agency acceptable to the AHJ. Please confirm that an engineered judgment for penetration firestopping from any listed manufacturer will be acceptable in lieu of providing a tested assembly.						<b>ANSWER:</b>  REFERENCE: RFI Response P1-0150, Specification Section 07 84 13 (IFC Drawings for Main Package dated 3/31/14)  As stated in RFI Response P1-0150, "For applications, exceeding listed 3 ½ inches joint width, engineering judgment from the manufacturer (in this case Hilti) is required."  Per RFI Response P1-150 an engineering judgement from Hilti will be accepted. There are several other specified manufacturers for penetration fireproofing, 07 84 13 1.7 C requires that penetration firestopping be UL/Intertek/FM Global tested assemblies, and tested by a qualified testing agency acceptable to the AHJ. Please confirm that an engineered judgment for penetration firestopping from any listed manufacturer will be acceptable in lieu of providing a tested assembly.
<b>P1-0151</b>	<b>HD for CMU Wall Types</b>	<b>Closed</b>	<b>0P</b>	<b>06/13/2014</b>	<b>06/23/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference: 1/A1-3100						<b>ANSWER:</b>  Reference: 1/A1-3100



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	<p>1/A1-3100 shows CMU wall types with the symbol "HD". 4/A-0022 does not define what this symbol indicates. A-0015 defines "HD" as "heavy duty" or "hot dipped", neither of which appears to apply. Please provide information on CMU marked as "HD".</p>					<p>1/A1-3100 shows CMU wall types with the symbol "HD". 4/A-0022 does not define what this symbol indicates. A-0015 defines "HD" as "heavy duty" or "hot dipped", neither of which appears to apply. Please provide information on CMU marked as "HD".</p>
<b>P1-0151.1</b>	<b>Information for HD Designated CMU walls</b>	<b>Closed</b>	<b>0P</b>	<b>07/08/2014</b>	<b>07/18/2014</b>	<b>07/22/2014</b>
	<p><b>From:</b> Webcor Construction LP      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: RFI Response P1-0151</p> <p>As stated in RFI Response P1-0151, "Detail 4 on drawing A1-0022 has been updated to reflect that "HD" stands for additional reinforcing. Drawing A1-0022 shall be issued with a future ASI and shall reflect this change."</p> <p>Per RFI Response P1-0151, the designation "HD" stands for "additional reinforcing". The response does not indicate what the additional reinforcement requirements are to be. Please provide the reinforcement requirements.</p>					<p><b>ANSWER:</b></p> <p>REFERENCE: RFI Response P1-0151</p> <p>As stated in RFI Response P1-0151, "Detail 4 on drawing A1-0022 has been updated to reflect that "HD" stands for additional reinforcing. Drawing A1-0022 shall be issued with a future ASI and shall reflect this change."</p> <p>Per RFI Response P1-0151, the designation "HD" stands for "additional reinforcing". The response does not indicate what the additional reinforcement requirements are to be. Please provide the reinforcement requirements.</p>
<b>P1-0152</b>	<b>Concrete Curing</b>	<b>Closed</b>	<b>0P</b>	<b>06/13/2014</b>	<b>06/23/2014</b>	<b>07/11/2014</b>
	<p><b>From:</b> Webcor Construction LP      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference: 04 22 00 3.2.B.1</p> <p>04 22 00 3.2 B 1 Requires that concrete cure for 28 days above 70 degrees Fahrenheit prior to installation of setting materials. This will require heating blankets be placed and maintained for 28 days prior to setting the block, adding cost to the topping slab. Please confirm this requirement is necessary, and the added costs acceptable.</p>					<p><b>ANSWER:</b></p> <p>Reference: 04 22 00 3.2.B.1</p> <p>04 22 00 3.2 B 1 Requires that concrete cure for 28 days above 70 degrees Fahrenheit prior to installation of setting materials. This will require heating blankets be placed and maintained for 28 days prior to setting the block, adding cost to the topping slab. Please confirm this requirement is necessary, and the added costs acceptable.</p>
<b>P1-0153</b>	<b>CMU Drawings Differing from Industry Standard</b>	<b>Closed</b>	<b>0P</b>	<b>06/13/2014</b>	<b>06/23/2014</b>	<b>07/02/2014</b>





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<b>P1-0154.1</b>	<b>Footings Details per Sheets L1-7612 and L1-7613</b>	<b>Closed</b>	<b>0P</b>	<b>07/22/2014</b>	<b>08/01/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI Response P1-0154 A1-2913 through A1-2917 (IFC Drawings for Main Package dated 3/31/14) L1-7612 and L1-7613 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: RFI Response P1-0154 A1-2913 through A1-2917 (IFC Drawings for Main Package dated 3/31/14) L1-7612 and L1-7613 (IFC Drawings for Main Package dated 3/31/14)				
RFI Response P1-0154 references Sheets A1-2913 through A1-2917 for the location of CMU wall foundations.		RFI Response P1-0154 references Sheets A1-2913 through A1-2917 for the location of CMU wall foundations.				
The CMU wall footings depicted on Sheets L1-7612 and L1-7613 (walls adjacent to seismic joints) are not depicted on Sheets A1-2913 through A1-2917.		The CMU wall footings depicted on Sheets L1-7612 and L1-7613 (walls adjacent to seismic joints) are not depicted on Sheets A1-2913 through A1-2917.				
Please provide layout information for the footings shown on Sheets L1-7612 and L1-7613.		Please provide layout information for the footings shown on Sheets L1-7612 and L1-7613.				
<b>P1-0155</b>	<b>Hardware for B1401B</b>	<b>Closed</b>	<b>0P</b>	<b>06/13/2014</b>	<b>06/23/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: A1-9703 and locate door B1401B		Reference: A1-9703 and locate door B1401B				
B1401B is a galvanized steel tube gate and is not assigned a hardware set. Please specify hardware for the gate.		B1401B is a galvanized steel tube gate and is not assigned a hardware set. Please specify hardware for the gate.				
<b>P1-0155.1</b>	<b>Documentation for Changes to Door 01401B</b>	<b>Void</b>	<b>0P</b>	<b>07/08/2014</b>	<b>07/18/2014</b>	
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI Response P1-0155		REFERENCE: RFI Response P1-0155				
As stated in RFI Response P1-0155, "A1-9703 is the Ground Level Door Schedule yet the door referenced uses Concourse Level door nomenclature. We believe the door in question is actually 01401B, if this is the case the gate does not need a hardware set. In a future ASI the following will be documented:		As stated in RFI Response P1-0155, "A1-9703 is the Ground Level Door Schedule yet the door referenced uses Concourse Level door nomenclature. We believe the door in question is actually 01401B, if this is the case the gate does not need a hardware set. In a future ASI the following will be documented:				



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	door number will be deleted from the zone plan (drawing A1-2304) door 01401B will be deleted from the door schedule (drawing A1-9703) Refer to sheet A1-7523 for gate detailing and requirements."					door number will be deleted from the zone plan (drawing A1-2304) door 01401B will be deleted from the door schedule (drawing A1-9703) Refer to sheet A1-7523 for gate detailing and requirements."
	The RFI Response P1-0155 references future documentation. The documentation referenced has not been received. Please provide the documentation referenced in the response to RFI P1-0155.					The RFI Response P1-0155 references future documentation. The documentation referenced has not been received. Please provide the documentation referenced in the response to RFI P1-0155.
<b>P1-0156</b>	<b>Openings around Ground Level Columns</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> reference: A1-2104  Per the architectural slab plans (see below for example) there are openings around the ground level columns with galvanized steel plate supporting the slab above. Structural drawings (see below for example) does not show these openings. Are these openings required? Please revise the documents to conform with each other.						<b>ANSWER:</b> reference: A1-2104  Per the architectural slab plans (see below for example) there are openings around the ground level columns with galvanized steel plate supporting the slab above. Structural drawings (see below for example) does not show these openings. Are these openings required? Please revise the documents to conform with each other.



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P1-0157	Base Plate Detail at Ground Level	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: 2/A1-9311  Per architectural (see below) metal plates on curbs conceal the base plate for columns at transfer girders at ground level. The contract documents do not appear to detail the size or attachment of the metal plate or associated curb. Please provide detailing for the metal plate and associated curb.						ANSWER: reference: 2/A1-9311  Per architectural (see below) metal plates on curbs conceal the base plate for columns at transfer girders at ground level. The contract documents do not appear to detail the size or attachment of the metal plate or associated curb. Please provide detailing for the metal plate and associated curb.
P1-0158	Tubular Lighting	Void	0P	06/11/2014	06/21/2014	
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: 3/S1-2305  Per the contract amendment for TG07.2, the tubular lighting devices have been deleted. However, the details for the tubular daylight devices still appear on the plans (see below for example). Please revise the contract documents to delete the associated detailing.						ANSWER: reference: 3/S1-2305  Per the contract amendment for TG07.2, the tubular lighting devices have been deleted. However, the details for the tubular daylight devices still appear on the plans (see below for example). Please revise the contract documents to delete the associated detailing.
P1-0159	Bus Bridge and Bus Deck	Closed	0P	06/11/2014	06/21/2014	06/24/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: S-2063, S1-2502, A1-2502  Per the Bus Ramp Drawing S-2063, the elevation at T.O. AC is 56.10 (56' - 1-3/16") where the bridge connects to the transit center. Per S1-2502 T.O. Steel at H line (1' - 7" away from the bus bridge connection) is 56' - 4" (2-13/16" higher than the bus ramp AC). Per A1-2502, drive aisle high point along GL-H is 57' - 11-1/4" (1' - 10-1/16" higher than the T.O. Bus Bridge AC). Please review the grades at this location to confirm the bus bridge aligns with the top of bus deck.						ANSWER: reference: S-2063, S1-2502, A1-2502  Per the Bus Ramp Drawing S-2063, the elevation at T.O. AC is 56.10 (56' - 1-3/16") where the bridge connects to the transit center. Per S1-2502 T.O. Steel at H line (1' - 7" away from the bus bridge connection) is 56' - 4" (2-13/16" higher than the bus ramp AC). Per A1-2502, drive aisle high point along GL-H is 57' - 11-1/4" (1' - 10-1/16" higher than the T.O. Bus Bridge AC). Please review the grades at this location to confirm the bus bridge aligns with the top of bus deck.
P1-0159.1	Revision to Detail 2/A1-8378 to Match RFI Responses	Closed	0P	07/08/2014	07/18/2014	07/22/2014
From: Webcor Construction LP Tram Nguyen						



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	<p><b>REQUEST:</b></p> <p>REFERENCE: RFI Response P1-0159, RFI Response B-0008, Detail 2/A1-8378</p> <p>As stated in RFI Response P1-0159, "Per S1-2502, 56'-4" top of steel elevation at GL-H is correct. Per A1-2502, 57'-11 1/4" drive aisle high point along GL-H is correct."</p> <p>Per RFI Response P1-0159, the Bus Deck Drive Aisle high point at the bus bridge is 57'-11 1/4". Per RFI Response B-0008, the top of bus bridge paving is 57'-11 3/4". Per Detail 2/A1-8378, the transition from the Bus Bridge to the Bus Deck Drive Aisle is at 57'-11 1/4". Please revise Detail 2/A1-8378 to reflect the elevations confirmed in RFI Responses P1-0159 and B-0008.</p>					
<b>P1-0160</b>	<b>Structural Slab Detail</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>07/11/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Zachary Moore</p> <p><b>REQUEST:</b></p> <p>reference: S1-2502, A1-2892, A1-9532</p> <p>Per S1-2502 and A1-2892, the structural slab extends 1'-7" south past GL H to meet the bus bridge. Per A1-9532 the bus deck topping slab stops 3'-4" north of GL H. This will leave a 4'-11" area with no topping slab between the bus deck and bus bridge. Please revise the drawing to show the desired condition.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: RFI Response P1-0159, RFI Response B-0008, Detail 2/A1-8378</p> <p>As stated in RFI Response P1-0159, "Per S1-2502, 56'-4" top of steel elevation at GL-H is correct. Per A1-2502, 57'-11 1/4" drive aisle high point along GL-H is correct."</p> <p>Per RFI Response P1-0159, the Bus Deck Drive Aisle high point at the bus bridge is 57'-11 1/4". Per RFI Response B-0008, the top of bus bridge paving is 57'-11 3/4". Per Detail 2/A1-8378, the transition from the Bus Bridge to the Bus Deck Drive Aisle is at 57'-11 1/4". Please revise Detail 2/A1-8378 to reflect the elevations confirmed in RFI Responses P1-0159 and B-0008.</p>					
	<p><b>ANSWER:</b></p> <p>reference: S1-2502, A1-2892, A1-9532</p> <p>Per S1-2502 and A1-2892, the structural slab extends 1'-7" south past GL H to meet the bus bridge. Per A1-9532 the bus deck topping slab stops 3'-4" north of GL H. This will leave a 4'-11" area with no topping slab between the bus deck and bus bridge. Please revise the drawing to show the desired condition.</p>					





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<b>P1-0161</b>	<b>Topping slab at bus deck</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: A1-6102  There does not appear to be a detail depicting the edge of topping slab and associated waterproofing where the bus deck meets the bus bridge. Per A1-6102 the connection between bus bridge is under study. Please provide the details for the connection between the bus bridge and bus deck. This will affect bid documents for TG07.6 Topping Slabs and TG13.2 Waterproofing.						<b>ANSWER:</b> reference: A1-6102  There does not appear to be a detail depicting the edge of topping slab and associated waterproofing where the bus deck meets the bus bridge. Per A1-6102 the connection between bus bridge is under study. Please provide the details for the connection between the bus bridge and bus deck. This will affect bid documents for TG07.6 Topping Slabs and TG13.2 Waterproofing.
<b>P1-0162</b>	<b>Expansion Material</b>	<b>Closed</b>	<b>0P</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> attached: S-6113  Per the bus bridge plans S-6113, a 24 wide piece of expansion material is to be inserted between the bus bridge and bus deck. This does not appear to be a waterproof assembly, and may leak onto the building/people below. Please confirm this is acceptable.						<b>ANSWER:</b> attached: S-6113  Per the bus bridge plans S-6113, a 24 wide piece of expansion material is to be inserted between the bus bridge and bus deck. This does not appear to be a waterproof assembly, and may leak onto the building/people below. Please confirm this is acceptable.
<b>P1-0163</b>	<b>Expansion material Detail</b>	<b>Closed</b>	<b>0P</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>07/01/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: S-6113, S1-5000  Per the bus bridge plans S-6113, there is a 2" wide piece of expansion material sitting on a ledge at the bus deck edge of deck. Per S1-5000, there is no ledge at the typical edge of structural slab detail. Please revise the details to match.						<b>ANSWER:</b> reference: S-6113, S1-5000  Per the bus bridge plans S-6113, there is a 2" wide piece of expansion material sitting on a ledge at the bus deck edge of deck. Per S1-5000, there is no ledge at the typical edge of structural slab detail. Please revise the details to match.
<b>P1-0164</b>	<b>Structural Slab notch</b>	<b>Closed</b>	<b>0P</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						





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<b>P1-0165.1</b>	<b>Rail Requirements for the Path of Travel to the Superintendent Station</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: RFI Response P1-0165, Sheet A1-8168, Sheet A1-8169, CBC Section 1003.5, CBC Section 1010.19						
As stated in RFI Response P1-0165, "Ramp rails information can be found on A1-8168 and A1-8169. However, sheets A1-8168 and A1-8169 have been omitted in TG08.10. Bus Deck Superintendent's Station will be a pre-fabricated booth per VE mitigation meetings. Documentation for pre-fabricated booths to be issued in a future package."						
A1-8168 and A1-8169 show rails for the ramp west of the superintendent station, but omits rails at the 8.1% ramp/curb cut south of the superintendent station and across the drive aisle from it.						
- Per CBC Section 1003.5, slopes along the path of egress which exceed 5% must comply with CBC Section 1010.						
- Per CBC Section 1010.9 requires ramps with a rise greater than 6" to have handrails on both sides.						
- Per CBC Section 1003.5 contrasting finish surfaces in lieu of handrails are only acceptable where the difference in elevation is 6" or less (in this case the vertical travel is approx. 9").						
Please confirm that portions of the path of travel to the Superintendent Station which exceed 5%, and have a vertical travel greater than 6" do not require rails.						
<b>ANSWER:</b>						
REFERENCE: RFI Response P1-0165, Sheet A1-8168, Sheet A1-8169, CBC Section 1003.5, CBC Section 1010.19						
As stated in RFI Response P1-0165, "Ramp rails information can be found on A1-8168 and A1-8169. However, sheets A1-8168 and A1-8169 have been omitted in TG08.10. Bus Deck Superintendent's Station will be a pre-fabricated booth per VE mitigation meetings. Documentation for pre-fabricated booths to be issued in a future package."						
A1-8168 and A1-8169 show rails for the ramp west of the superintendent station, but omits rails at the 8.1% ramp/curb cut south of the superintendent station and across the drive aisle from it.						
- Per CBC Section 1003.5, slopes along the path of egress which exceed 5% must comply with CBC Section 1010.						
- Per CBC Section 1010.9 requires ramps with a rise greater than 6" to have handrails on both sides.						
- Per CBC Section 1003.5 contrasting finish surfaces in lieu of handrails are only acceptable where the difference in elevation is 6" or less (in this case the vertical travel is approx. 9").						
Please confirm that portions of the path of travel to the Superintendent Station which exceed 5%, and have a vertical travel greater than 6" do not require rails.						
<b>P1-0166</b>	<b>Tactile Warning Surfaces at Superintendent Station</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						
Reference: A1-2502						
The accessible path of travel to the Bus Deck Superintendent Station appears to be missing tactile warning surfaces at the ramps						
<b>ANSWER:</b>						
Reference: A1-2502						
The accessible path of travel to the Bus Deck Superintendent Station appears to be missing tactile warning surfaces at the ramps						



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<b>P1-0166.1</b>	<b>Drawings for Concrete Work at Bus Deck Level</b>	<b>Closed</b>	<b>0P</b>	<b>07/08/2014</b>	<b>07/18/2014</b>	<b>08/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI Response P1-0166		REFERENCE: RFI Response P1-0166				
As stated in RFI Response P1-0166, "The Bus Deck Superintendent's Station will be a pre-fabricated booth per VE mitigation meetings. Documentation for pre-fabricated booths to be issued in a future package."		As stated in RFI Response P1-0166, "The Bus Deck Superintendent's Station will be a pre-fabricated booth per VE mitigation meetings. Documentation for pre-fabricated booths to be issued in a future package."				
RFI Response P1-0166 indicates that documentation for pre-fabricated booths will be issued in a future package. The work around the pre-fabricated booths is not included in the same bid package as the pre-fabricated booth. Please provide the information regarding tactile warnings at the ramps adjacent to the pre-fabricated booths.		RFI Response P1-0166 indicates that documentation for pre-fabricated booths will be issued in a future package. The work around the pre-fabricated booths is not included in the same bid package as the pre-fabricated booth. Please provide the information regarding tactile warnings at the ramps adjacent to the pre-fabricated booths.				
<b>P1-0167</b>	<b>Ramps on Architectural Drawings</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: A1-2502, A1-8168		Reference: A1-2502, A1-8168				
Ramps as shown on A1-2502 do not match A1-8168. Please revise the drawings to agree with each other.		Ramps as shown on A1-2502 do not match A1-8168. Please revise the drawings to agree with each other.				
<b>P1-0167.1</b>	<b>Revised Drawings for Concrete Work at Bus Deck Level</b>	<b>Closed</b>	<b>0P</b>	<b>07/08/2014</b>	<b>07/18/2014</b>	<b>08/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: RFI Response P1-0167		REFERENCE: RFI Response P1-0167				
As stated in RFI Response P1-0167, "Sheet A1-8168 has been omitted in TG08.10. Bus Deck Superintendent's Station will be a pre-fabricated booth per VE mitigation meetings. Documentation for pre-fabricated booths to be issued in a future package."		As stated in RFI Response P1-0167, "Sheet A1-8168 has been omitted in TG08.10. Bus Deck Superintendent's Station will be a pre-fabricated booth per VE mitigation meetings. Documentation for pre-fabricated booths to be issued in a future package."				
RFI Response P1-0167 indicates that documentation for pre-fabricated booths will be issued in a future package. The work around the pre-fabricated booths is not included in the same bid package as the pre-fabricated booth. Please provide the information regarding slopes at the		RFI Response P1-0167 indicates that documentation for pre-fabricated booths will be issued in a future package. The work around the pre-fabricated booths is not included in the same bid package as the pre-fabricated booth. Please provide the information				



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	ramps adjacent to the pre-fabricated booths.					regarding slopes at the ramps adjacent to the pre-fabricated booths.
<b>P1-0168</b>	<b>Stem Wall at Bus Deck Superintendent Station</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference:A1-8168  A1-8168 shows concrete stem walls supporting the Bus Deck Superintendent Station and adjacent ramp. There do not appear to be details showing these walls (wall thickness, reinforcement, connection to superstructure, waterproofing, etc.). Please provide details for this area.						<b>ANSWER:</b> Reference:A1-8168  A1-8168 shows concrete stem walls supporting the Bus Deck Superintendent Station and adjacent ramp. There do not appear to be details showing these walls (wall thickness, reinforcement, connection to superstructure, waterproofing, etc.). Please provide details for this area.
<b>P1-0168.1</b>	<b>Revised Stem Wall Drawings for Concrete Work at Bus Deck Level</b>	<b>Closed</b>	<b>0P</b>	<b>07/09/2014</b>	<b>07/19/2014</b>	<b>09/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b> REFERENCE: RFI Response P1-0168  As stated in RFI Response P1-0168, "Sheet A1-8168 has been omitted in TG08.10. Bus Deck Superintendent's Station will be a pre-fabricated booth per VE mitigation meetings. Documentation for pre-fabricated booths to be issued in a future package."  RFI Response P1-0166 indicates that documentation for pre-fabricated booths will be issued in a future package. The work around the pre-fabricated booths is not included in the same bid package as the pre-fabricated booth. Please provide the information regarding structures supporting the pre-fabricated booths.						<b>ANSWER:</b> REFERENCE: RFI Response P1-0168  As stated in RFI Response P1-0168, "Sheet A1-8168 has been omitted in TG08.10. Bus Deck Superintendent's Station will be a pre-fabricated booth per VE mitigation meetings. Documentation for pre-fabricated booths to be issued in a future package."  RFI Response P1-0166 indicates that documentation for pre-fabricated booths will be issued in a future package. The work around the pre-fabricated booths is not included in the same bid package as the pre-fabricated booth. Please provide the information regarding structures supporting the pre-fabricated booths.



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<b>P1-0169</b>	<b>Coating for Metal Surfaces</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: 05 15 21			Reference: 05 15 21			
Per 05 15 21 Steel Castings (see attached), steel castings (most notably the cast nodes) are to be furnished as bare metal. This was confirmed in the construction issues meeting held on 04/17/14 (see attached), with the indication that coating of the bare metal is to be included in TG16.5 Painting. 09 97 16 High Performance Coatings - Superstructure Package (see attached) identifies a coating system for exterior exposed factory-primed metal surfaces, and a coating system for galvanized steel, but not a coating system for exterior exposed unprimed/non-galvanized metal surfaces. Please provide the coating system for all unprimed/non-galvanized metal surfaces.			Per 05 15 21 Steel Castings (see attached), steel castings (most notably the cast nodes) are to be furnished as bare metal. This was confirmed in the construction issues meeting held on 04/17/14 (see attached), with the indication that coating of the bare metal is to be included in TG16.5 Painting. 09 97 16 High Performance Coatings - Superstructure Package (see attached) identifies a coating system for exterior exposed factory-primed metal surfaces, and a coating system for galvanized steel, but not a coating system for exterior exposed unprimed/non-galvanized metal surfaces. Please provide the coating system for all unprimed/non-galvanized metal surfaces.			
Revised: IFC Set 09 97 15 2.5 B calls out for "Shop Primer" in the "Coating System for Exposed Unprimed and Non-Galvanized Steel Surfaces." Based on this information the painter (who in not under contract yet) would need to go to the steel erector's shop to prime the steel (which is currently in production) with the materials called out in 05 10 00 and 05 12 13. Based on the current schedule, the unprimed/non-galvanized steel will be erected before the painter is under contract. In addition, adding priming of these surfaces to the steel scope of work would negatively impact the delivery and erection schedule of steel. Please revise the spec to provide priming of unprimed and non-galvanized metal in the field, or confirm that it is acceptable for steel erection to be delayed.			Revised: IFC Set 09 97 15 2.5 B calls out for "Shop Primer" in the "Coating System for Exposed Unprimed and Non-Galvanized Steel Surfaces." Based on this information the painter (who in not under contract yet) would need to go to the steel erector's shop to prime the steel (which is currently in production) with the materials called out in 05 10 00 and 05 12 13. Based on the current schedule, the unprimed/non-galvanized steel will be erected before the painter is under contract. In addition, adding priming of these surfaces to the steel scope of work would negatively impact the delivery and erection schedule of steel. Please revise the spec to provide priming of unprimed and non-galvanized metal in the field, or confirm that it is acceptable for steel erection to be delayed.			



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<b>P1-0169.1</b>	<b>Clarification for the Preparation of Cast Nodes</b>	<b>Closed</b>	<b>0P</b>	<b>07/08/2014</b>	<b>07/18/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (All Attached) - RFI Response P1-0169 - Specification Section 05 10 00 (IFC Drawings for Main Package dated 3/31/14) - Sheet S1-1023 - Submittal TG0701-99C.1		REFERENCE: (All Attached) - RFI Response P1-0169 - Specification Section 05 10 00 (IFC Drawings for Main Package dated 3/31/14) - Sheet S1-1023 - Submittal TG0701-99C.1				
Per RFI Response P1-0169 (Attached), cast nodes are governed by Specification Section 05 10 00 (Attached), and shall be shop primed.		Per RFI Response P1-0169 (Attached), cast nodes are governed by Specification Section 05 10 00 (Attached), and shall be shop primed.				
- Per Specification Section 05 10 00 3.2 P 1 (See attached Specification Section 05 10 00), "Unless otherwise specified in Section 3.2.P.3, shop prime all structural steel..." - Per Specification Section 05 10 00 3.2 P 3 d (See attached Specification Section 05 10 00), "Surfaces requiring preheat are not to be primed until preheat has been performed." - Per structural drawings (Example Sheet S1-1023 attached) cast nodes are to be field welded to the adjacent tube columns.		- Per Specification Section 05 10 00 3.2 P 1 (See attached Specification Section 05 10 00), "Unless otherwise specified in Section 3.2.P.3, shop prime all structural steel..." - Per Specification Section 05 10 00 3.2 P 3 d (See attached Specification Section 05 10 00), "Surfaces requiring preheat are not to be primed until preheat has been performed." - Per structural drawings (Example Sheet S1-1023 attached) cast nodes are to be field welded to the adjacent tube columns.				
In order to weld the cast nodes to the adjacent tube columns, the cast node must be preheated as confirmed in the reviewed welding procedure from Submittal TG0701-99C.1 (Attached). Since surfaces to be preheated are not to be shop primed, base contract documents explicitly mandate that cast nodes are not to be primed until after welded in the field.		In order to weld the cast nodes to the adjacent tube columns, the cast node must be preheated as confirmed in the reviewed welding procedure from Submittal TG0701-99C.1 (Attached). Since surfaces to be preheated are not to be shop primed, base contract documents explicitly mandate that cast nodes are not to be primed until after welded in the field.				
Please confirm TJPA intends to add shop priming of cast nodes (Note: Adding shop priming of cast nodes will negatively impact the budget and schedule). If the addition of shop priming is not desired, please provide a specification for the field priming of unprimed and non-galvanized metal as requested in RFI P1-0169 (See attached RFI Response P1-0169).		Please confirm TJPA intends to add shop priming of cast nodes (Note: Adding shop priming of cast nodes will negatively impact the budget and schedule). If the addition of shop priming is not desired, please provide a specification for the field priming of unprimed and non-galvanized metal as requested in RFI P1-0169 (See attached RFI Response P1-0169).				
<b>P1-0170</b>	<b>Bin Cart Lift Unit</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						







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<b>P1-0174</b>	<b>Quality Control Spec</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> reference: 11 13 00  1.6.C.2 - During the qualification process TJPA stated they would have this requirement removed since two hours travel time from the building site limits the amount of firms able to qualify and bid on this package. Revise accordingly.						<b>ANSWER:</b> reference: 11 13 00  1.6.C.2 - During the qualification process TJPA stated they would have this requirement removed since two hours travel time from the building site limits the amount of firms able to qualify and bid on this package. Revise accordingly.
<b>P1-0174.1</b>	<b>Max Travel Distance Spec</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Reference: 11 13 00  RFI P1-0174 response: "Regarding the item in this RFI, TJPA stated the specification requirement should not be changed"  1.6.C.2 states "Firm shall be located no more than 2 hours normal travel time from building site" During the qualification process TJPA stated they would have this requirement removed since two hours travel time from the building site limits the amount of firms able to qualify and bid on this package. Revise accordingly.						<b>ANSWER:</b> Reference: 11 13 00  RFI P1-0174 response: "Regarding the item in this RFI, TJPA stated the specification requirement should not be changed"  1.6.C.2 states "Firm shall be located no more than 2 hours normal travel time from building site" During the qualification process TJPA stated they would have this requirement removed since two hours travel time from the building site limits the amount of firms able to qualify and bid on this package. Revise accordingly.
<b>P1-0175</b>	<b>Warranty Defects</b>	<b>Closed</b>	<b>0P</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>07/02/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> reference: 11 13 00 1.8.C - Clarify why the warranty is expected to cover defects in design. Shouldn't this be covered under the design team? This is not a design build scope.						<b>ANSWER:</b> reference: 11 13 00 1.8.C - Clarify why the warranty is expected to cover defects in design. Shouldn't this be covered under the design team? This is not a design build scope.
<b>P1-0176</b>	<b>Recessed dock levelers</b>	<b>Closed</b>	<b>0P</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	reference 11 13 00					reference 11 13 00
	2.2.B.2.a - Clarify how high is "sufficient height to enable lip to extend and clear truck bed before contact."					2.2.B.2.a - Clarify how high is "sufficient height to enable lip to extend and clear truck bed before contact."
<b>P1-0177</b>	<b>Bin Lifters</b>	<b>Closed</b>	<b>0P</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: A1-3100						Reference: A1-3100
Provide details for Bin Lifters.						Provide details for Bin Lifters.
<b>P1-0177.1</b>	<b>Additional Information Bin Lifters</b>	<b>Closed</b>	<b>0P</b>	<b>07/02/2014</b>	<b>07/12/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: RFI P1-0177, A1-3100 RFI Response P1-0177: "The Bin Cart Lift Unit is an equipments that is to be bid. Please refer to the specification section 11 30 00 2.2 G, for model number, capacity and other specifications. The installation and operation of the unit shall be as per manufacturer's standard and manual. See RFI P1-170."						Reference: RFI P1-0177, A1-3100 RFI Response P1-0177: "The Bin Cart Lift Unit is an equipments that is to be bid. Please refer to the specification section 11 30 00 2.2 G, for model number, capacity and other specifications. The installation and operation of the unit shall be as per manufacturer's standard and manual. See RFI P1-170."
Provide details, including embeds and attachments, for Bin Cart Lifter and Bin Cart Lifter Control Box mounting.						Provide details, including embeds and attachments, for Bin Cart Lifter and Bin Cart Lifter Control Box mounting.



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P1-0178	Loading Dock Equipment	Closed	0P	06/10/2014	06/20/2014	06/24/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: 11 13 00  Construction section 2.2.B.7 states "Fabricate dock-leveler frame from structural- and formed-steel shapes" while section 1.6.B states "Provide all assemblies as complete unit produced by a single manufacturer, including necessary accessories, fittings and anchorages." Clarify the intent of the construction section 2.2.B.7 and if the intent is to either fabricate pieces or to have all assemblies provided by a single producer. If the intent is to fabricate pieces provide all details and criteria for fabrication.		ANSWER: reference: 11 13 00  Construction section 2.2.B.7 states "Fabricate dock-leveler frame from structural- and formed-steel shapes" while section 1.6.B states "Provide all assemblies as complete unit produced by a single manufacturer, including necessary accessories, fittings and anchorages." Clarify the intent of the construction section 2.2.B.7 and if the intent is to either fabricate pieces or to have all assemblies provided by a single producer. If the intent is to fabricate pieces provide all details and criteria for fabrication.				
P1-0179	Communication System Details	Closed	0P	06/10/2014	06/20/2014	06/24/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: 11 13 00  2.2.C.6 - Provide details of all warning signs, signal lights, alarms.		ANSWER: 11 13 00  2.2.C.6 - Provide details of all warning signs, signal lights, alarms.				
P1-0180	Dock Lights Detail	Closed	0P	06/10/2014	06/20/2014	06/24/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: 11 13 00  2.2.E - Provide details for Cool Head Incandescent Dock Light - double arm light with min 60 inch reach.		ANSWER: reference: 11 13 00  2.2.E - Provide details for Cool Head Incandescent Dock Light - double arm light with min 60 inch reach.				
P1-0180.1	Dock Light Details	Closed	0P	07/11/2014	07/21/2014	07/21/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: Reference 11 13 00 2.2E, RFI P1-0180, A1-3191  Response to RFI P1-0180 states: Detail 6 on drawing A1-		ANSWER: Reference 11 13 00 2.2E, RFI P1-0180, A1-3191  Response to RFI P1-0180 states: Detail 6 on drawing				



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	<p>3191 will be updated to illustrate the cool head incandescent dock light. This drawing will be issued with a future ASI and will reflect this change.</p> <p>Provide updated detail.</p>					
	<p>A1-3191 will be updated to illustrate the cool head incandescent dock light. This drawing will be issued with a future ASI and will reflect this change.</p> <p>Provide updated detail.</p>					
<b>P1-0181</b>	<b>Preset Anchor Bolts</b>	<b>Closed</b>	<b>0P</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>07/11/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Zachary Moore</p> <p><b>REQUEST:</b></p> <p>reference: 11 13 00</p> <p>3.3.C - States "If preset anchor bolts, cast-in-place inserts, or threaded studs welded to embedded plates or angles are not provided, attach dock bumpers by drilling and anchoring with expansion anchors and bolts." Who would be providing preset anchor bolts? Where are these detailed?</p>					
		</				



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<b>P1-0183</b>	<b>TG08.3 Skylight Fall Protection</b>	<b>Closed</b>	<b>0P</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP Jonathan Flaming						
<b>REQUEST:</b> Reference: WW1-1706 and A1-8401  Do the Skylights at GL 11 and GL 28 have fall protection integral to the frame work? WW1-1706 show safety tie-back around perimeter but A1-8401 does not show fall protection. Clarify extent of fall protection if any to be used at GL 11 and GL 28 Skylights.						<b>ANSWER:</b> Reference: WW1-1706 and A1-8401  Do the Skylights at GL 11 and GL 28 have fall protection integral to the frame work? WW1-1706 show safety tie-back around perimeter but A1-8401 does not show fall protection. Clarify extent of fall protection if any to be used at GL 11 and GL 28 Skylights.
<b>P1-0183.1</b>	<b>TG08.3 Skylight Fall Protection Tie-offs</b>	<b>Closed</b>	<b>0P</b>	<b>07/07/2014</b>	<b>07/17/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference: P1-0183, IFC Main Set WW1-1706 and addendum 11 A1-8401 The response to P1-0183 states that tie-offs are to be integral to the skylight's perimeter rail, and that cleaning of the skylights at grids 11 & 28 can be done from the ground adjacent the skylights. Please confirm that tie-offs are needed if the skylights can be cleaned from the ground. Additionally, Cal OSHA regulations for Personal Fall Arrest Systems states "Anchorages to which personal fall arrest equipment is attached shall be capable of supporting at least 5,000 lbs per employee attached..." and California Building Code states that the rails must be designed to support 200 lbs. If there are to be tie-offs integral to the perimeter rail surrounding skylights at GL 11 and 28, please confirm that the rail is designed to support a point load of 5000 lbs.						<b>ANSWER:</b> Reference: P1-0183, IFC Main Set WW1-1706 and addendum 11 A1-8401 The response to P1-0183 states that tie-offs are to be integral to the skylight's perimeter rail, and that cleaning of the skylights at grids 11 & 28 can be done from the ground adjacent the skylights. Please confirm that tie-offs are needed if the skylights can be cleaned from the ground. Additionally, Cal OSHA regulations for Personal Fall Arrest Systems states "Anchorages to which personal fall arrest equipment is attached shall be capable of supporting at least 5,000 lbs per employee attached..." and California Building Code states that the rails must be designed to support 200 lbs. If there are to be tie-offs integral to the perimeter rail surrounding skylights at GL 11 and 28, please confirm that the rail is designed to support a point load of 5000 lbs.
<b>P1-0183.2</b>	<b>Additional Information TG08.3 Skylight Fall Protection Tie offs</b>	<b>Closed</b>	<b>0P</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference WW1-1703 and WW1-1706  Provide connection details and locations for each the tie-off anchor required for Skylights at GL 11 and 28. Coordinate Window Washing, Architectural and Structural drawings. Are the tie-off anchors around Skylights at GL						<b>ANSWER:</b> Reference WW1-1703 and WW1-1706  Provide connection details and locations for each the tie-off anchor required for Skylights at GL 11 and 28. Coordinate Window Washing, Architectural and Structural drawings. Are the tie-off anchors around



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<div>11 and 28 integral to the skylight or are they stand alone.</div> <div>Skylights at GL 11 and 28 integral to the skylight or are they stand alone.</div>						
<b>P1-0184</b>	<b>TG08.7 Bearing Support for W-13 Glass Floor</b>	<b>Closed</b>	<b>0P</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Jonathan Flaming						
<b>REQUEST:</b> Reference: S1-6010 and S1-6020  Bearing support shown in detail 8/S1-6010 is not the same as that shown in detail 1 and 2/S1-6020. Please clarify bearing support to be used.		<b>ANSWER:</b> Reference: S1-6010 and S1-6020  Bearing support shown in detail 8/S1-6010 is not the same as that shown in detail 1 and 2/S1-6020. Please clarify bearing support to be used.				
<b>P1-0184.1</b>	<b>TG08.7 Bearing Support for W-13 Glass Floor Detail</b>	<b>Closed</b>	<b>0P</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Reference: P1-0184, 8/S1-6010, and 1 and 2/S1-6020  RFI P1-0184 Response: "Detail 8/S1-6010 does not show a bearing support, the detail shows a cross section of the structural steel. The detail has been updated to add more dimension lines to clearly indicate that 8/S1-6010 does indeed match the details 1/S1-6020 and 2/S1/6020. This will be issued in a future Addendum. The bearing itself is described in great detail in the specification section on drawing S1-6020. "  P1-0184.1: Response to P1-0184 refers to a revised detail to be issued in a future bid package. The detail is required to accurately bid the associated trade package. Please provide the revised detail referenced in the RFI response.		<b>ANSWER:</b> Reference: P1-0184, 8/S1-6010, and 1 and 2/S1-6020  RFI P1-0184 Response: "Detail 8/S1-6010 does not show a bearing support, the detail shows a cross section of the structural steel. The detail has been updated to add more dimension lines to clearly indicate that 8/S1-6010 does indeed match the details 1/S1-6020 and 2/S1/6020. This will be issued in a future Addendum. The bearing itself is described in great detail in the specification section on drawing S1-6020. "  P1-0184.1: Response to P1-0184 refers to a revised detail to be issued in a future bid package. The detail is required to accurately bid the associated trade package. Please provide the revised detail referenced in the RFI response.				
<b>P1-0185</b>	<b>TG08.7 Glass Floor Support</b>	<b>Closed</b>	<b>0P</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Jonathan Flaming						



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<b>P1-0188.1</b>	<b>Top Coat and Color for Intumescent Coated Steel</b>	<b>Closed</b>	<b>0P</b>	<b>07/22/2014</b>	<b>08/01/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 07 81 23 2.4 C Specification Section 09 97 15		REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 07 81 23 2.4 C Specification Section 09 97 15				
Specification Section 07 81 23 2.4 C indicates that the top coat is to be an aliphantic urethane as specified in Specification Section 09 97 15.		Specification Section 07 81 23 2.4 C indicates that the top coat is to be an aliphantic urethane as specified in Specification Section 09 97 15.				
Specification Section 09 97 15 does not have a coating system for steel coated in intumescent fireproofing.		Specification Section 09 97 15 does not have a coating system for steel coated in intumescent fireproofing.				
Please provide the requested top coat and color.		Please provide the requested top coat and color.				
<b>P1-0189</b>	<b>Paint Finish Schedule</b>	<b>Closed</b>	<b>0P</b>	<b>06/18/2014</b>	<b>06/28/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>		<b>ANSWER:</b>				
reference: 09 91 00 2.4 A, 2.4 B and A1-9601 - A1-9606		reference: 09 91 00 2.4 A, 2.4 B and A1-9601 - A1-9606				
09 91 00 2.4 A indicates that paint colors are indicated on the finish and material schedule. 09 91 00 2.4 B indicates that the number of colors to be used will be determined by the TJPA Representative. Room finish schedules (A1-9601 - A1-9606), nor the paint finish schedule (09 91 00 3.7) appear to provide the paint colors. There does not appear to be a specification or drawings titled "Finish and Material Schedule". Please provide the paint colors and layout.		09 91 00 2.4 A indicates that paint colors are indicated on the finish and material schedule. 09 91 00 2.4 B indicates that the number of colors to be used will be determined by the TJPA Representative. Room finish schedules (A1-9601 - A1-9606), nor the paint finish schedule (09 91 00 3.7) appear to provide the paint colors. There does not appear to be a specification or drawings titled "Finish and Material Schedule". Please provide the paint colors and layout.				
<b>P1-0190</b>	<b>Intumescent Paint</b>	<b>Closed</b>	<b>0P</b>	<b>06/18/2014</b>	<b>06/28/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>		<b>ANSWER:</b>				
- 09 91 00 3.7 indicates there is an intumescent paint for plywood backing panels. Architectural, electrical, and telecommunication drawings call out for ¾" pressure treated, fire retardant plywood backing. Finish schedules show the interior walls of electrical and low voltage rooms		- 09 91 00 3.7 indicates there is an intumescent paint for plywood backing panels. Architectural, electrical, and telecommunication drawings call out for ¾" pressure treated, fire retardant plywood backing. Finish schedules show the interior walls of electrical				



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P1-0191	<b>Plywood installation</b>  From: Webcor Construction LP Zachary Moore  <b>REQUEST:</b> reference: contract documents and 06 10 53 The contract documents call out for call out for plate steel to be installed over concrete or CMU walls (2/A1-9144 for example). The finish schedule, electrical, and telecom drawings call out for ¾" plywood to be installed over the walls in these locations. 06 10 53 does not provide information on fasteners/ spacing of fasteners, and there do not appear to be details for fastening plywood through steel plated concrete/CMU walls. Please provide information on installation of plywood panels to steel plated conc./CMU walls.	Closed	0P	06/18/2014	06/28/2014	08/05/2014
	<b>ANSWER:</b> reference: contract documents and 06 10 53 The contract documents call out for call out for plate steel to be installed over concrete or CMU walls (2/A1-9144 for example). The finish schedule, electrical, and telecom drawings call out for ¾" plywood to be installed over the walls in these locations. 06 10 53 does not provide information on fasteners/ spacing of fasteners, and there do not appear to be details for fastening plywood through steel plated concrete/CMU walls. Please provide information on installation of plywood panels to steel plated conc./CMU walls.					
P1-0192	<b>Required Mockups</b>  From: Webcor Construction LP Zachary Moore  <b>REQUEST:</b> reference: A1-8164 addendum 11 IFC main set  Indicate on the drawing what is intended to be mocked up now that the redline drawings (issued on May 16, 2014) have deleted the chamber mockups.	Closed	0P	06/18/2014	06/28/2014	08/20/2014
	<b>ANSWER:</b> reference: A1-8164 addendum 11 IFC main set  Indicate on the drawing what is intended to be mocked up now that the redline drawings (issued on May 16, 2014) have deleted the chamber mockups.					



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P1-0193	LEED Requirements Specific to Glazing	Closed	0P	06/18/2014	06/28/2014	07/11/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: 08 71 13  Please confirm that this specification does not have any LEED requirements. Additionally 08 71 13 was specific to TG08.1 Design Build Glazing however that trade package has been deleted, if it is needed for TG08.10 revise title accordingly.						ANSWER: reference: 08 71 13  Please confirm that this specification does not have any LEED requirements. Additionally 08 71 13 was specific to TG08.1 Design Build Glazing however that trade package has been deleted, if it is needed for TG08.10 revise title accordingly.
P1-0194	Glass Type	Closed	0P	06/18/2014	06/28/2014	06/24/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: A/A1-8166  Please clarify the type of glass above the sill as indicated in this detail.						ANSWER: reference: A/A1-8166  Please clarify the type of glass above the sill as indicated in this detail.
P1-0194.1	Glass Type Detail	Closed	0P	07/03/2014	07/13/2014	08/05/2014
From: Webcor Construction LP Zachary Moore						
REQUEST: reference: RFI P1-0194, A/A1-8166  RFI P1-0194 Response: "Revised sheet A1-8166 and Glass Types Specification 08 80 03 to be issued in future TG08.10 ASI."  P1-0194.1: Response to P1-0194 refers to revised details to be issued in a future bid package. The details are required to accurately bid the associated trade package. Please provide the revised details referenced in the RFI response						ANSWER: reference: RFI P1-0194, A/A1-8166  RFI P1-0194 Response: "Revised sheet A1-8166 and Glass Types Specification 08 80 03 to be issued in future TG08.10 ASI."  P1-0194.1: Response to P1-0194 refers to revised details to be issued in a future bid package. The details are required to accurately bid the associated trade package. Please provide the revised details referenced in the RFI response
P1-0195	Glass Type	Closed	0P	06/18/2014	06/28/2014	06/24/2014
From: Webcor Construction LP Zachary Moore						
REQUEST:						ANSWER:



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	reference: C/A1-8166  Please clarify the type of glass above the sill as indicated in this detail.					reference: C/A1-8166  Please clarify the type of glass above the sill as indicated in this detail.
<b>P1-0195.1</b>	<b>Glass Type</b>	<b>Closed</b>	<b>0P</b>	<b>07/07/2014</b>	<b>07/17/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Reference: RFI P1-0195, TG08.2/TG08.10 - Design/Build Glazing Systems addendum #11 C/A1-8166 Response to P1-0195 refers to revised details to be issued in a future bid package. The details are required to accurately bid the associated trade package. Please provide the revised details referenced in the RFI response.						<b>ANSWER:</b> Reference: RFI P1-0195, TG08.2/TG08.10 - Design/Build Glazing Systems addendum #11 C/A1-8166 Response to P1-0195 refers to revised details to be issued in a future bid package. The details are required to accurately bid the associated trade package. Please provide the revised details referenced in the RFI response.
<b>P1-0196</b>	<b>Door Schedule</b>	<b>Closed</b>	<b>0P</b>	<b>06/18/2014</b>	<b>06/28/2014</b>	<b>07/02/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> reference: A1-9711  Please complete the door schedule to include motor sizes to power the overhead coiling doors.						<b>ANSWER:</b> reference: A1-9711  Please complete the door schedule to include motor sizes to power the overhead coiling doors.
<b>P1-0197</b>	<b>W-6 at Superintendents Station</b>	<b>Closed</b>	<b>0P</b>	<b>06/18/2014</b>	<b>06/28/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Reference: 08 44 25 3.10 H Please confirm that the W-6 Sliding Windows At Bus Deck Superintendents Station is no longer required as part of the W-6.						<b>ANSWER:</b> Reference: 08 44 25 3.10 H Please confirm that the W-6 Sliding Windows At Bus Deck Superintendents Station is no longer required as part of the W-6.



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<b>P1-0198</b>	<b>Operator Booths</b>	<b>Closed</b>	<b>0P</b>	<b>06/18/2014</b>	<b>06/28/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b> reference: A1-8275, Detail 6  Please clarify that the two operator booths are no longer part of W-6.		<b>ANSWER:</b>  reference: A1-8275, Detail 6  Please clarify that the two operator booths are no longer part of W-6.				
<b>P1-0199</b>	<b>Details for Pop out framed within W-2</b>	<b>Closed</b>	<b>0P</b>	<b>06/18/2014</b>	<b>06/28/2014</b>	<b>07/01/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b> reference: A1-8174 and detail 1 Please provide details for pop outs framed with the standard W-2 aluminum mullion system.		<b>ANSWER:</b>  reference: A1-8174 and detail 1 Please provide details for pop outs framed with the standard W-2 aluminum mullion system.				
<b>P1-0200</b>	<b>PVC Detail</b>	<b>Closed</b>	<b>0P</b>	<b>06/18/2014</b>	<b>06/28/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>  Reference: 5/A1-8883  Per 5/A1-8883 2 ea. 2" dia. PVC pipes penetrate the train box wall for draining FJC2 at ground level. Per 5/A1-8883, concrete is to be poured tight to the PVC pipe, and no sealant/link seal/waterproofing is shown around the pipes within the wall. This may lead to the PVC pipe breaking at the wall if the concrete shifts, and a point for water intrusion at the train box wall. Please confirm this is the desired detail at this location.		<b>ANSWER:</b>  Reference: 5/A1-8883  Per 5/A1-8883 2 ea. 2" dia. PVC pipes penetrate the train box wall for draining FJC2 at ground level. Per 5/A1-8883, concrete is to be poured tight to the PVC pipe, and no sealant/link seal/waterproofing is shown around the pipes within the wall. This may lead to the PVC pipe breaking at the wall if the concrete shifts, and a point for water intrusion at the train box wall. Please confirm this is the desired detail at this location.				
<b>P1-0201</b>	<b>Bus Crash Rail CIP detail</b>	<b>Closed</b>	<b>0P</b>	<b>06/19/2014</b>	<b>06/29/2014</b>	<b>07/22/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>  It has been indicated that the bus crash rail is to be changed to CIP concrete. The bus crash rails pass across seismic joints within the building. Please provide the seismic joint specification and details for this work.		<b>ANSWER:</b>  It has been indicated that the bus crash rail is to be changed to CIP concrete. The bus crash rails pass across seismic joints within the building. Please provide the seismic joint specification and details for this work.				



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<b>P1-0201.1</b>	<b>Fastening Requirements for 1/8" Thick SS Bent Metal Plates</b>	<b>Closed</b>	<b>0P</b>	<b>07/23/2014</b>	<b>08/02/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sketch SKA-3680 of RFI Response P1-0201		REFERENCE: Sketch SKA-3680 of RFI Response P1-0201				
Per Sketch SKA-3680 provided in RFI Response P1-0201, 1/8" thick SS bent metal plates are to be installed at the Bus Deck Level Crashrail.		Per Sketch SKA-3680 provided in RFI Response P1-0201, 1/8" thick SS bent metal plates are to be installed at the Bus Deck Level Crashrail.				
Design requirements for attaching the SS bent metal plates referenced in SKA-3680 are not provided.		Design requirements for attaching the SS bent metal plates referenced in SKA-3680 are not provided.				
Please provide fastening requirements (anchor size & spacing, or engineering requirements) for 1/8" thick SS bent metal plates fastened on one side to be installed at the bus crash rail.		Please provide fastening requirements (anchor size & spacing, or engineering requirements) for 1/8" thick SS bent metal plates fastened on one side to be installed at the bus crash rail.				
<b>P1-0202</b>	<b>Call outs Marked "FUTURE"</b>	<b>Closed</b>	<b>0P</b>	<b>06/19/2014</b>	<b>06/29/2014</b>	<b>07/02/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Details 1,2,3,4, 5 of sheet A1-7589A. Multiple call outs are marked "(FUTURE)". Please confirm there is no scope of work related to these call-outs for Phase 1 work.		Reference: Details 1,2,3,4, 5 of sheet A1-7589A. Multiple call outs are marked "(FUTURE)". Please confirm there is no scope of work related to these call-outs for Phase 1 work.				
<b>P1-0203</b>	<b>Terrazo marked "FUTURE"</b>	<b>Closed</b>	<b>0P</b>	<b>06/19/2014</b>	<b>06/29/2014</b>	<b>07/02/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Detail 4 of sheet A1-7589A. Detail 4 calls out to "INFILL FRAME WITH TERRAZZO FINISH (FUTURE)". Please confirm that this infill of terrazzo will not be part of phase 1.		Reference: Detail 4 of sheet A1-7589A. Detail 4 calls out to "INFILL FRAME WITH TERRAZZO FINISH (FUTURE)". Please confirm that this infill of terrazzo will not be part of phase 1.				
<b>P1-0204</b>	<b>Future Loading of W-5 Cladding</b>	<b>Closed</b>	<b>0P</b>	<b>06/19/2014</b>	<b>06/29/2014</b>	<b>07/02/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Detail 1 / A1-7844. Detail 1 shows a future W5 Wall Cladding (as part of Phase 2) which will be attached		Reference: Detail 1 / A1-7844. Detail 1 shows a future W5 Wall Cladding (as part of Phase 2) which will be				



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	<p>to a train platform partition (partition type 31 as part of Phase 1). Specification 09 22 19 requires that the metal framing of the partition be designed by the Trade Contractor. Is the trade contractor to include the loading of the future W-5 cladding in his design calculations for the metal framing? If so please provide the loading of the future W-5 cladding to be used in the design calculations.</p>					<p>attached to a train platform partition (partition type 31 as part of Phase 1). Specification 09 22 19 requires that the metal framing of the partition be designed by the Trade Contractor. Is the trade contractor to include the loading of the future W-5 cladding in his design calculations for the metal framing? If so please provide the loading of the future W-5 cladding to be used in the design calculations.</p>
<b>P1-0205</b>	<b>FJC3A spec and Drawing information</b>	<b>Closed</b>	<b>0P</b>	<b>06/19/2014</b>	<b>06/29/2014</b>	<b>07/28/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> <p>Per 1/A1-8894 FJC3A is to be installed at the bus deck level guardrail, and to refer to specification. 07 09 15 indicates that FJC3A is specified in 07 09 13. 07 09 13 does not indicate the manufacturer, type, gutter, or integration into FJC3. There does not appear to be details showing extent of FJC3A, or tie-in to adjacent expansion joints. Please provide omitted specification and drawing information.</p>						<b>ANSWER:</b> <p>Per 1/A1-8894 FJC3A is to be installed at the bus deck level guardrail, and to refer to specification. 07 09 15 indicates that FJC3A is specified in 07 09 13. 07 09 13 does not indicate the manufacturer, type, gutter, or integration into FJC3. There does not appear to be details showing extent of FJC3A, or tie-in to adjacent expansion joints. Please provide omitted specification and drawing information.</p>
<b>P1-0206</b>	<b>Galv. Metal Plate at Fireproofed Beam</b>	<b>Closed</b>	<b>0P</b>	<b>06/20/2014</b>	<b>06/30/2014</b>	<b>07/22/2014</b>
<b>From:</b> Webcor Construction LP Scott Shope						
<b>REQUEST:</b> <p>Reference: 3/A1-8894 and 3/A1-8897</p> <p>Per 3/A1-8894 dated 03/31/14 and 3/A1-8897 dated 03/31/14, galv. metal plate is covering a fireproofed beam is to be furnished and installed at the edge of the bus deck and roof park levels. Structural does not appear to show these plates. Please provide size, location, attachment requirements, and fireproofing details for these plates.</p>						<b>ANSWER:</b> <p>Reference: 3/A1-8894 and 3/A1-8897</p> <p>Per 3/A1-8894 dated 03/31/14 and 3/A1-8897 dated 03/31/14, galv. metal plate is covering a fireproofed beam is to be furnished and installed at the edge of the bus deck and roof park levels. Structural does not appear to show these plates. Please provide size, location, attachment requirements, and fireproofing details for these plates.</p>



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<b>P1-0206.1</b>	<b>Design Requirements for Galvanized Metal Plate per Detail 3/A1-8897</b>	<b>Closed</b>	<b>0P</b>	<b>07/23/2014</b>	<b>08/02/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0206, Detail 3/A1-8897 (IFC Drawings for Main Package dated 3/31/14)  Per RFI Response P1-0206, there is no galv. metal plate noted nor required for the seismic joint assembly at Roof Park Level (3/A1-8897).  Per Detail 3/A1-8897 (see attached) there is a "galv. metal plate covering fireproofed structural beam" which is not detailed within the documents (it is unknown if it is required for a seismic joint assembly).  Please provide the design requirements (size, extent, attachment, fireproofing, etc.) for the galvanized metal plate covering fireproofed structural beam depicted on Detail 3/A1-8897.						<b>ANSWER:</b>  REFERENCE: RFI Response P1-0206, Detail 3/A1-8897 (IFC Drawings for Main Package dated 3/31/14)  Per RFI Response P1-0206, there is no galv. metal plate noted nor required for the seismic joint assembly at Roof Park Level (3/A1-8897).  Per Detail 3/A1-8897 (see attached) there is a "galv. metal plate covering fireproofed structural beam" which is not detailed within the documents (it is unknown if it is required for a seismic joint assembly).  Please provide the design requirements (size, extent, attachment, fireproofing, etc.) for the galvanized metal plate covering fireproofed structural beam depicted on Detail 3/A1-8897.
<b>P1-0207</b>	<b>Bus Crash Rail Leave Out Areas</b>	<b>Closed</b>	<b>0P</b>	<b>06/20/2014</b>	<b>06/30/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>  Reference: SKS-358 (S1-8000)  Per SKS-358 (S1-8000) dated 06/18/14, the continuous bus crash rail is to have #5 rebar stubbed out of the structural slab. Areas of the bus crash rail are to be left out in order to accommodate material landing areas. Please confirm it is acceptable to use form savers at the leave out areas of the bus crash rails.						<b>ANSWER:</b>  Reference: SKS-358 (S1-8000)  Per SKS-358 (S1-8000) dated 06/18/14, the continuous bus crash rail is to have #5 rebar stubbed out of the structural slab. Areas of the bus crash rail are to be left out in order to accommodate material landing areas. Please confirm it is acceptable to use form savers at the leave out areas of the bus crash rails.
<b>P1-0208</b>	<b>Alum. Checker Plate</b>	<b>Closed</b>	<b>0P</b>	<b>06/20/2014</b>	<b>06/30/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Scott Shope						
<b>REQUEST:</b>  Reference: A1-7504, A1-7506, A1-7507  Per the VE, Alternate Second Mitigation issued on 5/16/14 Alum. Checker Plate was removed from the stairwell walls however sheets A1-7504, A1-7506, & A1-7507 still show						<b>ANSWER:</b>  Reference: A1-7504, A1-7506, A1-7507  Per the VE, Alternate Second Mitigation issued on 5/16/14 Alum. Checker Plate was removed from the stairwell walls however sheets A1-7504, A1-7506, &





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	Alum. Checker Plate. Please confirm keep these locations or delete them.					A1-7507 still show Alum. Checker Plate. Please confirm keep these locations or delete them.
P1-0209	Topping Slab Reinforcement	Closed	0P	06/20/2014	06/30/2014	07/22/2014
From: Webcor Construction LP                      Scott Shope						
REQUEST:						ANSWER:
Reference: A1-9520						Reference: A1-9520
Per the topping slab notes (A1-9520 for instance), reinforcement is called out. No details are referenced for special requirements within the slab (penetrations, column leave outs, construction joints, etc.). Please confirm no additional reinforcement (trimmers at openings, keyways at construction joints, expansion joints at walls, etc.) is needed within the topping slabs other than what is required by the topping slab notes.						Per the topping slab notes (A1-9520 for instance), reinforcement is called out. No details are referenced for special requirements within the slab (penetrations, column leave outs, construction joints, etc.). Please confirm no additional reinforcement (trimmers at openings, keyways at construction joints, expansion joints at walls, etc.) is needed within the topping slabs other than what is required by the topping slab notes.
P1-0210	Galvanized Plate Detail	Closed	0P	06/23/2014	07/03/2014	07/22/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:						ANSWER:
reference: A & B/A1-8890						reference: A & B/A1-8890
A & B/A1-8890 note "Extent of Galvanized Plate" along GL 10 and 20 between GL H to GL F, and GL D to GL B. The plans, section, and details of these areas do not appear to show galvanized steel plate in these locations. Please provide plans, sections, and details for the referenced galvanized steel plates, or remove the notes if they are not applicable.						A & B/A1-8890 note "Extent of Galvanized Plate" along GL 10 and 20 between GL H to GL F, and GL D to GL B. The plans, section, and details of these areas do not appear to show galvanized steel plate in these locations. Please provide plans, sections, and details for the referenced galvanized steel plates, or remove the notes if they are not applicable.



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<b>P1-0211</b>	<b>IFRM Layout</b>	<b>Closed</b>	<b>CR</b>	<b>06/23/2014</b>	<b>07/03/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> reference: Per A/A1-8662  Per A/A1-8662, IFRM-2 is located on the tubular portion of the light column, stopping below the cast node. Per E/A1-8662, IFRM-1 is to be applied to the tubular portion of the light column below the cast node, and IFRM-2 is to be applied to the cast node. Please revise the details to match the desired IFRM layout.		<b>ANSWER:</b> reference: Per A/A1-8662  Per A/A1-8662, IFRM-2 is located on the tubular portion of the light column, stopping below the cast node. Per E/A1-8662, IFRM-1 is to be applied to the tubular portion of the light column below the cast node, and IFRM-2 is to be applied to the cast node. Please revise the details to match the desired IFRM layout.				
<b>P1-0212</b>	<b>Hot dipped galvanized Plates</b>	<b>Closed</b>	<b>0P</b>	<b>06/23/2014</b>	<b>07/03/2014</b>	<b>07/22/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Per 1/A1-8894  Per 1/A1-8894 there are hot dipped galvanized plates below the Bus Deck expansion joints (see below), and to refer to structural. Per 1/S1-5005 (see below) these plates are not shown. Please provide the size, dimensions, and attachment for these plates.		<b>ANSWER:</b> Per 1/A1-8894  Per 1/A1-8894 there are hot dipped galvanized plates below the Bus Deck expansion joints (see below), and to refer to structural. Per 1/S1-5005 (see below) these plates are not shown. Please provide the size, dimensions, and attachment for these plates.				
<b>P1-0213</b>	<b>Clarification on Recoating of Damaged Surfaces</b>	<b>Closed</b>	<b>0P</b>	<b>06/25/2014</b>	<b>07/05/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b> REFERENCE: Specification 07 81 23 3.7 B (IFC Drawings for Main Package dated 3/31/14)  Per Specification 07 81 23 3.7 B, the subcontractor is to "repair areas cut-out or damaged as result of testing. Make repair area invisible under normal lighting conditions at the site from a distance of 2 feet." This requirement may exceed industry standard and result in recoating the entire surface in order to make the required repairs.  Please confirm TJPAs intends to have surfaces damaged due to testing entirely recoated to meet the invisibility at 2 feet requirement.		<b>ANSWER:</b> REFERENCE: Specification 07 81 23 3.7 B (IFC Drawings for Main Package dated 3/31/14)  Per Specification 07 81 23 3.7 B, the subcontractor is to "repair areas cut-out or damaged as result of testing. Make repair area invisible under normal lighting conditions at the site from a distance of 2 feet." This requirement may exceed industry standard and result in recoating the entire surface in order to make the required repairs.  Please confirm TJPAs intends to have surfaces damaged due to testing entirely recoated to meet the invisibility at 2 feet requirement.				



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<b>P1-0214</b>	<b>Request for FJC2 Joint Specification at Ground Level</b>	<b>Closed</b>	<b>0P</b>	<b>06/25/2014</b>	<b>07/05/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Drawing A1-8880, Specification 07 09 13 (IFC Drawings for Main Package dated 3/31/14)  Per drawing A1-8880, there are 2 ea. FJC2 expansion joints. One of the FJC2 joints is located at the W-12 exterior glass system (GL 20), and one on the Ground Level adjacent to Beale Street (GL 35). FJC2 as specified in 07 09 13 appears to be for the W-12 system, and 07 09 15 does not appear to specify a seismic joint for FJC2.  Please provide a specification for the FJC2 joint located at the Ground Level.						<b>ANSWER:</b>  REFERENCE: Drawing A1-8880, Specification 07 09 13 (IFC Drawings for Main Package dated 3/31/14)  Per drawing A1-8880, there are 2 ea. FJC2 expansion joints. One of the FJC2 joints is located at the W-12 exterior glass system (GL 20), and one on the Ground Level adjacent to Beale Street (GL 35). FJC2 as specified in 07 09 13 appears to be for the W-12 system, and 07 09 15 does not appear to specify a seismic joint for FJC2.  Please provide a specification for the FJC2 joint located at the Ground Level.
<b>P1-0215</b>	<b>Panel Noise and Vibration</b>	<b>Closed</b>	<b>0P</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference 2014/06/16 W-1 Re-issued for Bid 08 44 27 2.2N Noise and/or vibration due to wind are listed as an unacceptable conditions. There is the possibility that wind will create noise and/or vibration in properly installed panels because of factors such as (but not limited to) perforations and gaps between panels. If provisions remains in the specifications, they may result in additional cost in the bids and/or limit the bidding pool. Confirm that noise and/or vibration caused by wind within properly installed panels will be acceptable.						<b>ANSWER:</b>  Reference 2014/06/16 W-1 Re-issued for Bid 08 44 27 2.2N Noise and/or vibration due to wind are listed as an unacceptable conditions. There is the possibility that wind will create noise and/or vibration in properly installed panels because of factors such as (but not limited to) perforations and gaps between panels. If provisions remains in the specifications, they may result in additional cost in the bids and/or limit the bidding pool. Confirm that noise and/or vibration caused by wind within properly installed panels will be acceptable.
<b>P1-0215.1</b>	<b>Exterior Awning Noise Due to Wind</b>	<b>Void</b>	<b>0P</b>	<b>08/13/2014</b>	<b>08/23/2014</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 08 44 27 Section 2.2N (TG08.2 Issued for Rebid Documents)						<b>ANSWER:</b>  Reference Specification Section 08 44 27 Section 2.2N (TG08.2 Issued for Rebid Documents)



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P1-0216	<p>Please confirm that noise due to wind moving through the patterns cut out in the Aluminum Panels of the Exterior Awning and through the gaps between the panels Exterior Awing and noise due to wind moving above and below the entire system is not to be considered an unacceptable condition. The contractor is providing the pattern per the contract documents and they will have no control over any noise due to wind moving through the predetermined pattern, gaps between each panel and wind moving above and below the entire system. The contractor should only be responsible for noise due to vibration caused by poor workmanship and should not be responsible for any noise due to the inherent design of the W-1 Exterior Awning.</p>	Closed	0P	07/01/2014	07/11/2014	07/11/2014
	<p><b>Touch up Specification</b></p> <p><b>From:</b> Webcor Construction LP                      Zachary Moore</p> <p><b>REQUEST:</b></p> <p>Reference 08 44 27 3.10 F.1.b Specification Section 08 44 27 calls for touchup so that repair is invisible from a distance of 2 ft. According to PDCA P1-04 TOUCH-UP PAINTING AND DAMAGE REPAIR (see attached), Section 2.3 states: "...In order to determine whether a surface has been "properly painted" it shall be examined without magnification at a distance of thirty-nine (39) inches or one (1) meter, or more, under finished lighting conditions and from a normal viewing position."</p> <p>Based on PDCA recommendations, anything less than 39¿ may result an unnecessary increase in cost to the bids. Additionally, the nearest normal viewing position of the final constructed awning will be several meters. Please confirm that the 2 ft. requirement is to remain.</p>					
P1-0217	<p>Please confirm that noise due to wind moving through the patterns cut out in the Aluminum Panels of the Exterior Awning and through the gaps between the panels Exterior Awing and noise due to wind moving above and below the entire system is not to be considered an unacceptable condition. The contractor is providing the pattern per the contract documents and they will have no control over any noise due to wind moving through the predetermined pattern, gaps between each panel and wind moving above and below the entire system. The contractor should only be responsible for noise due to vibration caused by poor workmanship and should not be responsible for any noise due to the inherent design of the W-1 Exterior Awning.</p>	Closed	0P	07/02/2014	07/12/2014	07/22/2014
	<p><b>Multiple Seismic Joint Assembly Manufacturers per Specification Section 07 09 13 Closed</b></p> <p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p>					
	<p><b>ANSWER:</b></p> <p>Reference 08 44 27 3.10 F.1.b Specification Section 08 44 27 calls for touchup so that repair is invisible from a distance of 2 ft. According to PDCA P1-04 TOUCH-UP PAINTING AND DAMAGE REPAIR (see attached), Section 2.3 states: "...In order to determine whether a surface has been "properly painted" it shall be examined without magnification at a distance of thirty-nine (39) inches or one (1) meter, or more, under finished lighting conditions and from a normal viewing position."</p> <p>Based on PDCA recommendations, anything less than 39¿ may result an unnecessary increase in cost to the bids. Additionally, the nearest normal viewing position of the final constructed awning will be several meters. Please confirm that the 2 ft. requirement is to remain.</p>					
	<p><b>ANSWER:</b></p>					



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	<p>REFERENCE: Specification Section 07 09 13 2.2 A 1 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Per Specification Section 07 09 13 2.2 A 1, Construction Specialties, Inc. is the basis of design for the seismic joint assemblies. As such, the details associated with the seismic joints which were bid out to Shimmick and Skanska were based upon Construction Specialties, Inc. Specification 07 09 13 2.2 A goes on to list another 3 manufacturers which can be used. Since each manufacturer may require different block-outs, embeds, attachment to structure, etc., there could be some added unforeseen costs if a bidder uses a product which is not the basis of design.</p> <p>Would TJPA like us to direct the bidders to cover any costs associated with any changes required due to the use of a product which is not the basis of design, or would TJPA like to carry the risk/costs associated with this?</p>					
P1-0218	Inconsistent Specification and Drawing References for Joint Sizes and Movement	Closed				
From: Webcor Construction LP		Tram Nguyen				
REQUEST:						
REFERENCE: Specification Section 07 09 13 2.4 B, Detail 2/A-8880, Detail 1/A1-8880 (IFC Drawings for Main Package dated 3/31/14)						
Per Specification Section 07 09 13 2.4 B, there is a chart indicating joint sizes and movement requirements on Detail 2/A-8880. The chart is on Detail 1/A1-8880. Please revise the specification section to agree with the drawing.						
	<p>REFERENCE: Specification Section 07 09 13 2.2 A 1 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Per Specification Section 07 09 13 2.2 A 1, Construction Specialties, Inc. is the basis of design for the seismic joint assemblies. As such, the details associated with the seismic joints which were bid out to Shimmick and Skanska were based upon Construction Specialties, Inc. Specification 07 09 13 2.2 A goes on to list another 3 manufacturers which can be used. Since each manufacturer may require different block-outs, embeds, attachment to structure, etc., there could be some added unforeseen costs if a bidder uses a product which is not the basis of design.</p> <p>Would TJPA like us to direct the bidders to cover any costs associated with any changes required due to the use of a product which is not the basis of design, or would TJPA like to carry the risk/costs associated with this?</p>					
0P				07/02/2014	07/12/2014	07/11/2014
ANSWER:						
REFERENCE: Specification Section 07 09 13 2.4 B, Detail 2/A-8880, Detail 1/A1-8880 (IFC Drawings for Main Package dated 3/31/14)						
Per Specification Section 07 09 13 2.4 B, there is a chart indicating joint sizes and movement requirements on Detail 2/A-8880. The chart is on Detail 1/A1-8880. Please revise the specification section to agree with the drawing.						



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<b>P1-0219</b>	<b>Seismic Joint Type FJC1 Within the Roof Park Landscaping/Paving</b>	<b>Closed</b>	<b>0P</b>	<b>07/02/2014</b>	<b>07/12/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Sheet A1-8880 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Sheet A1-8880 (IFC Drawings for Main Package dated 3/31/14)
Per Sheet A1-8880, Seismic Joint Type FJC1 is located within the roof park landscape/paving. Is Seismic Joint Type FJC1 to be deferred with the rest of the roof park?						Per Sheet A1-8880, Seismic Joint Type FJC1 is located within the roof park landscape/paving. Is Seismic Joint Type FJC1 to be deferred with the rest of the roof park?
<b>P1-0220</b>	<b>Park Deferral Waterproofing</b>	<b>Closed</b>	<b>0P</b>	<b>07/02/2014</b>	<b>07/02/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: Addendum #11 Dated 05-16-14 A1-8404 detail 1						Reference: Addendum #11 Dated 05-16-14 A1-8404 detail 1
Due to the deferral of the roof park landscaping items that would have been covered, the waterproofing and drainage gutter assembly for the W-10 Skylight, be exposed to UV and weather for a prolonged period of time. Please provide any provisions necessary to mitigate this changed condition.						Due to the deferral of the roof park landscaping items that would have been covered, the waterproofing and drainage gutter assembly for the W-10 Skylight, be exposed to UV and weather for a prolonged period of time. Please provide any provisions necessary to mitigate this changed condition.
<b>P1-0221</b>	<b>Inconsistent Drawing and Specification Titles for Expansion/Seismic Joints</b>	<b>Closed</b>	<b>0P</b>	<b>07/02/2014</b>	<b>07/02/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Sheet A1-8880 , Specification Section 07 09 13 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Sheet A1-8880 , Specification Section 07 09 13 (IFC Drawings for Main Package dated 3/31/14)
Sheet A1-8880 is titled "Expansion Joint Schedule". Specification Section 07 09 13 is titled "Seismic Joint Assemblies". Please change one of the titles to match the other.						Sheet A1-8880 is titled "Expansion Joint Schedule". Specification Section 07 09 13 is titled "Seismic Joint Assemblies". Please change one of the titles to match the other.
<b>P1-0222</b>	<b>Revision to Adjoining Material for Joint Types CJC1 and CJC2</b>	<b>Closed</b>	<b>0P</b>	<b>07/02/2014</b>	<b>07/12/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Sheet A1-8880 (IFC Drawings for Main						REFERENCE: Sheet A1-8880 (IFC Drawings for Main



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	<p>Package dated 3/31/14)</p> <p>Per Sheet A1-8880, the adjoining material for Joint Type CJC1 and Joint Type CJC3 is GFRC. Please revise the table to reflect the actual adjacent material to be used.</p>					<p>Package dated 3/31/14)</p> <p>Per Sheet A1-8880, the adjoining material for Joint Type CJC1 and Joint Type CJC3 is GFRC. Please revise the table to reflect the actual adjacent material to be used.</p>
<b>P1-0223</b>	<b>Finish Clarification for Joint Type FJC6</b>  <b>From:</b> Webcor Construction LP      Tram Nguyen	<b>Closed</b>	<b>0P</b>	<b>07/02/2014</b>	<b>07/12/2014</b>	<b>07/11/2014</b>
	<b>REQUEST:</b>  REFERENCE: Sheet A1-8880 , Specification Section 07 09 13 3.4 G (IFC Drawings for Main Package dated 3/31/14)  Per Sheet A1-8880, the finish for Joint Type FJC6 and adjacent materials are to be selected by Tennant, while Specification Section 07 09 13 3.4 G indicates it is to be recessed to receive terrazzo. Please provide the desired finish.					<b>ANSWER:</b>  REFERENCE: Sheet A1-8880 , Specification Section 07 09 13 3.4 G (IFC Drawings for Main Package dated 3/31/14)  Per Sheet A1-8880, the finish for Joint Type FJC6 and adjacent materials are to be selected by Tennant, while Specification Section 07 09 13 3.4 G indicates it is to be recessed to receive terrazzo. Please provide the desired finish.
<b>P1-0224</b>	<b>Absence of Joint Type FJC7 at Bus Ramp Per Sheet A1-8880</b>  <b>From:</b> Webcor Construction LP      Tram Nguyen	<b>Closed</b>	<b>0P</b>	<b>07/02/2014</b>	<b>07/12/2014</b>	<b>07/11/2014</b>
	<b>REQUEST:</b>  REFERENCE: Sheet A1-8880 (IFC Drawings for Main Package dated 3/31/14)  Per Sheet A1-8880, Joint Type FJC7 is located where the bus ramp abuts the building. Sheet A1-8880 does not appear to show an expansion joint at this location. Please revise Sheet A1-8880 to show the location of Joint Type FJC7 or provide details on the location of Joint Type FJC7.					<b>ANSWER:</b>  REFERENCE: Sheet A1-8880 (IFC Drawings for Main Package dated 3/31/14)  Per Sheet A1-8880, Joint Type FJC7 is located where the bus ramp abuts the building. Sheet A1-8880 does not appear to show an expansion joint at this location. Please revise Sheet A1-8880 to show the location of Joint Type FJC7 or provide details on the location of Joint Type FJC7.









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P1-0228	FJC8. Sheet A1-8880 does not show that there is a Joint Type FJC8. Please provide the location and information for Joint Type FJC8 on Sheet A1-8880, or delete from the specification section.	Closed	0P	07/02/2014	07/12/2014	07/11/2014
	Specification Section 07 09 13 3.4 I calls out Joint Type FJC8. Sheet A1-8880 does not show that there is a Joint Type FJC8. Please provide the location and information for Joint Type FJC8 on Sheet A1-8880, or delete from the specification section.					
P1-0228	<b>Specification for Joint Type RJC1 per Sheet A1-8880</b>	Closed	0P	07/02/2014	07/12/2014	07/11/2014
	<b>From:</b> Webcor Construction LP      Tram Nguyen					
P1-0229	<b>REQUEST:</b> REFERENCE: Sheet A1-8880, Specification 07 09 13 (IFC Drawings for Main Package dated 3/31/14)  Sheet A1-8880 calls out Joint Type RJC1. Specification Section 07 09 13 does not appear to specify this seismic joint. Please provide the specifications for this product.	Closed	0P	07/02/2014	07/12/2014	07/11/2014
	<b>ANSWER:</b> REFERENCE: Sheet A1-8880, Specification 07 09 13 (IFC Drawings for Main Package dated 3/31/14)  Sheet A1-8880 calls out Joint Type RJC1. Specification Section 07 09 13 does not appear to specify this seismic joint. Please provide the specifications for this product.					
P1-0229	<b>Adjacent Material for Joint Type WJC7</b>	Closed	0P	07/02/2014	07/12/2014	07/11/2014
	<b>From:</b> Webcor Construction LP      Tram Nguyen					
P1-0229	<b>REQUEST:</b> REFERENCE: Sheet A1-8880, Detail 3/A1-8897 (IFC Drawings for Main Package dated 3/31/14)  Per Sheet A1-8880, Joint Type WJC7 is within the park level exterior fascia, and bus deck exterior fascia. Sheet A1-8880 calls for concrete and metal panel as the adjacent materials. Detail 3/A1-8897, calls for W-18 as the adjacent material. Please confirm these are the correct adjacent materials, or revise drawings and schedule.	Closed	0P	07/02/2014	07/12/2014	07/11/2014
	<b>ANSWER:</b> REFERENCE: Sheet A1-8880, Detail 3/A1-8897 (IFC Drawings for Main Package dated 3/31/14)  Per Sheet A1-8880, Joint Type WJC7 is within the park level exterior fascia, and bus deck exterior fascia. Sheet A1-8880 calls for concrete and metal panel as the adjacent materials. Detail 3/A1-8897, calls for W-18 as the adjacent material. Please confirm these are the correct adjacent materials, or revise drawings and schedule.					



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<b>P1-0230</b>	<b>Information for Specification Section 03 30 03 per Sheet A1-9522</b>	<b>Closed</b>	<b>0P</b>	<b>07/02/2014</b>	<b>07/12/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: Sheet A1-9522 (Attached), Specification Section 03 30 03 (IFC Drawings for Main Package dated 3/31/14)						
Per the notes on Sheet A1-9522, the concrete topping slab in the following locations are to be provided with the required thickness and as described by the structural engineer in Specification Section 03 30 03:						
<ul style="list-style-type: none"><li>- Train Platform &amp; Lower Concourse Level</li><li>- Vehicle and Bicycle Ramp</li><li>- Bus Deck Level Topping &amp; Curbs</li><li>- All Other Concrete Toppings</li></ul>						
No documentation is provided for Specification Section 03 30 03. Please provide the specifications for section 03 30 03.						
<b>ANSWER:</b>						
REFERENCE: Sheet A1-9522 (Attached), Specification Section 03 30 03 (IFC Drawings for Main Package dated 3/31/14)						
Per the notes on Sheet A1-9522, the concrete topping slab in the following locations are to be provided with the required thickness and as described by the structural engineer in Specification Section 03 30 03:						
<ul style="list-style-type: none"><li>- Train Platform &amp; Lower Concourse Level</li><li>- Vehicle and Bicycle Ramp</li><li>- Bus Deck Level Topping &amp; Curbs</li><li>- All Other Concrete Toppings</li></ul>						
No documentation is provided for Specification Section 03 30 03. Please provide the specifications for section 03 30 03.						
<b>P1-0230.1</b>	<b>Details and Specifications for Concrete Sloping to Zero Thickness</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>07/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: RFI Response P1-0230						
Per RFI Response P1-0230, topping slab concrete is to be as specified in Specification Section 03 30 02. Per Specification Section 03 30 02, all concrete mixes, excluding the concrete fill at basket columns, have a 3/4" aggregate.						
In several locations (Train Platform Level GL 2 - 4 and GL A - F for example) show the topping slabs sloping to zero thickness. This condition will not allow the use of aggregate or reinforcing as noted on the revised sheets in the response to RFI Response P1-0230.						
Please provide details and specifications for sloping of concrete to zero thickness.						
<b>ANSWER:</b>						
REFERENCE: RFI Response P1-0230						
Per RFI Response P1-0230, topping slab concrete is to be as specified in Specification Section 03 30 02. Per Specification Section 03 30 02, all concrete mixes, excluding the concrete fill at basket columns, have a 3/4" aggregate.						
In several locations (Train Platform Level GL 2 - 4 and GL A - F for example) show the topping slabs sloping to zero thickness. This condition will not allow the use of aggregate or reinforcing as noted on the revised sheets in the response to RFI Response P1-0230.						
Please provide details and specifications for sloping of concrete to zero thickness.						
<b>P1-0230.2</b>	<b>Concrete Ramp Details for Structural Slabs Poured per Slab Plans</b>	<b>Closed</b>	<b>0P</b>	<b>07/22/2014</b>	<b>08/01/2014</b>	<b>08/05/2014</b>



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P1-0230.3	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div></div> <div>REQUEST:  REFERENCE: RFI Response P1-0230.1, Detail 5/S1-5003 (IFC Drawings for Main Package dated 3/31/14)  RFI Response P1-0230.1 references Detail 5/S1-5003 for the installation of small ramps. Per Detail 5/S1-5003, there is a minimum slab thickness of 2" at the ramp, to saw cut the concrete at the nose of the ramp, and to roughen the surface of the structural slab to receive the ramp.  Architectural slab edge plans do not show depressions within the structural slabs where these ramps exist (topping slab ramps or housekeeping pad/mechanical pad ramps). As such, a 2"x40" swath of concrete would need to be removed where concrete has been poured per the slab plans (note - mat slab rebar is 1.5" clr. from the t.o. slab, 1 way slab and composite deck rebar is 3/4" clr. from the t.o. slab).  Please provide revised concrete ramp details where structural slabs have been poured per the slab plans.</div>	Closed	0P	08/14/2014	08/24/2014	08/27/2014
	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div></div> <div>REQUEST:  REFERENCE: Detail 5/S1-5003 (IFC Drawings for Main Package dated 3/31/14)  Detail 5/S1-5003 requires a minimum 2" thickness of concrete for the ramp. In order to maintain a minimum 2" thickness, a depression in the slab will need to be provided at the ramps. Depressions are shown on the slab edge plans.  Train platform, lower concourse decks, and associated reinforcement are part of TG06, not TG07.2.  Please revise the plans to show depressions and fixes for rebar at the 2" depressions, or provide a different material to form the ramps which can be tapered to the thickness required (for instance Ardex).</div>					
P1-0230.3	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div></div> <div>REQUEST:  REFERENCE: RFI Response P1-0230.1, Detail 5/S1-5003 (IFC Drawings for Main Package dated 3/31/14)  RFI Response P1-0230.1 references Detail 5/S1-5003 for the installation of small ramps. Per Detail 5/S1-5003, there is a minimum slab thickness of 2" at the ramp, to saw cut the concrete at the nose of the ramp, and to roughen the surface of the structural slab to receive the ramp.  Architectural slab edge plans do not show depressions within the structural slabs where these ramps exist (topping slab ramps or housekeeping pad/mechanical pad ramps). As such, a 2"x40" swath of concrete would need to be removed where concrete has been poured per the slab plans (note - mat slab rebar is 1.5" clr. from the t.o. slab, 1 way slab and composite deck rebar is 3/4" clr. from the t.o. slab).  Please provide revised concrete ramp details where structural slabs have been poured per the slab plans.</div>	Closed	0P	08/14/2014	08/24/2014	08/27/2014
	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div></div> <div>REQUEST:  REFERENCE: Detail 5/S1-5003 (IFC Drawings for Main Package dated 3/31/14)  Detail 5/S1-5003 requires a minimum 2" thickness of concrete for the ramp. In order to maintain a minimum 2" thickness, a depression in the slab will need to be provided at the ramps. Depressions are shown on the slab edge plans.  Train platform, lower concourse decks, and associated reinforcement are part of TG06, not TG07.2.  Please revise the plans to show depressions and fixes for rebar at the 2" depressions, or provide a different material to form the ramps which can be tapered to the thickness required (for instance Ardex).</div>					



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	<p>Alternatively, all ramps, with exception to one, can be removed at the ADA path of travel and a painted step can create a warning stripe.</p>					<p>Alternatively, all ramps, with exception to one, can be removed at the ADA path of travel and a painted step can create a warning stripe.</p>
P1-0231	Details for Embedded Plate per Detail 2/A1-9228	Closed	0P	07/08/2014	07/18/2014	07/22/2014
	<p>From: Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: RFI P1-0063 Response, Detail 2/A1-9228, Sheet SKA-3667 (IFC Drawings for Main Package dated 3/31/2014)</p> <p>Per RFI P1-0063 response, "Detail 2/A1-9228 has been updated. Detail reference 4/A-0026 SIM has been removed. Refer to the attached SKA-3667."</p> <p>The revised detail shows an embedded plate at the B.O. Lower Concourse Deck. No information is provided for the embeded plate. Please provide information on embedded plate.</p>					<p><b>ANSWER:</b></p> <p>REFERENCE: RFI P1-0063 Response, Detail 2/A1-9228, Sheet SKA-3667 (IFC Drawings for Main Package dated 3/31/2014)</p> <p>Per RFI P1-0063 response, "Detail 2/A1-9228 has been updated. Detail reference 4/A-0026 SIM has been removed. Refer to the attached SKA-3667."</p> <p>The revised detail shows an embedded plate at the B.O. Lower Concourse Deck. No information is provided for the embeded plate. Please provide information on embedded plate.</p>
P1-0232	Details for Expansion Joint Located at the Bus Bridge, Bus Deck, and Building Co	Closed	0P	07/11/2014	07/21/2014	08/05/2014
	<p>From: Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Details 2 &amp; 3/A1-8378 (IFC Drawings for Main Package dated 3/31/14), Detail 4/SKA-3609 (ASI 119 dated 6/18/14)</p> <p>Details 2 &amp; 3/ A1-8378 depict an expansion joint to be placed where the bus bridge conjoins with the bus deck. The same expansion joint is not shown in Detail 4/SKA-3609 at the same location.</p> <p>Please provide details and specifications showing how the expansion joint interacts with the bus deck, buildling, and both sides of the new concrete bus crash rail.</p>					<p><b>ANSWER:</b></p> <p>REFERENCE: Details 2 &amp; 3/A1-8378 (IFC Drawings for Main Package dated 3/31/14), Detail 4/SKA-3609 (ASI 119 dated 6/18/14)</p> <p>Details 2 &amp; 3/ A1-8378 depict an expansion joint to be placed where the bus bridge conjoins with the bus deck. The same expansion joint is not shown in Detail 4/SKA-3609 at the same location.</p> <p>Please provide details and specifications showing how the expansion joint interacts with the bus deck, building, and both sides of the new concrete bus crash rail.</p>



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P1-0233	ASI Precedence	Closed	0P	07/11/2014	07/11/2014	08/05/2014
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b> reference: A1-3100 (SKA-3670) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.		<b>ANSWER:</b> reference: A1-3100 (SKA-3670) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.				
P1-0234	Maximum Tree Weight	Closed	0P	07/11/2014	07/21/2014	07/22/2014
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b> Reference: Landscape General L-0006 and L-0007  While sizing the maximum load for the rooftop cranes it was communicated by PWP and TT that the maximum tree size will be no greater than 25,000 lbs. How is the design team going to ensure that no trees shall be greater than 25,000 lbs when they arrive on site? Provide appropriate limits in the specifications and drawings.		<b>ANSWER:</b> Reference: Landscape General L-0006 and L-0007  While sizing the maximum load for the rooftop cranes it was communicated by PWP and TT that the maximum tree size will be no greater than 25,000 lbs. How is the design team going to ensure that no trees shall be greater than 25,000 lbs when they arrive on site? Provide appropriate limits in the specifications and drawings.				
P1-0235	Tree Growth Weight	Closed	0P	07/11/2014	07/21/2014	07/22/2014
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b> Reference: L-0006 and L-0007  While sizing the maximum load for the rooftop cranes it was communicated by PWP and TT that the maximum tree size will be no greater than 25,000 lbs and that the roof is designed for a maximum of 26,000 lbs. Confirm that the structure will be able to accommodate the additional tree weight which will incur over the years as the trees grow.		<b>ANSWER:</b> Reference: L-0006 and L-0007  While sizing the maximum load for the rooftop cranes it was communicated by PWP and TT that the maximum tree size will be no greater than 25,000 lbs and that the roof is designed for a maximum of 26,000 lbs. Confirm that the structure will be able to accommodate the additional tree weight which will incur over the years as the trees grow.				
P1-0236	Details for Expansion Joint Covers in Concrete Barrier	Void	0P	07/11/2014	07/21/2014	08/05/2014
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				



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<b>P1-0238</b>	<b>Part B Documentation Absent from Contract Documents</b>	<b>Closed</b>	<b>0P</b>	<b>07/11/2014</b>	<b>07/21/2014</b>	<b>07/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Specification 07 09 13 1.1 B  Specification Section 07 09 13 1.1 B states, "Refer to Part B Documents applicable to this Section."  "Part B" documents do not appear to be a part of the contract documents. Please provide the Part B Documents referenced in Specification Section 07 09 13 1.1 B.		<b>ANSWER:</b>  REFERENCE: Specification 07 09 13 1.1 B  Specification Section 07 09 13 1.1 B states, "Refer to Part B Documents applicable to this Section."  "Part B" documents do not appear to be a part of the contract documents. Please provide the Part B Documents referenced in Specification Section 07 09 13 1.1 B.				
<b>P1-0239</b>	<b>Updated Table of Contents</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/14/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: ASI 119 and spec 00 01 10 Marked up Table of Contents dated 06/18/2014 and 07/02/2014 does not reflect the consolidation of specifications outlined by the Narrative. Based on the description, 12 93 30 and 12 93 30/APA have been fully consolidated into 28 16 44 and 28 16 44/APA. They should show as being deleted or revised if they are still meant to be included. At the moment there is conflicting information without this update. The TOC does not reflect the date of the most current marked up specs either. Provide clarification and an updated TOC.		<b>ANSWER:</b>  Reference: ASI 119 and spec 00 01 10 Marked up Table of Contents dated 06/18/2014 and 07/02/2014 does not reflect the consolidation of specifications outlined by the Narrative. Based on the description, 12 93 30 and 12 93 30/APA have been fully consolidated into 28 16 44 and 28 16 44/APA. They should show as being deleted or revised if they are still meant to be included. At the moment there is conflicting information without this update. The TOC does not reflect the date of the most current marked up specs either. Provide clarification and an updated TOC.				
<b>P1-0240</b>	<b>Incorrect Specification Reference</b>	<b>Void</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: ASI 119 and 00 50 00 00 50 00 markup dated 06/18/2014 - Section 2.5.H needs to refer to section 28 16 44, not 12 93 30. Please correct or provide clarification as to what they need to refer to in 12 93 30.		<b>ANSWER:</b>  Reference: ASI 119 and 00 50 00 00 50 00 markup dated 06/18/2014 - Section 2.5.H needs to refer to section 28 16 44, not 12 93 30. Please correct or provide clarification as to what they need to refer to in 12 93 30.				
<b>P1-0241</b>	<b>Blockout or Sleeve Locations</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/14/2014</b>	<b>07/22/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						





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<div><div><b>REQUEST:</b><p>Reference: ASI 119 and A1-3100 Routing for lines from HPU to Pull Box are indicated. Clarify if blockouts or sleeves are being added for TG07.2 to accommodate this, as discussed at RFI meeting on 05/23/2014. If so, provide for every location. It is not clear on these drawings.</p></div><div><b>ANSWER:</b><p>Reference: ASI 119 and A1-3100 Routing for lines from HPU to Pull Box are indicated. Clarify if blockouts or sleeves are being added for TG07.2 to accommodate this, as discussed at RFI meeting on 05/23/2014. If so, provide for every location. It is not clear on these drawings.</p></div></div>						
<b>P1-0241.1</b>	<b>Blockout or Sleeve Locations</b>	<b>Closed</b>	<b>0P</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/21/2014</b>
<div><div><b>From:</b> Webcor Construction LP      Zachary Moore</div><div><b>REQUEST:</b><p>reference : ASI 121: A1-2870, A1-2862, A1-2864, A1-2866, and A1-2867</p><p>RFI P1-0241 response: "Refer to Edge of Slab sheets in ASI-0121 issued on 07/18/2014. Design Team does not assign scope for Trade Groups."</p><p>Edge of Slab sheets do not show block outs for bollard and barrier hydraulic and electrical lines to each of the HPUs as was discussed in RFI meeting on 05/23/2014.</p><ul style="list-style-type: none"><li>- A1-2870 - Provide sleeves or blockouts for retractable bollards and barriers in the shoring wall area at vehicle ramp so that hydraulic and electrical lines may reach the HPU.</li><li>- A1-2862 - Provide sleeves or blockouts for retractable bollards and barriers at GL 1 and Natoma St. so that hydraulic and electrical lines may reach the HPU.</li><li>- A1-2864 - Provide blockout and/or sleeves for Natoma loading dock area so that hydraulic and electrical lines may reach the HPU.</li><li>- A1-2866 and A1-2867 - Confirm that 4" SL's on GL 27 and 343.5 are for the wedge barrier hydraulic and electrical lines to reach the HPU below. If not, provide sleeves or blockouts so that hydraulic and electrical lines may connect to the HPU.</li></ul></div><div><b>ANSWER:</b><p>reference : ASI 121: A1-2870, A1-2862, A1-2864, A1-2866, and A1-2867</p><p>RFI P1-0241 response: "Refer to Edge of Slab sheets in ASI-0121 issued on 07/18/2014. Design Team does not assign scope for Trade Groups."</p><p>Edge of Slab sheets do not show block outs for bollard and barrier hydraulic and electrical lines to each of the HPUs as was discussed in RFI meeting on 05/23/2014.</p><ul style="list-style-type: none"><li>- A1-2870 - Provide sleeves or blockouts for retractable bollards and barriers in the shoring wall area at vehicle ramp so that hydraulic and electrical lines may reach the HPU.</li><li>- A1-2862 - Provide sleeves or blockouts for retractable bollards and barriers at GL 1 and Natoma St. so that hydraulic and electrical lines may reach the HPU.</li><li>- A1-2864 - Provide blockout and/or sleeves for Natoma loading dock area so that hydraulic and electrical lines may reach the HPU.</li><li>- A1-2866 and A1-2867 - Confirm that 4" SL's on GL 27 and 343.5 are for the wedge barrier hydraulic and electrical lines to reach the HPU below. If not, provide sleeves or blockouts so that hydraulic and electrical lines may connect to the HPU.</li></ul></div></div>						
<b>P1-0242</b>	<b>Blockout or Sleeve Locations</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>07/29/2014</b>







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<b>P1-0245</b>	<b>Information on Steel Plates for Utility Pads</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/14/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Sheet A1-3002 (IFC Drawings for Main Package dated 3/31/14)  As shown on Sheet A1-3002, steel plates have been added to the utility pads. No information is provided regarding the thickness of the steel plates, attachment details, or grounding details.  Please provide the thickness, attachment, and grounding details for the steel plates referenced on Sheet A1-3002.		<b>ANSWER:</b>  REFERENCE: Sheet A1-3002 (IFC Drawings for Main Package dated 3/31/14)  As shown on Sheet A1-3002, steel plates have been added to the utility pads. No information is provided regarding the thickness of the steel plates, attachment details, or grounding details.  Please provide the thickness, attachment, and grounding details for the steel plates referenced on Sheet A1-3002.				
<b>P1-0246</b>	<b>ASI 118 and 119 Discrepancy</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/14/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  reference: ASI 118, 119 and A1-7416 (SK-3524) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.		<b>ANSWER:</b>  reference: ASI 118, 119 and A1-7416 (SK-3524) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.				
<b>P1-0247</b>	<b>ASI 118 and 119 Discrepancy</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: ASI 118, 119 and A1-7418 (SKA-3526) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.		<b>ANSWER:</b>  Reference: ASI 118, 119 and A1-7418 (SKA-3526) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.				
<b>P1-0248</b>	<b>Information on Checker Plate on Loading Dock CMU Walls</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/14/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Detail B/A1-3101 (IFC Drawings for Main		<b>ANSWER:</b>  REFERENCE: Detail B/A1-3101 (IFC Drawings for				



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<b>P1-0251</b>	<b>ASI 118 and 119 Discrepancy sheet A1-7420 (SKA-3528)</b>  <b>From:</b> Webcor Construction LP                      Zachary Moore  <b>REQUEST:</b>  Reference: ASI 118, 119 and A1-7420 (SKA-3528) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/14/2014</b>	<b>08/05/2014</b>
	<b>ANSWER:</b>  Reference: ASI 118, 119 and A1-7420 (SKA-3528) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.					
<b>P1-0252</b>	<b>Indicator Light Pylons and Photo Electric Beam Pylons</b>  <b>From:</b> Webcor Construction LP                      Zachary Moore  <b>REQUEST:</b>  Reference: ASI 118 and 119 - A1-7420 (Detail C), A1-7420 Detail C - Indicator light pylons and photo electric beam pylons are missing from either side of the retractable bollard sets based on A1-7418 and 28 16 44. Confirm that indicator light pylons and photo electric beam pylons are intended to be included in this detail and provide an updated detail.	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>08/05/2014</b>
	<b>ANSWER:</b>  Reference: ASI 118 and 119 - A1-7420 (Detail C), A1-7420 Detail C - Indicator light pylons and photo electric beam pylons are missing from either side of the retractable bollard sets based on A1-7418 and 28 16 44. Confirm that indicator light pylons and photo electric beam pylons are intended to be included in this detail and provide an updated detail.					
<b>P1-0253</b>	<b>ASI 118 and 119 Discrepancy sheet A1-7421 (SKA-3529)</b>  <b>From:</b> Webcor Construction LP                      Zachary Moore  <b>REQUEST:</b>  Reference: A1-7421 (SKA-3529) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>07/29/2014</b>
	<b>ANSWER:</b>  Reference: A1-7421 (SKA-3529) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.					
<b>P1-0254</b>	<b>Corner Guard Specification Details</b>  <b>From:</b> Webcor Construction LP                      Tram Nguyen  <b>REQUEST:</b>  REFERENCE: Specification Section 05 50 00 2.5M (IFC Drawings for Main Package dated 3/31/14)  Specification Section 05 50 00 2.5M states: "M. Corner Guards For Concrete Columns/Concrete Block Walls: as	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>08/05/2014</b>
	<b>ANSWER:</b>  REFERENCE: Specification Section 05 50 00 2.5M (IFC Drawings for Main Package dated 3/31/14)  Specification Section 05 50 00 2.5M states: "M. Corner Guards For Concrete Columns/Concrete Block					



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	<p>detailed, 4' x 4" x ½" fabricated aluminum angles, 4'-0" high minimum and as shown on drawings with anchor straps at 12" o.c."</p> <p>The referenced details do not appear to be provided. Please provide the details called out in Specification Section 05 50 00 2.5M.</p>					
P1-0254.1	VOID	Void	CR	08/14/2014	08/24/2014	
	From: Webcor Construction LP                      Tram Nguyen					
	REQUEST:		ANSWER:			
P1-0255	ASI 118 and 119 Discrepancy sheet A1-7422 (SKA-3530)	Closed	0P	07/14/2014	07/14/2014	07/29/2014
	From: Webcor Construction LP                      Zachary Moore					
	REQUEST:		ANSWER:			
	Reference: ASi 118, 119 and A1-7422 (SKA-3530) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.					
P1-0256	ASI 118 and 119 Discrepancy sheet A1-7423 (SKA-3531)	Closed	0P	07/14/2014	07/24/2014	08/05/2014
	From: Webcor Construction LP                      Zachary Moore					
	REQUEST:		ANSWER:			
	Reference: ASI 118, 119 and A1-7423 (SKA-3531) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.					



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<b>P1-0257</b>	<b>ASI 118 and 119 Discrepancy sheet A1-7424 (SKA-3532)</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: ASI 118, 119 and A1-7424 (SKA-3532)  There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.		<b>ANSWER:</b> reference: ASI 118, 119 and A1-7424 (SKA-3532)  There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.				
<b>P1-0258</b>	<b>Manufacturers On-site for Bollard/Barrier Installation</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/14/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b> Reference: 28 16 44 1.2.A ASI 119  1.2.A states that "Contractor is responsible for having the manufacturer of the product on site during the installation process to monitor the work associated with their product." Clarify how many hours the manufacturer needs to be on site. Is the Contractor required to have the manufacturer on site full time?		<b>ANSWER:</b> Reference: 28 16 44 1.2.A ASI 119  1.2.A states that "Contractor is responsible for having the manufacturer of the product on site during the installation process to monitor the work associated with their product." Clarify how many hours the manufacturer needs to be on site. Is the Contractor required to have the manufacturer on site full time?				
<b>P1-0259</b>	<b>ASI 118 and 119 Discrepancy sheet A1-8720 (SKA-3542)</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference: ASI 118, 119 and A1-8720 (SKA-3542)  There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It appears that neither ASI on its own is the most current for this sheet. Provide a revised sheet incorporating the most current information.		<b>ANSWER:</b> Reference: ASI 118, 119 and A1-8720 (SKA-3542)  There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It appears that neither ASI on its own is the most current for this sheet. Provide a revised sheet incorporating the most current information.				
<b>P1-0260</b>	<b>ASI 118 and 119 Discrepancy sheet A1-8721 (SKA-3543)</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						



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<b>REQUEST:</b> Reference: ASI 118, 119 and A1-8721 (SKA-3543) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It appears that neither ASI on its own is the most current for this sheet. Provide a revised sheet incorporating the most current information.						
<b>ANSWER:</b> Reference: ASI 118, 119 and A1-8721 (SKA-3543) There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for this sheet. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It appears that neither ASI on its own is the most current for this sheet. Provide a revised sheet incorporating the most current information.						
<b>P1-0261</b>	<b>Difference Between Bollards</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference: 28 16 44 section 2.2 -Clarify the difference between Bollard Type 1 and Bollard Type 1A. They have the exact same requirements. -Clarify the difference between Bollard Types 2, 2A, 2B and 2C. They have the exact same requirements. -Clarify the difference between Bollard Types 3 and 3A. They have the exact same requirements. -Clarify the difference between Bollard Type BOL-1 and BOL-2. They have the exact same requirements.						
<b>ANSWER:</b> Reference: 28 16 44 section 2.2 -Clarify the difference between Bollard Type 1 and Bollard Type 1A. They have the exact same requirements. -Clarify the difference between Bollard Types 2, 2A, 2B and 2C. They have the exact same requirements. -Clarify the difference between Bollard Types 3 and 3A. They have the exact same requirements. -Clarify the difference between Bollard Type BOL-1 and BOL-2. They have the exact same requirements.						
<b>P1-0262</b>	<b>Blockout or Sleeve Detail</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Reference: ASI 118 drawings A1-2206, A1-2203, A1-2207, and A1-2210  -Clarify if blockouts or sleeves are being added for TG07.2 to accommodate the HPU locations on the lower concourse, as discussed at RFI meeting on 05/23/2014. If so, provide for every location. It is not clear on these drawings.						
<b>ANSWER:</b> Reference: ASI 118 drawings A1-2206, A1-2203, A1-2207, and A1-2210  -Clarify if blockouts or sleeves are being added for TG07.2 to accommodate the HPU locations on the lower concourse, as discussed at RFI meeting on 05/23/2014. If so, provide for every location. It is not clear on these drawings.						
<b>P1-0263</b>	<b>Blockout or Sleeve Detail</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	

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	<p><b>From:</b> Webcor Construction LP                      Zachary Moore</p> <p><b>REQUEST:</b></p> <p>Reference: ASI 118 and A1-2203</p> <p>Clarify if blockouts or sleeves are being added for TG07.2 to accommodate the HPU locations on the lower concourse, as discussed at RFI meeting on 05/23/2014. If so, provide for every location. It is not clear on these drawings.</p>					<p><b>ANSWER:</b></p> <p>Reference: ASI 118 and A1-2203</p> <p>Clarify if blockouts or sleeves are being added for TG07.2 to accommodate the HPU locations on the lower concourse, as discussed at RFI meeting on 05/23/2014. If so, provide for every location. It is not clear on these drawings.</p>
<b>P1-0264</b>	<b>Difference between Type 2 Bollards</b>	<b>Closed</b>	<b>CR</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	
	<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference: 28 16 44 Section 2.2.C, 2.2.D, 2.2.E and 2.2.F:</p> <p>Clarify the difference between Bollard Types 2, 2A, 2B and 2C. They have the exact same requirements.</p>					<p><b>ANSWER:</b></p> <p>Reference: 28 16 44 Section 2.2.C, 2.2.D, 2.2.E and 2.2.F:</p> <p>Clarify the difference between Bollard Types 2, 2A, 2B and 2C. They have the exact same requirements.</p>
<b>P1-0265</b>	<b>Difference between Bollards Type 3</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	
	<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference: 28 16 44 Section 2.2.G and 2.2.H</p> <p>Clarify the difference between Bollard Types 3 and 3A. They have the exact same requirements.</p>					<p><b>ANSWER:</b></p> <p>Reference: 28 16 44 Section 2.2.G and 2.2.H</p> <p>Clarify the difference between Bollard Types 3 and 3A. They have the exact same requirements.</p>
<b>P1-0266</b>	<b>Blockout or Sleeve Detail</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	
	<p><b>From:</b> Webcor Construction LP                      Zachary Moore</p> <p><b>REQUEST:</b></p> <p>Reference: ASI 118 and A1-2207</p> <p>Clarify if blockouts or sleeves are being added for TG07.2 to accommodate the HPU locations on the lower concourse, as discussed at RFI meeting on 05/23/2014. If so, provide for every location. It is not clear on these drawings.</p>					<p><b>ANSWER:</b></p> <p>Reference: ASI 118 and A1-2207</p> <p>Clarify if blockouts or sleeves are being added for TG07.2 to accommodate the HPU locations on the lower concourse, as discussed at RFI meeting on 05/23/2014. If so, provide for every location. It is not clear on these drawings.</p>





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<b>P1-0267</b>	<b>Blockout or Sleeve Detail</b>	<b>Closed</b>	<b>0P</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference: ASI 118 and A1-2210 Clarify if blockouts or sleeves are being added for TG07.2 to accommodate the HPU locations on the lower concourse, as discussed at RFI meeting on 05/23/2014. If so, provide for every location. It is not clear on these drawings.						<b>ANSWER:</b> Reference: ASI 118 and A1-2210 Clarify if blockouts or sleeves are being added for TG07.2 to accommodate the HPU locations on the lower concourse, as discussed at RFI meeting on 05/23/2014. If so, provide for every location. It is not clear on these drawings.
<b>P1-0268</b>	<b>Difference between BOL-1 and BOL-2</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b> Reference: 28 16 44 2.2.J and 2.2.K ASI 119  Clarify the difference between Bollard Type BOL-1 and BOL-2. They have the exact same requirements.						<b>ANSWER:</b> Reference: 28 16 44 2.2.J and 2.2.K ASI 119  Clarify the difference between Bollard Type BOL-1 and BOL-2. They have the exact same requirements.
<b>P1-0269</b>	<b>Vehicle Pulse</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> Reference: ASI 119 and 28 16 44 2.6.B.3 - Clarify how "b. Pulse on vehicle arrival" and "c. Pulse on vehicle departure" are different from "d. Pulse on arrival and departure."						<b>ANSWER:</b> Reference: ASI 119 and 28 16 44 2.6.B.3 - Clarify how "b. Pulse on vehicle arrival" and "c. Pulse on vehicle departure" are different from "d. Pulse on arrival and departure."
<b>P1-0270</b>	<b>API or SDK Interface</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b> Reference: 28 16 44 2.8.B.4 ASI 119  2.8.B.4 states that "Any features not available through the API or SDK interface shall be described as an exception by the Contractor as a part of the Contractor's bid." Since exceptions are not allowed in any bids for this project and would render the bid invalid, the statement needs to be removed.						<b>ANSWER:</b> Reference: 28 16 44 2.8.B.4 ASI 119  2.8.B.4 states that "Any features not available through the API or SDK interface shall be described as an exception by the Contractor as a part of the Contractor's bid." Since exceptions are not allowed in any bids for this project and would render the bid invalid, the statement needs to be removed.



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<b>P1-0271</b>	<b>Bid Exceptions</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: ASI 119 and 28 16 44  2.8.D.2 states "Any features not able to operate in a virtualized server environment shall be described as an exception by the Contractor as a part of the Contractor's bid." Since exceptions are not allowed in any bids for this project and would render the bid invalid, the statement needs to be removed.						<b>ANSWER:</b>  Reference: ASI 119 and 28 16 44  2.8.D.2 states "Any features not able to operate in a virtualized server environment shall be described as an exception by the Contractor as a part of the Contractor's bid." Since exceptions are not allowed in any bids for this project and would render the bid invalid, the statement needs to be removed.
<b>P1-0272</b>	<b>Construction Manager</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference: 28 16 44 2.8.B.6 ASI 119  2.8.B.6 states that "The Contractor shall furnish the API or SDK to the Construction Manager at the time of system procurement. Any updates to the API or SDK shall be provided to the Construction manager at the time of release throughout the lifecycle of the project." "Construction Manager" needs to be replaced by "TJPA".						<b>ANSWER:</b>  Reference: 28 16 44 2.8.B.6 ASI 119  2.8.B.6 states that "The Contractor shall furnish the API or SDK to the Construction Manager at the time of system procurement. Any updates to the API or SDK shall be provided to the Construction manager at the time of release throughout the lifecycle of the project." "Construction Manager" needs to be replaced by "TJPA".
<b>P1-0273</b>	<b>Secondary Controllers</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: ASI 119 and 28 16 44/APA  1.3.A.5 - Confirm that Secondary Controllers are not required at the Minna Loading, Natoma Loading, Natoma Pedestrian Area East and Natoma Pedestrian Area West based on the table provided. Without Secondary Controllers these areas are unable to comply with section 3.5.G Single Layer Anti-Ram Barrier Entry Sequence and 3.5.H. Single Layer Anti-Ram Barrier Exit Sequence.						<b>ANSWER:</b>  Reference: ASI 119 and 28 16 44/APA  1.3.A.5 - Confirm that Secondary Controllers are not required at the Minna Loading, Natoma Loading, Natoma Pedestrian Area East and Natoma Pedestrian Area West based on the table provided. Without Secondary Controllers these areas are unable to comply with section 3.5.G Single Layer Anti-Ram Barrier Entry Sequence and 3.5.H. Single Layer Anti-Ram Barrier Exit Sequence.



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P1-0274	Sally Port Exit Sequence	Closed	0P	07/15/2014	07/25/2014	08/05/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference: 28 16 44/APA 3.5.E.5 ASI 119		Reference: 28 16 44/APA 3.5.E.5 ASI 119				
Sally Port Configuration Exit Sequence: Vehicle Operator Initiated via Sensors (Howard Street Exit Only) - Confirm that a card reader is not required upon exit of the Sally Port, as it is upon entry, and that the barrier opens automatically upon exit of any vehicle as outlined in this sequence.		Sally Port Configuration Exit Sequence: Vehicle Operator Initiated via Sensors (Howard Street Exit Only) - Confirm that a card reader is not required upon exit of the Sally Port, as it is upon entry, and that the barrier opens automatically upon exit of any vehicle as outlined in this sequence.				
P1-0275	Anti Ram Barrier Card Reader	Closed	0P	07/15/2014	07/25/2014	08/20/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:		ANSWER:				
Reference: ASI 119 and 28 16 44/APA		Reference: ASI 119 and 28 16 44/APA				
3.5.G and 3.5.H - Based on the sequence of operations outlined for single layer anti-ram barriers in section 3.5.G and 3.5.H confirm that card readers are not required for any of the single layer anti-ram barriers upon entry or exit and that they will all be controlled manually by an operator.		3.5.G and 3.5.H - Based on the sequence of operations outlined for single layer anti-ram barriers in section 3.5.G and 3.5.H confirm that card readers are not required for any of the single layer anti-ram barriers upon entry or exit and that they will all be controlled manually by an operator.				
P1-0276	Sequence of Operation Anti-Ram Barriers	Closed	0P	07/15/2014	07/25/2014	08/25/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference: 28 16 44/APA ASI 119		Reference: 28 16 44/APA ASI 119				
Provide sequence of operation for Bus Plaza Anti-Ram Barriers and their integration with the traffic control system, including but not limited to, types of output required (when does the traffic light change color, which traffic light changes color for each barrier, what stage of barrier operation aligns with each output) and extent of interaction with traffic lights. For example, do lights at Howard and Mission street change color depending on status of lights at the Bus Plaza Anti-Ram Barriers?		Provide sequence of operation for Bus Plaza Anti-Ram Barriers and their integration with the traffic control system, including but not limited to, types of output required (when does the traffic light change color, which traffic light changes color for each barrier, what stage of barrier operation aligns with each output) and extent of interaction with traffic lights. For example, do lights at Howard and Mission street change color depending on status of lights at the Bus Plaza Anti-Ram Barriers?				



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<b>P1-0277</b>	<b>Vehicle Barrier Timing</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>  Reference: ASI 119 and 28 16 44/APA  3.5.L. - Provide time period required before vehicle barrier should automatically rise.		<b>ANSWER:</b>  Reference: ASI 119 and 28 16 44/APA  3.5.L. - Provide time period required before vehicle barrier should automatically rise.				
<b>P1-0278</b>	<b>Specifications for Stone Threshold References</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Details 2 & 5/A1-9001, Details 2 & 5/A1-9002 (IFC Drawings for Main Package dated 3/31/14)  Details throughout the plans (for example, Details 2 & 5/A1-9001 and Details 2 & 5/A1-9001) call for "Stone Thresholds" to be used. The specifications do not reference "Stone Thresholds".  Please confirm that thresholds called out to be "Stone Thresholds" are to be furnished and installed as specified in Specification Section 09 30 00 2.5 "Marble Thresholds".		<b>ANSWER:</b>  REFERENCE: Details 2 & 5/A1-9001, Details 2 & 5/A1-9002 (IFC Drawings for Main Package dated 3/31/14)  Details throughout the plans (for example, Details 2 & 5/A1-9001 and Details 2 & 5/A1-9001) call for "Stone Thresholds" to be used. The specifications do not reference "Stone Thresholds".  Please confirm that thresholds called out to be "Stone Thresholds" are to be furnished and installed as specified in Specification Section 09 30 00 2.5 "Marble Thresholds".				
<b>P1-0279</b>	<b>Details for Threaded Insert Drainage</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Detail 6/A1-9321 (IFC Drawings for Main Package dated 3/31/14)  Detail 6/A1-9321 references drain holes at the bottom of the threaded insert, but does not depict the relationship of the drain hole to the substrate.  Please provide details for draining of the threaded insert.		<b>ANSWER:</b>  REFERENCE: Detail 6/A1-9321 (IFC Drawings for Main Package dated 3/31/14)  Detail 6/A1-9321 references drain holes at the bottom of the threaded insert, but does not depict the relationship of the drain hole to the substrate.  Please provide details for draining of the threaded insert.				



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<b>P1-0280</b>	<b>Confirmation for Corrosion Expert Requirement</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
REFERENCE: Specification Section 07 09 15 1.6 H (IFC Drawings for Main Package dated 3/31/14) Specification Section 07 09 15 2.8 C (IFC Drawings for Main Package dated 3/31/14)  Specification Section 07 09 15 1.6 H states, "Engage a California-licensed Corrosion Engineer who is an expert in corrosion, to conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of curtain wall and aluminum panels and provide report."  Specification Section 07 09 15 2.8 C states, "Comply with recommendations of the corrosion engineer approved by the TPJA Representative, as specified above."  As agreed upon between TJPA, PMPC, and Webcor-Obayashi, the language "Conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report" will be used in lieu of the language in Specification Sections 07 09 15 1.6 H and 07 09 15 2.8 C  Please confirm that the language, "Conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report." is to be used in lieu of the language currently required by 07 09 15 1.6 H and 07 09 15 2.8 C.			REFERENCE: Specification Section 07 09 15 1.6 H (IFC Drawings for Main Package dated 3/31/14) Specification Section 07 09 15 2.8 C (IFC Drawings for Main Package dated 3/31/14)  Specification Section 07 09 15 1.6 H states, "Engage a California-licensed Corrosion Engineer who is an expert in corrosion, to conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of curtain wall and aluminum panels and provide report."  Specification Section 07 09 15 2.8 C states, "Comply with recommendations of the corrosion engineer approved by the TPJA Representative, as specified above."  As agreed upon between TJPA, PMPC, and Webcor-Obayashi, the language "Conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report" will be used in lieu of the language in Specification Sections 07 09 15 1.6 H and 07 09 15 2.8 C  Please confirm that the language, "Conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report." is to be used in lieu of the language currently required by 07 09 15 1.6 H and 07 09 15 2.8 C.			



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<b>P1-0281</b>	<b>Specification for Joint Type FJC8 at Vehicle/Bike Ramp</b>	<b>Closed</b>	<b>0P</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0227, Sketch SKA-3736.4, Detail 2/A1-8885 (ASI 118 dated 6/20/2014)  Per RFI Response P1-0227 received on 07/10/14, FJC8 is to be furnished and installed between the W-13 and terrazzo flooring at Ground Level GL 23 (see SKA-3736.4).  Per ASI 118, Detail 2/A1-8885 received 07/07/14, FJC8 is located at the vehicle/bicycle ramp.  Please clarify the type and specification of FJC8 located at the vehicle/bicycle ramp.						<b>ANSWER:</b>  REFERENCE: RFI Response P1-0227, Sketch SKA-3736.4, Detail 2/A1-8885 (ASI 118 dated 6/20/2014)  Per RFI Response P1-0227 received on 07/10/14, FJC8 is to be furnished and installed between the W-13 and terrazzo flooring at Ground Level GL 23 (see SKA-3736.4).  Per ASI 118, Detail 2/A1-8885 received 07/07/14, FJC8 is located at the vehicle/bicycle ramp.  Please clarify the type and specification of FJC8 located at the vehicle/bicycle ramp.
<b>P1-0282</b>	<b>1" Moving Joint at Vehicular &amp; Bicycle Ramp</b>	<b>Closed</b>	<b>0P</b>	<b>07/16/2014</b>	<b>07/26/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Sheet A1-7402 (ASI 117 dated 4/23/14)  Per Sheet A1-7402 of ASI 117, there is a $\frac{1}{2}$ " movement joint at the top of the vehicular and bicycle ramps, but no details or specifications are provided showing the construction of this joint.  Please provide details and specifications for the $\frac{1}{2}$ " movement joint at the vehicle and bicycle ramps.						<b>ANSWER:</b>  REFERENCE: Sheet A1-7402 (ASI 117 dated 4/23/14)  Per Sheet A1-7402 of ASI 117, there is a $\frac{1}{2}$ " movement joint at the top of the vehicular and bicycle ramps, but no details or specifications are provided showing the construction of this joint.  Please provide details and specifications for the $\frac{1}{2}$ " movement joint at the vehicle and bicycle ramps.
<b>P1-0283</b>	<b>R1 Requirements for Wall Type 3</b>	<b>Closed</b>	<b>0P</b>	<b>07/16/2014</b>	<b>07/26/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Detail 11/A1-9208 (ASI 118 dated 6/20/14), A-0024 (IFC Drawings for Main Package dated 3/31/14)  Per Detail 11/A1-9208 of ASI 118, and associated zone plans, concrete columns are to be framed in with wall type 3-R1. Per Sheet A-0024, there is no Comment R1 for Wall Type 3.						<b>ANSWER:</b>  REFERENCE: Detail 11/A1-9208 (ASI 118 dated 6/20/14), A-0024 (IFC Drawings for Main Package dated 3/31/14)  Per Detail 11/A1-9208 of ASI 118, and associated zone plans, concrete columns are to be framed in with wall type 3-R1. Per Sheet A-0024, there is no Comment R1 for Wall Type 3.



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	Please provide the R1 requirements for Wall Type 3.					
P1-0284	Category 3 Stairs, Platforms, and Rails for Phase 1	Closed	0P	07/16/2014	07/16/2014	08/20/2014
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Sheet SKA-3076 (dated 3/13/14)</p> <p>TJPA did not accept TG07.5R Bid Additive Alternate No. 01 to furnish and install Category 3 stairs, platforms, and rails as shown on SKA-3076 (Attached).</p> <p>Category 3 metal stairs, platforms, and rails still show on the contract documents, however TJPA has indicated they do not intend for the Category 3 metal stairs, platforms, and rails to be installed as shown.</p> <p>Please confirm stairs, platforms, and rails are not to be provided as part of Phase 1 at Category 3 locations, or provide direction for the desired access at these locations.</p>					<p><b>ANSWER:</b></p> <p>REFERENCE: Sheet SKA-3076 (dated 3/13/14)</p> <p>TJPA did not accept TG07.5R Bid Additive Alternate No. 01 to furnish and install Category 3 stairs, platforms, and rails as shown on SKA-3076 (Attached).</p> <p>Category 3 metal stairs, platforms, and rails still show on the contract documents, however TJPA has indicated they do not intend for the Category 3 metal stairs, platforms, and rails to be installed as shown.</p> <p>Please confirm stairs, platforms, and rails are not to be provided as part of Phase 1 at Category 3 locations, or provide direction for the desired access at these locations.</p>



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<b>P1-0285</b>	<b>Concrete Curbs at Lower Concourse, Zone 4</b>	<b>Closed</b>	<b>0P</b>	<b>07/16/2014</b>	<b>07/26/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Sheet A1-2207 (ASI 118 dated 6/20/14)  Sheet A1-2224 of ASI 118 calls out concrete curb types at the following locations:  GL 14/G, GL 19/C, GL 24/H, GL 31/F.7, GL 32/C, and GL 32/G  The concrete curbs at the above locations are depicted with dashed lines. Are concrete curbs called out but shown dashed to be furnished and installed as part of Phase 1?						<b>ANSWER:</b>  REFERENCE: Sheet A1-2207 (ASI 118 dated 6/20/14)  Sheet A1-2224 of ASI 118 calls out concrete curb types at the following locations:  GL 14/G, GL 19/C, GL 24/H, GL 31/F.7, GL 32/C, and GL 32/G  The concrete curbs at the above locations are depicted with dashed lines. Are concrete curbs called out but shown dashed to be furnished and installed as part of Phase 1?
<b>P1-0286</b>	<b>Specification for Chain Linked Fences</b>	<b>Closed</b>	<b>0P</b>	<b>07/16/2014</b>	<b>07/26/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: A1-2104 (ASI 119 dated 6/20/14)  ASI 119 calls out for a chain link fence at several locations (A1-2104 for instance), but no chain link fence specification exists.  Please provide a specification for chain link fences.						<b>ANSWER:</b>  REFERENCE: A1-2104 (ASI 119 dated 6/20/14)  ASI 119 calls out for a chain link fence at several locations (A1-2104 for instance), but no chain link fence specification exists.  Please provide a specification for chain link fences.







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sloping the concrete at this condition.						
<b>P1-0290</b>	<b>Painted Galvanized Flashing per ASI 119 and Specification Section 07 62 00 2.3 A</b>	<b>Closed</b>	<b>0P</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
REFERENCE: ASI 119 (dated 6/20/14), Specification Section 07 62 00 2.3A (IFC Drawings for Main Package dated 3/31/14)			REFERENCE: ASI 119 (dated 6/20/14), Specification Section 07 62 00 2.3A (IFC Drawings for Main Package dated 3/31/14)			
ASI 119 calls for painted galvanized flashing to be used in several locations (for instance, Detail B/A1-7407 of ASI 119). Specification Section 07 62 00 2.3 A states, "Galvanized Steel: Do not use."			ASI 119 calls for painted galvanized flashing to be used in several locations (for instance, Detail B/A1-7407 of ASI 119). Specification Section 07 62 00 2.3 A states, "Galvanized Steel: Do not use."			
Please provide a specification for the painted galvanized flashing.			Please provide a specification for the painted galvanized flashing.			



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<b>P1-0291</b>	<b>Engineering Requirements for Spray On Fireproofing per Specification Section 01 80 50 1.1 B</b>	<b>Closed</b>	<b>0P</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 01 80 50 1.1 B Specification Section 01 80 50 1.2 A Specification Section 07 81 00 Specification Section 07 81 23			REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 01 80 50 1.1 B Specification Section 01 80 50 1.2 A Specification Section 07 81 00 Specification Section 07 81 23			
Per Specification Section 01 80 50 1.1 B, "the requirements of this Section apply when to all non-structural components, unless otherwise noted."			Per Specification Section 01 80 50 1.1 B, "the requirements of this Section apply when to all non-structural components, unless otherwise noted."			
Per Specification Section 01 80 50 1.2 A: "Non-structural building components are components that are not part of the building structural system whether inside or outside the building enclosure, above and below grade. Nonstructural components of the Transbay Transit Center include, but not necessarily limited, to the following: 1. Ceilings, glazing, awnings, and similar components and assemblies. 2. Electrical: Power and lighting components and systems; substations; switchgear and switchboards; auxiliary engine-generator sets; transfer switches; motor control centers; motor generators; selector and controller panels; fire protection and alarm components and systems; and telephone and communication components and systems. 3. Mechanical: Heating, ventilating, and air-conditioning components and systems; plumbing components and systems; fire sprinkler components and systems. 4. Vertical Transportation: Mechanical, electrical and supporting elements for transport systems, including elevators and escalators. "			Per Specification Section 01 80 50 1.2 A: "Non-structural building components are components that are not part of the building structural system whether inside or outside the building enclosure, above and below grade. Nonstructural components of the Transbay Transit Center include, but not necessarily limited, to the following: 1. Ceilings, glazing, awnings, and similar components and assemblies. 2. Electrical: Power and lighting components and systems; substations; switchgear and switchboards; auxiliary engine-generator sets; transfer switches; motor control centers; motor generators; selector and controller panels; fire protection and alarm components and systems; and telephone and communication components and systems. 3. Mechanical: Heating, ventilating, and air-conditioning components and systems; plumbing components and systems; fire sprinkler components and systems. 4. Vertical Transportation: Mechanical, electrical and supporting elements for transport systems, including elevators and escalators. "			
Specification Section 07 81 00 Sprayed Fire Resistive Material and Specification Section 07 81 23 Intumescent Fire Resistive Material do not include items specifically noted in the non-structural list, however, Specification Section 01 80 50 1.2 A explicitly states the list is not all inclusive. In addition, 07 81 00 and 07 81 23 do not specifically exclude the requirements associated with 01 80 50.			Specification Section 07 81 00 Sprayed Fire Resistive Material and Specification Section 07 81 23 Intumescent Fire Resistive Material do not include items specifically noted in the non-structural list, however, Specification Section 01 80 50 1.2 A explicitly states the list is not all inclusive. In addition, 07 81 00 and 07 81 23 do not specifically exclude the requirements associated with 01 80 50.			



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P1-0292	<p>Please confirm material furnished and installed under Specification Sections 07 81 00 and 07 81 23 are not required to have engineering as required by Specification Section 01 80 50. If engineering is required for items furnished and installed under Specification Sections 07 81 00 and 07 81 23, please specifically identify which elements are to be engineered.</p>	Closed	0P	07/18/2014	07/28/2014	08/20/2014
	<p><b>From:</b> Webcor Construction LP      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Detail 6/A1-9306 (ASI 119 dated 6/18/14)</p> <p>Per Detail 6/A1-9306, the HD galvanized rail embed shows a drain hole in the bottom of the embed. The detail goes on to show the bottom of the embed seated in grout which will prevent water from draining out of the drain hole.</p> <p>Please confirm plugging the rail embed drain hole as shown in 6/A1-9306 is acceptable.</p>					
	<p>Please confirm material furnished and installed under Specification Sections 07 81 00 and 07 81 23 are not required to have engineering as required by Specification Section 01 80 50. If engineering is required for items furnished and installed under Specification Sections 07 81 00 and 07 81 23, please specifically identify which elements are to be engineered.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Detail 6/A1-9306 (ASI 119 dated 6/18/14)</p> <p>Per Detail 6/A1-9306, the HD galvanized rail embed shows a drain hole in the bottom of the embed. The detail goes on to show the bottom of the embed seated in grout which will prevent water from draining out of the drain hole.</p> <p>Please confirm plugging the rail embed drain hole as shown in 6/A1-9306 is acceptable.</p>					

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<b>P1-0293</b>	<b>Bus Bridge Crash Rail Material Confirmation</b>	<b>Closed</b>	<b>0P</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>08/20/2014</b>
<div> <div> <b>From:</b> Webcor Construction LP           Tram Nguyen         </div> <div> <b>REQUEST:</b> <p>REFERENCE: Detail 2/A1-8378 (ASI 119 dated 6/18/14)</p> <p>Per the current Bus Bridge Contract Documents, the Bus Bridge Crash Rail is metal as it approaches the Bus Deck.</p> <p>Per Detail 2/A1-8378 of ASI 119, the Bus Bridge Crash Rail is changing to match the concrete crash rail of the Bus Deck.</p> <p>Please confirm the Bus Bridge Crash Rail design, and coordinate drawings to match.</p> </div> <div> <b>ANSWER:</b> <p>REFERENCE: Detail 2/A1-8378 (ASI 119 dated 6/18/14)</p> <p>Per the current Bus Bridge Contract Documents, the Bus Bridge Crash Rail is metal as it approaches the Bus Deck.</p> <p>Per Detail 2/A1-8378 of ASI 119, the Bus Bridge Crash Rail is changing to match the concrete crash rail of the Bus Deck.</p> <p>Please confirm the Bus Bridge Crash Rail design, and coordinate drawings to match.</p> </div> </div>						



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<b>P1-0294</b>	<b>Method of Construction for Concrete Column Steel Jacket Base</b>	<b>Closed</b>	<b>0P</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>08/20/2014</b>
<div> <div> <b>From:</b> Webcor Construction LP           Tram Nguyen         </div> <div> <b>REQUEST:</b> <p>REFERENCE: Detail 1/A1-9208 (ASI 118 dated 6/20/14), Detail 6/S1-3503 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Per Detail 1/A1-9208 of ASI 118, the concrete column steel jacket base are welded studs/post installed expansion anchors, and to refer to structural for sizing.</p> <p>Per Detail 6/S1-3503, welded studs are to be used at the lower concourse, post-installed anchors are to be used at the mat slab (train box), and jacket base plate is poured integrally to the slab whether welded studs or post-installed anchors are used.</p> <p>Based upon the current construction and bid schedules, steel jackets will not be procured prior to pouring mat slab and lower concourse deck.</p> <p>Please confirm post applied anchors can be furnished and installed at all locations column steel jacketing is required, and that the jacket base plate can be placed on top of the slab/deck where the slab/deck has been poured prior to column steel jacket installation.</p> </div> <div> <b>ANSWER:</b> <p>REFERENCE: Detail 1/A1-9208 (ASI 118 dated 6/20/14), Detail 6/S1-3503 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Per Detail 1/A1-9208 of ASI 118, the concrete column steel jacket base are welded studs/post installed expansion anchors, and to refer to structural for sizing.</p> <p>Per Detail 6/S1-3503, welded studs are to be used at the lower concourse, post-installed anchors are to be used at the mat slab (train box), and jacket base plate is poured integrally to the slab whether welded studs or post-installed anchors are used.</p> <p>Based upon the current construction and bid schedules, steel jackets will not be procured prior to pouring mat slab and lower concourse deck.</p> <p>Please confirm post applied anchors can be furnished and installed at all locations column steel jacketing is required, and that the jacket base plate can be placed on top of the slab/deck where the slab/deck has been poured prior to column steel jacket installation.</p> </div> </div>						



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<b>P1-0295</b>	<b>Thickness of AESS Closure Elements</b>	<b>Closed</b>	<b>0P</b>	<b>07/21/2014</b>	<b>07/31/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Sheet A1-8690 Sheets A1-8692 through A1-8695  Sheet A1-8690 and Sheets A1-8692 through A1-8695 call out for AESS closure elements to be welded per structural, and finished per the AESS specification.  However, the details for the AESS closure elements do not call out the thickness of the material.  Please provide the thickness of the AESS closure element material.						<b>ANSWER:</b>  REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Sheet A1-8690 Sheets A1-8692 through A1-8695  Sheet A1-8690 and Sheets A1-8692 through A1-8695 call out for AESS closure elements to be welded per structural, and finished per the AESS specification.  However, the details for the AESS closure elements do not call out the thickness of the material.  Please provide the thickness of the AESS closure element material.
<b>P1-0296</b>	<b>Utilization of Sheet A1-8378 for Joint Type FJC7</b>	<b>Closed</b>	<b>0P</b>	<b>07/23/2014</b>	<b>08/02/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0224, Sheet A1-8378 (IFC Drawings for Main Package dated 3/31/14)  Per RFI Response P1-0224, details for FJC7 are located on Sheet A1-8378 ¿ Formed Aluminum Panel System (W-9) Sheet Typical Details. The answer to RFI P1-224 was then voided.  The plans do not provide a path to direct expansion joint bidders to this sheet (no detail references are given on the plan sheets, and FJC7 is not part of the W-9 system).  Please confirm details on Sheet A1-8378 are to be used in relation to bidding, furnishing, and installing FJC7.						<b>ANSWER:</b>  REFERENCE: RFI Response P1-0224, Sheet A1-8378 (IFC Drawings for Main Package dated 3/31/14)  Per RFI Response P1-0224, details for FJC7 are located on Sheet A1-8378 ¿ Formed Aluminum Panel System (W-9) Sheet Typical Details. The answer to RFI P1-224 was then voided.  The plans do not provide a path to direct expansion joint bidders to this sheet (no detail references are given on the plan sheets, and FJC7 is not part of the W-9 system).  Please confirm details on Sheet A1-8378 are to be used in relation to bidding, furnishing, and installing FJC7.
<b>P1-0297</b>	<b>Liquidated Damages</b>	<b>Closed</b>	<b>0P</b>	<b>07/23/2014</b>	<b>08/02/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>						<b>ANSWER:</b>



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Reference: Specification 34 41 13 Paragraph 3.9.E

Liquidated Damages are mentioned within Specification 34 41 13 para. 3.9.E. and should be deleted. If LD's are to be incurred they should be addressed within the contract terms under Division 00/01 specifications. Please advise.

Reference: Specification 34 41 13 Paragraph 3.9.E

Liquidated Damages are mentioned within Specification 34 41 13 para. 3.9.E. and should be deleted. If LD's are to be incurred they should be addressed within the contract terms under Division 00/01 specifications. Please advise.





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P1-0298	Corrosion Engineer	Closed	0P	07/23/2014	08/02/2014	08/05/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:		ANSWER:				
Reference: Specification 05 12 13 1.7 I (IFC Drawings for Main Package dated 3/31/14) Specification 05 12 13 2.4 C (IFC Drawings for Main Package dated 3/31/14)  Specification Section 05 12 13 1.7 I states, "Engage a California-licensed Corrosion Engineer who is an expert in corrosion, to conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of curtain wall and aluminum panels and provide report."  Specification Section 05 12 13 2.4 C states, "Comply with recommendations of the corrosion engineer approved by the TPJA Representative, as specified above."  As agreed upon between TJPA, PMPC, and Webcor- Obayashi, the language "Conduct a component-by- component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report" will be used in lieu of the language in Specification Sections 05 12 13 1.7 I and 05 12 13 2.4 C  Please confirm that the language, "Conduct a component- by-component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report." is to be used in lieu of the language currently required by 05 12 13 1.7 I and 05 12 13 2.4 C.		Reference: Specification 05 12 13 1.7 I (IFC Drawings for Main Package dated 3/31/14) Specification 05 12 13 2.4 C (IFC Drawings for Main Package dated 3/31/14)  Specification Section 05 12 13 1.7 I states, "Engage a California-licensed Corrosion Engineer who is an expert in corrosion, to conduct a component-by- component analysis of potential corrosion resulting from galvanic action between materials, for components of curtain wall and aluminum panels and provide report."  Specification Section 05 12 13 2.4 C states, "Comply with recommendations of the corrosion engineer approved by the TPJA Representative, as specified above."  As agreed upon between TJPA, PMPC, and Webcor- Obayashi, the language "Conduct a component-by- component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report" will be used in lieu of the language in Specification Sections 05 12 13 1.7 I and 05 12 13 2.4 C  Please confirm that the language, "Conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report." is to be used in lieu of the language currently required by 05 12 13 1.7 I and 05 12 13 2.4 C.				



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<b>P1-0299</b>	<b>Waterproofing at Escalator Closure per Detail 3/A1-7552</b>	<b>Closed</b>	<b>0P</b>	<b>07/23/2014</b>	<b>08/02/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Detail 3/A1-7552 (IFC Drawings for Main Package dated 3/31/14), RFI Response P1-0089						REFERENCE: Detail 3/A1-7552 (IFC Drawings for Main Package dated 3/31/14), RFI Response P1-0089
Per Detail 3/A1-7552, WPM-6 is to be installed at the escalator closure at the glass guardrail.						Per Detail 3/A1-7552, WPM-6 is to be installed at the escalator closure at the glass guardrail.
- Per RFI Response P1-0089, WPM-6 has been removed - The area between the sheathing and escalator as depicted in Detail 3/A1-7522 is inaccessible for waterproofing						- Per RFI Response P1-0089, WPM-6 has been removed - The area between the sheathing and escalator as depicted in Detail 3/A1-7522 is inaccessible for waterproofing
Please confirm that WPM-6 will be removed from the location referenced in Detail 3/A1-7552.						Please confirm that WPM-6 will be removed from the location referenced in Detail 3/A1-7552.
<b>P1-0300</b>	<b>Design Requirements for Galvanized Metal Plate per Detail 3/A1-8894</b>	<b>Closed</b>	<b>0P</b>	<b>07/23/2014</b>	<b>07/23/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Detail 3/A1-8894 (ASI 119 dated 6/18/14)						REFERENCE: Detail 3/A1-8894 (ASI 119 dated 6/18/14)
Per Detail 3/A1-8894 of ASI 119 there is a "galv. metal plate covering fireproofed beam" which is not detailed within the documents (it is unknown if it is required for a seismic joint assembly).						Per Detail 3/A1-8894 of ASI 119 there is a "galv. metal plate covering fireproofed beam" which is not detailed within the documents (it is unknown if it is required for a seismic joint assembly).
Please provide the design requirements (size, extent, attachment, fireproofing, etc.) for the galvanized metal plate covering fireproofed structural beam depicted on Detail 3/A1-8894.						Please provide the design requirements (size, extent, attachment, fireproofing, etc.) for the galvanized metal plate covering fireproofed structural beam depicted on Detail 3/A1-8894.
<b>P1-0301</b>	<b>BOL-3 Conflict wth spec</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP                      Zachary Moore						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: ASI 119-A1-7027 (SKA-3495), 28 16 44						Reference: ASI 119-A1-7027 (SKA-3495), 28 16 44
Detail 6, 7 and 8 indicate that BOL-3 is ""6"" o x 1/4"" THK HD GALV STL CONC FILLED PIPE TYP"". This is in conflict with what is specified for BOL-3 in 28 16 44.						Detail 6, 7 and 8 indicate that BOL-3 is ""6"" o x 1/4"" THK HD GALV STL CONC FILLED PIPE TYP"". This is in conflict with what is specified for BOL-3 in 28 16



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	Clarify which is meant to be correct.					44. Clarify which is meant to be correct.
<b>P1-0302</b>	<b>Conflicting Detail with Description</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: ASI 119 and IFC L1-7360 and 28 16 44  Details on L1-7360 are in conflict with the outlined bollard descriptions, including but not limited to, presence of sleeve, diameter of sleeve, diameter of bollard. Clarify which is meant to be correct.						<b>ANSWER:</b> reference: ASI 119 and IFC L1-7360 and 28 16 44  Details on L1-7360 are in conflict with the outlined bollard descriptions, including but not limited to, presence of sleeve, diameter of sleeve, diameter of bollard. Clarify which is meant to be correct.
<b>P1-0303</b>	<b>Bollard Type Depicted</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: ASI 119 and IFC - L1-7360 Clarify which bollard type is depicted in the details on L1-7360. There is no type indicated.						<b>ANSWER:</b> reference: ASI 119 and IFC - L1-7360 Clarify which bollard type is depicted in the details on L1-7360. There is no type indicated.
<b>P1-0304</b>	<b>Leed Requirments</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: 28 16 44 Confirm there are no LEED requirements for 28 16 44, such as VOC requirements for example.						<b>ANSWER:</b> reference: 28 16 44 Confirm there are no LEED requirements for 28 16 44, such as VOC requirements for example.
<b>P1-0305</b>	<b>Bollard "Special Footing"</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: L-0002, 28 16 44, 28 16 44/APA						<b>ANSWER:</b> reference: L-0002, 28 16 44, 28 16 44/APA



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	<p>On Legend L-0002 a ""Special Footing"" is called out for Bollard types 1A, 1B, 1C, 2A, 2B, 2C, and 3A. Please provide specifications and details as for each ""special footing"" required on each bollard type. No detail about ""special footing"" is provided in spec 28 16 44 or 28 16 44/APA.</p>					<p>On Legend L-0002 a ""Special Footing"" is called out for Bollard types 1A, 1B, 1C, 2A, 2B, 2C, and 3A. Please provide specifications and details as for each ""special footing"" required on each bollard type. No detail about ""special footing"" is provided in spec 28 16 44 or 28 16 44/APA.</p>
<b>P1-0306</b>	<b>Maintenance Requirement</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: 28 16 44 Section 3.7 - Confirm TJPA wants 5 years of maintenance included in the bid for TG07.9 Bollards and Barriers.						<b>ANSWER:</b> reference: 28 16 44 Section 3.7 - Confirm TJPA wants 5 years of maintenance included in the bid for TG07.9 Bollards and Barriers.
<b>P1-0307</b>	<b>Seismic Design Criteria For Bollards</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: 01 80 50 Confirm that spec section 01 80 50 Seismic Design Criteria for Nonstructural Components does not apply to TG07.9 Bollards and Barriers since it is not referenced in 28 16 44.						<b>ANSWER:</b> reference: 01 80 50 Confirm that spec section 01 80 50 Seismic Design Criteria for Nonstructural Components does not apply to TG07.9 Bollards and Barriers since it is not referenced in 28 16 44.





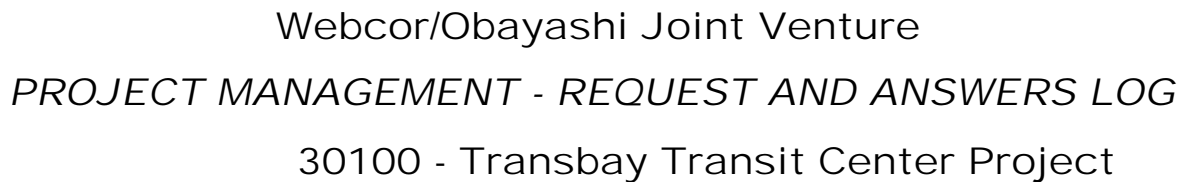
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	<p>reference: 28 16 44 - section 2.6.C Clarify and provide details for location of Probe Detectors. Clarify if the probe detectors are for the wedge barriers or retractable bollards. Provide details, specifications, operation and output requirements for the probe detectors.</p>					<p>reference: 28 16 44 - section 2.6.C Clarify and provide details for location of Probe Detectors. Clarify if the probe detectors are for the wedge barriers or retractable bollards. Provide details, specifications, operation and output requirements for the probe detectors.</p>
<b>P1-0312</b>	<b>System integration Requirements</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b>  28 16 44 - section 3.8.D ""All software and IP devices of this system shall be integrated as a part of and shall be fully compatible with operating in the Converged IP-based Network to be installed at the Transbay Transit Center."" Confirm that all requirements to comply with the future system integration are stated in this section.						<b>ANSWER:</b>  28 16 44 - section 3.8.D ""All software and IP devices of this system shall be integrated as a part of and shall be fully compatible with operating in the Converged IP-based Network to be installed at the Transbay Transit Center."" Confirm that all requirements to comply with the future system integration are stated in this section.
<b>P1-0313</b>	<b>ASI 118 and 119 Discrepancies</b>	<b>Closed</b>	<b>0P</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b>  See attached sheets, A1-7416, A1-7416 (SKA-3524), A1-7418, A1-7418 (SKA-3526), A1-7419, A1-7419 (SKA-3527), A1-7420, A1-7420 (SKA-3528), A1-7421, A1-7421 (SKA-3529), A1-7422, A1-7422 (SKA-3530), A1-7423, A1-7423 (SKA-3531), A1-7424, A1-7424 (SKA-3532), A1-8720, A1-8720 (SKA-3542), A1-8721, A1-8721 (SKA-3543). There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for these sheets. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.						<b>ANSWER:</b>  See attached sheets, A1-7416, A1-7416 (SKA-3524), A1-7418, A1-7418 (SKA-3526), A1-7419, A1-7419 (SKA-3527), A1-7420, A1-7420 (SKA-3528), A1-7421, A1-7421 (SKA-3529), A1-7422, A1-7422 (SKA-3530), A1-7423, A1-7423 (SKA-3531), A1-7424, A1-7424 (SKA-3532), A1-8720, A1-8720 (SKA-3542), A1-8721, A1-8721 (SKA-3543). There are discrepancies between ASI 118 (Addendum #4) and ASI 119 for these sheets. ASI 118 and 119 are both dated 06/20/2014. Confirm that ASI 119 is meant to take precedence. It does not appear to be the most current version.
<b>P1-0314</b>	<b>Lithocrete Sole Source</b>	<b>Closed</b>	<b>0P</b>	<b>07/28/2014</b>	<b>07/28/2014</b>	<b>08/21/2014</b>



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<p><b>From:</b> Webcor Construction LP                      Zachary Moore</p> <p><b>REQUEST:</b></p> <p>reference: 03 33 13 1.1 A  Specification section 03 33 13 1.1 A - requires the contractor to sole source Lithocrete a product that is protected by various US Patents  a.     Provide performance criteria for the design basis along with acceptable alternatives</p>						<p><b>ANSWER:</b></p> <p>reference: 03 33 13 1.1 A  Specification section 03 33 13 1.1 A - requires the contractor to sole source Lithocrete a product that is protected by various US Patents  a.     Provide performance criteria for the design basis along with acceptable alternatives</p>
<b>P1-0315</b>	<b>Structural Information on OCS Trough Support</b>	<b>Closed</b>	<b>0P</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>08/05/2014</b>
<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Detail 3/A1-8551 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Detail 3/A1-8551 indicates that information on the OCS Trough Support is located on structural drawings.</p> <p>S1-9010 does not provide information on this member.</p> <p>Please provide structural information on the OCS Trough Support.</p>						<p><b>ANSWER:</b></p> <p>REFERENCE: Detail 3/A1-8551 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Detail 3/A1-8551 indicates that information on the OCS Trough Support is located on structural drawings.</p> <p>S1-9010 does not provide information on this member.</p> <p>Please provide structural information on the OCS Trough Support.</p>



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<b>P1-0315.1</b>	<b>Design Requirements for OCS Trough Support</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: RFI Response P1-0315  Per RFI Response P1-0315, "The structural drawings provide information for the OCS HSS vertical supports. Detail 3/A1-8551 shows a suggested field method of fastening the OCS assembly to the vertical HSS structural supports by way of an HSS sleeve. Contractor may modify this detail as they see fit to install OCS assembly with coordination with SFMTA. Moreover, RFI T-1417.3 response has already recommended the CM/GC schedule a meeting with all the stake holders to resolve all OCS questions."  The above response does not provide design requirements for the OCS Trough Support.  Please provide the design requirements for the OCS Trough Support to be administered to the Bidders.						
						<b>ANSWER:</b>  REFERENCE: RFI Response P1-0315  Per RFI Response P1-0315, "The structural drawings provide information for the OCS HSS vertical supports. Detail 3/A1-8551 shows a suggested field method of fastening the OCS assembly to the vertical HSS structural supports by way of an HSS sleeve. Contractor may modify this detail as they see fit to install OCS assembly with coordination with SFMTA. Moreover, RFI T-1417.3 response has already recommended the CM/GC schedule a meeting with all the stake holders to resolve all OCS questions."  The above response does not provide design requirements for the OCS Trough Support.  Please provide the design requirements for the OCS Trough Support to be administered to the Bidders.
<b>P1-0316</b>	<b>SFMTA Design &amp; Construction Documents for Muni Bus Plaza Ceiling &amp; OCS System</b>	<b>Closed</b>	<b>0P</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>09/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Description 3 A of Muni Bus Plaza Ceiling and OCS System Description/A1-8551 (IFC Drawings for Main Package dated 3/31/14)  Muni Bus Plaza Ceiling and OCS System Description 3 A on Sheet A1-8551 states, "Refer to SFMTA design and construction documents for work all OCS assembly components and installation."  No SFMTA design and construction documents have been provided.  Please provide the referenced SFMTA design and construction documents.						
						<b>ANSWER:</b>  REFERENCE: Description 3 A of Muni Bus Plaza Ceiling and OCS System Description/A1-8551 (IFC Drawings for Main Package dated 3/31/14)  Muni Bus Plaza Ceiling and OCS System Description 3 A on Sheet A1-8551 states, "Refer to SFMTA design and construction documents for work all OCS assembly components and installation."  No SFMTA design and construction documents have been provided.  Please provide the referenced SFMTA design and construction documents.
<b>P1-0317</b>	<b>OV Series Sheets for OCS System</b>	<b>Closed</b>	<b>0P</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>09/12/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						





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	<p><b>REQUEST:</b></p> <p>REFERENCE: Sheets A1-8550 through A1-8552 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Sheets A1-8550 through A1-8552 indicate that coordination of the OCS system must be performed with the 2OV Series2 sheets (see 4/A1-8551 for an example).</p> <p>OV sheets have not been issued.</p> <p>Please provide the OV sheets.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Sheets A1-8550 through A1-8552 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Sheets A1-8550 through A1-8552 indicate that coordination of the OCS system must be performed with the 2OV Series2 sheets (see 4/A1-8551 for an example).</p> <p>OV sheets have not been issued.</p> <p>Please provide the OV sheets.</p>					
<b>P1-0318</b>	<b>Part B Documentation per Specification Section 07 81 00</b>	<b>Closed</b>	<b>0P</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>08/20/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Specification Section 07 81 00 1.1 B (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 07 81 00 1.1 B states, "Refer to Part B Documents applicable to the Section."</p> <p>Part B Documents are not included in this specification section.</p> <p>Please provide the "Part B Documents" referenced in Specification Section 07 81 00 1.1 B.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Specification Section 07 81 00 1.1 B (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 07 81 00 1.1 B states, "Refer to Part B Documents applicable to the Section."</p> <p>Part B Documents are not included in this specification section.</p> <p>Please provide the "Part B Documents" referenced in Specification Section 07 81 00 1.1 B.</p>					



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<b>P1-0319</b>	<b>AESS Category for Light Columns</b>	<b>Closed</b>	<b>0P</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: AESS Matrix/A1-8660, Detail E/A1-8660 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: AESS Matrix/A1-8660, Detail E/A1-8660 (IFC Drawings for Main Package dated 3/31/14)				
The AESS Matrix on Sheet A1-8660 calls for AESS Category 1 finish on the light column.		The AESS Matrix on Sheet A1-8660 calls for AESS Category 1 finish on the light column.				
However, Detail E/A1-8660 calls for AESS Category 2 finish on the upper portion of the light column.		However, Detail E/A1-8660 calls for AESS Category 2 finish on the upper portion of the light column.				
Please confirm the desired AESS Category requirements for the light column.		Please confirm the desired AESS Category requirements for the light column.				
<b>P1-0320</b>	<b>Lithocrete Alternatives</b>	<b>Closed</b>	<b>0P</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Specification section 03 33 13 1.2 (4) - lists US Patents that must be complied with that are restricted to installation of the Lithocrete.		Specification section 03 33 13 1.2 (4) - lists US Patents that must be complied with that are restricted to installation of the Lithocrete.				
a. Provide performance criteria that will be acceptable along with a listing of alternatives to the US Patents		a. Provide performance criteria that will be acceptable along with a listing of alternatives to the US Patents				
<b>P1-0321</b>	<b>Testing Requirments</b>	<b>Closed</b>	<b>0P</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Specification Section 03 33 13 1.3 C Testing:		Specification Section 03 33 13 1.3 C Testing:				
a. Confirm that Trade Group will provide all on-site testing, method of procedure for testing submittals, and all reports		a. Confirm that Trade Group will provide all on-site testing, method of procedure for testing submittals, and all reports				
<b>P1-0322</b>	<b>Acceptable Manufactures</b>	<b>Closed</b>	<b>0P</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Specification section 03 33 13 1.5 - Manufactures,		Specification section 03 33 13 1.5 - Manufactures,				





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<b>P1-0325</b>	<b>Lithocrete Surface Seeded Aggregate</b>	<b>Closed</b>	<b>0P</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Specification Section 03 33 13 2.5 C Lithocrete Surface Seeded Aggregate a. Provide the Artist's sample # 07-133J for Trade Group to match b. Provide an acceptable list of samples and/or alternative to Lithocrete samples		<b>ANSWER:</b> Specification Section 03 33 13 2.5 C Lithocrete Surface Seeded Aggregate a. Provide the Artist's sample # 07-133J for Trade Group to match b. Provide an acceptable list of samples and/or alternative to Lithocrete samples				
<b>P1-0326</b>	<b>Integral concrete coloring</b>	<b>Closed</b>	<b>0P</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Specification Section 03 33 13 2.7 A Integral Concrete Coloring a. Provide color that is referenced - drawings do not contain a specified color callout for Shaw Alley		<b>ANSWER:</b> Specification Section 03 33 13 2.7 A Integral Concrete Coloring a. Provide color that is referenced - drawings do not contain a specified color callout for Shaw Alley				
<b>P1-0327</b>	<b>Lithocrete Surface Seeded Aggregate Alternatives</b>	<b>Closed</b>	<b>0P</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Specification Section 03 33 13 3.4 Lithocrete Surface Seeded Aggregate Installation a. Provide a list of acceptable alternatives that are not single source protected by US Patents b. Provide a list of acceptable manufactures that are not single source protected by US Patents c. Provide a list of acceptable products that are not single source protected by US Patents		<b>ANSWER:</b> Specification Section 03 33 13 3.4 Lithocrete Surface Seeded Aggregate Installation a. Provide a list of acceptable alternatives that are not single source protected by US Patents b. Provide a list of acceptable manufactures that are not single source protected by US Patents c. Provide a list of acceptable products that are not single source protected by US Patents				
<b>P1-0328</b>	<b>Curing Product Alternatives</b>	<b>Closed</b>	<b>0P</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Specification Section 03 33 13 3.6 - Curing a. Provide criteria for curing for acceptable products in		<b>ANSWER:</b> Specification Section 03 33 13 3.6 - Curing a. Provide criteria for curing for acceptable products in				



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	addition to Lithocrete ®					addition to Lithocrete ®
<b>P1-0329</b>	<b>Sealing Alternatives</b>	<b>Closed</b>	<b>0P</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Specification Section 03 33 13 3.7 - Sealing a. Provide a list of acceptable alternative products that are not single source protected by US Patents b. Provide a criteria and acceptable means of application that is not single source protected by US Patents						<b>ANSWER:</b> Specification Section 03 33 13 3.7 - Sealing a. Provide a list of acceptable alternative products that are not single source protected by US Patents b. Provide a criteria and acceptable means of application that is not single source protected by US Patents
<b>P1-0330</b>	<b>Waterproofing System with Skylight</b>	<b>Closed</b>	<b>0P</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> Reference A1-8404D ASI 120 Reference 08 63 03 ASI 120  A1-8404D shows a WPM-3 waterproofing system around the perimeter of the W-10 skylight systems at GL 11 and 28. Specification 08 63 03 gives requirements for a WMP-10 type waterproofing system. As the skylights are on the roof and will need to tie into the roof waterproofing system confirm which type of waterproofing to be provided. This needs to be answered immediately.						<b>ANSWER:</b> Reference A1-8404D ASI 120 Reference 08 63 03 ASI 120  A1-8404D shows a WPM-3 waterproofing system around the perimeter of the W-10 skylight systems at GL 11 and 28. Specification 08 63 03 gives requirements for a WMP-10 type waterproofing system. As the skylights are on the roof and will need to tie into the roof waterproofing system confirm which type of waterproofing to be provided. This needs to be answered immediately.



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P1-0331	Door Type Clarification	Closed	0P	07/29/2014	08/08/2014	08/21/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:		ANSWER:				
Reference: A1-3100 & A1-3100 (SKA-3670) in ASI 119		Reference: A1-3100 & A1-3100 (SKA-3670) in ASI 119				
Both A1-3100 and A1-3100 (SKA-3670) were issued in ASI 119. A1-3100 (SKA-3670) shows a W-2 overhead coiling door while A1-3100 shows a prefinished aluminum overhead coiling door on Grid Line C between 5 and 6. Please clarify what type of door is to be furnish and installed at this location.		Both A1-3100 and A1-3100 (SKA-3670) were issued in ASI 119. A1-3100 (SKA-3670) shows a W-2 overhead coiling door while A1-3100 shows a prefinished aluminum overhead coiling door on Grid Line C between 5 and 6. Please clarify what type of door is to be furnish and installed at this location.				
P1-0332	Prefinished Aluminum Overhead Door Detail	Closed	0P	07/29/2014	08/08/2014	08/05/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:		ANSWER:				
Reference: A1-3100, A1-8153 & A1-8180 in ASI 119		Reference: A1-3100, A1-8153 & A1-8180 in ASI 119				
A1-3100 calls for a prefinished aluminum overhead coiling door, and refers to 3/A1-8153 for details. Detail 3/A1-8153 references A1-8180, which shows an overhead glass folding door. Please revise details to show requirements for the prefinished aluminum overhead coiling door.		A1-3100 calls for a prefinished aluminum overhead coiling door, and refers to 3/A1-8153 for details. Detail 3/A1-8153 references A1-8180, which shows an overhead glass folding door. Please revise details to show requirements for the prefinished aluminum overhead coiling door.				
P1-0333	AS-Built	Closed	0P	07/29/2014	07/29/2014	08/21/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:		ANSWER:				
Reference 08 44 33		Reference 08 44 33				
Section 1.1 24.j calls to strike requirement for AS-Built drawings. However section 1.14 states that this specification is to comply with Article 3.09 of the General Conditions and Sections 01 17 20 apply to this section. Confirm whether AS-Built drawings are required for this specification.		Section 1.1 24.j calls to strike requirement for AS-Built drawings. However section 1.14 states that this specification is to comply with Article 3.09 of the General Conditions and Sections 01 17 20 apply to this section. Confirm whether AS-Built drawings are required for this specification.				



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P1-0334	Installer Experience	Closed	0P	07/29/2014	08/08/2014	08/21/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:						ANSWER:
Reference 08 44 33 Section 1.09 F 4 and 5 references installers experience. Confirm "companies" will be used in lieu of "installers" agreed upon between TJPA, PMPC, and Webcor-Obayashi.						Reference 08 44 33 Section 1.09 F 4 and 5 references installers experience. Confirm "companies" will be used in lieu of "installers" agreed upon between TJPA, PMPC, and Webcor-Obayashi.
P1-0335	Confirmation for Corrosion Expert Requirement	Closed	0P	07/29/2014	08/08/2014	10/21/2014
From: Webcor Construction LP                      Zachary Moore						
REQUEST:						ANSWER:
Reference 08 44 23 Specification Section 08 44 23 2.11 C states, "Comply with recommendations of the corrosion engineer approved by the TPJA Representative, as specified above."  As agreed upon between TJPA, PMPC, and Webcor-Obayashi, the language "Conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report" will be used in lieu of the language in Specification Sections 08 44 23 2.11 C  Please confirm that the language, "Conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report." is to be used in lieu of the language currently required by 08 44 23 2.11 C.						Reference 08 44 23 Specification Section 08 44 23 2.11 C states, "Comply with recommendations of the corrosion engineer approved by the TPJA Representative, as specified above."  As agreed upon between TJPA, PMPC, and Webcor-Obayashi, the language "Conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report" will be used in lieu of the language in Specification Sections 08 44 23 2.11 C  Please confirm that the language, "Conduct a component-by-component analysis of potential corrosion resulting from galvanic action between materials, for components of the work of this section and provide report." is to be used in lieu of the language currently required by 08 44 23 2.11 C.
P1-0336	Detail Reference per Detail 9/A1-9580	Closed	0P	07/31/2014	08/10/2014	08/21/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:						ANSWER:
REFERENCE: Detail 9/A1-9580, Detail 7/A1-9321 (IFC Drawings for Main Package dated 3/31/14)  Detail 9/A1-9580 calls out Detail 7/A1-9321.						REFERENCE: Detail 9/A1-9580, Detail 7/A1-9321 (IFC Drawings for Main Package dated 3/31/14)  Detail 9/A1-9580 calls out Detail 7/A1-9321.



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	<p>Detail 7/A1-9321 is not shown on Sheet A1-9321.</p> <p>Please provide the correct call out for the on Detail 9/A1-9580.</p>					<p>Detail 7/A1-9321 is not shown on Sheet A1-9321.</p> <p>Please provide the correct call out for the on Detail 9/A1-9580.</p>
P1-0337	<b>Detail Reference for Typical Driveway Aisle Assembly</b>  <b>From:</b> Webcor Construction LP                      Tram Nguyen	Closed	0P	07/31/2014	08/10/2014	08/21/2014
	<b>REQUEST:</b>  REFERENCE: Detail 8/A1-9585, Detail 1/A1-9585 (IFC Drawings for Main Package dated 3/31/14)  Detail 8/A1-9585 references Detail 1/A1-9585 for the typical driveway aisle assembly.  Detail 1/A1-9585 is a section at terrazzo floor with radiant heating (Grand Hall).  Please provide the correct call out for the driveway aisle assembly on Detail 8/A1-9585.					<b>ANSWER:</b>  REFERENCE: Detail 8/A1-9585, Detail 1/A1-9585 (IFC Drawings for Main Package dated 3/31/14)  Detail 8/A1-9585 references Detail 1/A1-9585 for the typical driveway aisle assembly.  Detail 1/A1-9585 is a section at terrazzo floor with radiant heating (Grand Hall).  Please provide the correct call out for the driveway aisle assembly on Detail 8/A1-9585.
P1-0338	<b>Roof Park Building Protection Slab and Waterproofing Details</b>  <b>From:</b> Webcor Construction LP                      Tram Nguyen	Closed	0P	07/31/2014	08/10/2014	09/11/2014
	<b>REQUEST:</b>  REFERENCE: A1-2603 (ASI 122 dated 7/23/14)  Sheet A1-2603 shows a Roof Park Building located at GL 6 between GL D and GL G.  Sheet A1-2913 shows no protection slab in this area and details for waterproofing are not provided for this area.  Please confirm this area is not to receive a protection slab and provide the waterproofing details for this area.					<b>ANSWER:</b>  REFERENCE: A1-2603 (ASI 122 dated 7/23/14)  Sheet A1-2603 shows a Roof Park Building located at GL 6 between GL D and GL G.  Sheet A1-2913 shows no protection slab in this area and details for waterproofing are not provided for this area.  Please confirm this area is not to receive a protection slab and provide the waterproofing details for this area.
P1-0339	<b>Base Bid Item Confirmation for Alternate No. 27 - Delete Roof Park Level Café</b>	Closed	0P	07/31/2014	08/10/2014	08/21/2014





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<div><div><p><b>From:</b> Webcor Construction LP      Tram Nguyen</p><p><b>REQUEST:</b></p><p>REFERENCE: Specification Section 01 10 30 APE E.1.15, Sheet A1-2605 (ASI 122 dated 7/23/14)</p><p>Specification Section 01 10 30APE E.1.15 - Alternate No. 27 states, "Provide lid over structural foundation complete with waterproof membrane." then references Sheet A1-2605.</p><p>Sheet A1-2605 of ASI 122 notes, "Defer W-20 glass cafe. Retain circular foundation wall with topping slab. All utilities should be stubbed up below the topping slab."</p><p>In accordance with Sheet A1-2605 of ASI 112, please confirm that Specification Section 01 10 30APE E.1.15 - Alternate No. 27 is accepted as a base bid item.</p></div><div><p><b>ANSWER:</b></p><p>REFERENCE: Specification Section 01 10 30 APE E.1.15, Sheet A1-2605 (ASI 122 dated 7/23/14)</p><p>Specification Section 01 10 30APE E.1.15 - Alternate No. 27 states, "Provide lid over structural foundation complete with waterproof membrane." then references Sheet A1-2605.</p><p>Sheet A1-2605 of ASI 122 notes, "Defer W-20 glass cafe. Retain circular foundation wall with topping slab. All utilities should be stubbed up below the topping slab."</p><p>In accordance with Sheet A1-2605 of ASI 112, please confirm that Specification Section 01 10 30APE E.1.15 - Alternate No. 27 is accepted as a base bid item.</p></div></div>						
<b>P1-0340</b>	<b>Waterproofing Membrane for Roof Park Level per Alternate No. 27</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/21/2014</b>
<div><div><p><b>From:</b> Webcor Construction LP      Tram Nguyen</p><p><b>REQUEST:</b></p><p>REFERENCE: Specification Section 01 10 30 APE E.1.15</p><p>Specification Section 01 10 30APE E.1.15 - Alternate No. 27 states, "Provide lid over structural foundation complete with waterproof membrane. Extend main plaza paving system over café foundation with required substrate as indicated on Landscape drawings."</p><p>Should Alternate No. 27 be accepted as a base bid item for the Roof Park Level, the waterproofing membrane specified (Sika Sarnafil PVC G476-20) is not UV rated and per the manufacturer cannot be exposed for more than 3 months.</p><p>Please confirm that the waterproofing membrane specified above is to be used at the Roof Park Level per Alternate No. 27.</p></div><div><p><b>ANSWER:</b></p><p>REFERENCE: Specification Section 01 10 30 APE E.1.15</p><p>Specification Section 01 10 30APE E.1.15 - Alternate No. 27 states, "Provide lid over structural foundation complete with waterproof membrane. Extend main plaza paving system over café foundation with required substrate as indicated on Landscape drawings."</p><p>Should Alternate No. 27 be accepted as a base bid item for the Roof Park Level, the waterproofing membrane specified (Sika Sarnafil PVC G476-20) is not UV rated and per the manufacturer cannot be exposed for more than 3 months.</p><p>Please confirm that the waterproofing membrane specified above is to be used at the Roof Park Level per Alternate No. 27.</p></div></div>						
<b>P1-0341</b>	<b>Manhole Access Openings in Future Café PK560</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>09/04/2014</b>



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	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div></div> <div><div>REQUEST:</div><div>REFERENCE: Details 1 and A/A1-8630A (IFC Drawings for Main Package dated 3/31/14)</div><div>Detail 1/A1-2605 depicts two 30" diameter manhole access openings with steel plate covers to be installed in Future Café PK560. The manhole access openings will create access points for water to enter the crawl space depicted in Detail A/A1-2605.</div><div>The crawl space beneath Future Cafe PK560 illustrated in Detail A/A1-2605 is not equipped with drainage or waterproofing to combat water entry.</div><div>Please confirm that this is acceptable or provide waterproofing details for the crawl space beneath Future Cafe PK560.</div></div>					
P1-0342	Concrete Slab and Adjacent Wall Connection Details	Closed	0P	07/31/2014	08/10/2014	08/21/2014
	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div></div> <div><div>REQUEST:</div><div>REFERENCE: Detail A/A1-8630A, Detail 1/S1-3281A (IFC Drawings for Main Package dated 3/31/14)</div><div>Detail A/A1-8630A depicts the concrete slab in Future Café PK560 to be level with the top of the adjacent concrete wall.</div><div>Detail 1/S1-3281A depicts the concrete slab meeting below the top of the adjacent concrete wall.</div><div>Please coordinate the details for the connection of the concrete slab and adjacent concrete wall to match.</div></div>					



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<b>P1-0343</b>	<b>Deferral of Pylon Footings per ASI 122</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: ASI 122 dated 7/23/14, Detail 4/S1-3281A (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: ASI 122 dated 7/23/14, Detail 4/S1-3281A (IFC Drawings for Main Package dated 3/31/14)				
Per ASI 122 sheet notes, all pylons are deferred.		Per ASI 122 sheet notes, all pylons are deferred.				
If pylon footings are to be deferred, the anchor bolts will also be deferred. This will prevent footings from being installed as currently designed in Detail 4/S1-3281A.		If pylon footings are to be deferred, the anchor bolts will also be deferred. This will prevent footings from being installed as currently designed in Detail 4/S1-3281A.				
Note that pylon footing is currently included in the Trade Package TG07.2 contract.		Note that pylon footing is currently included in the Trade Package TG07.2 contract.				
Please provide revised footing details, or designate the anchorbolts to be used.		Please provide revised footing details, or designate the anchorbolts to be used.				
<b>P1-0344</b>	<b>Access Opening Details per Detail A/A1-8630A</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>09/04/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail A/A1-8630A, Detail 1/S1-2650A (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Detail A/A1-8630A, Detail 1/S1-2650A (IFC Drawings for Main Package dated 3/31/14)				
Detail A/A1-8630A calls for an access opening in wall.		Detail A/A1-8630A calls for an access opening in wall.				
Detail 1/S1-2650A does not illustrate an access opening in wall at the same location.		Detail 1/S1-2650A does not illustrate an access opening in wall at the same location.				
Please coordinate the details for an access opening in wall to match.		Please coordinate the details for an access opening in wall to match.				
<b>P1-0345</b>	<b>W10X54 Beam Connections to Adjacent Concrete Walls and Piers</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail 1/S1-2650A, Detail 5/S1-3281A (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Detail 1/S1-2650A, Detail 5/S1-3281A (IFC Drawings for Main Package dated 3/31/14)				
Detail 1/S1-2650A depicts W10X54 members connecting to adjacent concrete walls and piers, referring to Detail		Detail 1/S1-2650A depicts W10X54 members connecting to adjacent concrete walls and piers,				



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	<p>5/S1-3281A for exterior pier details.</p> <p>Detail 5/S1-3281A does not show the connection details for the W10X54 beams to the adjacent concrete walls and piers.</p> <p>Please provide details and specifications for the connection of the W10X54 beams to the adjacent concrete walls and piers.</p>					<p>referring to Detail 5/S1-3281A for exterior pier details.</p> <p>Detail 5/S1-3281A does not show the connection details for the W10X54 beams to the adjacent concrete walls and piers.</p> <p>Please provide details and specifications for the connection of the W10X54 beams to the adjacent concrete walls and piers.</p>
<b>P1-0346</b>	<b>Topping Slab for Loading Dock 01461</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/25/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Sheet A1-9527, L1-2304 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Sheet A1-9527, L1-2304 (IFC Drawings for Main Package dated 3/31/14)
Sheet A1-9527 shows a non-traffic rated topping slab at Loading Dock 01461.						Sheet A1-9527 shows a non-traffic rated topping slab at Loading Dock 01461.
Sheet L1-2304 shows a non-traffic rated approach to Loading Dock 01461.						Sheet L1-2304 shows a non-traffic rated approach to Loading Dock 01461.
Please confirm Loading Dock 01461 and the approach to it are not to have traffic rated topping slabs/paving.						Please confirm Loading Dock 01461 and the approach to it are not to have traffic rated topping slabs/paving.
<b>P1-0347</b>	<b>Bollards on Curb Cuts</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>07/31/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b>						<b>ANSWER:</b>
L1-2304 through L1-2304 Sheets L1-2304 through L1-2307, for example at gridline 17-19, have multiple curb cuts with bollards located in the center of them. Please confirm spacing between all bollards meets ADA standards at all curb cuts.						L1-2304 through L1-2304 Sheets L1-2304 through L1-2307, for example at gridline 17-19, have multiple curb cuts with bollards located in the center of them. Please confirm spacing between all bollards meets ADA standards at all curb cuts.



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<b>P1-0348</b>	<b>Concrete Pylon Deferral</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> ASI 122 - A1-2602 (SKA-3704) through A1-2607 (SKA-3710) Note says "Defer all pylons; associated lighting; security cameras and signs. Retain the concrete footing for the pylon." Provide requirements for pylon footings in this deferred state, including but not limited to, conduit tie in requirements, future use requirements, embed or drill and epoxy requirements.						<b>ANSWER:</b> ASI 122 - A1-2602 (SKA-3704) through A1-2607 (SKA-3710) Note says "Defer all pylons; associated lighting; security cameras and signs. Retain the concrete footing for the pylon." Provide requirements for pylon footings in this deferred state, including but not limited to, conduit tie in requirements, future use requirements, embed or drill and epoxy requirements.
<b>P1-0349</b>	<b>Phasing Requirements</b>	<b>Closed</b>	<b>0P</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>09/05/2014</b>
<b>From:</b> Webcor Construction LP      Zachary Moore						
<b>REQUEST:</b> ASI 122 - A1-2602 (SKA-3704) Note says "Defer the interiors of public restrooms and janitor's closet. Provide plumbing stub out within phased temporary building limit or within the bus deck ceiling." Provide phasing of the rooftop restaurant, including any temporary conditions that need to be accommodated for. Provide specifications and details for all phasing requirements.						<b>ANSWER:</b> ASI 122 - A1-2602 (SKA-3704) Note says "Defer the interiors of public restrooms and janitor's closet. Provide plumbing stub out within phased temporary building limit or within the bus deck ceiling." Provide phasing of the rooftop restaurant, including any temporary conditions that need to be accommodated for. Provide specifications and details for all phasing requirements.
<b>P1-0350</b>	<b>Traffic Coating at Bus Deck Level</b>	<b>Closed</b>	<b>0P</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b> REFERENCE: Sheets A1-9532 through A1-9534 (IFC Drawings for Main Package dated 3/31/14)  Per Sheets A1-9532 through A1-9534, the topping slab is to receive a scratch coat. The scratch coat may have too high of an amplitude to receive traffic coating. In addition, the contract documents do not require a traffic coating at the drive aisle.  Per the email from Mark O'Dell sent on 7/29/14, traffic coating will be added to the Bus Deck Level. Details and specifications have not been provided for the traffic coating at the Bus Deck Level.						<b>ANSWER:</b> REFERENCE: Sheets A1-9532 through A1-9534 (IFC Drawings for Main Package dated 3/31/14)  Per Sheets A1-9532 through A1-9534, the topping slab is to receive a scratch coat. The scratch coat may have too high of an amplitude to receive traffic coating. In addition, the contract documents do not require a traffic coating at the drive aisle.  Per the email from Mark O'Dell sent on 7/29/14, traffic coating will be added to the Bus Deck Level. Details and specifications have not been provided for the traffic coating at the Bus Deck Level.





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<b>P1-0353</b>	<b>Specification for Planting Materials</b>	<b>Closed</b>	<b>0P</b>	<b>08/07/2014</b>	<b>08/17/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: 32 93 00 Specification 32 93 00, 2.1.A states to provide 50% of concrete mixes within a total travel distance of 500 miles of the project site. This Specification is for Planting Materials, not Concrete Mixes. Please update this section so that it accurately describes the intent of this LEED Credit.						<b>ANSWER:</b> reference: 32 93 00 Specification 32 93 00, 2.1.A states to provide 50% of concrete mixes within a total travel distance of 500 miles of the project site. This Specification is for Planting Materials, not Concrete Mixes. Please update this section so that it accurately describes the intent of this LEED Credit.
<b>P1-0354</b>	<b>Dimensional Layouts for Chairs and Benches</b>	<b>Closed</b>	<b>0P</b>	<b>08/07/2014</b>	<b>08/17/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: L1-2612 through L1-2617 No dimensional layouts are provided for chairs or benches on the referenced drawings. Please provide dimensional layouts for all chairs and benches. For example, refer to L1-2614 GL 16/C where the typical spacing is indicated but the first and last chair or bench is not dimensioned.						<b>ANSWER:</b> reference: L1-2612 through L1-2617 No dimensional layouts are provided for chairs or benches on the referenced drawings. Please provide dimensional layouts for all chairs and benches. For example, refer to L1-2614 GL 16/C where the typical spacing is indicated but the first and last chair or bench is not dimensioned.
<b>P1-0355</b>	<b>Wood Decking Dimensional Layout</b>	<b>Closed</b>	<b>0P</b>	<b>08/07/2014</b>	<b>08/17/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: L1-2606 and L1-9607 L1-2606 shows wood decking at GL 29. 2/L1-9607 shows the detail for this deck, but neither provide a dimensional layout. Please provide the dimensional layout for this deck.						<b>ANSWER:</b> reference: L1-2606 and L1-9607 L1-2606 shows wood decking at GL 29. 2/L1-9607 shows the detail for this deck, but neither provide a dimensional layout. Please provide the dimensional layout for this deck.
<b>P1-0355.1</b>	<b>Dimensional Layouts for Deck on L1-9607</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b> Reference P1-0355, L1-2616, L1-9607 IFC Main Set						<b>ANSWER:</b> Reference P1-0355, L1-2616, L1-9607 IFC Main Set



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	<p>There is no detail for the deck dimensions on L1-2616. Provide dimensions for L1-9607.</p>					<p>There is no detail for the deck dimensions on L1-2616. Provide dimensions for L1-9607.</p>
<b>P1-0356</b>	<b>General Note 19 Tree Grow Period</b>	<b>Closed</b>	<b>0P</b>	<b>08/07/2014</b>	<b>08/17/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: L-0001, L-0006, L-0007 General Note 19 states that there will be a contract grow period of three years. Based on the current schedule where the earliest contract award date is June 2014 and installation of large trees begin December 2016, this time period is not possible. Please revise accordingly.						<b>ANSWER:</b> reference: L-0001, L-0006, L-0007 General Note 19 states that there will be a contract grow period of three years. Based on the current schedule where the earliest contract award date is June 2014 and installation of large trees begin December 2016, this time period is not possible. Please revise accordingly.
<b>P1-0357</b>	<b>Dimensions for curvature</b>	<b>Closed</b>	<b>0P</b>	<b>08/07/2014</b>	<b>08/17/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> reference: L1-2612 through L1-2617 There are several sections of the referenced drawings that have curvature which is not dimensioned, i.e. the curvature referenced on the attached mark-up of GL 12 on L1-2613 at C.3 and G. Please provide radii for all curves shown on Park Level Zone Layout Plans L1-2612 through L1-2617.						<b>ANSWER:</b> reference: L1-2612 through L1-2617 There are several sections of the referenced drawings that have curvature which is not dimensioned, i.e. the curvature referenced on the attached mark-up of GL 12 on L1-2613 at C.3 and G. Please provide radii for all curves shown on Park Level Zone Layout Plans L1-2612 through L1-2617.
<b>P1-0358</b>	<b>Drain Connection Detail</b>	<b>Closed</b>	<b>0P</b>	<b>08/07/2014</b>	<b>08/17/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP Zachary Moore						
<b>REQUEST:</b> L1-3202 & L1-3203 between GL J & K refer to Civil drawings for location of the connection to the drainage ¿ please provide this detail of the connection in the Civil drawings.						<b>ANSWER:</b> L1-3202 & L1-3203 between GL J & K refer to Civil drawings for location of the connection to the drainage ¿ please provide this detail of the connection in the Civil drawings.





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<b>P1-0359</b>	<b>Missing Detail 9/L1-7304</b>	<b>Closed</b>	<b>0P</b>	<b>08/13/2014</b>	<b>08/23/2014</b>	<b>12/22/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference L1-7302 and 9/L1-7304 (IFC Main Set 03/31/2014):  L1-7302 calls for "Plant Rootball Typ" to be installed in the planters, and refers to 9/L1-7304 for details. 9/L1-7304 does not exist. Please provide the type of plants to be installed within the planters shown in L1-7302.		<b>ANSWER:</b>  Reference L1-7302 and 9/L1-7304 (IFC Main Set 03/31/2014):  L1-7302 calls for "Plant Rootball Typ" to be installed in the planters, and refers to 9/L1-7304 for details. 9/L1-7304 does not exist. Please provide the type of plants to be installed within the planters shown in L1-7302.				
<b>P1-0360</b>	<b>Testing and Reporting Requirements for Shaw Alley Art Installation</b>	<b>Closed</b>	<b>0P</b>	<b>08/13/2014</b>	<b>08/23/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 12 93 40 (IFC Main Set 03/31/2014):  Specification Section 12 93 40 includes design requirements and detailed information for the illuminated pavers and benches; please indicate if UL and/or NEMA testing and reporting is required, including the testing and rating requirements if needed.		<b>ANSWER:</b>  Reference Specification Section 12 93 40 (IFC Main Set 03/31/2014):  Specification Section 12 93 40 includes design requirements and detailed information for the illuminated pavers and benches; please indicate if UL and/or NEMA testing and reporting is required, including the testing and rating requirements if needed.				
<b>P1-0361</b>	<b>Liquidated Damages</b>	<b>Closed</b>	<b>0P</b>	<b>08/13/2014</b>	<b>08/23/2014</b>	<b>08/25/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 34 41 13 (IFC Main Set 03/31/14)  Specification Section 34 41 13 Traffic Signals, Controller and Cabinet 3.9 & 3.10 includes language that assesses Liquidated Damages to the contractor; these should be deleted and addressed within the contract terms Div 00/01. Please delete and address within the Division 00/01 Specifications.		<b>ANSWER:</b>  Reference Specification Section 34 41 13 (IFC Main Set 03/31/14)  Specification Section 34 41 13 Traffic Signals, Controller and Cabinet 3.9 & 3.10 includes language that assesses Liquidated Damages to the contractor; these should be deleted and addressed within the contract terms Div 00/01. Please delete and address within the Division 00/01 Specifications.				
<b>P1-0362</b>	<b>Waterproofing at Concrete Bus Crash Rail</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>09/11/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						



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	<p><b>REQUEST:</b></p> <p>REFERENCE: A1-8675 (ASI 119 dated 6/18/14)</p> <p>The details on Sheet A1-8675 of ASI 119 call for WPM-2 waterproofing to be installed at the bus deck crash rail.</p> <p>There are no details depicting the relationship of the waterproofing from the bus deck level slab to the concrete bus crash rail.</p> <p>Please provide the details for waterproofing at this location.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: A1-8675 (ASI 119 dated 6/18/14)</p> <p>The details on Sheet A1-8675 of ASI 119 call for WPM-2 waterproofing to be installed at the bus deck crash rail.</p> <p>There are no details depicting the relationship of the waterproofing from the bus deck level slab to the concrete bus crash rail.</p> <p>Please provide the details for waterproofing at this location.</p>					
<b>P1-0363</b>	<b>Walk-Off Mats at Ground Level</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/21/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Detail 1 &amp; 2/A1-8214D (ASI 120 dated 7/11/14), Sheet A1-2305 (ASI 119 dated 6/20/14)</p> <p>Detail 1 &amp; 2/ A1-8214D of ASI 120 depicts walk-off mats located at W-3 sliding doors.</p> <p>Per Sheet A1-2305 of ASI 119, walk-off mats at the ground level are to be deleted.</p> <p>Please confirm that walk-off mats are to be installed at W-3 sliding doors per ASI 120 or coordinate the drawings and details to match.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Detail 1 &amp; 2/A1-8214D (ASI 120 dated 7/11/14), Sheet A1-2305 (ASI 119 dated 6/20/14)</p> <p>Detail 1 &amp; 2/ A1-8214D of ASI 120 depicts walk-off mats located at W-3 sliding doors.</p> <p>Per Sheet A1-2305 of ASI 119, walk-off mats at the ground level are to be deleted.</p> <p>Please confirm that walk-off mats are to be installed at W-3 sliding doors per ASI 120 or coordinate the drawings and details to match.</p>					



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<b>P1-0364</b>	<b>One Story Building Reference Per Specification Section 07 54 19 1.1 A 2</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/22/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 07 54 19 1.1 A 2 b (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Specification Section 07 54 19 1.1 A 2 b (IFC Drawings for Main Package dated 3/31/14)				
Specification Section 07 54 19 1.1 A 2 b states, "Roofing low slope roofs on one story structure east of Beale Street with a one-ply PVC membrane, fully adhered to a cover board and insulation."		Specification Section 07 54 19 1.1 A 2 b states, "Roofing low slope roofs on one story structure east of Beale Street with a one-ply PVC membrane, fully adhered to a cover board and insulation."				
The contract documents do not give any information on a one story structure east of Beale St.		The contract documents do not give any information on a one story structure east of Beale St.				
Please provide information on the referenced one story structure.		Please provide information on the referenced one story structure.				
<b>P1-0365</b>	<b>Fireproofing at Park Level W-4 Steel Ring Beam</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/25/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail 2/A1-8237D (ASI 120 dated 7/11/14)		REFERENCE: Detail 2/A1-8237D (ASI 120 dated 7/11/14)				
Detail 2/A1-8237D of ASI 120 calls for fireproofing at the steel ring beam.		Detail 2/A1-8237D of ASI 120 calls for fireproofing at the steel ring beam.				
No fireproofing is shown at the column supporting the steel ring beam.		No fireproofing is shown at the column supporting the steel ring beam.				
Please confirm that fireproofing is not required at the column shown in Detail 2/A1-8237D. If fireproofing is required at the column supporting the steel ring beam, please provide the details and type for the required fireproofing.		Please confirm that fireproofing is not required at the column shown in Detail 2/A1-8237D. If fireproofing is required at the column supporting the steel ring beam, please provide the details and type for the required fireproofing.				
<b>P1-0366</b>	<b>Top of Step Elevation for Stair 601</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sheet L1-3606 (IFC Drawings for Main Package dated 3/31/14), Details 1 & 2/A1-8651 (ASI 122 dated 7/23/14)		REFERENCE: Sheet L1-3606 (IFC Drawings for Main Package dated 3/31/14), Details 1 & 2/A1-8651 (ASI 122 dated 7/23/14)				



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	<p>Detail 3/A1-8652 (ASI 122 dated 7/23/14)</p> <p>Sheet L1-3606 shows a top of step elevation of 86.90 at Stair 601.</p> <p>Details 1 &amp; 2/A1-8651 shows a top of step elevation of 86'-9" at Stair 601 then refers to Detail 3/A1-8652.</p> <p>Detail 3/A1-8652 also shows a top of step elevation of 86'-9".</p> <p>Please confirm the correct elevation and coordinate the plans and details to match.</p>					<p>Detail 3/A1-8652 (ASI 122 dated 7/23/14)</p> <p>Sheet L1-3606 shows a top of step elevation of 86.90 at Stair 601.</p> <p>Details 1 &amp; 2/A1-8651 shows a top of step elevation of 86'-9" at Stair 601 then refers to Detail 3/A1-8652.</p> <p>Detail 3/A1-8652 also shows a top of step elevation of 86'-9".</p> <p>Please confirm the correct elevation and coordinate the plans and details to match.</p>
<b>P1-0367</b>	<b>Deferral of Expansion Joint Covers per ASI 122</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: ASI 122 - VE Round 3 dated 7/23/14						REFERENCE: ASI 122 - VE Round 3 dated 7/23/14
Per ASI 122 dated 7/23/14, the Roof Park expansion joints have not been deferred, but their covers have been deferred.						Per ASI 122 dated 7/23/14, the Roof Park expansion joints have not been deferred, but their covers have been deferred.
Expansion joints and all associated components are sold as a unit. This will affect the warranty (depending if the park is built out) and result in added cost with little benefit.						Expansion joints and all associated components are sold as a unit. This will affect the warranty (depending if the park is built out) and result in added cost with little benefit.
Please confirm TJPA wants W/O to contract out the entire roof top park expansion joint assembly at grid lines 10 and 20 except the cover.						Please confirm TJPA wants W/O to contract out the entire roof top park expansion joint assembly at grid lines 10 and 20 except the cover.



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<b>P1-0368</b>	<b>Dampproofing Requirements at Trench and Slot Drain Channels</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Specification Section 07 11 16 1.1 A 4, Sheet L1-7318, Sheet L1-7381 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Specification Section 07 11 16 1.1 A 4, Sheet L1-7318, Sheet L1-7381 (IFC Drawings for Main Package dated 3/31/14)
Specification Section 07 11 16 1.1 A 4 calls out for site dampproofing at trench and slot drain channels.						Specification Section 07 11 16 1.1 A 4 calls out for site dampproofing at trench and slot drain channels.
Trench drain details do not show dampproofing at these locations. (See Sheet L1-7318 and Sheet L1-7381 for examples.)						Trench drain details do not show dampproofing at these locations. (See Sheet L1-7318 and Sheet L1-7381 for examples.)
Please provide the details and locations requiring dampproofing that the trench and slot drain channels.						Please provide the details and locations requiring dampproofing that the trench and slot drain channels.
<b>P1-0369</b>	<b>Waterproofing at Walk-Off Mat per Detail 6/A1-9307</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Detail 6/A1-9307 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Detail 6/A1-9307 (IFC Drawings for Main Package dated 3/31/14)
Detail 6/A1-9307 calls out for ¿continuous WPM¿, but does not identify what type of waterproofing to install.						Detail 6/A1-9307 calls out for ¿continuous WPM¿, but does not identify what type of waterproofing to install.
Please provide the type of waterproofing to be installed at the condition called out on Detail 6/A1-9307.						Please provide the type of waterproofing to be installed at the condition called out on Detail 6/A1-9307.
<b>P1-0370</b>	<b>Protection Board and Waterproofing Membrane Details at Public Restroom</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Detail 5/A1-9041, Detail 10/A1-9042 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Detail 5/A1-9041, Detail 10/A1-9042 (IFC Drawings for Main Package dated 3/31/14)
Detail 5/A1-9041 calls out for "protection board over waterproof membrane" directly below the topping slab. Detail 10/A1-9042 calls out for "protection board and waterproofing where required" directly under topping slab.						Detail 5/A1-9041 calls out for "protection board over waterproof membrane" directly below the topping slab. Detail 10/A1-9042 calls out for "protection board and waterproofing where required" directly under topping slab.



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<b>P1-0371</b>	<p>A1-9041 &amp; A1-9042 are public restroom typical detail sheets.</p> <p>Please identify where waterproofing is required within the public restrooms, and what type of waterproofing is to be used.</p> <p><b>WPM-5 Crystalline Waterproofing at Escalator Pit per Detail 4/A1-7550</b></p> <p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Detail 4/A1-7550 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Detail 4/A1-7550 calls out for WPM-5 (crystalline waterproofing) over plate steel. WPM-5 crystalline waterproofing is specifically intended for sealing of concrete, and will not work on steel.</p> <p>Please provide direction to the type of waterproofing to be used at this condition, and associated waterproofing details.</p>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/25/2014</b>
	<p><b>ANSWER:</b></p> <p>REFERENCE: Detail 4/A1-7550 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Detail 4/A1-7550 calls out for WPM-5 (crystalline waterproofing) over plate steel. WPM-5 crystalline waterproofing is specifically intended for sealing of concrete, and will not work on steel.</p> <p>Please provide direction to the type of waterproofing to be used at this condition, and associated waterproofing details.</p>					
<b>P1-0372</b>	<p><b>WPM -2 Call Out at Muni Bus Plaza</b></p> <p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Detail 2/A1-7822 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Detail 2/A1-7822 calls out for WPM-2 at the Muni Bus Plaza. Typically WPM-1A is used at exterior Ground Level.</p> <p>Please confirm the WPM-2 as shown on Detail 2/A1-7822 is to be revised to WPM-1A. If not, please provide the extents of WPM-2 at Ground Level.</p>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/14/2014</b>	<b>08/22/2014</b>
	<p><b>ANSWER:</b></p> <p>REFERENCE: Detail 2/A1-7822 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Detail 2/A1-7822 calls out for WPM-2 at the Muni Bus Plaza. Typically WPM-1A is used at exterior Ground Level.</p> <p>Please confirm the WPM-2 as shown on Detail 2/A1-7822 is to be revised to WPM-1A. If not, please provide the extents of WPM-2 at Ground Level.</p>					
<b>P1-0373</b>	<b>Concrete Paving Specification Clarification</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/21/2014</b>



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	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Specification Section 07 19 25, Specification Section 07 19 23</p> <p>Specification Section 07 19 25 and Specification Section 07 19 23 both indicate they are to be used for concrete paving.</p> <p>Please clarify which specification section is to be used for concrete paving.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Specification Section 07 19 25, Specification Section 07 19 23</p> <p>Specification Section 07 19 25 and Specification Section 07 19 23 both indicate they are to be used for concrete paving.</p> <p>Please clarify which specification section is to be used for concrete paving.</p>					
<b>P1-0374</b>	<b>Details for Site Fluid-Applied Waterproofing (WPM-4)</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>09/03/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Specification Section 07 13 00</p> <p>Specification Section 07 13 00 Site Fluid-Applied Waterproofing (WPM-4) appears to be related to roof park elements only.</p> <p>Does this occur anywhere else? No details for WPM-4 appear in architectural or landscape drawings.</p> <p>Please provide details for WPM-4 site fluid-applied waterproofing.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Specification Section 07 13 00</p> <p>Specification Section 07 13 00 Site Fluid-Applied Waterproofing (WPM-4) appears to be related to roof park elements only.</p> <p>Does this occur anywhere else? No details for WPM- 4 appear in architectural or landscape drawings.</p> <p>Please provide details for WPM-4 site fluid-applied waterproofing.</p>					



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<b>P1-0375</b>	<b>Sloped Topping Specifications</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Detail 4/A1-7552, Detail 8/A1-7552 (IFC Drawings for Main Package dated 3/31/14)  Several locations show a "sloped topping" which is ¼" thick or less (see 4/A1-7552 & 8/A1-7552) for creating positive slope at waterproofing.  No product is specified for the "sloped topping". Please provide a product that will allow for slopping from less than 2" to 0" thickness (including from 1/2" to 0").						<b>ANSWER:</b>  REFERENCE: Detail 4/A1-7552, Detail 8/A1-7552 (IFC Drawings for Main Package dated 3/31/14)  Several locations show a "sloped topping" which is ¼" thick or less (see 4/A1-7552 & 8/A1-7552) for creating positive slope at waterproofing.  No product is specified for the "sloped topping". Please provide a product that will allow for slopping from less than 2" to 0" thickness (including from 1/2" to 0").
<b>P1-0376</b>	<b>Exterior Awning Noise Due to Wind</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/14/2014</b>	<b>09/01/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 08 44 27 (W-2 Reissued for Bid Set)  Please confirm that noise due to wind moving through the patterns cut out in the Aluminum Panels of the Exterior Awning and through the gaps between the panels Exterior Awning and noise due to wind moving above and below the entire system is not to be considered an unacceptable condition. The contractor is providing the pattern per the contract documents and they will have no control over any noise due to wind moving through the predetermined pattern, gaps between each panel and wind moving above and below the entire system. The contractor should only be responsible for noise due to vibration caused by poor workmanship and should not be responsible for any noise due to the inherent design of the W-1 Exterior Awning.						<b>ANSWER:</b>  Reference Specification Section 08 44 27 (W-2 Reissued for Bid Set)  Please confirm that noise due to wind moving through the patterns cut out in the Aluminum Panels of the Exterior Awning and through the gaps between the panels Exterior Awning and noise due to wind moving above and below the entire system is not to be considered an unacceptable condition. The contractor is providing the pattern per the contract documents and they will have no control over any noise due to wind moving through the predetermined pattern, gaps between each panel and wind moving above and below the entire system. The contractor should only be responsible for noise due to vibration caused by poor workmanship and should not be responsible for any noise due to the inherent design of the W-1 Exterior Awning.
<b>P1-0377</b>	<b>WPM-5 Crystalline Waterproofing at Escalator Pit per Detail 4/A1-7552</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/14/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Detail 4/A1-7552 (IFC Drawings for Main						<b>ANSWER:</b>  REFERENCE: Detail 4/A1-7552 (IFC Drawings for





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	<p>Package dated 3/31/14)</p> <p>Detail 4/A1-7552 shows WPM-5 crystalline waterproofing extending into a drain body within an escalator pit.</p> <p>Crystalline waterproofing is a trowelled on waterproofing which seals concrete, and is not a membrane which can be lapped into a drain body.</p> <p>Please revise the detail to show the extent of WPM-5 crystalline waterproofing to be used at the drain.</p>					
	<p>Main Package dated 3/31/14)</p> <p>Detail 4/A1-7552 shows WPM-5 crystalline waterproofing extending into a drain body within an escalator pit.</p> <p>Crystalline waterproofing is a trowelled on waterproofing which seals concrete, and is not a membrane which can be lapped into a drain body.</p> <p>Please revise the detail to show the extent of WPM-5 crystalline waterproofing to be used at the drain.</p>					
<b>P1-0378</b>	<b>WPM-9 Call Outs at Elevator Pits</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: Detail 3/A1-7576, Detail 2/A1-7577, Detail 3/A1-7577, Specification Section 07 19 25 (IFC Drawings for Main Package dated 3/31/14)						
Detail 3/A1-7576, Detail 2/A1-7577, and Detail 3/A1-7577 call out for WPM-9 over drywall cant strips within elevator pits.						
Per Specification Section 07 19 25, WPM-9 is for concrete surfaces only.						
Please confirm WPM-9 is to be used over drywall where shown on the contract documents or provide the correct WPM system for the drywall.						
<b>ANSWER:</b>						
REFERENCE: Detail 3/A1-7576, Detail 2/A1-7577, Detail 3/A1-7577, Specification Section 07 19 25 (IFC Drawings for Main Package dated 3/31/14)						
Detail 3/A1-7576, Detail 2/A1-7577, and Detail 3/A1-7577 call out for WPM-9 over drywall cant strips within elevator pits.						
Per Specification Section 07 19 25, WPM-9 is for concrete surfaces only.						
Please confirm WPM-9 is to be used over drywall where shown on the contract documents or provide the correct WPM system for the drywall.						



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<b>P1-0379</b>	<b>Slab Sloping Details at Roof Park Level</b>	<b>Void</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Details 1 & 2/A1-8646 (ASI 122 dated 07/23/14)  Details 1 & 2/A1-8646 of ASI 122 show a slab sloping in an east-west direction, above the structural slab below waterproofing.  Slab plans do not identify these slabs.  Please provide information on slabs sloping east-west on the Roof Park Level.						<b>ANSWER:</b>  REFERENCE: Details 1 & 2/A1-8646 (ASI 122 dated 07/23/14)  Details 1 & 2/A1-8646 of ASI 122 show a slab sloping in an east-west direction, above the structural slab below waterproofing.  Slab plans do not identify these slabs.  Please provide information on slabs sloping east-west on the Roof Park Level.
<b>P1-0380</b>	<b>WPM-5 Crystalline Waterproofing at Lover Concourse Level Telecom Service Vault Closed</b>		<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>08/21/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Detail 1/A1-9250, Detail 3/A1-9251, Detail 4/A1-9251 (ASI 118 dated 6/20/14)  Detail 1/A1-9250 of ASI 118 shows WPM-5 crystalline waterproofing being applied to the bottom of metal deck and the bent plate at the edge of deck.  Details 3 & 4/A1-9251 of ASI 118 shows WPM-5 crystalline waterproofing going over a metal sleeve.  Crystalline waterproofing is not compatible with metal.  Please revise waterproofing detail where WPM-5 crystalline waterproofing is shown over metal deck, bent plate, and metal sleeve.						<b>ANSWER:</b>  REFERENCE: Detail 1/A1-9250, Detail 3/A1-9251, Detail 4/A1-9251 (ASI 118 dated 6/20/14)  Detail 1/A1-9250 of ASI 118 shows WPM-5 crystalline waterproofing being applied to the bottom of metal deck and the bent plate at the edge of deck.  Details 3 & 4/A1-9251 of ASI 118 shows WPM-5 crystalline waterproofing going over a metal sleeve.  Crystalline waterproofing is not compatible with metal.  Please revise waterproofing detail where WPM-5 crystalline waterproofing is shown over metal deck, bent plate, and metal sleeve.
<b>P1-0381</b>	<b>W-2 Schedule of Unit Prices</b>	<b>Closed</b>	<b>0P</b>	<b>08/20/2014</b>	<b>08/30/2014</b>	<b>09/22/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference 08 44 25 (ASI 120 and Div. 00/01 Specifications)						<b>ANSWER:</b>  Reference 08 44 25 (ASI 120 and Div. 00/01 Specifications)



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	<p>Specification Section 08 44 25 subsection 1.3 calls for a unit price for Temporary Retail Façade and Glass Retail Façade for 2, 3 and 4 bay modules. There is not a Schedule of unit prices specification for the W-2 system in the Div. 00/01 dated August 11, 2014. If a unit price is required for the Temporary Retail Façade it needs to be included in the Div. 00/01. Currently the only unit price specifications listed are 01 10 20/APA for TG03, 10 10 20/APB for TG04.5.1, 01 10 20/APC for TG04.3, 01 10 20/APD for TG04.4, 01 10 20/APE for TG04.1, 01 10 20/APF for TG04.2, 01 10 20/APG for TG04.6, 01 10 20/API for TG06.0, and 01 10 20/APJ for TG18.1. If unit pricing for Temporary and Glass Retail Façade is desired, provide unit price specification, otherwise revise specification to not include unit pricing language.</p>					<p>Specification Section 08 44 25 subsection 1.3 calls for a unit price for Temporary Retail Façade and Glass Retail Façade for 2, 3 and 4 bay modules. There is not a Schedule of unit prices specification for the W-2 system in the Div. 00/01 dated August 11, 2014. If a unit price is required for the Temporary Retail Façade it needs to be included in the Div. 00/01. Currently the only unit price specifications listed are 01 10 20/APA for TG03, 10 10 20/APB for TG04.5.1, 01 10 20/APC for TG04.3, 01 10 20/APD for TG04.4, 01 10 20/APE for TG04.1, 01 10 20/APF for TG04.2, 01 10 20/APG for TG04.6, 01 10 20/API for TG06.0, and 01 10 20/APJ for TG18.1. If unit pricing for Temporary and Glass Retail Façade is desired, provide unit price specification, otherwise revise specification to not include unit pricing language.</p>
<b>P1-0382</b>	<b>Glazing Schedule of Alternates</b>	<b>Closed</b>	<b>0P</b>	<b>08/20/2014</b>	<b>08/30/2014</b>	<b>09/10/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference 01 10 30/APE (Div. 00/01 Specifications 08/11)						Reference 01 10 30/APE (Div. 00/01 Specifications 08/11)
The schedule of alternates for the design build glazing systems was not included in the August 11, 2014 issuance of the Div. 00/01 specifications. Any alternates that are desired for the TG08.10 Glass Curtain Wall (W-2, 3, 4, 6, and 8) need to be included in the Div. 00/01 specifications. Currently the only alternates that affect the TG08.10 package are alternate # 4 which deletes the Beale Street Lobby and #31 which changes the W-2 glass from low iron glass to standard clear. If any other alternates are desired, provide them in the 01 10 30/APE specification.						The schedule of alternates for the design build glazing systems was not included in the August 11, 2014 issuance of the Div. 00/01 specifications. Any alternates that are desired for the TG08.10 Glass Curtain Wall (W-2, 3, 4, 6, and 8) need to be included in the Div. 00/01 specifications. Currently the only alternates that affect the TG08.10 package are alternate # 4 which deletes the Beale Street Lobby and #31 which changes the W-2 glass from low iron glass to standard clear. If any other alternates are desired, provide them in the 01 10 30/APE specification.
<b>P1-0383</b>	<b>Ballistic Resistant Window in Room B1269</b>	<b>Closed</b>	<b>0P</b>	<b>08/20/2014</b>	<b>08/30/2014</b>	<b>08/25/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	Reference A1-9855 Detail 2 (IFC Main Set)  Provide specification for the glass type for the ballistic resistant window, window frame and pass thru drawer required for the reception window in room B1269. In addition provide details of how the frame attaches to the pass thru drawer mounted on the bottom of the window.				Reference A1-9855 Detail 2 (IFC Main Set)  Provide specification for the glass type for the ballistic resistant window, window frame and pass thru drawer required for the reception window in room B1269. In addition provide details of how the frame attaches to the pass thru drawer mounted on the bottom of the window.	
<b>P1-0384</b>	<b>Planting Strip at Exhaust Ventilation Strap</b>  <b>From:</b> Webcor Construction LP      Andrew Kitchen	<b>Closed</b>	<b>0P</b>	<b>08/20/2014</b>	<b>08/30/2014</b>	<b>08/22/2014</b>
	<b>REQUEST:</b>  Reference: 3/A1-8176 IFC Main Set  Detail 3/A1-8716 shows a planting strip at the exhaust ventilation shaft near the bus ramps; Landscaping drawings do not provide any information regarding this area. Please confirm that planting strip is desired, and detail reference in landscaping for types of plants if any.		<b>ANSWER:</b>  Reference: 3/A1-8176 IFC Main Set  Detail 3/A1-8716 shows a planting strip at the exhaust ventilation shaft near the bus ramps; Landscaping drawings do not provide any information regarding this area. Please confirm that planting strip is desired, and detail reference in landscaping for types of plants if any.			



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<b>P1-0385</b>	<b>Access Panels at Stair 201</b>	<b>Closed</b>	<b>0P</b>	<b>08/21/2014</b>	<b>08/31/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Sheet A1-2302 (ASI 119 dated 6/20/14) Detail 1/A1-7001 (IFC Drawings for Main Package dated 3/31/14) Detail A/A1-7850 (ASI 122 dated 7/23/14) Detail B/A1-5102 (ASI 122 dated 7/23/14)  Sheet A1-2302 (ASI 119) and Detail 1/A1-7001 (IFC) both depict an access panel at the Stair 201B on the Ground Level and references Detail A/A1-7850 (ASI 122) and Detail B/A1-5102 (ASI 122).  - Detail A/A1-7850 (ASI 122) does not show an access panel/wall/fence at the same location depicted in Sheet A1-2302 and Detail 1/A1-7001.  - Detail B/A1-5102 (ASI 122) does not show an access panel/wall/fence at the same location depicted in Sheet A1-2302 and Detail 1/A1-7001.  Please provide details for the access panel/wall/fence called out on Sheet A1-2302 and Detail 1/A1-7001.						REFERENCE: Sheet A1-2302 (ASI 119 dated 6/20/14) Detail 1/A1-7001 (IFC Drawings for Main Package dated 3/31/14) Detail A/A1-7850 (ASI 122 dated 7/23/14) Detail B/A1-5102 (ASI 122 dated 7/23/14)  Sheet A1-2302 (ASI 119) and Detail 1/A1-7001 (IFC) both depict an access panel at the Stair 201B on the Ground Level and references Detail A/A1-7850 (ASI 122) and Detail B/A1-5102 (ASI 122).  - Detail A/A1-7850 (ASI 122) does not show an access panel/wall/fence at the same location depicted in Sheet A1-2302 and Detail 1/A1-7001.  - Detail B/A1-5102 (ASI 122) does not show an access panel/wall/fence at the same location depicted in Sheet A1-2302 and Detail 1/A1-7001.  Please provide details for the access panel/wall/fence called out on Sheet A1-2302 and Detail 1/A1-7001.
<b>P1-0386</b>	<b>WPM-5 Called out in Detail 6/A1-7554 at Second Level W-2 Escalator Pit</b>	<b>Closed</b>	<b>0P</b>	<b>08/21/2014</b>	<b>08/31/2014</b>	<b>08/25/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Detail 6/A1-7554 (IFC Drawings for Main Package dated 3/31/14)  Detail 6/A1-7554 calls for WPM-5 crystalline waterproofing to cover insulation and a steel mullion at the second level of the W-2 escalator pit.  WPM-5 crystalline waterproofing is specifically intended for sealing of concrete, and will not work on steel.  Please provide direction to the type of waterproofing to be used at this condition, and associated waterproofing details.						REFERENCE: Detail 6/A1-7554 (IFC Drawings for Main Package dated 3/31/14)  Detail 6/A1-7554 calls for WPM-5 crystalline waterproofing to cover insulation and a steel mullion at the second level of the W-2 escalator pit.  WPM-5 crystalline waterproofing is specifically intended for sealing of concrete, and will not work on steel.  Please provide direction to the type of waterproofing to be used at this condition, and associated waterproofing details.



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<b>P1-0387</b>	<b>Corner Guards Dimension Details per ASI 123</b>	<b>Closed</b>	<b>0P</b>	<b>08/21/2014</b>	<b>08/31/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: Specification Section 05 50 00 2.5 M 1 & 2 (ASI 123 dated 8/6/14)						
Specification Section 05 50 00 2.5 M 1 & 2 of ASI 123 states, "1. For Concrete Columns/Concrete Block Walls: as detailed, 8' x 4" x 1/2" fabricated aluminum angles, 8'-0" high typical or as shown on drawings with anchor straps at 12: o.c. 2. For Gypsum Board Walls: as detailed, 8' x 4" x 1/2" fabricated aluminum angles, 8'-0" high typical or as shown on drawings. Flush countersunk fasteners"						
A standard size for corner guards are 4" x 4" x 1/2". Please confirm that corner guards are required to be the dimensions referenced in Specification Section 05 50 00 2.5 M 1 & 2.						
<b>ANSWER:</b>						
REFERENCE: Specification Section 05 50 00 2.5 M 1 & 2 (ASI 123 dated 8/6/14)						
Specification Section 05 50 00 2.5 M 1 & 2 of ASI 123 states, "1. For Concrete Columns/Concrete Block Walls: as detailed, 8' x 4" x 1/2" fabricated aluminum angles, 8'-0" high typical or as shown on drawings with anchor straps at 12: o.c. 2. For Gypsum Board Walls: as detailed, 8' x 4" x 1/2" fabricated aluminum angles, 8'-0" high typical or as shown on drawings. Flush countersunk fasteners"						
A standard size for corner guards are 4" x 4" x 1/2". Please confirm that corner guards are required to be the dimensions referenced in Specification Section 05 50 00 2.5 M 1 & 2.						
<b>P1-0388</b>	<b>Metal Framing Design-Build Compliance with Specification Section 08 05 13</b>	<b>Closed</b>	<b>0P</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 05 41 00 1.6 G Specification Section 09 22 19 1.6 K Specidication Specification 08 05 13 Sheet A-0023						
Specification Section 05 41 00 1.6 G and Specification Section 09 22 19 1.6 K require design-build work complying with Specification Section 08 05 13 "where shown on drawings and on Partition Schedule drawing A-0023."						
Sheet A-0023 does not indicate any wall type needing to meet the design requirements of Specification Section 08 05 13.						
Please confirm that none of the wall types identified on Sheet A-0023 are required meet the design requirements of Specification Section 08 05 13.						
<b>ANSWER:</b>						
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 05 41 00 1.6 G Specification Section 09 22 19 1.6 K Specidication Specification 08 05 13 Sheet A-0023						
Specification Section 05 41 00 1.6 G and Specification Section 09 22 19 1.6 K require design-build work complying with Specification Section 08 05 13 "where shown on drawings and on Partition Schedule drawing A-0023."						
Sheet A-0023 does not indicate any wall type needing to meet the design requirements of Specification Section 08 05 13.						
Please confirm that none of the wall types identified on Sheet A-0023 are required meet the design requirements of Specification Section 08 05 13.						



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<b>P1-0389</b>	<b>CJC4 Expansion Joint at Second Level GL 10</b>	<b>Closed</b>	<b>0P</b>	<b>08/21/2014</b>	<b>08/31/2014</b>	<b>09/01/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Sheet A1-8880 (ASI 123 dated 8/6/14) Sheet A1-9604 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Sheet A1-8880 (ASI 123 dated 8/6/14) Sheet A1-9604 (IFC Drawings for Main Package dated 3/31/14)
Sheet A1-8880 of ASI 123 directs for the installation of a CJC4 expansion joint within the gypsum board ceiling at GL 10 on the Second Level.						Sheet A1-8880 of ASI 123 directs for the installation of a CJC4 expansion joint within the gypsum board ceiling at GL 10 on the Second Level.
Per Sheet A1-9604 of IFC Main Package, the ceiling for Room 02320 is exposed to structure, not a gypsum board ceiling.						Per Sheet A1-9604 of IFC Main Package, the ceiling for Room 02320 is exposed to structure, not a gypsum board ceiling.
Please confirm no ceiling expansion joint is to be installed at Second Level GL10.						Please confirm no ceiling expansion joint is to be installed at Second Level GL10.
<b>P1-0390</b>	<b>WJC4 Adjoining Materials</b>	<b>Closed</b>	<b>0P</b>	<b>08/21/2014</b>	<b>08/31/2014</b>	<b>09/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Sheet A1-8880 (ASI 123 dated 8/6/14) Detail 2/A1-8178 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Sheet A1-8880 (ASI 123 dated 8/6/14) Detail 2/A1-8178 (IFC Drawings for Main Package dated 3/31/14)
Sheet A1-8880 of ASI 123 calls out for WJC4 to have adjoining materials of gypsum board.						Sheet A1-8880 of ASI 123 calls out for WJC4 to have adjoining materials of gypsum board.
Detail 2/A1-8178 of IFC Main Package does not show gypsum board in the plain of the wall joint.						Detail 2/A1-8178 of IFC Main Package does not show gypsum board in the plain of the wall joint.
Please confirm no gypsum board is required in plain of WJC4.						Please confirm no gypsum board is required in plain of WJC4.
<b>P1-0391</b>	<b>Insulated Soffit References per A1-9603</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Sheet A1-9603 (ASI 119 dated 6/20/14)						REFERENCE: Sheet A1-9603 (ASI 119 dated 6/20/14)
Sheet A1-9603 references insulated soffits at Rooms						



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	01361, 01381, 01425, 01426, 01441, 01483, 01603.  Insulated soffits are not shown on RCP or sections.  Please confirm that insulated soffits are not required at the referenced locations.					Sheet A1-9603 references insulated soffits at Rooms 01361, 01381, 01425, 01426, 01441, 01483, 01603.  Insulated soffits are not shown on RCP or sections.  Please confirm that insulated soffits are not required at the referenced locations.
<b>P1-0392</b>	<b>Ceiling Type for Room 01603</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Sheet A1-9603 (ASI 119 dated 6/20/14), Detail 3/A1-4306 (IFC Drawings for Main Package dated 3/31/14)  Sheet A1-9603 references a gypsum board ceiling at Room 01603.  Sheet A1-4306 and Detail 3/A1-8511 call for a metal ceiling at this location.  Please confirm ceiling type.						<b>ANSWER:</b>  REFERENCE: Sheet A1-9603 (ASI 119 dated 6/20/14), Detail 3/A1-4306 (IFC Drawings for Main Package dated 3/31/14)  Sheet A1-9603 references a gypsum board ceiling at Room 01603.  Sheet A1-4306 and Detail 3/A1-8511 call for a metal ceiling at this location.  Please confirm ceiling type.





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<b>P1-0393</b>	<b>Insulation Beneath the Radiant Floor Heating System</b>	<b>Void</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Specification Section 07 21 00 1.1 2, Specification Section 07 21 00 3.2 B, Sheet A1-2982 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 07 21 00 1.1 2 and Specification Section 07 21 00 3.2 B indicate insulation is to be furnished and installed beneath the radiant floor heating system. Per Specification Section 07 21 00 2.3 R INS-16 is to be used in radiant terrazzo floor areas. Sheet A1- 2982 shows INS 13 at the radiant flooring.  Per Specification Section 01 10 30/APE E.1.7 Alternate 15, the deletion of radiant flooring is to be priced.  Please confirm insulation requirements are not to change as part of Alternate 15.		<b>ANSWER:</b>  REFERENCE: Specification Section 07 21 00 1.1 2, Specification Section 07 21 00 3.2 B, Sheet A1-2982 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 07 21 00 1.1 2 and Specification Section 07 21 00 3.2 B indicate insulation is to be furnished and installed beneath the radiant floor heating system. Per Specification Section 07 21 00 2.3 R INS-16 is to be used in radiant terrazzo floor areas. Sheet A1-2982 shows INS 13 at the radiant flooring.  Per Specification Section 01 10 30/APE E.1.7 Alternate 15, the deletion of radiant flooring is to be priced.  Please confirm insulation requirements are not to change as part of Alternate 15.				
<b>P1-0394</b>	<b>Insulation Specification for Parapets</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>09/04/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Specification Section 07 21 00 1.1 1 d, Specification Section 07 21 00 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 07 21 00 1.1 1 d indicates Specification Section 07 21 00 does not apply to parapets.  Several details (for example, Details 1 & 2/A1-7870) show insulation within parapets.  Please provide a specification for insulation shown within parapets.		<b>ANSWER:</b>  REFERENCE: Specification Section 07 21 00 1.1 1 d, Specification Section 07 21 00 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 07 21 00 1.1 1 d indicates Specification Section 07 21 00 does not apply to parapets.  Several details (for example, Details 1 & 2/A1-7870) show insulation within parapets.  Please provide a specification for insulation shown within parapets.				
<b>P1-0395</b>	<b>Request for Specification Section 07 12 11 Cellular Foamed Glass Insulation</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				



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	<p>REFERENCE: Specification Section 07 21 00, Specification Section 07 12 11 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 07 21 00 references Specification Section 07 12 11 for Cellular Foamed Glass Insulation.</p> <p>Specification Section 07 12 11 has not been issued.</p> <p>Please provide the referenced spec.</p>					
<b>P1-0396</b>	<b>Request for Specification Section 07 27 00</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>09/05/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Specification Section 07 21 00 3.2 D, Specification Section 07 27 00 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 07 21 00 3.2 D references Specification Section 07 27 00.</p> <p>Specification Section 07 27 00 has not been issued.</p> <p>Please provide the referenced specification.</p>					
<b>P1-0397</b>	<b>Design-Build Requirement for Roof Hatches</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>10/02/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Specification Section 07 72 33 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 07 72 33 requires design-build requirements for roof hatches.</p> <p>Roof hatches is not typically a D-B scope of work.</p> <p>Please confirm TJPA desires engineering for roof hatches.</p>					



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<b>P1-0398</b>	<b>Tubular H.O.G. Protective Enclosure and Stainless Steel Surface Collar Details</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 08 62 50 3.5 Sheet A1-8877 Specification Section 08 62 50 3.5 A 2  Specification Section 08 62 50 3.5 Tubular Daylighting Devices references Tubular H.O.G Protective Enclosure and Stainless Steel Surface Collar. Sheet A1-8877 does not identify these pieces.  In addition, Specification Section 08 62 50 3.5 A 2 references seismic angle braces and Structural drawings. Structural drawings do not identify these pieces.  Please provide details and structural information on Tubular H.O.G Protective Enclosures and Stainless Steel Surface Collars referenced in Specification Section 08 62 50 3.5.						
<b>ANSWER:</b>						
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 08 62 50 3.5 Sheet A1-8877 Specification Section 08 62 50 3.5 A 2  Specification Section 08 62 50 3.5 Tubular Daylighting Devices references Tubular H.O.G Protective Enclosure and Stainless Steel Surface Collar. Sheet A1-8877 does not identify these pieces.  In addition, Specification Section 08 62 50 3.5 A 2 references seismic angle braces and Structural drawings. Structural drawings do not identify these pieces.  Please provide details and structural information on Tubular H.O.G Protective Enclosures and Stainless Steel Surface Collars referenced in Specification Section 08 62 50 3.5.						
<b>P1-0399</b>	<b>Revision of Referenced Specifcaton Section 09 90 00</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>08/22/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: Specification Section 09 21 16 1.6 E, Specification Section 09 90 00 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 09 21 16 1.6 E references Specification Section 09 90 00.  Specification 09 90 00 is not part of the contract documents.  Please revise the referenced specification number.						
<b>ANSWER:</b>						
REFERENCE: Specification Section 09 21 16 1.6 E, Specification Section 09 90 00 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 09 21 16 1.6 E references Specification Section 09 90 00.  Specification 09 90 00 is not part of the contract documents.  Please revise the referenced specification number.						
<b>P1-0400</b>	<b>Portable Lighting Requirement per Specification Section 09 21 16 1.6 E</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
<b>ANSWER:</b>						





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	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div></div> <div>REQUEST:<div>REFERENCE: Specification Section 09 25 50 3.2 A 4, Specification Section 09 31 00 (IFC Drawings for Main Package dated 3/31/14)</div><div>Specification Section 09 25 50 3.2 A 4 references Specification Section 09 31 00.</div><div>Specification Section 09 31 00 is not part of the contract documents.</div><div>Please revise the referenced specification number.</div></div> <div>ANSWER:<div>REFERENCE: Specification Section 09 25 50 3.2 A 4, Specification Section 09 31 00 (IFC Drawings for Main Package dated 3/31/14)</div><div>Specification Section 09 25 50 3.2 A 4 references Specification Section 09 31 00.</div><div>Specification Section 09 31 00 is not part of the contract documents.</div><div>Please revise the referenced specification number.</div></div>					
P1-0403	Acclimизation Requirements for ACT Installation	Closed	0P	08/22/2014	09/01/2014	08/27/2014
	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div></div> <div>REQUEST:<div>REFERENCE: Specification Section 09 51 00 1.9 A, Specification Section 09 51 00 3.2 A (IFC Drawings for Main Package dated 3/31/14)</div><div>Specification Section 09 51 00 1.9 A requires temperatures be not less than 70 degrees F at least 3 days prior to and 3 days after installation of ACT.</div><div>Specification Section 09 51 00 3.2 A requires temperatures of at least 72 degrees F 3 days prior and after installation of ACT.</div><div>Please clarify acclimatization requirement. (NOTE: changing the requirement to maintaining minimum requirements of the product manufacturer may reduce acclimatization costs).</div></div> <div>ANSWER:<div>REFERENCE: Specification Section 09 51 00 1.9 A, Specification Section 09 51 00 3.2 A (IFC Drawings for Main Package dated 3/31/14)</div><div>Specification Section 09 51 00 1.9 A requires temperatures be not less than 70 degrees F at least 3 days prior to and 3 days after installation of ACT.</div><div>Specification Section 09 51 00 3.2 A requires temperatures of at least 72 degrees F 3 days prior and after installation of ACT.</div><div>Please clarify acclimatization requirement. (NOTE: changing the requirement to maintaining minimum requirements of the product manufacturer may reduce acclimatization costs).</div></div>					





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<b>P1-0406</b>	<b>Correction of Fahrenheit to Celsius Calculation</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Specification Section 09 69 00 1.9 A (IFC Drawings for Main Package dated 3/31/14)  Specification Section 09 69 00 1.9 A requires an ambient temperature between 50-85 degrees F (100-290 degrees C).  The Fahrenheit to Celsius calculation is incorrect. Please revise the requirement.		<b>ANSWER:</b>  REFERENCE: Specification Section 09 69 00 1.9 A (IFC Drawings for Main Package dated 3/31/14)  Specification Section 09 69 00 1.9 A requires an ambient temperature between 50-85 degrees F (100-290 degrees C).  The Fahrenheit to Celsius calculation is incorrect. Please revise the requirement.				
<b>P1-0407</b>	<b>Information on Fire Extinguisher Cabinets</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>10/16/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Specification Section 10 44 13 2.2 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 44 13 2.2 indicates fire extinguisher cabinets are "TBD".  Please provide information on the fire extinguisher cabinets.		<b>ANSWER:</b>  REFERENCE: Specification Section 10 44 13 2.2 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 44 13 2.2 indicates fire extinguisher cabinets are "TBD".  Please provide information on the fire extinguisher cabinets.				
<b>P1-0408</b>	<b>Interior Bird Deterrent Systems Specifications</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Specification Section 10 81 13 1.1 a (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 81 13 1.1 a indicates that Specification Section 10 81 13 specifies bird deterrent systems at the exterior of the building only.  Sheet A1-8251 indicates that bird control is to be installed within the Grand Hall.  Please provide the specification for interior bird deterrent systems.		<b>ANSWER:</b>  REFERENCE: Specification Section 10 81 13 1.1 a (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 81 13 1.1 a indicates that Specification Section 10 81 13 specifies bird deterrent systems at the exterior of the building only.  Sheet A1-8251 indicates that bird control is to be installed within the Grand Hall.  Please provide the specification for interior bird deterrent systems.				





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<b>P1-0409</b>	<b>Drill Holes for the Birdwire System</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>09/04/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 81 13 3.2 C 2 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Specification Section 10 81 13 3.2 C 2 (IFC Drawings for Main Package dated 3/31/14)				
Specification Section 10 81 13 3.2 C 2 indicates that the installer of the Birdwire System is to drill ¼" holes in the substrate for mounting.		Specification Section 10 81 13 3.2 C 2 indicates that the installer of the Birdwire System is to drill ¼" holes in the substrate for mounting.				
These holes may void the warranty of the substrate, and create a possible point of water intrusion.		These holes may void the warranty of the substrate, and create a possible point of water intrusion.				
Please confirm this is acceptable, or revise specification to allow for installation with adhesive only.		Please confirm this is acceptable, or revise specification to allow for installation with adhesive only.				
<b>P1-0410</b>	<b>Painting of ACT-1 Ceiling for B1268</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>09/01/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sheet A1-9602 (ASI 119 dated 6/20/14)		REFERENCE: Sheet A1-9602 (ASI 119 dated 6/20/14)				
Sheet A1-9602 of ASI 119 dictates that ACT-1 ceiling within Room B1268 is to be painted.		Sheet A1-9602 of ASI 119 dictates that ACT-1 ceiling within Room B1268 is to be painted.				
No other ACT-1 ceilings are scheduled to be painted.		No other ACT-1 ceilings are scheduled to be painted.				
Please confirm the ACT-1 ceiling within Room B1268 is not to be painted.		Please confirm the ACT-1 ceiling within Room B1268 is not to be painted.				
<b>P1-0411</b>	<b>ACT Ceiling Type for Room 01223</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>08/22/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sheet A1-9603 (ASI 119 dated 6/20/14)		REFERENCE: Sheet A1-9603 (ASI 119 dated 6/20/14)				
Sheet A1-9603 of ASI 119 calls out the ceiling finish of Room 01223 as "ACT".		Sheet A1-9603 of ASI 119 calls out the ceiling finish of Room 01223 as "ACT".				
Typically on the finish schedules, the type of ACT is called out (i.e. ACT-1).		Typically on the finish schedules, the type of ACT is called out (i.e. ACT-1).				





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	<p>Please confirm all ceilings called out on the finish schedule to be "ACT" are to be Ceiling Type ACT-1.</p>					<p>Please confirm all ceilings called out on the finish schedule to be "ACT" are to be Ceiling Type ACT-1.</p>
<b>P1-0412</b>	<b>AWP-1 Call Out per Specification Section 09 77 23</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>09/05/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE:  Sheet A1-9606 (ASI 122 dated 7/23/14)  Sheet A1-9804 (ASI 118 dated 6/20/14)  Sheet A1-9601 (ASI 119 dated 6/20/14)</p> <p>Sheet A1-9606 indicates that Specification Section 09 77 23 Acoustic Wall Treatment is identified on the finish schedules as AWP-1. AWP-1 does not appear on any of the finish schedules.</p> <p>- Per A1-9804, Rooms B1232 and B1239 have AWP-1 on the walls.</p> <p>- Per A1-9601, the wall finish of B1232 and B1239 is painted gypsum.</p> <p>Please coordinate the finish schedule to show the rooms to receive Acoustic Wall Treatment.</p>					<p><b>ANSWER:</b></p> <p>REFERENCE:  Sheet A1-9606 (ASI 122 dated 7/23/14)  Sheet A1-9804 (ASI 118 dated 6/20/14)  Sheet A1-9601 (ASI 119 dated 6/20/14)</p> <p>Sheet A1-9606 indicates that Specification Section 09 77 23 Acoustic Wall Treatment is identified on the finish schedules as AWP-1. AWP-1 does not appear on any of the finish schedules.</p> <p>- Per A1-9804, Rooms B1232 and B1239 have AWP-1 on the walls.</p> <p>- Per A1-9601, the wall finish of B1232 and B1239 is painted gypsum.</p> <p>Please coordinate the finish schedule to show the rooms to receive Acoustic Wall Treatment.</p>



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<b>P1-0413</b>	<b>Wall Protection Systems Locations per Specification Section 10 26 00</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Specification Section 10 26 00 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Specification Section 10 26 00 (IFC Drawings for Main Package dated 3/31/14)
Specification Section 10 26 00 is for Wall Protection Systems.						Specification Section 10 26 00 is for Wall Protection Systems.
The documents do not identify locations for these products.						The documents do not identify locations for these products.
Please confirm that no products listed within Specification Section 10 26 00 are to be furnished or installed on Phase 1.						Please confirm that no products listed within Specification Section 10 26 00 are to be furnished or installed on Phase 1.
<b>P1-0414</b>	<b>Structural Information for HSS Post and Column Baseplate</b>	<b>Closed</b>	<b>0P</b>	<b>08/22/2014</b>	<b>09/01/2014</b>	<b>09/03/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Detail 1/A1-9858 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Detail 1/A1-9858 (IFC Drawings for Main Package dated 3/31/14)
A.) Detail 1/A1-9858 calls out for: - HSS Post - Column Baseplate secured to Structural Slab Detail 1/A1-9858 indicates the structural information is located on the structural plans. Structural plans do not show this steel.						A.) Detail 1/A1-9858 calls out for: - HSS Post - Column Baseplate secured to Structural Slab Detail 1/A1-9858 indicates the structural information is located on the structural plans. Structural plans do not show this steel.
B.) Detail 3/A1-9858 calls for: - Structural Lintel Beam - Shear Plate welded to HSS and bolted to Lintel Beam - Double Steel Angle spaced at every 3'-0" alternating sides Detail 3/A1-9858 indicates the structural information is located on the structural plans. Structural plans do not show these elements.						B.) Detail 3/A1-9858 calls for: - Structural Lintel Beam - Shear Plate welded to HSS and bolted to Lintel Beam - Double Steel Angle spaced at every 3'-0" alternating sides Detail 3/A1-9858 indicates the structural information is located on the structural plans. Structural plans do not show these elements.
Please provide the referenced structural information for steel members and attachment.						Please provide the referenced structural information for steel members and attachment.



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<b>P1-0415</b>	<b>Pricing Coordination with PG&amp;E</b>	<b>Void</b>	<b>0P</b>	<b>08/26/2014</b>	<b>09/05/2014</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference Note #8 C1-8001 - C1-8005 ASI 123  Note #8 on Civil drawing(s) 8001 - 8005 contains the following: "Contractor shall coordinate with PG&E via TJPA for service connection work prior to start of construction. Location of PG&E service point is undermined and subject to change."  Please provide clarity for the Trade Group bidders on how TJPA would like them to provide pricing, for this unknown element with an undetermined location and/or duration of time necessary for coordination.						<b>ANSWER:</b> Reference Note #8 C1-8001 - C1-8005 ASI 123  Note #8 on Civil drawing(s) 8001 - 8005 contains the following: "Contractor shall coordinate with PG&E via TJPA for service connection work prior to start of construction. Location of PG&E service point is undermined and subject to change."  Please provide clarity for the Trade Group bidders on how TJPA would like them to provide pricing, for this unknown element with an undetermined location and/or duration of time necessary for coordination.
<b>P1-0416</b>	<b>Finish Details for Landscape Drawings</b>	<b>Closed</b>	<b>0P</b>	<b>08/26/2014</b>	<b>08/26/2014</b>	<b>09/01/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference A1-2870, S1-2310 and S1-3201 (IFC Main set)  Architectural drawing A1-2870 directs the contractor to the Structural drawings for information regarding the concrete finish elevations; Structural drawings S1-2310 & S1-3201 provide elevation for top of concrete at the train box only; landscaping drawings do not provide any finish information ¿ (see highlighted section) as this is not included in the landscaping.  Please provide information regarding the highlighted section on the attached drawing L1-2330 and/or drawing number wherein concrete sectional details can be found, including topping slab to concrete on grade. Provide details for elevations, expansion joints, contraction joints, landscaping if any, civil site work if any.						<b>ANSWER:</b> Reference A1-2870, S1-2310 and S1-3201 (IFC Main set)  Architectural drawing A1-2870 directs the contractor to the Structural drawings for information regarding the concrete finish elevations; Structural drawings S1-2310 & S1-3201 provide elevation for top of concrete at the train box only; landscaping drawings do not provide any finish information ¿ (see highlighted section) as this is not included in the landscaping.  Please provide information regarding the highlighted section on the attached drawing L1-2330 and/or drawing number wherein concrete sectional details can be found, including topping slab to concrete on grade. Provide details for elevations, expansion joints, contraction joints, landscaping if any, civil site work if any.
<b>P1-0417</b>	<b>VE Item 4a</b>	<b>Closed</b>	<b>0P</b>	<b>08/26/2014</b>	<b>09/05/2014</b>	<b>09/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference all Landscaping drawings						<b>ANSWER:</b> Reference all Landscaping drawings



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P1-0418	<p>Revision of Corrosion Engineer Requirement Anywhere Stated</p> <p>From: Webcor Construction LP      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Specification Sections requiring a Corrosion Engineer</p> <p>Specifications currently require a corrosion engineer to conduct a component-by-component analysis of potential corrosion and and provide the engineer's report.</p> <p>Per discussions with TJPA, PMPC, and WOJV, the use of a corrosion engineer is no longer required.</p> <p>Please confirm that all specification sections currently requiring a corrosion engineer will be removed and the only requirement is to provide the report as approved by the TJPA Representative.</p>	Closed	0P	08/28/2014	09/07/2014	09/08/2014
	<p>Recently it was discussed that VE item number 4a (see attached); removal the banding at the topping slabs has been accepted by TJPA. Please provide new landscaping drawings and details with information to allow the bidders to incorporate this into their pricing.</p>					
	<p>Recently it was discussed that VE item number 4a (see attached); removal the banding at the topping slabs has been accepted by TJPA. Please provide new landscaping drawings and details with information to allow the bidders to incorporate this into their pricing.</p>					



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<b>P1-0419</b>	<b>Application of Specification Section 01 80 50</b>	<b>Closed</b>	<b>0P</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>09/14/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 01 80 50 1.1 B (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Specification Section 01 80 50 1.1 B (IFC Drawings for Main Package dated 3/31/14)				
Specification Section 01 80 50 1.1 B states, "The requirements of this Section apply to all non-structural components, unless otherwise noted."		Specification Section 01 80 50 1.1 B states, "The requirements of this Section apply to all non-structural components, unless otherwise noted."				
Per previous meetings with PMPC, 01 80 50 is to be specifically referenced in each specification to which it applies.		Per previous meetings with PMPC, 01 80 50 is to be specifically referenced in each specification to which it applies.				
Please provide the areas and specification sections to which Specification Section 01 50 80 Seismic Design Criteria for Nonstructural Components applies.		Please provide the areas and specification sections to which Specification Section 01 50 80 Seismic Design Criteria for Nonstructural Components applies.				
<b>P1-0420</b>	<b>Framing Details for Access Panels Within Gypsum Board Ceilings</b>	<b>Closed</b>	<b>0P</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>09/05/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Access panels will be required within gypsum board ceilings.		Access panels will be required within gypsum board ceilings.				
Typical access panel ceiling framing has not been provided.		Typical access panel ceiling framing has not been provided.				
Please provide the desired framing detail for access panels within gypsum board ceilings.		Please provide the desired framing detail for access panels within gypsum board ceilings.				
<b>P1-0421</b>	<b>VOID</b>	<b>Void</b>	<b>0P</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
<b>P1-0422</b>	<b>Fire Extinguisher Locations per Specification Section 10 44 13 1.3 C 2 c</b>	<b>Closed</b>	<b>0P</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>10/16/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 44 13 1.3 C 2 c		REFERENCE: Specification Section 10 44 13 1.3 C 2				

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<b>P1-0424</b>	<b>Installation of Electrical Components per Specification Section 09 24 00 1.1 C 2</b>	<b>Closed</b>	<b>0P</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>09/17/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 09 24 00 1.1 C 2 (ASI 124 dated 8/18/14)		REFERENCE: Specification Section 09 24 00 1.1 C 2 (ASI 124 dated 8/18/14)				
Per Specification Section 09 24 00 1.1 C 2 of ASI 124, the raceways, wiring, and pathways needed for concealed electrical systems i.e., lighting fixtures, exit lighting, power, way finding, automatic door power, along with low voltage wires (concealed or exposed as indicated in drawings are to be furnished by others, but installed by the Portland Cement Plaster Trade Package.		Per Specification Section 09 24 00 1.1 C 2 of ASI 124, the raceways, wiring, and pathways needed for concealed electrical systems i.e., lighting fixtures, exit lighting, power, way finding, automatic door power, along with low voltage wires (concealed or exposed as indicated in drawings are to be furnished by others, but installed by the Portland Cement Plaster Trade Package.				
TG10.4 - Electrical, Communications, Security, and Integrated Networks, is to furnish and install all electrical work unless specifically referenced in other trade packages (for example, some low voltage is in the Doors and Hardware Trade Package).		TG10.4 - Electrical, Communications, Security, and Integrated Networks, is to furnish and install all electrical work unless specifically referenced in other trade packages (for example, some low voltage is in the Doors and Hardware Trade Package).				
Please remove installation of electrical out of the specification for Specification Section 09 24 00.		Please remove installation of electrical out of the specification for Specification Section 09 24 00.				
<b>P1-0425</b>	<b>Test Methods per Specification Section 09 24 00 1.7 G 2</b>	<b>Closed</b>	<b>0P</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>09/11/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 09 24 00 1.7 G 2 (ASI 124 dated 8/18/14)		REFERENCE: Specification Section 09 24 00 1.7 G 2 (ASI 124 dated 8/18/14)				
Specification Section 09 24 00 1.7 G 2 a indicates that TJPA's Inspection Agent will conduct testing of exterior enclosure welds, "utilizing one of the following test methods."		Specification Section 09 24 00 1.7 G 2 a indicates that TJPA's Inspection Agent will conduct testing of exterior enclosure welds, "utilizing one of the following test methods."				
No test methods are listed.		No test methods are listed.				
Please provide the referenced testing methods.		Please provide the referenced testing methods.				
<b>P1-0426</b>	<b>Confirm the Use of Scanning and Other Means to Locate Reinforcing Bars</b>	<b>Closed</b>	<b>0P</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>09/12/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				



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P1-0428	Portland Cement Plaster Color	Closed	0P	08/28/2014	09/07/2014	09/15/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Specification Section 09 24 00		REFERENCE: Specification Section 09 24 00				
Specification Section 09 24 00 2.7 B indicates that the cement plaster color is to be Gray. No other integral colors are noted.		Specification Section 09 24 00 2.7 B indicates that the cement plaster color is to be Gray. No other integral colors are noted.				
If the cement plaster requires paint, the plaster will need to cure for an extended duration before paint is applied. This will impact the duration the scaffold is in place.		If the cement plaster requires paint, the plaster will need to cure for an extended duration before paint is applied. This will impact the duration the scaffold is in place.				
Please note that TG16.1 Drywall/Framing is currently scoped to leave the scaffold in place if the cement plaster requires painting.		Please note that TG16.1 Drywall/Framing is currently scoped to leave the scaffold in place if the cement plaster requires painting.				
Please confirm that the gray cement plaster is to remain exposed and does not require paint.		Please confirm that the gray cement plaster is to remain exposed and does not require paint.				
P1-0429	Doors 01901A and 01920A per Sheet A1-9703	Closed	0P	09/03/2014	09/13/2014	09/05/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Sheet A1-9703 (ASI 124 dated 8/18/14)		REFERENCE: Sheet A1-9703 (ASI 124 dated 8/18/14)				
Sheet A1-9703 of ASI 124 calls out Doors 01901A and 01920A.		Sheet A1-9703 of ASI 124 calls out Doors 01901A and 01920A.				
The plans do not show these doors.		The plans do not show these doors.				
Please provide a plan showing Doors 01901A and 01920A.		Please provide a plan showing Doors 01901A and 01920A.				
P1-0430	Wall Type Clarification at Stair 601A	Closed	0P	09/03/2014	09/13/2014	09/04/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE:		REFERENCE:				
Sheet A1-2306 (ASI 124 dated 8/18/14)		Sheet A1-2306 (ASI 124 dated 8/18/14)				
Detail 1/A1-7020, Details 2 & 4/A1-7702 (IFC Drawings for Main Package dated 3/31/14)		Detail 1/A1-7020, Details 2 & 4/A1-7702 (IFC Drawings for Main Package dated 3/31/14)				



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	Sheet A1-2306 and Detail 1/A1-7020 call out for Wall Type 61 to be used at Stair 601A.					Sheet A1-2306 and Detail 1/A1-7020 call out for Wall Type 61 to be used at Stair 601A.
	Details 2 & 4/A1-7702 calls out for these walls to be Wall Type 10.					Details 2 & 4/A1-7702 calls out for these walls to be Wall Type 10.
	Please clarify the wall type to be used at this location.					Please clarify the wall type to be used at this location.
<b>P1-0431</b>	<b>Locations Where Wall Type 30 and 61 Support W-5 System</b>	<b>Closed</b>	<b>0P</b>	<b>09/03/2014</b>	<b>09/13/2014</b>	<b>09/05/2014</b>
	<b>From:</b> Webcor Construction LP      Tram Nguyen					
	<b>REQUEST:</b>  REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Sheet A1-0023 Detail 1/A1-7020  Sheet A1-0023 describe Wall Types 30 and 61 as supporting the Colored Glass Cladding System (W-5).  Details (Detail 1/A1-7020, for instance) show the Colored Glass Cladding System (W-5) as being supported separately from Wall Types 30 and 61.  Please confirm Wall Types 30 and 61 are only supporting the W-5 where the W-5 system is specifically shown to hang from Wall Types 30 and 61 on the drawings.					<b>ANSWER:</b>  REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Sheet A1-0023 Detail 1/A1-7020  Sheet A1-0023 describe Wall Types 30 and 61 as supporting the Colored Glass Cladding System (W-5).  Details (Detail 1/A1-7020, for instance) show the Colored Glass Cladding System (W-5) as being supported separately from Wall Types 30 and 61.  Please confirm Wall Types 30 and 61 are only supporting the W-5 where the W-5 system is specifically shown to hang from Wall Types 30 and 61 on the drawings.



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<b>REQUEST:</b>  Reference Specification 09 68 13 IFC Main Set (03/31/2014)  09 68 13 1.7 E and 09 68 13 1.8 A dictate specific environmental requirements for installation of tile carpeting. These requirements may differ from the manufacturer's written installation requirements. Please confirm the specified environmental conditions are to be provided in lieu of the requirements established within manufacturer's written installation instructions, or revise specification to require environmental conditions to meet the manufacturer's written installation instructions.						<b>ANSWER:</b>  Reference Specification 09 68 13 IFC Main Set (03/31/2014)  09 68 13 1.7 E and 09 68 13 1.8 A dictate specific environmental requirements for installation of tile carpeting. These requirements may differ from the manufacturer's written installation requirements. Please confirm the specified environmental conditions are to be provided in lieu of the requirements established within manufacturer's written installation instructions, or revise specification to require environmental conditions to meet the manufacturer's written installation instructions.
<b>P1-0439</b>	<b>Automatic Smoke and Fire Curtains at Elevator Doors</b>	<b>Closed</b>	<b>CR</b>	<b>09/08/2014</b>	<b>09/18/2014</b>	<b>09/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 10 44 00 ASI 121 (07/18/2014)  Specification 10 44 00 section 1.1 A states that automatic smoke and fire curtains are at all elevator doors. The architectural and electrical drawings do not indicate that automatic smoke and fire curtains exist at all elevator doors. Please confirm if all elevator doors receive automatic smoke and fire curtains.  If yes, please revise: 1) the architectural elevator jamb/head details and/or detail callouts and, 2) coordinate revisions with the electrical drawings.  If no, please provide: 1) a matrix of the elevators that require automatic smoke and fire curtains and 2) revise architectural details and/or detail callouts. 3) coordinate revisions with the electrical drawings.  Please note the following coordination conflicts currently exist between the architectural and electrical drawings and should be addressed in the answers above: -E1-2204 shows power for smoke curtains at elevators PE403 and PE404 but the architectural detail for the elevator does not show an automatic smoke and fire						<b>ANSWER:</b>  Reference Specification Section 10 44 00 ASI 121 (07/18/2014)  Specification 10 44 00 section 1.1 A states that automatic smoke and fire curtains are at all elevator doors. The architectural and electrical drawings do not indicate that automatic smoke and fire curtains exist at all elevator doors. Please confirm if all elevator doors receive automatic smoke and fire curtains.  If yes, please revise: 1) the architectural elevator jamb/head details and/or detail callouts and, 2) coordinate revisions with the electrical drawings.  If no, please provide: 1) a matrix of the elevators that require automatic smoke and fire curtains and 2) revise architectural details and/or detail callouts. 3) coordinate revisions with the electrical drawings.  Please note the following coordination conflicts currently exist between the architectural and electrical drawings and should be addressed in the answers above: -E1-2204 shows power for smoke curtains at elevators



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	<p>curtain at the door head/jamb. Coordinate and revise accordingly.</p> <p>-The electrical drawings show power for smoke and fire curtains for Lower Concourse Passenger Elevators 301,302, 502, 503, 704 and 705 but the architectural detail for the elevator does not show an automatic smoke and fire curtain at the door head/jamb. In addition the door head/jamb details are for service instead of passenger elevators. Coordinate and revise accordingly.</p> <p>-Electrical drawings show power for automatic smoke and fire curtains for PE202 on the ground level. Provide elevator door jamb detail for PE202 on the ground level that includes automatic smoke and fire curtain.</p> <p>-PE403, PE404, PE502, PE503, PE704 and PE705 all have details that show a smoke and fire curtain on the ground level. Electrical drawings do not have power for smoke and fire curtains at these locations on the ground level. Coordinate and revise accordingly.</p> <p>-PE403 and PE404 electrical drawings show power for automatic smoke and fire curtains on the second level however there is not a detail for the elevator jamb/head condition that shows the automatic smoke and fire curtain. Coordinate and revise accordingly.</p> <p>-PE704 and PE705 3/A1-7204 references 4/A1-7830 which is regarding stair 501 and not PE704 and PE705. Revise callout for the elevator jamb/head condition for the second level.</p> <p>-PE302, PE303, PE502, and PE503 provide detail of elevator jamb/head condition, electrical drawings show an automatic smoke and fire curtain on the bus deck level for these elevators however there are not details for that.</p>				<p>PE403 and PE404 but the architectural detail for the elevator does not show an automatic smoke and fire curtain at the door head/jamb. Coordinate and revise accordingly.</p> <p>-The electrical drawings show power for smoke and fire curtains for Lower Concourse Passenger Elevators 301,302, 502, 503, 704 and 705 but the architectural detail for the elevator does not show an automatic smoke and fire curtain at the door head/jamb. In addition the door head/jamb details are for service instead of passenger elevators. Coordinate and revise accordingly.</p> <p>-Electrical drawings show power for automatic smoke and fire curtains for PE202 on the ground level. Provide elevator door jamb detail for PE202 on the ground level that includes automatic smoke and fire curtain.</p> <p>-PE403, PE404, PE502, PE503, PE704 and PE705 all have details that show a smoke and fire curtain on the ground level. Electrical drawings do not have power for smoke and fire curtains at these locations on the ground level. Coordinate and revise accordingly.</p> <p>-PE403 and PE404 electrical drawings show power for automatic smoke and fire curtains on the second level however there is not a detail for the elevator jamb/head condition that shows the automatic smoke and fire curtain. Coordinate and revise accordingly.</p> <p>-PE704 and PE705 3/A1-7204 references 4/A1-7830 which is regarding stair 501 and not PE704 and PE705. Revise callout for the elevator jamb/head condition for the second level.</p> <p>-PE302, PE303, PE502, and PE503 provide detail of elevator jamb/head condition, electrical drawings show an automatic smoke and fire curtain on the bus deck level for these elevators however there are not details for that.</p>			
P1-0440	Access Hatch per Detail 4/A1-9220	Closed	0P	09/08/2014	09/18/2014	09/14/2014		



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<hr/>						
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 08 31 20 1.1 A Detail 4/A1-9220						
Specification Section 08 31 20 1.1 A Floor Doors references "single-leaf floor doors at water tanks."						
Detail 4/A1-9220 illustrates a single-leaf door to be installed at the Train Platform Level Water Storage Tank but calls out "Access Hatch". There are no specifications for Access Hatches.						
Please confirm Specification Section 08 31 20 Floor Doors is to be used for all single-leaf doors/access hatches/openings at water tanks or provide an Access Hatch specification.						
<b>P1-0441      Single Colored Polished Concrete at Bus Deck</b>		<b>Closed</b>	<b>0P</b>	<b>09/08/2014</b>	<b>09/18/2014</b>	<b>09/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: Specification Section 01 10 30/APE E.1.12 (IFC Drawings for Main Package dated 3/31/14) Sheets A1-9562 through A1-9567 (ASI 119 dated 6/18/14)						
Per the Deduct Alternates meeting between TJPA, AAI, WOJV, and PCMP on 8/20/14, Alternate No. 24 (Specification Section 01 10 30/APE E.1.12) to substitute Terrazzo floor with Single Color Polished Concrete was accepted.						
Sheets A1-9562 through A1-9567 currently call for Terrazzo flooring at the Bus Deck Level.						
Please provide drawings and specifications that reflect the Single Color Polished Concrete to be installed at the Bus Deck Level.						
<b>ANSWER:</b>						
REFERENCE: Specification Section 01 10 30/APE E.1.12 (IFC Drawings for Main Package dated 3/31/14) Sheets A1-9562 through A1-9567 (ASI 119 dated 6/18/14)						
Per the Deduct Alternates meeting between TJPA, AAI, WOJV, and PCMP on 8/20/14, Alternate No. 24 (Specification Section 01 10 30/APE E.1.12) to substitute Terrazzo floor with Single Color Polished Concrete was accepted.						
Sheets A1-9562 through A1-9567 currently call for Terrazzo flooring at the Bus Deck Level.						
Please provide drawings and specifications that reflect the Single Color Polished Concrete to be installed at the Bus Deck Level.						
<hr/>						
<b>P1-0442</b>	<b>Single Colored Polished Concrete at Beale St. Lobby</b>	<b>Closed</b>	<b>0P</b>	<b>09/08/2014</b>	<b>09/18/2014</b>	<b>09/24/2014</b>











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<b>P1-0446</b>	<b>Terrazzo Control Sample</b>	<b>Closed</b>	<b>0P</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/23/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 09 66 23 2.3.B.5.a.1.b and 09 66 23 2.5.A IFC Main Set (03/31/2014)  09 66 23 2.3.B.5.a.1.b and 09 66 23 2.5.A direct bidders to match TJPAs and the Artists control sample. No control sample has been provided. Please provide the referenced control sample.		<b>ANSWER:</b>  Reference Specification Section 09 66 23 2.3.B.5.a.1.b and 09 66 23 2.5.A IFC Main Set (03/31/2014)  09 66 23 2.3.B.5.a.1.b and 09 66 23 2.5.A direct bidders to match TJPAs and the Artists control sample. No control sample has been provided. Please provide the referenced control sample.				
<b>P1-0447</b>	<b>Custom Color Recipes for Terrazzo</b>	<b>Closed</b>	<b>CR</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/23/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 09 66 23 2.3.B.5.b IFC Main Set (03/31/2014)  09 66 23 2.3.B.5.b directs bidders to budget for up to 20 different custom colors and metallic infill. No final color recipes have been provided. This will not allow for consistent bid inclusions, and may lead to increased cost of the bids. Please provide specific recipes for all of the desired terrazzo colors.		<b>ANSWER:</b>  Reference Specification Section 09 66 23 2.3.B.5.b IFC Main Set (03/31/2014)  09 66 23 2.3.B.5.b directs bidders to budget for up to 20 different custom colors and metallic infill. No final color recipes have been provided. This will not allow for consistent bid inclusions, and may lead to increased cost of the bids. Please provide specific recipes for all of the desired terrazzo colors.				
<b>P1-0448</b>	<b>Bronze Floor Inserts</b>	<b>Closed</b>	<b>0P</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/24/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 09 66 23 2.3.C IFC Main Set (03/31/2014)  09 66 23 2.3.C indicates that 56 bronze floor inserts are to be furnished and installed as part of the terrazzo work. Please see the following and provide responses accordingly:  1) No drawings or specifications (grade of bronze, required finish, patina, etc.) have been provided showing the bronze floor inserts. Please provide these drawings/specifications.		<b>ANSWER:</b>  Reference Specification Section 09 66 23 2.3.C IFC Main Set (03/31/2014)  09 66 23 2.3.C indicates that 56 bronze floor inserts are to be furnished and installed as part of the terrazzo work. Please see the following and provide responses accordingly:  1) No drawings or specifications (grade of bronze, required finish, patina, etc.) have been provided showing the bronze floor inserts. Please provide these drawings/specifications.				



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	<p>2) 09 66 23 2.3.C states that there are 6 different designs of bronze inserts, but the list below only shows 5. Please revise so these are no longer in conflict.</p> <p>3) 09 66 23 2.3.C.5 shows "12 inch TBD". Please provide the missing information that is currently shown as "TBD".</p>					<p>2) 09 66 23 2.3.C states that there are 6 different designs of bronze inserts, but the list below only shows 5. Please revise so these are no longer in conflict.</p> <p>3) 09 66 23 2.3.C.5 shows "12 inch TBD". Please provide the missing information that is currently shown as "TBD".</p>
<b>P1-0449</b>	<b>Terrazzo Anchoring Devices</b>	<b>Closed</b>	<b>0P</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/23/2014</b>
	<p><b>From:</b> Webcor Construction LP      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference Specification Section 09 66 23 2.4.F IFC Main Set (03/31/2014)</p> <p>09 66 23 2.4.F indicates that anchoring devices are as indicated on the drawings. No anchoring devices are shown on the drawings. Please provide anchoring device information.</p>					<p><b>ANSWER:</b></p> <p>Reference Specification Section 09 66 23 2.4.F IFC Main Set (03/31/2014)</p> <p>09 66 23 2.4.F indicates that anchoring devices are as indicated on the drawings. No anchoring devices are shown on the drawings. Please provide anchoring device information.</p>



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<b>P1-0450</b>	<b>Environmental Conditions for Epoxy Terrazzo Flooring</b>	<b>Closed</b>	<b>0P</b>	<b>09/11/2014</b>	<b>09/21/2014</b>	<b>09/23/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification Section 09 66 23 1.9.A IFC Main Set (03/31/2014)			Reference Specification Section 09 66 23 1.9.A IFC Main Set (03/31/2014)			
09 66 23 1.9.A states ¿The minimum slab temperature shall be 60 degrees F before commencing installation, during installation, and for at least 72 hours after installation is complete. The substrate temperature must be at least 5 degrees F above the dew point during installation.¿ The Manufacturer's written installation instructions state "Throughout the application process, substrate temperature should be 60°F ¿ 90°F. Substrate temperature must be at least 5°F above the dew point. Applications on concrete substrate should occur while temperature is falling to lessen offgassing. The material should not be applied in direct sunlight, if possible. Protect material from freezing prior to installation.." The specifications do not match manufacturer¿s written installation instructions. Please confirm the environmental conditions required within 09 66 22 shall override the manufacturer¿s written installation instructions.			09 66 23 1.9.A states ¿The minimum slab temperature shall be 60 degrees F before commencing installation, during installation, and for at least 72 hours after installation is complete. The substrate temperature must be at least 5 degrees F above the dew point during installation.¿ The Manufacturer's written installation instructions state "Throughout the application process, substrate temperature should be 60°F ¿ 90°F. Substrate temperature must be at least 5°F above the dew point. Applications on concrete substrate should occur while temperature is falling to lessen offgassing. The material should not be applied in direct sunlight, if possible. Protect material from freezing prior to installation.." The specifications do not match manufacturer¿s written installation instructions. Please confirm the environmental conditions required within 09 66 22 shall override the manufacturer¿s written installation instructions.			



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P1-0451	Environmental Conditions for Terrazzo Epoxy Crack Suppression System	Closed	0P	09/11/2014	09/21/2014	09/23/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference Specification Section 09 66 22 1.8.A IFC Main Set (03/31/2014)		Reference Specification Section 09 66 22 1.8.A IFC Main Set (03/31/2014)				
09 66 22 1.8.A states ¿The minimum slab temperature shall be 60 degrees F before commencing installation, during installation, and for at least 72 hours after installation is complete. The substrate temperature must be at least 5 degrees F above the dew point during installation.¿ The Manufacturer's written installation instructions state "During installation and initial cure cycle substrate and ambient air temperature must be at a minimum of 60°F (16°C). Substrate temperature must be at least 5°F (3°C) above the dew point. The specifications do not match manufacturer¿s written installation instructions. Please confirm the environmental conditions required within 09 66 22 shall override the manufacturer¿s written installation instructions.		09 66 22 1.8.A states ¿The minimum slab temperature shall be 60 degrees F before commencing installation, during installation, and for at least 72 hours after installation is complete. The substrate temperature must be at least 5 degrees F above the dew point during installation.¿ The Manufacturer's written installation instructions state "During installation and initial cure cycle substrate and ambient air temperature must be at a minimum of 60°F (16°C). Substrate temperature must be at least 5°F (3°C) above the dew point. The specifications do not match manufacturer¿s written installation instructions. Please confirm the environmental conditions required within 09 66 22 shall override the manufacturer¿s written installation instructions.				
P1-0452	Attached Electronic File for Terrazzo	Closed	0P	09/11/2014	09/21/2014	09/24/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference Specification Section 09 66 23 1.1.A.2 and 1.3.D IFC Main Set (03/31/2014)		Reference Specification Section 09 66 23 1.1.A.2 and 1.3.D IFC Main Set (03/31/2014)				
09 66 23 1.1.A.2 and 09 66 23 1.3.D reference an ¿attached electronic file¿. No electronic file for Terrazzo has been provided. Please provide the referenced electronic file.		09 66 23 1.1.A.2 and 09 66 23 1.3.D reference an ¿attached electronic file¿. No electronic file for Terrazzo has been provided. Please provide the referenced electronic file.				
P1-0453	Bicycle Wheel Channel in Terrazzo Treads	Closed	0P	09/11/2014	09/21/2014	09/23/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference Specification Section 09 66 23 1.1.A.5 and 1.3.E.2 IFC Main Set (03/31/2014)		Reference Specification Section 09 66 23 1.1.A.5 and 1.3.E.2 IFC Main Set (03/31/2014)				
09 66 23 1.1.A.5 and 09 66 23 1.3.E.2 call out for a		09 66 23 1.1.A.5 and 09 66 23 1.3.E.2 call out for a				



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P1-0454	<p>stainless steel bicycle wheel channel in terrazzo treads. Details at terrazzo stairs do not call out a bicycle wheel channel. Please confirm no bicycle wheel channel is required at terrazzo treads.</p> <p><b>Terrazzo Color Schemes</b></p> <p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference Specification Section 09 66 23 1.3.E.1 and 1.4.D.1 IFC Main Set (03/31/2014)</p> <p>09 66 23 1.3.E.1 and 09 66 23 1.4.D.1 mention color schemes/ terrazzo color identification. No color schemes/ terrazzo color identification has been provided. Please provide color schemes/ Terrazzo color identification so that the terrazzo can be accurately priced.</p>	Closed	0P	09/11/2014	09/21/2014	09/23/2014
	<p><b>ANSWER:</b></p> <p>Reference Specification Section 09 66 23 1.3.E.1 and 1.4.D.1 IFC Main Set (03/31/2014)</p> <p>09 66 23 1.3.E.1 and 09 66 23 1.4.D.1 mention color schemes/ terrazzo color identification. No color schemes/ terrazzo color identification has been provided. Please provide color schemes/ Terrazzo color identification so that the terrazzo can be accurately priced.</p>					
P1-0455	<p><b>Unit Price for Epoxy Flooring</b></p> <p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference Specification Section 09 66 23 1.4.C.1 IFC Main Set (03/31/2014) and 01 10 20 Div. 00/01 (08/11/2014)</p> <p>09 66 23 1.4.C.1 states to provide a unit price per 2 (two) samples of 6 inch square epoxy flooring in excess of 20 colors. 01 10 20 (and its appendices) do not call out this unit price. Please revise these specifications to match.</p>	Closed	0P	09/11/2014	09/21/2014	09/23/2014
	<p><b>ANSWER:</b></p> <p>Reference Specification Section 09 66 23 1.4.C.1 IFC Main Set (03/31/2014) and 01 10 20 Div. 00/01 (08/11/2014)</p> <p>09 66 23 1.4.C.1 states to provide a unit price per 2 (two) samples of 6 inch square epoxy flooring in excess of 20 colors. 01 10 20 (and its appendices) do not call out this unit price. Please revise these specifications to match.</p>					



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P1-0456	LEED Credit MR5 for Terrazzo	Closed	0P	09/11/2014	09/11/2014	09/24/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference Specification Section 09 66 23 1.5.C IFC Main Set (03/31/2014)		Reference Specification Section 09 66 23 1.5.C IFC Main Set (03/31/2014)				
09 66 23 1.5.C calls out for LEED Credit MR 5, which requests information on the terrazzo's materials extraction, processing and location of manufacture. No specific aggregates have been specified for this scope of work. Please confirm there is no minimum amount of material for 09 66 23 which must be extracted, processed and manufactured within the straight-line total travel distance described in 09 66 23 1.5.C.		09 66 23 1.5.C calls out for LEED Credit MR 5, which requests information on the terrazzo's materials extraction, processing and location of manufacture. No specific aggregates have been specified for this scope of work. Please confirm there is no minimum amount of material for 09 66 23 which must be extracted, processed and manufactured within the straight-line total travel distance described in 09 66 23 1.5.C.				
P1-0457	FI Requirements for Terrazzo	Closed	0P	09/12/2014	09/22/2014	11/19/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference Specification Section 03 30 02, 09 66 22, 09 66 23 IFC Main Set (03/31/2014)		Reference Specification Section 03 30 02, 09 66 22, 09 66 23 IFC Main Set (03/31/2014)				
03 30 02 3.6.A.2 states to comply with dimensional tolerance limitations given by ACI 117 for Concrete Finishes, and 03 30 02 3.6.A.4.c states "FI numbers shall not apply to unshored slabs or shored slabs with camber." ACI 117 4.4.1 states that slabs on structural steel or precast concrete have no deviation from elevation requirement. However, 09 66 22 3.2.B states that the Terrazzo Trade Subcontractor must level the floor to a specific FI, and 09 66 23 3.3.F refers to 09 66 22 3.2.B for FI requirements for Epoxy Terrazzo Flooring. As such, the concrete filled metal pan deck and topping slabs have a FI requirement which conflicts with the allowable FI referenced in 09 66 22 and 09 66 23. In addition, the concrete substrate may continue to move after installation of terrazzo. Please provide direction on installation of terrazzo epoxy crack suppression and epoxy terrazzo flooring where their FI requirements conflict with the FI requirements of the concrete substrate.		03 30 02 3.6.A.2 states to comply with dimensional tolerance limitations given by ACI 117 for Concrete Finishes, and 03 30 02 3.6.A.4.c states "FI numbers shall not apply to unshored slabs or shored slabs with camber." ACI 117 4.4.1 states that slabs on structural steel or precast concrete have no deviation from elevation requirement. However, 09 66 22 3.2.B states that the Terrazzo Trade Subcontractor must level the floor to a specific FI, and 09 66 23 3.3.F refers to 09 66 22 3.2.B for FI requirements for Epoxy Terrazzo Flooring. As such, the concrete filled metal pan deck and topping slabs have a FI requirement which conflicts with the allowable FI referenced in 09 66 22 and 09 66 23. In addition, the concrete substrate may continue to move after installation of terrazzo. Please provide direction on installation of terrazzo epoxy crack suppression and epoxy terrazzo flooring where their FI requirements conflict with the FI requirements of the concrete substrate.				



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<b>P1-0458</b>	<b>Overhead Coiling Doors and Grilles Interface Cabinet Location</b>	<b>Closed</b>	<b>0P</b>	<b>09/12/2014</b>	<b>09/22/2014</b>	<b>09/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Specification 08 33 23 2.3.B and 08 33 16 Section 2.3.B IFC Main Set (03/31/2014)			Specification 08 33 23 2.3.B and 08 33 16 Section 2.3.B IFC Main Set (03/31/2014)			
Specifications state ¿TJPA Representative prior to installation shall approve the exact style and finish of each cabinet.¿ Please provide the exact style and finish for the interface cabinet required for the overhead coiling doors and grilles so that the bidders will know what to price.			Specifications state ¿TJPA Representative prior to installation shall approve the exact style and finish of each cabinet.¿ Please provide the exact style and finish for the interface cabinet required for the overhead coiling doors and grilles so that the bidders will know what to price.			
<b>P1-0459</b>	<b>Aluminum Enclosures at Jambs</b>	<b>Closed</b>	<b>0P</b>	<b>09/15/2014</b>	<b>09/25/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Specification 08 33 16 and 08 33 23 2.6.H IFC Main Set (03/31/2014)			Specification 08 33 16 and 08 33 23 2.6.H IFC Main Set (03/31/2014)			
This section is specific to Aluminum Enclosures at Jambs for the overhead coiling doors and grilles. Detail 2/A1-8153 and 2/A1-8154 show aluminum head conditions for the overhead doors and grilles. Please confirm the aluminum finish at the jamb is the same as the finish for the aluminum head closures. If not provide finish for aluminum head closures.			This section is specific to Aluminum Enclosures at Jambs for the overhead coiling doors and grilles. Detail 2/A1-8153 and 2/A1-8154 show aluminum head conditions for the overhead doors and grilles. Please confirm the aluminum finish at the jamb is the same as the finish for the aluminum head closures. If not provide finish for aluminum head closures.			
<b>P1-0460</b>	<b>Missing Callout on Bathroom Elevation Drawings</b>	<b>Closed</b>	<b>0P</b>	<b>09/16/2014</b>	<b>09/26/2014</b>	<b>09/29/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference A1-9060 IFC Main Set (03/31/2014)			Reference A1-9060 IFC Main Set (03/31/2014)			
Details 4-8 on A1-9060 show bathroom elevation drawings. There is no detail showing the transition from wall tile to painted GWB (This is shown as 2 black lines, see red mark-ups on the attached drawing for clarification). Please provide details for the transition from wall tile to painted GWB.			Details 4-8 on A1-9060 show bathroom elevation drawings. There is no detail showing the transition from wall tile to painted GWB (This is shown as 2 black lines, see red mark-ups on the attached drawing for clarification). Please provide details for the transition from wall tile to painted GWB.			



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<b>P1-0462</b>	<b>Thickness Requirement for Insulation or Topping Slab</b>	<b>Closed</b>	<b>0P</b>	<b>09/17/2014</b>	<b>09/27/2014</b>	<b>09/29/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b> REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Sheet A1-2980 Detail 2/A1-2982 Specification Section 07 21 00 2.3 B Details 4 & 5/A1-9321 (ASI 123 dated 3/31/14)  1.) Expanded Polystyrene Insulation (INS-2) is called for on Sheet A1-2980 at the Lower Concourse Level and Detail 2/A1-2982 at the Grand Hall. The sheet notes refer to Specification Section 07 21 00 for additional information on insulation types. Specification Section 07 21 00 2.3 B states "thickness as indicated".  2.) Details 4 & 5/A1-9321 call for insulation at the Bus Deck Level.  Plans and sections do not show the thickness of the the slab or insulation at the Lower Concourse Level, Grand Hall, or Bus Deck Level.  Please provide the thickness required for the insulation or topping slab to be installed at the Lower Concourse Level, Grand Hall, and Bus Deck Level.			<b>ANSWER:</b> REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Sheet A1-2980 Detail 2/A1-2982 Specification Section 07 21 00 2.3 B Details 4 & 5/A1-9321 (ASI 123 dated 3/31/14)  1.) Expanded Polystyrene Insulation (INS-2) is called for on Sheet A1-2980 at the Lower Concourse Level and Detail 2/A1-2982 at the Grand Hall. The sheet notes refer to Specification Section 07 21 00 for additional information on insulation types. Specification Section 07 21 00 2.3 B states "thickness as indicated".  2.) Details 4 & 5/A1-9321 call for insulation at the Bus Deck Level.  Plans and sections do not show the thickness of the the slab or insulation at the Lower Concourse Level, Grand Hall, or Bus Deck Level.  Please provide the thickness required for the insulation or topping slab to be installed at the Lower Concourse Level, Grand Hall, and Bus Deck Level.			



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<b>P1-0463</b>	<b>Topping Slab Requirements for Rooms B1365, B1387, B1390</b>	<b>Closed</b>	<b>0P</b>	<b>09/17/2014</b>	<b>09/27/2014</b>	<b>09/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (ASI 121 dated 7/18/14) Sheet A1-2203 Sheet A1-9523		REFERENCE: (ASI 121 dated 7/18/14) Sheet A1-2203 Sheet A1-9523				
Sheet A1-2203 refers to Sheet A1-9523 for Slab Topping Details for rooms B1365 (Oversized Equipment Storage), B1387 (Maintenance Equipment Repair Shop), and B1390 (Maintenance Workshop).		Sheet A1-2203 refers to Sheet A1-9523 for Slab Topping Details for rooms B1365 (Oversized Equipment Storage), B1387 (Maintenance Equipment Repair Shop), and B1390 (Maintenance Workshop).				
Sheet A1-9523 does not require traffic rated topping slabs for rooms B1365, B1387, or B1390. However, these rooms are indicated for the use of oversized equipment storage and maintenance.		Sheet A1-9523 does not require traffic rated topping slabs for rooms B1365, B1387, or B1390. However, these rooms are indicated for the use of oversized equipment storage and maintenance.				
Please confirm rooms B1365 (Oversized Equipment Storage), B1387 (Maintenance Equipment Repair Shop), and B1390 (Maintenance Workshop) are not to receive traffic rated topping slabs.		Please confirm rooms B1365 (Oversized Equipment Storage), B1387 (Maintenance Equipment Repair Shop), and B1390 (Maintenance Workshop) are not to receive traffic rated topping slabs.				
<b>P1-0464</b>	<b>Overhead Door at SE201</b>	<b>Closed</b>	<b>0P</b>	<b>09/17/2014</b>	<b>09/27/2014</b>	<b>09/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference A1-2202 ASI 121 (07/18/2014)		Reference A1-2202 ASI 121 (07/18/2014)				
Service Elevator SE201 has been deferred. Please provide direction as to whether or not the rated overhead door on the lower concourse for SE 201 should be installed at this time.		Service Elevator SE201 has been deferred. Please provide direction as to whether or not the rated overhead door on the lower concourse for SE 201 should be installed at this time.				
<b>P1-0465</b>	<b>Smoke Curtains at PE203</b>	<b>Closed</b>	<b>0P</b>	<b>09/17/2014</b>	<b>10/11/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference A1-2302 (ASI 119 06/20/2014)		Reference A1-2302 (ASI 119 06/20/2014)				
Passenger Elevator 203 has been deferred. Are the smoke curtains for that elevator deferred as well or are they to be installed now as part of TG08.9?		Passenger Elevator 203 has been deferred. Are the smoke curtains for that elevator deferred as well or are they to be installed now as part of TG08.9?				

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P1-0466	Sloped Tile on Bathroom Floor Plans	Open	0P	09/18/2014	09/28/2014	10/31/2014
<div><div>From: Webcor Construction LP</div><div>Andrew Kitchen</div></div>						
REQUEST:			ANSWER:			
Reference: 2/A1-9001, 2/A1-9002, 2/A1-9004, 2/A1-9012, 2/A1-9015, 2/A1-9017, 2/A1-9021, 2/A1-9023, 2/A1-9024, 3/A1-9033, 3/A1-9034, 5/A1-9041, 10/A1-9042 (IFC Main Set 03/31/2014)			Reference: 2/A1-9001, 2/A1-9002, 2/A1-9004, 2/A1-9012, 2/A1-9015, 2/A1-9017, 2/A1-9021, 2/A1-9023, 2/A1-9024, 3/A1-9033, 3/A1-9034, 5/A1-9041, 10/A1-9042 (IFC Main Set 03/31/2014)			
Bathroom floor plans show sloping adjacent to the drain, but flat at the remainder of the field tile (see 2/A1-9001, 2/A1-9002, 2/A1-9004, 2/A1-9012, 2/A1-9015, 2/A1-9017, 2/A1-9021, 2/A1-9023, 2/A1-9024, 3/A1-9033, 3/A1-9034). 5/A1-9041 and 10/A1-9042 call out for a setting bed on topping slab at the restroom floors. Please confirm only the areas around the floor drains, as shown on the floor plans, are to be sloped, and that a setting bed is not required where tile is not sloped (i.e. the tile is thin set on the topping slab).			Bathroom floor plans show sloping adjacent to the drain, but flat at the remainder of the field tile (see 2/A1-9001, 2/A1-9002, 2/A1-9004, 2/A1-9012, 2/A1-9015, 2/A1-9017, 2/A1-9021, 2/A1-9023, 2/A1-9024, 3/A1-9033, 3/A1-9034). 5/A1-9041 and 10/A1-9042 call out for a setting bed on topping slab at the restroom floors. Please confirm only the areas around the floor drains, as shown on the floor plans, are to be sloped, and that a setting bed is not required where tile is not sloped (i.e. the tile is thin set on the topping slab).			
P1-0467	Metal Ceiling Connection to W-2	Closed	0P	09/19/2014	09/29/2014	10/28/2014
<div><div>From: Webcor Construction LP</div><div>Andrew Kitchen</div></div>						
REQUEST:			ANSWER:			
Reference 2/A1-7513 ASI 127			Reference 2/A1-7513 ASI 127			
2/A1-7513 shows a closure piece that is not fastened or sealed to the W-2 support. Please confirm if a fastener and/or sealant is provided at this locations. If yes, please provide details to fill void between closure piece and W-2 support.			2/A1-7513 shows a closure piece that is not fastened or sealed to the W-2 support. Please confirm if a fastener and/or sealant is provided at this locations. If yes, please provide details to fill void between closure piece and W-2 support.			
P1-0468	Metal Ceiling Field Modifications	Closed	0P	09/19/2014	09/29/2014	10/16/2014
<div><div>From: Webcor Construction LP</div><div>Andrew Kitchen</div></div>						
REQUEST:			ANSWER:			
Reference Specification Section 09 51 23 2.9.A.8 ASI 127			Reference Specification Section 09 51 23 2.9.A.8 ASI 127			
09 51 23 23 2.9.A.8 discusses creating penetrations in metal ceiling panels and prohibits any field modifications. Please confirm if this language applies only to penetrations or if it applies to modifying panels at edge			09 51 23 23 2.9.A.8 discusses creating penetrations in metal ceiling panels and prohibits any field modifications. Please confirm if this language applies			

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	<p>condition as well. Where the panels meet finishes installed by others and the structural steel superstructure it's likely that field modifications will be required to accommodate as-built conditions. Please confirm that this is acceptable.</p>					<p>only to penetrations or if it applies to modifying panels at edge condition as well. Where the panels meet finishes installed by others and the structural steel superstructure it's likely that field modifications will be required to accommodate as-built conditions. Please confirm that this is acceptable.</p>
<b>P1-0469</b>	<b>Power Bollards</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/20/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference Specification Section 12 93 00 IFC Main Set (03/31/2014)</p> <p>The above referenced specification includes Power Bollards. No Power Bollards are shown on the drawings. Please provide locations for Power Bollards, and all details or remove from specification.</p>					<p><b>ANSWER:</b></p> <p>Reference Specification Section 12 93 00 IFC Main Set (03/31/2014)</p> <p>The above referenced specification includes Power Bollards. No Power Bollards are shown on the drawings. Please provide locations for Power Bollards, and all details or remove from specification.</p>
<b>P1-0470</b>	<b>Cafe Chairs and Tables</b>	<b>Void</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	
	<p><b>From:</b> Webcor Construction LP                      Andrew Kitchen</p> <p><b>REQUEST:</b></p> <p>Reference Specification Section 12 93 00 1.1.A.6 and 1.1.A.7 IFC Main Set (03/31/2014)</p> <p>The above referenced specification shows Café Chairs and Café Tables. Clarify if these are meant to be purchased with the TG13.1 Rooftop Landscaping package or if the TJPA wants to procure them at a later time once plans for the rooftop cafe are finalized. At the moment the rooftop cafe is a deductive alternate with a temporary lid over the stem walls.</p>					<p><b>ANSWER:</b></p> <p>Reference Specification Section 12 93 00 1.1.A.6 and 1.1.A.7 IFC Main Set (03/31/2014)</p> <p>The above referenced specification shows Café Chairs and Café Tables. Clarify if these are meant to be purchased with the TG13.1 Rooftop Landscaping package or if the TJPA wants to procure them at a later time once plans for the rooftop cafe are finalized. At the moment the rooftop cafe is a deductive alternate with a temporary lid over the stem walls.</p>



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P1-0471	Power Bollard Locations	Closed	0P	09/23/2014	10/03/2014	10/01/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference Specification Section 12 93 00 1.4.C.1 IFC Main Set (03/31/2014)		Reference Specification Section 12 93 00 1.4.C.1 IFC Main Set (03/31/2014)				
Clarify that the reference to bollards in this statement refers only to power bollards and not to any other types of bollards. Clarify where the power bollards are located.		Clarify that the reference to bollards in this statement refers only to power bollards and not to any other types of bollards. Clarify where the power bollards are located.				
P1-0472	Rope Net Structure on Play Structure	Closed	0P	09/23/2014	10/03/2014	10/01/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference Specification Section 12 93 20 2.3.A IFC Main Set (03/31/2014)		Reference Specification Section 12 93 20 2.3.A IFC Main Set (03/31/2014)				
Specification Section 12 93 20 2.3.A.1 for the Play Structure states "2500: CUSTOM ROPE NET STRUCTURE 'Transbay Transit Center' (2550S)" and Specification Section 12 93 20 2.3.A.2 states "Or Equal."		Specification Section 12 93 20 2.3.A.1 for the Play Structure states "2500: CUSTOM ROPE NET STRUCTURE 'Transbay Transit Center' (2550S)" and Specification Section 12 93 20 2.3.A.2 states "Or Equal."				
a. Has the design team set this up with the manufacturers specifically for this project?		a. Has the design team set this up with the manufacturers specifically for this project?				
b. If so, how does the Trade Subcontractor produce an equal product?		b. If so, how does the Trade Subcontractor produce an equal product?				
P1-0473	Color and Finish Details for Steel Posts	Closed	0P	09/23/2014	10/03/2014	10/01/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference Specification Section 12 93 20 2.4.I IFC Main Set (03/31/2014)		Reference Specification Section 12 93 20 2.4.I IFC Main Set (03/31/2014)				
Specification calls for custom color on the steel posts, but doesn't provide any details. Please provide custom color and finish details.		Specification calls for custom color on the steel posts, but doesn't provide any details. Please provide custom color and finish details.				
P1-0474	Labor by Building Contractor Statement	Closed	0P	09/23/2014	10/03/2014	10/02/2014
From: Webcor Construction LP                      Andrew Kitchen						

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<b>P1-0477</b>	<b>Sand Layer</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/02/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference Specification Section 32 15 10 1.3G, 32 14 40 IFC Main Set (03/31/2014)  Specification states "Sand Layer: refer to 32 14 40". Specification 32 14 40 does not mention anything about a Sand Layer. Please define Sand Layer.		<b>ANSWER:</b> Reference Specification Section 32 15 10 1.3G, 32 14 40 IFC Main Set (03/31/2014)  Specification states "Sand Layer: refer to 32 14 40". Specification 32 14 40 does not mention anything about a Sand Layer. Please define Sand Layer.				
<b>P1-0478</b>	<b>RPM Color</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference Specification Section 32 15 10 2.2.B IFC Main Set (03/31/2014)  Specification states that the RPM color is to be selected by TJPA Representative. Please define the color and finish of the RPM.		<b>ANSWER:</b> Reference Specification Section 32 15 10 2.2.B IFC Main Set (03/31/2014)  Specification states that the RPM color is to be selected by TJPA Representative. Please define the color and finish of the RPM.				
<b>P1-0479</b>	<b>Humidity or Fog Limitations for RPM</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference Specification Section 32 15 10 3.4.A IFC Main Set (03/31/2014)  Specification references the weather limitations for RPM, but does not mention any humidity or fog limitations. Are there any humidity or fog limitations? If so, please provide.		<b>ANSWER:</b> Reference Specification Section 32 15 10 3.4.A IFC Main Set (03/31/2014)  Specification references the weather limitations for RPM, but does not mention any humidity or fog limitations. Are there any humidity or fog limitations? If so, please provide.				
<b>P1-0480</b>	<b>Missing Sections in 32 15 10</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/02/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference Specification Section 32 15 10 3.12.A.2 IFC Main Set (03/31/2014)  Specification references Section 3.11.A.1 and 3.11.A.3. These sections do not exist in Specification 32 15 10.		<b>ANSWER:</b> Reference Specification Section 32 15 10 3.12.A.2 IFC Main Set (03/31/2014)  Specification references Section 3.11.A.1 and 3.11.A.3. These sections do not exist in Specification				





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	Please revise section 3.12.A.2 to reference the correct sections.					32 15 10. Please revise section 3.12.A.2 to reference the correct sections.
<b>P1-0481</b>	<b>Incorrect Reference in 32 15 10</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/02/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference Specification Section 32 15 10 3.12.B.2 IFC Main Set (03/31/2014)  Specification references Section 3.11.B.1. This is an incorrect reference. Please revise section 3.12.B.2 to reference the correct section.						<b>ANSWER:</b> Reference Specification Section 32 15 10 3.12.B.2 IFC Main Set (03/31/2014)  Specification references Section 3.11.B.1. This is an incorrect reference. Please revise section 3.12.B.2 to reference the correct section.
<b>P1-0482</b>	<b>Geotextile Fabric Source</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference Specification Section 32 11 24 2.1.A, 2.2.A IFC Main Set (03/31/2014)  Specification says that 100% of materials are to be sourced regionally. The Geotextile Fabric manufacturer listed in this Specification is from Georgia, which would cause great difficulty in achieving this credit. Please revise the Specification so that the LEED credit is achievable with the given manufacturers.						<b>ANSWER:</b> Reference Specification Section 32 11 24 2.1.A, 2.2.A IFC Main Set (03/31/2014)  Specification says that 100% of materials are to be sourced regionally. The Geotextile Fabric manufacturer listed in this Specification is from Georgia, which would cause great difficulty in achieving this credit. Please revise the Specification so that the LEED credit is achievable with the given manufacturers.



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<b>P1-0483</b>	<b>Recycled Content Requirements</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/21/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 32 11 24 2.1.A IFC Main Set (03/31/2014)  Recycled content submittal requirements are included in Part 1. Since there is no LEED MRc4 requirement listed in section 2.1, confirm that there are no recycled content requirements.						<b>ANSWER:</b>  Reference Specification Section 32 11 24 2.1.A IFC Main Set (03/31/2014)  Recycled content submittal requirements are included in Part 1. Since there is no LEED MRc4 requirement listed in section 2.1, confirm that there are no recycled content requirements.
<b>P1-0484</b>	<b>MRc5 Option 2 Language</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>12/03/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 32 01 90 1.5.B and 2.1.A IFC Main Set (03/31/2014)  In Specification 32 01 90 1.5B and 2.1.A, MRc5 Option 2 is not listed. The LEED language in this Specification needs to match Specification 01 81 13. Please revise 32 01 90 to add MRc5 Option 2.						<b>ANSWER:</b>  Reference Specification Section 32 01 90 1.5.B and 2.1.A IFC Main Set (03/31/2014)  In Specification 32 01 90 1.5B and 2.1.A, MRc5 Option 2 is not listed. The LEED language in this Specification needs to match Specification 01 81 13. Please revise 32 01 90 to add MRc5 Option 2.
<b>P1-0485</b>	<b>2 Year Maintenance Period After Final Completion</b>	<b>Void</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference Specification Section 32 01 90 3.2.A IFC Main Set (03/31/2014)  Specification states "Continuously maintain plants, planted areas, and irrigation system during progress of Work, and for a minimum period of 2 years after date of Final Completion and until the TJPA accepts maintenance responsibility." Confirm that TJPA wants to bid the 2 year maintenance period after Final Completion.						<b>ANSWER:</b>  Reference Specification Section 32 01 90 3.2.A IFC Main Set (03/31/2014)  Specification states "Continuously maintain plants, planted areas, and irrigation system during progress of Work, and for a minimum period of 2 years after date of Final Completion and until the TJPA accepts maintenance responsibility." Confirm that TJPA wants to bid the 2 year maintenance period after Final Completion.
<b>P1-0486</b>	<b>Maintenance Fertilization Program</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	<p>Reference Specification Section 32 01 90 2.2.C IFC Main Set (03/31/2014)</p> <p>Clarify where the details of "the maintenance fertilization program accepted by the TJPA Representative" may be found. Please provide all details of the "maintenance fertilization program."</p>					
<b>P1-0487</b>	<b>Maintenance Period Charges</b>	<b>Void</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> <p>Reference Specification Section 32 01 90 3.9.B.2 IFC Main Set (03/31/2014)</p> <p>Specification states "Continue maintenance of landscape Work until the date that the TJPA accepts maintenance responsibility." Confirm that for pricing, the trade subcontractor should include two years worth of maintenance and then a monthly fee will be charged for any time period extending beyond the two years. Confirm if the acceptance of TJPA maintenance responsibility will be in writing.</p>						
<hr/>						
<b>P1-0488</b>	<b>MRc5 Option 2 Language</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>11/21/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> <p>Reference Specification Section 32 91 20 1.2.A.2 and 2.1.A IFC Main Set (03/31/2014)</p> <p>In Specification 32 91 20 1.2.A.2 and 2.1.A, MRc5 Option 2 is not listed. The LEED language in this Specification needs to match Specification 01 81 13. Please revise 32 91 20 to add MRc5 Option 2.</p>						



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<b>P1-0489</b>	<b>Pesticides and Herbicides on Greywater Treatment Wetland</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification Section 32 91 20 2.2.E IFC Main Set (03/31/2014)		Reference Specification Section 32 91 20 2.2.E IFC Main Set (03/31/2014)				
This states that "The use of pesticides and herbicides is forbidden." Confirm that this applies only to the Greywater Treatment Wetland and not to any other portion of the roof.		This states that "The use of pesticides and herbicides is forbidden." Confirm that this applies only to the Greywater Treatment Wetland and not to any other portion of the roof.				
<b>P1-0490</b>	<b>Bid Alternates in Technical Specifications</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/03/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification Section 32 91 20 2.2.F IFC Main Set (03/31/2014)		Reference Specification Section 32 91 20 2.2.F IFC Main Set (03/31/2014)				
Section 32 91 20 2.2.F lists a Bid Alternate. Bid Alternates should not be in the technical specifications, but instead placed in the Alternate Specification. Remove from this specification section and place in the Alternate specification.		Section 32 91 20 2.2.F lists a Bid Alternate. Bid Alternates should not be in the technical specifications, but instead placed in the Alternate Specification. Remove from this specification section and place in the Alternate specification.				
<b>P1-0491</b>	<b>Geosynthetic Fill Requirements</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification Section 32 34 10 3.3.B IFC Main Set (03/31/2014)		Reference Specification Section 32 34 10 3.3.B IFC Main Set (03/31/2014)				
Confirm that these requirements are in alignment with and do not conflict with any concrete specifications.		Confirm that these requirements are in alignment with and do not conflict with any concrete specifications.				
<b>P1-0492</b>	<b>Bid Alternates in Technical Specifications</b>	<b>Closed</b>	<b>0P</b>	<b>09/23/2014</b>	<b>10/03/2014</b>	<b>10/03/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification Section 32 14 40 2.2.A.2, 2.3.A.2, 2.3.B.2 IFC Main Set (03/31/2014)		Reference Specification Section 32 14 40 2.2.A.2, 2.3.A.2, 2.3.B.2 IFC Main Set (03/31/2014)				
Section 32 14 40 2.2.A.2 lists a Bid Alternate. Bid		Section 32 14 40 2.2.A.2 lists a Bid Alternate. Bid				



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P1-0495	Unit Pricing	Closed	0P	09/23/2014	10/03/2014	12/03/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference Specification Section 32 91 30 3.6.A.1 IFC Main Set (03/31/2014)		Reference Specification Section 32 91 30 3.6.A.1 IFC Main Set (03/31/2014)				
This section states to "Provide allowance for 15 eight-hour days of adjustment grading work with a work crew of six using rakes and shovels to smooth and shape the planting area surfaces. Provide unit cost per day." If a unit price is desired, it needs to be part of the Unit Price Specification. Unit Prices should not be in the technical specifications. Please remove and place in the Unit Price Specification.		This section states to "Provide allowance for 15 eight- hour days of adjustment grading work with a work crew of six using rakes and shovels to smooth and shape the planting area surfaces. Provide unit cost per day." If a unit price is desired, it needs to be part of the Unit Price Specification. Unit Prices should not be in the technical specifications. Please remove and place in the Unit Price Specification.				
P1-0496	Landscape Drawing Table of Contents	Closed	0P	09/30/2014	10/10/2014	10/20/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference L-000 ASI 127		Reference L-000 ASI 127				
L-0000 shows the table of contents with dates clouded and sheet names restored. In ASI 119 all sheets above ground level were deleted per the park deferral. Confirm whether the sheet deletions from ASI 119 are meant to remain deleted or if they are reinstated as is shown in ASI 127.		L-0000 shows the table of contents with dates clouded and sheet names restored. In ASI 119 all sheets above ground level were deleted per the park deferral. Confirm whether the sheet deletions from ASI 119 are meant to remain deleted or if they are reinstated as is shown in ASI 127.				
P1-0497	Updated Tree Planting Sheets	Closed	0P	09/30/2014	10/10/2014	09/30/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference L-0006 and L-0007 ASI 127		Reference L-0006 and L-0007 ASI 127				
The tree schedule has been updated in ASI 127, but new Tree Planting sheets have not been issued with these revisions. For example, LA36 has been added while AC 36 and DD 48 are missing from the Tree Planting Schedule. When will updated Tree Planting sheets be issued?		The tree schedule has been updated in ASI 127, but new Tree Planting sheets have not been issued with these revisions. For example, LA36 has been added while AC 36 and DD 48 are missing from the Tree Planting Schedule. When will updated Tree Planting sheets be issued?				



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<b>P1-0498</b>	<b>Benches on Bus Deck Level</b>	<b>Closed</b>	<b>0P</b>	<b>09/30/2014</b>	<b>10/10/2014</b>	<b>10/21/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference A1-2502, A1-2502A (ASI 127), A1-2503-2504 (ASI 119), A1-2505 (ASI 123), A1-2506 (ASI 119), A1-2507 (ASI 123)  There are what appear to be benches on the Bus Deck, see attached markups in blue on A1-2506 (SKA-3634), but there are no call outs identifying exactly what they are. Confirm if these are benches. If they are not benches clarify what they are meant to be. Provide details, dimensions and specifications.						<b>ANSWER:</b>  Reference A1-2502, A1-2502A (ASI 127), A1-2503-2504 (ASI 119), A1-2505 (ASI 123), A1-2506 (ASI 119), A1-2507 (ASI 123)  There are what appear to be benches on the Bus Deck, see attached markups in blue on A1-2506 (SKA-3634), but there are no call outs identifying exactly what they are. Confirm if these are benches. If they are not benches clarify what they are meant to be. Provide details, dimensions and specifications.
<b>P1-0499</b>	<b>8/A1-8532</b>	<b>Closed</b>	<b>0P</b>	<b>09/30/2014</b>	<b>10/10/2014</b>	<b>10/14/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference 6/A1-8532 (IFC Main Set)  6/A1-8532 references detail 8/A1-8532. Detail 8/A1-8532 does not exist. Please provide this detail.						<b>ANSWER:</b>  Reference 6/A1-8532 (IFC Main Set)  6/A1-8532 references detail 8/A1-8532. Detail 8/A1-8532 does not exist. Please provide this detail.
<b>P1-0500</b>	<b>Ceiling Type 2</b>	<b>Closed</b>	<b>0P</b>	<b>09/30/2014</b>	<b>10/10/2014</b>	<b>10/16/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference 9/A1-8511 and 9/A1-8530 (IFC Main Set) and 09 51 23 (ASI 124)  9/A1-8511 and 9/A1-8530 call for Ceiling Type 2. According to Specification Section 09 51 23 2.5.A, Ceiling Type 2 was deleted. Please delete all callouts for Ceiling Type 2 in the drawings.						<b>ANSWER:</b>  Reference 9/A1-8511 and 9/A1-8530 (IFC Main Set) and 09 51 23 (ASI 124)  9/A1-8511 and 9/A1-8530 call for Ceiling Type 2. According to Specification Section 09 51 23 2.5.A, Ceiling Type 2 was deleted. Please delete all callouts for Ceiling Type 2 in the drawings.
<b>P1-0501</b>	<b>Reveal Between W-14 and Cast Nodes</b>	<b>Closed</b>	<b>0P</b>	<b>09/30/2014</b>	<b>10/10/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference A1-8520 (ASI 124)						<b>ANSWER:</b>  Reference A1-8520 (ASI 124)



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P1-0502	Sheet A1-8504  From: Webcor Construction LP  Andrew Kitchen  <b>REQUEST:</b> Reference Detail 2/A1-8531 ASI 124  Detail 2/A1-8531 calls out "Secondary Structure See A1-8504." A1-8504 was not issued in the IFC Main Set or any subsequent ASIs. Please provide sheet A1-8504.	Closed	0P	09/30/2014	10/10/2014	10/07/2014
						<b>ANSWER:</b> Reference Detail 2/A1-8531 ASI 124  Detail 2/A1-8531 calls out "Secondary Structure See A1-8504." A1-8504 was not issued in the IFC Main Set or any subsequent ASIs. Please provide sheet A1-8504.





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<b>P1-0503</b>	<b>Rigid Insulation Above W-14</b>	<b>Closed</b>	<b>0P</b>	<b>09/30/2014</b>	<b>10/10/2014</b>	<b>10/13/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference ASI 124 Drawings		Reference ASI 124 Drawings				
The following sheets show a new W-14 Metal Ceiling location, without deleting the old rigid insulation location: 2/A1-8501, 3/A1-8510, 1/A1-8511, 1/A1-8530, 4/A1-8530, 5/A1-8530, 9/A1-8530, 4/A1-8531, 3/A1-8532, 5/A1-8532, 7/A1-8532. Additionally, the following sheets do not show rigid insulation redrawn with the new redline markups: 2/A1-8501, 5/A1-8510, 1/A1-8511, 1/A1-8521. It seems that there was no intent to place rigid insulation above horizontal or diagonal soffit locations. a) Please confirm there is no rigid insulation located above horizontal or diagonal soffits. b) Please revise these drawings so that the rigid insulation is properly located, and so that all old rigid insulation locations are deleted.		The following sheets show a new W-14 Metal Ceiling location, without deleting the old rigid insulation location: 2/A1-8501, 3/A1-8510, 1/A1-8511, 1/A1-8530, 4/A1-8530, 5/A1-8530, 9/A1-8530, 4/A1-8531, 3/A1-8532, 5/A1-8532, 7/A1-8532. Additionally, the following sheets do not show rigid insulation redrawn with the new redline markups: 2/A1-8501, 5/A1-8510, 1/A1-8511, 1/A1-8521. It seems that there was no intent to place rigid insulation above horizontal or diagonal soffit locations. a) Please confirm there is no rigid insulation located above horizontal or diagonal soffits. b) Please revise these drawings so that the rigid insulation is properly located, and so that all old rigid insulation locations are deleted.				
<b>P1-0504</b>	<b>W-14 Connection to Large Fan Sleeve</b>	<b>Closed</b>	<b>0P</b>	<b>09/30/2014</b>	<b>10/10/2014</b>	<b>10/13/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference A1-8531 ASI 124		Reference A1-8531 ASI 124				
Detail 3/A1-8531 (ASI 124) shows the W-14 Metal Ceiling angle abutted to the Large Fan Sleeve but does not provide a connection detail. In addition, a potential concern if a static connection is provided here is that vibration and/or movement caused by the fan may cause damage to ceiling, as well as the fan sleeve itself. Please provide detail for the connection between the W-14 Metal Ceiling and the Large Fan Sleeve.		Detail 3/A1-8531 (ASI 124) shows the W-14 Metal Ceiling angle abutted to the Large Fan Sleeve but does not provide a connection detail. In addition, a potential concern if a static connection is provided here is that vibration and/or movement caused by the fan may cause damage to ceiling, as well as the fan sleeve itself. Please provide detail for the connection between the W-14 Metal Ceiling and the Large Fan Sleeve.				
<b>P1-0505</b>	<b>Reveal Between W-14 and OCS Switch Shield</b>	<b>Closed</b>	<b>0P</b>	<b>09/30/2014</b>	<b>10/10/2014</b>	<b>10/14/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference A1-8552 (ASI 124)		Reference A1-8552 (ASI 124)				



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P1-0506	<b>Fiberglass OCS Switch Shield</b>  From: Webcor Construction LP      Andrew Kitchen  <b>REQUEST:</b> Reference A1-8552 (ASI 124)  Detail 2/A1-8552 shows a Fiberglass OCS Switch Shield. There is no specification for the Fiberglass OCS Switch Shield in the Contract Documents. Please provide specification for the Fiberglass OCS Switch Shield.	Closed	0P	09/30/2014	10/10/2014	10/13/2014
						<b>ANSWER:</b> Reference A1-8552 (ASI 124)  Detail 2/A1-8552 shows a Fiberglass OCS Switch Shield. There is no specification for the Fiberglass OCS Switch Shield in the Contract Documents. Please provide specification for the Fiberglass OCS Switch Shield.



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<b>P1-0507</b>	<b>Concrete Lid Details for Stair 501A and 502A</b>	<b>Closed</b>	<b>0P</b>	<b>10/01/2014</b>	<b>10/11/2014</b>	<b>10/13/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: (ASI 127 dated 9/12/14) Sheet A1-2305, Detail 8/A1-7010						REFERENCE: (ASI 127 dated 9/12/14) Sheet A1-2305, Detail 8/A1-7010
(ASI 123 dated 8/6/14) Detail 1/A1-7009						(ASI 123 dated 8/6/14) Detail 1/A1-7009
(ASI 119 dated 6/18/14) Detail A/A1-7110						(ASI 119 dated 6/18/14) Detail A/A1-7110
(ASI 118 dated 6/20/14) Detail A/A1-3021						(ASI 118 dated 6/20/14) Detail A/A1-3021
(IFC Drawings for Main Package dated 3/31/14) Detail A/A1-7803, Detail E/A1-7803, Detail 2/A1-7830						(IFC Drawings for Main Package dated 3/31/14) Detail A/A1-7803, Detail E/A1-7803, Detail 2/A1-7830
Sheet A1-2305 of ASI 127 shows Stair 501A and 502A at the Ground Level.						Sheet A1-2305 of ASI 127 shows Stair 501A and 502A at the Ground Level.
- Stair 501A references Detail 8/A1-7010 and Detail A/A1-7110 for details. Detail A/A1-7110 depicts a concrete lid at Stair 501A.						- Stair 501A references Detail 8/A1-7010 and Detail A/A1-7110 for details. Detail A/A1-7110 depicts a concrete lid at Stair 501A.
- Stair 502A references Detail 1/A1-7009 and Detail A/A1-3021 for details. Detail A/A1-3021 depicts a concrete lid at Stair 502A.						- Stair 502A references Detail 1/A1-7009 and Detail A/A1-3021 for details. Detail A/A1-3021 depicts a concrete lid at Stair 502A.
- Details A & E/A1-7803 reference Detail 2/A1-7830. Detail 2/A1-7830 also depicts a concrete lid.						- Details A & E/A1-7803 reference Detail 2/A1-7830. Detail 2/A1-7830 also depicts a concrete lid.
Structural does not call for concrete lids at Stair 501A and 502A. Please identify and provide details for the structural slab required at Stair 501A and Stair 502A.						Structural does not call for concrete lids at Stair 501A and 502A. Please identify and provide details for the structural slab required at Stair 501A and Stair 502A.



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P1-0508	CET-1 Size	Closed	0P	10/01/2014	10/11/2014	10/20/2014
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Reference: P1-0434, P1-0435		Reference: P1-0434, P1-0435				
The sketch attached to RFI P1-0435 (A1-9606) shows the size of CET-1 as 6' x 6'. The attached Specification 09 30 00 for RFI P1-0434 shows the size of CET-1 as 6" x 6". Please revise so that the drawing and the specification match for CET-1.		The sketch attached to RFI P1-0435 (A1-9606) shows the size of CET-1 as 6' x 6'. The attached Specification 09 30 00 for RFI P1-0434 shows the size of CET-1 as 6" x 6". Please revise so that the drawing and the specification match for CET-1.				
P1-0509	Concrete Mix for Bus Crash Rail	Closed	0P	10/03/2014	10/13/2014	10/09/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 03 30 02		REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Specification Section 03 30 02				
Specification Section 03 30 02 Cast-In-Place Concrete does not provide information on the concrete mix required for the Bus Crash Rails.		Specification Section 03 30 02 Cast-In-Place Concrete does not provide information on the concrete mix required for the Bus Crash Rails.				
Please confirm the concrete mix to be used is "All other concrete" or provide the specifications for the concrete mix to be used at the Bus Crash Rails.		Please confirm the concrete mix to be used is "All other concrete" or provide the specifications for the concrete mix to be used at the Bus Crash Rails.				
P1-0510	Clarification on Special Hardware Sets	Closed	SSI	10/08/2014	10/18/2014	10/22/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
This question contains Sensitive Security Information and is available only to individuals who have been granted access to the document that is the basis for the question.		The response to this question is available only to bidders who have been granted access to the TJPA's secure website.				
P1-0511	Worker Employment Requirements for Division 10 Signage	Closed	0P	10/08/2014	10/18/2014	10/31/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 16		REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 16				



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	Specification Section 10 14 19 Specification Section 10 14 26 Specification Section 10 14 33 Specification Section 10 14 36 Specification Section 10 14 63 Specification Section 10 14 66 Specification Section 10 14 73 Specification Section 10 14 83 Specification Section 10 18 00  The following Specification Sections require bidders to "employ workers that will be employed during the construction at the Project."  - Specification Section 10 14 16 1.3 E 3 Cast and Etched Graphics - Specification Section 10 14 19 1.3 E 3 Dimensional Lettering - Specification Section 10 14 26 G 3 Pylon Signage - Specification Section 10 14 33 1.3 F 3 Illuminated Panel Signage - Specification Section 10 14 36 1.3 E 3 Non-Illuminated Panel Signage - Specification Section 10 14 63 1.4 J 3 Electronic Panel Signage - Specification Section 10 14 66 1.4 G 3 Floating Signage - Specification Section 10 14 73 1.3 F 3 Glass Signage - Specification Section 10 14 83 1.4 E 3 Interpretive Graphics - Specification Section 10 18 00 1.4 H 3 Informational Kiosks  Subcontractors will not be able to guaranty employment of specific workers producing material. Please confirm it is acceptable that personnel may change throughout the project.					
	Specification Section 10 14 19 Specification Section 10 14 26 Specification Section 10 14 33 Specification Section 10 14 36 Specification Section 10 14 63 Specification Section 10 14 66 Specification Section 10 14 73 Specification Section 10 14 83 Specification Section 10 18 00  The following Specification Sections require bidders to "employ workers that will be employed during the construction at the Project."  - Specification Section 10 14 16 1.3 E 3 Cast and Etched Graphics - Specification Section 10 14 19 1.3 E 3 Dimensional Lettering - Specification Section 10 14 26 G 3 Pylon Signage - Specification Section 10 14 33 1.3 F 3 Illuminated Panel Signage - Specification Section 10 14 36 1.3 E 3 Non- Illuminated Panel Signage - Specification Section 10 14 63 1.4 J 3 Electronic Panel Signage - Specification Section 10 14 66 1.4 G 3 Floating Signage - Specification Section 10 14 73 1.3 F 3 Glass Signage - Specification Section 10 14 83 1.4 E 3 Interpretive Graphics - Specification Section 10 18 00 1.4 H 3 Informational Kiosks  Subcontractors will not be able to guaranty employment of specific workers producing material. Please confirm it is acceptable that personnel may change throughout the project.					
P1-0512	Warranty Period Requirement per Specification Section 10 14 16 Cast and Etched + Closed		0P	10/08/2014	10/18/2014	10/20/2014
From: Webcor Construction LP		Tram Nguyen				
REQUEST:		ANSWER:				
REFERENCE: (ASI 127 dated 9/12/14)		REFERENCE: (ASI 127 dated 9/12/14)				



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	Specification Section 10 14 16  Specification Section 10 14 16 1.6 A 2 Cast and Etched Graphics indicates the warranty period is subject to final approval by TJPA representative.  This requirement may impact pricing of the bid.  Please provide a specific time period for the warranty.					
	Specification Section 10 14 16  Specification Section 10 14 16 1.6 A 2 Cast and Etched Graphics indicates the warranty period is subject to final approval by TJPA representative.  This requirement may impact pricing of the bid.  Please provide a specific time period for the warranty.					
<b>P1-0513</b>	<b>Incorrect Specification Reference for Stone Boulders with Inset Metal Letterforms</b>	<b>Closed</b>				
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 16 Specification Section 32 14 41  Specification Section 10 14 16 2.1 A Cast and Etched Graphics states,  "Stone boulders with inset metal letterforms. Refer to Section 32 14 41.2.1 for 'stone' selection and specifications."  The referenced specification is the LEED Requirements for Mortar-Set Stone Paving.  Please revise the referenced specification to the correct specification section.						
<b>ANSWER:</b>  REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 16 Specification Section 32 14 41  Specification Section 10 14 16 2.1 A Cast and Etched Graphics states,  "Stone boulders with inset metal letterforms. Refer to Section 32 14 41.2.1 for 'stone' selection and specifications."  The referenced specification is the LEED Requirements for Mortar-Set Stone Paving.  Please revise the referenced specification to the correct specification section.						
			<b>0P</b>	<b>10/08/2014</b>	<b>10/18/2014</b>	<b>10/15/2014</b>



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<b>P1-0514</b>	<b>LEED Credits MR 5.1 &amp; MR 5.2 Requirement per Specification Section 10 14 16</b>	<b>Closed</b>	<b>0P</b>	<b>10/08/2014</b>	<b>10/18/2014</b>	<b>10/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 16			REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 16			
Specification Section 10 14 16 1.2 B 2 Cast and Etched Graphics indicates that information to receive LEED Credits MR 5.1 and MR 5.2 (regional materials) is required. No minimum requirement for regional materials is given.			Specification Section 10 14 16 1.2 B 2 Cast and Etched Graphics indicates that information to receive LEED Credits MR 5.1 and MR 5.2 (regional materials) is required. No minimum requirement for regional materials is given.			
In addition, Specification Section 10 14 16 2.3 Cast and Etched Graphics lists Metal Arts (Mandan, ND), Matthews International (Pittsburgh, PA) and Gemini Signs (Marlborough, MA). The listed manufacturers will not qualify for LEED Credits MR 5.1 and MR. 5.2 .			In addition, Specification Section 10 14 16 2.3 Cast and Etched Graphics lists Metal Arts (Mandan, ND), Matthews International (Pittsburgh, PA) and Gemini Signs (Marlborough, MA). The listed manufacturers will not qualify for LEED Credits MR 5.1 and MR. 5.2 .			
Please confirm that it is acceptable for none of the cast and etched graphics specified in Specification Section 10 14 16 are required to be extracted or manufactured within the limits called out in LEED Credits MR 5.1 and/or MR 5.2.			Please confirm that it is acceptable for none of the cast and etched graphics specified in Specification Section 10 14 16 are required to be extracted or manufactured within the limits called out in LEED Credits MR 5.1 and/or MR 5.2.			



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<b>P1-0515</b>	<b>Use of Lead Expansion-Bolt Devices for Drilled in Place Anchors</b>	<b>Closed</b>	<b>0P</b>	<b>10/08/2014</b>	<b>10/08/2014</b>	<b>10/15/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 16 Specification Section 10 14 19 Specification Section 10 14 66  Specification Section 10 14 16 2.6 A Cast and Etched Graphics, Specification Section 10 14 19 2.3 A Dimensional Lettering, and Specification Section 10 14 66 2.3 A Floating Signage, call out for the use of "lead expansion-bolt devices".  The use of lead expansion-bolt devices could cause lead residue within the building, and dramatically increase the health and safety costs associated with construction as required by Specification Section 00 08 14 Health and Safety Criteria.  Please revise Specification Section 10 14 16, Specification Section 10 14 19, and Specification Section 10 14 66 to remove the use of lead expansion-bolt devices.			REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 16 Specification Section 10 14 19 Specification Section 10 14 66  Specification Section 10 14 16 2.6 A Cast and Etched Graphics, Specification Section 10 14 19 2.3 A Dimensional Lettering, and Specification Section 10 14 66 2.3 A Floating Signage, call out for the use of "lead expansion-bolt devices".  The use of lead expansion-bolt devices could cause lead residue within the building, and dramatically increase the health and safety costs associated with construction as required by Specification Section 00 08 14 Health and Safety Criteria.  Please revise Specification Section 10 14 16, Specification Section 10 14 19, and Specification Section 10 14 66 to remove the use of lead expansion-bolt devices.			







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<b>P1-0517</b>	<b>Clarification on Requirements for Electrical Power to Signage</b>	<b>Closed</b>	<b>0P</b>	<b>10/08/2014</b>	<b>10/18/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 19		REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 19				
Specification Section 10 14 19 3.1 B Dimensional Lettering indicates that electrical power is to be sized to accommodate the building.		Specification Section 10 14 19 3.1 B Dimensional Lettering indicates that electrical power is to be sized to accommodate the building.				
Per the response to Constructability Comment 7907, the specification requirement was to be changed to mandate signage be designed to accommodate the existing power allotments.		Per the response to Constructability Comment 7907, the specification requirement was to be changed to mandate signage be designed to accommodate the existing power allotments.				
Please revise specification as indicated in Constructability Comment 7907.		Please revise specification as indicated in Constructability Comment 7907.				
<b>P1-0518</b>	<b>Sign Type RS2 Exterior Vertical Flag Tenant ID per Specification</b>	<b>Closed</b>	<b>0P</b>	<b>10/08/2014</b>	<b>10/18/2014</b>	<b>10/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 33		REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 33				
Specification Section 10 14 33 2.2 D 1 Illuminated Panel Signage identifies Sign Type RS2 Exterior Vertical Flag Tenant ID.		Specification Section 10 14 33 2.2 D 1 Illuminated Panel Signage identifies Sign Type RS2 Exterior Vertical Flag Tenant ID.				
Sheet SG-0010, Sheet SG-001, and the Sign Message Schedule do not identify Sign Type RS2. In addition, the plan sheets do not identify RS2 signs.		Sheet SG-0010, Sheet SG-001, and the Sign Message Schedule do not identify Sign Type RS2. In addition, the plan sheets do not identify RS2 signs.				
Please confirm there are no RS2 signs required for Phase 1.		Please confirm there are no RS2 signs required for Phase 1.				
<b>P1-0519</b>	<b>Messages for Signage Type RS4 Exterior Square Flag Tenant ID</b>	<b>Closed</b>	<b>0P</b>	<b>10/08/2014</b>	<b>10/18/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 33		REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 33				
Specification Section 10 14 33 2.2 D 2 Illuminated Panel		Specification Section 10 14 33 2.2 D 2 Illuminated				



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	<p>Signage provide specifications for Sign Type RS4 Exterior Square Flag Tenant ID.</p> <p>Per the Sign Message Schedule, all of the messages for Sign Type RS4 are shown as (TENANT LOGO TBD).</p> <p>Please provide the messages for Signage Type RS4, or confirm Sign Type RS4 is not to be provided as part of Phase 1 work.</p>					<p>Panel Signage provide specifications for Sign Type RS4 Exterior Square Flag Tenant ID.</p> <p>Per the Sign Message Schedule, all of the messages for Sign Type RS4 are shown as (TENANT LOGO TBD).</p> <p>Please provide the messages for Signage Type RS4, or confirm Sign Type RS4 is not to be provided as part of Phase 1 work.</p>
<b>P1-0520</b>	<b>Review and Finalize Construction Schedule Subcontractor Requirement</b>	<b>Closed</b>	<b>0P</b>	<b>10/08/2014</b>	<b>10/18/2014</b>	<b>10/17/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 16 Specification Section 10 14 19 Specification Section 10 14 26 Specification Section 10 14 33 Specification Section 10 14 36 Specification Section 10 14 63 Specification Section 10 14 66 Specification Section 10 14 73 Specification Section 10 14 83 Specification Section 10 18 00						REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 16 Specification Section 10 14 19 Specification Section 10 14 26 Specification Section 10 14 33 Specification Section 10 14 36 Specification Section 10 14 63 Specification Section 10 14 66 Specification Section 10 14 73 Specification Section 10 14 83 Specification Section 10 18 00
The following Specification Sections requires the sign subcontractor to review and finalize the construction schedule:						The following Specification Sections requires the sign subcontractor to review and finalize the construction schedule:
<ul style="list-style-type: none"><li>- Specification Section 10 14 16 1.3 F 1 Cast and Etched Graphics</li><li>- Specification Section 10 14 19 1.3 F1 Dimensional Lettering</li><li>- Specification Section 10 14 26 H 1 Pylon Signage</li><li>- Specification Section 10 14 33 1.3 G 1 Illuminated Panel Signage</li><li>- Specification Section 10 14 36 1.3 F 1 Non-Illuminated Panel Signage</li><li>- Specification Section 10 14 63 1.4 K 1 Electronic Panel Signage</li><li>- Specification Section 10 14 66 1.4 H 1 Floating Signage</li><li>- Specification Section 10 14 73 1.3 G 1 Glass Signage</li></ul>						<ul style="list-style-type: none"><li>- Specification Section 10 14 16 1.3 F 1 Cast and Etched Graphics</li><li>- Specification Section 10 14 19 1.3 F1 Dimensional Lettering</li><li>- Specification Section 10 14 26 H 1 Pylon Signage</li><li>- Specification Section 10 14 33 1.3 G 1 Illuminated Panel Signage</li><li>- Specification Section 10 14 36 1.3 F 1 Non-Illuminated Panel Signage</li><li>- Specification Section 10 14 63 1.4 K 1 Electronic Panel Signage</li><li>- Specification Section 10 14 66 1.4 H 1 Floating Signage</li></ul>



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	<p>- Specification Section 10 18 00 1.4 I 1 Informational Kiosks</p> <p>Subcontractors are not responsible to establish the construction schedule. Please remove the requirement for the signage contractor to review and finalize the construction schedule.</p>					
	<p>- Specification Section 10 14 73 1.3 G 1 Glass Signage</p> <p>- Specification Section 10 18 00 1.4 I 1 Informational Kiosks</p> <p>Subcontractors are not responsible to establish the construction schedule. Please remove the requirement for the signage contractor to review and finalize the construction schedule.</p>					
<b>P1-0521</b>	<b>Torsion Hinges and Locking Fasteners for Aluminum Ceilings</b>	<b>Closed</b>	<b>0P</b>	<b>10/09/2014</b>	<b>10/19/2014</b>	<b>10/21/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						
Reference Specification Section 09 51 23						
09 51 23 2.9.A.10 states ¿Torsion Hinges and Locking Fasteners: Provide the following on each aluminum ceiling punched panel.¿ The redline mark-up version of 09 51 23 from ASI 124 shows ¿Torsion Hinges and Locking Fasteners: Provide the following on each aluminum ceiling punched panel¿ as deleted. Please confirm that the sentence from 2.9.A.10 is to be removed from ASI 127.						
<b>ANSWER:</b>						
Reference Specification Section 09 51 23						
09 51 23 2.9.A.10 states ¿Torsion Hinges and Locking Fasteners: Provide the following on each aluminum ceiling punched panel.¿ The redline mark-up version of 09 51 23 from ASI 124 shows ¿Torsion Hinges and Locking Fasteners: Provide the following on each aluminum ceiling punched panel¿ as deleted. Please confirm that the sentence from 2.9.A.10 is to be removed from ASI 127.						



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<b>P1-0522</b>	<b>Perforations in Aluminum Panels</b>	<b>Closed</b>	<b>0P</b>	<b>10/09/2014</b>	<b>10/19/2014</b>	<b>10/21/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference 09 51 23 ASI 127  09 51 23 2.5.B.10.A, among other sections in this specification, discusses providing perforation holes in the aluminum ceiling panels. The perforation holes are visible on some of the drawings for the aluminum ceiling, but there are no callouts, and there is no specified hole pattern shown. Please confirm there are to be perforation holes in the aluminum panels, and if so, please provide pattern and any other specifications needed.		<b>ANSWER:</b>  Reference 09 51 23 ASI 127  09 51 23 2.5.B.10.A, among other sections in this specification, discusses providing perforation holes in the aluminum ceiling panels. The perforation holes are visible on some of the drawings for the aluminum ceiling, but there are no callouts, and there is no specified hole pattern shown. Please confirm there are to be perforation holes in the aluminum panels, and if so, please provide pattern and any other specifications needed.				
<b>P1-0523</b>	<b>Sign Message Schedule Reference Revision</b>	<b>Closed</b>	<b>0P</b>	<b>10/10/2014</b>	<b>10/20/2014</b>	<b>10/21/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Specification Section 00 03 50 1.1 A 4 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 00 03 50 1.1 A 4 Available Project Information references the Sign Message Schedule, dated March 31, 2014.  A revised Sign Message Schedule dated September 12, 2014 was issued with ASI 127.  Please revise Specification Section 00 03 50 to reflect the new Sign Message Schedule.		<b>ANSWER:</b>  REFERENCE: Specification Section 00 03 50 1.1 A 4 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 00 03 50 1.1 A 4 Available Project Information references the Sign Message Schedule, dated March 31, 2014.  A revised Sign Message Schedule dated September 12, 2014 was issued with ASI 127.  Please revise Specification Section 00 03 50 to reflect the new Sign Message Schedule.				
<b>P1-0524</b>	<b>Incorrect Sign Message Schedule Location Associations</b>	<b>Closed</b>	<b>0P</b>	<b>10/10/2014</b>	<b>10/20/2014</b>	<b>10/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Sheet SG1-2102 (ASI 124 dated 8/18/2014)  Per the Sign Message Schedule dated 9/12/14, Signs EP1 T-02-012, E4 T-02-013, and ST1 T-02-014 are associated		<b>ANSWER:</b>  REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Sheet SG1-2102 (ASI 124 dated 8/18/2014)  Per the Sign Message Schedule dated 9/12/14, Signs EP1 T-02-012, E4 T-02-013, and ST1 T-02-014 are				



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<b>P1-0527</b>	<b>Egress Information for Sign Types EP1 and EP2</b>	<b>Closed</b>	<b>0P</b>	<b>10/10/2014</b>	<b>10/20/2014</b>	<b>10/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Detail 2/SG1-6201 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Detail 2/SG1-6201 (IFC Drawings for Main Package dated 3/31/14)
Key Note 1 of Detail 2/SG1-6201 indicates that the signage contractor is to develop exiting signage for Sign Type EP1 Evacuation Plan Stair Mount and EP2 Evacuation Plan Elevator Mount.						Key Note 1 of Detail 2/SG1-6201 indicates that the signage contractor is to develop exiting signage for Sign Type EP1 Evacuation Plan Stair Mount and EP2 Evacuation Plan Elevator Mount.
Please confirm the egress information as shown on Sheet A-1360 through Sheet A-1365 is to be used to develop the exiting signage.						Please confirm the egress information as shown on Sheet A-1360 through Sheet A-1365 is to be used to develop the exiting signage.
<b>P1-0528</b>	<b>Locations for Pylon Signage</b>	<b>Closed</b>	<b>0P</b>	<b>10/10/2014</b>	<b>10/20/2014</b>	<b>10/23/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 2/SG1-6000 Detail 4/SG1-6001 Detail 2/SG1-6002 Detail 3/SG1-6003						REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 2/SG1-6000 Detail 4/SG1-6001 Detail 2/SG1-6002 Detail 3/SG1-6003
The following details show General Note A indicating the exact location of pylons is as shown on Landscape Drawings:						The following details show General Note A indicating the exact location of pylons is as shown on Landscape Drawings:
Detail 2/SG1-6000 Detail 4/SG1-6001 Detail 2/SG1-6002 Detail 3/SG1-6003						Detail 2/SG1-6000 Detail 4/SG1-6001 Detail 2/SG1-6002 Detail 3/SG1-6003
Landscape Drawings do not dimension the location of the pylons. Please provide the exact locations of the pylon signage.						Landscape Drawings do not dimension the location of the pylons. Please provide the exact locations of the pylon signage.
<b>P1-0529</b>	<b>"End of Section" Correction to Specification Section 10 14 26</b>	<b>Closed</b>	<b>0P</b>	<b>10/10/2014</b>	<b>10/20/2014</b>	<b>10/15/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>



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<p>REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14)</p> <p>Specification Section 10 14 26 Pylon Signage Page 14 calls out "End of Section 10 14 19". Please revise the "End of Section" specification reference number to 10 14 26.</p>			<p>REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14)</p> <p>Specification Section 10 14 26 Pylon Signage Page 14 calls out "End of Section 10 14 19". Please revise the "End of Section" specification reference number to 10 14 26.</p>			
P1-0530	Pylon Signage Electrical Power Requirements per Specifications and Constructability Comments	Closed	0P	10/10/2014	10/20/2014	10/21/2014
<p>From: Webcor Construction LP      Tram Nguyen</p> <p>REQUEST:</p> <p>REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14) Details 6 &amp; 7/SG1-6001 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 10 14 26 3.1 Pylon Signage directs that the signage subcontractor is to verify that electrical power is sized and located to accommodate signs. In addition, Details 6 &amp; 7/SG1-6001 indicate that the signage contractor is to provide power requirements to the TJPA representative.</p> <p>Per Constructability Comments 7909, 7910, and 7911, the signage specification are to be revised to require the signage contractor to design the signs within the electrical circuiting power allotments in the current design, and include additional electrical circuits in their signage bid if additional power is determined to be required.</p> <p>Please revise Specification 10 14 26 as indicated in the response to Constructability Comments 7909, 7910, and 7911.</p>			<p>ANSWER:</p> <p>REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14) Details 6 &amp; 7/SG1-6001 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 10 14 26 3.1 Pylon Signage directs that the signage subcontractor is to verify that electrical power is sized and located to accommodate signs. In addition, Details 6 &amp; 7/SG1-6001 indicate that the signage contractor is to provide power requirements to the TJPA representative.</p> <p>Per Constructability Comments 7909, 7910, and 7911, the signage specification are to be revised to require the signage contractor to design the signs within the electrical circuiting power allotments in the current design, and include additional electrical circuits in their signage bid if additional power is determined to be required.</p> <p>Please revise Specification 10 14 26 as indicated in the response to Constructability Comments 7909, 7910, and 7911.</p>			
P1-0531	Warranty Period Requirement per Specification Section 10 14 26 Pylon Signage	Closed	0P	10/10/2014	10/20/2014	10/20/2014
<p>From: Webcor Construction LP      Tram Nguyen</p> <p>REQUEST:</p>			<p>ANSWER:</p>			





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	REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14)  Specification Section 10 14 26 1.7 A 2 Pylon Signage indicates that the warranty period is subject to final approval by TJPA. Please provide warranty period.					REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14)  Specification Section 10 14 26 1.7 A 2 Pylon Signage indicates that the warranty period is subject to final approval by TJPA. Please provide warranty period.
P1-0532	Revision of Specification Section 10 14 26 2.1 B 4 r to Match Provided Information	Closed	0P	10/10/2014	10/20/2014	10/28/2014
	From: Webcor Construction LP  Tram Nguyen					
	REQUEST:  REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14)  Specification Section 10 14 26 2.1 B 4 r Pylon Signage indicates that the integrated speaker is "TBD Refer to telecommunication sheets for specifications."  Specification Section 10 14 26 2.1 B 4 r 1 - 6 go on to specify a product.  Please revise 10 14 26 2.1 B 4 r to agree with the provided information.					ANSWER:  REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14)  Specification Section 10 14 26 2.1 B 4 r Pylon Signage indicates that the integrated speaker is "TBD Refer to telecommunication sheets for specifications."  Specification Section 10 14 26 2.1 B 4 r 1 - 6 go on to specify a product.  Please revise 10 14 26 2.1 B 4 r to agree with the provided information.



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P1-0533	GFIC Outlet Requirement for Sign Types PD1 and PD2	Closed	0P	10/10/2014	10/20/2014	11/10/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Detail 3/SG1-6003 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Detail 3/SG1-6003 (IFC Drawings for Main Package dated 3/31/14)				
Per Key Note 9 on Detail 3/SG1-6003, Sign Type PD1 Bus Deck Dock Identification and Sign Type PD2 Muni Plaza Identification are to be provided with a GFIC duplex outlet.		Per Key Note 9 on Detail 3/SG1-6003, Sign Type PD1 Bus Deck Dock Identification and Sign Type PD2 Muni Plaza Identification are to be provided with a GFIC duplex outlet.				
Sheet E1-2306 and Sheet E1-2502 through Sheet E1-2507 do not show outlets or circuiting for outlets at these locations.		Sheet E1-2306 and Sheet E1-2502 through Sheet E1-2507 do not show outlets or circuiting for outlets at these locations.				
Please confirm no outlets are required at these locations, or provide circuiting information.		Please confirm no outlets are required at these locations, or provide circuiting information.				
P1-0534	Verbiage for Sign Type R1 Roof Access Marker	Closed	0P	10/10/2014	10/20/2014	10/21/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Detail 2/SG1-6060 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Detail 2/SG1-6060 (IFC Drawings for Main Package dated 3/31/14)				
Key Note 2 on Detail 2/SG1-6060 indicates that the final name for park to be provided by TJPA representative to establish the verbiage on Sign Type R1 Roof Access Marker. This is not biddable with the current information.		Key Note 2 on Detail 2/SG1-6060 indicates that the final name for park to be provided by TJPA representative to establish the verbiage on Sign Type R1 Roof Access Marker. This is not biddable with the current information.				
Please provide the verbiage for Sign R1, information on amount of letters to be priced, or allowance amount to be used.		Please provide the verbiage for Sign R1, information on amount of letters to be priced, or allowance amount to be used.				
P1-0535	Incomplete Key Note 2 on Detail 3/SG1-6050	Closed	0P	10/10/2014	10/20/2014	10/20/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Detail 3/SG1-6050 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Detail 3/SG1-6050 (IFC Drawings for Main Package dated 3/31/14)				



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P1-0536	Switchable Lighting Circuit Requirement for Pylon Signage per Specification Secti Closed		0P	10/10/2014	10/20/2014	11/03/2014
From: Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14)				
The following Specification Sections for Pylon Signage call out for pylon sign lighting to be on a switchable lighting circuit:		The following Specification Sections for Pylon Signage call out for pylon sign lighting to be on a switchable lighting circuit:				
10 14 26 2.1 B 1 q 10 14 26 2.1 B 2 q 10 14 26 2.1 B 3 q 10 14 26 2.1 B 4 s 10 14 26 2.1 B 5 s 10 14 26 2.1 C 1 y 4 10 14 26 2.1 C 2 z 4		10 14 26 2.1 B 1 q 10 14 26 2.1 B 2 q 10 14 26 2.1 B 3 q 10 14 26 2.1 B 4 s 10 14 26 2.1 B 5 s 10 14 26 2.1 C 1 y 4 10 14 26 2.1 C 2 z 4				
The electrical plans do not show the pylons as being on a switchable lighting circuit. Please revise the Electrical Drawings/Pylon Signage Specifications to agree with each other.		The electrical plans do not show the pylons as being on a switchable lighting circuit. Please revise the Electrical Drawings/Pylon Signage Specifications to agree with each other.				



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<b>P1-0537</b>	<b>Usage of Sign Type E3 Exit in Phase 1</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/27/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 14 36 (ASI 127 dated 9/12/14) Sheet SG1-6200 (IFC Drawings for Main Package dated 3/31/14) Signage Message Schedule (ASI 127 dated 9/12/14) Signage Plan Sheets  Specification Section 10 14 36 2.2 J 2 Non-Illuminated Panel Signage identifies Sign Type E3 Exit and Sheet SG1-6200 identifies signage information for Sign Type E3 Exit.  The Sign Message Schedule and Signage Plan Sheets do not identify a Sign Type E3 Exit.  If Sign Type E3 is not to be used, please revise Specification Section 10 14 36. If Sign type E3 is to be used please revise plans and reports to identify their locations.		REFERENCE: Specification Section 10 14 36 (ASI 127 dated 9/12/14) Sheet SG1-6200 (IFC Drawings for Main Package dated 3/31/14) Signage Message Schedule (ASI 127 dated 9/12/14) Signage Plan Sheets  Specification Section 10 14 36 2.2 J 2 Non-Illuminated Panel Signage identifies Sign Type E3 Exit and Sheet SG1-6200 identifies signage information for Sign Type E3 Exit.  The Sign Message Schedule and Signage Plan Sheets do not identify a Sign Type E3 Exit.  If Sign Type E3 is not to be used, please revise Specification Section 10 14 36. If Sign type E3 is to be used please revise plans and reports to identify their locations.				
<b>P1-0538</b>	<b>Designation for Sign Type EP1 per Specification Section 10 14 26</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 14 36 (ASI 127 dated 9/12/14) Sheet SG1-6201 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 14 36 2.2 J 7 Non-Illuminated Panel Signage identifies Sign Type EP1 as both Evacuation Plan at Stair and Evacuation Plan at Elevator.  Sheet SG1-6201 calls out Sign Type EP1 Evacuation Plan Stair Mount and Stair Type EP2 Evacuation Plan Elevator Mount.  Please revise Specification Section 10 14 36 and drawings to match.		REFERENCE: Specification Section 10 14 36 (ASI 127 dated 9/12/14) Sheet SG1-6201 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 14 36 2.2 J 7 Non-Illuminated Panel Signage identifies Sign Type EP1 as both Evacuation Plan at Stair and Evacuation Plan at Elevator.  Sheet SG1-6201 calls out Sign Type EP1 Evacuation Plan Stair Mount and Stair Type EP2 Evacuation Plan Elevator Mount.  Please revise Specification Section 10 14 36 and drawings to match.				



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<b>P1-0539</b>	<b>Temperature Range for Thermal Movement Design Requirements Clarification</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/17/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Specification Section 10 14 63 (ASI 127 dated 9/12/14)						REFERENCE: Specification Section 10 14 63 (ASI 127 dated 9/12/14)
Specification Section 10 14 63 1.2 A 3 a Electronic Panel Signage indicates that the temperature range for thermal movement design requirements are 120 deg. F (67 deg. C), ambient: 180 deg. F (100 deg. C), material surfaces.						Specification Section 10 14 63 1.2 A 3 a Electronic Panel Signage indicates that the temperature range for thermal movement design requirements are 120 deg. F (67 deg. C), ambient: 180 deg. F (100 deg. C), material surfaces.
67 deg. C = 152.6 deg. F, 100 deg. C = 212 deg. F.						67 deg. C = 152.6 deg. F, 100 deg. C = 212 deg. F.
Please provide the correct temperature range to be used for thermal movement design requirements.						Please provide the correct temperature range to be used for thermal movement design requirements.
<b>P1-0540</b>	<b>Warranty Period Requirement for Electronic Panel Signage</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/14/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Specification Section 10 14 63 (ASI 127 dated 9/12/14)						REFERENCE: Specification Section 10 14 63 (ASI 127 dated 9/12/14)
Specification Section 10 14 63 1.7 A 2 Electronic Panel Signage indicates that the warranty period is subject to final approval by TJPA representative.						Specification Section 10 14 63 1.7 A 2 Electronic Panel Signage indicates that the warranty period is subject to final approval by TJPA representative.
Please provide the desired warranty period.						Please provide the desired warranty period.
<b>P1-0541</b>	<b>Door Hardware for Prefabricated Buildings</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/13/2014</b>	<b>10/21/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference 13 34 24 2.3.L ASI 127						Reference 13 34 24 2.3.L ASI 127
Specification states that Door hardware shall be supplied by the hardware contractor. The Prefabricated Buildings Subcontractor's scope includes the furnish and install of the door hardware. Please revise this specification so that the Prefabricated Buildings Trade Subcontractor supplies the door hardware.						Specification states that Door hardware shall be supplied by the hardware contractor. The Prefabricated Buildings Subcontractor's scope includes the furnish and install of the door hardware. Please revise this specification so that the Prefabricated Buildings Trade Subcontractor supplies



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P1-0542	Electrical Power Requirement for Sign Types SM1, SX1, and SS1	Closed	0P	10/13/2014	10/13/2014	10/17/2014
<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE:  Specification Section 10 14 63 (ASI 127 dated 9/12/14)  Detail 2/SG1-6011 (ASI 124 dated 8/8/14)  Detail 2/SG1-6020 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 10 14 63 3.1 B Electronic Panel  Signage directs that electrical power is sized to accommodate Sign Type SM1 Wall Mounted Schedule Board, Sign Type SX1 Grand Hall Schedule Board, and Sign Type SS1 Textural Display.</p> <p>In addition General Sheet Note F on Detail 2/SG1-6011 and General Sheet Note I on Detail 2/SG1-6020 direct the sign subcontractor is to provide power requirements to TJPA Representative for these signs as well.</p> <p>The response to Constructability Comment 7921 indicates the signage specifications will be revised to require the design-build signage contractor to design the signs within the electrical circuiting power allotments in the current design, and to include additional electrical circuits in their signage bid if additional power is determined to be required.</p> <p>Please revise the specifications and general notes to agree with the response to Constructability Comment 7921.</p>						
<p>the door hardware.</p> <p><b>ANSWER:</b></p> <p>REFERENCE:  Specification Section 10 14 63 (ASI 127 dated 9/12/14)  Detail 2/SG1-6011 (ASI 124 dated 8/8/14)  Detail 2/SG1-6020 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 10 14 63 3.1 B Electronic Panel  Signage directs that electrical power is sized to accommodate Sign Type SM1 Wall Mounted Schedule Board, Sign Type SX1 Grand Hall Schedule Board, and Sign Type SS1 Textural Display.</p> <p>In addition General Sheet Note F on Detail 2/SG1-6011 and General Sheet Note I on Detail 2/SG1-6020 direct the sign subcontractor is to provide power requirements to TJPA Representative for these signs as well.</p> <p>The response to Constructability Comment 7921 indicates the signage specifications will be revised to require the design-build signage contractor to design the signs within the electrical circuiting power allotments in the current design, and to include additional electrical circuits in their signage bid if additional power is determined to be required.</p> <p>Please revise the specifications and general notes to agree with the response to Constructability Comment 7921.</p>						



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<b>P1-0543</b>	<b>Paint Colors for Prefabricated Buildings</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/13/2014</b>	<b>10/27/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference 13 34 24 1.5.B & 2.2.H ASI 124  Specification 1.5.B states "Color swatches demonstrating available colors and patterns for specified finishes shall be supplied for final approval." Additionally Specification 2.2.H states that paint color shall be selected by TJPA Representative. Please define the colors and patterns before bid so that this package can be accurately priced.		<b>ANSWER:</b>  Reference 13 34 24 1.5.B & 2.2.H ASI 124  Specification 1.5.B states "Color swatches demonstrating available colors and patterns for specified finishes shall be supplied for final approval." Additionally Specification 2.2.H states that paint color shall be selected by TJPA Representative. Please define the colors and patterns before bid so that this package can be accurately priced.				
<b>P1-0544</b>	<b>Signage Details and Types Required for Add Alternate 18</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/13/2014</b>	<b>10/28/2014</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Specification Section 01 10 30 (IFC Drawings for Main Package dated 3/31/14) Specification Section 10 14 63 (ASI 127 dated 9/12/14)  Specification Section 01 10 30/APE E.1.9 Schedule of Alternates for Main Package and Specification Section 10 14 63 2.2 G Electronic Panel Signage describe Add Alternate 18 in which the signage contractor is to provide pricing to incorporate LED screens into the W-5 Wall System of the Beale Street Lobby.  Signage details and types are not given to establish the basis of pricing the alternate.  Please provide the signage details and types to be used in pricing Add Alternate 18.		<b>ANSWER:</b>  REFERENCE: Specification Section 01 10 30 (IFC Drawings for Main Package dated 3/31/14) Specification Section 10 14 63 (ASI 127 dated 9/12/14)  Specification Section 01 10 30/APE E.1.9 Schedule of Alternates for Main Package and Specification Section 10 14 63 2.2 G Electronic Panel Signage describe Add Alternate 18 in which the signage contractor is to provide pricing to incorporate LED screens into the W-5 Wall System of the Beale Street Lobby.  Signage details and types are not given to establish the basis of pricing the alternate.  Please provide the signage details and types to be used in pricing Add Alternate 18.				
<b>P1-0545</b>	<b>Wall Wainscoting in Booth 2 Restrooms</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/13/2014</b>	<b>10/22/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference 13 34 24 2.4.B.3 ASI 127  Specification calls out Wall Wainscoting in the Restrooms		<b>ANSWER:</b>  Reference 13 34 24 2.4.B.3 ASI 127  Specification calls out Wall Wainscoting in the				



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	<p>of Booth Type 2 as "Stainless Steel panel four (5) feet in height above wall base.¿</p> <p>a) Is the correct height 4 ft or 5 ft? b) Please define the stainless steel type, thickness, finish, and the method of fastening to the substrate.</p>					<p>Restrooms of Booth Type 2 as "Stainless Steel panel four (5) feet in height above wall base.¿</p> <p>a) Is the correct height 4 ft or 5 ft? b) Please define the stainless steel type, thickness, finish, and the method of fastening to the substrate.</p>
P1-0546	<p><b>Power Requirements for Floating Signage per Specification Section 10 14 66</b></p> <p>From: Webcor Construction LP      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Specification Section 10 14 66 (ASI 127 dated 9/12/14) Detail 2/SG1-6202 (IFC Drawings for Main Package dated 3/31/14) Detail 3/SG1-6030 (ASI 124 dated 8/18/14)</p> <p>Specification Section 10 14 66 Floating Signage indicates that the signage contractor is to provide power requirements to TJPA/verify electrical power is sized to accommodate Sign Types BS1 Wall Mounted Blade Sign, BS3 4-Icon Overhead Directional, BS4 6-Icon Overhead Directional, SD1 Zone 2 Post-Mounted Bicycle Directional Sign, and SD5 Zone 2 Wall-Mounted Vehicular Directional Warning Sign in the following subsections:</p> <ul style="list-style-type: none"><li>- 10 14 66 2.2 C 1 p (Sign Type BS1)</li><li>- 10 14 66 2.2 C 2 n (Sign Type BS3)</li><li>- 10 14 66 2.2 C 3 o (Sign Type BS4)</li><li>- 10 14 66 2.2 C 4 m (Sign Type SD1)</li><li>- 10 14 66 2.2 C 6 m (Sign Type SD5)</li><li>- 10 14 66 3.1 B</li></ul> <p>In addition, General Sheet Note H on Detail 2/SG1-6202 and General Sheet Note H on Detail 3/SG1-6030 requires the sign subcontractor to provide power requirements to TJPA representative.</p> <p>Per the response to Constructability Comments 7924 the signage specifications will be revised to require the design-build signage contractor to design the signs within the electrical circuiting power allotments in the current design, and to include additional electrical circuits in their</p>	Closed	0P	10/13/2014	10/13/2014	10/20/2014
	<p><b>ANSWER:</b></p> <p>REFERENCE: Specification Section 10 14 66 (ASI 127 dated 9/12/14) Detail 2/SG1-6202 (IFC Drawings for Main Package dated 3/31/14) Detail 3/SG1-6030 (ASI 124 dated 8/18/14)</p> <p>Specification Section 10 14 66 Floating Signage indicates that the signage contractor is to provide power requirements to TJPA/verify electrical power is sized to accommodate Sign Types BS1 Wall Mounted Blade Sign, BS3 4-Icon Overhead Directional, BS4 6-Icon Overhead Directional, SD1 Zone 2 Post-Mounted Bicycle Directional Sign, and SD5 Zone 2 Wall-Mounted Vehicular Directional Warning Sign in the following subsections:</p> <ul style="list-style-type: none"><li>- 10 14 66 2.2 C 1 p (Sign Type BS1)</li><li>- 10 14 66 2.2 C 2 n (Sign Type BS3)</li><li>- 10 14 66 2.2 C 3 o (Sign Type BS4)</li><li>- 10 14 66 2.2 C 4 m (Sign Type SD1)</li><li>- 10 14 66 2.2 C 6 m (Sign Type SD5)</li><li>- 10 14 66 3.1 B</li></ul> <p>In addition, General Sheet Note H on Detail 2/SG1-6202 and General Sheet Note H on Detail 3/SG1-6030 requires the sign subcontractor to provide power requirements to TJPA representative.</p> <p>Per the response to Constructability Comments 7924 the signage specifications will be revised to require the design-build signage contractor to design the signs</p>					





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	<p>signage bid if additional power is determined to be required.</p> <p>Please revise the specifications and general notes to agree with the response to Constructability Comments 7924.</p>					
	<p>within the electrical circuiting power allotments in the current design, and to include additional electrical circuits in their signage bid if additional power is determined to be required.</p> <p>Please revise the specifications and general notes to agree with the response to Constructability Comments 7924.</p>					
P1-0547	Exterior Window Dimensions for Prefabricated Buildings	Open	0P	10/13/2014	10/13/2014	11/10/2014
	From: Webcor Construction LP                      Andrew Kitchen					
	REQUEST:					
	Reference 13 34 24 (ASI 127)					
	The Contract Documents do not dimension the exterior window glazing for the Prefabricated Guard Booths. Please dimension the exterior window glazing before bid so that this package can be accurately priced.					



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<b>P1-0549</b>	<b>Incorrect Reference on A1-7418</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/29/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  A1-7418 ASI 127  Detail 1 calls out "For Guardhouse detail plans ref to dwg sheet A1-7423." A1-7423 was deleted in ASI 127. Please revise this detail.		<b>ANSWER:</b>  A1-7418 ASI 127  Detail 1 calls out "For Guardhouse detail plans ref to dwg sheet A1-7423." A1-7423 was deleted in ASI 127. Please revise this detail.				
<b>P1-0550</b>	<b>Room 01642 Callout</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/20/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference A1-9703 ASI 124  Detail 1 shows Room 01642 and Door 01642A as the GGT Supervisor Booth. A1-2306 shows Room 01642 as SFMTA. Please revise A1-9703 so that Room 01642 and Door 01642A correspond to the SFMTA Booth.		<b>ANSWER:</b>  Reference A1-9703 ASI 124  Detail 1 shows Room 01642 and Door 01642A as the GGT Supervisor Booth. A1-2306 shows Room 01642 as SFMTA. Please revise A1-9703 so that Room 01642 and Door 01642A correspond to the SFMTA Booth.				
<b>P1-0551</b>	<b>Missing Details for Hardware Sets</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/20/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference A1-9703 and A1-9705 ASI 124, 08 78 10 IFC Main Set  The redline markups on these sheets show hardware Set SH-35 and SH-36 for the Prefabricated Guard Booth doors. There are no details for these hardware sets in the Specification. Please provide specifications for SH-35 and SH-36.		<b>ANSWER:</b>  Reference A1-9703 and A1-9705 ASI 124, 08 78 10 IFC Main Set  The redline markups on these sheets show hardware Set SH-35 and SH-36 for the Prefabricated Guard Booth doors. There are no details for these hardware sets in the Specification. Please provide specifications for SH-35 and SH-36.				
<b>P1-0552</b>	<b>SFMTA Booth Restroom Doors</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/20/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Reference A1-9703 ASI 124		<b>ANSWER:</b>  Reference A1-9703 ASI 124				



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	<p>Detail 1 shows the SFMTA Supervisor Booth restroom doors (01640A and 01641A) as Door Type A with a SS/GL Finish. Door Type A is a Hollow Metal Door with no glass. Please confirm whether the door finish is HM or SS/GL and update the door schedule accordingly.</p>					<p>Detail 1 shows the SFMTA Supervisor Booth restroom doors (01640A and 01641A) as Door Type A with a SS/GL Finish. Door Type A is a Hollow Metal Door with no glass. Please confirm whether the door finish is HM or SS/GL and update the door schedule accordingly.</p>
<b>P1-0553</b>	<b>Lights L182A and L183</b>	<b>Closed</b>	<b>0P</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> <p>Reference 13 34 24 2.3.O (ASI 127), E1-4310 (ASI 124), 26 51 00/APA (ASI 118)</p> <p>13 34 24 2.3.O states lighting equipment and devices for the Guard Booth be provided per lighting specifications and drawing E1-4310. E1-4310 shows that the guard booth contains lighting types L182A and L183. L182A and L183 were removed from the L Series Light Types in 26 51 00/APA with ASI 118. Please revise the documents so that the lights shown for the Guard Booth are in the specifications.</p>						<b>ANSWER:</b> <p>Reference 13 34 24 2.3.O (ASI 127), E1-4310 (ASI 124), 26 51 00/APA (ASI 118)</p> <p>13 34 24 2.3.O states lighting equipment and devices for the Guard Booth be provided per lighting specifications and drawing E1-4310. E1-4310 shows that the guard booth contains lighting types L182A and L183. L182A and L183 were removed from the L Series Light Types in 26 51 00/APA with ASI 118. Please revise the documents so that the lights shown for the Guard Booth are in the specifications.</p>
<b>P1-0554</b>	<b>Verbiage for Sign Type SS1</b>	<b>Closed</b>	<b>0P</b>	<b>10/15/2014</b>	<b>10/25/2014</b>	<b>10/28/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b> <p>REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)</p> <p>The Sign Message Schedule indicates that Sign Type SS1, Message A, will be determined. Per 10 14 63 2.2 G, Sign SS1 is to be an LED screen.</p> <p>Please confirm lettering is to be added to the LED screen, and provide the verbiage for Sign Type SS1, amount of letters to be priced, or allowance amount to be used.</p>						<b>ANSWER:</b> <p>REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)</p> <p>The Sign Message Schedule indicates that Sign Type SS1, Message A, will be determined. Per 10 14 63 2.2 G, Sign SS1 is to be an LED screen.</p> <p>Please confirm lettering is to be added to the LED screen, and provide the verbiage for Sign Type SS1, amount of letters to be priced, or allowance amount to be used.</p>



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<b>P1-0555</b>	<b>Screen Type for Sign Type SS1</b>	<b>Closed</b>	<b>0P</b>	<b>10/15/2014</b>	<b>10/25/2014</b>	<b>10/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Specification Section 10 14 63 (ASI 127 dated 9/12/14) Detail 4/SG1-6010 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 14 63 2.2 G Electronic Panel Signage calls for LED screens per Add Alternate No. 18.  Detail 4/SG1-6010 depicts an LCD screen to be installed.  Please provide the screen type required for Sign Type SS1 and revise the plans and specifications to match.						REFERENCE: Specification Section 10 14 63 (ASI 127 dated 9/12/14) Detail 4/SG1-6010 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 14 63 2.2 G Electronic Panel Signage calls for LED screens per Add Alternate No. 18.  Detail 4/SG1-6010 depicts an LCD screen to be installed.  Please provide the screen type required for Sign Type SS1 and revise the plans and specifications to match.
<b>P1-0556</b>	<b>Display Type for Sign Type SS1</b>	<b>Closed</b>	<b>0P</b>	<b>10/15/2014</b>	<b>10/25/2014</b>	<b>10/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Specification Section 10 14 63 (ASI 127 dated 9/12/14) Detail 4/SG1-6010 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 14 63 2.2 G Electronic Panel Signage calls for LED screens per Add Alternate No. 18.  Detail 4/SG1-6010 depicts an LCD screen to be installed.  Please clarify if Sign Type SS1 is to be an LED or LCD screen and revise the plans and specifications to match.						REFERENCE: Specification Section 10 14 63 (ASI 127 dated 9/12/14) Detail 4/SG1-6010 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 14 63 2.2 G Electronic Panel Signage calls for LED screens per Add Alternate No. 18.  Detail 4/SG1-6010 depicts an LCD screen to be installed.  Please clarify if Sign Type SS1 is to be an LED or LCD screen and revise the plans and specifications to match.
<b>P1-0557</b>	<b>Verbiage for Message A of Sign Type CT1 Clipper Tag on Elevator</b>	<b>Closed</b>	<b>0P</b>	<b>10/15/2014</b>	<b>10/25/2014</b>	<b>10/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>



P1-0559	Message Designation for Signage per Sign Message Schedule	Closed	0P	10/15/2014	10/25/2014	10/31/2014
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<b>P1-0563</b>	<b>Power Requirements for Sign Type SD5 Vehicular Direction Warning Sign</b>	<b>Closed</b>	<b>0P</b>	<b>10/15/2014</b>	<b>10/25/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 14 66 (ASI 127 dated 9/12/14) Sheet E1-2203 (ASI 118 dated 6/20/14) Sheet E1-4203 (ASI 118 dated 6/20/14)  Per Specification Section 10 14 66 2.2 C 6 m Floating Signage, Sign Type SD5 Vehicular Direction Warning Sign is to have power, but Sheets E1-2203 and E1-4203 do not show power for Sign Type SD5 at the vehicular ramp.  Per Specification Section 10 14 66 2.2 C 4 m Floating Signage, Sign Type SD1 Post-Mounted Bicycle Directional Sign is to have power, but Sheet E1-2310 does not show power to the SD1 signs.  Please provide electrical information for all floating signage.		REFERENCE: Specification Section 10 14 66 (ASI 127 dated 9/12/14) Sheet E1-2203 (ASI 118 dated 6/20/14) Sheet E1-4203 (ASI 118 dated 6/20/14)  Per Specification Section 10 14 66 2.2 C 6 m Floating Signage, Sign Type SD5 Vehicular Direction Warning Sign is to have power, but Sheets E1-2203 and E1-4203 do not show power for Sign Type SD5 at the vehicular ramp.  Per Specification Section 10 14 66 2.2 C 4 m Floating Signage, Sign Type SD1 Post-Mounted Bicycle Directional Sign is to have power, but Sheet E1-2310 does not show power to the SD1 signs.  Please provide electrical information for all floating signage.				
<b>P1-0564</b>	<b>Warranty Period Requirement for Floating Signage</b>	<b>Closed</b>	<b>0P</b>	<b>10/15/2014</b>	<b>10/25/2014</b>	<b>10/16/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 14 66 (ASI 127 dated 9/12/14)  Per Specification Section 10 14 66 1.7 A 2 Floating Signage, the warranty period is subject to final approval by the TJPA representative.  Please provide the desire warranty period.		REFERENCE: Specification Section 10 14 66 (ASI 127 dated 9/12/14)  Per Specification Section 10 14 66 1.7 A 2 Floating Signage, the warranty period is subject to final approval by the TJPA representative.  Please provide the desire warranty period.				
<b>P1-0565</b>	<b>Mounting Height for Sign Types BS1, BS3, and BS4</b>	<b>Closed</b>	<b>0P</b>	<b>10/15/2014</b>	<b>10/25/2014</b>	<b>10/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 14 66 (ASI 127 dated 9/12/14) Sheet SG1-6030 (ASI 124 dated 8/18/14)		REFERENCE: Specification Section 10 14 66 (ASI 127 dated 9/12/14)				





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	<p>Per Specification Section 10 14 66 Floating Signage subsections 2.2 C 1 i, 2.2 C 2 i, and 2.2 C 3 n, the mounting height above grade for Sign Types BS1 Wall-Mounted Blade Sign, BS3 4-Icon Overhead Directional, and BS4 6-Icon Overhead Directional is 9'-0".</p> <p>Per SG1-6030, the min. mounting height for Sign Types BS1, BS3, and BS4 9'-2" AFF.</p> <p>Please revise the drawings and specifications to agree with each other.</p>					
				Sheet SG1-6030 (ASI 124 dated 8/18/14)		
				Per Specification Section 10 14 66 Floating Signage subsections 2.2 C 1 i, 2.2 C 2 i, and 2.2 C 3 n, the mounting height above grade for Sign Types BS1 Wall-Mounted Blade Sign, BS3 4-Icon Overhead Directional, and BS4 6-Icon Overhead Directional is 9'-0".		
				Per SG1-6030, the min. mounting height for Sign Types BS1, BS3, and BS4 9'-2" AFF.		
				Please revise the drawings and specifications to agree with each other.		
P1-0566	Mounting Height for Sign Type SD5	Closed	0P	10/15/2014	10/25/2014	10/24/2014
	From: Webcor Construction LP      Tram Nguyen					
	REQUEST:			ANSWER:		
	REFERENCE: Specification Section 10 14 66 (ASI 127 dated 9/12/14) Detail 4/SG1-6202 (IFC Drawings for Main Package dated 3/31/14)			REFERENCE: Specification Section 10 14 66 (ASI 127 dated 9/12/14) Detail 4/SG1-6202 (IFC Drawings for Main Package dated 3/31/14)		
	Specification Section 10 14 66 2.2 C 6 i Floating Signage Indicates that the mounting height for Sign Type SD5 Vehicular Direction Warning Sign is as indicated on the Construction Intent Drawings.			Specification Section 10 14 66 2.2 C 6 i Floating Signage Indicates that the mounting height for Sign Type SD5 Vehicular Direction Warning Sign is as indicated on the Construction Intent Drawings.		
	Detail 4/SG1-6202 does not provide the mounting height for Sign Type SD5.			Detail 4/SG1-6202 does not provide the mounting height for Sign Type SD5.		
	Please provide the mounting height for Sign Type SD5.			Please provide the mounting height for Sign Type SD5.		



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P1-0567	Warranty Period Requirement for Specification Section 10 14 73 Glass Signage	Closed	0P	10/16/2014	10/26/2014	10/17/2014
From: Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE:						REFERENCE:
Specification Section 10 14 73 (ASI 127 dated 9/12/14)						Specification Section 10 14 73 (ASI 127 dated 9/12/14)
Per Specification Section 10 14 73 1.6 A 2 Glass Signage the warranty period is subject to final approval by the TJPA representative.						Per Specification Section 10 14 73 1.6 A 2 Glass Signage the warranty period is subject to final approval by the TJPA representative.
Please provide the desire warranty period.						Please provide the desire warranty period.
P1-0568	Electrical Power Requirement for Sign Type BG1	Closed	0P	10/16/2014	10/26/2014	10/22/2014
From: Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE:						REFERENCE:
Specification Section 10 14 73 (ASI 127 dated 9/12/14)						Specification Section 10 14 73 (ASI 127 dated 9/12/14)
Specification Section 10 14 73 1.3 E and Specification Section 10 14 73 3.1 B Glass Signage indicate that Sign Type BG1 Applied Message Band on Glass is powered. Specification Section 10 14 73 2.2 and Sheet SG1-6191 do not show electrical components associated with Sign Type BG1.						Specification Section 10 14 73 1.3 E and Specification Section 10 14 73 3.1 B Glass Signage indicate that Sign Type BG1 Applied Message Band on Glass is powered. Specification Section 10 14 73 2.2 and Sheet SG1-6191 do not show electrical components associated with Sign Type BG1.
In addition, the response to Constructability Comment 7927 states "The signage specifications will be revised to require the design-build signage contractor to design the signs within the electrical circuiting power allotments in the current design, and to include additional electrical circuits in their signage bid if additional power is determined to be required."						In addition, the response to Constructability Comment 7927 states "The signage specifications will be revised to require the design-build signage contractor to design the signs within the electrical circuiting power allotments in the current design, and to include additional electrical circuits in their signage bid if additional power is determined to be required."
Please confirm electrical power is not required to be supplied to Sign Type BG1 and delete the electrical references, or revise the contract documents to reflect the response to Constructability Comment 7927.						Please confirm electrical power is not required to be supplied to Sign Type BG1 and delete the electrical references, or revise the contract documents to reflect the response to Constructability Comment 7927.



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<b>P1-0569</b>	<b>Scheduling Language per Specification Section 10 14 83 Interpretive Graphics</b>	<b>Closed</b>	<b>0P</b>	<b>10/16/2014</b>	<b>10/26/2014</b>	<b>10/21/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 14 83 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 10 14 83 (ASI 127 dated 9/12/14)				
Specification Section 10 14 83 2.4 G Interpretive Graphics states that signage development is independent of schedule, and to allow adequate time for development of product.		Specification Section 10 14 83 2.4 G Interpretive Graphics states that signage development is independent of schedule, and to allow adequate time for development of product.				
The signage subcontractor will be required to complete the work as required by the construction schedule.		The signage subcontractor will be required to complete the work as required by the construction schedule.				
Please remove the schedule language associated with 10 14 83 2.4 G.		Please remove the schedule language associated with 10 14 83 2.4 G.				
<b>P1-0570</b>	<b>Drawings for Interpretive Graphics</b>	<b>Closed</b>	<b>0P</b>	<b>10/16/2014</b>	<b>10/26/2014</b>	<b>10/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 14 83 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 10 14 83 (ASI 127 dated 9/12/14)				
Specification Section 10 14 83 3.3 A Interpretive Graphics directs that the signage subcontractor is to locate signs and accessories where indicated.		Specification Section 10 14 83 3.3 A Interpretive Graphics directs that the signage subcontractor is to locate signs and accessories where indicated.				
No drawing has been provided locating the interpretive graphics. Please provide a document laying out the interpretive graphics.		No drawing has been provided locating the interpretive graphics. Please provide a document laying out the interpretive graphics.				
<b>P1-0571</b>	<b>Aluminum Thickness for Interpretive Graphic Signage</b>	<b>Closed</b>	<b>0P</b>	<b>10/16/2014</b>	<b>10/26/2014</b>	<b>10/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 14 83 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 10 14 83 (ASI 127 dated 9/12/14)				
Specification Section 10 14 83 2.1 A 1 a Interpretive Graphics states that the thickness of aluminum for		Specification Section 10 14 83 2.1 A 1 a Interpretive				





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<p><b>From:</b> Webcor Construction LP</p> <p><b>Tram Nguyen</b></p> <p><b>REQUEST:</b></p> <p>REFERENCE:</p> <p>Specification Section 10 18 00 (ASI 127 dated 9/12/14)</p> <p>Specification Section 10 18 00 1.7 A 2 Informational Kiosks indicates that the warranty period for informational kiosks is subject to final approval by TJPA.</p> <p>Please provide the desired warranty duration to be bid.</p>						<p><b>ANSWER:</b></p> <p>REFERENCE:</p> <p>Specification Section 10 18 00 (ASI 127 dated 9/12/14)</p> <p>Specification Section 10 18 00 1.7 A 2 Informational Kiosks indicates that the warranty period for informational kiosks is subject to final approval by TJPA.</p> <p>Please provide the desired warranty duration to be bid.</p>
<b>P1-0575</b>	<b>Electrical Power Requirements for Informational Kiosks</b>	<b>Closed</b>	<b>0P</b>	<b>10/17/2014</b>	<b>10/27/2014</b>	<b>10/21/2014</b>
<p><b>From:</b> Webcor Construction LP</p> <p><b>Tram Nguyen</b></p> <p><b>REQUEST:</b></p> <p>REFERENCE:</p> <p>Specification Section 10 18 00 (ASI 127 dated 9/12/14)</p> <p>Detail 2/SG1-6020 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Detail 3/SG1-6021 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Detail 2/SG1-6022 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 10 18 00 3.1 B Informational Kiosks indicates that the subcontractor is to verify that electrical power is sized to accommodate the informational kiosks. General Note I on Detail 2/SG1-6020, General Note H on Detail 3/SG1-6021, and General Note H on Detail 2/SG1-6022 indicate that the sign subcontractor is to provide power requirements to TJPA.</p> <p>Per the response to Constructability Comments 7932 ¿ 7934, 7938 and 7939 state that the signage specifications will be revised to require the design-build signage contractor to design the signs within the electrical circuiting power allotments in the current design, and to include additional electrical circuits in their signage bid if additional power is determined to be required.</p> <p>Pleaser revise the plans and specifications to agree with the response to the associated constructability comments.</p>						<p><b>ANSWER:</b></p> <p>REFERENCE:</p> <p>Specification Section 10 18 00 (ASI 127 dated 9/12/14)</p> <p>Detail 2/SG1-6020 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Detail 3/SG1-6021 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Detail 2/SG1-6022 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Specification Section 10 18 00 3.1 B Informational Kiosks indicates that the subcontractor is to verify that electrical power is sized to accommodate the informational kiosks. General Note I on Detail 2/SG1-6020, General Note H on Detail 3/SG1-6021, and General Note H on Detail 2/SG1-6022 indicate that the sign subcontractor is to provide power requirements to TJPA.</p> <p>Per the response to Constructability Comments 7932 ¿ 7934, 7938 and 7939 state that the signage specifications will be revised to require the design-build signage contractor to design the signs within the electrical circuiting power allotments in the current design, and to include additional electrical circuits in their signage bid if additional power is determined to be required.</p> <p>Pleaser revise the plans and specifications to agree</p>



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with the response to the associated constructability comments.						
P1-0576	Electrical Power Requirements for Informational Kiosks	Closed	0P	10/17/2014	10/27/2014	10/28/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:			ANSWER:			
REFERENCE:			REFERENCE:			
Specification Section 10 18 00 (ASI 127 dated 9/12/14)			Specification Section 10 18 00 (ASI 127 dated 9/12/14)			
Specification Section 10 18 00 refers to the ¿Construction Intent Drawings¿ throughout the specification (ex. 10 18 00 1.3 F).			Specification Section 10 18 00 refers to the ¿Construction Intent Drawings¿ throughout the specification (ex. 10 18 00 1.3 F).			
Please confirm that ¿Construction Intent Drawings¿ is to be revised to ¿Signage Drawings¿, or provide the referenced construction intent drawings.			Please confirm that ¿Construction Intent Drawings¿ is to be revised to ¿Signage Drawings¿, or provide the referenced construction intent drawings.			



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<b>P1-0577</b>	<b>Message &amp; Graphic Requirements for Sign Type KC1</b>	<b>Closed</b>	<b>0P</b>	<b>10/17/2014</b>	<b>10/27/2014</b>	<b>10/22/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Detail 1/SG1-6020 (IFC Drawings for Main Package dated 3/31/14) Sign Message Schedule (ASI 127 dated 9/12/14)  Specification Section 10 18 00 2.2 C 1 i Informational Kiosks indicates that Sign Type KC1 Combined Touchscreen & Digital Display has 2 faces with graphics.  Per Detail 1/SG1-6020 a graphic is only present on the front face of the kiosk.  Per the Sign Message Schedule, Message D is (NO MESSAGE).  Please clarify the messages/graphics to be used for Sign Type KC1.		REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Detail 1/SG1-6020 (IFC Drawings for Main Package dated 3/31/14) Sign Message Schedule (ASI 127 dated 9/12/14)  Specification Section 10 18 00 2.2 C 1 i Informational Kiosks indicates that Sign Type KC1 Combined Touchscreen & Digital Display has 2 faces with graphics.  Per Detail 1/SG1-6020 a graphic is only present on the front face of the kiosk.  Per the Sign Message Schedule, Message D is (NO MESSAGE).  Please clarify the messages/graphics to be used for Sign Type KC1.				
<b>P1-0578</b>	<b>Sign Type Clarification per Detail 1/SG1-6020</b>	<b>Closed</b>	<b>0P</b>	<b>10/17/2014</b>	<b>10/27/2014</b>	<b>10/28/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail 1/SG1-6020 (IFC Drawings for Main Package dated 3/31/14)  Detail 1/SG1-6020 is titled KC1: Combined Touchscreen & Digital Display.  The referenced detail notes one of the elevations is for KP1 One-Sided Digital Display Panel Kiosk - With Speakers.  Please clarify the sign type within Detail 1/SG1-6020.		REFERENCE: Detail 1/SG1-6020 (IFC Drawings for Main Package dated 3/31/14)  Detail 1/SG1-6020 is titled KC1: Combined Touchscreen & Digital Display.  The referenced detail notes one of the elevations is for KP1 One-Sided Digital Display Panel Kiosk - With Speakers.  Please clarify the sign type within Detail 1/SG1-6020.				







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P1-0580	Ticket and Added Value Machine Information	Open	0P	10/17/2014	10/27/2014	
From: Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14)				
The following Specification Sections indicate that the ticket and added value machines are not in the contract, but the signage contractor is to coordinate with the machine manufacturer for power, dimensions and support structure:		The following Specification Sections indicate that the ticket and added value machines are not in the contract, but the signage contractor is to coordinate with the machine manufacturer for power, dimensions and support structure:				
10 18 00 2.2 F 1 n 10 18 00 2.2 F 1 o 10 18 00 2.2 F 2 n 10 18 00 2.2 F 2 o		10 18 00 2.2 F 1 n 10 18 00 2.2 F 1 o 10 18 00 2.2 F 2 n 10 18 00 2.2 F 2 o				
In addition, final quantities are to be determined by the TJPA representative.		In addition, final quantities are to be determined by the TJPA representative.				
Please provide the manufacturer of the ticket and added value machines, and final quantities for the machines.		Please provide the manufacturer of the ticket and added value machines, and final quantities for the machines.				



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P1-0581	Power Requirements for Ticket and Added Value Machines	Open	0P	10/17/2014	10/27/2014	12/09/2014
From: Webcor Construction LP      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14)				
The following Specification Sections indicate that the ticket and added value machines have separate power requirements from the associated kiosks:		The following Specification Sections indicate that the ticket and added value machines have separate power requirements from the associated kiosks:				
10 18 00 2.2 F 1 n 10 18 00 2.2 F 1 o 10 18 00 2.2 F 2 n 10 18 00 2.2 F 2 o		10 18 00 2.2 F 1 n 10 18 00 2.2 F 1 o 10 18 00 2.2 F 2 n 10 18 00 2.2 F 2 o				
Electrical drawings do not show separate power for the ticket and added value machines.		Electrical drawings do not show separate power for the ticket and added value machines.				
Please confirm no additional circuits are required for the ticket and added value machines, or revise the electrical documents to reflect the desired power and data requirements.		Please confirm no additional circuits are required for the ticket and added value machines, or revise the electrical documents to reflect the desired power and data requirements.				



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P1-0582	Message for Sign Types RR5 and RR6	Closed	0P	10/21/2014	10/31/2014	10/24/2014
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Sheet SG1-6200 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Sheet SG1-6200 (IFC Drawings for Main Package dated 3/31/14)				
The Sign Message Schedule does not provide a message for Sign RR6-P-02-017, Sign RR6-U-06-016, Sign RR5-U-06-015		The Sign Message Schedule does not provide a message for Sign RR6-P-02-017, Sign RR6-U-06-016, Sign RR5-U-06-015				
Per Detail 6/SG1-6200 Sign Types RR5 and RR6 are to receive Restroom messages.		Per Detail 6/SG1-6200 Sign Types RR5 and RR6 are to receive Restroom messages.				
Specification Section 10 14 36 2.2 J 17 Sign Type RR5 designates "Toilet Room Door. Women" and Specification Section 10 14 36 2.2 J 18 designates Sign Type RR6 designates Toilet Room Door. Men".		Specification Section 10 14 36 2.2 J 17 Sign Type RR5 designates "Toilet Room Door. Women" and Specification Section 10 14 36 2.2 J 18 designates Sign Type RR6 designates Toilet Room Door. Men".				
Please provide the required messages and graphics for Sign Types RR5 and RR6.		Please provide the required messages and graphics for Sign Types RR5 and RR6.				
P1-0583	Verbiage for Sign Type SM1 Wall-Mounted Schedule Board	Closed	0P	10/21/2014	10/31/2014	10/24/2014
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (ASI 127 dated 9/12/14) Sign Message Schedule Specification Section 10 14 63		REFERENCE: (ASI 127 dated 9/12/14) Sign Message Schedule Specification Section 10 14 63				
The Sign Message Schedule indicates that Sign Type SM1 Wall-Mounted Schedule Board a message will be determined. Per Specification Section 10 14 63 2.2 G Electronic Panel Signage, Sign Type SM1 is to be an LED screen.		The Sign Message Schedule indicates that Sign Type SM1 Wall-Mounted Schedule Board a message will be determined. Per Specification Section 10 14 63 2.2 G Electronic Panel Signage, Sign Type SM1 is to be an LED screen.				
Please confirm lettering is to be added to the LED screen, and provide the verbiage for Sign Type SM1 Wall-Mounted Schedule Board, amount of letters to be priced, or allowance amount to be used.		Please confirm lettering is to be added to the LED screen, and provide the verbiage for Sign Type SM1 Wall-Mounted Schedule Board, amount of letters to be priced, or allowance amount to be used.				
P1-0584	Screen Type for Sign Type SM1 Wall-Mounted Schedule Board	Closed	0P	10/21/2014	10/31/2014	10/28/2014
<b>From:</b> Webcor Construction LP      Tram Nguyen						



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P1-0586	Verbiage for Sign Type ID1 Room Name with Braille	Closed	0P	10/21/2014	10/31/2014	10/28/2014
From: Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)  Per the Sign Message Schedule, several message locations indicate "MSG TBD" or "MESSAGE TBD" for Sign Type ID1 Room Name with Braille. For example, ID1-U-02-008 Message A, ID1-U-04-010 Message, and ID1-G-04-012 Message.  Please provide the ¿TBD¿ verbiage, or a basis of bid for additional lettering required to be on Sign Type ID1 is to be determined.		<b>ANSWER:</b>  REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)  Per the Sign Message Schedule, several message locations indicate "MSG TBD" or "MESSAGE TBD" for Sign Type ID1 Room Name with Braille. For example, ID1-U-02-008 Message A, ID1-U-04-010 Message, and ID1-G-04-012 Message.  Please provide the ¿TBD¿ verbiage, or a basis of bid for additional lettering required to be on Sign Type ID1 is to be determined.				
P1-0587	Verbiage for Sign Type AC1 Art Commission Plaque	Closed	0P	10/21/2014	10/31/2014	10/28/2014
From: Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)  The Sign Message Schedule states "Art Program message to be provided by TJPA" for Sign Type AC1 Art Commission Plaque Message A.  Detail 6/SG1-6201 titled AC1 - Art Commission Plaque depicts language for the plaque.  Please confirm that the verbiage depicted in Detail 6/SG1-6201 is what will be required for Sign Type AC1 or provide the correct Art Program message referenced in the Sign Message Schedule.  In addition, if Detail 6/SG1-6201 is to be used for verbiage, please revise the Sign Message Schedule to reference the correct location of Message A.		<b>ANSWER:</b>  REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)  The Sign Message Schedule states "Art Program message to be provided by TJPA" for Sign Type AC1 Art Commission Plaque Message A.  Detail 6/SG1-6201 titled AC1 - Art Commission Plaque depicts language for the plaque.  Please confirm that the verbiage depicted in Detail 6/SG1-6201 is what will be required for Sign Type AC1 or provide the correct Art Program message referenced in the Sign Message Schedule.  In addition, if Detail 6/SG1-6201 is to be used for verbiage, please revise the Sign Message Schedule to reference the correct location of Message A.				
P1-0588	Electronic Display Options for Sign Type PD1 Bus Deck Dock ID	Closed	0P	10/21/2014	10/21/2014	10/31/2014
From: Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE:		<b>ANSWER:</b>  REFERENCE:				



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	<p>Detail 3/SG1-6003 (IFC Drawings for Main Package dated 3/31/14) Specification 10 14 26 (ASI 127 dated 9/12/14)</p> <p>Key Note 5 on Detail 3/SG1-6003 indicates that the 40¿ electronic display on Sign Type PD1 Bus Deck Dock ID is to be selected based upon performance.</p> <p>Per Specification Section 10 14 26 2.1 C 1 o Pylon Signage there are 3 ea. named flat panel displays specified.</p> <p>Please confirm the flat panel displays for Sign Type PD1 are to be selected per Specification Section 10 14 26 and remove the reference to performance requirements.</p> <p>However, if the flat panel diplays are to be selected based upon performance, please provide the performance requirements.</p>					
P1-0589	Information for Audible Button on Sign Type PD2 Muni Plaza Bus ID	Open	0P	10/21/2014	10/31/2014	10/28/2014
	<p>From: Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 4/SG1-6003 Detail 3/SG1-6003</p> <p>Per Detail 4/SG1-6003 and Key Note 12 on Detail 3/SG1-6003, there is an audible ¿Next Bus¿ push button on Sign Type PD2 Muni Plaza Bus ID.</p> <p>The audible ¿Next Bus¿ button is not specified, nor is there any information on electrical information (power requirements, data requirements).</p> <p>Please provide a specification and electrical requirements for the ¿Next Bus¿ button.</p>					
			<p><b>ANSWER:</b></p> <p>REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 4/SG1-6003 Detail 3/SG1-6003</p> <p>Per Detail 4/SG1-6003 and Key Note 12 on Detail 3/SG1-6003, there is an audible ¿Next Bus¿ push button on Sign Type PD2 Muni Plaza Bus ID.</p> <p>The audible ¿Next Bus¿ button is not specified, nor is there any information on electrical information (power requirements, data requirements).</p> <p>Please provide a specification and electrical requirements for the ¿Next Bus¿ button.</p>			
P1-0590	Message Confirmation for PD2-G-06-0616	Closed	0P	10/21/2014	10/31/2014	10/28/2014



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	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)</p> <p>The messages for PD2-G-06-016 have an open quotation mark. Message B and Message D currently reads: MUN-2, 38 Geary, 38L Geary Limited".</p> <p>Please confirm the message as shown is correct or provide the correct message.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)</p> <p>The messages for PD2-G-06-016 have an open quotation mark. Message B and Message D currently reads: MUN-2, 38 Geary, 38L Geary Limited".</p> <p>Please confirm the message as shown is correct or provide the correct message.</p>					
<b>P1-0591</b>	<b>Route Information for Sign Type PD2 Muni Plaza Bus ID</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>11/06/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Detail 4/SG1-6003 (IFC Drawings for Main Package dated 3/31/14)</p> <p>In multiple locations, it is indicated that final route information for Sign Type PD2 Muni Plaza Bus ID is to be provided by TJPA (ex. PD2-G-06-014).</p> <p>Detail 4/SG1-6003 does not show any layout information for the referenced route information.</p> <p>Please provide the referenced information and the layout for that information.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Detail 4/SG1-6003 (IFC Drawings for Main Package dated 3/31/14)</p> <p>In multiple locations, it is indicated that final route information for Sign Type PD2 Muni Plaza Bus ID is to be provided by TJPA (ex. PD2-G-06-014).</p> <p>Detail 4/SG1-6003 does not show any layout information for the referenced route information.</p> <p>Please provide the referenced information and the layout for that information.</p>					
<b>P1-0592</b>	<b>Graphic Requirements for Sign PD2-G-06-014</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/28/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)</p> <p>Sign PD2-G-06-014 on the Sign Message Schedule does not call for a graphic (Muni, Marin, etc.).</p> <p>Please confirm no graphics are required at this location.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)</p> <p>Sign PD2-G-06-014 on the Sign Message Schedule does not call for a graphic (Muni, Marin, etc.).</p> <p>Please confirm no graphics are required at this location.</p>					



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<b>P1-0593</b>	<b>Vinyl Graphics per Muni Requirements</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Detail 3/SG1-6003 (IFC Drawings for Main Package dated 3/31/14)  Key Note 14 of Detail 3/SG1-6003 indicates that applied vinyl graphics are specified per Muni's requirement.  Please provide Muni's requirement.						<b>ANSWER:</b>  REFERENCE: Detail 3/SG1-6003 (IFC Drawings for Main Package dated 3/31/14)  Key Note 14 of Detail 3/SG1-6003 indicates that applied vinyl graphics are specified per Muni's requirement.  Please provide Muni's requirement.
<b>P1-0594</b>	<b>Accessible Variants for Sign Types KT1 and KT2</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/28/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Detail 2/SG1-6021 (IFC Drawings for Main Package dated 3/31/14)  Detail 2/SG1-6021 identifies an accessible variant for Sign Type KT1 Ticket Vending Kiosk and Sign Type KT2 Ticket Vending Kiosk with Digital Display.  The signage plan sheets do not identify locations for the accessible variants required at Sign Types KT1 and KT2.  Please confirm no accessible variants at Sign Types KT1 and KT2 are required, or provide the locations of the accessible variants.						<b>ANSWER:</b>  REFERENCE: Detail 2/SG1-6021 (IFC Drawings for Main Package dated 3/31/14)  Detail 2/SG1-6021 identifies an accessible variant for Sign Type KT1 Ticket Vending Kiosk and Sign Type KT2 Ticket Vending Kiosk with Digital Display.  The signage plan sheets do not identify locations for the accessible variants required at Sign Types KT1 and KT2.  Please confirm no accessible variants at Sign Types KT1 and KT2 are required, or provide the locations of the accessible variants.
<b>P1-0595</b>	<b>Graphic Requirements for Sign Type KT1 Ticket Vending Kiosk</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Sheet SG1-6021 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 18 00 2.2 F 1 i Informational Kiosks indicates that Sign Type KT1 Ticket Vending Kiosk						<b>ANSWER:</b>  REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Sheet SG1-6021 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 18 00 2.2 F 1 i Informational





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	<p>is equipped with 2 faces of graphics.</p> <p>Per Sheet SG1-6021 and the Sign Message Schedule, only one graphic is shown.</p> <p>Please confirm KT1 Signs are only to receive 1 graphic.</p>					<p>Kiosks indicates that Sign Type KT1 Ticket Vending Kiosk is equipped with 2 faces of graphics.</p> <p>Per Sheet SG1-6021 and the Sign Message Schedule, only one graphic is shown.</p> <p>Please confirm KT1 Signs are only to receive 1 graphic.</p>
<b>P1-0596</b>	<b>Graphics and Messages for Sign Type KT2 Ticket Vending Kiosk with Digital Displ</b> Closed		<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Sheet SG1-6021 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Sheet SG1-6021 (IFC Drawings for Main Package dated 3/31/14)				
Specification Section 10 18 00 2.2 F 2 i Informational Kiosks indicates that Sign Type KT2 Ticket Vending Kiosk with Digital Display is equipped with 2 faces of graphics.		Specification Section 10 18 00 2.2 F 2 i Informational Kiosks indicates that Sign Type KT2 Ticket Vending Kiosk with Digital Display is equipped with 2 faces of graphics.				
Per Sheet SG1-6021 has only one graphic is shown. In addition, per the Sign Message Schedule, one message is given and the other is has ¿(NO MESSAGE)¿.		Per Sheet SG1-6021 has only one graphic is shown. In addition, per the Sign Message Schedule, one message is given and the other is has ¿(NO MESSAGE)¿.				
Please clarify the graphics/messages to be provided on Sign Type KT2.		Please clarify the graphics/messages to be provided on Sign Type KT2.				



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P1-0597	Verbiage Layout for Sign Type PS1 Sidewalk Pylon	Closed	0P	10/21/2014	10/31/2014	10/31/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Sheet SG1-6000 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Sheet SG1-6000 (IFC Drawings for Main Package dated 3/31/14)				
The Sign Message Schedule calls out for ¿TRANSBAY TRANSIT CENTER¿ to be installed at several of the Sign Type PS1 Sidewalk Pylon sign locations.		The Sign Message Schedule calls out for ¿TRANSBAY TRANSIT CENTER¿ to be installed at several of the Sign Type PS1 Sidewalk Pylon sign locations.				
Sheet SG1-6000 does not provide material or layout of the verbiage (i.e. where on each face the verbiage is to appear, is it painted on, font size, word separation, etc.).		Sheet SG1-6000 does not provide material or layout of the verbiage (i.e. where on each face the verbiage is to appear, is it painted on, font size, word separation, etc.).				
Please provide the layout for verbiage required to be on Sign Type PS1.		Please provide the layout for verbiage required to be on Sign Type PS1.				
P1-0598	Verbiage for Sign Type PS1 Sidewalk Pylon	Closed	0P	10/21/2014	10/31/2014	10/31/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)		REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)				
Per the Sign Message Schedule, there are messages on PS1-G-02-023 and PS1-G-02-036 which are to be determined.		Per the Sign Message Schedule, there are messages on PS1-G-02-023 and PS1-G-02-036 which are to be determined.				
Please provide the ¿TBD¿ messages for Sign Type PS1 Sidewalk Pylon, amount of letters to be priced, or allowance amount to be used.		Please provide the ¿TBD¿ messages for Sign Type PS1 Sidewalk Pylon, amount of letters to be priced, or allowance amount to be used.				
P1-0599	Spacing Dimensions for Sign Type PS2 Sidewalk Pylon	Closed	0P	10/21/2014	10/31/2014	11/03/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Detail 1/SG1-6001 (IFC Drawings for Main Package dated 3/31/14) Sign Message Schedule (ASI 127 dated 9/12/14)		REFERENCE: Detail 1/SG1-6001 (IFC Drawings for Main Package dated 3/31/14) Sign Message Schedule (ASI 127 dated 9/12/14)				

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	<p>Per Detail 1/SG1-6001 the area above the PS2 Base is 5'-6", and the surface applied icons are 6" with a 3-5/8" separation.</p> <p>Based upon these dimensions, the maximum amount of icon which can be applied is 6.</p> <p>Per the Sign Message Schedule, the PS2 signs have 1 side which exceed this maximum. Please revise the spacing or number of required icons.</p>					
P1-0600	<p><b>Sole Source for Pylon Signage per Specification Section 10 14 26</b></p> <p>From: Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14) Specification Section 00 05 20 (dated 8/11/14)</p> <p>Per Specification Section 10 14 26 Pylon Signage (Specification Section 10 14 26 2.1 B 1 e for example), all of the pylon signage material is to be made of Kastone by Kreysler and Associates.</p> <p>Per Specification Section 00 05 20 2.07 Agreement, sole source procurement is only acceptable with prior approval by TJPA.</p> <p>Please confirm it is acceptable to sole source all of the signage pylon material.</p>	Closed	OP	10/21/2014	10/31/2014	10/30/2014
	<p>Per Detail 1/SG1-6001 the area above the PS2 Base is 5'-6", and the surface applied icons are 6" with a 3-5/8" separation.</p> <p>Based upon these dimensions, the maximum amount of icon which can be applied is 6.</p> <p>Per the Sign Message Schedule, the PS2 signs have 1 side which exceed this maximum. Please revise the spacing or number of required icons.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Specification Section 10 14 26 (ASI 127 dated 9/12/14) Specification Section 00 05 20 (dated 8/11/14)</p> <p>Per Specification Section 10 14 26 Pylon Signage (Specification Section 10 14 26 2.1 B 1 e for example), all of the pylon signage material is to be made of Kastone by Kreysler and Associates.</p> <p>Per Specification Section 00 05 20 2.07 Agreement, sole source procurement is only acceptable with prior approval by TJPA.</p> <p>Please confirm it is acceptable to sole source all of the signage pylon material.</p>					



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<b>P1-0601</b>	<b>Painting Specifications for Signage Materials M8 and M13</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Color/Font/Material/Symbol/Icons Report Detail 4/SG1-6001  Per the Color/Font/Material/Symbol/Icons Report, Materials M8 and M13 require painting in some locations (Sign Types PS4 Rooftop Directional Pylon Sign with Exit Sign and RR4 Restroom Signage for example).  While colors are provided, no specification for painting of the signs has been provided (i.e. multiple manufacturers, mil thickness, prep requirements, etc.).  Please provide a specification (including light enhancing paint called out on Key Note 3C on Detail 4/SG1-6001) for signage to receive paint.		<b>ANSWER:</b>  REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Color/Font/Material/Symbol/Icons Report Detail 4/SG1-6001  Per the Color/Font/Material/Symbol/Icons Report, Materials M8 and M13 require painting in some locations (Sign Types PS4 Rooftop Directional Pylon Sign with Exit Sign and RR4 Restroom Signage for example).  While colors are provided, no specification for painting of the signs has been provided (i.e. multiple manufacturers, mil thickness, prep requirements, etc.).  Please provide a specification (including light enhancing paint called out on Key Note 3C on Detail 4/SG1-6001) for signage to receive paint.				
<b>P1-0602</b>	<b>Unidentified Icons per Sign Message Schedule</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>11/13/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Color/Font/Material/Symbol/Icons Report  The Sign Message Schedule calls out for Icons not identified on the Color/Font/Material/Symbol/Icons Report (PS6-G-05-022 calls for Icons ASU-1, ACT-2 and ARU-1 for example).  Please coordinate all of the icons called out on the Sign Message Schedule with the information provided on the Color/Font/Material/Symbol/Icons Report.		<b>ANSWER:</b>  REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Color/Font/Material/Symbol/Icons Report  The Sign Message Schedule calls out for Icons not identified on the Color/Font/Material/Symbol/Icons Report (PS6-G-05-022 calls for Icons ASU-1, ACT-2 and ARU-1 for example).  Please coordinate all of the icons called out on the Sign Message Schedule with the information provided on the Color/Font/Material/Symbol/Icons Report.				
<b>P1-0603</b>	<b>Clipper Tagging Devices for Sign Type PS6 Interior Directional Pylon with Clipper `</b> Closed		<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/29/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				





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<b>P1-0605</b>	<b>Verbiage for Sign Type PS6 Interior Directional Pylon with Clipper Tagging</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Detail 3/SG1-6001 (IFC Drawings for Main Package dated 3/31/14)  Per the Sign Message Schedule, Sign PS6-G-06-006 Message D is ¿MUNI BUS PLAZA (FINAL NAME TBD BY TJPA)¿.  Detail 3/SG1-6001 does not provide material or layout of the verbiage (i.e. where on each face the verbiage is to appear, is it painted on, font size, word separation, etc.).  Please provide the layout for verbiage required to be on Sign Type PS6 Interior Directional Pylon with Clipper Tagging signage. In addition, please provide the ¿TBD¿ verbiage, or a basis of bid for additional lettering.		REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Detail 3/SG1-6001 (IFC Drawings for Main Package dated 3/31/14)  Per the Sign Message Schedule, Sign PS6-G-06-006 Message D is ¿MUNI BUS PLAZA (FINAL NAME TBD BY TJPA)¿.  Detail 3/SG1-6001 does not provide material or layout of the verbiage (i.e. where on each face the verbiage is to appear, is it painted on, font size, word separation, etc.).  Please provide the layout for verbiage required to be on Sign Type PS6 Interior Directional Pylon with Clipper Tagging signage. In addition, please provide the ¿TBD¿ verbiage, or a basis of bid for additional lettering.				
<b>P1-0606</b>	<b>Required Font and Size for Sign Type PD1 Bus Deck Dock ID Signage</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>11/03/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail 1/SG1-6003 (IFC Drawings for Main Package dated 3/31/14)  Detail 1/SG1-6003 does not identify the font or size for the graphics on Sign Type PD1 Bus Deck Dock ID (3 fonts and associated sizes are shown on the Color/Font/Material/Symbol/Icons Report).  Please provide the desired font and size for the verbiage on Sign Type PD1.		REFERENCE: Detail 1/SG1-6003 (IFC Drawings for Main Package dated 3/31/14)  Detail 1/SG1-6003 does not identify the font or size for the graphics on Sign Type PD1 Bus Deck Dock ID (3 fonts and associated sizes are shown on the Color/Font/Material/Symbol/Icons Report).  Please provide the desired font and size for the verbiage on Sign Type PD1.				
<b>P1-0607</b>	<b>LCD Requirement for Sign Type PD1 Bus Deck Dock ID</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (IFC Drawings for Main Package dated		REFERENCE: (IFC Drawings for Main Package dated				



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3/31/14) Detail 1/SG1-6003 Detail 3/SG1-6003  Per Detail 1/SG1-6003, a 22" LCD is required on both sides of Sign Type PD1 Bus Deck Dock ID.  Per Key Note 10 on Detail 3/SG1-6003 the 22" LCD is to be viewed by drivers.  Please confirm a 22" LCD is required on both sides of Sign Type PD1 per Detail 1/SG1-6003.	3/31/14) Detail 1/SG1-6003 Detail 3/SG1-6003  Per Detail 1/SG1-6003, a 22" LCD is required on both sides of Sign Type PD1 Bus Deck Dock ID.  Per Key Note 10 on Detail 3/SG1-6003 the 22" LCD is to be viewed by drivers.  Please confirm a 22" LCD is required on both sides of Sign Type PD1 per Detail 1/SG1-6003.					
<b>P1-0608</b>	<b>LCD Specification for Sign Type PD1 Bus Deck Dock ID</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Detail 1/SG1-6003 (IFC Drawings for Main Package dated 3/31/14) Specification Section 10 14 26 (ASI 127 dated 9/12/14)  Per Detail 1/SG1-6003, a 22" LCD is required on both sides of Sign Type PD1 Bus Deck Dock ID.  Specification Section 10 14 26 2.1 C Pylon Signage does not specify a 22" LCD for Sign Type PD1.  Please specify a 22" LCD for Sign Type PD1.						
<b>ANSWER:</b>  REFERENCE: Detail 1/SG1-6003 (IFC Drawings for Main Package dated 3/31/14) Specification Section 10 14 26 (ASI 127 dated 9/12/14)  Per Detail 1/SG1-6003, a 22" LCD is required on both sides of Sign Type PD1 Bus Deck Dock ID.  Specification Section 10 14 26 2.1 C Pylon Signage does not specify a 22" LCD for Sign Type PD1.  Please specify a 22" LCD for Sign Type PD1.						



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<b>P1-0609</b>	<b>Verbiage and Layout for Sign Type PS6 Interior Directional Pylon with Clipper Tag</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Detail 3/SG1-6001 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14) Detail 3/SG1-6001 (IFC Drawings for Main Package dated 3/31/14)				
Per the Sign Message Schedule, Sign PS6-G-06-006 Message D is ¿MUNI BUS PLAZA (FINAL NAME TBD BY TJPA)¿.		Per the Sign Message Schedule, Sign PS6-G-06-006 Message D is ¿MUNI BUS PLAZA (FINAL NAME TBD BY TJPA)¿.				
Detail 3/SG1-6001 does not provide material or layout of the verbiage (i.e. where on each face the verbiage is to appear, is it painted on, font size, word separation, etc.).		Detail 3/SG1-6001 does not provide material or layout of the verbiage (i.e. where on each face the verbiage is to appear, is it painted on, font size, word separation, etc.).				
Please provide the layout for verbiage required to be on Sign Type PS6 Interior Directional Pylon with Clipper Tagging. In addition, please provide the ¿TBD¿ verbiage, or a basis of bid for additional lettering.		Please provide the layout for verbiage required to be on Sign Type PS6 Interior Directional Pylon with Clipper Tagging. In addition, please provide the ¿TBD¿ verbiage, or a basis of bid for additional lettering.				
<b>P1-0610</b>	<b>Key Note 2A per Detail 4/SG1-6003</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/27/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 4/SG1-6003 Detail 3/SG1-6003		REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 4/SG1-6003 Detail 3/SG1-6003				
Detail 4/SG1-6003 calls out Key Note 2A.		Detail 4/SG1-6003 calls out Key Note 2A.				
There is no note 2A on Detail 3/SG1-6003.		There is no note 2A on Detail 3/SG1-6003.				
Please provide Note 2A.		Please provide Note 2A.				
<b>P1-0611</b>	<b>Commercial Grade LCD Types Required for Sign Types SM1, SS1, and SX1</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail 3/SG1-6010 (IFC Drawings for Main Package dated		REFERENCE: Detail 3/SG1-6010 (IFC Drawings for Main Package dated				





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	<p>3/31/14) Detail 2/SG1-6011 (IFC Drawings for Main Package dated 3/31/14) Specification Section 10 14 63 (ASI 127 dated 9/12/14)</p> <p>Per Key Note 1 on Detail 3/SG1-6010, the commercial grade LCD for Sign Types SM1 Wall-Mounted Schedule Board and SS1 Digital Display are to be selected based on performance requirements.</p> <p>Per Key Note 1 on Detail 2/SG1-6011, the commercial grade LCD for Sign Type SX1 Grand Hall Schedule Board is also to be selected based on performance requirements.</p> <p>Per Specification Section 10 14 63 2.2 B Electronic Panel Signage, there is a specific list of named LCD's specified for these sign types.</p> <p>Please confirm the LCD's for Sign Types SM1, SS1, and SX1 are to be selected based upon Specification Section 10 14 63, and not performance requirements.</p>					
	<p>dated 3/31/14) Detail 2/SG1-6011 (IFC Drawings for Main Package dated 3/31/14) Specification Section 10 14 63 (ASI 127 dated 9/12/14)</p> <p>Per Key Note 1 on Detail 3/SG1-6010, the commercial grade LCD for Sign Types SM1 Wall-Mounted Schedule Board and SS1 Digital Display are to be selected based on performance requirements.</p> <p>Per Key Note 1 on Detail 2/SG1-6011, the commercial grade LCD for Sign Type SX1 Grand Hall Schedule Board is also to be selected based on performance requirements.</p> <p>Per Specification Section 10 14 63 2.2 B Electronic Panel Signage, there is a specific list of named LCD's specified for these sign types.</p> <p>Please confirm the LCD's for Sign Types SM1, SS1, and SX1 are to be selected based upon Specification Section 10 14 63, and not performance requirements.</p>					
<b>P1-0612</b>	<b>Sole Source for Informational Kiosks per Specification Section 10 18 00</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/31/2014</b>
	<p><b>From:</b> Webcor Construction LP      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Specification Section 00 05 20 (dated 8/11/14)</p> <p>Per Specification Section 10 18 00 2.1 A 1 (10 18 00 2.2 C 1 c, etc.) Informational Kiosks, all of the informational kiosk base material to be made of Kastone by Kreysler and Associates.</p> <p>Per Specification Section 00 05 20 2.07 Agreement, sole source procurement is only acceptable with prior approval by TJPA.</p> <p>Please confirm it is acceptable to sole source all of the informational kiosk base material.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Specification Section 00 05 20 (dated 8/11/14)</p> <p>Per Specification Section 10 18 00 2.1 A 1 (10 18 00 2.2 C 1 c, etc.) Informational Kiosks, all of the informational kiosk base material to be made of Kastone by Kreysler and Associates.</p> <p>Per Specification Section 00 05 20 2.07 Agreement, sole source procurement is only acceptable with prior approval by TJPA.</p> <p>Please confirm it is acceptable to sole source all of the informational kiosk base material.</p>					



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<b>P1-0613</b>	<b>Verbiage for Sign Type KC1</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)		REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)				
Per the Sign Message Schedule, the messages for KC1-G-04-005 are TBD.		Per the Sign Message Schedule, the messages for KC1-G-04-005 are TBD.				
Please provide the ¿TBD¿ verbiage, or a basis of bid for additional lettering required to be on Sign Type KC1 Combined Touchscreen Directory and Digital Display signage.		Please provide the ¿TBD¿ verbiage, or a basis of bid for additional lettering required to be on Sign Type KC1 Combined Touchscreen Directory and Digital Display signage.				
<b>P1-0614</b>	<b>Braille Raster Bead Locations for Sign Type KC1 Combined Touchscreen &amp; Digital Closed</b>		<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>11/13/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCED: Detail 1/SG1-6020 (IFC Drawings for Main Package dated 3/31/14)		REFERENCED: Detail 1/SG1-6020 (IFC Drawings for Main Package dated 3/31/14)				
Per Detail 1/SG1-6020 braille raster beads are to be installed on the perforated aluminum access panel for Sign Type KC1 Combined Touchscreen & Digital Display.		Per Detail 1/SG1-6020 braille raster beads are to be installed on the perforated aluminum access panel for Sign Type KC1 Combined Touchscreen & Digital Display.				
Please provide the message and exact dimensions for locating the braille raster beads.		Please provide the message and exact dimensions for locating the braille raster beads.				
<b>P1-0615</b>	<b>Footing Requirements per Specification Section 10 18 00 Informational Kiosks</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14)				
Per Specification Section 10 18 00 1.2 Informational Kiosks, the signage subcontractor is to provide informational kiosks capable of withstanding the effects of wind loads based on criteria defined on the structural drawings, seismic performance criteria defined in Specification Section 01 80 50 Seismic Design Criteria for Nonstructural Components, and thermal movements		Per Specification Section 10 18 00 1.2 Informational Kiosks, the signage subcontractor is to provide informational kiosks capable of withstanding the effects of wind loads based on criteria defined on the structural drawings, seismic performance criteria defined in Specification Section 01 80 50 Seismic				



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	<p>defined in Specification Section 10 18 00 Informational Kiosks. There is no requirement to design the substrate upon which the informational kiosks are located.</p> <p>In several locations (Signs KT2-G-05-56 through KT2-G-05-61 and KM1-G-05-055 in the Grand Hall for example), informational kiosks sit on top of the topping slab with no footing down to structure.</p> <p>Please confirm that as long as the design requirements called out in Specification Section 10 18 00 1.2 are achieved, it is acceptable for the informational kiosks to be attached only to the topping slab with no footing through to structure.</p>					
	<p>Design Criteria for Nonstructural Components, and thermal movements defined in Specification Section 10 18 00 Informational Kiosks. There is no requirement to design the substrate upon which the informational kiosks are located.</p> <p>In several locations (Signs KT2-G-05-56 through KT2-G-05-61 and KM1-G-05-055 in the Grand Hall for example), informational kiosks sit on top of the topping slab with no footing down to structure.</p> <p>Please confirm that as long as the design requirements called out in Specification Section 10 18 00 1.2 are achieved, it is acceptable for the informational kiosks to be attached only to the topping slab with no footing through to structure.</p>					
<b>P1-0616</b>	<b>Braille Raster Bead Locations for Sign Type KT1 Ticket Vending Kiosk</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/30/2014</b>
	<p><b>From:</b> Webcor Construction LP      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Detail 1/SG1-6021 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Per Detail 1/SG1-6021 braille raster beads are to be installed on the perforated aluminum access panel for Sign Type KT1 Ticket Vending Kiosk.</p> <p>Please provide the message and exact dimensions for locating the braille raster beads.</p>					
	<p><b>ANSWER:</b></p> <p>REFERENCE: Detail 1/SG1-6021 (IFC Drawings for Main Package dated 3/31/14)</p> <p>Per Detail 1/SG1-6021 braille raster beads are to be installed on the perforated aluminum access panel for Sign Type KT1 Ticket Vending Kiosk.</p> <p>Please provide the message and exact dimensions for locating the braille raster beads.</p>					



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<b>P1-0617</b>	<b>Braille Raster Bead Locations for Sign Type KT2 Ticket Vending Kiosk with Digital</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail 4/SG1-6021 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Detail 4/SG1-6021 (IFC Drawings for Main Package dated 3/31/14)				
Per Detail 4/SG1-6021 braille raster beads are to be installed on the perforated aluminum access panel for Sign Type KT2 Ticket Vending Kiosk with Digital Display.		Per Detail 4/SG1-6021 braille raster beads are to be installed on the perforated aluminum access panel for Sign Type KT2 Ticket Vending Kiosk with Digital Display.				
Please provide the message and exact dimensions for locating the braille raster beads.		Please provide the message and exact dimensions for locating the braille raster beads.				
<b>P1-0618</b>	<b>Graphic Requirements for Sign Type KT2 Ticket Vending Kiosk with Digital Display</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 18 00 Sign Message Schedule		REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 18 00 Sign Message Schedule				
Per Specification Section 10 18 00 2.2 F 2 i Informational Kiosks, Sign Type KT2 Ticket Vending Kiosk with Digital Display has 2 faces with graphics.		Per Specification Section 10 18 00 2.2 F 2 i Informational Kiosks, Sign Type KT2 Ticket Vending Kiosk with Digital Display has 2 faces with graphics.				
Per the Sign Message Schedule, all of the KT2 Signs only have 1 graphic.		Per the Sign Message Schedule, all of the KT2 Signs only have 1 graphic.				
Please confirm KT2 Signs are only to receive 1 graphic.		Please confirm KT2 Signs are only to receive 1 graphic.				
<b>P1-0619</b>	<b>Nominal Depth Requirement for Sign Type KT2</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 4/SG1-6021 Detail 6/SG1-6021		REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 4/SG1-6021 Detail 6/SG1-6021				
Per Detail 4/SG1-6021, the nominal depth of Sign Type KT2 Ticket Vending Kiosk with Digital Display is 1½-5½.		Per Detail 4/SG1-6021, the nominal depth of Sign Type KT2 Ticket Vending Kiosk with Digital Display is				



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	<p>Per Detail 6/SG1-6021, the nominal depth of Sign Type KT2 is 1 1/2"-0 1/2".</p> <p>Please clarify the nominal depth of Sign Type KT2.</p>					<p>1 1/2"-5 1/2".</p> <p>Per Detail 6/SG1-6021, the nominal depth of Sign Type KT2 is 1 1/2"-0 1/2".</p> <p>Please clarify the nominal depth of Sign Type KT2.</p>
<b>P1-0620</b>	<b>Key Note 12 per Detail 4/SG1-6021</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/27/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 4/SG1-6021 Detail 3/SG1-6021						REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 4/SG1-6021 Detail 3/SG1-6021
Detail 4/SG1-6021 calls out Key Note 12.						Detail 4/SG1-6021 calls out Key Note 12.
Detail 3/SG1-6021 does not define Key Note 12.						Detail 3/SG1-6021 does not define Key Note 12.
Please define Key Note 12.						Please define Key Note 12.
<b>P1-0621</b>	<b>Surface Applied Digital Print per Detail 3/SG1-6021</b>	<b>Closed</b>	<b>0P</b>	<b>10/21/2014</b>	<b>10/31/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Detail 3/SG1-6021 (IFC Drawings for Main Package dated 3/31/14)						REFERENCE: Detail 3/SG1-6021 (IFC Drawings for Main Package dated 3/31/14)
Key Note 9 on Detail 3/SG1-6021 indicate that the static graphic panels called out are to receive a surface applied digital print.						Key Note 9 on Detail 3/SG1-6021 indicate that the static graphic panels called out are to receive a surface applied digital print.
No information has been provided on the surface applied digital print.						No information has been provided on the surface applied digital print.
Please confirm the referenced surface applied digital print is not included in the contract, or provide information on the surface applied digital print.						Please confirm the referenced surface applied digital print is not included in the contract, or provide information on the surface applied digital print.



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<b>P1-0622</b>	<b>Message Requirements for Sign Type KP1 One-Sided Digital Display Panel</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Detail 3/SG1-6022 (IFC Drawings for Main Package dated 3/31/14) Sign Message Schedule (IFC Drawings for Main Package dated 3/31/14)  Per Specification Section 10 18 00 2.2 E 1 i, Sign Type KP1 One-Sided Digital Display Panel has 2 faces with graphics.  Per Detail 3/SG1-6022, Sign Type KP1 has 1 icon.  Per the Sign Message Schedule, all of the Sign Type KP1 signs have ¿(NO MESSAGE)¿.  Please clarify the messages to be installed with Sign Type KP1.		REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Detail 3/SG1-6022 (IFC Drawings for Main Package dated 3/31/14) Sign Message Schedule (IFC Drawings for Main Package dated 3/31/14) Per Specification Section 10 18 00 2.2 E 1 i, Sign Type KP1 One-Sided Digital Display Panel has 2 faces with graphics.  Per Detail 3/SG1-6022, Sign Type KP1 has 1 icon.  Per the Sign Message Schedule, all of the Sign Type KP1 signs have ¿(NO MESSAGE)¿.  Please clarify the messages to be installed with Sign Type KP1.				
<b>P1-0623</b>	<b>Message Requirements for Sign Type KP3 Two-Sided Digital Display Panel with Sp</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14)  Per Specification Section 10 18 00 2.2 E 2 i, Sign Type KP3 Two-Sided Digital Display Panel with Speakers has 2 faces with graphics.  Per the Sign Message Schedule, all of the KP3 Signs have ¿(NO MESSAGE)¿.  Please clarify the messages to be installed with Sign Type KP3.		REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14)  Per Specification Section 10 18 00 2.2 E 2 i, Sign Type KP3 Two-Sided Digital Display Panel with Speakers has 2 faces with graphics.  Per the Sign Message Schedule, all of the KP3 Signs have ¿(NO MESSAGE)¿.  Please clarify the messages to be installed with Sign Type KP3.				



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P1-0624	Key Note 5D per Detail 4/SG1-6022	Closed	0P	10/23/2014	11/02/2014	10/31/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 4/SG1-6022 Detail 2/SG1-6022		REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 4/SG1-6022 Detail 2/SG1-6022				
Detail 4/SG1-6022 calls out Key Note 5D.		Detail 4/SG1-6022 calls out Key Note 5D.				
Detail 2/SG1-6022 does not identify Key Note 5D.		Detail 2/SG1-6022 does not identify Key Note 5D.				
Please provide information on Key Note 5D.		Please provide information on Key Note 5D.				
P1-0625	Drawing Titles for Detail 3/SG1-6022	Closed	0P	10/23/2014	11/02/2014	10/31/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Detail 3/SG1-6002 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Detail 3/SG1-6002 (IFC Drawings for Main Package dated 3/31/14)				
Detail 3/SG1-6022 title is ¿KP1: One-sided Digital Display Panel Kiosk ¿ With Speakers¿.		Detail 3/SG1-6022 title is ¿KP1: One-sided Digital Display Panel Kiosk ¿ With Speakers¿.				
Titles within the detail call out KP2.		Titles within the detail call out KP2.				
Please coordinate the drawing titles.		Please coordinate the drawing titles.				
P1-0626	Speaker Requirements for Sign Type KP1 One-Sided Display Panel	Closed	0P	10/23/2014	11/02/2014	10/31/2014
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
REFERENCE: Detail 3/SG1-6022 (IFC Drawings for Main Package dated 3/31/14) Specification Section 10 18 00 (ASI 127 dated 9/12/14)		REFERENCE: Detail 3/SG1-6022 (IFC Drawings for Main Package dated 3/31/14) Specification Section 10 18 00 (ASI 127 dated 9/12/14)				
Detail 3/SG1-6022 title is ¿KP1: One-sided Digital Display Panel Kiosk ¿ With Speakers¿.		Detail 3/SG1-6022 title is ¿KP1: One-sided Digital Display Panel Kiosk ¿ With Speakers¿.				
Specification Section 10 18 00 2.2 E 1 does not call out speakers, and drawing details within Detail 3/SG1-6022 does not call out speakers.		Specification Section 10 18 00 2.2 E 1 does not call out speakers, and drawing details within Detail 3/SG1-				



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P1-0627	<b>Sole Sourcing for Sign Type KP3 Two-Sided Digital Display Panel with Speakers</b>  From: Webcor Construction LP      Tram Nguyen  <b>REQUEST:</b>  REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Specification Section 00 05 20 (dated 8/11/14)  Per Specification Section 10 18 00 2.2 E 2 o Informational Kiosks, speakers within the Sign Type KP3 Two-Sided Digital Display Panel with Speakers are to be Intellovox Duran Audio Model DC115.  Per 00 05 20 2.07 Agreement, sole source procurement is only acceptable with prior approval by TJPA.  Please confirm it is acceptable to sole source all of the speakers for Sign Type KP3.	Closed	0P	10/23/2014	11/02/2014	10/28/2014
			<b>ANSWER:</b>  REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Specification Section 00 05 20 (dated 8/11/14)  Per Specification Section 10 18 00 2.2 E 2 o Informational Kiosks, speakers within the Sign Type KP3 Two-Sided Digital Display Panel with Speakers are to be Intellovox Duran Audio Model DC115.  Per 00 05 20 2.07 Agreement, sole source procurement is only acceptable with prior approval by TJPA.  Please confirm it is acceptable to sole source all of the speakers for Sign Type KP3.			





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P1-0628	Messages for Sign Type KM1 Static Map and Digital Display Panel	Closed	0P	10/23/2014	11/02/2014	10/31/2014
From: Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Detail 1/SG1-6023 (IFC Drawings for Main Package dated 3/31/14)  Per Specification Section 10 18 00 2.2 D 1 i Informational Kiosks, Sign Type KM1 Static Map and Digital Display Panel has 2 faces with graphics.  Per Detail 1/SG1-6023 and the Sign Message Schedule, all of the KM1 Signs have ¿(NO MESSAGE)¿.  Please clarify the messages to be installed with Sign Type KM1.						REFERENCE: Specification Section 10 18 00 (ASI 127 dated 9/12/14) Detail 1/SG1-6023 (IFC Drawings for Main Package dated 3/31/14)  Per Specification Section 10 18 00 2.2 D 1 i Informational Kiosks, Sign Type KM1 Static Map and Digital Display Panel has 2 faces with graphics.  Per Detail 1/SG1-6023 and the Sign Message Schedule, all of the KM1 Signs have ¿(NO MESSAGE)¿.  Please clarify the messages to be installed with Sign Type KM1.
P1-0629	Mounting Requirements for Sign Types BS3 and BS4	Closed	0P	10/23/2014	11/02/2014	10/30/2014
From: Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 66 Specification Section 09 51 23  Specification Section 10 14 66 2.2 C 3 d Floating Signage, indicates that Sign Type BS4 6-Icon Overhead Directional is to be ceiling mounted.  Specification Section 10 14 66 2.2 C 2 d Floating Signage, indicates that Sign Type BS3 4-Icon Overhead Directional is to be ceiling mounted.  Specification Section 09 51 22 Aluminum Ceilings does not address the mounting of Sign Types BS3 or BS4.  Please confirm Sign Types BS3 and BS4 signs are to be directly supported from the structural deck, or coordinate Specification Section 10 14 66 with Specification Section 09 51 22.						REFERENCE: (ASI 127 dated 9/12/14) Specification Section 10 14 66 Specification Section 09 51 23  Specification Section 10 14 66 2.2 C 3 d Floating Signage, indicates that Sign Type BS4 6-Icon Overhead Directional is to be ceiling mounted.  Specification Section 10 14 66 2.2 C 2 d Floating Signage, indicates that Sign Type BS3 4-Icon Overhead Directional is to be ceiling mounted.  Specification Section 09 51 22 Aluminum Ceilings does not address the mounting of Sign Types BS3 or BS4.  Please confirm Sign Types BS3 and BS4 signs are to be directly supported from the structural deck, or coordinate Specification Section 10 14 66 with Specification Section 09 51 22.



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<b>P1-0630</b>	<b>Verbiage for Sign Type BG1 Applied Message Band on Glass</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)		REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)				
Sign BG1-G-10-010 on the Sign Message Schedule indicates that Message A will be determined by the owner.		Sign BG1-G-10-010 on the Sign Message Schedule indicates that Message A will be determined by the owner.				
Please provide the verbiage for Sign Type BG1 Applied Message Band on Glass, or provide a basis of bid.		Please provide the verbiage for Sign Type BG1 Applied Message Band on Glass, or provide a basis of bid.				
<b>P1-0631</b>	<b>Message Arrangement for Sign Type BS4</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)		REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)				
The Sign Message Schedule calls out that Message D for Signs BS4-B-06-003, BS4-B-04-002, and BS4-B-03-001 is to be ¿MESSAGING LEFT TO RIGHT¿.		The Sign Message Schedule calls out that Message D for Signs BS4-B-06-003, BS4-B-04-002, and BS4-B-03-001 is to be ¿MESSAGING LEFT TO RIGHT¿.				
Please confirm this is direction for arrangement of the other messaging on the sign and move it to the notes column, or provide layout for the message.		Please confirm this is direction for arrangement of the other messaging on the sign and move it to the notes column, or provide layout for the message.				
<b>P1-0632</b>	<b>Content for Sign Type IG1 Small Interpretative Panel</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail 1/SG1-6080 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Detail 1/SG1-6080 (IFC Drawings for Main Package dated 3/31/14)				
1/SG1-6080 states that signage bidders are to ¿see message schedule for content¿ for Sign Type IG1 Small Interpretative Panel.		1/SG1-6080 states that signage bidders are to ¿see message schedule for content¿ for Sign Type IG1 Small Interpretative Panel.				
The Sign Message Schedule does not provide any information on Sign Type IG1.		The Sign Message Schedule does not provide any information on Sign Type IG1.				



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	Please provide the artwork for Sign Type IG1.				Please provide the artwork for Sign Type IG1.	
<b>P1-0633</b>	<b>Final Artwork for Sign Type IG1 Small Interpretative Panel</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/28/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Detail 3/SG1-6080 (IFC Drawings for Main Package dated 3/31/14)  Key Note 1 on Detail 3/SG1-6080 indicates that the ¿final artworks to be provided¿ for Sign Type IG1 Small Interpretative Panel. No artwork has been provided.  Please provide the artwork for Sign Type IG1, or provide a basis of bid.					<b>ANSWER:</b>  REFERENCE: Detail 3/SG1-6080 (IFC Drawings for Main Package dated 3/31/14)  Key Note 1 on Detail 3/SG1-6080 indicates that the ¿final artworks to be provided¿ for Sign Type IG1 Small Interpretative Panel. No artwork has been provided.  Please provide the artwork for Sign Type IG1, or provide a basis of bid.	
<b>P1-0634</b>	<b>Vinyl Graphic for Sign Type FD1 per Detail 1/SG1-6190</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  REFERENCE: Specification Section 10 14 36 (ASI 127 dated 9/12/14) Detail 1/SG1-6190 (IFC Drawings for Main Package dated 3/31/14)  Per Specification Section 10 14 36 2.2 J 10 a Non-Illuminated Panel Signage, Sign Type FD1 Floor Directory at Elevator is to be a surface screen copy on existing elevator lobby wall surface.  Per Detail 1/SG1-6190, Sign Type FD1 is to be a vinyl graphic.  Please confirm Sign Type FD1 is to be a vinyl graphic.					<b>ANSWER:</b>  REFERENCE: Specification Section 10 14 36 (ASI 127 dated 9/12/14) Detail 1/SG1-6190 (IFC Drawings for Main Package dated 3/31/14)  Per Specification Section 10 14 36 2.2 J 10 a Non-Illuminated Panel Signage, Sign Type FD1 Floor Directory at Elevator is to be a surface screen copy on existing elevator lobby wall surface.  Per Detail 1/SG1-6190, Sign Type FD1 is to be a vinyl graphic.  Please confirm Sign Type FD1 is to be a vinyl graphic.	



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<b>P1-0635</b>	<b>Finish Color for Sign Type FD1 Floor Directory at Elevator</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 1/SG1-6190 Detail 3/SG1-6190		REFERENCE: (IFC Drawings for Main Package dated 3/31/14) Detail 1/SG1-6190 Detail 3/SG1-6190				
Per Detail 1/SG1-6190, the finish color for Sign Type FD1 Floor Directory at Elevator is to be P9.		Per Detail 1/SG1-6190, the finish color for Sign Type FD1 Floor Directory at Elevator is to be P9.				
Per Key Note 2 on Detail 3/SG1-6190, the finish color for Sign Type FD1 is to be P1.		Per Key Note 2 on Detail 3/SG1-6190, the finish color for Sign Type FD1 is to be P1.				
Please clarify the color to be used for Sign Type FD1.		Please clarify the color to be used for Sign Type FD1.				
<b>P1-0636</b>	<b>Installation Height for Sign Type FD1 Floor Directory at Elevator</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail 4/SG1-6190 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Detail 4/SG1-6190 (IFC Drawings for Main Package dated 3/31/14)				
Detail 4/SG1-6190 does not show the installation height of Sign Type FD1 Floor Directory at Elevator.		Detail 4/SG1-6190 does not show the installation height of Sign Type FD1 Floor Directory at Elevator.				
Please provide the installation height of Sign Type FD1.		Please provide the installation height of Sign Type FD1.				
<b>P1-0637</b>	<b>Verbiage for Sign Type SD3 Intercom Box Instructional Plaque</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>11/03/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)		REFERENCE: Sign Message Schedule (ASI 127 dated 9/12/14)				
Per the Sign Message Schedule, Sign Type SD3 Intercom Box Instructional Plaque is to be determined by the owner.		Per the Sign Message Schedule, Sign Type SD3 Intercom Box Instructional Plaque is to be determined by the owner.				
Please provide the TBD verbiage for Sign Type SD3, or provide a basis of bid.		Please provide the TBD verbiage for Sign Type SD3, or provide a basis of bid.				



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<b>P1-0638</b>	<b>Usage of Sign Type ISA1 International Symbol of Accessibility</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail 4/SG1-6201 (IFC Drawings for Main Package dated 3/31/14) Specification Section 10 14 36 (ASI 127 dated 9/12/14)  Sign Type ISA1 International Symbol of Accessibility is detailed on Detail 4/SG1-6201 and specified in Specification Section 10 14 36 Non-Illuminated Panel Signage, but not called out on the signage plans or Sign Message Schedule.  Please confirm Sign Type ISA1 is not required for the project.		REFERENCE: Detail 4/SG1-6201 (IFC Drawings for Main Package dated 3/31/14) Specification Section 10 14 36 (ASI 127 dated 9/12/14)  Sign Type ISA1 International Symbol of Accessibility is detailed on Detail 4/SG1-6201 and specified in Specification Section 10 14 36 Non-Illuminated Panel Signage, but not called out on the signage plans or Sign Message Schedule.  Please confirm Sign Type ISA1 is not required for the project.				
<b>P1-0639</b>	<b>Elevator Shaft Painting Requirements</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/29/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCES: Specification Section 09 91 00 (ASI 127 dated 9/12/14)  Per the responses to RFI's P1-0099 through P1-0099.2, the interior of elevator shafts are to be painted.  Per Specification Section 09 91 00 1.1 D 12 Paint of ASI 127, elevator shafts are not to be painted.  Please confirm elevator shafts are not to be painted, or revise the specification show the interior of elevator shafts to be painted.		REFERENCES: Specification Section 09 91 00 (ASI 127 dated 9/12/14)  Per the responses to RFI's P1-0099 through P1-0099.2, the interior of elevator shafts are to be painted.  Per Specification Section 09 91 00 1.1 D 12 Paint of ASI 127, elevator shafts are not to be painted.  Please confirm elevator shafts are not to be painted, or revise the specification show the interior of elevator shafts to be painted.				
<b>P1-0640</b>	<b>Cement Plaster Mockup Painting Requirement</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 09 91 00 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 09 91 00 (ASI 127 dated 9/12/14)				



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	<p>Specification Section 09 91 00 1.7 C 1 Paint, indicates that painters are to refer to Specification Section 09 24 00 Portland Cement Plastering for painting of the cement plaster mockup.</p> <p>Specification Section 09 24 00 Portland Cement Plastering does not call out for the cement plaster mockup to be painted, and RFI P1-0428 indicates that the cement plaster is not to be painted.</p> <p>Please remove reference to painting the cement plaster mockup in 09 91 00.</p>					
<b>P1-0641</b>	<b>Incorrect Specification Reference for Painting Gypsum Board Mockup</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 09 91 00 Specification Section 09 21 16						
Specification Section 09 91 00 1.7 C Paint states, "Refer to Section 09 24 00 for painting cement plaster mockup and Section 09 29 00 for painting gypsum board mockup." The correct Specification Section for Gypsum Board is 09 21 16.						
Please revise the specification to correctly reference Specification Section 09 21 16 Gypsum Board.						
<b>ANSWER:</b>						
REFERENCE: (ASI 127 dated 9/12/14) Specification Section 09 91 00 Specification Section 09 21 16						
Specification Section 09 91 00 1.7 C Paint states, "Refer to Section 09 24 00 for painting cement plaster mockup and Section 09 29 00 for painting gypsum board mockup." The correct Specification Section for Gypsum Board is 09 21 16.						
Please revise the specification to correctly reference Specification Section 09 21 16 Gypsum Board.						



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<b>P1-0642</b>	<b>Impending Rain for Paint Application</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 09 91 00 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 09 91 00 (ASI 127 dated 9/12/14)				
Specification Section 09 91 00 1.8 A 4 c Paint indicates that paint is not to be applied when there is a threat of impending rain.		Specification Section 09 91 00 1.8 A 4 c Paint indicates that paint is not to be applied when there is a threat of impending rain.				
This requirement precludes installation of paint in areas that cannot be impacted by rain (i.e. interior spaces, covered spaces, etc.).		This requirement precludes installation of paint in areas that cannot be impacted by rain (i.e. interior spaces, covered spaces, etc.).				
Please revise to indicate environmental conditions are to be per the manufacturer's written installation instructions.		Please revise to indicate environmental conditions are to be per the manufacturer's written installation instructions.				
<b>P1-0643</b>	<b>High Performance Coatings Requirements for Bus Crash Rail</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 09 97 15 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 09 97 15 (ASI 127 dated 9/12/14)				
Specification Section 09 97 15 1.1 A 4 High Performance Coatings identifies the bus crash rail as an exposed steel surface to be painted with High Performance Coatings.		Specification Section 09 97 15 1.1 A 4 High Performance Coatings identifies the bus crash rail as an exposed steel surface to be painted with High Performance Coatings.				
Currently the bus crash rail is to be structural concrete.		Currently the bus crash rail is to be structural concrete.				
Please revise specification omitting the bus crash rail.		Please revise specification omitting the bus crash rail.				
<b>P1-0644</b>	<b>High Performance Coating Requirements for Steel Surfaces</b>	<b>Closed</b>	<b>0P</b>	<b>10/23/2014</b>	<b>11/02/2014</b>	<b>10/29/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 09 97 15 (ASI 127 dated 9/12/14)		REFERENCE: Specification Section 09 97 15 (ASI 127 dated 9/12/14)				



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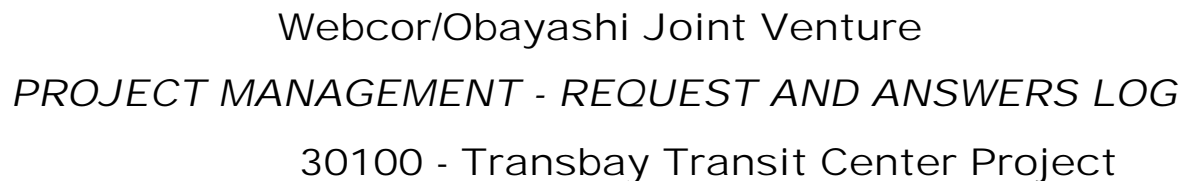
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	<p>Specification Section 09 97 15 1.3 B High Performance Coatings makes the subcontractor responsible to ¿approve the preparation of the substrates to be painted and application of the primer, which are both to be performed in the steel fabricator¿s plant."</p> <p>Based upon the current construction schedule, steel is being primed and shipped to the project prior to Trade Package TG016.5 Paint being bid.</p> <p>Please revise language to indicate the paint subcontractor shall review the surfaces to be painted and report deficient installation of material to general contractor prior to commencing with the work.</p>					<p>9/12/14)</p> <p>Specification Section 09 97 15 1.3 B High Performance Coatings makes the subcontractor responsible to ¿approve the preparation of the substrates to be painted and application of the primer, which are both to be performed in the steel fabricator¿s plant."</p> <p>Based upon the current construction schedule, steel is being primed and shipped to the project prior to Trade Package TG016.5 Paint being bid.</p> <p>Please revise language to indicate the paint subcontractor shall review the surfaces to be painted and report deficient installation of material to general contractor prior to commencing with the work.</p>
P1-0645	<p><b>Application Conditions for High Performance Coatings</b></p> <p>From: Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE:</p> <p>Specification Section 09 97 15 (ASI 127 dated 9/12/14)</p> <p>Specification Section 09 97 15 1.10 A High Performance Coatings requires that High Performance Coatings are applied only when temperature of surfaces to be coated and surrounding air temperatures are between 45 and 95 degrees F.</p> <p>Specification Section 09 97 15 1.10 B requires that High Performance Coatings are not applied when relative humidity exceeds 85%.</p> <p>These conditions do not match the manufacturer¿s written installation requirements for the specified products (ex. Carboguard 890 VOC, the minimum application surface and ambient temperature is 50 degrees F, and maximum allowable humidity is 80%).</p> <p>Please revise the site condition requirements to meet the</p>	Closed	0P	10/24/2014	11/03/2014	10/27/2014
						<p><b>ANSWER:</b></p> <p>REFERENCE:</p> <p>Specification Section 09 97 15 (ASI 127 dated 9/12/14)</p> <p>Specification Section 09 97 15 1.10 A High Performance Coatings requires that High Performance Coatings are applied only when temperature of surfaces to be coated and surrounding air temperatures are between 45 and 95 degrees F.</p> <p>Specification Section 09 97 15 1.10 B requires that High Performance Coatings are not applied when relative humidity exceeds 85%.</p> <p>These conditions do not match the manufacturer¿s written installation requirements for the specified products (ex. Carboguard 890 VOC, the minimum application surface and ambient temperature is 50 degrees F, and maximum allowable humidity is 80%).</p>





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	<p>manufacturer's written installation instructions.</p>					
P1-0646	<p><b>Paint Samples for High Performance Coatings</b></p> <p><b>From:</b> Webcor Construction LP      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>REFERENCE: Specification Section 09 97 15 (ASI 127 dated 9/12/14)</p> <p>Specification Section 09 97 15 2.4 A High Performance Coatings indicates that paint colors are to match the TJPA representative's control samples.</p> <p>Please provide the referenced color samples.</p>	Closed	0P	10/24/2014	11/03/2014	11/06/2014
	<p><b>ANSWER:</b></p> <p>REFERENCE: Specification Section 09 97 15 (ASI 127 dated 9/12/14)</p> <p>Specification Section 09 97 15 2.4 A High Performance Coatings indicates that paint colors are to match the TJPA representative's control samples.</p> <p>Please provide the referenced color samples.</p>					



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<b>P1-0647</b>	<b>Wall Finish Callouts per Room Finish Schedule</b>	<b>Closed</b>	<b>0P</b>	<b>10/24/2014</b>	<b>11/03/2014</b>	<b>10/29/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 09 91 00 (ASI 127 dated 9/12/14) Sheet A1-9601 (ASI 127 dated 9/12/14) Sheet A1-9602 (ASI 119 dated 6/20/14) Sheet A1-9603 (ASI 119 dated 6/20/14) Sheet A1-9606 (ASI 122 dated 7/23/14)  Specification Section 09 91 00 1.1 D 2 Painting, excludes the painting of concrete and CMU surfaces.  Specification Section 09 91 00 3.7 Painting, specifies paint products for concrete and CMU surfaces.  The Room Finish Schedules on Sheets A1-9601 through A1-9603 and Sheet A1-9606 call for paint finishes on concrete and CMU walls.  Please confirm that painting of concrete and CMU surfaces are required at locations called out in the Room Finish Schedules and revise Specification Section 09 91 00 to match.		REFERENCE: Specification Section 09 91 00 (ASI 127 dated 9/12/14) Sheet A1-9601 (ASI 127 dated 9/12/14) Sheet A1-9602 (ASI 119 dated 6/20/14) Sheet A1-9603 (ASI 119 dated 6/20/14) Sheet A1-9606 (ASI 122 dated 7/23/14)  Specification Section 09 91 00 1.1 D 2 Painting, excludes the painting of concrete and CMU surfaces.  Specification Section 09 91 00 3.7 Painting, specifies paint products for concrete and CMU surfaces.  The Room Finish Schedules on Sheets A1-9601 through A1-9603 and Sheet A1-9606 call for paint finishes on concrete and CMU walls.  Please confirm that painting of concrete and CMU surfaces are required at locations called out in the Room Finish Schedules and revise Specification Section 09 91 00 to match.				
<b>P1-0648</b>	<b>Information for LAM-4 Formica Color</b>	<b>Closed</b>	<b>0P</b>	<b>10/28/2014</b>	<b>11/07/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sheet A1-9610 (ASI 122 dated 7/23/14) Specification Section 06 40 00 (ASI 127 dated 9/12/14)  Sheet A1-9610 indicates LAM-4 is "XXXXXXXX by Formica For Cabinet Base" and reference Specification Section 06 40 00 Interior Architectural Woodwork.  Specification Section 06 40 00 does not provide a laminate color for LAM-4. Please provide the laminate color for LAM-4.		REFERENCE: Sheet A1-9610 (ASI 122 dated 7/23/14) Specification Section 06 40 00 (ASI 127 dated 9/12/14)  Sheet A1-9610 indicates LAM-4 is "XXXXXXXX by Formica For Cabinet Base" and reference Specification Section 06 40 00 Interior Architectural Woodwork.  Specification Section 06 40 00 does not provide a laminate color for LAM-4. Please provide the laminate color for LAM-4.				
<b>P1-0649</b>	<b>Millwork for Security Desk and Info Desk in the Grand Hall</b>	<b>Closed</b>	<b>0P</b>	<b>10/28/2014</b>	<b>11/07/2014</b>	<b>11/06/2014</b>







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<b>P1-0658</b>	<b>MRC5 Option 2 Language</b>	<b>Closed</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference 32 91 00, 2.1.A (IFC Main Set)  32 91 00 2.1.A does not contain any MRc5 Option 2 language. Please add MRc5 Option 2 language to 32 91 00.		<b>ANSWER:</b>  Reference 32 91 00, 2.1.A (IFC Main Set)  32 91 00 2.1.A does not contain any MRc5 Option 2 language. Please add MRc5 Option 2 language to 32 91 00.				
<b>P1-0659</b>	<b>Stability Test Lab</b>	<b>Closed</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference 32 91 00, 2.2.D.b. (IFC Main Set)  There is only one laboratory listed to perform stability tests. Is this the only qualified laboratory in the country? If not, allow for an "or equal" laboratory to perform stability tests.		<b>ANSWER:</b>  Reference 32 91 00, 2.2.D.b. (IFC Main Set)  There is only one laboratory listed to perform stability tests. Is this the only qualified laboratory in the country? If not, allow for an "or equal" laboratory to perform stability tests.				
<b>P1-0660</b>	<b>Nutrient Amendment Program</b>	<b>Closed</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference 32 91 00, 2.3 (IFC Main Set)  Clarify why the Contractor is meant to submit a preliminary proposal of Nutrient Amendment Program for Maintenance of Soil at time of bid? Award will not be impacted by this information.		<b>ANSWER:</b>  Reference 32 91 00, 2.3 (IFC Main Set)  Clarify why the Contractor is meant to submit a preliminary proposal of Nutrient Amendment Program for Maintenance of Soil at time of bid? Award will not be impacted by this information.				
<b>P1-0661</b>	<b>Misspelled Word in 32 91 00 3.1.A</b>	<b>Closed</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference 32 91 00, 3.1.A (IFC Main Set)  Confirm that this is meant to read as "repairs to damaged utilities at Contractor's own expense" and not "repairs to		<b>ANSWER:</b>  Reference 32 91 00, 3.1.A (IFC Main Set)  Confirm that this is meant to read as "repairs to damaged utilities at Contractor's own expense" and				



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	damaged utilities at Contract's own expense."					not "repairs to damaged utilities at Contract's own expense."
<b>P1-0662</b>	<b>Work Adjacent to Planting Soils</b>	<b>Open</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference 32 91 00, 3.1.C (IFC Main Set)  32 91 00, 3.1.C states as follows, "Verify that all work requiring access through or adjacent to areas where Planting Soils are to be placed has been completed and no further access (other than exterior planting installation) will be required. In the event that access will be required, access must be approved by TJPA Representative or Construction Manager and will be subject to replacing soil areas disturbed." This requirement will greatly impact the schedule, especially with decisions still pending on the Rooftop Restaurant and Café. The planting areas are throughout the roof park and all adjacent areas are not likely to finish before any soils will be placed. Revise accordingly.						<b>ANSWER:</b> Reference 32 91 00, 3.1.C (IFC Main Set)  32 91 00, 3.1.C states as follows, "Verify that all work requiring access through or adjacent to areas where Planting Soils are to be placed has been completed and no further access (other than exterior planting installation) will be required. In the event that access will be required, access must be approved by TJPA Representative or Construction Manager and will be subject to replacing soil areas disturbed." This requirement will greatly impact the schedule, especially with decisions still pending on the Rooftop Restaurant and Café. The planting areas are throughout the roof park and all adjacent areas are not likely to finish before any soils will be placed. Revise accordingly.
<b>P1-0663</b>	<b>TJPA Representative During Placement of Planting Soils</b>	<b>Closed</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference 32 91 00, 3.2.A.4 (IFC Main Set)  Confirm that the Landscape Architect will act as the TJPA Representative staking all trees and shrubs during placement of the planting soil and that they will be readily available during the entire planting and soil placement process.						<b>ANSWER:</b> Reference 32 91 00, 3.2.A.4 (IFC Main Set)  Confirm that the Landscape Architect will act as the TJPA Representative staking all trees and shrubs during placement of the planting soil and that they will be readily available during the entire planting and soil placement process.



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<b>P1-0664</b>	<b>Incorrect Reference in 32 91 00</b>	<b>Closed</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>12/15/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> Reference 32 91 00, 3.3.D.1.b (IFC Main Set)  This refers to "Section 2.3.C" which does not exist. Revise accordingly.						<b>ANSWER:</b> Reference 32 91 00, 3.3.D.1.b (IFC Main Set)  This refers to "Section 2.3.C" which does not exist. Revise accordingly.
<b>P1-0665</b>	<b>SSc7.1 Language</b>	<b>Closed</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/03/2014</b>	<b>12/11/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> Reference 03 33 12, 1.4.A.4  03 33 12, 1.4.A.4 states that "LEED submittal language for SSc7.1 is being added, but no minimum requirement language is being included at this time because the roof is over 50% vegetated and the ground level paving is shaded by architectural devices." Provide a minimum requirement or remove the phrase "at this time" if no minimum is required.						<b>ANSWER:</b> Reference 03 33 12, 1.4.A.4  03 33 12, 1.4.A.4 states that "LEED submittal language for SSc7.1 is being added, but no minimum requirement language is being included at this time because the roof is over 50% vegetated and the ground level paving is shaded by architectural devices." Provide a minimum requirement or remove the phrase "at this time" if no minimum is required.
<b>P1-0666</b>	<b>SSc7.2</b>	<b>Closed</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b> Reference 03 33 12, 1.4.A.5 (IFC Main Set)  03 33 12, 1.4.A.5 states that "LEED submittal language for SSc7.2 is being added, but no minimum requirement language is being included at this time because the roof is over 50% vegetated and the ground level paving is shaded by architectural devices." Provide a minimum requirement or remove the phrase "at this time" if no minimum is required.						<b>ANSWER:</b> Reference 03 33 12, 1.4.A.5 (IFC Main Set)  03 33 12, 1.4.A.5 states that "LEED submittal language for SSc7.2 is being added, but no minimum requirement language is being included at this time because the roof is over 50% vegetated and the ground level paving is shaded by architectural devices." Provide a minimum requirement or remove the phrase "at this time" if no minimum is required.
<b>P1-0667</b>	<b>Incomplete Sentence in 03 33 12</b>	<b>Closed</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						







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<b>P1-0670</b>	<b>Weather Requirements</b>	<b>Closed</b>	<b>0P</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference 03 33 12, 1.8.A (IFC Main Set)  Provide maximum and minimum temperature ranges that correspond to "extreme cold and heat". Define "hot dry weather"; provide temperature and humidity limits.						<b>ANSWER:</b>  Reference 03 33 12, 1.8.A (IFC Main Set)  Provide maximum and minimum temperature ranges that correspond to "extreme cold and heat". Define "hot dry weather"; provide temperature and humidity limits.
<b>P1-0671</b>	<b>Detail at the Amphitheater/Great Lawn Stairs</b>	<b>Closed</b>	<b>0P</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>  Reference L1-2630, L1-8633, L1-8687, L1-9621, L1-9622, 04 43 00, 01 10 30/APE (IFC Main Set)  Clarify if the amphitheater/great lawn stairs, between gridlines 3 and 4 are stone slab stairs as described in specification 04 43 00, or are wood deck as shown on L1-9621 and L1-9622. L1-2630 points to details that show stone (1/L1-8633, 1/L1-8687) and wood (L1-9622). The Note in details 2/L1-9621, 3/9621 and 1/L1-9622 refer to Alternate 33 which is for modification of stone paving. Sheet L1-9621 is referenced under Alternate 33 in specification 01 10 30/APE, but L1-9622 is not. Sheet L1-9621 and L1-9622 are labeled as wood deck and details refer to boards, not slabs and are using concrete masonry anchors. These details conflict with L1-8633 which shows the stairs as stone slab using stone stair anchor pins. L1-8687 also conflicts with L1-9621 and L1-9622. Overall there appear to be conflicting details for the amphitheater/great lawn stairs. Clarify which set of details are meant to be followed and delete any extraneous details. Correct 01 10 30/APE E.1.21 Alternate No. 33 reference drawings and specification sections.						<b>ANSWER:</b>  Reference L1-2630, L1-8633, L1-8687, L1-9621, L1-9622, 04 43 00, 01 10 30/APE (IFC Main Set)  Clarify if the amphitheater/great lawn stairs, between gridlines 3 and 4 are stone slab stairs as described in specification 04 43 00, or are wood deck as shown on L1-9621 and L1-9622. L1-2630 points to details that show stone (1/L1-8633, 1/L1-8687) and wood (L1-9622). The Note in details 2/L1-9621, 3/9621 and 1/L1-9622 refer to Alternate 33 which is for modification of stone paving. Sheet L1-9621 is referenced under Alternate 33 in specification 01 10 30/APE, but L1-9622 is not. Sheet L1-9621 and L1-9622 are labeled as wood deck and details refer to boards, not slabs and are using concrete masonry anchors. These details conflict with L1-8633 which shows the stairs as stone slab using stone stair anchor pins. L1-8687 also conflicts with L1-9621 and L1-9622. Overall there appear to be conflicting details for the amphitheater/great lawn stairs. Clarify which set of details are meant to be followed and delete any extraneous details. Correct 01 10 30/APE E.1.21 Alternate No. 33 reference drawings and specification sections.



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<b>P1-0672</b>	<b>Stone Stairs</b>	<b>Closed</b>	<b>0P</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference 04 43 00, 1.5.C.1, L1-9621, L1-9622 (IFC Main Set)		Reference 04 43 00, 1.5.C.1, L1-9621, L1-9622 (IFC Main Set)				
Specification 04 43 00, 1.5.C.1 states to "Construct a full width section of stone stair in length and with curve transitions as indicated in area shown on Drawings." There is no area indicated within the Drawings showing the section to be constructed as the in field mockup. Provide the area in the Drawings to be constructed as the in field mockup as stated in section 04 43 00, 1.5.C.1.		Specification 04 43 00, 1.5.C.1 states to "Construct a full width section of stone stair in length and with curve transitions as indicated in area shown on Drawings." There is no area indicated within the Drawings showing the section to be constructed as the in field mockup. Provide the area in the Drawings to be constructed as the in field mockup as stated in section 04 43 00, 1.5.C.1.				
<b>P1-0673</b>	<b>Incorrect Callout on L1-2630</b>	<b>Closed</b>	<b>0P</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference L1-2630 (IFC Main Set)		Reference L1-2630 (IFC Main Set)				
Confirm that the call out for 5/L1-9622 on 2/L1-2630 should be 1/L1-9622.		Confirm that the call out for 5/L1-9622 on 2/L1-2630 should be 1/L1-9622.				
<b>P1-0674</b>	<b>Seismic Joint Tray</b>	<b>Closed</b>	<b>0P</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference 05 60 00, 1.4.E.1		Reference 05 60 00, 1.4.E.1				
Specification 05 60 00, 1.4.E.1 states "Seismic Joint tray: Provide area as indicated in drawings with all associated infill materials and sealants with the tray." There is no area indicated within the Drawings showing the section to be constructed as the mockup. Provide the area in the Drawings to be constructed as the mockup.		Specification 05 60 00, 1.4.E.1 states "Seismic Joint tray: Provide area as indicated in drawings with all associated infill materials and sealants with the tray." There is no area indicated within the Drawings showing the section to be constructed as the mockup. Provide the area in the Drawings to be constructed as the mockup.				



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<b>P1-0675</b>	<b>Wood Deck Mockup</b>	<b>Closed</b>	<b>0P</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference 06 15 35, 1.4.C.1 (IFC Main SeT)  Specification 06 15 35, 1.4.C.1 states "Install shims, nailer boards, and horizontal and vertical deck boards on the concrete structure for an area as indicated in the Drawings." There is no area indicated within the Drawings showing the section to be constructed as the wood deck mockup. Provide the area in the Drawings to be constructed as the wood deck mockup.						<b>ANSWER:</b> Reference 06 15 35, 1.4.C.1 (IFC Main SeT)  Specification 06 15 35, 1.4.C.1 states "Install shims, nailer boards, and horizontal and vertical deck boards on the concrete structure for an area as indicated in the Drawings." There is no area indicated within the Drawings showing the section to be constructed as the wood deck mockup. Provide the area in the Drawings to be constructed as the wood deck mockup.
<b>P1-0676</b>	<b>Radial Wood Stair Mockup</b>	<b>Closed</b>	<b>0P</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference 06 15 35, 1.4.D.1 (IFC Main Set)  Specification 06 15 35, 1.4.D.1 states "Install shims, nailer boards, and horizontal stair deck boards on the concrete structure for an area as indicated in the Drawings." There is no area indicated within the Drawings showing the section to be constructed as the radial wood stair mockup. Provide the area in the Drawings to be constructed as the radial wood stair mockup.						<b>ANSWER:</b> Reference 06 15 35, 1.4.D.1 (IFC Main Set)  Specification 06 15 35, 1.4.D.1 states "Install shims, nailer boards, and horizontal stair deck boards on the concrete structure for an area as indicated in the Drawings." There is no area indicated within the Drawings showing the section to be constructed as the radial wood stair mockup. Provide the area in the Drawings to be constructed as the radial wood stair mockup.
<b>P1-0677</b>	<b>Bus Jet Fountain Typical Radius for Mockup</b>	<b>Closed</b>	<b>0P</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference 08 81 00, 1.3.D.3.a (IFC Main Set)  Specification 08 81 00, 1.3.D.3.a states to "Provide typical radius as indicated in the drawings." There is no typical radius for the Bus Jet Fountain glass wall panels. Each is specific to their precast module. Provide the typical radius to be used for the mockup.						<b>ANSWER:</b> Reference 08 81 00, 1.3.D.3.a (IFC Main Set)  Specification 08 81 00, 1.3.D.3.a states to "Provide typical radius as indicated in the drawings." There is no typical radius for the Bus Jet Fountain glass wall panels. Each is specific to their precast module. Provide the typical radius to be used for the mockup.



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<b>P1-0678</b>	<b>Mockup of Guying Systems</b>	<b>Closed</b>	<b>0P</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference 32 93 00, 1.6.B.1 (IFC Main Set)  Specification 32 93 00, 1.6.B.1 states to "Prepare mock-up of guying system, as shown in landscape Drawings." There is no detail for the guying system in the landscape drawings. Provide the detail of the guying system to be used in the mock-up.						<b>ANSWER:</b> Reference 32 93 00, 1.6.B.1 (IFC Main Set)  Specification 32 93 00, 1.6.B.1 states to "Prepare mock-up of guying system, as shown in landscape Drawings." There is no detail for the guying system in the landscape drawings. Provide the detail of the guying system to be used in the mock-up.
<b>P1-0679</b>	<b>Spacing for Bamboo Mockup</b>	<b>Closed</b>	<b>0P</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b> Reference 32 93 00, 1.6.C.3 (IFC Main Set)  Specification 32 93 00, 1.6.C.3 states to "Provide mock-up of at least twelve plants at spacing indicated on Drawings with minimum one perimeter edge." There is no spacing indicated for the bamboo in the drawings. Provide the spacing required for the bamboo and the bamboo bracing system.						<b>ANSWER:</b> Reference 32 93 00, 1.6.C.3 (IFC Main Set)  Specification 32 93 00, 1.6.C.3 states to "Provide mock-up of at least twelve plants at spacing indicated on Drawings with minimum one perimeter edge." There is no spacing indicated for the bamboo in the drawings. Provide the spacing required for the bamboo and the bamboo bracing system.
<b>P1-0680</b>	<b>Layout for Handrail Illumination Strips</b>	<b>Closed</b>	<b>0P</b>	<b>11/13/2014</b>	<b>11/13/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b> REFERENCE: Specification Section 10 14 43 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 14 43 3.2 C Photo-Luminescent Exit Accessories indicates that handrail illumination strips are to be installed on all handrails including intermediate handrails.  No details are given for layout of handrail illumination strips on handrails.  Please provide layout details for the handrail illumination strips.						<b>ANSWER:</b> REFERENCE: Specification Section 10 14 43 (IFC Drawings for Main Package dated 3/31/14)  Specification Section 10 14 43 3.2 C Photo-Luminescent Exit Accessories indicates that handrail illumination strips are to be installed on all handrails including intermediate handrails.  No details are given for layout of handrail illumination strips on handrails.  Please provide layout details for the handrail illumination strips.



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<b>P1-0681</b>	<b>Locations for Pylon Signage Photo-Luminescent Exit Accessories</b>	<b>Open</b>	<b>0P</b>	<b>11/13/2014</b>	<b>11/23/2014</b>	<b>01/23/2015</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Specification Section 10 14 43 (IFC Drawings for Main Package dated 3/31/14)		REFERENCE: Specification Section 10 14 43 (IFC Drawings for Main Package dated 3/31/14)				
Specification Section 10 14 43 1.1 A Photo-Luminescent Exit Accessories indicates that photo-luminescent stair & leading edge nosings, demarcation way-fining strips, obstacle markers, final exit door signs, intermediate directional signage, door hardware markings, and directional & door signs are to be furnish and installed on the project.		Specification Section 10 14 43 1.1 A Photo-Luminescent Exit Accessories indicates that photo-luminescent stair & leading edge nosings, demarcation way-fining strips, obstacle markers, final exit door signs, intermediate directional signage, door hardware markings, and directional & door signs are to be furnish and installed on the project.				
The location of these items are not identified within the contract documents.		The location of these items are not identified within the contract documents.				
Please confirm photo-luminescent stair & leading edge nosings, demarcation way-fining strips, obstacle markers, final exit door signs, intermediate directional signage, door hardware markings, and directional & door signs are not required for the project, or provide locations for each item.		Please confirm photo-luminescent stair & leading edge nosings, demarcation way-fining strips, obstacle markers, final exit door signs, intermediate directional signage, door hardware markings, and directional & door signs are not required for the project, or provide locations for each item.				
<b>P1-0682</b>	<b>Location of Room 0521 Maintenance per Finish Schedule</b>	<b>Closed</b>	<b>0P</b>	<b>11/13/2014</b>	<b>11/23/2014</b>	<b>11/19/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Sheet A1-9603 (ASI 119 dated 6/20/14)		REFERENCE: Sheet A1-9603 (ASI 119 dated 6/20/14)				
Sheet A1-9603 references Room 01521 Maintenance, but that room does not exist on the plans.		Sheet A1-9603 references Room 01521 Maintenance, but that room does not exist on the plans.				
Please revise the finish schedule to delete the room, or provide location of the room.		Please revise the finish schedule to delete the room, or provide location of the room.				
<b>P1-0683</b>	<b>Details for Typical Top of Concrete Wall Details</b>	<b>Closed</b>	<b>0P</b>	<b>11/13/2014</b>	<b>11/23/2014</b>	<b>11/20/2014</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
REFERENCE: Detail 5/S1-9051 (IFC Drawings for Main Package dated		REFERENCE: Detail 5/S1-9051 (IFC Drawings for Main Package dated				



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3/31/14)	Per Detail 5/S1-9051 the top of non-bearing concrete wall has a 1" min. gap filled with compressible joint filler.					
	Architectural plans do not provide typical top of concrete wall details showing fire rated assemblies (CMU walls are shown, but not conc.).					
	Please provide typical top of concrete wall details.					
P1-0684	Elastomeric Coating and Color for Roof Park Perimeter Concrete Wall	Closed	0P	11/13/2014	11/23/2014	11/18/2014
From:	Webcor Construction LP					
	Tram Nguyen					
REQUEST:						
REFERENCE:						
RFI T-1857						
The response to RFI T-1857 indicates an elastomeric coating specified in Division 09 is to be furnished and installed on the Roof Park Perimeter Concrete Wall.						
No elastomeric coating has been specified for this condition.						
Please provide the elastomeric coating to be furnished and installed on the Roof Park Perimeter Concrete Wall, and its associated color.						
dated 3/31/14)	Per Detail 5/S1-9051 the top of non-bearing concrete wall has a 1" min. gap filled with compressible joint filler.					
	Architectural plans do not provide typical top of concrete wall details showing fire rated assemblies (CMU walls are shown, but not conc.).					
	Please provide typical top of concrete wall details.					
ANSWER:						
REFERENCE:						
RFI T-1857						
The response to RFI T-1857 indicates an elastomeric coating specified in Division 09 is to be furnished and installed on the Roof Park Perimeter Concrete Wall.						
No elastomeric coating has been specified for this condition.						
Please provide the elastomeric coating to be furnished and installed on the Roof Park Perimeter Concrete Wall, and its associated color.						



P1-180.1	VOID	Void	0P	07/11/2014	07/21/2014
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P1-328	<b>From:</b> Webcor Construction LP      Zachary Moore	Void	0P	07/29/2014	08/08/2014	
	<b>REQUEST:</b> VOID					
	<b>ANSWER:</b> VOID					
P1-550	<b>Room 01642 Callout</b>	Void	CR	10/13/2014	10/23/2014	
	<b>From:</b> Webcor Construction LP      Andrew Kitchen					
	<b>REQUEST:</b> Reference A1-9703 ASI 124					
	Detail 1 shows Room 01642 and Door 01642A as the GGT Supervisor Booth. A1-2306 shows Room 01642 as SFMTA. Please revise A1-9703 so that Room 01642 and Door 01642A correspond to the SFMTA Booth.					
	<b>ANSWER:</b> Reference A1-9703 ASI 124					
	Detail 1 shows Room 01642 and Door 01642A as the GGT Supervisor Booth. A1-2306 shows Room 01642 as SFMTA. Please revise A1-9703 so that Room 01642 and Door 01642A correspond to the SFMTA Booth.					





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From: Webcor Construction LP	Michael Spillane					
REQUEST:	<p>In Area 4, between soldier piles #41 and #42, there are approximately 13ea wall vertical bars that are in contact with the waterproofing due to CDSM Encroachment. Soldier Piles #41 and #42 were surveyed and shown to not encroach more than 1/2". However, at the elevation of wall lift 1, it appears that the CDSM encroaches vertically into the foundation wall. Due to the wall verticals having little or no clearance to the waterproofing, the first list wall vertical bars cannot be coupled onto the dowels protruding from the top of the haunch. Per discussion with TT field representative, Gerdau proposes the following:</p> <p>Option 1: Remove the waterproofing, chip the CDSM wall between soldier piles 41 and 42 to allow for clearance between the vertical bars and waterproofing. Vertical bars adjacent to soldier piles 41 and 42, up to 6 total, will be abandoned.</p> <p>Option 2: The dowels above the haunch will be slightly bent away from the CDSM wall to allow for threading of the first lift wall vertical bars. Prior to bending the bar, the haunch concrete will have to chipped out a minimum of 1.5 ft wide by 1ft deep to allow for hickey bar access. Once the first lift vertical bar is threaded onto the dowel, then the vertical bar will be transitioned back into vertical alignment with a slight bend over approximately 6ft. Note that this transition will require the wall horizontals to be bent and cross-ties will need to be shortened to follow the profile of the wall verticals.</p> <p>Please confirm if proposed options are acceptable.</p>					
ANSWER:	<p>In Area 4, between soldier piles #41 and #42, there are approximately 13ea wall vertical bars that are in contact with the waterproofing due to CDSM Encroachment. Soldier Piles #41 and #42 were surveyed and shown to not encroach more than 1/2". However, at the elevation of wall lift 1, it appears that the CDSM encroaches vertically into the foundation wall. Due to the wall verticals having little or no clearance to the waterproofing, the first list wall vertical bars cannot be coupled onto the dowels protruding from the top of the haunch. Per discussion with TT field representative, Gerdau proposes the following:</p> <p>Option 1: Remove the waterproofing, chip the CDSM wall between soldier piles 41 and 42 to allow for clearance between the vertical bars and waterproofing. Vertical bars adjacent to soldier piles 41 and 42, up to 6 total, will be abandoned.</p> <p>Option 2: The dowels above the haunch will be slightly bent away from the CDSM wall to allow for threading of the first lift wall vertical bars. Prior to bending the bar, the haunch concrete will have to chipped out a minimum of 1.5 ft wide by 1ft deep to allow for hickey bar access. Once the first lift vertical bar is threaded onto the dowel, then the vertical bar will be transitioned back into vertical alignment with a slight bend over approximately 6ft. Note that this transition will require the wall horizontals to be bent and cross-ties will need to be shortened to follow the profile of the wall verticals.</p> <p>Please confirm if proposed options are acceptable.</p>					
RFI T-1124	SSS - Plate Grade Substitution	Closed	CR	01/21/2014	01/31/2014	01/27/2014
From: Skanska USA Civil West California Dis	Ryan Clayton					
REQUEST:	<p>Note SS-1 on drawing S-0007 states plate used for built-up shapes as follows: "ASTM A572, Grade 50, UON (58 ksi max yield for plates used for beam flanges) ASTM 709,</p>					
ANSWER:	<p>Note SS-1 on drawing S-0007 states plate used for built-up shapes as follows: "ASTM A572, Grade 50, UON (58 ksi max yield for plates used for beam</p>					



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	<p>Grade 70W where specifically specified."</p> <p>ASTM standards state the maximum plate thickness available in ASTM Grade 50 is 4", and for Grade 42 is 6" Numerous locations on the Moment Frame Columns specify thickened web plates that exceed 4" in thickness.</p> <p>Is it acceptable to use ASTM A572 Grade 42 for plate thicknesses over 4"? If not, please specify required material and grade.</p>					<p>flanges) ASTM 709, Grade 70W where specifically specified."</p> <p>ASTM standards state the maximum plate thickness available in ASTM Grade 50 is 4", and for Grade 42 is 6" Numerous locations on the Moment Frame Columns specify thickened web plates that exceed 4" in thickness.</p> <p>Is it acceptable to use ASTM A572 Grade 42 for plate thicknesses over 4"? If not, please specify required material and grade.</p>
<b>RFI T-1151</b>	<b>SSS - AESS Mockup Sequence Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>02/05/2014</b>	<b>02/15/2014</b>	<b>02/10/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> <p>Please clarify the sequence the contractor is being asked to provide for the AESS mockup. Is the contractor required to provide a mockup (and have A/E review/approve) in the field at grid line 11 (South) per A1-8660 prior to fabricating all AESS elements as indicated in 05 12 14?</p>						<b>ANSWER:</b> <p>Please clarify the sequence the contractor is being asked to provide for the AESS mockup. Is the contractor required to provide a mockup (and have A/E review/approve) in the field at grid line 11 (South) per A1-8660 prior to fabricating all AESS elements as indicated in 05 12 14?</p>
<b>RFI T-1196</b>	<b>SSS - Rebar Coupler Attachment Plate</b>	<b>Closed</b>	<b>CR</b>	<b>02/25/2014</b>	<b>03/07/2014</b>	<b>03/04/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> <p>See attached sketch CD RFI # 315 SK1 for the rebar coupler attachment plate shown in detail 9/S1-3702 that is shown extended past the end of the transfer girder by 6". All other locations for similar plates are shown with the plates flush to the end of the girder. Please confirm the plates at these locations are intended to extend past the end of the girder by 6".</p>						<b>ANSWER:</b> <p>See attached sketch CD RFI # 315 SK1 for the rebar coupler attachment plate shown in detail 9/S1-3702 that is shown extended past the end of the transfer girder by 6". All other locations for similar plates are shown with the plates flush to the end of the girder. Please confirm the plates at these locations are intended to extend past the end of the girder by 6".</p>
<b>RFI T-1220</b>	<b>BGP - SFPUC Grounding Details</b>	<b>Closed</b>	<b>01</b>	<b>03/06/2014</b>	<b>03/16/2014</b>	<b>03/17/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						



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<b>REQUEST:</b>						
Reference: E1-6006, E1-3212						
ASI 113 Revision Narrative, plan sheet EI-6006 states, "Moved SFPUC grounding layouts to sheet EI- 3213 for clarification."						
Sheet EI-3213 was not issued as a contract document, and is not listed as such on the drawing index, E- 0000. It is also not included in any ASI issued to date. Please provide plan sheet EI-3212.						
<b>ANSWER:</b>						
Reference: E1-6006, E1-3212						
ASI 113 Revision Narrative, plan sheet EI-6006 states, "Moved SFPUC grounding layouts to sheet EI- 3213 for clarification."						
Sheet EI-3213 was not issued as a contract document, and is not listed as such on the drawing index, E- 0000. It is also not included in any ASI issued to date. Please provide plan sheet EI-3212.						
<b>RFI T-1233</b>	<b>SSS - HSS Sleeve for Light Column Anchor Bolts</b>	Closed	CR	03/14/2014	03/24/2014	03/25/2014
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>						
AISC Code of Standard Practice allows for variation of 1/8" between the centers of any two Anchor Rods within an Anchor-Rod Group and an accumulated variation of ¼" between centers of Anchor-Rod Groups. To account for this variation and any slight offset of the galvanized duct around the light column anchor bars, Skanska requests to increase the size of the HSS tube welded to the underside of the top anchor plate.						
Please confirm it is acceptable to provide a 5.0"x 0.125" HSS sleeve as indicated in the attached sketch. Upon approval, this revision will be incorporated into the Light Column Anchor Bolt shop drawings and submitted for record.						
<b>ANSWER:</b>						
AISC Code of Standard Practice allows for variation of 1/8" between the centers of any two Anchor Rods within an Anchor-Rod Group and an accumulated variation of ¼" between centers of Anchor-Rod Groups. To account for this variation and any slight offset of the galvanized duct around the light column anchor bars, Skanska requests to increase the size of the HSS tube welded to the underside of the top anchor plate.						
Please confirm it is acceptable to provide a 5.0"x 0.125" HSS sleeve as indicated in the attached sketch. Upon approval, this revision will be incorporated into the Light Column Anchor Bolt shop drawings and submitted for record.						



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<b>RFI T-1305</b>	<b>SSS - Deck Support at MF Protected Zones</b>	<b>Closed</b>	<b>CR</b>	<b>04/04/2014</b>	<b>04/14/2014</b>	<b>04/16/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 382 SK1 for items 1 & 2: 1) The bent deck support plates per detail 8/S1-5003 will not work at the shaped flanges. Supply an alternate detail. 2) The bent deck support plates per detail 8/S1-5003 occur inside the "Protected Zone." Confirm welding is acceptable or supply an alternate solution.						<b>ANSWER:</b> See attached CD RFI # 382 SK1 for items 1 & 2: 1) The bent deck support plates per detail 8/S1-5003 will not work at the shaped flanges. Supply an alternate detail. 2) The bent deck support plates per detail 8/S1-5003 occur inside the "Protected Zone." Confirm welding is acceptable or supply an alternate solution.
<b>RFI T-1344</b>	<b>BGP - Bike Ramp Column Jacket Ring Plate Welded Studs</b>	<b>Closed</b>	<b>01</b>	<b>04/22/2014</b>	<b>05/02/2014</b>	<b>04/29/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> Please reference S 1-3503 rev 2 detail 6, and A 1-9213 rev 0 detail 7 & 8.  Detail 6 on S1-3503 calls for 1/2" thick ring with 8" long welded studs at base of column, to be used for installation of column jackets. Details 8 & 9 on AI-9213 show the above 1/2" ring and jacket is required where a column extends through the bicycle ramp.  Please confirm there is not a conflict when using 1/2" thick plate and 8" welded stud, in a 8" bike ramp slab.						<b>ANSWER:</b> Please reference S 1-3503 rev 2 detail 6, and A 1-9213 rev 0 detail 7 & 8.  Detail 6 on S1-3503 calls for 1/2" thick ring with 8" long welded studs at base of column, to be used for installation of column jackets. Details 8 & 9 on AI-9213 show the above 1/2" ring and jacket is required where a column extends through the bicycle ramp.  Please confirm there is not a conflict when using 1/2" thick plate and 8" welded stud, in a 8" bike ramp slab.
<b>RFI T-1585</b>	<b>BGP - Deformed Bars at Seismic Joint Embed</b>	<b>Closed</b>	<b>01</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/15/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> Please reference S1-3010 and RFI #T-1547 response.  Per RFI T-1547 response, please confirm deformed bars on seismic joint embeds on Detail 4 ofSI-3010 do not require a head.						<b>ANSWER:</b> Please reference S1-3010 and RFI #T-1547 response.  Per RFI T-1547 response, please confirm deformed bars on seismic joint embeds on Detail 4 ofSI-3010 do not require a head.
<b>RFI T-1594</b>	<b>BGP - Plumbing Sleeve Location Near GL 19.1</b>	<b>Closed</b>	<b>CR</b>	<b>08/08/2014</b>	<b>08/18/2014</b>	<b>08/08/2014</b>
<b>From:</b> Shimmick Construction Company, Inc. Sylvia Hartanto						







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<b>SHIMM00-0361</b>	<b>Dewaering Well Re-Route</b>	<b>Open</b>	<b>CR</b>	<b>10/21/2013</b>	<b>10/31/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Scott Bunnell						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference Detail 6/A1-8711 and S1-3201 of the Contract Drawings and the attached drawing.			Please reference Detail 6/A1-8711 and S1-3201 of the Contract Drawings and the attached drawing.			
SCCI is requesting to re-route all 2" dewatering well lines as proposed in the attached drawings. The re-route is to eliminate any potential conflicts with future work (bracing removal, wall waterproofing, rebar, and for/pour/strip). Upon completion of the use of the dewatering system, the line will be cut below the sleeve, capped and grouted in with the trestle block-out pour back. The line will be poured in place with the future mat and concourse slabs and all 3 wall lifts. The line will also be capped at the top of the final wall lift.			SCCI is requesting to re-route all 2" dewatering well lines as proposed in the attached drawings. The re-route is to eliminate any potential conflicts with future work (bracing removal, wall waterproofing, rebar, and for/pour/strip). Upon completion of the use of the dewatering system, the line will be cut below the sleeve, capped and grouted in with the trestle block-out pour back. The line will be poured in place with the future mat and concourse slabs and all 3 wall lifts. The line will also be capped at the top of the final wall lift.			
Is this acceptable?			Is this acceptable?			
<b>SHIMM000-0001</b>	<b>BGP - Construction Joint Layout</b>	<b>Closed</b>	<b>01</b>	<b>11/15/2012</b>	<b>11/25/2012</b>	<b>11/15/2012</b>
<b>From:</b> Shimmick Construction Company, Inc. Tyler Shell						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please confirm that the construction joint layouts for the Lower Concourse, Foundation Walls and Mat Slab as shown on sheet SL-025 (Exhibit A) are acceptable. Please note that the construction joint lengths of the Mat Slab exceed 120 linear feet in (7) of the specified areas.			Please confirm that the construction joint layouts for the Lower Concourse, Foundation Walls and Mat Slab as shown on sheet SL-025 (Exhibit A) are acceptable. Please note that the construction joint lengths of the Mat Slab exceed 120 linear feet in (7) of the specified areas.			
<b>SHIMM000-0002</b>	<b>BGP - Foundation Wall Horizontal Construction Joint Elevation</b>	<b>Closed</b>	<b>01</b>	<b>11/27/2012</b>	<b>12/07/2012</b>	<b>11/27/2012</b>
<b>From:</b> Shimmick Construction Company, Inc. Tyler Shell						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Drawings: S1-3201, SCCI#11 & #12 Reference Specification: 03 30 20			Reference Drawings: S1-3201, SCCI#11 & #12 Reference Specification: 03 30 20			
Please see attached drawings showing conflicts between the temporary waler lookouts and the horizontal wall construction joints as shown on drawing S1-3201. Please provide direction			Please see attached drawings showing conflicts between the temporary waler lookouts and the horizontal wall construction joints as shown on drawing S1-3201. Please provide direction			



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<b>SHIMM000-0003</b>	<b>BGP - UV damage to Modified Bitumen Waterproofing</b>	<b>Closed</b>	<b>01</b>	<b>01/11/2013</b>	<b>01/21/2013</b>	<b>01/11/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Specification Reference: 07 12 10			Specification Reference: 07 12 10			
Most of the self-adhering modified butimens are damaged by long-term exposure to UV. Can this membrane be exposed to ultraviolet radiation for extended periods of time? If so, how long?			Most of the self-adhering modified butimens are damaged by long-term exposure to UV. Can this membrane be exposed to ultraviolet radiation for extended periods of time? If so, how long?			
<b>SHIMM000-0004</b>	<b>BGP - Modified Bitumen Waterproofing</b>	<b>Closed</b>	<b>01</b>	<b>01/11/2013</b>	<b>01/21/2013</b>	<b>01/11/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification: 07 12 10			Reference Specification: 07 12 10			
A two-ply self-adhered modified bitumen waterproofing system has been specified for this blind side application (Section 1.1 of Specifications). It is unusual for any modified bitumen system to be used in a blind side application (i.e., where the waterproofing is installed before the structure is constructed). Section 2.2 of the Specifications lists only one potential manufacturer, Laurenco Waterproofing Systems. The Laurenco system is a bitumen modified with chloroprene rubber and applied with a cold adhesive. The required waterproofing membrane properties listed in Section 2.4.B are identical to those published by Laurenco. We cannot find any other other modified bitumen manufactured with chloroprene on the market. Are you aware of any other systems?			A two-ply self-adhered modified bitumen waterproofing system has been specified for this blind side application (Section 1.1 of Specifications). It is unusual for any modified bitumen system to be used in a blind side application (i.e., where the waterproofing is installed before the structure is constructed). Section 2.2 of the Specifications lists only one potential manufacturer, Laurenco Waterproofing Systems. The Laurenco system is a bitumen modified with chloroprene rubber and applied with a cold adhesive. The required waterproofing membrane properties listed in Section 2.4.B are identical to those published by Laurenco. We cannot find any other other modified bitumen manufactured with chloroprene on the market. Are you aware of any other systems?			
<b>SHIMM000-0005</b>	<b>BGP - Waterproofing Wall System Layers</b>	<b>Closed</b>	<b>01</b>	<b>01/11/2013</b>	<b>01/21/2013</b>	<b>01/11/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification: 07 12 10, 3.2-3.3			Reference Specification: 07 12 10, 3.2-3.3			
1. Section 3.2, D. requires the protection board horizontal construction joints to be shingled lapping the upper sheet over the lower sheet by 4 inches. What is the purpose of this shingle? Since the waterproofing membrane will not be adhered directly to the protection board and layers will			1. Section 3.2, D. requires the protection board horizontal construction joints to be shingled lapping the upper sheet over the lower sheet by 4 inches. What is the purpose of this shingle? Since the waterproofing membrane will not be adhered directly			



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	<p>be present between them (e.g. drainage composite w/filter fabric, insulation, felt), the shingle does not seem necessary. Please confirm.</p> <p>2.Section 3.2, F. reads "seal top edge of filter fabric to membrane". There is a layer of 1/2" thick insulation between drainage composite and waterproofing membrane. Please clarify.</p> <p>3. In addition to these items, there is also a concern about the number of layers used on this wall including the stability and durability prior to concrete placement. There is a large potential for problems such as creep of the adhesives securing the various layers together and loss of adhesion between layers. What is the purpose of the asphalt saturated felt layers, drainage composite, filter fabric and EPS insulation? Can some of these layers be eliminated? What level of adhesion is required between layers? Does this system of layers have sufficient rigidity to provide intimate contact between the waterproofing layer and</p>					<p>to the protection board and layers will be present between them (e.g. drainage composite w/filter fabric, insulation, felt), the shingle does not seem necessary. Please confirm.</p> <p>2.Section 3.2, F. reads "seal top edge of filter fabric to membrane". There is a layer of 1/2" thick insulation between drainage composite and waterproofing membrane. Please clarify.</p> <p>3. In addition to these items, there is also a concern about the number of layers used on this wall including the stability and durability prior to concrete placement. There is a large potential for problems such as creep of the adhesives securing the various layers together and loss of adhesion between layers. What is the purpose of the asphalt saturated felt layers, drainage composite, filter fabric and EPS insulation? Can some of these layers be eliminated? What level of adhesion is required between layers? Does this system of layers have sufficient rigidity to provide intimate contact between the waterproofing layer and</p>
<b>SHIMM000-0006</b>	<b>BGP - Horizontal Construction Joints - Foundation Walls</b>	<b>Closed</b>	<b>01</b>	<b>01/16/2013</b>	<b>01/26/2013</b>	<b>01/16/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification: 03 20 00 Reference Drawing: S1-3001			Reference Specification: 03 20 00 Reference Drawing: S1-3001			
Please reference detail 7 on Drawings S1-3001 and Specifications Section 03 20 00 3 .2-B. Structural details do not clearly show size of the foundation wall horizontal construction joint keyway. Specifications Section 03 20 00 3 .2-B, however, calls out for: "1-1 12 inch deep key type construction joint at the end of each placement for slabs, beams and walls unless otherwise noted on drawings". Since Specifications take precedence over the drawings in this case, SCCI believes that all horizontal construction joints in the foundation walls shall have 1 1/2" deep keyway.			Please reference detail 7 on Drawings S1-3001 and Specifications Section 03 20 00 3 .2-B. Structural details do not clearly show size of the foundation wall horizontal construction joint keyway. Specifications Section 03 20 00 3 .2-B, however, calls out for: "1-1 12 inch deep key type construction joint at the end of each placement for slabs, beams and walls unless otherwise noted on drawings". Since Specifications take precedence over the drawings in this case, SCCI believes that all horizontal construction joints in the foundation walls shall have 1 1/2" deep keyway.			



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SHIMM000-0007	BGP - WPM-1 - Mud Slab Finish for Waterproofing	Closed	01	01/17/2013	01/27/2013	01/31/2013
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Specification Section 07 12 10, 3.2			Specification Section 07 12 10, 3.2			
<p>The concrete surface profile (CSP) required by the waterproofing manufacturer Laurenc0, ranges between a CSP level of 2 and 4 as defined by the International Concrete Repair Institute (ICRI) of technical guide "Selecting and Specifying Concrete Surface Preparation for Sealers, Coatings, and Polymer Overlays." The ICRI defines the levels of CSP as 1 (nearly flat) to CSP Level 9 (very rough). The Laurenc0 waterproofing system requires "a good wood screed or broom finish...often referred to as a 'sidewalk' finish..Do not use a steel trowel finish." See attached excerpt of the manufacturer specification.</p> <p>1. Please confirm the specified ICRI CSP requirements as it relates to surface finish are to supersede the varying ASTM F-value requirements setforth in specification section 030300-3.6, B1 or provide a revised specification section 033000 incorporating the ICRI requirement.</p> <p>2. Please confirm a wood screed or broom finish is accpetable for the mud slab.</p>			<p>The concrete surface profile (CSP) required by the waterproofing manufacturer Laurenc0, ranges between a CSP level of 2 and 4 as defined by the International Concrete Repair Institute (ICRI) of technical guide "Selecting and Specifying Concrete Surface Preparation for Sealers, Coatings, and Polymer Overlays." The ICRI defines the levels of CSP as 1 (nearly flat) to CSP Level 9 (very rough). The Laurenc0 waterproofing system requires "a good wood screed or broom finish...often referred to as a 'sidewalk' finish..Do not use a steel trowel finish." See attached excerpt of the manufacturer specification.</p> <p>1. Please confirm the specified ICRI CSP requirements as it relates to surface finish are to supersede the varying ASTM F-value requirements setforth in specification section 030300-3.6, B1 or provide a revised specification section 033000 incorporating the ICRI requirement.</p> <p>2. Please confirm a wood screed or broom finish is accpetable for the mud slab.</p>			



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<b>SHIMM000-0008</b>	<b>BGP - Geothermal Pipe Penetration Sleeves at the Manifolds</b>	<b>Closed</b>	<b>01</b>	<b>01/30/2013</b>	<b>02/09/2013</b>	<b>01/30/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Drawing: A1-8710			Reference Drawing: A1-8710			
Per Detail 1 on plan sheet A1-8710, the pipe penetration sleeves are not to be anchored to any portion of the CDSM wall. The sole mounting connection for these pipe sleeves is the bitumen waterproofing membranes. The waterproofing membrane is not strong enough to use as anchorage for these sleeves even with temporary support. The likelihood of jeopardizing the membrane with the design in Detail 1 is high.			Per Detail 1 on plan sheet A1-8710, the pipe penetration sleeves are not to be anchored to any portion of the CDSM wall. The sole mounting connection for these pipe sleeves is the bitumen waterproofing membranes. The waterproofing membrane is not strong enough to use as anchorage for these sleeves even with temporary support. The likelihood of jeopardizing the membrane with the design in Detail 1 is high.			
S3H proposes a constructable solution. Please find attached the details for a constructable design. This design eliminates the waterproofing anchorage support of the penetration sleeve. Please advise.			S3H proposes a constructable solution. Please find attached the details for a constructable design. This design eliminates the waterproofing anchorage support of the penetration sleeve. Please advise.			
<b>SHIMM000-0009</b>	<b>BGP - Geothermal Loop Soil Compaction</b>	<b>Closed</b>	<b>01</b>	<b>03/04/2013</b>	<b>03/14/2013</b>	<b>03/04/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
After observing existing subgrade compactions varying from 88%-95%, it appears that the existing conditions of the soil could be below the 95% compaction requirement. In the case of a geothermal loop being installed in areas with existing soils below 95%, can the geothermal loop be compacted to the localized compaction level in accordance with ASTM DI557? For example, if the first field has an existing condition of 88% compaction, can the geothermal loop trenches be compacted to 88%? 95% compaction may not be possible with the existing soils and existing compaction in some areas.			After observing existing subgrade compactions varying from 88%-95%, it appears that the existing conditions of the soil could be below the 95% compaction requirement. In the case of a geothermal loop being installed in areas with existing soils below 95%, can the geothermal loop be compacted to the localized compaction level in accordance with ASTM DI557? For example, if the first field has an existing condition of 88% compaction, can the geothermal loop trenches be compacted to 88%? 95% compaction may not be possible with the existing soils and existing compaction in some areas.			
Please advise.			Please advise.			
<b>SHIMM000-0010</b>	<b>BGP - Schedule Dates for GLS/GLR Manifold Construction</b>	<b>Closed</b>	<b>01</b>	<b>03/05/2013</b>	<b>03/15/2013</b>	<b>03/05/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Exhibit "I" of the TG06.1 bid package is a conceptual			Exhibit "I" of the TG06.1 bid package is a conceptual			



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	<p>schedule. This schedule does not provide a date for the installation of the stainless steel geothermal manifold sleeve penetrations or manifolds themself. Please provide a date of installation for the sleeve penetrations and manifolds for each of the 15 fields.</p>					<p>schedule. This schedule does not provide a date for the installation of the stainless steel geothermal manifold sleeve penetrations or manifolds themself. Please provide a date of installation for the sleeve penetrations and manifolds for each of the 15 fields.</p>
SHIMM000-0011	<p><b>BGP - Geothermal Pipe Elevation</b></p> <p><b>From:</b> Shimmick Construction Company, Inc. Chris Williams</p> <p><b>REQUEST:</b></p> <p>Reference Drawing: M-5002</p> <p>Per drawing M-5002, Detail I, the GLS/GLR manifold piping is above the TG06 SOW demarcation line. Due to constructability concerns of the manifold, is it acceptable to install the manifold at a lower elevation below the TG06 SOW demarcation line?</p> <p>Please advise.</p>	Closed	01	03/06/2013	03/16/2013	03/06/2013
						<p><b>ANSWER:</b></p> <p>Reference Drawing: M-5002</p> <p>Per drawing M-5002, Detail I, the GLS/GLR manifold piping is above the TG06 SOW demarcation line. Due to constructability concerns of the manifold, is it acceptable to install the manifold at a lower elevation below the TG06 SOW demarcation line?</p> <p>Please advise.</p>
SHIMM000-0012	<p><b>BGP - Monitoring Instrument Penetrations</b></p> <p><b>From:</b> Shimmick Construction Company, Inc. Chris Williams</p> <p><b>REQUEST:</b></p> <p>Reference Drawing: A1-8711</p> <p>Per plan sheet A1-8711, Detail 3, the monitoring instrument penetration sleeve is to be place around the monitoring instrument itself. From the field, it appears that some of these monitoring instruments exist as drawn in Detail 3 (Picture 1) while others seem to be placed within an additional, larger sleeve (Picture 2) casing. This additional casing occurrence isn't accounted for in the contract documents. Please advise to this type of sleeve dimensions and detail. Please note that one of these types of monitoring instrument sleeves is located in the first area to be water proofed and poured for the protection slab.</p>	Closed	01	03/11/2013	03/21/2013	03/11/2013
						<p><b>ANSWER:</b></p> <p>Reference Drawing: A1-8711</p> <p>Per plan sheet A1-8711, Detail 3, the monitoring instrument penetration sleeve is to be place around the monitoring instrument itself. From the field, it appears that some of these monitoring instruments exist as drawn in Detail 3 (Picture 1) while others seem to be placed within an additional, larger sleeve (Picture 2) casing. This additional casing occurrence isn't accounted for in the contract documents. Please advise to this type of sleeve dimensions and detail. Please note that one of these types of monitoring instrument sleeves is located in the first area to be water proofed and poured for the protection slab.</p>
SHIMM000-0013	<p><b>BGP - Welding for Penetration Sleeves</b></p>	Closed	01	03/12/2013	03/22/2013	03/12/2013



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<p><b>From:</b> Shimmick Construction Company, Inc. Andy Khuu</p> <p><b>REQUEST:</b></p> <p>Reference RFI T-0411</p> <p>The Engineer's response to RFI T-0411 states that the collar ring and cap plate cannot be shop welded prior to being installed and that the collar must be welded onto the sleeve prior to the mat slab pour for access purposes. However, in the submittal comments to SUB-TG0600-036, the Engineer clearly states that the "contract documents specify a field weld of the steel ring such that the pile can be cut and removed without the ring installed." Without access to weld the collar after the mat slab has been poured, it isn't possible to weld the assembly in the field. Additionally, if the collar is to be welded prior to the pile being cut, damage will most likely occur to the ring plate or sleeve during the cutting process as stated in the submittal comments. With the comments to submittal TG0600-036 and the response to RFI T -0411 clearly contradictory, please provide the necessary construction sequencing to avoid damage to the assembly in the field and enable a constructible design.</p>						<p><b>ANSWER:</b></p> <p>Reference RFI T-0411</p> <p>The Engineer's response to RFI T-0411 states that the collar ring and cap plate cannot be shop welded prior to being installed and that the collar must be welded onto the sleeve prior to the mat slab pour for access purposes. However, in the submittal comments to SUB-TG0600-036, the Engineer clearly states that the "contract documents specify a field weld of the steel ring such that the pile can be cut and removed without the ring installed." Without access to weld the collar after the mat slab has been poured, it isn't possible to weld the assembly in the field. Additionally, if the collar is to be welded prior to the pile being cut, damage will most likely occur to the ring plate or sleeve during the cutting process as stated in the submittal comments. With the comments to submittal TG0600-036 and the response to RFI T -0411 clearly contradictory, please provide the necessary construction sequencing to avoid damage to the assembly in the field and enable a constructible design.</p>
<p><b>SHIMM000-0014</b></p> <p><b>BGP - Geothermal Risers in Leaking CDSM Wall</b></p> <p><b>From:</b> Shimmick Construction Company, Inc. Chris Williams</p> <p><b>REQUEST:</b></p> <p>With water leakage throughout the CDSM wall at many different locations, the likelihood of a geothermal loop riser being laid out in the location of a CDSM wall leak is high. In the event that the Geothermal Riser is located at the same location as a CDSM wall leak, what should S3H do? Should the riser be relocated to a portion of wall that isn't leaking? If the riser is to be embedded in the wall at the location of a leak, grouting the riser back into the wall will not be possible. Please advise.</p>		Closed	01	03/18/2013	03/28/2013	03/18/2013
<p><b>SHIMM000-0015</b></p> <p><b>BGP - Shoring Beam in Sump Pit</b></p> <p><b>From:</b> Shimmick Construction Company, Inc. Chris Williams</p> <p><b>REQUEST:</b></p>		Closed	01	03/18/2013	03/28/2013	03/18/2013
						<p><b>ANSWER:</b></p> <p>With water leakage throughout the CDSM wall at many different locations, the likelihood of a geothermal loop riser being laid out in the location of a CDSM wall leak is high. In the event that the Geothermal Riser is located at the same location as a CDSM wall leak, what should S3H do? Should the riser be relocated to a portion of wall that isn't leaking? If the riser is to be embedded in the wall at the location of a leak, grouting the riser back into the wall will not be possible. Please advise.</p>





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	<p>Reference Photo: attached</p> <p>Please find attached a photo of the sump pit near J Line in the Geothermal Field 1. In the pit, there is a H-beam from a previous shoring wall. There is potential for this beam to come in conflict with with geothermal loop. Is this beam to be removed? Please advise.</p>					
	<p>Reference Photo: attached</p> <p>Please find attached a photo of the sump pit near J Line in the Geothermal Field 1. In the pit, there is a H-beam from a previous shoring wall. There is potential for this beam to come in conflict with with geothermal loop. Is this beam to be removed? Please advise.</p>					
<b>SHIMM000-0016</b>	<b>BGP - Clarification of Mass Concrete Reporting Periods</b>	<b>Closed</b>	<b>01</b>	<b>03/25/2013</b>	<b>04/04/2013</b>	<b>03/25/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference specification section 03 30 20.3. 11.A (pg 24).			Please reference specification section 03 30 20.3. 11.A (pg 24).			
CTL Group "Thennal Control Plan Model ing" Figure 3 (submittal TG0600-20 1.1 It em #033000-0 I 1.1 pg 8), illustrates the max temperature di fferential is reached and has begun to drop at approximately 8 calendar days.			CTL Group "Thennal Control Plan Model ing" Figure 3 (submittal TG0600-20 1.1 It em #033000-0 I 1.1 pg 8), illustrates the max temperature di fferential is reached and has begun to drop at approximately 8 calendar days.			
SCCI will record temperature differentials at 6 hr intervals and report those readings on a daily (24 hr) basis. Is this acceptable?			SCCI will record temperature differentials at 6 hr intervals and report those readings on a daily (24 hr) basis. Is this acceptable?			







<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>drawing). The proposed top of concourse slab elevation is to be -5.42, WEST of Grid 9. Per the WPM-1 waterproofing system, the minimum overall tie-in dimension needed for the succeeding lift is approximately 1' -11" (see attached waterproofing drawing). The current elevation at the bottom of the 2nd level bracing lookouts is at approximately -6.15, EAST of Grid 9 (see concourse slab drawing). The proposed top of concourse slab elevation CJ is to be -7.67, EAST of Grid 9. Per the WPM-1 waterproofing system, the minimum overall tie-in dimension needed for the succeeding lift is approximately 1' -11" (see attached waterproofing drawing). In both locations, the minimum required dimension (1' -11") to tie-in to the next lift of waterproofing can not be reached with the current location of the 2nd level bracing lookouts and the proposed concourse slab elevations. SCCI is restricted in location for the CJ due to the absolute concourse slab location and elevation.</p> <p>Furthermore, a similar conflict exists in the 1st foundation wall lift and the 3rd level of bracing lookouts (see 1st wall lift drawing). With SCCI's current location of the CJ, there is virtually no room to allow for the waterproofing overlap to occur. SCCI fully understands its freedom to manipulate the location of the CJ's by lowering it approximately 2'. This will potentially change BBII's rebracing plans.</p> <p>Please advise.</p>					<p>(see concourse slab drawing). The proposed top of concourse slab elevation is to be -5.42, WEST of Grid 9. Per the WPM-1 waterproofing system, the minimum overall tie-in dimension needed for the succeeding lift is approximately 1' -11" (see attached waterproofing drawing). The current elevation at the bottom of the 2nd level bracing lookouts is at approximately -6.15, EAST of Grid 9 (see concourse slab drawing). The proposed top of concourse slab elevation CJ is to be -7.67, EAST of Grid 9. Per the WPM-1 waterproofing system, the minimum overall tie-in dimension needed for the succeeding lift is approximately 1' -11" (see attached waterproofing drawing). In both locations, the minimum required dimension (1' -11") to tie-in to the next lift of waterproofing can not be reached with the current location of the 2nd level bracing lookouts and the proposed concourse slab elevations. SCCI is restricted in location for the CJ due to the absolute concourse slab location and elevation.</p> <p>Furthermore, a similar conflict exists in the 1st foundation wall lift and the 3rd level of bracing lookouts (see 1st wall lift drawing). With SCCI's current location of the CJ, there is virtually no room to allow for the waterproofing overlap to occur. SCCI fully understands its freedom to manipulate the location of the CJ's by lowering it approximately 2'. This will potentially change BBII's rebracing plans.</p> <p>Please advise.</p>
<b>SHIMM000-0021</b>	<b>BGP - Differential Movement in Waterproofing Layers</b>	<b>Closed</b>	<b>01</b>	<b>04/26/2013</b>	<b>05/06/2013</b>	<b>04/26/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per the Engineer's response to Submittal TG0600-023.2, the Contractor is to install the waterproofing system to incorporate "provisions for differential movement". Please reference the contract documents that specify the design criteria for the differential movement of the structure. Please advise to a specification or drawing note that details such.			Per the Engineer's response to Submittal TG0600-023.2, the Contractor is to install the waterproofing system to incorporate "provisions for differential movement". Please reference the contract documents that specify the design criteria for the differential movement of the structure. Please advise to a specification or drawing note that details such.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>SHIMM000-0022</b>	<b>BGP - Testing of WPM-1 Seams</b>	<b>Closed</b>	<b>01</b>	<b>04/26/2013</b>	<b>05/06/2013</b>	<b>04/26/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification: 071210 - 3.5.B			Reference Specification: 071210 - 3.5.B			
The Specifications call for testing of "seams" independently by Applicator and Manufacturer. In the waterproofing pre-installation meeting on 3/27/13, the Manufacturer (Laurenco) and the Architect stated that testing of seams is not required as this is not a single-ply system. Please define "seam" and advise if testing of seams is required or not, and if it is, then to what extent?			The Specifications call for testing of "seams" independently by Applicator and Manufacturer. In the waterproofing pre-installation meeting on 3/27/13, the Manufacturer (Laurenco) and the Architect stated that testing of seams is not required as this is not a single-ply system. Please define "seam" and advise if testing of seams is required or not, and if it is, then to what extent?			
<b>SHIMM000-0023</b>	<b>BGP - Carlisle Miradrain 9900 Drainage Composite</b>	<b>Closed</b>	<b>01</b>	<b>04/26/2013</b>	<b>05/06/2013</b>	<b>04/26/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification: 07 12 10 2.5.C			Reference Specification: 07 12 10 2.5.C			
This section calls for "Drainage Composite: Three dimensional plastic rolls bonded to a geotextile on one or both faces: Mirafi Miradrain 9900, or equal with a minimum compressive strength of 30,000 psi." The waterproofing membrane manufacturer (Laurenco) states that the specified product "Miradrain 9900" no longer meets the performance requirements of the specifications since the woven filter fabric is no longer bonded at every dimple of the molded polystyrene core. Best Contracting has contacted the drainage composite manufacturer and they have confirmed that the woven filter fabric is bonded at every fourth dimple. Best Contracting has also performed a shop "mock up" using the aforementioned composite which resulted in complete separation and failure upon the installation of the waterproofing membrane. Please provide direction.			This section calls for "Drainage Composite: Three dimensional plastic rolls bonded to a geotextile on one or both faces: Mirafi Miradrain 9900, or equal with a minimum compressive strength of 30,000 psi." The waterproofing membrane manufacturer (Laurenco) states that the specified product "Miradrain 9900" no longer meets the performance requirements of the specifications since the woven filter fabric is no longer bonded at every dimple of the molded polystyrene core. Best Contracting has contacted the drainage composite manufacturer and they have confirmed that the woven filter fabric is bonded at every fourth dimple. Best Contracting has also performed a shop "mock up" using the aforementioned composite which resulted in complete separation and failure upon the installation of the waterproofing membrane. Please provide direction.			
<b>SHIMM000-0024</b>	<b>BGP - Additional Fasteners for Protection Board Installation</b>	<b>Closed</b>	<b>CR</b>	<b>05/02/2013</b>	<b>05/12/2013</b>	<b>05/02/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference Spec Section 07 12 10 - 3.2.D. Spec Section 07 12 10 - 3.2.D states the following: "Secure 1/4"			Please reference Spec Section 07 12 10 - 3.2.D. Spec Section 07 12 10 - 3.2.D states the following: "Secure			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>protection board to flanges of soldier piles with powder driven fasteners and washers spaced 12 inches o.c. Butt vertical joints . Maximum joint width : 1/4"..."</p> <p>The manufacturer of membrane waterproofing system (Laurenco) has indicated that due to "out of plane" piles, and relaxation of CDSM substrate requirement, they are requiring intermediate fasteners to hold the 1/4" protection board tight to the CDSM wall. Please review and advise.</p>				<p>1/4" protection board to flanges of soldier piles with powder driven fasteners and washers spaced 12 inches o.c. Butt vertical joints . Maximum joint width : 1/4"..."</p> <p>The manufacturer of membrane waterproofing system (Laurenco) has indicated that due to "out of plane" piles, and relaxation of CDSM substrate requirement, they are requiring intermediate fasteners to hold the 1/4" protection board tight to the CDSM wall. Please review and advise.</p>	
<b>SHIMM000-0025</b>	<b>BGP -Request for Revit Model</b>	<b>Closed</b>	<b>01</b>	<b>05/02/2013</b>	<b>05/12/2013</b>	<b>05/02/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Andy Khuu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
SCCI is requesting access to the latest, most up to date Structural and Architectural Revit models from the designers. The 3D database would be used for reference only and will not be used for construction. SCCI understands that the 3D Database is subject to change as the project design evolves. As a user of this 30 database, SCCI accepts the risk and acknowledge that the data is subject to change. SCCI also acknowledges the terms and conditions outlined in the Transbay Transit Specification Section 01 31 26.			SCCI is requesting access to the latest, most up to date Structural and Architectural Revit models from the designers. The 3D database would be used for reference only and will not be used for construction. SCCI understands that the 3D Database is subject to change as the project design evolves. As a user of this 30 database, SCCI accepts the risk and acknowledge that the data is subject to change. SCCI also acknowledges the terms and conditions outlined in the Transbay Transit Specification Section 01 31 26.			



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SHIMM000-0026	BGP-Geothermal Field Riser Pipe Termination	Closed	01	05/13/2013	05/23/2013	05/13/2013
<b>From:</b> Shimmick Construction Company, Inc. John Berggren						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification: 23 57 34 Reference Drawing: A1-8712, M1-5002			Reference Specification: 23 57 34 Reference Drawing: A1-8712, M1-5002			
Stainless sleeves as shown on A1-8712 and M1-5002 (copies attached) are not part of the S3H's scope of work. Reference is made to Note 1 and Note 2 on M-0006 (copy attached), the highlighted notes in Detail2 on A1-8712, and the SOW demarcation line in Detail A on M1-5002. S3H Inc. will terminate pipe at grade as shown in Detail A on M1-5002 with pressure guage to be modified by Others. Please confirm. [S3H RFI No. 36]			Stainless sleeves as shown on A1-8712 and M1-5002 (copies attached) are not part of the S3H's scope of work. Reference is made to Note 1 and Note 2 on M-0006 (copy attached), the highlighted notes in Detail2 on A1-8712, and the SOW demarcation line in Detail A on M1-5002. S3H Inc. will terminate pipe at grade as shown in Detail A on M1-5002 with pressure guage to be modified by Others. Please confirm. [S3H RFI No. 36]			
SHIMM000-0027	BGP - Temperature Probe Sleeve Penetration	Closed	01	05/13/2013	05/23/2013	05/13/2013
<b>From:</b> Shimmick Construction Company, Inc. John Berggren						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference is made to RFI T-0388.0 (copy attached) is stating the temperture probe piping shall be installed per Note 6 on Sheet M-0006. Per Note 2 on Sheet M-0006, the additional mechanical work shown above the demarcation line is for reference only and was not included in the TG06.1 package. Please confirm and clarify the design intent.  [S3H RFI No. 028]			Reference is made to RFI T-0388.0 (copy attached) is stating the temperture probe piping shall be installed per Note 6 on Sheet M-0006. Per Note 2 on Sheet M-0006, the additional mechanical work shown above the demarcation line is for reference only and was not included in the TG06.1 package. Please confirm and clarify the design intent.  [S3H RFI No. 028]			
SHIMM000-0028	BGP - Mat Slab Elevator Opening Embed Dimensions	Closed	01	05/10/2013	05/24/2013	05/15/2013
<b>From:</b> Webcor Construction LP                      Ian Corcorran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref. RFI T-0439.1  TJPA's response to RFI T-0439.1 stated "Final elevator post locations shall be coordinated with elevator manufacturer." The response has a second option to use a continuous L8x4x1/2 in lieu of the 1'-2" base. Please provide the elevator post locations if an elevator manufacturer has been selected? If not, SCCI is requesting to use continuous embeds. Please advise if this is acceptable.			Ref. RFI T-0439.1  TJPA's response to RFI T-0439.1 stated "Final elevator post locations shall be coordinated with elevator manufacturer." The response has a second option to use a continuous L8x4x1/2 in lieu of the 1'-2" base. Please provide the elevator post locations if an elevator manufacturer has been selected? If not, SCCI is requesting to use continuous embeds. Please advise if this is acceptable.			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
SHIMM000-0029	BGP - High Congestion Mockup Revit File	Closed	01	05/20/2013	05/30/2013	05/20/2013
<b>From:</b> Shimmick Construction Company, Inc. Jesse Dillon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Gerda is requesting the 3D Revit model of the isometric high congesting area shown in SI-3208/DI. This will allow Gerda to determine conflicts prior to fabri cation of rebar for the upcoming mock up. Please provide Revit file showing this area.			Gerda is requesting the 3D Revit model of the isometric high congesting area shown in SI-3208/DI. This will allow Gerda to determine conflicts prior to fabri cation of rebar for the upcoming mock up. Please provide Revit file showing this area.			
SHIMM000-0030	BGP - Lower Concourse and Mezzanine Plumbing	Closed	01	05/21/2013	05/31/2013	05/21/2013
<b>From:</b> Shimmick Construction Company, Inc. Jesse Dillon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference attached Contract Drawings PI-2202 IFB and PI-2202 IFC. Both IFB and IFC plumbing drawings have the callouts "BELOW GRADE PACKAGE FOR REFERENCE ONLY", "NOT FOR CONSTRUCTION" and do not contain the Architect's/Engineer's seal. This circumstance applies to all Lower Concourse Level and Mezzanine level Plumbing Contract Drawings, PI -2202 to PI-2211 and PI-2252. All Lower Concourse and Mezzanine plumbing depicted in these drawings is excluded from the Below Grade Package. The scope excluded from SCCI's work package includes, but is not limited to, floor drains, area drains, floor sinks and cleanouts. Please inform SCCI about which future package this scope is contained for coordination.			Please reference attached Contract Drawings PI-2202 IFB and PI-2202 IFC. Both IFB and IFC plumbing drawings have the callouts "BELOW GRADE PACKAGE FOR REFERENCE ONLY", "NOT FOR CONSTRUCTION" and do not contain the Architect's/Engineer's seal. This circumstance applies to all Lower Concourse Level and Mezzanine level Plumbing Contract Drawings, PI -2202 to PI-2211 and PI-2252. All Lower Concourse and Mezzanine plumbing depicted in these drawings is excluded from the Below Grade Package. The scope excluded from SCCI's work package includes, but is not limited to, floor drains, area drains, floor sinks and cleanouts. Please inform SCCI about which future package this scope is contained for coordination.			
SHIMM000-0031	BGP - S-3 Wall Stirrups Preassembled Using IDEA Machine	Closed	01	06/04/2013	06/14/2013	06/04/2013
<b>From:</b> Shimmick Construction Company, Inc. Andy Khuu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: RFI T -0340 and T -0526			Reference: RFI T -0340 and T -0526			
Approval was provided to utilize the IDEA machine per the response to RFI T-0340. Since the issuance of this response, approval has also been provided to utilize an S-3 stirrup in lieu of the T-9 hairpin within the walls per RFI 0526. Please confirm that it is acceptable, following the same criteria as outlined in the response to RFI 0340, to use the machine/welded holding wires to pre-assemble the stirrups within the wall reinforcing.			Approval was provided to utilize the IDEA machine per the response to RFI T-0340. Since the issuance of this response, approval has also been provided to utilize an S-3 stirrup in lieu of the T-9 hairpin within the walls per RFI 0526. Please confirm that it is acceptable, following the same criteria as outlined in the response to RFI 0340, to use the machine/welded holding wires to pre-assemble the stirrups within the wall reinforcing.			







**ANSWER:**

Reference: SK-2676,S

SCCI is in receipt of RFI T-0479.1 response outlining that there will be 8 additional areas requiring slab penetration detail per SKA-2676 and SKA-2677 (issued in original RFI#0479).

Please provide the elevations of 'intermediate' base of sleeve flat horizontal mud slab area for all 8 trestle piles, pin piles or bridge piers.





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SHIMM000-0037	BGP - Mass Concrete Specifications	Closed	01	07/17/2013	07/27/2013	07/17/2013
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Please reference attached letter published by Jon Feld, CTL Group, dated July 8, 2013. This letter contains further analysis of the "Performance-Based Temperature Differential Limit", also referred to as "Strength-Based Temperature Difference Limit", for Mat Slab mix # 1557204.</p> <p>This additional analysis was prepared per RFI response T-0585, in which the reviewer found this PBTDL method to be acceptable based on satisfying four (4) conditions. See below:</p> <p>1. The attached analysis was specifically developed for mix #1557204</p> <p>2. SCCI confirms that all remaining mass concrete specification requirements shall still apply.</p> <p>3. Shimmick Construction will be providing field quality control and the required concrete maturity measurements through the "Concrete Maturity HardTrack System". Reference attached HardTrack system data and example concrete maturity data. This system has been procured by Shimmick Construction, and has been successfully tested on multiple mock-ups.</p> <p>4. It is confirmed that Shimmick Construction shall remain responsible for providing a mat foundation that meets requirements of the contract documents.</p> <p>Please confirm conditions have been satisfied. This analysis will be submitted as a supplement to the Mass Concrete Plan (TG0600-20 1.1)</p>			<p>Please reference attached letter published by Jon Feld, CTL Group, dated July 8, 2013. This letter contains further analysis of the "Performance-Based Temperature Differential Limit", also referred to as "Strength-Based Temperature Difference Limit", for Mat Slab mix # 1557204.</p> <p>This additional analysis was prepared per RFI response T-0585, in which the reviewer found this PBTDL method to be acceptable based on satisfying four (4) conditions. See below:</p> <p>1. The attached analysis was specifically developed for mix #1557204</p> <p>2. SCCI confirms that all remaining mass concrete specification requirements shall still apply.</p> <p>3. Shimmick Construction will be providing field quality control and the required concrete maturity measurements through the "Concrete Maturity HardTrack System". Reference attached HardTrack system data and example concrete maturity data. This system has been procured by Shimmick Construction, and has been successfully tested on multiple mock-ups.</p> <p>4. It is confirmed that Shimmick Construction shall remain responsible for providing a mat foundation that meets requirements of the contract documents.</p> <p>Please confirm conditions have been satisfied. This analysis will be submitted as a supplement to the Mass Concrete Plan (TG0600-20 1.1)</p>			



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
SHIMM000-0038	<b>BGP - Geothermal Loop Excavation in Zone 4</b>  From: Webcor Construction LP Jackson Tukuafu	Closed	CR	07/19/2013	07/29/2013	07/19/2013
<b>REQUEST:</b>  Per discussions in the Trade subcontractor meeting with Turner, BBII, and WOJV, it is apparent that BBII has been directed to demo the buttress shafts in Zone 4 to bottom of mud slab elevation. Per the geothermal trenching and backfill specification 31 23 34, 1.1.A.1, the only slot excavation in CDSM/concrete is to be in the wall panels. The specification does not require slot excavation/demolition for the horizontal field loops. Per Plan sheet GT-5201, the buttress shafts are to be demolished to a maximum of 4' below subgrade elevation (bottom of mud slab). The geothermal pipe is to be installed at 15" below the bottom of mud slab elevation, well within the 4' below mud slab demolition elevation. Please confirm that the geothermal loops in zone 4 will be trenched in soil like the rest of the project and as detail in the geothermal trenching and backfill specification (31 23 34).		<b>ANSWER:</b>  Per discussions in the Trade subcontractor meeting with Turner, BBII, and WOJV, it is apparent that BBII has been directed to demo the buttress shafts in Zone 4 to bottom of mud slab elevation. Per the geothermal trenching and backfill specification 31 23 34, 1.1.A.1, the only slot excavation in CDSM/concrete is to be in the wall panels. The specification does not require slot excavation/demolition for the horizontal field loops. Per Plan sheet GT-5201, the buttress shafts are to be demolished to a maximum of 4' below subgrade elevation (bottom of mud slab). The geothermal pipe is to be installed at 15" below the bottom of mud slab elevation, well within the 4' below mud slab demolition elevation. Please confirm that the geothermal loops in zone 4 will be trenched in soil like the rest of the project and as detail in the geothermal trenching and backfill specification (31 23 34).				
SHIMM000-0107	<b>BGP - Concourse Slab Embeds and Trestle Pile Conflicts</b>  From: Shimmick Construction Company, Inc. Ben Gordon	Closed	01	04/09/2013	04/19/2013	09/20/2013
<b>REQUEST:</b>  Ref: S1-2202, S1-2203 and S1-2205  Please reference attached drawings S1-2202, S1-2203 and S1-2205 with pile locations overlaid. There are three locations where the trestle piles interfere with the embedded assemblies at elevator and escalator openings/pits. Please advise.		<b>ANSWER:</b>  Ref: S1-2202, S1-2203 and S1-2205  Please reference attached drawings S1-2202, S1-2203 and S1-2205 with pile locations overlaid. There are three locations where the trestle piles interfere with the embedded assemblies at elevator and escalator openings/pits. Please advise.				
SHIMM000-0141.1	<b>BGP - Moment Beam and Pile Conflicts</b>  From: Shimmick Construction Company, Inc. Ben Gordon	Open	01	07/29/2013	08/08/2013	
<b>REQUEST:</b>  Reference: SCI RFI-141, RFI T-0510.1  Please reference SCCI RFI # 141 and W/O RFI T-0510.1. The response to T-0510.1 provided details for rectifying the MFB conflict at internal Bracing Pin Pile #8. It was		<b>ANSWER:</b>  Reference: SCI RFI-141, RFI T-0510.1  Please reference SCCI RFI # 141 and W/O RFI T-0510.1. The response to T-0510.1 provided details for rectifying the MFB conflict at internal Bracing Pin Pile				



Webcor/Obayashi Joint Venture  
*PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG*  
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	<p>made clear this solution could only be used at Pin Pile #8. No guidance was provided for the additional five MFB conflicts shown in SCCI RFI #141.</p> <p>Please provide information for the five additional MFB and pile conflicts shown in SCCI RFI# 141.</p>					<p>#8. It was made clear this solution could only be used at Pin Pile #8. No guidance was provided for the additional five MFB conflicts shown in SCCI RFI #141.</p> <p>Please provide information for the five additional MFB and pile conflicts shown in SCCI RFI# 141.</p>
SHIMM000-0203.1	<b>BGP - Blockout -Reinforcement and Size Detail Needed at Dewatering Well and Co Closed</b>  From: Shimmick Construction Company, Inc. Ben Gordon  <b>REQUEST:</b> As a follow up to RFI#T0584 response:  1. General note GR9 on S-005 is not applicable for wall block out. Please provide block out detail for the reinforcement on the partition wall for blockout for: Dewatering Well #1, #21 and #22 2. For dewatering well #3- please provide detail for blockout for reinforcing at shearwall 3. Please provide size and extent for blockouts for all 4 dewatering wells.		CR	07/19/2013	08/02/2013	08/27/2013
						<b>ANSWER:</b> As a follow up to RFI#T0584 response:  1. General note GR9 on S-005 is not applicable for wall block out. Please provide block out detail for the reinforcement on the partition wall for blockout for: Dewatering Well #1, #21 and #22 2. For dewatering well #3- please provide detail for blockout for reinforcing at shearwall 3. Please provide size and extent for blockouts for all 4 dewatering wells.
SHIMM000-0204.1	<b>BGP - Locations of Electrical Outlets, Equipment, and Fixtures</b>  From: Shimmick Construction Company, Inc. Ben Gordon  <b>REQUEST:</b> Reference: Spec Section, 34 05 34  Per Specification Section 26 05 34, 3.2 B., the dimensions of the equipment fixtures and outlets are to be submitted via RFI for clarification pre pour. Attached is the layout for Electrical Room B2221 in the first Mat Slab pour.  Please confirm that these dimensions are acceptable so that the conduit can be laid out correctly.	Closed	01	07/31/2013	08/10/2013	
						<b>ANSWER:</b> Reference: Spec Section, 34 05 34  Per Specification Section 26 05 34, 3.2 B., the dimensions of the equipment fixtures and outlets are to be submitted via RFI for clarification pre pour. Attached is the layout for Electrical Room B2221 in the first Mat Slab pour.  Please confirm that these dimensions are acceptable so that the conduit can be laid out correctly.
SHIMM000-0204.2	<b>Locations of Electrical Outlets, Equipment, and Fixtures.</b>	Closed	01	08/23/2013	09/03/2013	



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please find attached the revised layout for Electrical Room B2221. Please confirm that the layout is acceptable.			Please find attached the revised layout for Electrical Room B2221. Please confirm that the layout is acceptable.			
<b>SHIMM000-0204.4</b>	<b>BGP - Locations of electrical Outlets, Equipment and Fixtures</b>	<b>Accepted</b>	<b>CR</b>	<b>09/12/2013</b>	<b>09/22/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per the RFI response, please find attached the revised layout for the Electrical Room B2221. This revised layout shows the dimensions of the conduit locations in respect to the interior walls. Additionally, dimensions showing the room location in respect to the grid lines are shown. please advise if it is acceptable.			Per the RFI response, please find attached the revised layout for the Electrical Room B2221. This revised layout shows the dimensions of the conduit locations in respect to the interior walls. Additionally, dimensions showing the room location in respect to the grid lines are shown. please advise if it is acceptable.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
SHIMM000-0233.1	BGP Bracing Removal Sequence - Area 5-13	Open	01	07/30/2013	08/09/2013	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The latest Webcor's weekly update schedule received by SCCI (Data date 06.17.2013 ), shows that:			The latest Webcor's weekly update schedule received by SCCI (Data date 06.17.2013 ), shows that:			
<p>- Bracing Removal- Level D" (BGSOX-1120) is the driving predecessor to "Wall Waterproofing} st lift" (BGSOX-4000)- in each area.</p> <p>- Bracing Removal- Level C" (BGSOX-4100) is the predecessor to "Wall Waterproofing- 2nd lift" (BGSOX-4110)- in each area</p> <p>- Bracing Removal- Level B" (BGSOX-6000) is the predecessor to "Wall Waterproofing- 3rd lift" (BGSOX-6010) in each area</p> <p>Based on the current schedule logic, the bracing will need to be modified to allow the removal of walers and struts in each area, separately and independently from each other. E.g: Any walers spanning two areas will need to be cut during removal of bracing so SCCI can proceed with the waterproofing install in that area, without having to wait for the adjacent area. This is applicable to Bracing Removal level B, C and D. As requested in RFI#233 response, please find attached bracings that SCCI assumes are going to be removed/ cut prior to SCCI's specific wall pour.</p> <p>Please confirm.</p>			<p>- Bracing Removal- Level D" (BGSOX-1120) is the driving predecessor to "Wall Waterproofing} st lift" (BGSOX-4000)- in each area.</p> <p>- Bracing Removal- Level C" (BGSOX-4100) is the predecessor to "Wall Waterproofing- 2nd lift" (BGSOX-4110)- in each area</p> <p>- Bracing Removal- Level B" (BGSOX-6000) is the predecessor to "Wall Waterproofing- 3rd lift" (BGSOX-6010) in each area</p> <p>Based on the current schedule logic, the bracing will need to be modified to allow the removal of walers and struts in each area, separately and independently from each other. E.g: Any walers spanning two areas will need to be cut during removal of bracing so SCCI can proceed with the waterproofing install in that area, without having to wait for the adjacent area. This is applicable to Bracing Removal level B, C and D. As requested in RFI#233 response, please find attached bracings that SCCI assumes are going to be removed/ cut prior to SCCI's specific wall pour.</p> <p>Please confirm.</p>			
SHIMM000-0242.1	BGP - 100% CD Phase 1 Documenation	Accepted	01	08/22/2013	09/01/2013	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The responses to SCCI's s RFI#0242/ WOJV RFI T-0633 refers to "100% CD Phase 1 Documentation" for the drawings that have not been issued in ASI#104 SCCI does not have access to and has not received the following drawings that are needed to finalize the pricing of ASI#104: A 1-2224-2231 A 1-2844-2846, 2848-2851 Please provide 1 00% CD Phase 1 Docs for the pages listed above			The responses to SCCI's s RFI#0242/ WOJV RFI T-0633 refers to "100% CD Phase 1 Documentation" for the drawings that have not been issued in ASI#104 SCCI does not have access to and has not received the following drawings that are needed to finalize the pricing of ASI#104: A 1-2224-2231 A 1-2844-2846, 2848-2851 Please provide 1 00% CD Phase 1 Docs for the pages listed above			



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SHIMM000-0252.1	BGP - Geothermal Loop Excavation in Zone 4  From: Shimmick Construction Company, Inc. Ben Gordon  <b>REQUEST:</b>  Reference: Spec Section 31 23 34  SCCI received the response to RFI-252 regarding the excavation of the geothermal loops in Zone 4. The response directed SCCI to conform to specification senction 31 23 34 regarding the ptential section 31 21 34 that cover buttress concrete demolition for the geothermal loops. SCCi is aware of the CDSM wall excavation required for the geothermal field risers, but is not aware of a geothermal specification requiring buttress shaft dempition for the geothermal loop trenches. Specificaion 31 23 34, Section 3.2 is very clear i the ful scope of the gound excavation in soild and wall riser excavation in CDSM, but it does not cover trenching in buttress shaft concrete.  Please advise.	Accepted	01	07/30/2013	08/09/2013	<b>ANSWER:</b>  Reference: Spec Section 31 23 34  SCCI received the response to RFI-252 regarding the excavation of the geothermal loops in Zone 4. The response directed SCCI to conform to specification senction 31 23 34 regarding the ptential section 31 21 34 that cover buttress concrete demolition for the geothermal loops. SCCi is aware of the CDSM wall excavation required for the geothermal field risers, but is not aware of a geothermal specification requiring buttress shaft dempition for the geothermal loop trenches. Specificaion 31 23 34, Section 3.2 is very clear i the ful scope of the gound excavation in soild and wall riser excavation in CDSM, but it does not cover trenching in buttress shaft concrete.  Please advise.
SHIMM000-0255	BGP - Plumbing Scope Clarification ASI 104  From: Shimmick Construction Company, Inc. Ben Gordon  <b>REQUEST:</b>  Reference: Drawing P1-6001, Spec Section 22 13 01  See attached marked up Rev 0 and Rev 1 Drawings P 1-6001. PI-6001 Rev 1 is a revision per AST 104. Rev 1 of the noted drawing does not have any "for reference only" notations in the details.  Is the intent of the Designers to significantly change the scope of TG06 work?  Please clarify the scope of work, i.e. applicable and non applicable details of the CD P1-6001 for the TG06 package.	Closed	01	07/26/2013	07/27/2013	07/26/2013  <b>ANSWER:</b>  Reference: Drawing P1-6001, Spec Section 22 13 01  See attached marked up Rev 0 and Rev 1 Drawings P 1-6001. PI-6001 Rev 1 is a revision per AST 104. Rev 1 of the noted drawing does not have any "for reference only" notations in the details.  Is the intent of the Designers to significantly change the scope of TG06 work?  Please clarify the scope of work, i.e. applicable and non applicable details of the CD P1-6001 for the TG06 package.
SHIMM000-0261	ASI#104- TG06's Scope Clarification  From: Shimmick Construction Company, Inc. Ben Gordon  <b>REQUEST:</b>  There are multiple changes between the Issued for	Closed	CR	07/26/2013	08/05/2013	07/26/2013  <b>ANSWER:</b>  There are multiple changes between the Issued for







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<b>SHIMM000-0264</b>	<b>BGP - Shear Wall Dowel and Shoring Pipe Bracing Conflict</b>	<b>Closed</b>	<b>01</b>	<b>07/24/2013</b>	<b>08/03/2013</b>	<b>08/07/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Drawing S1-3001, Spec Section 03 30 20			Reference: Drawing S1-3001, Spec Section 03 30 20			
A few potential conflicts exist between the typical shear wall vertical dowels and the 36" OD shoring Pipe Struts in Area 1. See attachment for locations of conflict.			A few potential conflicts exist between the typical shear wall vertical dowels and the 36" OD shoring Pipe Struts in Area 1. See attachment for locations of conflict.			
Based on Detail A shown in S1-3260, the typical shear wall verts will be lap spliced.			Based on Detail A shown in S1-3260, the typical shear wall verts will be lap spliced.			
Per the schedule in Detail 1-S1-3001, the #9 vertical shear wall reinforcement requires a 63" lap splice, which places the top of dowel at elevation -30'-5".			Per the schedule in Detail 1-S1-3001, the #9 vertical shear wall reinforcement requires a 63" lap splice, which places the top of dowel at elevation -30'-5".			
The centerline of Level D diagonal bracing atop Area 1 is shown to be at EL -29'-0" and the bottom of the 36" OD pipe strut at level D is at EL -30'-6".			The centerline of Level D diagonal bracing atop Area 1 is shown to be at EL -29'-0" and the bottom of the 36" OD pipe strut at level D is at EL -30'-6".			
The pipe strut will potentially encroach on the shear wall dowels since the vertical spacing is #9 at 10" OC.			The pipe strut will potentially encroach on the shear wall dowels since the vertical spacing is #9 at 10" OC.			
Please confirm that a 60" lap splice is acceptable at locations where conflicts exist, if not please provide solutions.			Please confirm that a 60" lap splice is acceptable at locations where conflicts exist, if not please provide solutions.			
<b>SHIMM000-0265</b>	<b>BGP Embedded Conduits in Mat Slab for the Light Column</b>	<b>Closed</b>	<b>01</b>	<b>07/24/2013</b>	<b>08/03/2013</b>	<b>08/02/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference attached drawing E1-2205 and E1-4105.			Please reference attached drawing E1-2205 and E1-4105.			
Per the attached lighting plan drawings, there are no electrical conduits shown to be embedded exclusively for the Light Column on drawing S1-6005.			Per the attached lighting plan drawings, there are no electrical conduits shown to be embedded exclusively for the Light Column on drawing S1-6005.			
Please confirm that there are no conduits required for the light column in both the concourse slab and mat slab or provide the location, route and size of the conduit at each level.			Please confirm that there are no conduits required for the light column in both the concourse slab and mat slab or provide the location, route and size of the conduit at each level.			
<b>SHIMM000-0266</b>	<b>BGP - Temporary Perimeter Lighting</b>	<b>Open</b>	<b>01</b>	<b>07/24/2013</b>	<b>08/03/2013</b>	<b>07/30/2013</b>







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	<p>Reference: Attached Drawings</p> <p>The highlighted areas on the attached re-shoring drawings show re-shoring struts against some of the oval shaped columns. In order to construct the concrete columns SCCI will need at least 30" of clearance between the column face and the struts.</p> <p>Please confirm that the reshoring struts will be moved enough to provide needed clearance.</p>					
SHIMM000-0269	BGP - 1st Street 48" Bridge Pile Asbuilts	Open	01	07/25/2013	08/04/2013	07/31/2013
	<p>From: Shimmick Construction Company, Inc. Ben Gordon</p> <p><b>REQUEST:</b></p> <p>Reference: Drawing S1-3003</p> <p>48" temporary bridge piles (00 1 through 010 in the drawing attached) under the 1st Street temporary bridge exceed the 48" diameter required per Detail 6 on Plan Sheet S 1-3003. The varying diameter of each temporary bridge pile is the result of the pile being a 48" CIDH concrete pile instead of a steel pile like the rest of the slab penetrations. As typical of a CIDH pile, the surface profile varies much greater than the 1 /2" gap tolerance required per Detail 6 on S 1-3003. Attached is an as built of the 48" piles with their varying diameters. In consequence the penetration sleeves will not fit the current conditions of the 48" piles.</p> <p>Please advise how to proceed.</p>					
	<p>Reference: Attached Drawings</p> <p>The highlighted areas on the attached re-shoring drawings show re-shoring struts against some of the oval shaped columns. In order to construct the concrete columns SCCI will need at least 30" of clearance between the column face and the struts.</p> <p>Please confirm that the reshoring struts will be moved enough to provide needed clearance.</p>					
	<p><b>ANSWER:</b></p> <p>Reference: Drawing S1-3003</p> <p>48" temporary bridge piles (00 1 through 010 in the drawing attached) under the 1st Street temporary bridge exceed the 48" diameter required per Detail 6 on Plan Sheet S 1-3003. The varying diameter of each temporary bridge pile is the result of the pile being a 48" CIDH concrete pile instead of a steel pile like the rest of the slab penetrations. As typical of a CIDH pile, the surface profile varies much greater than the 1 /2" gap tolerance required per Detail 6 on S 1-3003. Attached is an as built of the 48" piles with their varying diameters. In consequence the penetration sleeves will not fit the current conditions of the 48" piles.</p> <p>Please advise how to proceed.</p>					



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<b>SHIMM000-0270</b>	<b>BGP - Clear Cover to Mat Reinforcing at CDSM Pile Encroachment</b>  <b>From:</b> Shimmick Construction Company, Inc. Ben Gordon  <b>REQUEST:</b> Reference: Drawing S1-3201, Spec Section 03 30 20  Per Section 1 on S1-3201, the mat slab reinforcing is shown with 6" of clear cover from the outside face of the concrete wall. When the outside face wall and mat foundation step in and out due to CDSM encroachment, the 6" clear dimension shown on 1/S1-3201 will be encroached upon.  Please confirm this is acceptable. This would apply in any area where the wall thickness is being reduced due to encroaching CDSM Pile.	<b>Closed</b>	<b>01</b>	<b>07/30/2013</b>	<b>08/09/2013</b>	<b>08/07/2013</b>
	<b>ANSWER:</b> Reference: Drawing S1-3201, Spec Section 03 30 20  Per Section 1 on S1-3201, the mat slab reinforcing is shown with 6" of clear cover from the outside face of the concrete wall. When the outside face wall and mat foundation step in and out due to CDSM encroachment, the 6" clear dimension shown on 1/S1-3201 will be encroached upon.  Please confirm this is acceptable. This would apply in any area where the wall thickness is being reduced due to encroaching CDSM Pile.					
<b>SHIMM000-0272</b>	<b>bgp - Pin Pile Encroachment</b>  <b>From:</b> Shimmick Construction Company, Inc. Filip Filipic  <b>REQUEST:</b> See attached photo.  Pin pile No. 6 is encroaching into the future RCW. This RCW is not part of TG06 package, but the form savers for future walls are. With the pin pile in the way SCCI will not be able to install form savers in the area of encroachment.  Please advise.	<b>Accepted</b>	<b>CR</b>	<b>08/23/2013</b>	<b>09/02/2013</b>	
	<b>ANSWER:</b> See attached photo.  Pin pile No. 6 is encroaching into the future RCW. This RCW is not part of TG06 package, but the form savers for future walls are. With the pin pile in the way SCCI will not be able to install form savers in the area of encroachment.  Please advise.					
<b>SHIMM000-0274</b>	<b>BGP - Rebracing Conflict RKB 15</b>  <b>From:</b> Shimmick Construction Company, Inc. Filip Filipic  <b>REQUEST:</b> Reference: PSK-2022, Spec Section 22 13 01  Please see attached.  After performed layout of the drainage system in the mechanical room SCCI has discovered that the reshoring raker base plate of RKB#15 lands over the floor cleanout. Top of floor cleanout is supposed to be set to FFE (EL - 35.42) which is 3" above the top of mat slab. Floor cleanout at this location will be protruding into the raker's	<b>Open</b>	<b>01</b>	<b>08/15/2013</b>	<b>08/25/2013</b>	<b>08/15/2013</b>
	<b>ANSWER:</b> Reference: PSK-2022, Spec Section 22 13 01  Please see attached.  After performed layout of the drainage system in the mechanical room SCCI has discovered that the reshoring raker base plate of RKB#15 lands over the floor cleanout. Top of floor cleanout is supposed to be set to FFE (EL -35.42) which is 3" above the top of mat slab. Floor cleanout at this location will be					



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	base plate.  Please advise on how to proceed on this matter.					protruding into the raker's base plate.  Please advise on how to proceed on this matter.
<b>SHIMM000-0275</b>	<b>BGP - Rebracing Conflict RKB 16</b>  <b>From:</b> Shimmick Construction Company, Inc. Filip Filipic  <b>REQUEST:</b> Reference: PSK-2022, Spec Section 22 13 01  Please see attached.  After performed layout of the drainage system in the mechanical room SCCI has discovered that the reshoring raker base plate ofRKB#16 lands over the floor drain. Top of floor drain is supposed to be set to FFE (EL -35.42) which is 3" above the top of mat slab. Floor cleanout at this location will be protruding into the raker's base plate.  Please advise on how to proceed on this matter.	<b>Open</b>	<b>01</b>	<b>08/15/2013</b>	<b>08/25/2013</b>	<b>08/15/2013</b>  <b>ANSWER:</b> Reference: PSK-2022, Spec Section 22 13 01  Please see attached.  After performed layout of the drainage system in the mechanical room SCCI has discovered that the reshoring raker base plate ofRKB#16 lands over the floor drain. Top of floor drain is supposed to be set to FFE (EL -35.42) which is 3" above the top of mat slab. Floor cleanout at this location will be protruding into the raker's base plate.  Please advise on how to proceed on this matter.





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SHIMM000-0290	BGP - Couplers for Future Construction	Accepted	01	08/19/2013	08/29/2013	
<b>From:</b> Shimmick Construction Company, Inc. Filip Filipic						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference drawings: S1-3001, S1-3206			Reference drawings: S1-3001, S1-3206			
See attached photo of the form savers that are going to be used for the couplers for future construction as depicted on Detail 4 of S1-3206, and Detail 6 of S1-3001. SCCI believes that Detail 6 on S1-3001 is not applicable due to the following:			See attached photo of the form savers that are going to be used for the couplers for future construction as depicted on Detail 4 of S1-3206, and Detail 6 of S1-3001. SCCI believes that Detail 6 on S1-3001 is not applicable due to the following:			
1. As shown on the attached photo, epoxy coated form savers have tin cap incorporated into coupler's body. This tin cap will protect the rebar until the future construction.			1. As shown on the attached photo, epoxy coated form savers have tin cap incorporated into coupler's body. This tin cap will protect the rebar until the future construction.			
2. Whatever tar intended to be used with form savers is not compatible with the Grace waterproofing.			2. Whatever tar intended to be used with form savers is not compatible with the Grace waterproofing.			
3. Detail 6 on S1-3001 is a detail for the slabs, where future walls are to be constructed.			3. Detail 6 on S1-3001 is a detail for the slabs, where future walls are to be constructed.			
SCCI proposes to install the couplers for future construction as shown on Det. 4 S1-3206 with form savers set against the waterproofing membrane. Care shall be taken to ensure that waterproofing is not damaged.			SCCI proposes to install the couplers for future construction as shown on Det. 4 S1-3206 with form savers set against the waterproofing membrane. Care shall be taken to ensure that waterproofing is not damaged.			
Is this acceptable?			Is this acceptable?			



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<b>SHIMM000-0291</b>	<b>BGP - FF&amp;FL Values for Mat Slab and Concourse Slab</b>	<b>Accepted</b>	<b>01</b>	<b>08/23/2013</b>	<b>09/03/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Don Muns						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1. Please confirm the contract documetns (TG06.0) do not specify a FF value for the Mat Slab.			1. Please confirm the contract documetns (TG06.0) do not specify a FF value for the Mat Slab.			
2. Also, please reference ACI 302.1R and contract specification 033020.3.6.B. ACI 302.1R does not provide any recommendations on F-numbers for broomed surfaces. Furthermore, table 8.15.3.b of ACI 302.1R (page 46) demonstrates to achieve FF value of 20 for a slab on grade, it must be a smooth, floated surface.			2. Also, please reference ACI 302.1R and contract specification 033020.3.6.B. ACI 302.1R does not provide any recommendations on F-numbers for broomed surfaces. Furthermore, table 8.15.3.b of ACI 302.1R (page 46) demonstrates to achieve FF value of 20 for a slab on grade, it must be a smooth, floated surface.			
Please clarify if the designer intends to have a rough broom/rake finish, or intends to have the concourse slab finished to a value of 20.			Please clarify if the designer intends to have a rough broom/rake finish, or intends to have the concourse slab finished to a value of 20.			
3. Please confirm the concrete finish within the train box			3. Please confirm the concrete finish within the train box			
<b>SHIMM000-0292.1</b>	<b>Cast-In-Place Concrete - FF &amp; FL Values for Concourse Slab</b>	<b>Closed</b>	<b>CR</b>	<b>10/02/2013</b>	<b>10/12/2013</b>	<b>02/13/2014</b>
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
This RFI is being submitted in resposne to RFI response T-0691. Please refernce TG0600 contract specificaion section 033020.3.6.B. Section 3.6.B specifies a FF value of 20 for the surface of the lower concourse slab.			This RFI is being submitted in resposne to RFI response T-0691. Please refernce TG0600 contract specificaion section 033020.3.6.B. Section 3.6.B specifies a FF value of 20 for the surface of the lower concourse slab.			
Table 8.15.3b of ACI 302.1R (page 46) statres that to achieve a surface with an FF value of 20, it must be a smooth floated surface. ACI 302.1R does not provide any recommendations of "F" numbers for broomed surfaces.			Table 8.15.3b of ACI 302.1R (page 46) statres that to achieve a surface with an FF value of 20, it must be a smooth floated surface. ACI 302.1R does not provide any recommendations of "F" numbers for broomed surfaces.			
Please clarifiy if th edesigner intends to have a rough broom/rake finish, or intends to have the concourse slab finished to a FF value of 20.			Please clarifiy if th edesigner intends to have a rough broom/rake finish, or intends to have the concourse slab finished to a FF value of 20.			





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SHIMM000-0293	BGP - ASI-104 Electrical Clarifications	Closed	CR	08/22/2013	09/01/2013	08/22/2013
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Note B on the SKE-01-3201, SKE-01-3202, & SKE-02-3201(from RFI T-0633 response) indicate that all electrical equipment shown in halftone is to be included in the later phase 2 construction (outside of TG06.0 scope). With that, there is extensive electrical equipment (switch gear, panels, etc) that are shown in full tone on the drawings. Please clarify whether or not this electrical equipment is to be furnished and installed under the TG06.0 scope of work. Additionally, if it is required, please provide the specifications pertinent to the required equipment.			Note B on the SKE-01-3201, SKE-01-3202, & SKE-02-3201(from RFI T-0633 response) indicate that all electrical equipment shown in halftone is to be included in the later phase 2 construction (outside of TG06.0 scope). With that, there is extensive electrical equipment (switch gear, panels, etc) that are shown in full tone on the drawings. Please clarify whether or not this electrical equipment is to be furnished and installed under the TG06.0 scope of work. Additionally, if it is required, please provide the specifications pertinent to the required equipment.			
SHIMM000-0294	BGP - Rebar Configuration at Moment Beam with Incorporation of S-3 vs T-9 Ties	Accepted	01	08/23/2013	09/03/2013	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please find attached Gerdau's RFI#70.			Please find attached Gerdau's RFI#70.			
At the contractor's option, Gerdau would like to propose utilizing S-3 stirrups with only one T-9 tie (see attached sketch) for the vertical ties in the moment frame beam. This will be installed in lieu of installation all T-9 ties. This is done to avoid the constructability issues associated with alternating the hooks under the 1.5" of clear cover beneath the bottom beam bars.			At the contractor's option, Gerdau would like to propose utilizing S-3 stirrups with only one T-9 tie (see attached sketch) for the vertical ties in the moment frame beam. This will be installed in lieu of installation all T-9 ties. This is done to avoid the constructability issues associated with alternating the hooks under the 1.5" of clear cover beneath the bottom beam bars.			
Please confirm that the proposed reinforcing configuration is acceptable.			Please confirm that the proposed reinforcing configuration is acceptable.			
SHIMM000-0296	BGP - Drain Line and Micro Pile Conflict at K.5 5.5	Accepted	01	08/22/2013	08/30/2013	
<b>From:</b> Shimmick Construction Company, Inc. Filip Filipic						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached photo and CD PI-2030.			See attached photo and CD PI-2030.			
After performed layout of the drainage line system around GL K5 SCCI has discovered that a row of micro piles is in conflict with the 4" cast iron pipe drain line. SCCI suggest shifting the drain line run to clear the micro piles.			After performed layout of the drainage line system around GL K5 SCCI has discovered that a row of micro piles is in conflict with the 4" cast iron pipe drain line. SCCI suggest shifting the drain line run to clear the micro piles.			



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	Is this acceptable?					Is this acceptable?
SHIMM000-0297	BGP - Drain Line conflict with reinforcement at GL K3	Accepted	01	08/22/2013	08/30/2013	
From: Shimmick Construction Company, Inc. Filip Filipic						
REQUEST:			ANSWER:			
See attached photos and CD P 1-2030.			See attached photos and CD P 1-2030.			
Tails of the bottom rebar mat at the drainage pit are interfering with the construction of drainage lines and catch basin. SCCI proposes following: 1. Shift the catch basin to where it clears the reinforcement tails. 2. Cut the rebar tails to allow installation of the drainage lines and the catch basin.			Tails of the bottom rebar mat at the drainage pit are interfering with the construction of drainage lines and catch basin. SCCI proposes following: 1. Shift the catch basin to where it clears the reinforcement tails. 2. Cut the rebar tails to allow installation of the drainage lines and the catch basin.			
Please advise.			Please advise.			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
SHIMM000-0298	Additional Rebar Conflict for Plumbing Trim at GL2/D.4	Accepted	01	08/23/2013	09/03/2013	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Due to the density or the typical N-S top mat bars (#10) and additional bars (#11) near the elevator pit at Gridlines 2 and D.4, the additional trim rebar per 1/S1-3501 for interrupting the bars over the plumbing opening cannot be installed to the East of the plumbing opening within 3" of the opening. The alternative solution would be to install the additional steel in a new layer below the top mat; however, due to proximity of the piping to the steel the bars cannot be placed below the top mat. Gerdau proposes the folloing options:			Due to the density or the typical N-S top mat bars (#10) and additional bars (#11) near the elevator pit at Gridlines 2 and D.4, the additional trim rebar per 1/S1-3501 for interrupting the bars over the plumbing opening cannot be installed to the East of the plumbing opening within 3" of the opening. The alternative solution would be to install the additional steel in a new layer below the top mat; however, due to proximity of the piping to the steel the bars cannot be placed below the top mat. Gerdau proposes the folloing options:			
A. Omit the additional trim bars to the East of the trimmed opening.			A. Omit the additional trim bars to the East of the trimmed opening.			
B. Relocate the additional trim bars approximately 3'-0" East of the opening where the rebar spacing would allow for additional steel.			B. Relocate the additional trim bars approximately 3'-0" East of the opening where the rebar spacing would allow for additional steel.			
Please advise if proposed options are acceptable. (see attached SKS-1)			Please advise if proposed options are acceptable. (see attached SKS-1)			



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SHIMM000-0299	BGP - Additional Rebar Conflict for Floor Sink Trim GL B.7/2.7	Accepted	01	08/23/2013	09/02/2013	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached Gerdau's RFI#72			See attached Gerdau's RFI#72			
Due to the density of the typical N-S top mat bars (#10), additional N-S top mat bars (#11) and pin pile trim steel (#11 with lap splices directly over floor sink) near the floor sink at Gridlines 2.7 and B.7, the additional trim rebar per 1/S1-3501 for interrupting the bars over the plumbing opening cannot be installed on either side of the plumbing opening. The alternative solution would be to install the additional steel in a new layer below the top mat; however, due to the proximity of the plumbing piping to the steel the additional bars cannot be placed below the top mat. Also, the additional bar to the East of the opening would conflict with the pin pile. Gerdau proposes to cut top mat bars to allow for the floor sink installation and omit the additional trim bars.			Due to the density of the typical N-S top mat bars (#10), additional N-S top mat bars (#11) and pin pile trim steel (#11 with lap splices directly over floor sink) near the floor sink at Gridlines 2.7 and B.7, the additional trim rebar per 1/S1-3501 for interrupting the bars over the plumbing opening cannot be installed on either side of the plumbing opening. The alternative solution would be to install the additional steel in a new layer below the top mat; however, due to the proximity of the plumbing piping to the steel the additional bars cannot be placed below the top mat. Also, the additional bar to the East of the opening would conflict with the pin pile. Gerdau proposes to cut top mat bars to allow for the floor sink installation and omit the additional trim bars.			
Please advise if the proposed solution is acceptable.			Please advise if the proposed solution is acceptable.			
SHIMM000-0300	BGP - Glass Guard Rail Attachment by Others Clarification	Open	CR	09/12/2013	09/26/2013	09/20/2013
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Refer to drawing S1-3410.			Refer to drawing S1-3410.			
Please reference attached detail 7, S1-3410. Please confirm SCCI is to provide 3/8x7xcontinuous plate only, and no tthe tabs shown at 5'-0" OC.			Please reference attached detail 7, S1-3410. Please confirm SCCI is to provide 3/8x7xcontinuous plate only, and no tthe tabs shown at 5'-0" OC.			
SHIMM000-0301	BGP - Vehicle/Bike Beam End Suppoert Embed	Accepted	01	08/27/2013	09/09/2013	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference attached drawing S1-3411 .			Please reference attached drawing S1-3411 .			
Detail I calls for a W'x4"x 18" embed plate at the toe of the corbel. 1D/S1-3411 details this embed and shows it as 24" rather than 18".			Detail I calls for a W'x4"x 18" embed plate at the toe of the corbel. 1D/S1-3411 details this embed and shows it as 24" rather than 18".			
Please clarify the correct dimensions for this embed.			Please clarify the correct dimensions for this embed.			



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SHIMM000-0302	BGP - Catch Basin Requirements	Accepted	01	08/27/2013	09/06/2013	
<b>From:</b> Shimmick Construction Company, Inc. Filip Filipic						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached page from DBI's standard catch basin detail, and reference drawings P1-6001 and P1-2022 thru 2030.			See attached page from DBI's standard catch basin detail, and reference drawings P1-6001 and P1-2022 thru 2030.			
On 08/26/2013 during pressure testing inspection of the drainage lines in mat slab areas 1 and 2, DBI Plumbing Inspector has pointed out that all catch basins in the mat slab should be constructed per attached detail.			On 08/26/2013 during pressure testing inspection of the drainage lines in mat slab areas 1 and 2, DBI Plumbing Inspector has pointed out that all catch basins in the mat slab should be constructed per attached detail.			
Contract drawings do not show catch basins details with cleanouts, vents and trap primer connections. Constructing the catch basins per attached sheets constitutes a compensable change.			Contract drawings do not show catch basins details with cleanouts, vents and trap primer connections. Constructing the catch basins per attached sheets constitutes a compensable change.			
Please provide details and direction for construction of the catch basins.			Please provide details and direction for construction of the catch basins.			
SHIMM000-0303	BGP - Chamfer Bar Top Hook	Closed	01	08/29/2013	09/08/2013	08/29/2013
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached Gerdau's RFI#74. See attached SKS-74			See attached Gerdau's RFI#74. See attached SKS-74			
In an effort to prevent the chamfer bar from encroaching on the existing shoring waler beams, Gerdau would like to propose over bending the top hook and turning it into a standard 180 degree hook as shown on the attached sketch.			In an effort to prevent the chamfer bar from encroaching on the existing shoring waler beams, Gerdau would like to propose over bending the top hook and turning it into a standard 180 degree hook as shown on the attached sketch.			
Please advise if this is acceptable			Please advise if this is acceptable			
SHIMM000-0304	BGP - Drainage Conflicts with Reinforcement	Accepted	01	08/29/2013	09/08/2013	
<b>From:</b> Shimmick Construction Company, Inc. Filip Filipic						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached marked up contract drawings PSK-2022 and S1-3005			See attached marked up contract drawings PSK-2022 and S1-3005			
Some of the drainage lines and fixtures are designed to be			Some of the drainage lines and fixtures are designed			

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	<b>REQUEST:</b>  Reference Spec Section 033020.3.6.B.I.c. See attached photos for a visual reference.  Please reference TG06.0, BGP contract specifications 033020.3.6.B.I.c. SCCI is proposing to finish the top surface of the Mat foundation Slab, as a "Jitter Bug" finish. All other finishing requirements will remain the same.  Is this acceptable?					
	<b>ANSWER:</b>  Reference Spec Section 033020.3.6.B.I.c. See attached photos for a visual reference.  Please reference TG06.0, BGP contract specifications 033020.3.6.B.I.c. SCCI is proposing to finish the top surface of the Mat foundation Slab, as a "Jitter Bug" finish. All other finishing requirements will remain the same.  Is this acceptable?					
<b>SHIMM000-0308</b>	<b>BGP - Haunch Reinforcement Aternative Detail</b>	<b>Accepted</b>	<b>CR</b>	<b>08/30/2013</b>	<b>09/16/2013</b>	
	<b>From:</b> Shimmick Construction Company, Inc. Filip Filipic  <b>REQUEST:</b>  Reference Drawing: S1 -3201 Reference Spec: 03 20 00 Attached Gerdau Sketch: SKS-76.1, SKS-76.2, SKS-76.3  A portion of the #10 @ 8" haunch bars cannot be installed as fabricated due to conflicts with overhead obstructions (shoring walers and struts) and the dewatering well sleeves. Per discussions with Sean McNeil where bars cannot be installed due to the obstructions, a modified #1 0 haunch bar with an HRC 555 head can be installed in place of the typical haunch bar. The attached sketches (SKS-76.1 and SKS-76.2) depict the magnitude of the obstructions at the dewatering wells in Area 3.  Please confirm if this is acceptable.  Additionally, please provide the required embedment length for the headed tail of the modified haunch bar.					
	<b>ANSWER:</b>  Reference Drawing: S1 -3201 Reference Spec: 03 20 00 Attached Gerdau Sketch: SKS-76.1, SKS-76.2, SKS-76.3  A portion of the #10 @ 8" haunch bars cannot be installed as fabricated due to conflicts with overhead obstructions (shoring walers and struts) and the dewatering well sleeves. Per discussions with Sean McNeil where bars cannot be installed due to the obstructions, a modified #1 0 haunch bar with an HRC 555 head can be installed in place of the typical haunch bar. The attached sketches (SKS-76.1 and SKS-76.2) depict the magnitude of the obstructions at the dewatering wells in Area 3.  Please confirm if this is acceptable.  Additionally, please provide the required embedment length for the headed tail of the modified haunch bar.					



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SHIMM000-0308.1	BGP - Haunch Reinforcement Alternative Detail	Accepted	01	09/03/2013	09/13/2013	
From: Webcor Construction LP Jackson Tukuafu						
REQUEST: See attached Gerdau's RFI #79.  The RFI Response to RFI T -0702 stated that the 180 degree hook chamfer bars are acceptable where the bars conflict with the double shoring walers. The intent of the RFI was to request the use of the 180- degree hook for the chamfer bars throughout the structure regardless of whether or not the bars were below a double or single walers.  Please confirm that this is acceptable.		ANSWER: See attached Gerdau's RFI #79.  The RFI Response to RFI T -0702 stated that the 180 degree hook chamfer bars are acceptable where the bars conflict with the double shoring walers. The intent of the RFI was to request the use of the 180- degree hook for the chamfer bars throughout the structure regardless of whether or not the bars were below a double or single walers.  Please confirm that this is acceptable.				
SHIMM000-0309	BGP - Mat Slab Added Steel Interference	Accepted	01	08/31/2013	09/16/2013	
From: Shimmick Construction Company, Inc. Filip Filipic						
REQUEST: Reference Drawing S1-3003 and Spec Section 03 20 00 See attached Gerdau Sketch SK-77, BM-3b, BM-3t  Due to the location of select trestle and pin piles, the #9@16" (bottom mat) and #11@16" (top mat) added North-South layer reinforcement cannot be installed at the desired spacing. The proposed solution is to cut the added #9 or #11 bars, where interrupted by a pile, and add a hook of equal size or greater (#11 hook max) with a lap splice similar to the hooks used for the trestle and pin pile trim steel.  Please confirm if this is acceptable.		ANSWER: Reference Drawing S1-3003 and Spec Section 03 20 00 See attached Gerdau Sketch SK-77, BM-3b, BM-3t  Due to the location of select trestle and pin piles, the #9@16" (bottom mat) and #11@16" (top mat) added North-South layer reinforcement cannot be installed at the desired spacing. The proposed solution is to cut the added #9 or #11 bars, where interrupted by a pile, and add a hook of equal size or greater (#11 hook max) with a lap splice similar to the hooks used for the trestle and pin pile trim steel.  Please confirm if this is acceptable.				
SHIMM000-0310	BGP - Area 3- Partition Wall Pier Rebar Conflict With Plumbing Near GL3/C.3	Closed	01	09/03/2013	09/13/2013	
From: Shimmick Construction Company, Inc. Ben Gordon						
REQUEST: See attached Gerdau's RFI #078.  Near Gridlines 3/C.3, there is a conflict between the partition wall pier dowels and the installed 6" plumbing pipe (8" with insulation). The wall pier currently overlaps with the plumbing pipe by approximately 6". Gerdau proposes		ANSWER: See attached Gerdau's RFI #078.  Near Gridlines 3/C.3, there is a conflict between the partition wall pier dowels and the installed 6" plumbing pipe (8" with insulation). The wall pier currently overlaps with the plumbing pipe by approximately 6".				





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	<p>to move the wall pier to the East, or West to allow the dowels to clear the pipe.</p> <p>Please provide the acceptable direction (East or West) to shift the wall pier.</p> <p>Please note that there are conduits stub up on the East side that would need to be moved, should the opening is shifted towards the East.</p>					
SHIMM000-0311	<b>BGP - Couplers for Future Walls</b>  <b>From:</b> Shimmick Construction Company, Inc. Filip Filipic  <b>REQUEST:</b> Reference Det. 6 on S1-3001 See attached photo of the form savers that are going to be used as couplers for future walls.	Accepted	CR	09/03/2013	09/13/2013	<b>ANSWER:</b> Reference Det. 6 on S1-3001 See attached photo of the form savers that are going to be used as couplers for future walls.



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	<p>Due to the size of trestle pile, the adjustment of the shear head locations, as provided in RFI T-0703, cannot be achieved . Please provide direction on how to proceed.</p>					<p>shear reinforcement at gridlines G/15.</p> <p>Due to the size of trestle pile, the adjustment of the shear head locations, as provided in RFI T-0703, cannot be achieved . Please provide direction on how to proceed.</p>
<b>SHIMM000-0317</b>	<b>BGP - Trim Steel Requirements for Mat Slab</b>	<b>Open</b>	<b>CR</b>	<b>09/10/2013</b>	<b>09/20/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b> <p>Per field instructions, to help alleviate congestion in the mat reinforcing, and in particular, congestion resulting from add bars due to openings and penetrations, please confirm the following:</p> <p>1. Details 4 and 7 on Sheet S1-3009 in so far as they apply to trestle piles, pin piles, dewatering wells and piezometric pipes can be relaxed in terms of additional bars. For an even number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars)/2. For an odd number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars +1)/2.</p> <p>2. Detail 1 on Sheet S1-3501, which applies to sinks, can be relaxed in terms of additional bars. For an even number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars)/2. For an odd number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars +1)/2. The minimum requirement of 2 bars on either side of the opening need not apply.</p> <p>3. The number of bars and maintenance of clear spacing will take precedence over 8<math>\phi</math> or 4<math>\phi</math> module spacing as to minimize the number of potential bar interruptions (and minimize resulting add bars). Any bar may be displaced to avoid conflict. The maximum center-to-center spacing of any two adjacent bars may be as large as 16<math>\phi</math>. Clear spacing of 1 bar diameter shall be maintained between bars where bar relocation necessarily reduces spacing in the vicinity of relocation. Where bar relocation affects a lap splice, noncontact lap splices will be allowed up to 6<math>\phi</math> for</p>						<b>ANSWER:</b> <p>Per field instructions, to help alleviate congestion in the mat reinforcing, and in particular, congestion resulting from add bars due to openings and penetrations, please confirm the following:</p> <p>1. Details 4 and 7 on Sheet S1-3009 in so far as they apply to trestle piles, pin piles, dewatering wells and piezometric pipes can be relaxed in terms of additional bars. For an even number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars)/2. For an odd number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars +1)/2.</p> <p>2. Detail 1 on Sheet S1-3501, which applies to sinks, can be relaxed in terms of additional bars. For an even number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars)/2. For an odd number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars +1)/2. The minimum requirement of 2 bars on either side of the opening need not apply.</p> <p>3. The number of bars and maintenance of clear spacing will take precedence over 8<math>\phi</math> or 4<math>\phi</math> module spacing as to minimize the number of potential bar interruptions (and minimize resulting add bars). Any bar may be displaced to avoid conflict. The maximum center-to-center spacing of any two adjacent bars may be as large as 16<math>\phi</math>. Clear spacing of 1 bar diameter shall be maintained between bars where bar relocation</p>



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	<p>#10 and #11 bars. This remedy shall apply in particular when seeking to avoid interruptions at small penetrations such as risers, vents, sinks and conduits.</p> <p>4. Clear spacing of 1db minimum shall be maintained in all mat reinforcing except for contact lap splices.</p> <p>5. Measures to reduce congestion at other locations such as catch basins, sump pits, elevator pits, shoring bracing and bridge piers will be considered on a case-by-case basis during field coordination with Thornton Tomasetti's field representative.</p>					
	<p>necessarily reduces spacing in the vicinity of relocation. Where bar relocation affects a lap splice, noncontact lap splices will be allowed up to 6<sub>l</sub> for #10 and #11 bars. This remedy shall apply in particular when seeking to avoid interruptions at small penetrations such as risers, vents, sinks and conduits.</p> <p>4. Clear spacing of 1db minimum shall be maintained in all mat reinforcing except for contact lap splices.</p> <p>5. Measures to reduce congestion at other locations such as catch basins, sump pits, elevator pits, shoring bracing and bridge piers will be considered on a case-by-case basis during field coordination with Thornton Tomasetti's field representative.</p>					
SHIMM000-0318	BGP - Mat Slab CJ Layout Areas 2/4, 6	Accepted	CR	09/10/2013	09/20/2013	09/10/2013
	<p>From: Shimmick Construction Company, Inc. Ben Gordon</p> <p><b>REQUEST:</b></p> <p>Please reference the attached CJ layout drawing, CJ-04, regarding the proposed CJ layout for Areas 2/4 and Area 6. These changes are to eliminate conflict with diagonal pit rebar as well as micro piles. Please verify that these changes are acceptable.</p>					
	<p><b>ANSWER:</b></p> <p>Please reference the attached CJ layout drawing, CJ-04, regarding the proposed CJ layout for Areas 2/4 and Area 6. These changes are to eliminate conflict with diagonal pit rebar as well as micro piles. Please verify that these changes are acceptable.</p>					



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SHIMM000-0320	BGP - Sump Pump Conduit Terminations Between Grid Lines 1 & 12	Open	CR	09/12/2013	09/22/2013	
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per Detail 7 on plan sheet E1-6001, sump pump conduits for the below grade package are to be terminated 12" above the mat slab directly adjacent to the future train platform wall. With the train platform wall beginning at grid line 12 and moving east, where are the conduit terminations for the sumps to be installed west of grid line 12 where there is not a train platform? Is there a set dimension the conduit should be set away from the sump when the train platform is not present? Please advise. Please note that for the two sumps that have been poured			Per Detail 7 on plan sheet E1-6001, sump pump conduits for the below grade package are to be terminated 12" above the mat slab directly adjacent to the future train platform wall. With the train platform wall beginning at grid line 12 and moving east, where are the conduit terminations for the sumps to be installed west of grid line 12 where there is not a train platform? Is there a set dimension the conduit should be set away from the sump when the train platform is not present? Please advise. Please note that for the two sumps that have been poured			
in Area 3, the conduits were placed roughly 9' to the north of each sump opening to avoid the future train tracks. There are 8 total sumps west of grid line 12 with 6 of them left to be placed.			in Area 3, the conduits were placed roughly 9' to the north of each sump opening to avoid the future train tracks. There are 8 total sumps west of grid line 12 with 6 of them left to be placed.			
SHIMM000-0321	BGP - Pit Detail Near Grid E/34.5	Open	CR	09/17/2013	09/27/2013	
<b>From:</b> Webcor/Obayashi Joint Venture Bob Garcia						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The bridge pier pile (4'-0" diameter) near grid E/34.5 is shown in SI-2057 to be offset from the typical row of piles show along gridline 34.7. In addition, detail /SI-3007 depicts the pile being located within the pit that is located at gridline E/34.5. Based on field observations, it appears that the pile in question has been installed in line with the other piles on gridline 34.7 which could possibly result in the pile being outside of the pit.			The bridge pier pile (4'-0" diameter) near grid E/34.5 is shown in SI-2057 to be offset from the typical row of piles show along gridline 34.7. In addition, detail /SI-3007 depicts the pile being located within the pit that is located at gridline E/34.5. Based on field observations, it appears that the pile in question has been installed in line with the other piles on gridline 34.7 which could possibly result in the pile being outside of the pit.			
Please confirm if the pile is located within the pit as shown in S12057 and 1/SI-3007. If not, then please provide an alternative detail to 1/SI-3007.			Please confirm if the pile is located within the pit as shown in S12057 and 1/SI-3007. If not, then please provide an alternative detail to 1/SI-3007.			
SHIMM000-0322	BGP - Mat slab, Grade 75 #11 Reinforcing	Open	CR	09/17/2013	09/27/2013	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Due to mill shortages of grade 75 #10 reinforcing please confirm that at no cost to the Owner the implementation of grade 75 #11 reinforcing where required will be			Due to mill shortages of grade 75 #10 reinforcing please confirm that at no cost to the Owner the implementation of grade 75 #11 reinforcing where			



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	<p>acceptable for use within the typical mat reinforcing installed at 8" O.C.</p> <p>The use of the grade 75 # 11 rebar is expected to supplement the typical #1 0 bar in the following locations, 3rd and 4th layer of Area 6, and 4th layer of Area 7.</p>					
	<p>required will be acceptable for use within the typical mat reinforcing installed at 8" O.C.</p> <p>The use of the grade 75 # 11 rebar is expected to supplement the typical #1 0 bar in the following locations, 3rd and 4th layer of Area 6, and 4th layer of Area 7.</p>					
<b>SHIMM000-0323</b>	<b>BGP - Column C16 and Knock-Out Corbel - West Throat</b>	<b>Open</b>	<b>CR</b>	<b>09/17/2013</b>	<b>09/27/2013</b>	<b>09/18/2013</b>
<div><div><b>From:</b> Webcor/Obayashi Joint Venture</div><div>Bob Garcia</div></div>						
<b>REQUEST:</b> <p>Per previous discussion with TT field engineer, in the West throat shearwalls which contain integrated CI6 columns and vertical corbels to restrain the knock-out walls, only the CI6 column ties are required to penetrate the mat at the designated spacing for a distance of at least 12" below the lowest top mat elevation. The ties associated with the corbel are not required to penetrate the mat slab. This RFI confirms that the column and corbel ties, as placed, are acceptable based on the observation by the TT field engineer.</p>			<b>ANSWER:</b> <p>Per previous discussion with TT field engineer, in the West throat shearwalls which contain integrated CI6 columns and vertical corbels to restrain the knock-out walls, only the CI6 column ties are required to penetrate the mat at the designated spacing for a distance of at least 12" below the lowest top mat elevation. The ties associated with the corbel are not required to penetrate the mat slab. This RFI confirms that the column and corbel ties, as placed, are acceptable based on the observation by the TT field engineer.</p>			



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
SHIMM000-0324	BGP - Area 1- Confirming RFI- Knock Out Corbel and Haunch at SW Corner	Closed	CR	09/17/2013	09/27/2013	09/18/2013

From: Shimmick Construction Company, Inc. Ben Gordon

**REQUEST:**

Per field coordination with TT field engineer, please confirm it is acceptable to omit the pilaster ties of Detail 2/S1-3204 within the body of the haunch provided that:

- ¿ The pilaster West corner bar (Bar A in attached photo) is tied with 135 hooks in both directions
- ¿ Ties shall be #4 bars spaced at 4" o.c.
- ¿ The tie perpendicular to the South wall shall be developed a minimum of 14" into the South wall beyond the haunch.
- ¿ The tie parallel to the South wall shall be hooked around the pilaster East corner bar (Bar B in attached photo).
- ¿ In lieu of two individual ties, it is also acceptable to combine the ties into a single shape with a 90 degree bend at Bar A.
- ¿ The extent of the ties shall be from the top of the mat to the top of the haunch, after which Detail 2/S1-3204 will resume.
- ¿ The horizontal haunch bars shall terminate with a spliced matching hook.
- ¿ The horizontal formsaver bars for the future train tunnel shall be #7 @ 6" O.C. on the inside and outside face of the 3'-0" foundation wall.

**ANSWER:**

Per field coordination with TT field engineer, please confirm it is acceptable to omit the pilaster ties of Detail 2/S1-3204 within the body of the haunch provided that:

- ¿ The pilaster West corner bar (Bar A in attached photo) is tied with 135 hooks in both directions
- ¿ Ties shall be #4 bars spaced at 4" o.c.
- ¿ The tie perpendicular to the South wall shall be developed a minimum of 14" into the South wall beyond the haunch.
- ¿ The tie parallel to the South wall shall be hooked around the pilaster East corner bar (Bar B in attached photo).
- ¿ In lieu of two individual ties, it is also acceptable to combine the ties into a single shape with a 90 degree bend at Bar A.
- ¿ The extent of the ties shall be from the top of the mat to the top of the haunch, after which Detail 2/S1-3204 will resume.
- ¿ The horizontal haunch bars shall terminate with a spliced matching hook.
- ¿ The horizontal formsaver bars for the future train tunnel shall be #7 @ 6" O.C. on the inside and outside face of the 3'-0" foundation wall.





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
SHIMM000-0325	BGP - Area 6 CJ Layout Modifications	Open	CR	09/18/2013	09/28/2013	
<b>From:</b> Shimmick Construction Company, Inc. Filip Filipic						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached photos of the construction joint at mat slab area 6 South, near grid line 8.5, and CJ layout drawings.			See attached photos of the construction joint at mat slab area 6 South, near grid line 8.5, and CJ layout drawings.			
Due to congestion and access SCCI would like to shift the walls and concourse joints at this location 14.5" to the East. This adjustment does not affect any other structure's elements and complies with the CJ parameters outlined in the contract specifications.			Due to congestion and access SCCI would like to shift the walls and concourse joints at this location 14.5" to the East. This adjustment does not affect any other structure's elements and complies with the CJ parameters outlined in the contract specifications.			
Is this acceptable?			Is this acceptable?			
SHIMM000-0327	BGP - Area 6 East Bulkhead and Catch Basin Confl	Open	CR	09/20/2013	09/30/2013	
<b>From:</b> Shimmick Construction Company, Inc. Filip Filipic						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attachments.			See attachments.			
SCCI had to shift the construction joint between mat slab areas 6 and 7 Eastward due to the interference with the micropiles and trestle piles. This shift eCJ puts the bulkhead against the catch basin near GL G11.			SCCI had to shift the construction joint between mat slab areas 6 and 7 Eastward due to the interference with the micropiles and trestle piles. This shift eCJ puts the bulkhead against the catch basin near GL G11.			
in order to mitigate this conflict SCCI propose shifting the catch basin location 24" +/- (in East/Wet direction), on either side of the bulkhead/CJ.			in order to mitigate this conflict SCCI propose shifting the catch basin location 24" +/- (in East/Wet direction), on either side of the bulkhead/CJ.			
Is this acceptable.			Is this acceptable.			
SHIMM000-0328	BGP - Structural Slurry Primer in Mat Slab	Open	CR	09/24/2013	10/04/2013	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please refer to the attached letter, authored by Rober Foley (CEMEX QC), dated September 17, 2013.			Please refer to the attached letter, authored by Rober Foley (CEMEX QC), dated September 17, 2013.			
With limited site access, many Mat Slab pours will require a larger than normal amount of slick-line. To ensure that no slick-line gets plugged, SCCI is proposing to prime the slick-line with a structural slurry that will reach and exceed			With limited site access, many Mat Slab pours will require a larger than normal amount of slick-line. To ensure that no slick-line gets plugged, SCCI is proposing to prime the slick-line with a structural slurry			



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	<p>the specified design strength for the Mat Slab. A miniscule percentage of this primer will be deposited into the mat slab. This percentage would amount to .01 to .02 percent by volume.</p> <p>Please confirm the proposed SCCI method of slick-line priming is acceptable.</p>					
SHIMM000-0329	<b>BGP - Internal Bracing Level D Removal</b>  From: Webcor Construction LP Jackson Tukuafu	Open	CR	09/24/2013	10/10/2013	09/30/2013
	<p><b>REQUEST:</b></p> <p>Compiled concrete maturity data and break results from teh first mat slab pour show that after two weeks mat slab reaches between 4.5 and 5 KSI, this is approximeateley the end of the thermal control fo rthe mass concrete, as well.</p> <p>SCCI requests design team to allow TG03 Trade Contractor to remove level Dinterior bracing when mat slab concrete reaches 4.5 KSI.</p> <p>Is this acceptable.</p>					
						<p><b>ANSWER:</b></p> <p>Compiled concrete maturity data and break results from teh first mat slab pour show that after two weeks mat slab reaches between 4.5 and 5 KSI, this is approximeateley the end of the thermal control fo rthe mass concrete, as well.</p> <p>SCCI requests design team to allow TG03 Trade Contractor to remove level Dinterior bracing when mat slab concrete reaches 4.5 KSI.</p> <p>Is this acceptable.</p>
SHIMM000-0330	<b>BGP - Haunch Bar Grade and Size Increase</b>  From: Shimmick Construction Company, Inc. Ben Gordon	Open	CR	09/25/2013	10/05/2013	
	<p><b>REQUEST:</b></p> <p>Please confirm if it is acceptable to utilize Grade 75 #10 or #11 rebar in-lieu of the Grade 60 #10 rebar for the 3'-0" haunch.</p>					
						<p><b>ANSWER:</b></p> <p>Please confirm if it is acceptable to utilize Grade 75 #10 or #11 rebar in-lieu of the Grade 60 #10 rebar for the 3'-0" haunch.</p>



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SHIMM000-0331	BGP - Geothermal Fields 11, 12, & 13 Layout in Zone 4	Closed	CR	09/30/2013	10/10/2013	
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Attached are the two proposal drawings for the geothermal layout in zone 4. Please confirm which layout is acceptable, Option #1 or Option #2.			Attached are the two proposal drawings for the geothermal layout in zone 4. Please confirm which layout is acceptable, Option #1 or Option #2.			
SHIMM000-0332	BGP - Mat Slab Construction Joint Between Area 2 and Area 4	Closed	CR	10/01/2013	10/11/2013	
<b>From:</b> Shimmick Construction Company, Inc. Filip Filipic						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference TG0600-30.2 Submittal.			Reference TG0600-30.2 Submittal.			
As discussed in the prior progress meetings, SCCI plans to combine slab pours S102 and S104 into one pour without bulkhead forms in between. Is this acceptable?			As discussed in the prior progress meetings, SCCI plans to combine slab pours S102 and S104 into one pour without bulkhead forms in between. Is this acceptable?			
SHIMM000-0333	Loc. of Electrical Equipment and boxes for Elec. Room B2560	Open	CR	10/02/2013	10/02/2013	
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please find attached the revised layout for Electrical Room B2560 in Mat Slab Area 8. Please confirm that the layout is acceptable.			Please find attached the revised layout for Electrical Room B2560 in Mat Slab Area 8. Please confirm that the layout is acceptable.			
SHIMM000-0333.1	Loc. of Electrical Equipment and Boxes for Elec. Room B2560	Open	CR	10/28/2013	11/07/2013	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI #T-0782, drawing EI-2025, AI-2105, and Spec Section 26 05 34.			Please reference RFI #T-0782, drawing EI-2025, AI-2105, and Spec Section 26 05 34.			
RFI #T -0782 response proposes layout for electrical equipment and box layout in Electrical Room B2560 - Area I 0 in CAD format. See attached.			RFI #T -0782 response proposes layout for electrical equipment and box layout in Electrical Room B2560 - Area I 0 in CAD format. See attached.			
Please confirm that the layout is acceptable.			Please confirm that the layout is acceptable.			
SHIMM000-0333.2	BGP - Loc. of Electrical Equipment and Boxes for Elec. Room B2560	Closed	CR	11/25/2013	11/25/2013	





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<b>SHIMM000-0334.2</b>	<b>BGP - Loc. of Electrical Equipment and Boxes for Elec. Room B2441</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>11/25/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The response to RFI 0781.1 stated that the walls in the attached CAD layout were not properly coordinated and included an AAI mark-up. The AAI mark-up shows the incorrect gridline location for Electrical Room B2640. In addition, due to the fact that the walls at this location are knee walls with a 4" lip per 1/A1-9225 that was provided to SCCI in RFI T-0899 response received on 11/15; the walls submitted in RFI T-0781.1 are indeed coordinated correctly per the sketch dimensions (AAI sketch is based on platform drawing, not mat slab drawing room which SCCI based the layout from). This area will be included in the pour on 11/23/13 and the form savers and conduits have already been installed; there any layout changes incur additional costs.			The response to RFI 0781.1 stated that the walls in the attached CAD layout were not properly coordinated and included an AAI mark-up. The AAI mark-up shows the incorrect gridline location for Electrical Room B2640. In addition, due to the fact that the walls at this location are knee walls with a 4" lip per 1/A1-9225 that was provided to SCCI in RFI T-0899 response received on 11/15; the walls submitted in RFI T-0781.1 are indeed coordinated correctly per the sketch dimensions (AAI sketch is based on platform drawing, not mat slab drawing room which SCCI based the layout from). This area will be included in the pour on 11/23/13 and the form savers and conduits have already been installed; there any layout changes incur additional costs.			
Please confirm layout as shown is acceptable.			Please confirm layout as shown is acceptable.			
<b>SHIMM000-0335</b>	<b>Location of Electrical Equipment and Boxes for Electrical Room B2460</b>	<b>Open</b>	<b>CR</b>	<b>10/02/2013</b>	<b>10/12/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please find attached the revised layout for Electrical Room B2460 in Mat Slab Area 15. Please confirm that the layout is acceptable.			Please find attached the revised layout for Electrical Room B2460 in Mat Slab Area 15. Please confirm that the layout is acceptable.			
<b>SHIMM000-0335.1</b>	<b>Loc. of Electrical Equipment and Boxes for Elec. Room B2460</b>	<b>Open</b>	<b>CR</b>	<b>10/28/2013</b>	<b>11/07/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI #T-0780, drawings EI-2026 and AI-2104, and Spec Section 26 05 34.			Please reference RFI #T-0780, drawings EI-2026 and AI-2104, and Spec Section 26 05 34.			
RFI #T -0780 response proposes layout for electrical equipment and box layout in Electrical Room B2460 - Area 08 in CAD format. See attached.			RFI #T -0780 response proposes layout for electrical equipment and box layout in Electrical Room B2460 - Area 08 in CAD format. See attached.			
Please confirm that the layout is acceptable.			Please confirm that the layout is acceptable.			



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SHIMM000-0335.2	BGP - Loc. of Electrical Equipment and Boxes for Elec. Room B2460	Closed	CR	11/25/2013	12/05/2013	11/25/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:			ANSWER:			
The response to RFI 0780.1 stated that the walls in the attached CAD layout were not properly coordinated and included an AAI mark-up. The AAI mark-up shows the incorrect gridline location for Electrical Room B2640. In addition, due to the fact that the walls at this location are knee walls with a 4" lip per 1/A1-9225 that was provided to SCCI in RFI T-0899 response received on 11/15; the walls submitted in RFI T-0780.1 are indeed coordinated correctly per the sketch dimensions (AAI sketch is based on platform drawing, not mat slab drawing room which SCCI based the layout from). This area has already been poured with the form savers positioned per the CAD layout and as shown per ASI 107 Architectural drawings. Any changes in the layout of this area ill incur additional costs.			The response to RFI 0780.1 stated that the walls in the attached CAD layout were not properly coordinated and included an AAI mark-up. The AAI mark-up shows the incorrect gridline location for Electrical Room B2640. In addition, due to the fact that the walls at this location are knee walls with a 4" lip per 1/A1-9225 that was provided to SCCI in RFI T-0899 response received on 11/15; the walls submitted in RFI T-0780.1 are indeed coordinated correctly per the sketch dimensions (AAI sketch is based on platform drawing, not mat slab drawing room which SCCI based the layout from). This area has already been poured with the form savers positioned per the CAD layout and as shown per ASI 107 Architectural drawings. Any changes in the layout of this area ill incur additional costs.			
Please confirm layout as shown is acceptable.			Please confirm layout as shown is acceptable.			
SHIMM000-0336	Locations of Electrical Equipment and Boxes for Electrical Room B2461	Open	CR	10/02/2013	10/12/2013	
From: Shimmick Construction Company, Inc. Chris Williams						
REQUEST:			ANSWER:			
Please find attached the revised layout for Electrical Room B2461 in Mat Slab Area 8. Please confirm that the layout is acceptable.			Please find attached the revised layout for Electrical Room B2461 in Mat Slab Area 8. Please confirm that the layout is acceptable.			
SHIMM000-0336.1	Loc. of Electrical Equipment and Boxes for Elec_ Room B2461	Open	CR	10/28/2013	11/07/2013	
From: Shimmick Construction Company, Inc. Ben Gordon						
REQUEST:			ANSWER:			
Please reference RFI #T-0779, drawing EI-2024, and Spec Section 26 05 34.			Please reference RFI #T-0779, drawing EI-2024, and Spec Section 26 05 34.			
RFI #T - 0779 response proposes layout for electrical equipment and box layout in Electrical Room B2461 - Area 08 in CAD format. See attached.			RFI #T - 0779 response proposes layout for electrical equipment and box layout in Electrical Room B2461 - Area 08 in CAD format. See attached.			
Please confirm layout is acceptable.			Please confirm layout is acceptable.			



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<b>SHIMM000-0336.2</b>	<b>BGP - Loc. of Electrical Equipment and Boxes for Elec. Room B2461</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>11/25/2013</b>	<b>11/25/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The response to RFI 0779.1 stated that the walls in the attached CAD layout were not properly coordinated and included an AAI mark-up. The AAI mark-up shows the incorrect gridline location for Electrical Room B2640. In addition, due to the fact that the walls at this location are knee walls with a 4" lip per 1/A1-9225 that was provided to SCCI in RFI T-0899 response received on 11/15; the walls submitted in RFI T-0779.1 are indeed coordinated correctly per the sketch dimensions (AAI sketch is based on platform drawing, not mat slab drawing room which SCCI based the layout from). This area has already been poured with the form savers positioned per the CAD layout and as shown per ASI 107 Architectural drawings. Any changes in the layout of this area ill incur additional costs.			The response to RFI 0779.1 stated that the walls in the attached CAD layout were not properly coordinated and included an AAI mark-up. The AAI mark-up shows the incorrect gridline location for Electrical Room B2640. In addition, due to the fact that the walls at this location are knee walls with a 4" lip per 1/A1-9225 that was provided to SCCI in RFI T-0899 response received on 11/15; the walls submitted in RFI T-0779.1 are indeed coordinated correctly per the sketch dimensions (AAI sketch is based on platform drawing, not mat slab drawing room which SCCI based the layout from). This area has already been poured with the form savers positioned per the CAD layout and as shown per ASI 107 Architectural drawings. Any changes in the layout of this area ill incur additional costs.			
Please confirm layout as shown is acceptable.			Please confirm layout as shown is acceptable.			
<b>SHIMM000-0337</b>	<b>Loc. of Electrical Equipment and boxes for Elec. Room B2640</b>	<b>Open</b>	<b>CR</b>	<b>10/02/2013</b>	<b>10/12/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please find attached the revised layout for Electrical Room B2640 in Mat Slab Area 8. Please confirm that the layout is acceptable.			Please find attached the revised layout for Electrical Room B2640 in Mat Slab Area 8. Please confirm that the layout is acceptable.			
<b>SHIMM000-0337.2</b>	<b>BGP - Loc. of Electrical Equipment and Boxes for Elec. Room B2640</b>	<b>Closed</b>	<b>CR</b>	<b>11/19/2013</b>	<b>12/05/2013</b>	<b>11/25/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The response to RFI 0778.1 stated that the walls in the attached CAD layout were not properly coordinated and included an AAI mark-up. The AAI mark-up shows the incorrect gridline location for Electrical Room B2640. In addition, due to the fact that the walls at this location are knee walls with a 4" lip per 1/A1-9225 that was provided to SCCI in RFI T-0899 response received on 11/15; the walls submitted in RFI T-0778.1 are indeed coordinated correctly per the sketch dimensions (AAI sketch is based on platform drawing, not mat slab drawing room which			The response to RFI 0778.1 stated that the walls in the attached CAD layout were not properly coordinated and included an AAI mark-up. The AAI mark-up shows the incorrect gridline location for Electrical Room B2640. In addition, due to the fact that the walls at this location are knee walls with a 4" lip per 1/A1-9225 that was provided to SCCI in RFI T-0899 response received on 11/15; the walls submitted in RFI T-0778.1 are indeed coordinated correctly per the sketch dimensions (AAI sketch is based on			





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	SCCI based the layout from).  Please confirm layout as shown is acceptable.					platform drawing, not mat slab drawing room which SCCI based the layout from).  Please confirm layout as shown is acceptable.
<b>SHIMM000-0338</b>	<b>Clarification of Vehicle/Bike Beam End Supports</b>	<b>Open</b>	<b>CR</b>	<b>10/02/2013</b>	<b>10/12/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>  This RFI is being submitted in response to RFI resonse T-0453.1. Please confirm the Vehicle/Bike Ramp end support angles. Confirm the acute angle is 56 degrees and obtuse angle is 124 degrees.			<b>ANSWER:</b>  This RFI is being submitted in response to RFI resonse T-0453.1. Please confirm the Vehicle/Bike Ramp end support angles. Confirm the acute angle is 56 degrees and obtuse angle is 124 degrees.			
<b>SHIMM000-0339</b>	<b>Type C31/D22 Coupler Stagger</b>	<b>Open</b>	<b>CR</b>	<b>10/03/2013</b>	<b>10/03/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>  Detail 1/S1-3301 requires the couplers for the adjacent column vertical bars be staggered with a vertical distance of 24" or more; however, due to the pattern and spacing of vertical bars for the type C31/D22 detailed on S1-3306, the condition cannot be met.  Attached is a sketch of a proposed pattern for the vertical bars in the type C1/D22 columns, please confirm if it is acceptable.			<b>ANSWER:</b>  Detail 1/S1-3301 requires the couplers for the adjacent column vertical bars be staggered with a vertical distance of 24" or more; however, due to the pattern and spacing of vertical bars for the type C31/D22 detailed on S1-3306, the condition cannot be met.  Attached is a sketch of a proposed pattern for the vertical bars in the type C1/D22 columns, please confirm if it is acceptable.			







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SHIMM000-0342	Mat Slab S109 East Construction Joint Modifications	Open	CR	10/08/2013	10/18/2013	
	<p><b>From:</b> Shimmick Construction Company, Inc. Filip Filipic</p> <p><b>REQUEST:</b> See Attachments.</p> <p>After the layout of the East construction joint in the mat slab area 8, SCCI discovered several constructability issues with the mat keyway and other project structure elements.</p> <p>1. East construction joint in area 8 falls within the row of micropiles. For this area, SCCI intends to jog the joint 12" +/- to the East to clear the micropile conflict. 2. CJ at area 8 East runs thru the thickened slab section at GL 1.6G.3. In this area SCCI intends to shift the joint Eastward to capture the thickened section within the area 8 pour.</p> <p>Is this acceptable?</p>	<p><b>ANSWER:</b> See Attachments.</p> <p>After the layout of the East construction joint in the mat slab area 8, SCCI discovered several constructability issues with the mat keyway and other project structure elements.</p> <p>1. East construction joint in area 8 falls within the row of micropiles. For this area, SCCI intends to jog the joint 12" +/- to the East to clear the micropile conflict. 2. CJ at area 8 East runs thru the thickened slab section at GL 1.6G.3. In this area SCCI intends to shift the joint Eastward to capture the thickened section within the area 8 pour.</p> <p>Is this acceptable?</p>				
SHIMM000-0342	Mat Slab S109 East Construction Joint Modifications	Open	CR	10/08/2013	10/18/2013	
	<p><b>From:</b> Shimmick Construction Company, Inc. Filip Filipic</p> <p><b>REQUEST:</b> See Attachments.</p> <p>After the layout of the East construction joint in mat slab area 9 SCCI discovered several constructability issues with the mat keyway and other project structure elements.</p> <p>SCCI proposes to install the CJ between area 9 and 10 as shown on the attached sketches.</p> <p>Is this acceptable?</p>	<p><b>ANSWER:</b> See Attachments.</p> <p>After the layout of the East construction joint in mat slab area 9 SCCI discovered several constructability issues with the mat keyway and other project structure elements.</p> <p>SCCI proposes to install the CJ between area 9 and 10 as shown on the attached sketches.</p> <p>Is this acceptable?</p>				







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<b>SHIMM000-0349</b>	<b>Top Mat Reinforcement Placement Tolerance</b>	<b>Open</b>	<b>CR</b>	<b>10/10/2013</b>	<b>10/20/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: ACI 117.			Reference: ACI 117.			
Per discussions with TT Field Representative, please confirm if it is acceptable to increase the top mat slab reinforcement placement tolerance from +/-1/2" to +1/2" and -1". This would also change the concrete cover tolerance from -1/2" to +/-1/2".			Per discussions with TT Field Representative, please confirm if it is acceptable to increase the top mat slab reinforcement placement tolerance from +/-1/2" to +1/2" and -1". This would also change the concrete cover tolerance from -1/2" to +/-1/2".			
<b>SHIMM000-0350</b>	<b>Seismic Joint Drawing Discrepancies in Contract Drawings</b>	<b>Open</b>	<b>CR</b>	<b>10/14/2013</b>	<b>10/14/2013</b>	<b>10/14/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference detail 7/A1-8881 and 4/S1-3010 of the contract drawings.			Please reference detail 7/A1-8881 and 4/S1-3010 of the contract drawings.			
1. Detail 7/A1-881 shows several elements that are not shown on the structural drawing (highlighted in red). Please confirm these are required in the assembly and provide details for tabs, bolts and welds.			1. Detail 7/A1-881 shows several elements that are not shown on the structural drawing (highlighted in red). Please confirm these are required in the assembly and provide details for tabs, bolts and welds.			
2. The same detail depicts a "y" shaped object protruding from the seismic embed. What are these objects and what is their function?			2. The same detail depicts a "y" shaped object protruding from the seismic embed. What are these objects and what is their function?			
3. Detail 4/S1-3010 depicts a 3/4" diameter stud that is not shown on the Architectural drawings. Please clarify.			3. Detail 4/S1-3010 depicts a 3/4" diameter stud that is not shown on the Architectural drawings. Please clarify.			
<b>SHIMM000-0351</b>	<b>5/8"x6' Galvanized Steel Plate at Seismic Joint</b>	<b>Open</b>	<b>CR</b>	<b>10/14/2013</b>	<b>10/24/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Scott Bunnell						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference Detail 7/A1-8881 and 4/S1-3010 of the Contract Documents.			Please reference Detail 7/A1-8881 and 4/S1-3010 of the Contract Documents.			
1. Detail 7/A1-8881 (and other details on A1-8881) call for a 5/8"x6' galvanized steel plate secured to mud slab and soldier piles. This plate does not appear on the structural details for the seismic joint. What is the function of this plate?			1. Detail 7/A1-8881 (and other details on A1-8881) call for a 5/8"x6' galvanized steel plate secured to mud slab and soldier piles. This plate does not appear on the structural details for the seismic joint. What is the function of this plate?			



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	2. How is the plate secured ot the mud slab? How is it secured to the soldier beams? There does not appear to be access to weld directly to soldier beam.					2. How is the plate secured ot the mud slab? How is it secured to the soldier beams? There does not appear to be access to weld directly to soldier beam.
SHIMM000-0352	BGP - Temporary Power Route from Skid #5 to Zone #5	Closed	CR	10/16/2013	10/26/2013	10/16/2013
From: Shimmick Construction Company, Inc. Chris Williams						
REQUEST:			ANSWER:			
Please find attached a drawing of the proposed Temporary Power route from Skid #5 to Zone #5. Is this routing acceptable? Please advise.			Please find attached a drawing of the proposed Temporary Power route from Skid #5 to Zone #5. Is this routing acceptable? Please advise.			
SHIMM000-0353	U-Bar at CDSM Encroachment Near GL 16.9/J	Open	CR	10/17/2013	10/27/2013	
From: Shimmick Construction Company, Inc. Ben Gordon						
REQUEST:			ANSWER:			
Reference: RFI T-0742 - CDSM Soldier Pile Encroachment Area 9.			Reference: RFI T-0742 - CDSM Soldier Pile Encroachment Area 9.			
Per the response to RFI T-0742, the spacing of the verticals in the C21 embedded column at Gridlines 16.9/J was changed from 6" OC to 5" OC due to the CDSM soldier pile encroachment. As a result, there is an odd number (19) of verticals per layer which would leave one row of verticals to not be straddled by a U-bar. Gerdau proposes to widen the final U-bar in the embedded column and straddle 3 rows of vertical bars. See attached sketch for details. Please confirm if this is acceptable.			Per the response to RFI T-0742, the spacing of the verticals in the C21 embedded column at Gridlines 16.9/J was changed from 6" OC to 5" OC due to the CDSM soldier pile encroachment. As a result, there is an odd number (19) of verticals per layer which would leave one row of verticals to not be straddled by a U-bar. Gerdau proposes to widen the final U-bar in the embedded column and straddle 3 rows of vertical bars. See attached sketch for details. Please confirm if this is acceptable.			
SHIMM000-0354	BGP - Concourse Elevator Pit Sill Plates	Open	CR	10/16/2013	10/26/2013	
From: Shimmick Construction Company, Inc. Ben Gordon						
REQUEST:			ANSWER:			
Please reference A1-2824 through A1-2847 (BGP TG06.0 Contract Drawings). Please confirm all delvator it sill plates are not int he TG06.0 scope of work.			Please reference A1-2824 through A1-2847 (BGP TG06.0 Contract Drawings). Please confirm all delvator it sill plates are not int he TG06.0 scope of work.			













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	<p>Please see attached for observed conflicts (highlighted). Please confirm that the dimensions shown on the architectural plans at the slab openings are correct.</p>					
	<p>Please see attached for observed conflicts (highlighted). Please confirm that the dimensions shown on the architectural plans at the slab openings are correct.</p>					
<b>SHIMM000-0365</b>	<b>TG0600-103 Interior Wall Thickness Change Clarification</b>	<b>Open</b>	<b>CR</b>	<b>10/28/2013</b>	<b>11/07/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Submittal TG0600-0103 Per the submittal review notes for TG0600-0103, the train platform future interior wall thicknesses are increased in Areas 8 and 11. The reviewer has included a note "For 1'-4" walls use same coupler reinf as 14" walls. Coordinate with RFI T-0587." The note does not include 12" walls which were previously 10". Please confirm if the now 12" wall is to use the same coupler reinforcing as the 10" walls.			Reference: Submittal TG0600-0103 Per the submittal review notes for TG0600-0103, the train platform future interior wall thicknesses are increased in Areas 8 and 11. The reviewer has included a note "For 1'-4" walls use same coupler reinf as 14" walls. Coordinate with RFI T-0587." The note does not include 12" walls which were previously 10". Please confirm if the now 12" wall is to use the same coupler reinforcing as the 10" walls.			





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	architectural drawings.					attached architectural drawings.
SHIMM000-0369	Column Tie Change from T9 to T12  From: Shimmick Construction Company, Inc. Ben Gordon  REQUEST:  Please confirm if it is acceptable to replace the typical T9 column ties (90° or 135° bend on either end) with T12 ties (135° bends on both ends). See the attached sketch for further details.	Open	CR	11/05/2013	11/15/2013	ANSWER:  Please confirm if it is acceptable to replace the typical T9 column ties (90° or 135° bend on either end) with T12 ties (135° bends on both ends). See the attached sketch for further details.
SHIMM000-0374	BGP - Horizontal Cast-In Inserts: Walls 111,165, 164  From: Webcor Construction LP Claude Titcher  REQUEST:  1. Horizontal Cast-In inserts in 1st lift foundation wall 111 & 165, elevations -22.25, -27.08 and -31.92, were installed at elevations -22.08, -26.91 and -31.75 respectively. Please confirm this is acceptable? See attached sketch.  2. Horizontal Cast-In insert in 1st lift foundation wall 164, elevation -27.08, 13'-10" in length from East end of W164, was installed at elevation -27.20. Please confirm this is acceptable? Please note the remainder of the Cast-In insert in wall 164 was installed at elevation -27.08. See attached sketch.  Please note all other Horizontal Cast-In Inserts will be installed per approved comprehensive lift drawings.	Open	01	02/13/2014	02/13/2014	02/13/2014  ANSWER:  1. Horizontal Cast-In inserts in 1st lift foundation wall 111 & 165, elevations -22.25, -27.08 and -31.92, were installed at elevations -22.08, -26.91 and -31.75 respectively. Please confirm this is acceptable? See attached sketch.  2. Horizontal Cast-In insert in 1st lift foundation wall 164, elevation -27.08, 13'-10" in length from East end of W164, was installed at elevation -27.20. Please confirm this is acceptable? Please note the remainder of the Cast-In insert in wall 164 was installed at elevation -27.08. See attached sketch.  Please note all other Horizontal Cast-In Inserts will be installed per approved comprehensive lift drawings.



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SHIMM000-0376	BGP - ASI 107 - Concrete Curb and RCW - Concourse Level	Open	CR	11/11/2013	11/21/2013	11/11/2013
<div><div>From: Webcor Construction LP</div><div>Jackson Tukuafu</div></div>						
REQUEST:			ANSWER:			
<div>1. ASI #107 reissues A1-2222 to A1-2227 with changed note at the top right of page. Previously, CC= concrete curb were stated as "CC- Cone curb not in TG06". In ASI107, this note was revised to "Cone curb ref to A-00022 for cone curb schedule. Ref to structural dwgs for coupler details". Is it the intent to add the concrete curb scope into TG06 contract by the issuance of ASI 107?</div> <div>2. On the same changed note, RCW Previously stated "Reinf conc wall not in TG06 ref to structural dwgs". In ASI 107, this note is changed to RCW : "Reinf cone wall ref to structural dwgs". Is the intent to add the RCW scope into TG06 contract thru the issuance of ASI 107?</div>			<div>1. ASI #107 reissues A1-2222 to A1-2227 with changed note at the top right of page. Previously, CC= concrete curb were stated as "CC- Cone curb not in TG06". In ASI107, this note was revised to "Cone curb ref to A-00022 for cone curb schedule. Ref to structural dwgs for coupler details". Is it the intent to add the concrete curb scope into TG06 contract by the issuance of ASI 107?</div> <div>2. On the same changed note, RCW Previously stated "Reinf conc wall not in TG06 ref to structural dwgs". In ASI 107, this note is changed to RCW : "Reinf cone wall ref to structural dwgs". Is the intent to add the RCW scope into TG06 contract thru the issuance of ASI 107?</div>			



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SHIMM000-0377	ASI 107- Cone Curb and RCW- Concourse Level- Follow up to RFI SHIMM-00376	Open	CR	11/14/2013	11/24/2013	
<b>From:</b> Shimmick Construction Company, Inc. Sylvia Hartanto						
<b>REQUEST:</b>			<b>ANSWER:</b>			
SCCI is in receipt of response to RFI SHIM000-0376 in which WOJV requests SCCI to submit a cost proposal for revisions:1 Concrete curbs (CC) and 2. Reinforced Concrete Walls (RCW) as released in ASI 107. Please clarify the following: 1. SCCI to to price the construction of the concrete partitions (shown as 'ghost lines' in Architectural drawings) to the full height up to the ground level. This means that the construction of the concrete wall at concourse level cannot take place until level A bracing and Rebracing RA is taken out (after TG07.2 contractor build the ground level). 2. Since the RCW I concrete wall is now to be installed by TG06 contractor, dowels are to be installed (similar to platform rebar dowels in Area 3), instead offormsavers. Please confirm that this will not create inefficiency with TG06 or TG07 contractor. 3. 3.ASI 107 new notes on A1 -2222-A1-2231 state: "CC- Cone Curb- Ref to A-0022 for concrete curb schedule- refer to struct dwgs for coupler details." Please provide the most recent copy of A-0022. SCCI has not been able to locate concrete curb schedule in the most current A-0022 copy (IFC). Please provide curb reinforcing detail as well 4. Dwg AI-2222-AI-2223 also contain walls noted as "cone wall". Please confirm that SCCI is to treat these walls as RCW.			SCCI is in receipt of response to RFI SHIM000-0376 in which WOJV requests SCCI to submit a cost proposal for revisions:1 Concrete curbs (CC) and 2. Reinforced Concrete Walls (RCW) as released in ASI 107. Please clarify the following: 1. SCCI to to price the construction of the concrete partitions (shown as 'ghost lines' in Architectural drawings) to the full height up to the ground level. This means that the construction of the concrete wall at concourse level cannot take place until level A bracing and Rebracing RA is taken out (after TG07.2 contractor build the ground level). 2. Since the RCW I concrete wall is now to be installed by TG06 contractor, dowels are to be installed (similar to platform rebar dowels in Area 3), instead offormsavers. Please confirm that this will not create inefficiency with TG06 or TG07 contractor. 3. 3.ASI 107 new notes on A1 -2222-A1-2231 state: "CC- Cone Curb- Ref to A-0022 for concrete curb schedule- refer to struct dwgs for coupler details." Please provide the most recent copy of A-0022. SCCI has not been able to locate concrete curb schedule in the most current A-0022 copy (IFC). Please provide curb reinforcing detail as well 4. Dwg AI-2222-AI-2223 also contain walls noted as "cone wall". Please confirm that SCCI is to treat these walls as RCW.			



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<b>SHIMM000-0380</b>	<b>Seismic Joint Clarifications</b>	<b>Open</b>	<b>CR</b>	<b>11/14/2013</b>	<b>11/24/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Sylvia Hartanto						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference attached Details 7/AI-8881 (ASI #107) and 4/S1-301 0 (ASI #100).			Please reference attached Details 7/AI-8881 (ASI #107) and 4/S1-301 0 (ASI #100).			
1. Detail 7/A1-8881 calls for a "neoprene gasket compressed by bar and bolt typ". Please provide sizes for tabs and bolts. Also, provide welding instructions (if necessary).			1. Detail 7/A1-8881 calls for a "neoprene gasket compressed by bar and bolt typ". Please provide sizes for tabs and bolts. Also, provide welding instructions (if necessary).			
2. The same detail shows pipe penetrations through the seismic joint at both levels. Plumbing drawings show a 4" "SAN/ AD" running parallel to the seismic joint. Please confirm this pipe penetrates the joint. If so, provide locations off of grid and pipe sleeve dimensions. Also, provide details on how to seal this penetration (watertight).			2. The same detail shows pipe penetrations through the seismic joint at both levels. Plumbing drawings show a 4" "SAN/ AD" running parallel to the seismic joint. Please confirm this pipe penetrates the joint. If so, provide locations off of grid and pipe sleeve dimensions. Also, provide details on how to seal this penetration (watertight).			
3. Detail4/S1-301 0 shows a 3/4" Dia Headed Stud at 12" oc with 6" embed. Is this to be one row as the drawing shows?			3. Detail4/S1-301 0 shows a 3/4" Dia Headed Stud at 12" oc with 6" embed. Is this to be one row as the drawing shows?			
4. Detai14/S 1-3010 also calls for 4" diameter hole at 2'-0" oc. What is the purpose of these holes? If the clamping system is continuous, then what will support the rod at the hole locations? Please clarify.			4. Detai14/S 1-3010 also calls for 4" diameter hole at 2'-0" oc. What is the purpose of these holes? If the clamping system is continuous, then what will support the rod at the hole locations? Please clarify.			
<b>SHIMM000-0381</b>	<b>Seismic Joint Specification Clarifications</b>	<b>Open</b>	<b>CR</b>	<b>11/14/2013</b>	<b>11/24/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Sylvia Hartanto						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference Specifications Section 07 09 16 - 2.6.A.I. Section states "Provide joint assemblies in single lengths between changes in direction with vulcanized, mitered comers where joint changes directions or abuts other materials."			Please reference Specifications Section 07 09 16 - 2.6.A.I. Section states "Provide joint assemblies in single lengths between changes in direction with vulcanized, mitered comers where joint changes directions or abuts other materials."			
1. Please confirm that this is in reference to the Omega Seal gasket, and not the clamping system and embedded steel.			1. Please confirm that this is in reference to the Omega Seal gasket, and not the clamping system and embedded steel.			
2. Please confirm that it is acceptable to use clamping components with 4'-0" maximum lengths with butt joints not to exceed 1/8".			2. Please confirm that it is acceptable to use clamping components with 4'-0" maximum lengths with butt joints not to exceed 1/8".			
3. Please confirm that it is acceptable to use 14' max lengths on steel embed with butt joints not to exceed 1/8".			3. Please confirm that it is acceptable to use 14' max lengths on steel embed with butt joints not to exceed 1/8".			
<b>SHIMM000-0404</b>	<b>BGP - Geothermal Riser Pressure Gauge Location</b>	<b>Open</b>	<b>CR</b>	<b>12/20/2013</b>	<b>12/30/2013</b>	





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	<div><div>From: Shimmick Construction Company, Inc. Sylvia Hartanto</div><div>REQUEST:<div>Previous geothermal fields and risers had a "cat walk" behind the risers at grade. Additional pipe and 90s were added to bring the gauges up to grade to allow for pressure monitoring from this "catwalk." At fields 09-15 no cat walk exists, thus no location to access these gauges from.<div>Please provide the location for the geothermal riser gauges for inspection from Field 09 through Field 15.</div></div></div><div>ANSWER:<div>Previous geothermal fields and risers had a "cat walk" behind the risers at grade. Additional pipe and 90s were added to bring the gauges up to grade to allow for pressure monitoring from this "catwalk." At fields 09-15 no cat walk exists, thus no location to access these gauges from.<div>Please provide the location for the geothermal riser gauges for inspection from Field 09 through Field 15.</div></div></div></div>					
SHIMM000-0405	SCS - CDSM follow up question in response to RFI#T-1655 answer	Closed	01	11/06/2014	11/16/2014	11/06/2014
	<div><div>From: Shimmick Construction Company, Inc. Henry Chiang</div><div>REQUEST:<div>RPI #T-1655 SCS -0013 Response provided the following:<div>A. Confirmed notching depths with an updated sketch.</div><div>B. Referenced ASI 123 roadway at curb (low point) elevations to be used to calculate notching elevation.</div>Shimmick was the able to determine cut elevations for Minna Street from grid line 1 to grid line 18. CI-2001 shows extent of below grade train box and shoring wall at STA: 2+30.85. Using the same reference point on CI-4001 to determine the North West corner of the shoring wall in relation to roadway stationing Shimmick was then able to follow the south flow line proposed grade profile as the bottom of curb elevation for the south side of Minna Street elevations.<div><div>I. Please confirm that GL 1 correlates with STA: 2+32.85.</div><div>2. There isn't an equivalent elevation for Natoma Street (south shoring wall), Shimmick requests that reference point with stationing be provided and added to the C 1-4004 drawing.</div>Some areas lack a CI Flow Line Profile. Shimmick requests that a different reference point, such as top of ground level deck, be provided for<div><div>3. the west shoring wall from grid line A to X,</div><div>4. the south shoring wall from grid line X to start ofCI-4004 which is the first drawing sheet for Natoma Street, and</div><div>5. First street to Fremont for the south shoring wall.</div></div></div></div></div><div>ANSWER:<div>RPI #T-1655 SCS -0013 Response provided the following:<div>A. Confirmed notching depths with an updated sketch.</div><div>B. Referenced ASI 123 roadway at curb (low point) elevations to be used to calculate notching elevation.</div>Shimmick was the able to determine cut elevations for Minna Street from grid line 1 to grid line 18. CI-2001 shows extent of below grade train box and shoring wall at STA: 2+30.85. Using the same reference point on CI-4001 to determine the North West corner of the shoring wall in relation to roadway stationing Shimmick was then able to follow the south flow line proposed grade profile as the bottom of curb elevation for the south side of Minna Street elevations.<div><div>I. Please confirm that GL 1 correlates with STA: 2+32.85.</div><div>2. There isn't an equivalent elevation for Natoma Street (south shoring wall), Shimmick requests that reference point with stationing be provided and added to the C 1-4004 drawing.</div>Some areas lack a CI Flow Line Profile. Shimmick requests that a different reference point, such as top of ground level deck, be provided for<div><div>3. the west shoring wall from grid line A to X,</div><div>4. the south shoring wall from grid line X to start ofCI-4004 which is the first drawing sheet for Natoma Street, and</div><div>5. First street to Fremont for the south shoring wall.</div></div></div></div></div></div>					





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SHIMM000-0406	SCS - CDSM follow up question in response to RFI#T-1655 answer	Void	01	11/06/2014	11/16/2014	11/06/2014
<b>From:</b> Shimmick Construction Company, Inc. Henry Chiang						
<b>REQUEST:</b>			<b>ANSWER:</b>			
RPI #T-1655 SCS -0013 Response provided the following: A. Confirmed notching depths with an updated sketch. B. Referenced ASI 123 roadway at curb (low point) elevations to be used to calculate notching elevation. Shimmick was the able to determine cut elevations for Minna Street from grid line 1 to grid line 18. CI-2001 shows extent of below grade train box and shoring wall at STA: 2+30.85. Using the same reference point on CI-4001 to determine the North West corner of the shoring wall in relation to roadway stationing Shimmick was then able to follow the south flow line proposed grade profile as the bottom of curb elevation for the south side of Minna Street elevations. 1. Please confirm that GL 1 correlates with STA: 2+32.85. 2. There isn't an equivalent elevation for Natoma Street (south shoring wall), Shimmick requests that reference point with stationing be provided and added to the C 1-4004 drawing. Some areas lack a CI Flow Line Profile. Shimmick requests that a different reference point, such as top of ground level deck, be provided for 3. the west shoring wall from grid line A to X, 4. the south shoring wall from grid line X to start of CI-4004 which is the first drawing sheet for Natoma Street, and 5. First street to Fremont for the south shoring wall.			RPI #T-1655 SCS -0013 Response provided the following: A. Confirmed notching depths with an updated sketch. B. Referenced ASI 123 roadway at curb (low point) elevations to be used to calculate notching elevation. Shimmick was the able to determine cut elevations for Minna Street from grid line 1 to grid line 18. CI-2001 shows extent of below grade train box and shoring wall at STA: 2+30.85. Using the same reference point on CI-4001 to determine the North West corner of the shoring wall in relation to roadway stationing Shimmick was then able to follow the south flow line proposed grade profile as the bottom of curb elevation for the south side of Minna Street elevations. 1. Please confirm that GL 1 correlates with STA: 2+32.85. 2. There isn't an equivalent elevation for Natoma Street (south shoring wall), Shimmick requests that reference point with stationing be provided and added to the C 1-4004 drawing. Some areas lack a CI Flow Line Profile. Shimmick requests that a different reference point, such as top of ground level deck, be provided for 3. the west shoring wall from grid line A to X, 4. the south shoring wall from grid line X to start of CI-4004 which is the first drawing sheet for Natoma Street, and 5. First street to Fremont for the south shoring wall.			
SHIMM000-204.3	BGP - Locations of Electrical Outlets, Equipment, and Fixtures	Rejected	01	08/30/2013	09/09/2013	09/05/2013
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per the RFI response, please find attached the revised layout for the Electrical Room B2221. This revised layout shows the dimensions off of the interior walls as requested.  Please advise if it is acceptable.			Per the RFI response, please find attached the revised layout for the Electrical Room B2221. This revised layout shows the dimensions off of the interior walls as requested.  Please advise if it is acceptable.			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>SHIMM000-314.1</b>	<b>BGP - Embedded Conduits in Columns</b>	<b>Closed</b>	<b>01</b>	<b>09/04/2013</b>	<b>09/14/2013</b>	
<b>From:</b> Shimmick Construction Company, Inc. Chris Williams						
<b>REQUEST:</b>			<b>ANSWER:</b>			
In the MEP meeting on 9/4/13, the response to RFI T-0693 was clarified. To confirm conversations with the WSP Electrical Design representative, the only conduits to be embedded in columns per the RFI T-0693 response are to be fire management conduits per the locations depicted in the response. All other conduits (power recepticals etc) are to be stubbed up on the face of the columns and are not to be embedded in the column.			In the MEP meeting on 9/4/13, the response to RFI T-0693 was clarified. To confirm conversations with the WSP Electrical Design representative, the only conduits to be embedded in columns per the RFI T-0693 response are to be fire management conduits per the locations depicted in the response. All other conduits (power recepticals etc) are to be stubbed up on the face of the columns and are not to be embedded in the column.			
<b>SKAN000-385</b>	<b>SSS - Embedded Plate Scope Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>03/03/2014</b>	<b>03/13/2014</b>	<b>03/03/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1. Please confirm the embedded steel in the following details which is not connected to any TG07.1R steel is not in TG07.1R scope and will be supplied and installed by others: 4 & 5/S1-3002; 4 & 6/S1-3203; 2, 3, 6 & 7/S1-3205; 2/S1-3207; 3, 5 & 7/S1-3210; 4/S1-3281; 1/S1-3282; 1 & 10/S1-3411; 3 & 6/S1-3412; 6/S1-3502; 6/S1-3503; 2A/S1-3706; 4 & 7/S1-7604; 3 & 8/S1-7631; 5 & 9/S1-7660 and 6, 7 & 9/S1-9052.			1. Please confirm the embedded steel in the following details which is not connected to any TG07.1R steel is not in TG07.1R scope and will be supplied and installed by others: 4 & 5/S1-3002; 4 & 6/S1-3203; 2, 3, 6 & 7/S1-3205; 2/S1-3207; 3, 5 & 7/S1-3210; 4/S1-3281; 1/S1-3282; 1 & 10/S1-3411; 3 & 6/S1-3412; 6/S1-3502; 6/S1-3503; 2A/S1-3706; 4 & 7/S1-7604; 3 & 8/S1-7631; 5 & 9/S1-7660 and 6, 7 & 9/S1-9052.			
2. Please confirm the embedded steel in 8/S1-7602 which is clearly indicated below the scope delineation line is not in TG07.1R scope and will be supplied and installed by others. Skanska will field weld the 3/8" plate to the embed steel as indicated on see SK1.			2. Please confirm the embedded steel in 8/S1-7602 which is clearly indicated below the scope delineation line is not in TG07.1R scope and will be supplied and installed by others. Skanska will field weld the 3/8" plate to the embed steel as indicated on see SK1.			
3. Please confirm the embedded plate and angle in detail 1/S1-3705 will be supplied and installed by TG06 and Skanska will field weld the double angle connection to the embedded plate as indicated on SK2.			3. Please confirm the embedded plate and angle in detail 1/S1-3705 will be supplied and installed by TG06 and Skanska will field weld the double angle connection to the embedded plate as indicated on SK2.			
4. Details 9 & 11/S1-7600 indicate the top and bottom connections for stair 202 & 403 between the Train Platform Level and the bottom of the Lower Concourse slab. As the scope delineation line clearly shows the embedded plates will be supplied and installed by TG06. Once these embeds are poured in place the HSS post cannot be installed as detailed. Please confirm these HSS post will be supplied and installed by TG06 after the platform slab has been poured and before the Lower Concourse slab. See SK3 for clarification.			4. Details 9 & 11/S1-7600 indicate the top and bottom connections for stair 202 & 403 between the Train Platform Level and the bottom of the Lower Concourse slab. As the scope delineation line clearly shows the embedded plates will be supplied and installed by TG06. Once these embeds are poured in place the HSS post cannot be installed as detailed. Please confirm these HSS post will be supplied and installed by TG06 after the platform slab has been poured and before the Lower Concourse slab. See SK3 for clarification.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>SKAN000-385.1</b>	<b>SSS - Embedded Plate - Scope Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>03/25/2014</b>	<b>04/04/2014</b>	<b>03/25/2014</b>
<div><div><p><b>From:</b> Webcor Construction LP</p><p><b>Gregory Kemerer</b></p><p><b>REQUEST:</b></p><p>As per SK RFI 385 response, Skanska has the following comments on the details WO indicated are included in TG07.1R scope:</p><p>1) 3 &amp; 6/S1-3412: As no clear scope delineation line is indicated on these two details and the embeds are attached to TG07.1R steel, Skanska will provide the embedded plates for others to install as detailed on 3 &amp; 6/S1-3412 SK1.</p><p>2) 3 &amp; 8/S1-7631: Drawing S1-7102 partial plan at Roof Park level has not been issued to date. Therefore framing steel and decking at top of steel elevation 86' 1-1/4" was not included in our bid. Please provide this drawing and allocate a CO number for this work.</p><p>3) 8/S1-7602: Although the embedded plate is clearly indicated below the scope delineation line, for erection purposes Skanska will supply the angle with welded connection plate for TG06 to install as per SK2.</p><p>4) 1/S1-3705: As no clear scope delineation line is indicated and the embedded plate is attached to TG07.1R steel, Skanska will provide the embedded plate and angle for others to install as detailed as per SK3.</p><p>5) 9 &amp; 11/S1-7600: In RFI SK 385 Skanska questioned the scope and erectability of the HHS posts as detailed in 9&amp;11/S1-7600. The scope of the embedded angles was already confirmed by WO as not in TG07.1R scope in RFI T-1067 #6. As 11/S1-7600 occurs between GL1.4 &amp; 2 at the train platform level which has already been poured WO should verify this embed has been installed by TG06 as detailed.</p></div><div><p><b>ANSWER:</b></p><p>As per SK RFI 385 response, Skanska has the following comments on the details WO indicated are included in TG07.1R scope:</p><p>1) 3 &amp; 6/S1-3412: As no clear scope delineation line is indicated on these two details and the embeds are attached to TG07.1R steel, Skanska will provide the embedded plates for others to install as detailed on 3 &amp; 6/S1-3412 SK1.</p><p>2) 3 &amp; 8/S1-7631: Drawing S1-7102 partial plan at Roof Park level has not been issued to date. Therefore framing steel and decking at top of steel elevation 86' 1-1/4" was not included in our bid. Please provide this drawing and allocate a CO number for this work.</p><p>3) 8/S1-7602: Although the embedded plate is clearly indicated below the scope delineation line, for erection purposes Skanska will supply the angle with welded connection plate for TG06 to install as per SK2.</p><p>4) 1/S1-3705: As no clear scope delineation line is indicated and the embedded plate is attached to TG07.1R steel, Skanska will provide the embedded plate and angle for others to install as detailed as per SK3.</p><p>5) 9 &amp; 11/S1-7600: In RFI SK 385 Skanska questioned the scope and erectability of the HHS posts as detailed in 9&amp;11/S1-7600. The scope of the embedded angles was already confirmed by WO as not in TG07.1R scope in RFI T-1067 #6. As 11/S1-7600 occurs between GL1.4 &amp; 2 at the train platform level which has already been poured WO should verify this embed has been installed by TG06 as detailed.</p></div></div>						
<b>SKAN000-399</b>	<b>SSS - Non-Structural Steel Scope</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/17/2014</b>









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SKANS360-0001	test	Closed	CR	01/13/2014	01/23/2014	
<b>From:</b> Skanska USA Civil West California DisRyan Clayton						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 183.1 SK1A, SK1B, SK2A & SK2B for items 1 & 2: 1.) Confirm the elevator rail support connection with erection aids is acceptable as shown. 2.) Confirm the elevator rail support connection with erection aids is acceptable as shown.			See attached CD RFI # 183.1 SK1A, SK1B, SK2A & SK2B for items 1 & 2: 1.) Confirm the elevator rail support connection with erection aids is acceptable as shown. 2.) Confirm the elevator rail support connection with erection aids is acceptable as shown.			
T- 0851	BGP - Lower Concourse Shoring/Reshoring Calculation for Construction Live Load	Closed	CR	10/23/2013	11/02/2013	11/05/2013
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please refer to attached excerpt of Specification Section 03 10 00 - Concrete Formwork - Below Grade Package.  Per Article 3.2, Section A.2 of Spec Section 031000, the minimum construction live load design criteria for shoring and reshoring is 50 psf. The specification section is unclear whether the live load of 50psf is prior to or post concrete placement.  According to D.H. Charles (SCCI shoring designer), falsework projects of this application typically approach the falsework design for 50 psf before concrete is placed and 20 psf afterwards, while always maintaining a minimum design load (dead + live) of at least 100 psf. The attached list of of D.H. Charles project used the this same design approach. Falsework calculations are to follow.  Is the D.H. Charles design criteria acceptable?			Please refer to attached excerpt of Specification Section 03 10 00 - Concrete Formwork - Below Grade Package.  Per Article 3.2, Section A.2 of Spec Section 031000, the minimum construction live load design criteria for shoring and reshoring is 50 psf. The specification section is unclear whether the live load of 50psf is prior to or post concrete placement.  According to D.H. Charles (SCCI shoring designer), falsework projects of this application typically approach the falsework design for 50 psf before concrete is placed and 20 psf afterwards, while always maintaining a minimum design load (dead + live) of at least 100 psf. The attached list of of D.H. Charles project used the this same design approach. Falsework calculations are to follow.  Is the D.H. Charles design criteria acceptable?			
T-0001	Article 6 Changes in Work - Clarification	Closed	CR	10/11/2010	10/25/2010	11/03/2010
<b>From:</b> Webcor Construction LP Joanne Filipas						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Spec Section 00 07 00, Article 6 - Clarifications and Changes in Work  Article 6 in the General Condition specification section 00 07 00 defines the procedure for changes in work. The			Reference: Spec Section 00 07 00, Article 6 - Clarifications and Changes in Work  Article 6 in the General Condition specification section 00 07 00 defines the procedure for changes in work.			





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	<p>procedures defined throughout Article 6 are conflicting. According to section 6.01.A, CM/GC shall promptly comply and proceed with changes issued by the TJPA in the form of a Change Order or Field Order. Section 6.02.B states that the TJPA will respond to RFI's with written Clarification deemed necessary and consistent with the Contract Documents or a Field Order requiring minor changes in work. Per section 6.01.A, the CM/GC is to proceed with the Field Order immediately. However, according to section 6.03.A, CM/GC shall submit a Change Order Request within 21 days of written directive. Please advise if the CM/GC is to proceed with changes promptly and prior to approval or if the CM/GC shall receive approval prior to proceeding with any changed Work.</p>					
	<p>The procedures defined throughout Article 6 are conflicting. According to section 6.01.A, CM/GC shall promptly comply and proceed with changes issued by the TJPA in the form of a Change Order or Field Order. Section 6.02.B states that the TJPA will respond to RFI's with written Clarification deemed necessary and consistent with the Contract Documents or a Field Order requiring minor changes in work. Per section 6.01.A, the CM/GC is to proceed with the Field Order immediately. However, according to section 6.03.A, CM/GC shall submit a Change Order Request within 21 days of written directive. Please advise if the CM/GC is to proceed with changes promptly and prior to approval or if the CM/GC shall receive approval prior to proceeding with any changed Work.</p>					
<b>T-0002</b>	<b>Transit Center Building Address Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>10/20/2010</b>	<b>11/03/2010</b>	<b>10/28/2010</b>
	<p><b>From:</b> Webcor/Obayashi Joint Venture      Joanne Filipas</p> <p><b>REQUEST:</b></p> <p>Please clarify the building address for the Transbay Transit Center. This is required to complete our site specific Click Safety program, complete insurance documents, etc.</p>					
	<p><b>ANSWER:</b></p> <p>Please clarify the building address for the Transbay Transit Center. This is required to complete our site specific Click Safety program, complete insurance documents, etc.</p>					





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<b>T-0003</b>	<b>301 Mission Wall Specification Format</b>	<b>Closed</b>	<b>CR</b>	<b>11/17/2010</b>	<b>12/01/2010</b>	<b>11/23/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference Sheet: C-0001 issued 11/04/10; 301 Mission Interim Screen Wall - General Notes  We are in the process of preparing submittals for this project. In doing so we would like to know what specification division format would be most appropriate for us to submit and track these project documents. Please provide us with the desired specification division format as soon as possible so that our submittals can be processed with the proper efficiency.						<b>ANSWER:</b>  Reference Sheet: C-0001 issued 11/04/10; 301 Mission Interim Screen Wall - General Notes  We are in the process of preparing submittals for this project. In doing so we would like to know what specification division format would be most appropriate for us to submit and track these project documents. Please provide us with the desired specification division format as soon as possible so that our submittals can be processed with the proper efficiency.
<b>T-0004</b>	<b>Transbay Project Signs</b>	<b>Closed</b>	<b>CR</b>	<b>12/01/2010</b>	<b>12/15/2010</b>	<b>12/03/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Spec Section: 01 15 01  Webcor/Obayashi is initiating project sign procurement per Spec 01 15 01 and will require the artwork and locations for four 4x8 post mounted signs. What are required graphics/logo's for sign fabrication and where shall each sign be located.						<b>ANSWER:</b>  Spec Section: 01 15 01  Webcor/Obayashi is initiating project sign procurement per Spec 01 15 01 and will require the artwork and locations for four 4x8 post mounted signs. What are required graphics/logo's for sign fabrication and where shall each sign be located.
<b>T-0004.1</b>	<b>Transbay Project Signs</b>	<b>Closed</b>	<b>CR</b>	<b>04/01/2011</b>	<b>04/11/2011</b>	<b>04/12/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: RFI T-0004 Spec Section: 01 15 01  Response to RFI T-0004 read "Graphics for Project ID Signs specified per 01 15 01 will be issued to CMGC as soon as the names for mayor and SFCTA Board members are confirmed in early January, 2011. Information for locations will be issued prior to installation."  In a follow up to this RFI, Webcor/Obayashi's is initiating project sign procurement and will require the artwork and locations for four 4x8 post mounted signs. What are required graphics/logo's for sign fabrication and where						<b>ANSWER:</b>  Reference: RFI T-0004 Spec Section: 01 15 01  Response to RFI T-0004 read "Graphics for Project ID Signs specified per 01 15 01 will be issued to CMGC as soon as the names for mayor and SFCTA Board members are confirmed in early January, 2011. Information for locations will be issued prior to installation."  In a follow up to this RFI, Webcor/Obayashi's is initiating project sign procurement and will require the artwork and locations for four 4x8 post mounted signs.



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	shall each sign be located.				What are required graphics/logo's for sign fabrication and where shall each sign be located.	
<b>T-0005</b>	<b>Incorporation of Trade Subcontractor Schedule Submittals</b>	<b>Closed</b>	<b>CR</b>	<b>12/03/2010</b>	<b>12/13/2010</b>	<b>12/07/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Jim Tomaszewski						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Spec Section: 01 13 10 & 01 1310			Spec Section: 01 13 10 & 01 1310			
For TJPA convenience W/O requests that Trade Subcontractor Schedules (Section 01 13 10, 1.2.B) be incorporated into the Monthly Schedule Report (Section 01 13 10, 1.5.A) for the month following issuance of NTP for the specified trade package. A detailed section of the Narrative will be clearly identified and contain all of the narrative requirements of Section 01 13 10, 1.2.B.			For TJPA convenience W/O requests that Trade Subcontractor Schedules (Section 01 13 10, 1.2.B) be incorporated into the Monthly Schedule Report (Section 01 13 10, 1.5.A) for the month following issuance of NTP for the specified trade package. A detailed section of the Narrative will be clearly identified and contain all of the narrative requirements of Section 01 13 10, 1.2.B.			



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T-0006	301 Mission Wall Plywood Wall Barrier Proposal	Closed	CR	12/08/2010	12/18/2010	12/17/2010
From: Webcor Construction LP      David Hungerford						
REQUEST:		ANSWER:				
Reference: C-5000 and attached sketch		Reference: C-5000 and attached sketch				
During the Fremont Shoring/301 Mission Wall Coordination Meeting on 12-7-10, it was proposed that a plywood barrier wall be erected in lieu of the triton barrier shown on sheet C-5000 of the 301 Mission Street Interim Screen Wall drawings. This plywood barrier will block the view of the 301 Mission tenants and will allow for the early demolition of the existing screen wall (prior to the construction of the new "interim" screen wall). By doing this it will enable the demolition contractor to start the removal of the deep footings earlier than currently scheduled.		During the Fremont Shoring/301 Mission Wall Coordination Meeting on 12-7-10, it was proposed that a plywood barrier wall be erected in lieu of the triton barrier shown on sheet C-5000 of the 301 Mission Street Interim Screen Wall drawings. This plywood barrier will block the view of the 301 Mission tenants and will allow for the early demolition of the existing screen wall (prior to the construction of the new "interim" screen wall). By doing this it will enable the demolition contractor to start the removal of the deep footings earlier than currently scheduled.				
In addition, the deletion of the triton barrier will provide approximately 2' of additional driveway width for 301 Mission. Please review the attached preliminary sketch of the above mentioned plywood barrier and provide engineering/architectural comments and mark ups.		In addition, the deletion of the triton barrier will provide approximately 2' of additional driveway width for 301 Mission. Please review the attached preliminary sketch of the above mentioned plywood barrier and provide engineering/architectural comments and mark ups.				
T-0007	Field Order #2 - Issued for Programwide	Closed	CR	12/08/2010	12/18/2010	12/13/2010
From: Webcor Construction LP      Joanne Filipas						
REQUEST:		ANSWER:				
According to today's OAC meeting, the documents issued with FO#W0-002 are intended for project-wide review and not exclusively for the "BSE Contract" as stated in the Field Order. Please confirm.		According to today's OAC meeting, the documents issued with FO#W0-002 are intended for project-wide review and not exclusively for the "BSE Contract" as stated in the Field Order. Please confirm.				
T-0008	Specification Section 00 04 82 Cert. of Bidder Regarding Debarment and Suspensi Closed		CR	12/08/2010	12/18/2010	12/10/2010
From: Webcor Construction LP      Joanne Filipas						
REQUEST:		ANSWER:				
Per the TJPA, specification section 00 04 82, Certification of Bidder Regarding Debarment and Suspension, shall no longer be used. Please confirm.		Per the TJPA, specification section 00 04 82, Certification of Bidder Regarding Debarment and Suspension, shall no longer be used. Please confirm.				
If this is in fact true, please confirm this section will be removed from the project specifications.		If this is in fact true, please confirm this section will be				



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removed from the project specifications.						
<b>T-0009</b>	<b>301 Mission Wall Storage Location for Planter Boxes of 301 Mission Wall</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2010</b>	<b>12/20/2010</b>	<b>12/13/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: 301 Mission Interim Screen Wall Drawings Sheet C-1000  On sheet C-1000, there is a note for the (E) Planter boxes that says "(e) precast planter box (typ) to be remove and stored". Please designate a location for storing the (E) planter boxes.		<b>ANSWER:</b>  Reference: 301 Mission Interim Screen Wall Drawings Sheet C-1000  On sheet C-1000, there is a note for the (E) Planter boxes that says "(e) precast planter box (typ) to be remove and stored". Please designate a location for storing the (E) planter boxes.				
<b>T-0009.1</b>	<b>301 Mission Wall Storage Location for Planter Boxes of 301 Mission Wall</b>	<b>Closed</b>	<b>CR</b>	<b>12/17/2010</b>	<b>12/27/2010</b>	<b>12/29/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  In Transworld's review of the existing planter box condition at the 301 Mission Screen Wall, Transworld's viewpoint after close inspection of the site is that the planter boxes were originally installed with the intent of being permanent fixtures. There are connection points for these planter boxes that appear to be initial anchor points for original placement of these fixtures and there is concern that these planter boxes were never intended to be reinstalled/relocated after the initial installation. With all do skill and care, Transworld intends to relocate these planter boxes with mininmal damage. As a point of advisement, since these boxes do not appear to be designed for relocation, Transworld is concerned that such action will render these boxes unuseful. Please confirm that the design is to relocate these boxes in lieu of replacing them with new ones.		<b>ANSWER:</b>  In Transworld's review of the existing planter box condition at the 301 Mission Screen Wall, Transworld's viewpoint after close inspection of the site is that the planter boxes were originally installed with the intent of being permanent fixtures. There are connection points for these planter boxes that appear to be initial anchor points for original placement of these fixtures and there is concern that these planter boxes were never intended to be reinstalled/relocated after the initial installation. With all do skill and care, Transworld intends to relocate these planter boxes with mininmal damage. As a point of advisement, since these boxes do not appear to be designed for relocation, Transworld is concerned that such action will render these boxes unuseful. Please confirm that the design is to relocate these boxes in lieu of replacing them with new ones.				
<b>T-0010</b>	<b>EPA Permit Number</b>	<b>Closed</b>	<b>CR</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>12/16/2010</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						





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<b>T-0013</b>	<b>BSE IFC Table of Contents Discrepancy</b>	<b>Closed</b>	<b>01</b>	<b>01/05/2011</b>	<b>01/15/2011</b>	<b>01/11/2011</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b>  Ref IFC TOC dated 12/15/10 (attached)  We have received the revised Issued for Construction (IFC) drawings and specifications for the BSE package. The table of contents has check marks to indicate added specification sections. Specification section 02 41 19, Pile Removal is not noted with a check mark but a revised specification was issued. The excavation and backfill (31 23 10) section was not re-issued, however, a check mark is next to it.  Also, the revision logs at the end of each section need to be revised to show only the revision number and dates.  Please advise and re-issue.		<b>ANSWER:</b>  Ref IFC TOC dated 12/15/10 (attached)  We have received the revised Issued for Construction (IFC) drawings and specifications for the BSE package. The table of contents has check marks to indicate added specification sections. Specification section 02 41 19, Pile Removal is not noted with a check mark but a revised specification was issued. The excavation and backfill (31 23 10) section was not re-issued, however, a check mark is next to it.  Also, the revision logs at the end of each section need to be revised to show only the revision number and dates.  Please advise and re-issue.				
<b>T-0014</b>	<b>TG03 BSE IFC Drawing Set</b>	<b>Closed</b>	<b>01</b>	<b>01/06/2011</b>	<b>01/16/2011</b>	<b>01/07/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Masashi Kojima						
<b>REQUEST:</b>  We received multiple versions of PDF Drawings G-0000, A-0000, A-0005, and A-0010 (see the attached images) for TG03 IFC Drawing Set. Please confirm the following answer from PMPC via email on 1/5/2011. "Use the 1/3/2011 CD for the PDF files. Use the 1/4/2011 CD for the DWG and DWF files. Disregard the PDFs on the 1/4/2011 CD."		<b>ANSWER:</b>  We received multiple versions of PDF Drawings G-0000, A-0000, A-0005, and A-0010 (see the attached images) for TG03 IFC Drawing Set. Please confirm the following answer from PMPC via email on 1/5/2011. "Use the 1/3/2011 CD for the PDF files. Use the 1/4/2011 CD for the DWG and DWF files. Disregard the PDFs on the 1/4/2011 CD."				
<b>T-0015</b>	<b>301 Mission Wall - Concrete Mix Design</b>	<b>Closed</b>	<b>CR</b>	<b>01/07/2011</b>	<b>01/17/2011</b>	<b>01/13/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: Attached submittal package TG1901-001 review comments and letter from concrete supplier  Per the comments received on the concrete mix design submitted in submittal package TG1901-001, please confirm that the admixture for air entrainment shall be compliant with ASTM C260.		<b>ANSWER:</b>  Reference: Attached submittal package TG1901-001 review comments and letter from concrete supplier  Per the comments received on the concrete mix design submitted in submittal package TG1901-001, please confirm that the admixture for air entrainment shall be compliant with ASTM C260.				



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0016</b>	<b>BSE - Current Trainbox Structural Drawings</b>	<b>Closed</b>	<b>01</b>	<b>01/14/2011</b>	<b>01/24/2011</b>	<b>01/18/2011</b>
	<b>From:</b> Webcor/Obayashi Joint Venture      Masashi Kojima					
<b>REQUEST:</b>						
Transworld has been informed by their concrete supplier that ASTM C260 requires a mix of 6% air entrainment and such amounts of air entrainment are specified only in freeze/thaw areas for durability. The Bay Area is generally not considered a freeze/thaw area and therefore a mix with 6% air entrainment is not typically used. The concrete supplier, Bode Concrete, has provided a letter from BASF related to this specific issue.						
<b>ANSWER:</b>						
In order to accurately design and locate elements of the bracing, trestle and bridges, please provide the most up-to-date and reliable architectural and structural drawings (including cad files). Also, drawings (including CAD files) of the train box and any other component of the transit center that has the potential to conflict with the BSE scope of work.						
Transworld has been informed by their concrete supplier that ASTM C260 requires a mix of 6% air entrainment and such amounts of air entrainment are specified only in freeze/thaw areas for durability. The Bay Area is generally not considered a freeze/thaw area and therefore a mix with 6% air entrainment is not typically used. The concrete supplier, Bode Concrete, has provided a letter from BASF related to this specific issue.						
<b>T-0017</b>	<b>BSE - CDSM Wall Alignment</b>	<b>Closed</b>	<b>01</b>	<b>01/14/2011</b>	<b>01/24/2011</b>	<b>01/21/2011</b>
	<b>From:</b> Webcor/Obayashi Joint Venture      Masashi Kojima					
<b>REQUEST:</b>						
The response to pre-bid RFI #177 indicated that the CDSM shoring line alignment is expected to change "prior to installation". We request the revised re-alignment be provided to us as soon as possible. We are currently designing and issuing steel mill orders based on the current alignment. If the revision comes after mill orders are finalized we risk missing our rolling schedule thereby losing our bid date pricing.						
<b>ANSWER:</b>						
The response to pre-bid RFI #177 indicated that the CDSM shoring line alignment is expected to change "prior to installation". We request the revised re-alignment be provided to us as soon as possible. We are currently designing and issuing steel mill orders based on the current alignment. If the revision comes after mill orders are finalized we risk missing our rolling schedule thereby losing our bid date pricing.						







<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
<b>T-0019</b>	<b>301 Mission Wall - Stone Panel Anchorage to 301 Mission's Screen Wall</b>	<b>Closed</b>	<b>CR</b>	<b>01/18/2011</b>	<b>01/28/2011</b>	<b>01/31/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: Attached pages from the 2008 Building Code						Reference: Attached pages from the 2008 Building Code
After removing stone panels in the demolition of the original 301 Mission Wall, the existing system of the stone panels does not utilize an anchoring system for mounting the stone panels to the wall. In addition, section 6.2.2.4 of the 2008 Building code does not specify mechanical fasteners for masonry less than 2-5/8" thick. The stone thickness used on the new wall will match the thickness of the existing, which is approx 10mm thick. Therefore, according to section 6.3 of the 2008 Building Code, the stone panel system for the Transbay Interim Screen Wall that should be used is the adhesion application.						After removing stone panels in the demolition of the original 301 Mission Wall, the existing system of the stone panels does not utilize an anchoring system for mounting the stone panels to the wall. In addition, section 6.2.2.4 of the 2008 Building code does not specify mechanical fasteners for masonry less than 2-5/8" thick. The stone thickness used on the new wall will match the thickness of the existing, which is approx 10mm thick. Therefore, according to section 6.3 of the 2008 Building Code, the stone panel system for the Transbay Interim Screen Wall that should be used is the adhesion application.
Please confirm that Transworld can use the adhered method for the stone panels in lieu of mechanical fasteners.						Please confirm that Transworld can use the adhered method for the stone panels in lieu of mechanical fasteners.
<b>T-0019.1</b>	<b>301 Mission Wall - Stone Panel Anchorage to 301 Mission's Screen Wall</b>	<b>Closed</b>	<b>CR</b>	<b>02/07/2011</b>	<b>02/17/2011</b>	<b>02/10/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: RFI T-0019 and attached photos						Reference: RFI T-0019 and attached photos
RFI T-0019 requested samples of stone from the demolished 301 Mission Street Screen Wall in order to verify thickness of the stone that will be used on the wall, and confirm that a mechanical system had not been used to mount the stone. A sample has been shown to URS and pictures of that sample are attached to this RFI. Please confirm that mechanically fastened panels are not necessary and that a thin set adhesive application will be an acceptable means to setting the stone on the new screen wall.						RFI T-0019 requested samples of stone from the demolished 301 Mission Street Screen Wall in order to verify thickness of the stone that will be used on the wall, and confirm that a mechanical system had not been used to mount the stone. A sample has been shown to URS and pictures of that sample are attached to this RFI. Please confirm that mechanically fastened panels are not necessary and that a thin set adhesive application will be an acceptable means to setting the stone on the new screen wall.
<b>T-0020</b>	<b>BSE - Demo Contract Shoring Wall and Bracing</b>	<b>Closed</b>	<b>01</b>	<b>01/27/2011</b>	<b>02/07/2011</b>	<b>02/02/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	<p>Reference Sheet D-2203 and Specification Section 02 41 01</p> <p>The BSE contract drawings shows a temporary shoring and bracing that is installed by the demo contract and subsequently removed by the BSE contract. In order for Balfour Beatty to properly plan their work, they request the following information:</p> <p>1 - The shoring design drawings for the shoring wall on the east side of Fremont St. (shown on D-2203) that was submitted by the Demo Contractor.</p> <p>2 - As-built location of the above mentioned shoring wall.</p> <p>3 - Bracing drawings and details that submitted for the basement wall rakers that are schematically shown on detail 1 of sheet D-5100 and details 1 &amp; 2 on sheet D-5102</p>					
	<p>Reference Sheet D-2203 and Specification Section 02 41 01</p> <p>The BSE contract drawings shows a temporary shoring and bracing that is installed by the demo contract and subsequently removed by the BSE contract. In order for Balfour Beatty to properly plan their work, they request the following information:</p> <p>1 - The shoring design drawings for the shoring wall on the east side of Fremont St. (shown on D-2203) that was submitted by the Demo Contractor.</p> <p>2 - As-built location of the above mentioned shoring wall.</p> <p>3 - Bracing drawings and details that submitted for the basement wall rakers that are schematically shown on detail 1 of sheet D-5100 and details 1 &amp; 2 on sheet D-5102</p>					
<b>T-0021</b>	<b>BSE - Existing Unknown Concrete Wall</b>	<b>Closed</b>	<b>01</b>	<b>01/27/2011</b>	<b>02/07/2011</b>	<b>02/04/2011</b>
	<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference Drawing Set D and Specification Section 02 41 01</p> <p>Based upon Balfour Beatty observations of the site, there appears to be a concrete wall approximately 18in wide that is outside of the existing terminal basement walls adjacent to the 301 Mission Property line and the east side of Fremont St. that is not shown on BSE contract drawings or the existing Terminal drawings.</p> <p>Does this wall continue around the entire perimeter of the Zone 4 basement?</p> <p>Will this wall be removed by the demo contract prior to BSE NTP #02?</p> <p>Please provide as-builts of the wall location if is to remain.</p> <p>Does a similar wall exist around the basement walls in</p>					
	<p><b>ANSWER:</b></p> <p>Reference Drawing Set D and Specification Section 02 41 01</p> <p>Based upon Balfour Beatty observations of the site, there appears to be a concrete wall approximately 18in wide that is outside of the existing terminal basement walls adjacent to the 301 Mission Property line and the east side of Fremont St. that is not shown on BSE contract drawings or the existing Terminal drawings.</p> <p>Does this wall continue around the entire perimeter of the Zone 4 basement?</p> <p>Will this wall be removed by the demo contract prior to BSE NTP #02?</p> <p>Please provide as-builts of the wall location if is to remain.</p>					



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	Zone 2 and 3?					Does a similar wall exist around the basement walls in Zone 2 and 3?
<b>T-0021.1</b>	<b>BSE - As Built Location of Concrete Foundation Wall Along Fremont St.</b>	<b>Closed</b>	<b>01</b>	<b>03/01/2011</b>	<b>03/11/2011</b>	<b>03/15/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  Reference RFI #T-0021 (BBI #005) and Drawing Set D  Please provide BBII with as-built locations of the unforeseen concrete foundation wall within Fremont Street which is to remain in place. Please also provide as-built locations for the soldier pile & tie back wall which parallels Fremont Street adjacent to the Buttress. BBII and BECHO want to confirm that there is enough room for their equipment to drill the Buttress Shafts along Fremont Street, and to identify any potential conflicts.						<b>ANSWER:</b>  Reference RFI #T-0021 (BBI #005) and Drawing Set D  Please provide BBII with as-built locations of the unforeseen concrete foundation wall within Fremont Street which is to remain in place. Please also provide as-built locations for the soldier pile & tie back wall which parallels Fremont Street adjacent to the Buttress. BBII and BECHO want to confirm that there is enough room for their equipment to drill the Buttress Shafts along Fremont Street, and to identify any potential conflicts.
<b>T-0022</b>	<b>Quality Management System - Org. Chart</b>	<b>Closed</b>	<b>CR</b>	<b>01/28/2011</b>	<b>02/07/2011</b>	<b>02/08/2011</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b>  Ref - Attached Org. Chart  Please identify the appropriate personnel associated with the attached org. chart found the in the program Quality Management System.						<b>ANSWER:</b>  Ref - Attached Org. Chart  Please identify the appropriate personnel associated with the attached org. chart found the in the program Quality Management System.

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<b>T-0023</b>	<b>Construction Manager Quality Plan</b>	<b>Closed</b>	<b>CR</b>	<b>01/31/2011</b>	<b>02/10/2011</b>	<b>02/07/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Bob Garcia						
<b>REQUEST:</b> Page 30 Paragraph 8.5.5 of the QMS manual makes reference to "the construction management consultant's quality plan". Please advise when the Construction Managers Quality Plan for the TTC will be issued?			<b>ANSWER:</b> Page 30 Paragraph 8.5.5 of the QMS manual makes reference to "the construction management consultant's quality plan". Please advise when the Construction Managers Quality Plan for the TTC will be issued?			
<b>T-0024</b>	<b>Re-bracing for Revised SW Corner Alignment</b>	<b>Closed</b>	<b>CR</b>	<b>02/02/2011</b>	<b>02/11/2011</b>	<b>02/11/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b> Reference Sheet GT-1112 and Specification Section 31 55 00  The response to RFI T-0017 showed a revised CDSM wall alignment at the SW corner of zone 1 and the addition of the structural shear walls on wall X1-1. The RFI response implied that BBII's cross-lot bracing needed to be re-designed so there are no conflicts with the concrete columns and shear walls. In order to minimize the cost and impacts as a result of this change, BBII suggests using rakers for the re-bracing in this corner.  The cross lot bracing would be installed as specified for the initial excavation (ref stage 10 on GT-1112) similar to the layout shown on the attached sketch #1.  Then for the re-bracing stage 12 and stage 15 rakers could be used in locations shown in attachment sketch #2.  Would a design based on this concept be acceptable?  If not, BBII is available and willing to brainstorm additional ideas.			<b>ANSWER:</b> Reference Sheet GT-1112 and Specification Section 31 55 00  The response to RFI T-0017 showed a revised CDSM wall alignment at the SW corner of zone 1 and the addition of the structural shear walls on wall X1-1. The RFI response implied that BBII's cross-lot bracing needed to be re-designed so there are no conflicts with the concrete columns and shear walls. In order to minimize the cost and impacts as a result of this change, BBII suggests using rakers for the re-bracing in this corner.  The cross lot bracing would be installed as specified for the initial excavation (ref stage 10 on GT-1112) similar to the layout shown on the attached sketch #1.  Then for the re-bracing stage 12 and stage 15 rakers could be used in locations shown in attachment sketch #2.  Would a design based on this concept be acceptable?  If not, BBII is available and willing to brainstorm additional ideas.			
<b>T-0025</b>	<b>BSE - Request for Recent Groundwater Monitoring Data</b>	<b>Closed</b>	<b>01</b>	<b>02/02/2011</b>	<b>02/12/2011</b>	<b>02/11/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						



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	<p><b>REQUEST:</b></p> <p>Reference Specification Section 31 55 00 and GDR Table 7-2 (attached)</p> <p>The Project GDR table 7-2 shows the last GW level reading in Feb of 2010. Can BBII receive a copy of any readings taken within the last year?</p>					<p><b>ANSWER:</b></p> <p>Reference Specification Section 31 55 00 and GDR Table 7-2 (attached)</p> <p>The Project GDR table 7-2 shows the last GW level reading in Feb of 2010. Can BBII receive a copy of any readings taken within the last year?</p>
<b>T-0026</b>	<b>301 Mission Wall - Sample chip of paint color for exposed concrete</b>	<b>Closed</b>	<b>CR</b>	<b>02/07/2011</b>	<b>02/17/2011</b>	<b>02/10/2011</b>
	<p><b>From:</b> Webcor Construction LP                      David Hungerford</p> <p><b>REQUEST:</b></p> <p>Reference: A-5000 note 6</p> <p>Note 6 on sheet A-5000 states, "Color of paint for exposed concrete to match sample chip provided by TJPA representative". Please provide color sample chip per this note.</p>					<p><b>ANSWER:</b></p> <p>Reference: A-5000 note 6</p> <p>Note 6 on sheet A-5000 states, "Color of paint for exposed concrete to match sample chip provided by TJPA representative". Please provide color sample chip per this note.</p>
<b>T-0027</b>	<b>301 Mission Screen Wall - Dowels for Screen Wall</b>	<b>Closed</b>	<b>CR</b>	<b>02/08/2011</b>	<b>02/18/2011</b>	<b>02/18/2011</b>
	<p><b>From:</b> Webcor Construction LP                      David Hungerford</p> <p><b>REQUEST:</b></p> <p>Reference: Attached pictures</p> <p>Upon laying out the dowel embedment locations for the new concrete wall, the locations are very close to the edge of the existing manholes and vault lids. Transworld is concerned that the location of the doweling is too close to these existing items and does not believe it to be the intent. Please see attached pictures showing the areas of concern. Please respond ASAP with direction on where to place the dowels, as Transworld has no slack in the schedule to accomodate any stoppage of work.</p>					<p><b>ANSWER:</b></p> <p>Reference: Attached pictures</p> <p>Upon laying out the dowel embedment locations for the new concrete wall, the locations are very close to the edge of the existing manholes and vault lids. Transworld is concerned that the location of the doweling is too close to these existing items and does not believe it to be the intent. Please see attached pictures showing the areas of concern. Please respond ASAP with direction on where to place the dowels, as Transworld has no slack in the schedule to accomodate any stoppage of work.</p>









<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0030.1</b>	<b>301 Mission Screen Wall - Concrete sleeve installation</b>	<b>Closed</b>	<b>CR</b>	<b>02/24/2011</b>	<b>03/06/2011</b>	<b>03/03/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: RFI T-0030  The final measurement from the edge of the steel collar/frame at the existing manholes to the face of new wall is (+/-) 4-3/4", this dimension less form material (+/-) 3/4" to 1", results in the new cast in place concrete sleeve to be 4" thick at the point closest to the wall . Response to RFI T-0030 notes that the sleeve is to be 6" thick. Please clarify if the 4" thickness is acceptable.						<b>ANSWER:</b>  Reference: RFI T-0030  The final measurement from the edge of the steel collar/frame at the existing manholes to the face of new wall is (+/-) 4-3/4", this dimension less form material (+/-) 3/4" to 1", results in the new cast in place concrete sleeve to be 4" thick at the point closest to the wall . Response to RFI T-0030 notes that the sleeve is to be 6" thick. Please clarify if the 4" thickness is acceptable.
<b>T-0031</b>	<b>301 Mission Screen Wall - In-ground lighting</b>	<b>Closed</b>	<b>CR</b>	<b>02/09/2011</b>	<b>02/19/2011</b>	<b>02/21/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: Note 10 on C-2000  The new in-ground lighting as anticipated in plans and note 10 on page C - 2000 must be substituted because the contract design cannot be accommodated in the new construction. The contract design requires: 1) that the new lighting match the existing with the same model and size. The issue here is that the existing light fixtures are larger than can be accommodated within the thickness of the new construction.  2) that the existing electrical lines servicing the existing lights be disconnected so that it is reconnected to the new lights. The issue here is that the electrical lines for the existing light fixtures are embedded in the concrete curb that is to be removed. Upon removal of the existing concrete curb, there will be no existing electrical lines to reconnect for the new lighting power.  Please provide a new detail and instructions for the in-ground lighting.						<b>ANSWER:</b>  Reference: Note 10 on C-2000  The new in-ground lighting as anticipated in plans and note 10 on page C - 2000 must be substituted because the contract design cannot be accommodated in the new construction. The contract design requires: 1) that the new lighting match the existing with the same model and size. The issue here is that the existing light fixtures are larger than can be accommodated within the thickness of the new construction.  2) that the existing electrical lines servicing the existing lights be disconnected so that it is reconnected to the new lights. The issue here is that the electrical lines for the existing light fixtures are embedded in the concrete curb that is to be removed. Upon removal of the existing concrete curb, there will be no existing electrical lines to reconnect for the new lighting power.  Please provide a new detail and instructions for the in-ground lighting.





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<b>T-0031.1</b>	<b>301 Mission Wall - In-ground lighting</b>	<b>Closed</b>	<b>CR</b>	<b>03/31/2011</b>	<b>04/10/2011</b>	<b>04/06/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: Attached photos and sketch						Reference: Attached photos and sketch
Response to RFI T-0031 requested additional information.						Response to RFI T-0031 requested additional information.
1. See the attached pictures for the information known about the lights that were removed.						1. See the attached pictures for the information known about the lights that were removed.
2. The existing conduit is 3/4"						2. The existing conduit is 3/4"
3. Attached is a sketch and a photo showing the approximate location of the existing conduit.						3. Attached is a sketch and a photo showing the approximate location of the existing conduit.
There is one existing conduit on the south side of the wall protruding from the soil coming from the basement wall. The electrical conduit is approximately 6 feet east from the western transformer vault vent opening. Attached you can see the pictures of this conduit that is currently sticking out below the scaffolding planking.						There is one existing conduit on the south side of the wall protruding from the soil coming from the basement wall. The electrical conduit is approximately 6 feet east from the western transformer vault vent opening. Attached you can see the pictures of this conduit that is currently sticking out below the scaffolding planking.
4. Please advise the location and mounting details for the new lights.						4. Please advise the location and mounting details for the new lights.
<b>T-0031.2</b>	<b>301 Mission Wall - Light Fixtures</b>	<b>Closed</b>	<b>CR</b>	<b>06/29/2011</b>	<b>07/09/2011</b>	<b>07/13/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: Attached light specs						Reference: Attached light specs
Per field conversations with 301 Mission staff, the light fixture proposed in response to RFI T-0031.1 is not acceptable. Webcor-Obayashi has coordinated with 301 Mission management personnel and the lighting attachment to this RFI has been requested by 301 Mission. Confirm that the attached light specs are to be installed at the stucco slot locations.						Per field conversations with 301 Mission staff, the light fixture proposed in response to RFI T-0031.1 is not acceptable. Webcor-Obayashi has coordinated with 301 Mission management personnel and the lighting attachment to this RFI has been requested by 301 Mission. Confirm that the attached light specs are to be installed at the stucco slot locations.
<b>T-0032</b>	<b>301 Mission Screen Wall - Tie Beam Below Grade Connection to Screen Wall</b>	<b>Closed</b>	<b>CR</b>	<b>02/09/2011</b>	<b>02/19/2011</b>	<b>02/23/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: Attached photo						Reference: Attached photo



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T-0033	<p>301 Mission Screen Wall - Concrete Demo Scope of Work Clarification</p> <p><b>From:</b> Webcor Construction LP                      David Hungerford</p> <p><b>REQUEST:</b></p> <p>Reference: attached text document</p> <p>Please see attached text document explaining Transworld's request.</p> <p>Transworld Construction requests that TJPA, Turner Construction, and Webcor-Obayashi make a final determination as to work scope based on the documents and discussions provided herein. It is Transworld's contention and belief that the 301 Mission wall relocation work scope does not require Transworld to remove the (e) concrete structure below the dark gray colored curb. For clarity see Exhibit D, page 1 and page 2.</p> <p>Attached please see text explanation and Exhibits A, B, C, and D.</p>	Closed	CR	02/14/2011	02/24/2011	02/25/2011
	<p><b>ANSWER:</b></p> <p>Reference: attached text document</p> <p>Please see attached text document explaining Transworld's request.</p> <p>Transworld Construction requests that TJPA, Turner Construction, and Webcor-Obayashi make a final determination as to work scope based on the documents and discussions provided herein. It is Transworld's contention and belief that the 301 Mission wall relocation work scope does not require Transworld to remove the (e) concrete structure below the dark gray colored curb. For clarity see Exhibit D, page 1 and page 2.</p> <p>Attached please see text explanation and Exhibits A, B, C, and D.</p>					

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T-0035	BSE - Additional Trainbox Drawings	Closed	01	02/16/2011	02/26/2011	02/22/2011
<div><div><div>From: Webcor Construction LP</div><div>Nhi Tran</div></div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Reference Sheet S-3201 and Specification Section 31 55 00</p> <p>BBII believes that they do not have enough detailed drawings of the Train Box to properly design a conflict-free bracing system. BBII states that the architectural sections A1-6000 through A1-6231 lack detail regarding dimensions of structural components (i.e. beams, walls, ramps and etc.). The only structural section BBII currently has is on S-3201 and there appears to be a beam running along C line, however that beam is not identified in the table.</p> <p>BBII is requesting additional structural section and elevation drawings, specifically:</p> <ul style="list-style-type: none"><li>- A dimensioned longitudinal elevation of the entire trainbox, showing the most current location and depths of beams.</li><li>- Full cross section of typical trainbox as well as any other non typical section. Shown any cross slopes, high and low points of concrete.</li><li>- Detailed sections of the SW corner showing dimensions and elevations of any ramps or locations where there are on ground floor slabs.</li></ul> <p>BBII would prefer CAD files if possible, however hardcopies will work.</p>			<p>Reference Sheet S-3201 and Specification Section 31 55 00</p> <p>BBII believes that they do not have enough detailed drawings of the Train Box to properly design a conflict-free bracing system. BBII states that the architectural sections A1-6000 through A1-6231 lack detail regarding dimensions of structural components (i.e. beams, walls, ramps and etc.). The only structural section BBII currently has is on S-3201 and there appears to be a beam running along C line, however that beam is not identified in the table.</p> <p>BBII is requesting additional structural section and elevation drawings, specifically:</p> <ul style="list-style-type: none"><li>- A dimensioned longitudinal elevation of the entire trainbox, showing the most current location and depths of beams.</li><li>- Full cross section of typical trainbox as well as any other non typical section. Shown any cross slopes, high and low points of concrete.</li><li>- Detailed sections of the SW corner showing dimensions and elevations of any ramps or locations where there are on ground floor slabs.</li></ul> <p>BBII would prefer CAD files if possible, however hardcopies will work.</p>			



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T-0035.1	BSE - Request Structure Section Drawings	Closed	01	03/15/2011	03/25/2011	03/23/2011
From: Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference attached sheet		Reference attached sheet				
As discussed in 03/09/11 TG03 Design Team meeting, AAI said they would provide sections of the trainbox structure if BBII indentified where to take the cuts. Below is a list and the attached shows where BBII would like these taken		As discussed in 03/09/11 TG03 Design Team meeting, AAI said they would provide sections of the trainbox structure if BBII indentified where to take the cuts. Below is a list and the attached shows where BBII would like these taken				
CUT # - DESCRIPTION		CUT # - DESCRIPTION				
1.A - Full length section along Grid A		1.A - Full length section along Grid A				
1.E - Full length section along Grid E		1.E - Full length section along Grid E				
1.J - Full length section along Grid J unfolded along wall alignment		1.J - Full length section along Grid J unfolded along wall alignment				
2 - Full width section at Column Line 3		2 - Full width section at Column Line 3				
3 - Full width section at Column Line 7		3 - Full width section at Column Line 7				
4 - Full width section at Column Line 10.5		4 - Full width section at Column Line 10.5				
5 - Full width section at Column Line 18 (CL First St)		5 - Full width section at Column Line 18 (CL First St)				
6 - Full width section at Column Line 23		6 - Full width section at Column Line 23				
7 - Full width section at Column Line 26 (CL Freemont St)		7 - Full width section at Column Line 26 (CL Freemont St)				
8 - Full width section at Column Line 30		8 - Full width section at Column Line 30				
9 - Full width section at Column Line 34.5 (Beale St.)		9 - Full width section at Column Line 34.5 (Beale St.)				
10 - Section at "flare?"		10 - Section at "flare?"				
11 - Section at "flare?"		11 - Section at "flare?"				
Please provide either electronic 2D CAD files at for each section where BBII can dimension, or hardcopy drawings that are fully dimensioned.		Please provide either electronic 2D CAD files at for each section where BBII can dimension, or hardcopy drawings that are fully dimensioned.				



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<b>T-0036</b>	<b>BSE - Bracing Load Discrepancy</b>	<b>Closed</b>	<b>01</b>	<b>02/16/2011</b>	<b>02/26/2011</b>	<b>02/18/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet GT-1110, Specification Section 31 55 00, and attached memo		Reference Sheet GT-1110, Specification Section 31 55 00, and attached memo				
Please see the attached memo from BBII's bracing design engineer, PB&A.		Please see the attached memo from BBII's bracing design engineer, PB&A.				
PB&A are finding more than a slight discrepancy between the bracing loads given in the tables of GT-1110 when compared to loads they calculated using the "design profile" earth pressured diagram as shown on the same sheet.		PB&A are finding more than a slight discrepancy between the bracing loads given in the tables of GT-1110 when compared to loads they calculated using the "design profile" earth pressured diagram as shown on the same sheet.				
As required by note 6 on GT-1110, BBII is continuing their design with the forces given in the tables, however BBII feels it is prudent to note the variances.		As required by note 6 on GT-1110, BBII is continuing their design with the forces given in the tables, however BBII feels it is prudent to note the variances.				
BBII requests confirmation that the forces given in the tables of GT-1110 are correct.		BBII requests confirmation that the forces given in the tables of GT-1110 are correct.				
<b>T-0037</b>	<b>BSE - Request for Utility As-Builts</b>	<b>Closed</b>	<b>01</b>	<b>02/17/2011</b>	<b>02/28/2011</b>	<b>03/01/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheets U-2021 to U-2023, U-4005		Reference Sheets U-2021 to U-2023, U-4005				
BBII is requesting as-built data for the phase 1 electrical ductbanks at First St. and Fremont St. BBII is particularly interested in receiving the coordinates, elevations, width and depths of the ductbank where they intersect the CDSM wall as shown on utility drawings U-2021 through U-2023		BBII is requesting as-built data for the phase 1 electrical ductbanks at First St. and Fremont St. BBII is particularly interested in receiving the coordinates, elevations, width and depths of the ductbank where they intersect the CDSM wall as shown on utility drawings U-2021 through U-2023				
Additionally, BBII would like to receive more info on the phase 2 utilities shown in section X&Y on U-4005: - What material are these ducts and are they encased? - Can the spacing shown on U-4005 be shifted to accommodate bridge girder spacing?		Additionally, BBII would like to receive more info on the phase 2 utilities shown in section X&Y on U-4005: - What material are these ducts and are they encased? - Can the spacing shown on U-4005 be shifted to accommodate bridge girder spacing?				
<b>T-0037.1</b>	<b>BSE - Request for Utility As-Builts</b>	<b>Closed</b>	<b>01</b>	<b>03/24/2011</b>	<b>04/04/2011</b>	<b>04/13/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						





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T-0038.1	BSE - Shear Walls for Rebracing	Closed	01	11/06/2014	11/16/2014	11/17/2014
From: Webcor Construction LP Claude Titcher						
REQUEST: Reference RFI T-0038.  The response to RFI T-0038 states that once the Lower Concourse has reached design strength the permanent shear walls can be utilized as re-braces. The response also contains a sketch detailing the load path of the shear wall, however the sketch details the Lower Concourse shear walls being poured up to the foundation wall. The design has evolved since and the contract drawings now show a corridor running along the southwest corridor creating a gap between the shear wall and foundation wall. Please clarify the design intent and confirm shear walls can be used as re-bracing per contract document.						
						ANSWER: Reference RFI T-0038.  The response to RFI T-0038 states that once the Lower Concourse has reached design strength the permanent shear walls can be utilized as re-braces. The response also contains a sketch detailing the load path of the shear wall, however the sketch details the Lower Concourse shear walls being poured up to the foundation wall. The design has evolved since and the contract drawings now show a corridor running along the southwest corridor creating a gap between the shear wall and foundation wall. Please clarify the design intent and confirm shear walls can be used as re-bracing per contract document.
T-0039	301 Mission Screen Wall - Base Plate Dimensions	Closed	CR	02/17/2011	02/27/2011	02/23/2011
From: Webcor Construction LP David Hungerford						
REQUEST: Reference: 2/S-5000, D/S-5000, attached sketches  See the 301 Mission Screen Wall drawings, specifically details 2 and D/S-5000. Is it acceptable to use a base plate with dimensions 14" x 14", in lieu of the 14" x 18" per plan below the HSS 10" x 10"? See attached sketches of proposed anchor bolt mounting options A and B. If acceptable, please choose the detail you prefer.						
						ANSWER: Reference: 2/S-5000, D/S-5000, attached sketches  See the 301 Mission Screen Wall drawings, specifically details 2 and D/S-5000. Is it acceptable to use a base plate with dimensions 14" x 14", in lieu of the 14" x 18" per plan below the HSS 10" x 10"? See attached sketches of proposed anchor bolt mounting options A and B. If acceptable, please choose the detail you prefer.
T-0040	BSE - Proposed Bracing Removal Sequence	Closed	01	02/22/2011	03/04/2011	02/23/2011
From: Webcor Construction LP Nhi Tran						
REQUEST: Reference Sheet GT-1112 and attached proposal  Attached is a proposed sequence for bracing removal that involves removing the two lower layers of bracing after the structural slab and fillets are poured. BBII's shoring designer has done analysis at each stage of construction (see attached). The results show that removal of the two						
						ANSWER: Reference Sheet GT-1112 and attached proposal  Attached is a proposed sequence for bracing removal that involves removing the two lower layers of bracing after the structural slab and fillets are poured. BBII's shoring designer has done analysis at each stage of construction (see attached). The results show that





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	<p>lower levels after the slab has been poured produces less deflection than the fully excavated condition. The results are summarized for case west and case east on page 18 and 36 respectively.</p> <p>BBII believes this proposed sequence provides a tremendous value to the overall project by:</p> <ul style="list-style-type: none"><li>- Eliminating the coordination between the bracing and concrete trade subcontractors during the construction of the lower walls and concourse slab</li><li>- Eliminates a horizontal construction joint in the lower wall which significantly reduces construction cost and duration.</li><li>- Allows for better waterproofing product, by eliminating a construction joint and reduces patching of the membrane around shoring elements</li><li>- Allows for unobstructed construction of the lower walls and soffit shoring of the concourse level slab, which also reduces construction cost and duration</li></ul> <p>BBII is requesting evaluation by TJPA's design team to determine if this sequence is acceptable.</p>					<p>removal of the two lower levels after the slab has been poured produces less deflection than the fully excavated condition. The results are summarized for case west and case east on page 18 and 36 respectively.</p> <p>BBII believes this proposed sequence provides a tremendous value to the overall project by:</p> <ul style="list-style-type: none"><li>- Eliminating the coordination between the bracing and concrete trade subcontractors during the construction of the lower walls and concourse slab</li><li>- Eliminates a horizontal construction joint in the lower wall which significantly reduces construction cost and duration.</li><li>- Allows for better waterproofing product, by eliminating a construction joint and reduces patching of the membrane around shoring elements</li><li>- Allows for unobstructed construction of the lower walls and soffit shoring of the concourse level slab, which also reduces construction cost and duration</li></ul> <p>BBII is requesting evaluation by TJPA's design team to determine if this sequence is acceptable.</p>
<b>T-0041</b>	<b>BSE - COR and PCO Forms</b>	<b>Closed</b>	<b>01</b>	<b>02/23/2011</b>	<b>03/05/2011</b>	<b>03/16/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Spec. Section 00 07 00, 6.03E,		Reference Spec. Section 00 07 00, 6.03E,				
Per section 00 07 00, 6.03E, BBII requests for the form as mentioned to be supplied by TJPA, preferably in editable electronic format.		Per section 00 07 00, 6.03E, BBII requests for the form as mentioned to be supplied by TJPA, preferably in editable electronic format.				



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T-0043	301 Mission Screen Wall - Temporary Vault Plug at Utility Vault Opening	Closed	CR	02/25/2011	03/07/2011	03/23/2011
<div> <div> <b>From:</b> Webcor Construction LP David Hungerford </div> <div> <b>REQUEST:</b> <p>Regarding the transformer vault plug as shown on page C-5000; Transworld has been asked to submit some proposals as to how a plug should be installed. The original existing ventilation for the vault was open to the air at the original planters. This original ventilation was completely open and secured only by a metal grate to prevent access, but not water or air. As located on page C-5000, Transworld construction proposes to install 2 x 4 backing studs attached to the left and right vertical walls of the existing opening. These 2 x 4 backing studs will be adhered with powder actuated nails. Spanning across the backing studs Transworld construction proposes to install two 2 x 4 crossmembers which will be nailed to the 2 x 4 backing studs. This assembly can be seen in the attached pictures pages 1 and 2.</p> <p>The assembly noted above is option 1.</p> <p>Option 2- Added additional 2x4 crossmembers which would further restrict air flow to the (e) vault.</p> <p>Option 3- Nail on a plywood sheet that would enclose the entire vault vent opening.</p> <p>Option 4 - Nail on a plywood sheet and waterproof the plywood to prevent water intrusion as well.</p> <p>Note: Transworld Construction is concerned about restricting airflow into a vault that originally was designed to have this open vent. We are not familiar with any impact sealing this vent will have on the existing equipment.</p> </div> <div> <b>ANSWER:</b> <p>Regarding the transformer vault plug as shown on page C-5000; Transworld has been asked to submit some proposals as to how a plug should be installed. The original existing ventilation for the vault was open to the air at the original planters. This original ventilation was completely open and secured only by a metal grate to prevent access, but not water or air. As located on page C-5000, Transworld construction proposes to install 2 x 4 backing studs attached to the left and right vertical walls of the existing opening. These 2 x 4 backing studs will be adhered with powder actuated nails. Spanning across the backing studs Transworld construction proposes to install two 2 x 4 crossmembers which will be nailed to the 2 x 4 backing studs. This assembly can be seen in the attached pictures pages 1 and 2.</p> <p>The assembly noted above is option 1.</p> <p>Option 2- Added additional 2x4 crossmembers which would further restrict air flow to the (e) vault.</p> <p>Option 3- Nail on a plywood sheet that would enclose the entire vault vent opening.</p> <p>Option 4 - Nail on a plywood sheet and waterproof the plywood to prevent water intrusion as well.</p> <p>Note: Transworld Construction is concerned about restricting airflow into a vault that originally was designed to have this open vent. We are not familiar with any impact sealing this vent will have on the existing equipment.</p> </div> </div>						
T-0044	BSE - Pile Mat Slab Connection	Closed	01	02/25/2011	03/07/2011	03/02/2011
<div> <div> <b>From:</b> Webcor Construction LP Nhi Tran </div> <div> <b>REQUEST:</b> <p>Reference Sheet S-3003</p> <p>Reference Detail 2 on S-3003 - "Slip Detail @ Trestle Pile Mat Connection"</p> <p>Please confirm that this detail only applies to the trestle and not the bridge as stated.</p> </div> <div> <b>ANSWER:</b> <p>Reference Sheet S-3003</p> <p>Reference Detail 2 on S-3003 - "Slip Detail @ Trestle Pile Mat Connection"</p> <p>Please confirm that this detail only applies to the trestle and not the bridge as stated.</p> </div> </div>						



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<b>T-0045</b>	<b>301 Mission Screen Wall - Void Below Existing Embed</b>	<b>Closed</b>	<b>CR</b>	<b>03/02/2011</b>	<b>03/12/2011</b>	<b>03/17/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Attached pictures			Reference: Attached pictures			
The new 301 Mission screen wall location is to be laid out over an existing embed plate. At that plate #8 rebars are to be epoxied per RFI T-0027. Currently in the field the embed has been cut where the dowels are to be installed and holes are being drilled to the required 30" depth. It has been discovered that there are voids below the existing embed plate of up to 1.5". See attached pictures for some locations where this condition occurs. Please advise if this void is to be filled.			The new 301 Mission screen wall location is to be laid out over an existing embed plate. At that plate #8 rebars are to be epoxied per RFI T-0027. Currently in the field the embed has been cut where the dowels are to be installed and holes are being drilled to the required 30" depth. It has been discovered that there are voids below the existing embed plate of up to 1.5". See attached pictures for some locations where this condition occurs. Please advise if this void is to be filled.			
<b>T-0046</b>	<b>BSE - CLSM Slump</b>	<b>Closed</b>	<b>01</b>	<b>03/03/2011</b>	<b>03/13/2011</b>	<b>03/07/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification Section 03 30 01			Reference Specification Section 03 30 01			
The CLSM slump range for the Buttress Shoring Excavation Work is listed between 10" to 12". BBI has concerns about the CLSM mix segregating during placement with such a high slump. Please confirm if it is acceptable to provide a CLSM mix with a slump range of 7" +/- 1" in lieu of the 10" to 12" called for in the Specification.			The CLSM slump range for the Buttress Shoring Excavation Work is listed between 10" to 12". BBI has concerns about the CLSM mix segregating during placement with such a high slump. Please confirm if it is acceptable to provide a CLSM mix with a slump range of 7" +/- 1" in lieu of the 10" to 12" called for in the Specification.			
<b>T-0047</b>	<b>BSE - Joint Preconstruction Survey</b>	<b>Closed</b>	<b>01</b>	<b>03/03/2011</b>	<b>03/13/2011</b>	<b>03/11/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification Section 01 15 40 and attached list			Reference Specification Section 01 15 40 and attached list			
Attached is the list of buildings that BBI has identified for joint survey, in accordance with specification section 01 15 40. BBI requests confirmation of this list.			Attached is the list of buildings that BBI has identified for joint survey, in accordance with specification section 01 15 40. BBI requests confirmation of this list.			
Please provide BBI a contact for coordinating the joint survey effort. BBI would like to do this work on the week of March 14, 2011.			Please provide BBI a contact for coordinating the joint survey effort. BBI would like to do this work on the week of March 14, 2011.			



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<b>T-0047.1</b>	<b>BSE - Preconstruction Joint Survey Exteriors of Buildings</b>	<b>Closed</b>	<b>01</b>	<b>03/21/2011</b>	<b>03/31/2011</b>	<b>03/28/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference RFI #T-0047 and attached email						Reference RFI #T-0047 and attached email
Please confirm the exterior of the building, in accordance with item 1.5 D in the specification 01 15 40 Joint Survey, is also covered by the response of RFI T-0047 as well as the interior of the building.						Please confirm the exterior of the building, in accordance with item 1.5 D in the specification 01 15 40 Joint Survey, is also covered by the response of RFI T-0047 as well as the interior of the building.
If not, please contact "property owners within 25 feet of the construction excavation" and arrange the joint survey immediately.						If not, please contact "property owners within 25 feet of the construction excavation" and arrange the joint survey immediately.
<b>T-0048</b>	<b>BSE - Building Demolition in Zone 1</b>	<b>Closed</b>	<b>01</b>	<b>03/03/2011</b>	<b>03/13/2011</b>	<b>03/10/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference CR-T-005 and Sheet SKGT-0001-R1						Reference CR-T-005 and Sheet SKGT-0001-R1
CR T-005 appears to require additional building demolition. Please provide a schedule for this demolition work and an estimated completion date as this will potentially impact BBI's schedule and work sequence.						CR T-005 appears to require additional building demolition. Please provide a schedule for this demolition work and an estimated completion date as this will potentially impact BBI's schedule and work sequence.
<b>T-0049</b>	<b>BSE - Constructware</b>	<b>Closed</b>	<b>01</b>	<b>03/03/2011</b>	<b>03/13/2011</b>	<b>03/03/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Specification Section 01 10 40						Reference Specification Section 01 10 40
Specification Section 01 10 40 Article 1.6 B4 states: "TJPA will provide Trade Subcontractors with the necessary training and access to Constructware"						Specification Section 01 10 40 Article 1.6 B4 states: "TJPA will provide Trade Subcontractors with the necessary training and access to Constructware"
BBI would like to schedule this training and make arrangements for access. Please provide a contact to get this process started.						BBI would like to schedule this training and make arrangements for access. Please provide a contact to get this process started.



**ANSWER:**



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T-0051	Returned Submittal Comments	Closed	01	02/16/2011	02/26/2011	03/10/2011
<b>From:</b> Webcor Construction LP Daniel Foudy						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref Spec section 01 13 10		Ref Spec section 01 13 10				
According to the Action and Distribution (section 1.11) of the submittal specifications, Submittals shall be returned indicating one of the following:		According to the Action and Distribution (section 1.11) of the submittal specifications, Submittals shall be returned indicating one of the following:				
No Exceptions Taken		No Exceptions Taken				
Make Corrections Noted		Make Corrections Noted				
Revise and Resubmit		Revise and Resubmit				
Rejected		Rejected				
We have received submittals back as "Not Reviewed" or "For Record Only". Please confirm these responses are acceptable and should be incorporated into the specifications.		We have received submittals back as "Not Reviewed" or "For Record Only". Please confirm these responses are acceptable and should be incorporated into the specifications.				
T-0052	BSE - P Parcel	Closed	01	03/09/2011	03/19/2011	03/10/2011
<b>From:</b> Webcor Construction LP Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification Section 01 14 19, 1.4		Reference Specification Section 01 14 19, 1.4				
According to the referenced specification section, Parcel P is available as of November 1, 2010 and will be available until 2013. BBI was informed that this parcel will not be available for this contract.		According to the referenced specification section, Parcel P is available as of November 1, 2010 and will be available until 2013. BBI was informed that this parcel will not be available for this contract.				
Please confirm.		Please confirm.				
If this parcel is not available, are there any alternative parcels that will be available for construction staging?		If this parcel is not available, are there any alternative parcels that will be available for construction staging?				







Attached is a suggested detail as well as examples where it has been used before, for your consideration.





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T-0054	BSE - AC Overlay at Temporary Bridges	Closed	01	03/09/2011	03/19/2011	03/25/2011
From: Webcor Construction LP Nhi Tran						
REQUEST:		ANSWER:				
Reference Specification Section 01 53 13, 1.3.A.6 and attached material information		Reference Specification Section 01 53 13, 1.3.A.6 and attached material information				
For the temporary bridges, BBII will be using the attached structural bridge deck material from Big R Bridge. The troughs are filled completely with AC to the top of the decking, and an overlay will be applied over the top. BBII would like to use a 2" minimum overlay, resulting in an overall cross section with an average 4" thickness. Bridge geometry requirements specified in section 01 53 13 - 1.3.A.6 will be met without reducing the overlay thickness below the 2" minimum.		For the temporary bridges, BBII will be using the attached structural bridge deck material from Big R Bridge. The troughs are filled completely with AC to the top of the decking, and an overlay will be applied over the top. BBII would like to use a 2" minimum overlay, resulting in an overall cross section with an average 4" thickness. Bridge geometry requirements specified in section 01 53 13 - 1.3.A.6 will be met without reducing the overlay thickness below the 2" minimum.				
Please confirm this is acceptable.		Please confirm this is acceptable.				
T-0055	BSE - Request for Soil Parameters	Closed	01	03/09/2011	03/19/2011	03/14/2011
From: Webcor Construction LP Nhi Tran						
REQUEST:		ANSWER:				
Reference Sheet GT-1110 and Specification Section 31 55 00		Reference Sheet GT-1110 and Specification Section 31 55 00				
In the TG03 BSE Design Team Coordination meeting held on 03/09/2011, Arup said they would provide BBII with soil input parameters for use in BBI's model.		In the TG03 BSE Design Team Coordination meeting held on 03/09/2011, Arup said they would provide BBII with soil input parameters for use in BBI's model.				
Please provide BBI with this information.		Please provide BBI with this information.				
T-0056	BSE - CR T-006	Closed	01	03/09/2011	03/19/2011	03/10/2011
From: Webcor Construction LP Nhi Tran						
REQUEST:		ANSWER:				
Reference CR T-006		Reference CR T-006				
The Change Request documents do not indicate who will have the maintenance responsibility for the AC walkway.		The Change Request documents do not indicate who will have the maintenance responsibility for the AC walkway.				
BBII has the following questions: 1. Should BBII include pricing for maintenance? If this walkway is going to get placed on top of the 3"		BBII has the following questions: 1. Should BBII include pricing for maintenance?				



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<p>minus rubble, a fair amount of maintenance would be required.</p> <p>2. When is this walkway scheduled to be constructed? And if maintenance is needed, when would it start?</p> <p>3. Are the typical fence and K-rail shown in the section the same ones that are protecting the perimeter, or an additional row that creates a walkway that has both sides fenced, protecting the public from construction and vehicle traffic?</p> <p>BBII needs to have this information in order to provide accurate pricing for this Change Request T-006. Please advise.</p>						
<p>If this walkway is going to get placed on top of the 3" minus rubble, a fair amount of maintenance would be required.</p> <p>2. When is this walkway scheduled to be constructed? And if maintenance is needed, when would it start?</p> <p>3. Are the typical fence and K-rail shown in the section the same ones that are protecting the perimeter, or an additional row that creates a walkway that has both sides fenced, protecting the public from construction and vehicle traffic?</p> <p>BBII needs to have this information in order to provide accurate pricing for this Change Request T-006. Please advise.</p>						
<b>T-0056.1</b>	<b>BSE - CR T-006</b>	<b>Closed</b>	<b>01</b>	<b>03/24/2011</b>	<b>04/03/2011</b>	<b>04/12/2011</b>
<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI T-0056 and CR T-006			Reference RFI T-0056 and CR T-006			
Please confirm that any necessary repairs of the AC overlay are excluded from CR T-006 scope as discussed at the TG03 BSE - Design Coordination Meeting on 3/23/2011. Also, please provided additional sketches we discussed at the meeting as well. Finally, please provide a complete copy of Demo Contractor¿s change order related to CR T-006 to fully understand the limits of their responsibility.			Please confirm that any necessary repairs of the AC overlay are excluded from CR T-006 scope as discussed at the TG03 BSE - Design Coordination Meeting on 3/23/2011. Also, please provided additional sketches we discussed at the meeting as well. Finally, please provide a complete copy of Demo Contractor¿s change order related to CR T-006 to fully understand the limits of their responsibility.			



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<b>T-0057</b>	<b>BSE - Verticality and Sonic Testing on Drilled Piers and Shafts</b>	<b>Closed</b>	<b>01</b>	<b>03/10/2011</b>	<b>03/20/2011</b>	<b>03/11/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Sheet GT-5202 and Specification Section 31 63 29			Reference Sheet GT-5202 and Specification Section 31 63 29			
Specification Section 31 63 29, 3.8.1.3 states "The contractor shall perform a test to determine verticality of the steel tubes, or drilled holes, that are going to be used for the sonic tests."			Specification Section 31 63 29, 3.8.1.3 states "The contractor shall perform a test to determine verticality of the steel tubes, or drilled holes, that are going to be used for the sonic tests."			
BBII has been advised by a number of testing firms that verticality tests cannot be performed on steel tubes or PVC tubes tied to steel cages. Detail 12 on Drawing GT-5202 shows 4 equally spaced PVC or steel tubes tied to reinforcing steel cage. BBII has also been informed that, as of now, there is not a specification in existence that mentions vertical tolerances of CSL tubes.			BBII has been advised by a number of testing firms that verticality tests cannot be performed on steel tubes or PVC tubes tied to steel cages. Detail 12 on Drawing GT-5202 shows 4 equally spaced PVC or steel tubes tied to reinforcing steel cage. BBII has also been informed that, as of now, there is not a specification in existence that mentions vertical tolerances of CSL tubes.			
BBII is proposing to do the following in lieu of formally testing the CSL tubes for verticality: 1. BBII will make sure that the tubes are parallel and symmetrically placed. The cages and tubes will be properly inspected for positioning, spacing, parallelism prior to placing the cages into the hole. This is the most important inspection to ensure accurate CSL results. 2. Since the tubes are tied directly to a vertical cage, and the cages and casings are tested for verticality anyway, BBII will do a visual inspection to ensure that the tubes are sufficiently "vertical" for CSL testing purposes prior to placement of tremie concrete. 3. BBII will make sure that the cages are carefully lifted in a manner that limits the deflections of the cage to ensure that the CSL tubes do not fail at the joints.			BBII is proposing to do the following in lieu of formally testing the CSL tubes for verticality: 1. BBII will make sure that the tubes are parallel and symmetrically placed. The cages and tubes will be properly inspected for positioning, spacing, parallelism prior to placing the cages into the hole. This is the most important inspection to ensure accurate CSL results. 2. Since the tubes are tied directly to a vertical cage, and the cages and casings are tested for verticality anyway, BBII will do a visual inspection to ensure that the tubes are sufficiently "vertical" for CSL testing purposes prior to placement of tremie concrete. 3. BBII will make sure that the cages are carefully lifted in a manner that limits the deflections of the cage to ensure that the CSL tubes do not fail at the joints.			
Please confirm if this is acceptable.			Please confirm if this is acceptable.			



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T-0060	BSE - Underground Utilities Removal on 1st Street	Closed	01	03/11/2011	03/21/2011	03/23/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:			ANSWER:			
Reference Sheet D-2230			Reference Sheet D-2230			
Per Drawing D-2230 Note 2, "Unless specified otherwise all utilities to be removed have already been cut and capped outside limits of work by Transbay Transit Center Program Relocation of Utilities Project including future utilities installed by the Transbay Transit Center Program Relocation of Utilities Project. Contractor to coordinate removal of utilities with TJPA representative." Please confirm that the work described in Note 2 has been completed for all underground utilities on 1st St. If work has not yet been completed, please provide a list of utilities not yet abandoned and dates when the said utilities are to be cut and capped.			Per Drawing D-2230 Note 2, "Unless specified otherwise all utilities to be removed have already been cut and capped outside limits of work by Transbay Transit Center Program Relocation of Utilities Project including future utilities installed by the Transbay Transit Center Program Relocation of Utilities Project. Contractor to coordinate removal of utilities with TJPA representative." Please confirm that the work described in Note 2 has been completed for all underground utilities on 1st St. If work has not yet been completed, please provide a list of utilities not yet abandoned and dates when the said utilities are to be cut and capped.			







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<b>T-0062</b>	<b>BSE - Concrete Submittals</b>	<b>Closed</b>	<b>01</b>	<b>03/16/2011</b>	<b>03/26/2011</b>	<b>03/23/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification Section 03 30 00			Reference Specification Section 03 30 00			
BBII believes a number of the submittals listed under the Cast In Place concrete spec section are not applicable to the BSE package. - 03 30 00-1.6.A.5 Joint Locations for Concrete Slabs to receive a terrazzo finish ¿ None of the concrete work in this package is to receive flooring. - 03 30 00-1.6A.6 Preconstruction Survey - This is intended for locations where concrete interfaces with existing construction. The mud slab does not interface with existing concrete, and BBII is not anticipating using concrete at the temporary bridges. - 03 30 00-1.6.A.7 Survey of Flat Plate or Flat Slab Concrete Floors - No flat plates included in the BSE package. - 03 30 00-1.6.A.8 Survey of as-built floor conditions - This is applicable to finish floors only, which are not included in the BSE package. - 03 30 00-1.6.A.8 Structural Repairs - BBII does not believe there is any structural concrete requiring repair procedures in the BSE package. - 03 30 00-1.6.A.10 Patching defective concrete finishes - The concrete work in the BSE package is not finished or exposed concrete, so BBII does not believe patching procedures are necessary.			BBII believes a number of the submittals listed under the Cast In Place concrete spec section are not applicable to the BSE package. - 03 30 00-1.6.A.5 Joint Locations for Concrete Slabs to receive a terrazzo finish ¿ None of the concrete work in this package is to receive flooring. - 03 30 00-1.6A.6 Preconstruction Survey - This is intended for locations where concrete interfaces with existing construction. The mud slab does not interface with existing concrete, and BBII is not anticipating using concrete at the temporary bridges. - 03 30 00-1.6.A.7 Survey of Flat Plate or Flat Slab Concrete Floors - No flat plates included in the BSE package. - 03 30 00-1.6.A.8 Survey of as-built floor conditions - This is applicable to finish floors only, which are not included in the BSE package. - 03 30 00-1.6.A.8 Structural Repairs - BBII does not believe there is any structural concrete requiring repair procedures in the BSE package. - 03 30 00-1.6.A.10 Patching defective concrete finishes - The concrete work in the BSE package is not finished or exposed concrete, so BBII does not believe patching procedures are necessary.			
Please confirm that the above submittals are not necessary for the BSE contract.			Please confirm that the above submittals are not necessary for the BSE contract.			



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<b>T-0063</b>	<b>BSE - Request for Final EIS/EIR for Mitigation and Monitoring</b>	<b>Closed</b>	<b>01</b>	<b>03/16/2011</b>	<b>03/26/2011</b>	<b>03/21/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 01 35 65  BBII has been unable to obtain the report titled "Final EIS/EIR" dated November 29, 2007, as described in specification section 01 35 65, 1.1.A. The report requires the contractor to be responsible for mitigation measures and monitoring requirements that are included in the specification section.  Please provide BBII with this report.					<b>ANSWER:</b>  Reference Specification Section 01 35 65  BBII has been unable to obtain the report titled "Final EIS/EIR" dated November 29, 2007, as described in specification section 01 35 65, 1.1.A. The report requires the contractor to be responsible for mitigation measures and monitoring requirements that are included in the specification section.  Please provide BBII with this report.	
<b>T-0064</b>	<b>BSE - Demolition Contract Backfill Material</b>	<b>Closed</b>	<b>01</b>	<b>03/16/2011</b>	<b>03/26/2011</b>	<b>03/21/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  Reference photos (attached)  It appears that the demolition contractor is leaving large unprocessed rubble along the backside of some of the basement walls (See attached photos). Per the demolition drawings included in BBII's contract, all of the material in this area should be crushed/processed concrete at 3" minus. Handling material that does not meet these requirements will be considered a changed condition. Please advise.					<b>ANSWER:</b>  Reference photos (attached)  It appears that the demolition contractor is leaving large unprocessed rubble along the backside of some of the basement walls (See attached photos). Per the demolition drawings included in BBII's contract, all of the material in this area should be crushed/processed concrete at 3" minus. Handling material that does not meet these requirements will be considered a changed condition. Please advise.	
<b>T-0065</b>	<b>301 Mission Wall - Length of dowels in concrete wall</b>	<b>Closed</b>	<b>01</b>	<b>03/17/2011</b>	<b>03/27/2011</b>	<b>03/24/2011</b>
<b>From:</b> Webcor Construction LP                      David Hungerford						
<b>REQUEST:</b>  Reference: Sheet S-5000, RFI T-0042  The response to RFI T-0042 specifies for the new concrete wall height to be exposed above the existing pavers a minimum 18". To achieve this requirement, the overall concrete wall height must be increased 8", therefore also increasing the length of the dowels that are to be installed. The #8 embedment bars have already been purchased and fabricated. To achieve the higher wall height per					<b>ANSWER:</b>  Reference: Sheet S-5000, RFI T-0042  The response to RFI T-0042 specifies for the new concrete wall height to be exposed above the existing pavers a minimum 18". To achieve this requirement, the overall concrete wall height must be increased 8", therefore also increasing the length of the dowels that are to be installed. The #8 embedment bars have already been purchased and fabricated. To achieve the higher wall	



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T-0066	<b>BSE - Pile Survey for Buttress Area</b>  <b>From:</b> Webcor Construction LP      Nhi Tran	Closed	01	03/21/2011	03/31/2011	04/04/2011
<b>REQUEST:</b>  It is BBII's understanding that EBI has completed their survey of the existing timber piles in the buttress area, including the area that was previously missed.  Please provide BBII with the remaining timber pile survey information, as indicated at the TG03 BSE Design Coordination Meeting.		<b>ANSWER:</b>  It is BBII's understanding that EBI has completed their survey of the existing timber piles in the buttress area, including the area that was previously missed.  Please provide BBII with the remaining timber pile survey information, as indicated at the TG03 BSE Design Coordination Meeting.				



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<b>T-0067</b>	<b>BSE - Joint Preconstruction Survey</b>	<b>Closed</b>	<b>01</b>	<b>03/21/2011</b>	<b>03/31/2011</b>	<b>03/23/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI T-0047		Reference RFI T-0047				
Based on recent discussions, BBII is requesting confirmation of their understanding of Specification Section 01 15 40:		Based on recent discussions, BBII is requesting confirmation of their understanding of Specification Section 01 15 40:				
1. The inside survey of the adjacent buildings will be performed by ARUP and ARUP is in the process of performing these surveys. BBII will attend these surveys to the extent possible. ARUP will also provide monitoring of these buildings, including but not limited to, active crack monitoring. ARUP will make the initial survey and subsequent monitoring information available to BBII. BBII reserves its right to review this information and request to perform its own indoor survey at any of the surveyed buildings. ARUP is solely responsible for the accuracy of the information provided and the continuation of the monitoring effort. ARUP is also responsible for ensuring that the property owners concur with the surveying methods and the results.		1. The inside survey of the adjacent buildings will be performed by ARUP and ARUP is in the process of performing these surveys. BBII will attend these surveys to the extent possible. ARUP will also provide monitoring of these buildings, including but not limited to, active crack monitoring. ARUP will make the initial survey and subsequent monitoring information available to BBII. BBII reserves its right to review this information and request to perform its own indoor survey at any of the surveyed buildings. ARUP is solely responsible for the accuracy of the information provided and the continuation of the monitoring effort. ARUP is also responsible for ensuring that the property owners concur with the surveying methods and the results.				
2. The list of 19 buildings previously provided by BBII is accurate and is in conformance with ARUP's list.		2. The list of 19 buildings previously provided by BBII is accurate and is in conformance with ARUP's list.				
3. The TJPA will arrange for a survey of the outside of these buildings with the attendance of the property owners. BBII will attend with its professional photographer as required by the Specifications.		3. The TJPA will arrange for a survey of the outside of these buildings with the attendance of the property owners. BBII will attend with its professional photographer as required by the Specifications.				
<b>T-0067.1</b>	<b>BSE - Joint Preconstruction Survey Follow-Up</b>	<b>Closed</b>	<b>01</b>	<b>02/06/2012</b>	<b>02/16/2012</b>	<b>02/15/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Per 01 15 40 and confirmed within RFI #T-067: ARUP is to provide monitoring information from adjacent buildings including but not limited to, active crack monitoring. ARUP will make the initial survey and subsequent monitoring information available to BBII. Please provide this information.		Per 01 15 40 and confirmed within RFI #T-067: ARUP is to provide monitoring information from adjacent buildings including but not limited to, active crack monitoring. ARUP will make the initial survey and subsequent monitoring information available to BBII. Please provide this information.				



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<b>T-0067.2</b>	<b>BSE - Monitoring Information for 545 Mission</b>	<b>Closed</b>	<b>01</b>	<b>02/13/2012</b>	<b>02/13/2012</b>	<b>02/16/2012</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b>  Ref RFI T-0067 and T-0067.1  Please provide the monitoring information from 3/23/2011 through 11/01/2011 as agreed to in response to RFI T-0067.					<b>ANSWER:</b>  Ref RFI T-0067 and T-0067.1  Please provide the monitoring information from 3/23/2011 through 11/01/2011 as agreed to in response to RFI T-0067.	
<b>T-0068</b>	<b>BSE - Soil Encountered During Installation of Pile Removal Instrumentation</b>	<b>Closed</b>	<b>01</b>	<b>03/22/2011</b>	<b>04/01/2011</b>	<b>03/25/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  When ARUP was installing their pile removal instrumentation, they recorded the depths of the various soil layers they encountered.  Please provide BBII these depths for the pile extraction work.					<b>ANSWER:</b>  When ARUP was installing their pile removal instrumentation, they recorded the depths of the various soil layers they encountered.  Please provide BBII these depths for the pile extraction work.	
<b>T-0069</b>	<b>BSE - Revised Shoring Wall Layout Clarification</b>	<b>Closed</b>	<b>01</b>	<b>03/23/2011</b>	<b>04/02/2011</b>	<b>03/28/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  BBII believes there is an issue with some of the information provided regarding the revised shoring wall layout.  The following information was provided on drawing SKGT-0001-R1:  - The (x, y) distances of the intersection of the LOL's of segments X1-1 and R2-1 (Point P on attached sketch) from the intersection of 1-line and J-line: (x, y) = (73'-2 1/4", 166'-4"). - The (x, y) distances of the radial center of segment R2-1 (Point C on attached sketch) from the intersection of 1-line and J-line: (x, y) = (490'-7 1/4", 640'-10 1/4"). &#61607; The radius of the LOL of segment R2-1 as 633'-6".  The distance between the point P and point C can be calculated with the above information: &#61607; &#916;X = 490'-7 1/4" minus 73'-2 1/4" = 417'-5" =					<b>ANSWER:</b>  BBII believes there is an issue with some of the information provided regarding the revised shoring wall layout.  The following information was provided on drawing SKGT-0001-R1:  - The (x, y) distances of the intersection of the LOL's of segments X1-1 and R2-1 (Point P on attached sketch) from the intersection of 1-line and J-line: (x, y) = (73'-2 1/4", 166'-4"). - The (x, y) distances of the radial center of segment R2-1 (Point C on attached sketch) from the intersection of 1-line and J-line: (x, y) = (490'-7 1/4", 640'-10 1/4"). &#61607; The radius of the LOL of segment R2-1 as 633'-6".  The distance between the point P and point C can be calculated with the above information:	



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417.417 &#1607; &#916;Y = 640'-10¼" minus 166'-4" = 474'-6¼" = 474.521 &#1607; D = (&#916;X2 + &#916;Y2)1/2 = (417.4172 + 474.5212)1/2 = 632.053'	Using the distances provided on SKGT-0001-R1 gives a distance of 632.053' between point P and C. This distance must be 633'-6" because it lies along segment R2-1 and the radius of the arc is given. There must be an error in either the radius or one of the other given dimensions. BBII requests an expedited response as this information is critical to our work.			&#1607; &#916;X = 490'-7¼" minus 73'-2¼" = 417'-5" = 417.417 &#1607; &#916;Y = 640'-10¼" minus 166'-4" = 474'-6¼" = 474.521 &#1607; D = (&#916;X2 + &#916;Y2)1/2 = (417.4172 + 474.5212)1/2 = 632.053'		
<b>T-0070</b>	<b>BSE - Excavation Permit for Pre-trenching in the Public Right of Way</b>	<b>Closed</b>	<b>01</b>	<b>03/24/2011</b>	<b>04/04/2011</b>	<b>03/25/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 01 14 10 and attached sheet  BBII would like to confirm the following: - BBII is responsible for applying for Excavation Permits from the San Francisco Department of Public Works for all of the pre-trench excavations in the public right-of-way. - Per Specification Section 01 14 10 Appendix (attached), TJPA will compensate BBII for the excavation permit costs.						
						<b>ANSWER:</b>  Reference Specification Section 01 14 10 and attached sheet  BBII would like to confirm the following: - BBII is responsible for applying for Excavation Permits from the San Francisco Department of Public Works for all of the pre-trench excavations in the public right-of-way. - Per Specification Section 01 14 10 Appendix (attached), TJPA will compensate BBII for the excavation permit costs.
<b>T-0071</b>	<b>RFI T-0071 - 301 Mission Screen Wall - Waterproofing at South face</b>	<b>Closed</b>	<b>CR</b>	<b>03/25/2011</b>	<b>04/04/2011</b>	<b>04/05/2011</b>
<b>From:</b> Webcor Construction LP                      David Hungerford						
<b>REQUEST:</b>  Reference: Attached letter  Please see the attached letter dated March 16, 2011 by Erik Liu of Transworld.						
						<b>ANSWER:</b>  Reference: Attached letter  Please see the attached letter dated March 16, 2011 by Erik Liu of Transworld.



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0072</b>	<b>BSE - Concrete Sidewalk and SD Removal in Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>03/30/2011</b>	<b>04/09/2011</b>	<b>04/11/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference attached photos showing concrete sidewalk and sewer manhole in Zone 4, adjacent to 301 Mission building  The sidewalk and sewer manhole (as seen in the photos) is not in the BSE contract work and will need to be removed prior to pre-trenching. BBI is scheduled to start their pre-trenching activities on 04/11/2011.  Please advise.						<b>ANSWER:</b>  Reference attached photos showing concrete sidewalk and sewer manhole in Zone 4, adjacent to 301 Mission building  The sidewalk and sewer manhole (as seen in the photos) is not in the BSE contract work and will need to be removed prior to pre-trenching. BBI is scheduled to start their pre-trenching activities on 04/11/2011.  Please advise.
<b>T-0073</b>	<b>BSE - Request for Response Spectra</b>	<b>Closed</b>	<b>01</b>	<b>03/30/2011</b>	<b>04/09/2011</b>	<b>04/07/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 01 53 13  During a meeting with the San Francisco DBI & DPW, it was expressed that BBII must use response spectra generated by ARUP in the design of the temporary bridges. It was also noted that if the bridges are going to be in place for over 5 years, the design must be for a permanent structure and the specified ground motion may not be suitable. Therefore, BBII requests response spectra for a ground motion with a 10% probability of exceedence in 50 years as specified, as well as for a ground motion with a 7.5% probability of exceedence in 75 years.						<b>ANSWER:</b>  Reference Specification Section 01 53 13  During a meeting with the San Francisco DBI & DPW, it was expressed that BBII must use response spectra generated by ARUP in the design of the temporary bridges. It was also noted that if the bridges are going to be in place for over 5 years, the design must be for a permanent structure and the specified ground motion may not be suitable. Therefore, BBII requests response spectra for a ground motion with a 10% probability of exceedence in 50 years as specified, as well as for a ground motion with a 7.5% probability of exceedence in 75 years.
<b>T-0073.1</b>	<b>BSE - Request for Response Spectra</b>	<b>Closed</b>	<b>01</b>	<b>03/30/2011</b>	<b>04/09/2011</b>	<b>04/14/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Response to RFI#T-0073  During a meeting with the San Francisco DBI & DPW, it was expressed that BBII must use response spectra generated by ARUP in the design of the temporary bridges. It was also noted that if the bridges are going to be in place for over 5 years, the design must be for a permanent structure and the specified ground motion may						<b>ANSWER:</b>  Reference Response to RFI#T-0073  During a meeting with the San Francisco DBI & DPW, it was expressed that BBII must use response spectra generated by ARUP in the design of the temporary bridges. It was also noted that if the bridges are going to be in place for over 5 years, the design must be for a permanent structure and the specified ground



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<hr/>						
	not be suitable. Therefore, BBII requests response spectra for a ground motion with a 10% probability of exceedence in 50 years as specified, as well as for a ground motion with a 7.5% probability of exceedence in 75 years.					motion may not be suitable. Therefore, BBII requests response spectra for a ground motion with a 10% probability of exceedence in 50 years as specified, as well as for a ground motion with a 7.5% probability of exceedence in 75 years.
<b>T-0074</b>	<b>301 Mission Wall - Nelson Stud and Stirrup Locations</b>	<b>Closed</b>	<b>CR</b>	<b>04/01/2011</b>	<b>04/11/2011</b>	<b>04/01/2011</b>
<hr/>						
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: RFI T-0027						Reference: RFI T-0027
Per field conversation, please confirm that it is acceptable to install/weld nelson studs at 9" on center at locations in front of the vault intrusions into the concrete stem wall, where the #8 size dowels are also spaced at 9" on center, per RFI T-0027. The Nelson Stud spacing will match dowel embeddment locations. This spacing also facilitates the installation of rebar stirrups and provides two tie points, one being the dowel, and the other the nelson stud.						Per field conversation, please confirm that it is acceptable to install/weld nelson studs at 9" on center at locations in front of the vault intrusions into the concrete stem wall, where the #8 size dowels are also spaced at 9" on center, per RFI T-0027. The Nelson Stud spacing will match dowel embeddment locations. This spacing also facilitates the installation of rebar stirrups and provides two tie points, one being the dowel, and the other the nelson stud.
This work is currently ongoing and immediate confirmation is requested. Please confirm this layout is acceptable.						This work is currently ongoing and immediate confirmation is requested. Please confirm this layout is acceptable.
<hr/>						
<b>T-0075</b>	<b>BSE - Specification Section 32 12 17 and 32 12 18</b>	<b>Closed</b>	<b>01</b>	<b>04/04/2011</b>	<b>04/14/2011</b>	<b>04/05/2011</b>
<hr/>						
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
We noticed that the Specification 32 12 17 at the bid has been revised to 32 12 18 in the IFC Document. 1. Please confirm that the content of the specification "STREET EXCAVATION AND RESTORATION" was unchanged between pre-bid and post-bid. 2. Please confirm that the Trade Subcontractor shall continue to use the Specification Number 32 12 18 and TJPA shall revise the Table of Contents and other specification sections referring to "32 12 17."						We noticed that the Specification 32 12 17 at the bid has been revised to 32 12 18 in the IFC Document. 1. Please confirm that the content of the specification "STREET EXCAVATION AND RESTORATION" was unchanged between pre-bid and post-bid. 2. Please confirm that the Trade Subcontractor shall continue to use the Specification Number 32 12 18 and TJPA shall revise the Table of Contents and other specification sections referring to "32 12 17."





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<b>T-0076</b>	<b>BSE - Footing and Pile Removal at Bent 59 - 61</b>	<b>Closed</b>	<b>01</b>	<b>04/04/2011</b>	<b>04/14/2011</b>	<b>04/11/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  Reference Sheet D-1072, D-1030, D-1046, and D-5103 and Spec Section 01 35 65  Please advise the following as discussed with BBII on 03-28-2011 have been completed per the Demolition Contract: - Bent 59-61 - Removal of columns, footings and timber piles as required to complete 4'x4' x13' excavation below grade complete and backfilled. (Refer to drawings D-1072, D-1030, D-1046).		<b>ANSWER:</b>  Reference Sheet D-1072, D-1030, D-1046, and D-5103 and Spec Section 01 35 65  Please advise the following as discussed with BBII on 03-28-2011 have been completed per the Demolition Contract: - Bent 59-61 - Removal of columns, footings and timber piles as required to complete 4'x4' x13' excavation below grade complete and backfilled. (Refer to drawings D-1072, D-1030, D-1046).				
<b>T-0077</b>	<b>BSE - Monitoring Plans and Data for Zone 4 and Lot N</b>	<b>Closed</b>	<b>01</b>	<b>04/04/2011</b>	<b>04/14/2011</b>	<b>04/11/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 01 35 65  As discussed at the site walk through meeting on 03-28-2011 with BBII, BBII requests a copy of the demolition contract monitoring plan and any data in relation to demolition contract mitigation monitoring of Lot N and Zone 4.		<b>ANSWER:</b>  Reference Specification Section 01 35 65  As discussed at the site walk through meeting on 03-28-2011 with BBII, BBII requests a copy of the demolition contract monitoring plan and any data in relation to demolition contract mitigation monitoring of Lot N and Zone 4.				
<b>T-0078</b>	<b>BSE - Timber Piles Not Yet Surveyed by EBI</b>	<b>Closed</b>	<b>01</b>	<b>04/04/2011</b>	<b>04/14/2011</b>	<b>04/12/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  Reference attached photos and sketch  While BBII was excavating the trial pile extraction area and exposing the timber piles on 03/31/11, piles that were not surveyed by EBI were discovered on the eastern side of the TPE area close to pile 215053. Please advise on how to proceed.		<b>ANSWER:</b>  Reference attached photos and sketch  While BBII was excavating the trial pile extraction area and exposing the timber piles on 03/31/11, piles that were not surveyed by EBI were discovered on the eastern side of the TPE area close to pile 215053. Please advise on how to proceed.				
<b>T-0079</b>	<b>BSE - Existing Street Light Footing Locations</b>	<b>Closed</b>	<b>01</b>	<b>04/04/2011</b>	<b>04/14/2011</b>	<b>04/11/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				



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	<p>Reference Specification Section 02 41 01</p> <p>As discussed at the site walk through meeting 03-28-2011 with BBII, the pre-existing street light poles were relocated per demo contract. BBII was told the foundations and timber piles for the pre-existing street lights have not been removed.</p> <p>Please provide BBII with as-built drawings indicating the pre-existing street light locations. Pre-existing streetlight foundations will need to be removed before CDSM wall installation, if a conflict is identified.</p>					
<b>T-0080</b>	<b>BSE - Additional Timber Piles Not Surveyed by EBI</b>	<b>Closed</b>	<b>01</b>	<b>04/04/2011</b>	<b>04/14/2011</b>	<b>04/12/2011</b>
	<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference RFI#T-0078 and attached photos and sketch</p> <p>While BBII was excavating the trial pile extraction area and exposing the timber piles on 04/01/2011, piles that were not surveyed by EBI were discovered on the southern side of the TPE area close to piles 215044, 215043 and in the centre of the TPE area at 215054, as shown in the attached drawing. The pile next to 215054 was extracted due to its proximity to 215054. A total of 7 additional piles have now been discovered to date. Please advise BBII on how to proceed.</p>					
						<p><b>ANSWER:</b></p> <p>Reference RFI#T-0078 and attached photos and sketch</p> <p>While BBII was excavating the trial pile extraction area and exposing the timber piles on 04/01/2011, piles that were not surveyed by EBI were discovered on the southern side of the TPE area close to piles 215044, 215043 and in the centre of the TPE area at 215054, as shown in the attached drawing. The pile next to 215054 was extracted due to its proximity to 215054. A total of 7 additional piles have now been discovered to date. Please advise BBII on how to proceed.</p>
<b>T-0081</b>	<b>BSE - Revised Shoring Wall Alignment Dimension</b>	<b>Closed</b>	<b>01</b>	<b>04/05/2011</b>	<b>04/15/2011</b>	<b>04/11/2011</b>
	<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference attached sheet SKGT-0001-R1</p> <p>The dimension from gridline J to the intersection of wall segments 1-1 and X1-1 was not updated for the revised shoring wall alignment - see attached drawing for reference. Please provide the correct dimension.</p>					
						<p><b>ANSWER:</b></p> <p>Reference attached sheet SKGT-0001-R1</p> <p>The dimension from gridline J to the intersection of wall segments 1-1 and X1-1 was not updated for the revised shoring wall alignment - see attached drawing for reference. Please provide the correct dimension.</p>



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T-0082	BSE - Hazardous Material Removed From Site	Closed	01	04/05/2011	04/15/2011	04/11/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference Specification Section 00 03 35		Reference Specification Section 00 03 35				
Please confirm that all hazardous material has been removed from site per the extent of demolition contract drawings for Zone 4 and Lot N.		Please confirm that all hazardous material has been removed from site per the extent of demolition contract drawings for Zone 4 and Lot N.				
T-0083	BSE - Existing Utilities Decommissioning Lot N and Zone 4	Closed	01	04/05/2011	04/15/2011	04/13/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference Sheet D-2230 and Specification Section 02 41 01		Reference Sheet D-2230 and Specification Section 02 41 01				
Please provide as built drawings for all decommissioned utilities in Lot N and Zone 4 to BBII.		Please provide as built drawings for all decommissioned utilities in Lot N and Zone 4 to BBII.				
T-0083.1	BSE - Existing Utilities Decommissioning Lot N and Zone 4	Closed	01	04/05/2011	04/15/2011	05/24/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference Response to RFI#T-0083, Sheet D-2230 and Specification Section 02 41 01		Reference Response to RFI#T-0083, Sheet D-2230 and Specification Section 02 41 01				
The following response of RFI T-0083 is not acceptable and will become out of control of the RFI documentation process: "they are available in Demolition Contractor's trailer office for your viewing."		The following response of RFI T-0083 is not acceptable and will become out of control of the RFI documentation process: "they are available in Demolition Contractor's trailer office for your viewing."				
Please provide BBI with as built drawings for all utilities which has been decommissioned to date in Lot N and Zone 4 to BBII.		Please provide BBI with as built drawings for all utilities which has been decommissioned to date in Lot N and Zone 4 to BBII.				
T-0084	BSE - Existing Storm Drains Decommissioning in Lot N	Closed	01	04/05/2011	04/15/2011	04/11/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference Sheet D-2230 and Specification Section 02 41 01		Reference Sheet D-2230 and Specification Section 02 41 01				



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-0084.1	<p>There are 2 existing storm drain basins in Lot N not yet decommissioned. Please provide BBII the status of decommissioning or modification of these lines.</p> <p><b>BSE - Existing Storm Drains Decommissioning in Lot N</b></p> <p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference RFI#T-0084, Drawing Sheet D-2230, and Specification Section 02 41 01</p> <p>RFI response T-0084 has not provided clear direction for decommissioning these SD lines. The drawings indicate that the SD drain flows towards Beale Street and will conflict with the CDSM wall. Please advise on status for decommissioning the above SD lines.</p>	Closed	01	04/21/2011	05/01/2011	05/02/2011
	<p><b>ANSWER:</b></p> <p>Reference RFI#T-0084, Drawing Sheet D-2230, and Specification Section 02 41 01</p> <p>RFI response T-0084 has not provided clear direction for decommissioning these SD lines. The drawings indicate that the SD drain flows towards Beale Street and will conflict with the CDSM wall. Please advise on status for decommissioning the above SD lines.</p>					
T-0085	<p><b>BSE - Existing Site Conditions Lot N</b></p> <p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference Specification Section 01 15 40</p> <p>Prior to demolition work Lot N surface consisted of asphalt paving, however a majority of the Lot is not currently paved. BBII assumes that the lot will be restored to its original condition. Please confirm</p>	Closed	01	04/05/2011	04/15/2011	04/11/2011
	<p><b>ANSWER:</b></p> <p>Reference Specification Section 01 15 40</p> <p>Prior to demolition work Lot N surface consisted of asphalt paving, however a majority of the Lot is not currently paved. BBII assumes that the lot will be restored to its original condition. Please confirm</p>					



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<b>T-0086</b>	<b>BSE - Clean Debris From Adjacent Buildings To Lot N and Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>04/05/2011</b>	<b>04/15/2011</b>	<b>04/11/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 01 15 40  Please confirm that demolition contractor has satisfied the requirement to clean all dust and debris generated by demolition contract to the satisfaction of the adjacent building owners, and BBII will only be responsible for cleaning dust and debris generated by BBII during its own operations, after the turnover of these are completed.						<b>ANSWER:</b>  Reference Specification Section 01 15 40  Please confirm that demolition contractor has satisfied the requirement to clean all dust and debris generated by demolition contract to the satisfaction of the adjacent building owners, and BBII will only be responsible for cleaning dust and debris generated by BBII during its own operations, after the turnover of these are completed.
<b>T-0087</b>	<b>BSE - Zone 4 Gate</b>	<b>Closed</b>	<b>01</b>	<b>04/05/2011</b>	<b>04/15/2011</b>	<b>04/11/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Demo Contract Drawings  Per note 5 on drawing D-1006 of the demolition contract, each discreet fenced area shall have a minimum of two 16ft gates at the conclusion of demolition work. Currently, zone 4 only has one gate in place. BBII requests an additional gate be provided on the Fremont St. side of zone 4. BBII is available to meet and coordinate an ideal location.						<b>ANSWER:</b>  Reference Demo Contract Drawings  Per note 5 on drawing D-1006 of the demolition contract, each discreet fenced area shall have a minimum of two 16ft gates at the conclusion of demolition work. Currently, zone 4 only has one gate in place. BBII requests an additional gate be provided on the Fremont St. side of zone 4. BBII is available to meet and coordinate an ideal location.
<b>T-0088</b>	<b>BSE - Temporary Shoring Wall and Buttress Conflict</b>	<b>Closed</b>	<b>01</b>	<b>04/06/2011</b>	<b>04/16/2011</b>	<b>04/08/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Sheet GT-2201 and Specification Section 31 63 29  The temporary shoring wall installed under the demolition contract was moved East away from Fremont St. to avoid an unknown existing concrete wall. The as-built alignment of the wall now falls along the edge of the third column (C) of buttress shafts. In an effort to avoid conflicts with column C shafts generated by the revised temporary shoring wall alignment, BBII suggests that the buttress formation be moved 12" East.						<b>ANSWER:</b>  Reference Sheet GT-2201 and Specification Section 31 63 29  The temporary shoring wall installed under the demolition contract was moved East away from Fremont St. to avoid an unknown existing concrete wall. The as-built alignment of the wall now falls along the edge of the third column (C) of buttress shafts. In an effort to avoid conflicts with column C shafts generated by the revised temporary shoring wall alignment, BBII suggests that the buttress formation be moved 12" East.



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Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-0088.2	BSE - Temporary shoring wall and buttress conflict	Closed	CR	04/06/2011	04/27/2011	04/25/2011
From: Webcor/Obayashi Joint Venture      Nhi Tran						
REQUEST:		ANSWER:				
The response for RFI #T-0088.1 was not an acceptable answer to the question.		The response for RFI #T-0088.1 was not an acceptable answer to the question.				
Please provide exact revised layout as required.		Please provide exact revised layout as required.				
The Buttresses have exact Coordinate Locations to define the layout, as shown on GT-2201.		The Buttresses have exact Coordinate Locations to define the layout, as shown on GT-2201.				
The existing coordinates must be changed to reflect the new layout the TJPA desires.		The existing coordinates must be changed to reflect the new layout the TJPA desires.				
History		History				
Information from RFI#T-0088.1		Information from RFI#T-0088.1				
The response for RFI #T-0088 was not an answer to the question		The response for RFI #T-0088 was not an answer to the question				
Please provide an appropriate direction to start preparing the submittal and the work as soon as possible.		Please provide an appropriate direction to start preparing the submittal and the work as soon as possible.				
Answered By: George Metzger		Answered By: George Metzger				
Answered On: 20-Apr-2011		Answered On: 20-Apr-2011				
Answer:		Answer:				
The contractor may relocate the entire buttress structure up to 12 inches east of the design location in order to clear any conflict with the Fremont Street shoring wall.		The contractor may relocate the entire buttress structure up to 12 inches east of the design location in order to clear any conflict with the Fremont Street shoring wall.				
Contractor is requested to identify the new layout and any impacts prior to start of buttress construction.		Contractor is requested to identify the new layout and any impacts prior to start of buttress construction.				
-----		-----				
Information from RFI#T-0088		Information from RFI#T-0088				
Reference Sheet GT-2201 and Specification Section 31 63 29		Reference Sheet GT-2201 and Specification Section 31 63 29				
The temporary shoring wall installed under the demolition contract was moved East away from Fremont St. to avoid an unknown existing concrete wall. The as-built alignment of the wall now falls along the edge of the third column (C) of buttress shafts. In an effort to avoid conflicts with column C shafts generated by the revised temporary shoring wall alignment, BBII suggests that the buttress formation be moved 12" East.		The temporary shoring wall installed under the demolition contract was moved East away from Fremont St. to avoid an unknown existing concrete wall. The as-built alignment of the wall now falls along the edge of the third column (C) of buttress shafts. In an effort to avoid conflicts with column C shafts generated by the revised temporary shoring wall alignment, BBII suggests that the buttress formation be moved 12" East.				
Suggestion		Suggestion				
Cost Impact   Potentially   Cost Amount		Cost Impact   Potentially   Cost Amount				
Schedule Impact   Potentially   Days		Schedule Impact   Potentially   Days				



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Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
	<div><div>Answered By George Metzger</div><div>Date Answered 2011-04-20</div><div>Answer The contractor may relocate the entire buttress structure up to 12 inches east of the design location in order to clear any conflict with the Fremont Street shoring wall. Contractor is requested to identify the new layout and any impacts prior to start of buttress construction.</div></div>					<div><div>Suggestion</div><div>Cost Impact Potentially Cost Amount</div><div>Schedule Impact Potentially Days</div><div>Answered By George Metzger</div><div>Date Answered 2011-04-20</div><div>Answer The contractor may relocate the entire buttress structure up to 12 inches east of the design location in order to clear any conflict with the Fremont Street shoring wall. Contractor is requested to identify the new layout and any impacts prior to start of buttress construction.</div></div>
T-0088.3	BSE - Temporary shoring wall and buttress conflict	Closed	01	04/06/2011	04/27/2011	04/25/2011
	<div><div>From: Webcor Construction LP</div><div>Nhi Tran</div><div>REQUEST:</div><div>The response for RFI #T-0088.1 was not an acceptable answer to the question.</div><div>Please provide exact revised layout as required.</div><div>The Buttresses have exact Coordinate Locations to define the layout, as shown on GT-2201.</div><div>The existing coordinates must be changed to reflect the new layout the TJPA desires.</div><div>History</div><div>Information from RFI#T-0088.1</div><div>The response for RFI #T-0088 was not an answer to the question</div><div>Please provide an appropriate direction to start preparing the submittal and the work as soon as possible.</div><div>Answered By: George Metzger</div><div>Answered On: 20-Apr-2011</div><div>Answer:</div><div>The contractor may relocate the entire buttress structure up to 12 inches east of the design location in order to clear any conflict with the Fremont Street shoring wall.</div><div>Contractor is requested to identify the new layout and any impacts prior to start of buttress construction.</div></div>					<div><div>ANSWER:</div><div>The response for RFI #T-0088.1 was not an acceptable answer to the question.</div><div>Please provide exact revised layout as required.</div><div>The Buttresses have exact Coordinate Locations to define the layout, as shown on GT-2201.</div><div>The existing coordinates must be changed to reflect the new layout the TJPA desires.</div><div>History</div><div>Information from RFI#T-0088.1</div><div>The response for RFI #T-0088 was not an answer to the question</div><div>Please provide an appropriate direction to start preparing the submittal and the work as soon as possible.</div><div>Answered By: George Metzger</div><div>Answered On: 20-Apr-2011</div><div>Answer:</div><div>The contractor may relocate the entire buttress structure up to 12 inches east of the design location in order to clear any conflict with the Fremont Street shoring wall. Contractor is requested to identify the</div></div>





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	<p>----- Information from RFI#T-0088</p> <p>Reference Sheet GT-2201 and Specification Section 31 63 29</p> <p>The temporary shoring wall installed under the demolition contract was moved East away from Fremont St. to avoid an unknown existing concrete wall. The as-built alignment of the wall now falls along the edge of the third column (C) of buttress shafts. In an effort to avoid conflicts with column C shafts generated by the revised temporary shoring wall alignment, BBII suggests that the buttress formation be moved 12" East.</p> <p>Suggestion Cost Impact Potentially Cost Amount Schedule Impact Potentially Days Answered By George Metzger Date Answered 2011-04-20 Answer The contractor may relocate the entire buttress structure up to 12 inches east of the design location in order to clear any conflict with the Fremont Street shoring wall. Contractor is requested to identify the new layout and any impacts prior to start of buttress construction.</p>					
	<p>----- Information from RFI#T-0088</p> <p>Reference Sheet GT-2201 and Specification Section 31 63 29</p> <p>The temporary shoring wall installed under the demolition contract was moved East away from Fremont St. to avoid an unknown existing concrete wall. The as-built alignment of the wall now falls along the edge of the third column (C) of buttress shafts. In an effort to avoid conflicts with column C shafts generated by the revised temporary shoring wall alignment, BBII suggests that the buttress formation be moved 12" East.</p> <p>Suggestion Cost Impact Potentially Cost Amount Schedule Impact Potentially Days Answered By George Metzger Date Answered 2011-04-20 Answer The contractor may relocate the entire buttress structure up to 12 inches east of the design location in order to clear any conflict with the Fremont Street shoring wall. Contractor is requested to identify the new layout and any impacts prior to start of buttress construction.</p>					
T-0089	BSE - Existing Asphalt and Concrete Removed Zone 4	Closed	01	04/06/2011	04/16/2011	04/11/2011
	From: Webcor Construction LP Nhi Tran					
	REQUEST: Reference Sheet D-1001 and Demo Contract Dwgs D-1060, D-1072 and attached photos  Please see attached photos showing asphalt pavement at the entrance to zone 4 on the northeast corner. The referenced asphalt driveway is not in the BSE contract work and will need to be removed. Please advise.					
	ANSWER: Reference Sheet D-1001 and Demo Contract Dwgs D-1060, D-1072 and attached photos  Please see attached photos showing asphalt pavement at the entrance to zone 4 on the northeast corner. The referenced asphalt driveway is not in the BSE contract work and will need to be removed. Please advise.					



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<b>T-0090</b>	<b>BSE - Timber Piles Not Surveyed By EBI 04/04/11</b>	<b>Closed</b>	<b>01</b>	<b>04/06/2011</b>	<b>04/16/2011</b>	<b>04/13/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b> Reference attached photos and sketch  While BBII were excavating the trial pile extraction area and exposing the timber piles on 04/04/2011, piles that were not surveyed by EBI were discovered on the eastern side of the TPE area close to pile 215053 and in the western side of the TPE area at 215055 as shown in the attached drawing. The pile next to 215055 was extracted due to its proximity to 215055. A total of 10 additional piles have now been discovered to date. Please advise on how to proceed.						<b>ANSWER:</b> Reference attached photos and sketch  While BBII were excavating the trial pile extraction area and exposing the timber piles on 04/04/2011, piles that were not surveyed by EBI were discovered on the eastern side of the TPE area close to pile 215053 and in the western side of the TPE area at 215055 as shown in the attached drawing. The pile next to 215055 was extracted due to its proximity to 215055. A total of 10 additional piles have now been discovered to date. Please advise on how to proceed.
<b>T-0091</b>	<b>Reciept of Construction Documents</b>	<b>Closed</b>	<b>CR</b>	<b>04/06/2011</b>	<b>04/16/2011</b>	<b>04/08/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Per the 110325_MSTR_CD_Work_Plan schedule, transmitted to Webcor/Obayashi on March 28, 2011 and discussed in the OAC Meeting on April 6, 2011; confirm the following dates should be implemented in the next monthly schedule update:  1. Webcor/Obayashi will receive the 90% CD documents on August 24, 2011  2. Webcor/Obayashi will receive the 100% CD documents on December 2, 2011						<b>ANSWER:</b> Per the 110325_MSTR_CD_Work_Plan schedule, transmitted to Webcor/Obayashi on March 28, 2011 and discussed in the OAC Meeting on April 6, 2011; confirm the following dates should be implemented in the next monthly schedule update:  1. Webcor/Obayashi will receive the 90% CD documents on August 24, 2011  2. Webcor/Obayashi will receive the 100% CD documents on December 2, 2011
<b>T-0092</b>	<b>BSE - Timber Piles Not Surveyed By EBI 4/5/11</b>	<b>Closed</b>	<b>01</b>	<b>04/06/2011</b>	<b>04/16/2011</b>	<b>04/13/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b> Reference attached photos and sketch  While BBII was excavating the trial pile extraction area and exposing the timber piles on 4/5/11, two further piles that were not surveyed by EBI were discovered on the southern side of the TPE area close to piles 215043 and 215044. Following this, four additional piles to the north west of the area adjacent to 215067 and 215068 as shown						<b>ANSWER:</b> Reference attached photos and sketch  While BBII was excavating the trial pile extraction area and exposing the timber piles on 4/5/11, two further piles that were not surveyed by EBI were discovered on the southern side of the TPE area close to piles 215043 and 215044. Following this, four additional piles to the north west of the area adjacent to 215067



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	<p>in the attached drawing were discovered. A total of 16 additional piles have now been discovered to date. Please advise on how to proceed.</p>					<p>and 215068 as shown in the attached drawing were discovered. A total of 16 additional piles have now been discovered to date. Please advise on how to proceed.</p>
<b>T-0093</b>	<b>BSE - CDSM Wall Segment 35-1 Spacing Confirmation</b>	<b>Closed</b>	<b>01</b>	<b>04/07/2011</b>	<b>04/17/2011</b>	<b>04/08/2011</b>
	<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference Sheets GT-2103, GT-5101 and Specification Section 31 56 13</p> <p>In drawing GT-5101, the spacing of all shoring wall beams is specified as 4'-0". This is reflected in the drawings for all sections of the CDSM shoring wall except the east wall (Wall Segment 35-1). The beam spacing of this Segment (measured in AutoCad) is 3.94728'. This creates a dimension bust of approximately 2.4' over the length of the wall and significant problems based on the auger spacing. Please verify the spacing of beams in Wall Segment 35-1.</p>					<p><b>ANSWER:</b></p> <p>Reference Sheets GT-2103, GT-5101 and Specification Section 31 56 13</p> <p>In drawing GT-5101, the spacing of all shoring wall beams is specified as 4'-0". This is reflected in the drawings for all sections of the CDSM shoring wall except the east wall (Wall Segment 35-1). The beam spacing of this Segment (measured in AutoCad) is 3.94728'. This creates a dimension bust of approximately 2.4' over the length of the wall and significant problems based on the auger spacing. Please verify the spacing of beams in Wall Segment 35-1.</p>
<b>T-0094</b>	<b>BSE - Timber Piles Not Surveyed By EBI 04-06-11</b>	<b>Closed</b>	<b>01</b>	<b>04/08/2011</b>	<b>04/18/2011</b>	<b>04/13/2011</b>
	<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference attached photo and sketch</p> <p>While BBII were excavating the trial pile extraction area and exposing the timber piles on 4/6/11, an additional pile was found close to 215068 as shown on the attached drawing and photos. A total of 17 additional piles have now been discovered to date. Please advise on how to proceed.</p>					<p><b>ANSWER:</b></p> <p>Reference attached photo and sketch</p> <p>While BBII were excavating the trial pile extraction area and exposing the timber piles on 4/6/11, an additional pile was found close to 215068 as shown on the attached drawing and photos. A total of 17 additional piles have now been discovered to date. Please advise on how to proceed.</p>



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<b>T-0095</b>	<b>BSE - Zone 1 CDSM Test Section Relocation</b>	<b>Closed</b>	<b>01</b>	<b>04/11/2011</b>	<b>04/21/2011</b>	<b>04/14/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Sheet GT-2101, Specification Section 31 56 13 and attached drawing  Per discussion with ARUP at the Wednesday April 06, 2011 Design Coordination Meeting, the Engineer was willing to consider relocating the Zone 1 CDSM test panel as shown on Dwg. GT-2101 from Zone 1 and into Zone 2. BBII and DND Construction are therefore proposing to relocate the Zone 1 CDSM test panel to the location shown on the attached drawing, near gridline 10. Please confirm.						<b>ANSWER:</b>  Reference Sheet GT-2101, Specification Section 31 56 13 and attached drawing  Per discussion with ARUP at the Wednesday April 06, 2011 Design Coordination Meeting, the Engineer was willing to consider relocating the Zone 1 CDSM test panel as shown on Dwg. GT-2101 from Zone 1 and into Zone 2. BBII and DND Construction are therefore proposing to relocate the Zone 1 CDSM test panel to the location shown on the attached drawing, near gridline 10. Please confirm.
<b>T-0096</b>	<b>BSE - Old Existing Footing Along 301 Mission in Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>04/11/2011</b>	<b>04/21/2011</b>	<b>04/12/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 02 41 01  During Pre-Trench BBII found an existing footing along the Low Rise 301 Mission wall. The footing consists of bricks and concrete. It also has a perpendicular footing that come out from footing that is parallel to the 301 Mission building wall. BBII has exposed a 20 to 30ft section of this footing (approximately on Grid Line "A" between 30 and 32).  Please advise BBII as to how to proceed.						<b>ANSWER:</b>  Reference Specification Section 02 41 01  During Pre-Trench BBII found an existing footing along the Low Rise 301 Mission wall. The footing consists of bricks and concrete. It also has a perpendicular footing that come out from footing that is parallel to the 301 Mission building wall. BBII has exposed a 20 to 30ft section of this footing (approximately on Grid Line "A" between 30 and 32).  Please advise BBII as to how to proceed.
<b>T-0096.1</b>	<b>BSE - Old Existing Footing Along 301 Mission in Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>04/20/2011</b>	<b>04/30/2011</b>	<b>05/02/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference response to RFI T-0096 and Specification Section 02 41 01  BBII interprets the Response to RFI T-0096 (BBI 0067) as TJPA's approval for the removal of this unforeseen structure. Please confirm.  BBII proposes to follow the method outlined below for the removal of this unforeseen structure. Please confirm in						<b>ANSWER:</b>  Reference response to RFI T-0096 and Specification Section 02 41 01  BBII interprets the Response to RFI T-0096 (BBI 0067) as TJPA's approval for the removal of this unforeseen structure. Please confirm.  BBII proposes to follow the method outlined below for the removal of this unforeseen structure. Please





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T-0098	<p>drainage material has been removed because it was not affixed to the structure. The wall does not have any exterior waterproofing system.</p> <p>Upon installation of the CDSM shoring system, the cementious material will be against this wall. The existing wall is a 5' deep cantilevered beam on the backside of the existing garage shaft for 301 Mission. Does TJPA plan to install any waterproofing along this wall that can tolerate the installation of a CDSM shoring system?</p> <p>Please advise BBII of the TJPA's plan for waterproofing of this building.</p>	Closed	CR	04/12/2011	04/22/2011	04/21/2011
	<p><b>From:</b> Webcor Construction LP                      David Hungerford</p> <p><b>REQUEST:</b></p> <p>Reference: B/S-5000 and D/A-6000</p> <p>Detail B on sheet S-5000 shows the 10" tube steel centered on the 14" concrete wall below, however this is in conflict with D/A-6000 which shows the steel tube off set from the center of the wall. Please confirm per the 301 Mission subcontractor meeting conversation yesterday, that the tube steel is to be centered on the center of the wall as dimensioned in B/S-5000.</p>					
	<p><b>ANSWER:</b></p> <p>Reference: B/S-5000 and D/A-6000</p> <p>Detail B on sheet S-5000 shows the 10" tube steel centered on the 14" concrete wall below, however this is in conflict with D/A-6000 which shows the steel tube off set from the center of the wall. Please confirm per the 301 Mission subcontractor meeting conversation yesterday, that the tube steel is to be centered on the center of the wall as dimensioned in B/S-5000.</p>					



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T-0099	BSE - Depth of Fremont Street Shoring Wall in Zone 4	Closed	01	04/12/2011	04/22/2011	04/14/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:			ANSWER:			
Reference Sheet D-2203 and attached as-built, photos, and document CPM Activity Impacted - SX-BB42640			Reference Sheet D-2203 and attached as-built, photos, and document CPM Activity Impacted - SX-BB42640			
While excavating adjacent to the existing Fremont street shoring wall as shown on contract drawing D-2203, BBII has found the existing shoring wall's height to be approximately 2' shorter than the 14 feet depth indicated in the as-builts (attached). This wall does not provide adequate shoring height for BBII to excavate and expose the timber piles prior to extraction. (See attached photo for illustration)			While excavating adjacent to the existing Fremont street shoring wall as shown on contract drawing D-2203, BBII has found the existing shoring wall's height to be approximately 2' shorter than the 14 feet depth indicated in the as-builts (attached). This wall does not provide adequate shoring height for BBII to excavate and expose the timber piles prior to extraction. (See attached photo for illustration)			
The contract documents D-2203 and pre-bid Q&A response #182 (also attached) indicate this wall would accommodate the buttress area pile removal, however actual existing field conditions do not provide adequate shored depth			The contract documents D-2203 and pre-bid Q&A response #182 (also attached) indicate this wall would accommodate the buttress area pile removal, however actual existing field conditions do not provide adequate shored depth			
Please provide direction.			Please provide direction.			



Please Advise BBII as to how to proceed.





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<b>T-0100.1</b>	<b>BSE - Slurry Wall Along 301 Mission St Garage</b>	<b>Closed</b>	<b>01</b>	<b>04/20/2011</b>	<b>04/30/2011</b>	<b>05/02/2011</b>
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Nhi Tran</div></div><div><b>REQUEST:</b><p>Reference response to RFI T-0100 and Specification Section 02 41 01</p><p>BBII interprets the Response to RFI#T-0100 (BBI 0070) as TJPA's approval for the removal of this unforeseen structure. Please confirm.</p><p>BBII proposes to follow the method outlined below for the removal of this unforeseen structure. Please confirm in writing that the removal of this unforeseen structure is approved and that provided that it is performed with the method outlined below, no damage to adjacent buildings will occur.</p><p>Pre Trench Obstruction Removal Method</p><p>Location: Parallel along the 301 Mission St. Low Rise (Grid line A, approximately between lines 30 &amp; 34).</p><p>Obstructions: A very large mass of slurry.</p><p>Method: BBII will first expose the obstructions and use an excavator mounted and hand held jackhammer to demolish the large masses into smaller more manageable sizes. An excavator with a bucket will then clear the debris, until the debris is removed from the area of the CDSM Wall location. BBII will chase the obstruction as deep as it goes in order to remove all debris necessary for a clean location to construct the CDSM Wall. Due to the unknown depth of the obstruction, at BBII discretion Sheet Piles or trench boxes may be used to support trench walls. All OSHA approved, safe practices will be used by BBII employees during the Demolition.</p><p>Additional Details: As noted in the RFI response, the Archeologist has already examined the site. BBII (W/O) will notify the TJPA if additional structures or items are encountered.</p></div><div><b>ANSWER:</b><p>Reference response to RFI T-0100 and Specification Section 02 41 01</p><p>BBII interprets the Response to RFI#T-0100 (BBI 0070) as TJPA's approval for the removal of this unforeseen structure. Please confirm.</p><p>BBII proposes to follow the method outlined below for the removal of this unforeseen structure. Please confirm in writing that the removal of this unforeseen structure is approved and that provided that it is performed with the method outlined below, no damage to adjacent buildings will occur.</p><p>Pre Trench Obstruction Removal Method</p><p>Location: Parallel along the 301 Mission St. Low Rise (Grid line A, approximately between lines 30 &amp; 34).</p><p>Obstructions: A very large mass of slurry.</p><p>Method: BBII will first expose the obstructions and use an excavator mounted and hand held jackhammer to demolish the large masses into smaller more manageable sizes. An excavator with a bucket will then clear the debris, until the debris is removed from the area of the CDSM Wall location. BBII will chase the obstruction as deep as it goes in order to remove all debris necessary for a clean location to construct the CDSM Wall. Due to the unknown depth of the obstruction, at BBII discretion Sheet Piles or trench boxes may be used to support trench walls. All OSHA approved, safe practices will be used by BBII employees during the Demolition.</p><p>Additional Details: As noted in the RFI response, the Archeologist has already examined the site. BBII (W/O) will notify the TJPA if additional structures or items are encountered.</p></div></div>						
<b>T-0101</b>	<b>BSE - Pile Extraction Procedure Modification</b>	<b>Closed</b>	<b>01</b>	<b>04/14/2011</b>	<b>04/24/2011</b>	<b>04/15/2011</b>







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<b>T-0103.1</b>	<b>BSE - Existing Concrete Footing Gridline J Between Gridline 26.5-30</b>	<b>Closed</b>	<b>01</b>	<b>04/27/2011</b>	<b>05/07/2011</b>	<b>05/02/2011</b>
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Nhi Tran</div></div><div><b>REQUEST:</b><p>Reference RFI#T-0103 and Specification Section 02 41 01</p><p>BBII interprets the Response to RFI T-0103 (BBI 0074) as TJPA's approval for the removal of this unforeseen structure. Please confirm.</p><p>BBII proposes to follow the method outlined below for the removal of this unforeseen structure. Please confirm in writing that the removal of this unforeseen structure is approved and that provided that it is performed with the method outlined below, no damage to adjacent buildings will occur.</p><p>Pre Trench Obstruction Removal Method</p><p>Location: Parallel along the 177/181 Fremont Street (Grid line J, approximately between lines 26.5-30).</p><p>Obstructions: A large concrete structure.</p><p>Method: BBII will first expose the obstructions and use an excavator mounted and hand held jackhammer to demolish the large masses into smaller more manageable sizes. An excavator with a bucket will then clear the debris, until the debris is removed from the area of the CDSM Wall location. BBII will chase the obstruction as deep as it goes in order to remove all debris necessary for a clean location to construct the CDSM Wall. Due to the unknown depth of the obstruction, at BBII discretion Sheet Piles or trench boxes may be used to support trench walls. All OSHA approved, safe practices will be used by BBII employees during the Demolition.</p><p>Additional Details: As noted in the RFI response, the Archeologist has already examined the site. BBII (W/O) will notify the TJPA if additional structures or items are encountered.</p></div><div><b>ANSWER:</b><p>Reference RFI#T-0103 and Specification Section 02 41 01</p><p>BBII interprets the Response to RFI T-0103 (BBI 0074) as TJPA's approval for the removal of this unforeseen structure. Please confirm.</p><p>BBII proposes to follow the method outlined below for the removal of this unforeseen structure. Please confirm in writing that the removal of this unforeseen structure is approved and that provided that it is performed with the method outlined below, no damage to adjacent buildings will occur.</p><p>Pre Trench Obstruction Removal Method</p><p>Location: Parallel along the 177/181 Fremont Street (Grid line J, approximately between lines 26.5-30).</p><p>Obstructions: A large concrete structure.</p><p>Method: BBII will first expose the obstructions and use an excavator mounted and hand held jackhammer to demolish the large masses into smaller more manageable sizes. An excavator with a bucket will then clear the debris, until the debris is removed from the area of the CDSM Wall location. BBII will chase the obstruction as deep as it goes in order to remove all debris necessary for a clean location to construct the CDSM Wall. Due to the unknown depth of the obstruction, at BBII discretion Sheet Piles or trench boxes may be used to support trench walls. All OSHA approved, safe practices will be used by BBII employees during the Demolition.</p><p>Additional Details: As noted in the RFI response, the Archeologist has already examined the site. BBII (W/O) will notify the TJPA if additional structures or items are encountered.</p></div></div>						
<b>T-0104</b>	<b>BSE - Request for Report (PSI for Caltrans)</b>	<b>Closed</b>	<b>01</b>	<b>04/18/2011</b>	<b>04/28/2011</b>	<b>04/18/2011</b>





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Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
	<div>Reference: E &amp; C/S-5000</div> <div>Please see E &amp; C/S-5000. Transworld has attempted in their shop to set #10 SMS through the structural tube steel, as per plan. The attempt was unsuccessful, therefore Transworld tried the use of a Hilti X-U fastener into the structural steel. Attached are Hilti spec sheets for the X-U Universal Knurled Shank Fastener as well as a photo showing the X-U fastener through the structural steel. Welding is another option for connection to the tube steel. Please advise how Transworld is to fasten the metal stud to the structural tube steel.</div>					
T-0107	<div>BSE - Visual Test in Lieu of Formally Testing for Verticality in CSL Tubes</div> <div>From: Webcor Construction LP                      Nhi Tran</div> <div>REQUEST:</div> <div>Reference RFI#T-0057, Sheet GT-5202, Specification Section 31 63 29, and attached documents CPM Activity Impacted - Buttress Wall</div> <div>Below are three cases (A, B, and C) in which formally testing for verticality on CSL tubes, BBII argues would prove to be highly unusual and counter-productive:</div> <div>A. Specification Section 31.63.29.1.3 states "The contractor shall perform a test to determine verticality of the steel tubes, or drilled holes, that are going to be used for the sonic tests." Balfour Beatty has been advised by a number of testing firms that verticality tests cannot be performed on steel access tubes as well as piles reinforced with steel. Magnetic interference from steel reinforcement and steel tubes will cause the instrument to not function properly. BBII has also been advised by Terracon (please see attached email from Dextra), a reputable CSL testing firm that there are currently no known cases in the US where verticality of CSL tubes in steel reinforced piles have been formally tested.</div> <div>B. Attached is a case study that details the investigation of debonding that occurs when using PVC as CSL access tubes. The results of this study clearly show the use of</div>	Closed	01	04/20/2011	04/30/2011	04/22/2011
	<div>Reference: E &amp; C/S-5000</div> <div>Please see E &amp; C/S-5000. Transworld has attempted in their shop to set #10 SMS through the structural tube steel, as per plan. The attempt was unsuccessful, therefore Transworld tried the use of a Hilti X-U fastener into the structural steel. Attached are Hilti spec sheets for the X-U Universal Knurled Shank Fastener as well as a photo showing the X-U fastener through the structural steel. Welding is another option for connection to the tube steel. Please advise how Transworld is to fasten the metal stud to the structural tube steel.</div>					
	<div>ANSWER:</div> <div>Reference RFI#T-0057, Sheet GT-5202, Specification Section 31 63 29, and attached documents CPM Activity Impacted - Buttress Wall</div> <div>Below are three cases (A, B, and C) in which formally testing for verticality on CSL tubes, BBII argues would prove to be highly unusual and counter-productive:</div> <div>A. Specification Section 31.63.29.1.3 states "The contractor shall perform a test to determine verticality of the steel tubes, or drilled holes, that are going to be used for the sonic tests." Balfour Beatty has been advised by a number of testing firms that verticality tests cannot be performed on steel access tubes as well as piles reinforced with steel. Magnetic interference from steel reinforcement and steel tubes will cause the instrument to not function properly. BBII has also been advised by Terracon (please see attached email from Dextra), a reputable CSL testing firm that there are currently no known cases in the US where verticality of CSL tubes in steel reinforced piles have been formally tested.</div> <div>B. Attached is a case study that details the investigation of debonding that occurs when using</div>					



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	<p>steel tubes (BBII is proposing to use Sonitec tubes) should be preferred over PVC.</p> <p>C. After doing some research, the closest we came to find any mention of verticality in CSL tubes was this excerpt from EPA's website which states, "If the CSL access tubes are not installed in a near-vertical position and/or the distance between them varies significantly along the length of the shaft, errors in velocity calculations may occur." Judging by this approach to verticality in CSL tubes in most specs, BBII concludes that parallelism and symmetry between tubes are more important factors in ensuring accurate CSL test readings.</p> <p>In summary, BBII in lieu of formally testing the CSL tubes for verticality will perform a visual test making sure that the tubes are symmetrical (equally spaced) in a circle and parallel. This is the most important inspection to ensure accurate pulse readings.</p> <p>Please confirm that this is acceptable.</p>					<p>PVC as CSL access tubes. The results of this study clearly show the use of steel tubes (BBII is proposing to use Sonitec tubes) should be preferred over PVC.</p> <p>C. After doing some research, the closest we came to find any mention of verticality in CSL tubes was this excerpt from EPA's website which states, "If the CSL access tubes are not installed in a near-vertical position and/or the distance between them varies significantly along the length of the shaft, errors in velocity calculations may occur." Judging by this approach to verticality in CSL tubes in most specs, BBII concludes that parallelism and symmetry between tubes are more important factors in ensuring accurate CSL test readings.</p> <p>In summary, BBII in lieu of formally testing the CSL tubes for verticality will perform a visual test making sure that the tubes are symmetrical (equally spaced) in a circle and parallel. This is the most important inspection to ensure accurate pulse readings.</p> <p>Please confirm that this is acceptable.</p>
T-0108	<b>BSE - Building Adjacent Zone 3 Clean From Dust and Debris Generated By Demoli Closed</b>  From: Webcor Construction LP                      Nhi Tran  <b>REQUEST:</b>  Reference Specification Section 01 15 40  Please confirm that the demolition contractor has satisfied the requirement to clean all dust and debris generated by demolition contract to the satisfaction of the adjacent building owners, and BBII will only be responsible for cleaning dust and debris generated by BBII during its own operations, after the turnover of these are completed.		01	04/20/2011	04/30/2011	04/29/2011
						<b>ANSWER:</b>  Reference Specification Section 01 15 40  Please confirm that the demolition contractor has satisfied the requirement to clean all dust and debris generated by demolition contract to the satisfaction of the adjacent building owners, and BBII will only be responsible for cleaning dust and debris generated by BBII during its own operations, after the turnover of these are completed.
T-0108.1	<b>BSE - Building Adjacent Zone 3 Clean From Dust and Debris Generated By Demoli Closed</b>  From: Webcor Construction LP                      Nhi Tran  <b>REQUEST:</b>		01	05/04/2011	05/14/2011	05/18/2011
						<b>ANSWER:</b>





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	Reference response to RFI#T-0108 and Specification Section 01 15 40  W/O requests information on the measures used to clean the adjacent structures  ----- RFI#T-0108 - BSE - Building Adjacent Zone 3 Clean From Dust and Debris Generated By Demolition Work  Question - Reference Specification Section 01 15 40 Please confirm that the demolition contractor has satisfied the requirement to clean all dust and debris generated by demolition contract to the satisfaction of the adjacent building owners, and BBII will only be responsible for cleaning dust and debris generated by BBII during its own operations, after the turnover of these are completed.  Response - Confirmed. Demolition contractor has satisfied the requirement to clean all dust and debris generated by demolition contract to the satisfaction of the adjacent building owners to date. This was confirmed through conversation with both EBi and Singer Associates.					
T-0108.2	BSE - Building Adjacent Zone 3 Clean From Dust and Debris Generated By Demoli	Closed				
From: Webcor Construction LP	Nhi Tran					
REQUEST:	Reference response to RFI#T-0108, RFI#T-0108.1 and Specification Section 01 15 40  The response to RFI#T-0108.1 did not provide the requested information.  W/O requests information on the measures used to clean the adjacent structures					
	Reference response to RFI#T-0108 and Specification Section 01 15 40  W/O requests information on the measures used to clean the adjacent structures  ----- RFI#T-0108 - BSE - Building Adjacent Zone 3 Clean From Dust and Debris Generated By Demolition Work  Question - Reference Specification Section 01 15 40 Please confirm that the demolition contractor has satisfied the requirement to clean all dust and debris generated by demolition contract to the satisfaction of the adjacent building owners, and BBII will only be responsible for cleaning dust and debris generated by BBII during its own operations, after the turnover of these are completed.  Response - Confirmed. Demolition contractor has satisfied the requirement to clean all dust and debris generated by demolition contract to the satisfaction of the adjacent building owners to date. This was confirmed through conversation with both EBi and Singer Associates.					
01				05/04/2011	05/14/2011	05/27/2011
ANSWER:	Reference response to RFI#T-0108, RFI#T-0108.1 and Specification Section 01 15 40  The response to RFI#T-0108.1 did not provide the requested information.  W/O requests information on the measures used to clean the adjacent structures					







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<b>T-0110</b>	<b>BSE - Existing Utility Decommissioning Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>04/22/2011</b>	<b>05/02/2011</b>	<b>05/02/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference RFI#T-0083, Drawing Sheet D-2230, and Specification Section 02 41 01  RFI response to RFI#T-0083 issued on 4-15-2011 has not provided direction for decommissioning or abandoning these utilities per BBII drawing # D-2230 Note 2  Please advise on decommissioning the utilities after dewatering work has been completed.		<b>ANSWER:</b>  Reference RFI#T-0083, Drawing Sheet D-2230, and Specification Section 02 41 01  RFI response to RFI#T-0083 issued on 4-15-2011 has not provided direction for decommissioning or abandoning these utilities per BBII drawing # D-2230 Note 2  Please advise on decommissioning the utilities after dewatering work has been completed.				
<b>T-0111</b>	<b>301 Mission Wall - Torque Spec</b>	<b>Closed</b>	<b>CR</b>	<b>04/22/2011</b>	<b>05/02/2011</b>	<b>04/28/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: S-5000  In regards to the structural steel bolts at the 301 Mission Wall, please confirm that the torque spec is 150 ft-lbs, per attached email.		<b>ANSWER:</b>  Reference: S-5000  In regards to the structural steel bolts at the 301 Mission Wall, please confirm that the torque spec is 150 ft-lbs, per attached email.				
<b>T-0112</b>	<b>BSE - Project Control</b>	<b>Closed</b>	<b>01</b>	<b>04/22/2011</b>	<b>05/02/2011</b>	<b>05/10/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Sheet GT-0100 and Specification Section 01 10 50  Drawing GT-0100 shows four control points. BBII's surveyor, KCA Engineers, have surveyed their locations and found the following: 1) Survey Control Point #101: This point has been damaged - the brass disk is missing, though the rivet remains in the concrete sidewalk. There are score lines in the concrete BBII assumes would intersect on the brass disk. 2) Project Benchmark Point #54: KCA was able to locate this point. Please confirm that it is acceptable to use the coordinates of this point for horizontal control, even though it is listed as a benchmark. 3) Survey Control Point #106: KCA was unable to locate		<b>ANSWER:</b>  Reference Sheet GT-0100 and Specification Section 01 10 50  Drawing GT-0100 shows four control points. BBII's surveyor, KCA Engineers, have surveyed their locations and found the following: 1) Survey Control Point #101: This point has been damaged - the brass disk is missing, though the rivet remains in the concrete sidewalk. There are score lines in the concrete BBII assumes would intersect on the brass disk. 2) Project Benchmark Point #54: KCA was able to locate this point. Please confirm that it is acceptable to use the coordinates of this point for horizontal control, even though it is listed as a benchmark. 3) Survey Control Point #106: KCA was unable to				



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	<p>this point. 4) Survey Control Point #105: KCA was able to locate this point.</p> <p>With the current condition of the provided control points, KCA is not able to do a hard check on their survey work.</p> <p>Please confirm that all the control points above may be used for the TG03 BSE Trade Package. Please reset the damaged or missing points for KCA's use.</p>					
T-0112.1	<p><b>BSE - Project Control</b></p> <p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference RFI#T-0112, Transmittal No. 140-01593, Sheet GT-0100, Specification Section 01 10 50, and attached document</p> <p>Chaudhary's Transbay "Survey Grid Control Document" was transmitted to Ed Sum (TJPA) and Agnes Katanics (URS) on 5/18/11 (transmittal #140-01593, attached) following a meeting which took place on 5/17/11 with URS, F3, DPA and TJPA. In an effort to confirm the four survey control points shown on GT-0100, Chaudhary discovered that Point #101 and Point #106 were missing.</p> <p>Due to the missing points, W/O requests TJPA to either approve Chaudhary's Survey Grid Control Document included as part of transmittal #140-01593, or have the monuments missing from GT-0100 replaced.</p>	Closed	01	05/20/2011	05/30/2011	05/24/2011
	<p><b>ANSWER:</b></p> <p>Reference RFI#T-0112, Transmittal No. 140-01593, Sheet GT-0100, Specification Section 01 10 50, and attached document</p> <p>Chaudhary's Transbay "Survey Grid Control Document" was transmitted to Ed Sum (TJPA) and Agnes Katanics (URS) on 5/18/11 (transmittal #140-01593, attached) following a meeting which took place on 5/17/11 with URS, F3, DPA and TJPA. In an effort to confirm the four survey control points shown on GT-0100, Chaudhary discovered that Point #101 and Point #106 were missing.</p> <p>Due to the missing points, W/O requests TJPA to either approve Chaudhary's Survey Grid Control Document included as part of transmittal #140-01593, or have the monuments missing from GT-0100 replaced.</p>					



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T-0112.2	BSE - Project Control	Closed	CR	07/14/2011	07/24/2011	07/14/2011
From: Webcor Construction LP                      Tim Maxwell						
REQUEST:		ANSWER:				
Reference RFI #T-0112.1 and attached drawing		Reference RFI #T-0112.1 and attached drawing				
Last month Webcor/Obayashi was requested to mark an alleged property line @ 199 Fremont between Beale and Fremont streets per the 12-10-2008 CAD file data provided by the Bruce Storrs of DPW. Chaudhary & Associates completed the task and the results were forwarded for TJPA review on June 20, 2011 via Transmittal # 140-01864. In that transmittal it was recommended that alleged Property Line (PL) data points as indicated within the attached (coordinates added) be presented to Bruce Storrs of DPW for verification of PL data accuracy. Has this been accomplished and, if so, what was the outcome?		Last month Webcor/Obayashi was requested to mark an alleged property line @ 199 Fremont between Beale and Fremont streets per the 12-10-2008 CAD file data provided by the Bruce Storrs of DPW. Chaudhary & Associates completed the task and the results were forwarded for TJPA review on June 20, 2011 via Transmittal # 140-01864. In that transmittal it was recommended that alleged Property Line (PL) data points as indicated within the attached (coordinates added) be presented to Bruce Storrs of DPW for verification of PL data accuracy. Has this been accomplished and, if so, what was the outcome?				
Be advised that as previously confirmed in RFI #T- 112.1 Webcor/Obayashi is ONLY using Grid Control for construction reference, layout and staking.		Be advised that as previously confirmed in RFI #T- 112.1 Webcor/Obayashi is ONLY using Grid Control for construction reference, layout and staking.				
T-0113	BSE - Unforeseen Object - Metal Casing In Production Pile Extraction Area	Closed	01	04/22/2011	05/02/2011	04/25/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference attached sketch and photo		Reference attached sketch and photo				
While BBII was excavating the production pile extraction area and exposing the timber piles on 4/19/11, a metal casing was discovered close to pile 302050. Please advise on how to proceed.		While BBII was excavating the production pile extraction area and exposing the timber piles on 4/19/11, a metal casing was discovered close to pile 302050. Please advise on how to proceed.				
T-0114	BSE - Monitoring Plans and Data for Zone 3	Closed	01	04/27/2011	05/07/2011	05/12/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference Specification Section 01 35 65		Reference Specification Section 01 35 65				
As discussed at the site walk through meeting 4-18-2011; BBII requests a copy of the demolition contract monitoring		As discussed at the site walk through meeting 4-18-2011; BBII requests a copy of the demolition contract				



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	plan and any data in relation to demolition contract mitigation monitoring of Zone 3.				monitoring plan and any data in relation to demolition contract mitigation monitoring of Zone 3.	
<b>T-0115</b>	<b>BSE - Hazardous Material Removed From Site in Zone 3</b>	<b>Closed</b>	<b>01</b>	<b>04/27/2011</b>	<b>05/07/2011</b>	<b>05/02/2011</b>
<b>From:</b> Webcor Construction LP Nhi Tran						
<b>REQUEST:</b> Reference Specification Section 00 03 35  Please confirm that all hazardous material has been removed from site per the extent of demolition contract drawings for zones 3.					<b>ANSWER:</b> Reference Specification Section 00 03 35  Please confirm that all hazardous material has been removed from site per the extent of demolition contract drawings for zones 3.	
<b>T-0116</b>	<b>BSE - Demolition Contract Drawings</b>	<b>Closed</b>	<b>01</b>	<b>04/27/2011</b>	<b>05/07/2011</b>	<b>05/02/2011</b>
<b>From:</b> Webcor Construction LP Nhi Tran						
<b>REQUEST:</b> Please supply BBII with an electronic copy (PDF), of the 'issued for construction' drawings for the demolition contract (EBI).					<b>ANSWER:</b> Please supply BBII with an electronic copy (PDF), of the 'issued for construction' drawings for the demolition contract (EBI).	
<b>T-0116.1</b>	<b>BSE - Demolition Contract Drawings</b>	<b>Closed</b>	<b>01</b>	<b>05/03/2011</b>	<b>05/13/2011</b>	<b>05/03/2011</b>
<b>From:</b> Webcor Construction LP Nhi Tran						
<b>REQUEST:</b> Reference response to RFI#T-0116  Webcor-Obayashi cannot verify "issued for construction drawings" in PDF format for the demolition contract in the past communications. If the confirmed drawing set was sent to Webcor-Obayashi before, please let us know the transmittal number and the date. If not, please send us the drawing set immediately. ----- Please supply BBII with an electronic copy (PDF), of the 'issued for construction' drawings for the demolition contract (EBI).					<b>ANSWER:</b> Reference response to RFI#T-0116  Webcor-Obayashi cannot verify "issued for construction drawings" in PDF format for the demolition contract in the past communications. If the confirmed drawing set was sent to Webcor-Obayashi before, please let us know the transmittal number and the date. If not, please send us the drawing set immediately. ----- Please supply BBII with an electronic copy (PDF), of the 'issued for construction' drawings for the demolition contract (EBI).	



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<b>T-0117</b>	<b>BSE - As-built Drawings for Utility Decommissioning in Zone 3</b>	<b>Closed</b>	<b>01</b>	<b>04/27/2011</b>	<b>05/07/2011</b>	<b>05/02/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Demo Contract Drawing Sheets D-1202,D-1203, D-1204, D1205, D1206 and Specification Section 02 41 01  Please provide as-built drawings for all utilities that have been decommissioned, or cut and capped per the demolition contract for Zone 3.						<b>ANSWER:</b>  Reference Demo Contract Drawing Sheets D-1202,D-1203, D-1204, D1205, D1206 and Specification Section 02 41 01  Please provide as-built drawings for all utilities that have been decommissioned, or cut and capped per the demolition contract for Zone 3.
<b>T-0118</b>	<b>BSE - Crash Cushion Modules on Natoma &amp; Minna Street</b>	<b>Closed</b>	<b>01</b>	<b>04/27/2011</b>	<b>05/07/2011</b>	<b>05/02/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Demo Contract Drawing Sheet D-1007 - Note 5  Currently the crash cushion or k-rail as specified in the Demo Drawing D-1007 note 5 has not been installed. Please confirm the above will be installed by the demo contractor.						<b>ANSWER:</b>  Reference Demo Contract Drawing Sheet D-1007 - Note 5  Currently the crash cushion or k-rail as specified in the Demo Drawing D-1007 note 5 has not been installed. Please confirm the above will be installed by the demo contractor.
<b>T-0119</b>	<b>301 Mission Wall - Metal Stud Layout Alignment</b>	<b>Closed</b>	<b>CR</b>	<b>04/28/2011</b>	<b>05/08/2011</b>	<b>05/05/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: RFI T-0098, Sheet A-6000  Per response to RFI T-0098, the 10" x 10" tube steel columns are to be set in the center of the 14" concrete wall. The architectural drawings (sheet A-6000 dated 11/04/10) show 10" metal studs aligning with the 10" tube steel, however, per response to RFI T-0098, the tube steel is to shift in the architectural drawings 1/2" and align in the center of the concrete wall. Please confirm that the metal studs will remain per plan, and not shift as the steel tube has.						<b>ANSWER:</b>  Reference: RFI T-0098, Sheet A-6000  Per response to RFI T-0098, the 10" x 10" tube steel columns are to be set in the center of the 14" concrete wall. The architectural drawings (sheet A-6000 dated 11/04/10) show 10" metal studs aligning with the 10" tube steel, however, per response to RFI T-0098, the tube steel is to shift in the architectural drawings 1/2" and align in the center of the concrete wall. Please confirm that the metal studs will remain per plan, and not shift as the steel tube has.



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<b>T-0120</b>	<b>301 Mission Wall - Stone Panel Layout</b>	<b>Closed</b>	<b>CR</b>	<b>04/27/2011</b>	<b>05/07/2011</b>	<b>05/20/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: RFI T-0042  Per RFI T-0042, the concrete wall height increased to achieve a min 18" above the finished paver surface. Please clarify if the exposed concrete areas shown on A-5000 are to be min 18" above the pavers. If so, the 1st stone above the exposed concrete would have to be trimmed. Please clarify.						<b>ANSWER:</b>  Reference: RFI T-0042  Per RFI T-0042, the concrete wall height increased to achieve a min 18" above the finished paver surface. Please clarify if the exposed concrete areas shown on A-5000 are to be min 18" above the pavers. If so, the 1st stone above the exposed concrete would have to be trimmed. Please clarify.
<b>T-0121</b>	<b>301 Mission Wall - Aluminum Panel Layout</b>	<b>Closed</b>	<b>CR</b>	<b>04/27/2011</b>	<b>05/07/2011</b>	<b>05/10/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: A-5000  Regarding the aluminum panels on the 301 Mission wall, bottom panel at each end of the wall will need to be trimmed. The standard panel is 2'-11 1/2" tall, but the bottom panel measures out to be 2'-1"+/- on the west end and 2'-9"+/- on the east. Please confirm that this is acceptable. If not, please advise.						<b>ANSWER:</b>  Reference: A-5000  Regarding the aluminum panels on the 301 Mission wall, bottom panel at each end of the wall will need to be trimmed. The standard panel is 2'-11 1/2" tall, but the bottom panel measures out to be 2'-1"+/- on the west end and 2'-9"+/- on the east. Please confirm that this is acceptable. If not, please advise.
<b>T-0122</b>	<b>BSE - Hazardous Material Removed From Zone 3 (Potential Contaminated Material Closed</b>		<b>01</b>	<b>04/29/2011</b>	<b>05/09/2011</b>	<b>05/02/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 00 03 35, 1.2  During Investigation of Zone 3, BBII discovered potential lead based material existing on site. The specific area of concern is the pedestals on Fremont Street.  Please confirm that all contaminated material (specifically the referenced pedestals) as specified in the specification section 00 03 35, Article 1.2 has been removed and abated by the Demolition Contractor.  BBII is scheduled to remove these pedestals next week and cannot proceed with this critical work until it is confirmed that the site is cleared of lead based materials as required by the Specifications.						<b>ANSWER:</b>  Reference Specification Section 00 03 35, 1.2  During Investigation of Zone 3, BBII discovered potential lead based material existing on site. The specific area of concern is the pedestals on Fremont Street.  Please confirm that all contaminated material (specifically the referenced pedestals) as specified in the specification section 00 03 35, Article 1.2 has been removed and abated by the Demolition Contractor.  BBII is scheduled to remove these pedestals next week and cannot proceed with this critical work until it





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	<p>The TJPA's attention is directed to the following Section of the Specifications:</p> <p>SECTION 00 03 35 <i>¿</i> EXISTING CONDITIONS: HAZARDOUS MATERIALS</p> <p>"1.2 HAZARDOUS MATERIALS REPORTS A. The TJPA's environmental consultants have surveyed the facility for the presence of various hazardous materials. Materials investigated may include asbestos, lead, PCB ballasts, mercury containing lamps, contaminated soils, underground storage tanks, and other hazardous materials. The demolition contractor for the Demolition project (Evans Brothers Inc.) is responsible for removing and abating products containing asbestos, lead, or PCB ballast, and mercury-containing lamps."</p>				<p>is confirmed that the site is cleared of lead based materials as required by the Specifications.</p> <p>The TJPA's attention is directed to the following Section of the Specifications:</p> <p>SECTION 00 03 35 <i>¿</i> EXISTING CONDITIONS: HAZARDOUS MATERIALS</p> <p>"1.2 HAZARDOUS MATERIALS REPORTS A. The TJPA's environmental consultants have surveyed the facility for the presence of various hazardous materials. Materials investigated may include asbestos, lead, PCB ballasts, mercury containing lamps, contaminated soils, underground storage tanks, and other hazardous materials. The demolition contractor for the Demolition project (Evans Brothers Inc.) is responsible for removing and abating products containing asbestos, lead, or PCB ballast, and mercury-containing lamps."</p>	
<b>T-0123</b>	<b>301 Mission Wall - SASM and Insulation Tape Materials</b>	<b>Closed</b>	<b>CR</b>	<b>04/29/2011</b>	<b>05/09/2011</b>	<b>05/05/2011</b>
<p><b>From:</b> Webcor Construction LP      David Hungerford</p> <p><b>REQUEST:</b></p> <p>Reference: S-0002, A-6000</p> <p>Clarification is requested regarding the notes and details on Sheet S-0002, and A-6000 (see attached marked up sheets). Note 1 within the "WALL FINISH" section of the notes on page S-0002 says to use insulation separation tape between treated wood surfaces and steel framing. In note 2 on page S-0002, SASM is specified as a different material, but on the details of page A-6000 SASM is shown to be used in the same areas as is described for the insulation tape. It is the interpretation of Transworld that the insulation tape is to be used at all locations referenced on sheet A-6000 as "SASM". Please clarify if these two different materials are to be applied in the same areas.</p>		<p><b>ANSWER:</b></p> <p>Reference: S-0002, A-6000</p> <p>Clarification is requested regarding the notes and details on Sheet S-0002, and A-6000 (see attached marked up sheets). Note 1 within the "WALL FINISH" section of the notes on page S-0002 says to use insulation separation tape between treated wood surfaces and steel framing. In note 2 on page S-0002, SASM is specified as a different material, but on the details of page A-6000 SASM is shown to be used in the same areas as is described for the insulation tape. It is the interpretation of Transworld that the insulation tape is to be used at all locations referenced on sheet A-6000 as "SASM". Please clarify if these two different materials are to be applied in the same areas.</p>				





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<b>T-0123.1</b>	<b>301 Mission Wall - SASM and Insulation Tape Materials</b>	<b>Closed</b>	<b>CR</b>	<b>05/06/2011</b>	<b>05/16/2011</b>	<b>05/09/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: RFI T-0123, A-6000, S-0002  The response provided to RFI T-0123 is in conflict with the contract documents. The response requires the contractor to create a waterproofing barrier for the entire length of the wall; however, the contract documents do not indicate a complete waterproofing barrier. References to SASM on page A-6000 instructs the application of SASM at all points where pressure-treated or moisture resistant wood comes in contact with metal. This application instruction, therefore, would not result in a waterproof barrier along the entire length of the wall. Please clarify if on the details "SASM" was intended to read "insulation tape", because the application locations of the SASM, as per A-6000, are called out and described to be at all locations of the insulation tape defined on S-0002.  In the alternative, is it the intention of the design team to apply additional waterproofing not shown on the contract documents?						
						<b>ANSWER:</b>  Reference: RFI T-0123, A-6000, S-0002  The response provided to RFI T-0123 is in conflict with the contract documents. The response requires the contractor to create a waterproofing barrier for the entire length of the wall; however, the contract documents do not indicate a complete waterproofing barrier. References to SASM on page A-6000 instructs the application of SASM at all points where pressure-treated or moisture resistant wood comes in contact with metal. This application instruction, therefore, would not result in a waterproof barrier along the entire length of the wall. Please clarify if on the details "SASM" was intended to read "insulation tape", because the application locations of the SASM, as per A-6000, are called out and described to be at all locations of the insulation tape defined on S-0002.  In the alternative, is it the intention of the design team to apply additional waterproofing not shown on the contract documents?
<b>T-0124</b>	<b>301 Mission Wall - Dimension Between Screen Wall and Existing Garage Wall</b>	<b>Closed</b>	<b>CR</b>	<b>05/02/2011</b>	<b>05/12/2011</b>	<b>05/31/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: C-2000  The dimension between the new location of the 301 Wall and the existing garage wall is approx 8". Please advise as to how this gap is to be closed off.						
						<b>ANSWER:</b>  Reference: C-2000  The dimension between the new location of the 301 Wall and the existing garage wall is approx 8". Please advise as to how this gap is to be closed off.
<b>T-0124.1</b>	<b>301 Mission Wall Enclosure Panel Method of Connection</b>	<b>Closed</b>	<b>CR</b>	<b>09/01/2011</b>	<b>09/16/2011</b>	<b>09/13/2011</b>
<b>From:</b> Webcor Construction LP      Michael Constable						
<b>REQUEST:</b>  Reference: RFI T- 0124, URS response to RFI T- 0124  Per recent Change Order negotiations for the required 301 Mission Wall end panel per RFI # T-0124, the panel detail is now being revised to a two-piece, glued enclosure						
						<b>ANSWER:</b>  Reference: RFI T- 0124, URS response to RFI T- 0124  Per recent Change Order negotiations for the required 301 Mission Wall end panel per RFI # T-0124, the



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<b>T-0126</b>	<b>BSE - Confirmation of Utility Abandonment on Fremont St, East side of Phase 1 El</b>	<b>Closed</b>	<b>01</b>	<b>05/02/2011</b>	<b>05/12/2011</b>	<b>05/12/2011</b>
<div> <div> <b>From:</b> Webcor Construction LP Nhi Tran </div> <div> <b>REQUEST:</b> <p>In order to drive sheet piles for the hammer head wall location along Fremont St and the North West Corner of Zone 4, BBII requests confirmation of the abandonment of all utilities east of the PG&amp;E electrical duct bank. BBII also will need the As-Build drawing of the PG&amp;E duct bank location.</p> <p>BBI needs this information to proceed on the extra unforeseen concrete wall in the hammer head area of the buttress wall.</p> </div> <div> <b>ANSWER:</b> <p>In order to drive sheet piles for the hammer head wall location along Fremont St and the North West Corner of Zone 4, BBII requests confirmation of the abandonment of all utilities east of the PG&amp;E electrical duct bank. BBII also will need the As-Build drawing of the PG&amp;E duct bank location.</p> <p>BBI needs this information to proceed on the extra unforeseen concrete wall in the hammer head area of the buttress wall.</p> </div> </div>						
<b>T-0127</b>	<b>BSE - Openings Below Screen Wall at 301 Mission Building</b>	<b>Closed</b>	<b>01</b>	<b>05/04/2011</b>	<b>05/14/2011</b>	<b>05/16/2011</b>
<div> <div> <b>From:</b> Webcor Construction LP Nhi Tran </div> <div> <b>REQUEST:</b> <p>Reference Sheets GT-2201, GT-5102 Sec. 10, and attached photos</p> <p>In the northwest corner of Zone 4, BBII has exposed 2 openings below the screen wall in the 301 Mission structure. The first opening is located approximately 6 feet east of gridline 27 and the second opening is located approximately 8 feet east of gridline 29. These openings are approximately 18" x 36" in size. (See attached pictures).</p> <p>These openings are not shown on construction documents. Please advise how to proceed. BBII requests an expedited response prior to the end of this week, as this matter is pertinent to backfill operation.</p> </div> <div> <b>ANSWER:</b> <p>Reference Sheets GT-2201, GT-5102 Sec. 10, and attached photos</p> <p>In the northwest corner of Zone 4, BBII has exposed 2 openings below the screen wall in the 301 Mission structure. The first opening is located approximately 6 feet east of gridline 27 and the second opening is located approximately 8 feet east of gridline 29. These openings are approximately 18" x 36" in size. (See attached pictures).</p> <p>These openings are not shown on construction documents. Please advise how to proceed. BBII requests an expedited response prior to the end of this week, as this matter is pertinent to backfill operation.</p> </div> </div>						
<b>T-0128</b>	<b>BSE - Old Existing Concrete Floor Along 301 Mission in Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>05/05/2011</b>	<b>05/15/2011</b>	<b>05/12/2011</b>
<div> <div> <b>From:</b> Webcor Construction LP Nhi Tran </div> <div> <b>REQUEST:</b> <p>Reference Specification Section 02 41 01</p> <p>During pre-trenching, BBII found an existing concrete floor along the 301 Mission St garage wall. It is located between the 301 Mission building wall and the buttress area between Grid Line 29 and 30. BBII has exposed a 20ft-30ft</p> </div> <div> <b>ANSWER:</b> <p>Reference Specification Section 02 41 01</p> <p>During pre-trenching, BBII found an existing concrete floor along the 301 Mission St garage wall. It is located between the 301 Mission building wall and the buttress area between Grid Line 29 and 30. BBII has exposed</p> </div> </div>						



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	<p>section of this floor (approximately on Grid Line A between Grid Lines 29 and 30), and have demolished the slab within the pre-trench area that has been exposed. It appears to BBI that this unforeseen obstruction continues further into the buttress area. If this unforeseen obstruction continues further into the buttress area, it would have to be removed so the buttress construction can continue.</p> <p>Please advise on how to proceed.</p>					
	<p>a 20ft-30ft section of this floor (approximately on Grid Line A between Grid Lines 29 and 30), and have demolished the slab within the pre-trench area that has been exposed. It appears to BBI that this unforeseen obstruction continues further into the buttress area. If this unforeseen obstruction continues further into the buttress area, it would have to be removed so the buttress construction can continue.</p> <p>Please advise on how to proceed.</p>					
<b>T-0129</b>	<b>BSE - Unforeseen Timber Pile in Pre-Trench Along 301 Mission in Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>05/05/2011</b>	<b>05/15/2011</b>	<b>05/06/2011</b>
	<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference Specification Section 02 41 01 and attached photo</p> <p>During pre-trenching, BBI discovered existing timber piles along the 301 Mission St garage wall between Grid Lines 29 and 30. These piles are less than 1foot away from the 301 Mission St garage wall and within the CDSM shoring wall limits. These unforeseen piles need to be removed as soon as possible. Please advise on how to proceed.</p> <p>W/O requests that the Engineer Of Record (Arup) review this on site with BBI prior to responding.</p>					
	<p><b>ANSWER:</b></p> <p>Reference Specification Section 02 41 01 and attached photo</p> <p>During pre-trenching, BBI discovered existing timber piles along the 301 Mission St garage wall between Grid Lines 29 and 30. These piles are less than 1foot away from the 301 Mission St garage wall and within the CDSM shoring wall limits. These unforeseen piles need to be removed as soon as possible. Please advise on how to proceed.</p> <p>W/O requests that the Engineer Of Record (Arup) review this on site with BBI prior to responding.</p>					



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0130</b>	<b>301 Mission Wall - FCR 043 Concrete Wall Crack</b>	<b>Closed</b>	<b>CR</b>	<b>05/06/2011</b>	<b>05/16/2011</b>	<b>05/09/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Field Condition Report No. 043			Reference: Field Condition Report No. 043			
See attached FCR No. 043. The east end of the 301 Mission concrete wall has cracks and also spalled in one corner. This had been discussed on 05/02/11, in Transworld's subcontractor meeting with Turner, URS, TJPA, Webcor-Obayashi, and Transworld. Please advise as to how Transworld is to repair the spalled corner and cracks.			See attached FCR No. 043. The east end of the 301 Mission concrete wall has cracks and also spalled in one corner. This had been discussed on 05/02/11, in Transworld's subcontractor meeting with Turner, URS, TJPA, Webcor-Obayashi, and Transworld. Please advise as to how Transworld is to repair the spalled corner and cracks.			
<b>T-0130.1</b>	<b>301 Mission Wall - FCR 043 Concrete Wall Patch Material</b>	<b>Closed</b>	<b>CR</b>	<b>06/09/2011</b>	<b>06/19/2011</b>	<b>06/13/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: FCR #043, RFI T-0130, and attached product data			Reference: FCR #043, RFI T-0130, and attached product data			
Response to RFI T-0130 directs Transworld to repair the damaged concrete at the 301 Mission Wall, as described in Field Condition Report 043. Attached are product data sheets which satisfy the requirements noted in response to RFI T-0130. Please review and confirm that the attached materials are acceptable to patch the damaged concrete.			Response to RFI T-0130 directs Transworld to repair the damaged concrete at the 301 Mission Wall, as described in Field Condition Report 043. Attached are product data sheets which satisfy the requirements noted in response to RFI T-0130. Please review and confirm that the attached materials are acceptable to patch the damaged concrete.			
<b>T-0131</b>	<b>301 Mission Wall - Framing Modifications and Base Plate Conflict</b>	<b>Closed</b>	<b>CR</b>	<b>05/06/2011</b>	<b>05/16/2011</b>	<b>05/20/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: C/S-5000, B/A-6000, attached sketches, and referenced RFI's			Reference: C/S-5000, B/A-6000, attached sketches, and referenced RFI's			
Field verified measurements and layout for the location of the structural steel does not coordinate with the stucco inset locations as shown on detail C/S-5000. In addition framing around the perimeter of the wall (aluminum panel locations) had to be modified due to assembly and installation methods. (See attached pictures and sketches. This RFI addresses three framing issues. All issues have been discussed in the weekly 301 Mission Wall subcontractor meeting with URS, Turner, Transworld,			Field verified measurements and layout for the location of the structural steel does not coordinate with the stucco inset locations as shown on detail C/S-5000. In addition framing around the perimeter of the wall (aluminum panel locations) had to be modified due to assembly and installation methods. (See attached pictures and sketches. This RFI addresses three framing issues. All issues have been discussed in the weekly 301 Mission Wall subcontractor meeting			



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	<p>TJPA and Webcor-Obayashi.</p> <p>1.) In two of the four stucco slot locations, field conditions show that a portion of the base plate conflicts with the stucco slot. This base plate encroaches into the stucco panel per dimensions shown on the attached sketch. Please advise.</p> <p>2.) The structural steel had been relocated to CL of the wall (per RFI T-0098) and therefore studs around the steel per B/A-6000 could not be set per plan. Transworld has installed hat channel metal framing to the face of the structural steel tube using fasteners into the structural steel as per RFI T-0106 as well as modified the boxed framing per attached sketches around the perimeter of the wall. Sizes of metal framing were used to align with adjacent framing per plan. This work is currently installed, please confirm framing modifications per attached marked up details are acceptable.</p> <p>3.) Blocking a the top of the wall at the north side (between the framing and 8"x 8" tube steel) was not installed, as there was no room between the framing and steel. Framing was attached directly to the tube steel. See attached.</p> <p>Please confirm that the framing modifications in item 2 and 3 are acceptable and provide direction at the base plate conflict per item 1.</p>					
	<p>with URS, Turner, Transworld, TJPA and Webcor-Obayashi.</p> <p>1.) In two of the four stucco slot locations, field conditions show that a portion of the base plate conflicts with the stucco slot. This base plate encroaches into the stucco panel per dimensions shown on the attached sketch. Please advise.</p> <p>2.) The structural steel had been relocated to CL of the wall (per RFI T-0098) and therefore studs around the steel per B/A-6000 could not be set per plan. Transworld has installed hat channel metal framing to the face of the structural steel tube using fasteners into the structural steel as per RFI T-0106 as well as modified the boxed framing per attached sketches around the perimeter of the wall. Sizes of metal framing were used to align with adjacent framing per plan. This work is currently installed, please confirm framing modifications per attached marked up details are acceptable.</p> <p>3.) Blocking a the top of the wall at the north side (between the framing and 8"x 8" tube steel) was not installed, as there was no room between the framing and steel. Framing was attached directly to the tube steel. See attached.</p> <p>Please confirm that the framing modifications in item 2 and 3 are acceptable and provide direction at the base plate conflict per item 1.</p>					
<b>T-0132</b>	<b>BSE - Lead Based Paint On Bent Pedestals</b>	<b>Closed</b>	<b>01</b>	<b>05/06/2011</b>	<b>05/16/2011</b>	<b>05/09/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Masashi Kojima						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Please see information attached regarding the paint on the old bent Pedestals existing along Fremont Street. The information provided indicates the level of lead is above the permissible level. This area is now considered part of the lead abatement program; this work will be commencing on Saturday 5/7/2011. Cost of this Lead abatement will be charged to the owner.</p>			<p>Please see information attached regarding the paint on the old bent Pedestals existing along Fremont Street. The information provided indicates the level of lead is above the permissible level. This area is now considered part of the lead abatement program; this work will be commencing on Saturday 5/7/2011. Cost of this Lead abatement will be charged to the owner.</p>			



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T-0133	BSE - CDSM Test Section & Start of Work	Closed	01	05/09/2011	05/19/2011	05/10/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:						ANSWER:
Reference Specification Section 31 56 13, 1.6. F. 1-2						Reference Specification Section 31 56 13, 1.6. F. 1-2
Please confirm that the acceptance of Zone 4 Test Section strength and permeability results is the prerequisite to begin Zone 4 & 3 shoring work, and acceptance of the Zone 1/2 Test Section results is the prerequisite to begin work Zones 1 & 2.						Please confirm that the acceptance of Zone 4 Test Section strength and permeability results is the prerequisite to begin Zone 4 & 3 shoring work, and acceptance of the Zone 1/2 Test Section results is the prerequisite to begin work Zones 1 & 2.
T-0134	BSE - 301 Mission Guide Wall	Closed	01	05/09/2011	05/19/2011	05/12/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:						ANSWER:
Reference Sheet GT-2103, Specification Section 31 56 13, and attached sketch						Reference Sheet GT-2103, Specification Section 31 56 13, and attached sketch
Typically in CDSM shoring, a guide frame constructed from steel beams is used, which straddles the CDSM wall. The guide frame is used to align the augers, align and place beams, and expand/collapse the drill rods. The existing 301 Mission building wall is approximately 5-6" away from the outside of the CDSM shoring wall. As such it will not permit placement of a standard steel beam guide frame. Is it acceptable to construct a temporary concrete/rebar guide wall on the outside of the CDSM wall and adjacent to the existing 301 Mission footing wall? See attached sketch details of the proposed guide wall.						Typically in CDSM shoring, a guide frame constructed from steel beams is used, which straddles the CDSM wall. The guide frame is used to align the augers, align and place beams, and expand/collapse the drill rods. The existing 301 Mission building wall is approximately 5-6" away from the outside of the CDSM shoring wall. As such it will not permit placement of a standard steel beam guide frame. Is it acceptable to construct a temporary concrete/rebar guide wall on the outside of the CDSM wall and adjacent to the existing 301 Mission footing wall? See attached sketch details of the proposed guide wall.
T-0135	BSE - Unforeseen Timber Piles in Pre-Trench Along 301 Mission St. in Zone 4	Closed	01	05/10/2011	05/20/2011	05/12/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:						ANSWER:
Reference RFI#T-0129 and Specification Section 02 41 01						Reference RFI#T-0129 and Specification Section 02 41 01
The response to BBII RFI 094 [RFI #T-0129] regarding the unforeseen timber piles along 301 Mission Street, "Concrete to be placed in the remnant pile hole as rapidly as possible after pile removal of the adjacent pile."						The response to BBII RFI 094 [RFI #T-0129] regarding the unforeseen timber piles along 301 Mission Street, "Concrete to be placed in the remnant pile hole as rapidly as possible after pile removal of the adjacent











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T-0138	BSE - Unforeseen Timber Pile in Pre Trench Along 301 Mission St. in Zone 4 - Con	Closed	01	05/10/2011	05/20/2011	05/12/2011
<div><div>From: Webcor Construction LP</div><div>Nhi Tran</div></div>						
REQUEST:			ANSWER:			
Reference Response to RFI #T-0129 [BBI RFI 094] and Specification Section 02 41 01			Reference Response to RFI #T-0129 [BBI RFI 094] and Specification Section 02 41 01			
Using the current, approved means & methods set forth in RFI Response #T-0129, there is an extremely high probability that the vibratory hammer or casing will come into contact with the existing 301 Mission wall. Despite multiple tag lines and attempts to swing away from the wall, BBII cannot guarantee the equipment will not contact the wall.			Using the current, approved means & methods set forth in RFI Response #T-0129, there is an extremely high probability that the vibratory hammer or casing will come into contact with the existing 301 Mission wall. Despite multiple tag lines and attempts to swing away from the wall, BBII cannot guarantee the equipment will not contact the wall.			
BBII requests a revised methodology to extract the unforeseen timber piles or to protect the existing wall which will reduce the of damaging the wall at 301 Mission. BBII is willing to meet with the Engineer to discuss and develop this method.			BBII requests a revised methodology to extract the unforeseen timber piles or to protect the existing wall which will reduce the of damaging the wall at 301 Mission. BBII is willing to meet with the Engineer to discuss and develop this method.			



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T-0138.1	BSE - Unforeseen Timber Piles in Pre Trench Along 301 Mission St. in Zone 4 - Closed	Closed	01	05/20/2011	05/30/2011	05/23/2011
From: Webcor Construction LP Nhi Tran						
REQUEST:						
Reference response to RFI#T-0129, RFI#T-0138, Specification Section 02 41 01 and attached documents						
The response to BBII RFI 094 [RFI#T-0129] regarding the unforeseen timber piles along 301 Mission Street, "Concrete to be placed in the remnant pile hole as rapidly as possible after pile removal of the adjacent pile." Concrete is not compatible with CDSM mixing.						
After clarification on the issue in RFI Response #T-0138, BBII proposed and furnished Central Concrete Sand Slurry Mix FOA100CX under the direction of the Engineer. The Engineer of Record's field engineer reviewed, approved and observed the installation of this mix in the pile voids along 301 Mission Street. The mix was recommended by ARUP Field Engineer prior to placement in the field, please confirm that this mix design meets the field engineer's requirements.						
Attachments: Mix as requested is being submitted for record.						
ANSWER:						
Reference response to RFI#T-0129, RFI#T-0138, Specification Section 02 41 01 and attached documents						
The response to BBII RFI 094 [RFI#T-0129] regarding the unforeseen timber piles along 301 Mission Street, "Concrete to be placed in the remnant pile hole as rapidly as possible after pile removal of the adjacent pile." Concrete is not compatible with CDSM mixing.						
After clarification on the issue in RFI Response #T-0138, BBII proposed and furnished Central Concrete Sand Slurry Mix FOA100CX under the direction of the Engineer. The Engineer of Record's field engineer reviewed, approved and observed the installation of this mix in the pile voids along 301 Mission Street. The mix was recommended by ARUP Field Engineer prior to placement in the field, please confirm that this mix design meets the field engineer's requirements.						
Attachments: Mix as requested is being submitted for record.						
T-0139	BSE - Unforeseen Timber Pile in Pre Trench Along 301 Mission St. in Zone 4 - CR 1 Closed	Closed	01	05/10/2011	05/20/2011	05/11/2011
From: Webcor Construction LP Nhi Tran						
REQUEST:						
Reference Response to RFI #T-0129 [BBI RFI 094] and Specification Section 02 41 01						
Please clarify if the removal of the unforeseen timber piles along 301 Mission Street will be reimbursed by CR T-010.						
ANSWER:						
Reference Response to RFI #T-0129 [BBI RFI 094] and Specification Section 02 41 01						
Please clarify if the removal of the unforeseen timber piles along 301 Mission Street will be reimbursed by CR T-010.						
T-0140	BSE - Bridges Submittals	Closed	01	05/12/2011	05/22/2011	05/27/2011
From: Webcor Construction LP Nhi Tran						
REQUEST:						
Reference Specification Section 01 53 13						
BBII proposes breaking up the bridge submittals to allow						
ANSWER:						
Reference Specification Section 01 53 13						
BBII proposes breaking up the bridge submittals to						



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	<p>submittal fundamental structural drawings and calculations for the bridge, independent of accessories and specialized components necessary for a complete bridge package.</p> <p>Specifically, the first set of submittals would include Structural drawings and calculations for the bridge structure from the pavement and decking down - piers, cap beams, girders, abutments, and associated connections. Additionally, it will include standard edge railing/barriers.</p> <p>Follow on coordination submittals will include traffic coordination components, gates, hardware, locking mechanisms, fences, Muni OCS components, utility support details, surface grading and drainage.</p> <p>BBII believes that it will take some time to finalize a complete bridge package that satisfies all interested parties. Isolating the core bridge structure into it's own submittals will ensure that detailing and fabrication of the main components of the bridge will not be held up while working out the details.</p> <p>Please confirm this is acceptable</p>					
T-0141	<p><b>BSE - Inclinometers IW-5 to IW-8 Install Locations</b></p> <p>From: Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference Sheets GT-1301, GT-1302, GT-2201 &amp; 13/GT-5101 and Specification Section 31 56 13</p> <p>Please clarify if locations IW-5 to IW-8 exist. They are not shown on GT-1301 and GT-1302.</p>	Closed	01	05/12/2011	05/22/2011	05/16/2011
	<p><b>ANSWER:</b></p> <p>Reference Sheets GT-1301, GT-1302, GT-2201 &amp; 13/GT-5101 and Specification Section 31 56 13</p> <p>Please clarify if locations IW-5 to IW-8 exist. They are not shown on GT-1301 and GT-1302.</p>					
T-0142	<p><b>BSE - Instruments I-104 to I-107</b></p> <p>From: Webcor Construction LP                      Nhi Tran</p>	Closed	01	05/13/2011	05/23/2011	05/16/2011



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<b>REQUEST:</b>  Reference Sheets GT-1301, GT-1302, GT-2201, & 13/GT-5101 and Specification Section 31 56 13  On Sheet GT-2201, please confirm that Instrument I-104 to I-107 is detail 13/GT-5101.			<b>ANSWER:</b>  Reference Sheets GT-1301, GT-1302, GT-2201, & 13/GT-5101 and Specification Section 31 56 13  On Sheet GT-2201, please confirm that Instrument I-104 to I-107 is detail 13/GT-5101.			
<b>T-0143</b>	<b>BSE - Confirmation of Utility Decommissioning and As-Built for Fremont Street</b>	<b>Closed</b>	<b>01</b>	<b>05/16/2011</b>	<b>05/26/2011</b>	<b>05/20/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  Reference Sheet D-2230 and attached sketch  During BBII potholing work on the Fremont street hammer head, BBII exposed the existing live PG&E concrete duct bank. The duct bank is located under BBII Buttress drill pad (see attached sketch), the drill pad is scheduled to be poured 5-26-2011/5-27-2011. BBII has concerns that the duct bank will not be able to support the load for the drilling equipment. The concrete duct bank will need to be removed prior to drill pad installation. Please advise.			<b>ANSWER:</b>  Reference Sheet D-2230 and attached sketch  During BBII potholing work on the Fremont street hammer head, BBII exposed the existing live PG&E concrete duct bank. The duct bank is located under BBII Buttress drill pad (see attached sketch), the drill pad is scheduled to be poured 5-26-2011/5-27-2011. BBII has concerns that the duct bank will not be able to support the load for the drilling equipment. The concrete duct bank will need to be removed prior to drill pad installation. Please advise.			
<b>T-0144</b>	<b>BSE - Unknown Concrete Structure along 199 Fremont St in Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>05/18/2011</b>	<b>05/28/2011</b>	<b>05/24/2011</b>
<b>From:</b> Webcor Construction LP                      Masashi Kojima						
<b>REQUEST:</b>  Reference Specification Section 31 56 13  BBII discovered the unforeseen concrete structure in the attached photo. Tills concrete mass is unknown and is in direct conflict with the BSE CDSM wall. The concrete mass is approx 2ft wide and extends 8ft depth the entire between GL J 30-33.5 adjacent 199 Fremont Street building. During the excavation at 8ft there was water egress into the excavation from underneath the concrete structure see photos attached. BBII requests immediate direction from the TJPA on this issue.			<b>ANSWER:</b>  Reference Specification Section 31 56 13  BBII discovered the unforeseen concrete structure in the attached photo. Tills concrete mass is unknown and is in direct conflict with the BSE CDSM wall. The concrete mass is approx 2ft wide and extends 8ft depth the entire between GL J 30-33.5 adjacent 199 Fremont Street building. During the excavation at 8ft there was water egress into the excavation from underneath the concrete structure see photos attached. BBII requests immediate direction from the TJPA on this issue.			



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<b>T-0145</b>	<b>BSE - Existing Concrete Footing Gridline J between Gridline 26.5-30 along 181 Fre Closed</b>		<b>01</b>	<b>05/18/2011</b>	<b>05/28/2011</b>	<b>05/20/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification Section 02 41 00			Reference Specification Section 02 41 00			
BBII followed the method approved to remove a section of the unforeseen structure in RFI #74 & 74.1, and found a separate concrete footing bellow that. It is believed to be a footing that extends below the 177/181 Fremont St. building. The top of this footing is approximately 8 feet below the original grade, and it is approximately 3 feet wide, and 3 feet deep. BBII is concerned with the removal of this footing and the extensive rubble that was exposed below it. When a bucket of dirt was removed along the footing, a large amount of water gushed out, from below the 177/181 Fremont St. building, and through the large amount of stone rubble that was exposed. At this point the bottom of the footing was found, and the soil was quickly replaced. This footing is within the CDSM wall extents, and will have to be removed. Due to the fragile nature, and the age of the 177/181 Fremont St. building; please clearly describe and advise. Please See Attached Pictures.			BBII followed the method approved to remove a section of the unforeseen structure in RFI #74 & 74.1, and found a separate concrete footing bellow that. It is believed to be a footing that extends below the 177/181 Fremont St. building. The top of this footing is approximately 8 feet below the original grade, and it is approximately 3 feet wide, and 3 feet deep. BBII is concerned with the removal of this footing and the extensive rubble that was exposed below it. When a bucket of dirt was removed along the footing, a large amount of water gushed out, from below the 177/181 Fremont St. building, and through the large amount of stone rubble that was exposed. At this point the bottom of the footing was found, and the soil was quickly replaced. This footing is within the CDSM wall extents, and will have to be removed. Due to the fragile nature, and the age of the 177/181 Fremont St. building; please clearly describe and advise. Please See Attached Pictures.			
<b>T-0146</b>	<b>BSE - Additional Timber Piles Adjacent 177/181 Fremont Building South Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>05/19/2011</b>	<b>05/29/2011</b>	<b>05/20/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI#T-0103 and attached photo			Reference RFI#T-0103 and attached photo			
During BBII demolition of the unknown concrete structure along South side of Zone 4 adjacent 177/181 Fremont building (Refer to [RFI#T-0103] BBII RFI# 74), BBII discovered timber piles beneath the unknown concrete structure - see photos attached.			During BBII demolition of the unknown concrete structure along South side of Zone 4 adjacent 177/181 Fremont building (Refer to [RFI#T-0103] BBII RFI# 74), BBII discovered timber piles beneath the unknown concrete structure - see photos attached.			
The location timber piles are in conflict with the alignment of the CDSM wall. Please advise on the method of removal of the obstruction.			The location timber piles are in conflict with the alignment of the CDSM wall. Please advise on the method of removal of the obstruction.			
Note: BBII has concerns regarding the stability of the adjacent 177/181 Fremont Building (old brick structure).			Note: BBII has concerns regarding the stability of the adjacent 177/181 Fremont Building (old brick structure).			



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<b>T-0146.1</b>	<b>BSE - Additional Timber Piles Adjacent 177/181 Fremont Building South Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>05/20/2011</b>	<b>05/30/2011</b>	<b>05/20/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI#T-0146			Reference RFI#T-0146			
Please provide the TJPA's specific written direction and procedure on how to remove the unforeseen piles along North face of 181 Fremont Street according to the response for RFI T-0146.			Please provide the TJPA's specific written direction and procedure on how to remove the unforeseen piles along North face of 181 Fremont Street according to the response for RFI T-0146.			
The contractor cannot proceed on this extra and critical work without the specific direction and procedure provided in writing by the TJPA.			The contractor cannot proceed on this extra and critical work without the specific direction and procedure provided in writing by the TJPA.			



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<b>T-0146.2</b>	<b>BSE - Additional Timber Piles Adjacent 177/181 Fremont Building South Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>05/23/2011</b>	<b>06/02/2011</b>	<b>05/24/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI#T-0146.1			Reference RFI#T-0146.1			
Based on the joint meeting between W/O, BBII and the TJPA on 5/23/2011, BBII would like to confirm the following:			Based on the joint meeting between W/O, BBII and the TJPA on 5/23/2011, BBII would like to confirm the following:			
181 Fremont Street Pile Extraction:			181 Fremont Street Pile Extraction:			
1. BBII will install additional survey control to establish the back of the shoring wall limit.			1. BBII will install additional survey control to establish the back of the shoring wall limit.			
2. BBII will contact DND Construction to confirm the allowable distance between an existing pile and the back of the shoring wall.			2. BBII will contact DND Construction to confirm the allowable distance between an existing pile and the back of the shoring wall.			
3. BBII will expose, in the presence of the engineer, 3 piles at one time.			3. BBII will expose, in the presence of the engineer, 3 piles at one time.			
4. BBII and the Engineer will jointly determine the piles that can be left in place with reasonable assurance that they will not impact the shoring wall.			4. BBII and the Engineer will jointly determine the piles that can be left in place with reasonable assurance that they will not impact the shoring wall.			
5. BBII will install flat sheet piles between the building and the wood piles to prevent caving of soils under the building.			5. BBII will install flat sheet piles between the building and the wood piles to prevent caving of soils under the building.			
6. BBII will extract the wood piles with vibratory hammer, with the same stroking procedure without steel casing. BBII will perform dewatering enough to be able to connect the hammer to the pile.			6. BBII will extract the wood piles with vibratory hammer, with the same stroking procedure without steel casing. BBII will perform dewatering enough to be able to connect the hammer to the pile.			
7. BBII will backfill the void with low strength material Central Concrete Mix FOA100CX (RFI #T-0138.1).			7. BBII will backfill the void with low strength material Central Concrete Mix FOA100CX (RFI #T-0138.1).			
8. BBII will backfill the piles.			8. BBII will backfill the piles.			
9. BBII will remove the sheet piles and start over with Step 3.			9. BBII will remove the sheet piles and start over with Step 3.			
10. All of this work will be tracked and compensated on force account under CR T-010.			10. All of this work will be tracked and compensated on force account under CR T-010.			
11. Similar to the extraction in front of the 301 Mission garage wall, BBII will take every precaution to avoid damaging the adjacent wall; however, due to the proximity of the hammer to the wall, BBII will not guarantee not damaging the wall. If damage to the adjacent wall occurs in any phase of the pile extraction operation described above, BBII will be compensated for repairs under CR T-010 as well.			11. Similar to the extraction in front of the 301 Mission garage wall, BBII will take every precaution to avoid damaging the adjacent wall; however, due to the proximity of the hammer to the wall, BBII will not guarantee not damaging the wall. If damage to the adjacent wall occurs in any phase of the pile extraction operation described above, BBII will be compensated for repairs under CR T-010 as well.			
Please confirm the above as soon as possible. In addition, BBII requests immediate confirmation of allowable work hours for the work described above.			Please confirm the above as soon as possible. In addition, BBII requests immediate confirmation of allowable work hours for the work described above.			



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	<p>above, BBII will be compensated for repairs under CR T-010 as well.</p> <p>Please confirm the above as soon as possible. In addition, BBII requests immediate confirmation of allowable work hours for the work described above.</p>					<p>operation described above, BBII will be compensated for repairs under CR T-010 as well.</p> <p>Please confirm the above as soon as possible. In addition, BBII requests immediate confirmation of allowable work hours for the work described above.</p>
<b>T-0146.4</b>	<b>BSE - Additional Timber Piles Adjacent 177/181 Fremont Building South Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>05/27/2011</b>	<b>06/06/2011</b>	<b>05/31/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  Per Turner's request on 5/27/2011 this RFI is being asked, to modify the 177/181 Fremont pile extraction procedure as desired by ARUP:  Based on the revised proposal for unforeseen pile extraction work along 181 Fremont St. from ARUP, BBII (W/O) can agree with revisions as the follows: - Item 6 should read, "BBII will extract the piles with vibratory hammer only as necessary. BBII will use as little vibration as possible to remove the piles from the ground. BBII will perform dewatering enough to be able to connect the hammer to the pile." - Item 8 should read, "BBII will back fill the pile voids using a tremie pipe of minimum length 20ft attached to the concrete bucket. The tremie shall be inserted as far into the pile hole as possible prior to pouring the concrete, and the concrete shall be placed using normal tremie techniques. BBII will make efforts to pour the material into the void as possible, but BBII is not responsible to eliminate void completely." Other items shall remain the same.  Please also clarify that the response from RFI#T-0146.3 stating "Since this work has previously been classified as an "unknown obstruction" paid on force account; if there is damage to the 199 Masonry wall that the cost of repair is considered part of the force account work. BBII is to exert efforts to avoid damage and use the method of pulling the piles that gives least amount of risk for damage to the masonry wall." is this instead, meant to address the property and work related to 177/181 Fremont? If not, please address the question regarding 177/181 address.						<b>ANSWER:</b>  Per Turner's request on 5/27/2011 this RFI is being asked, to modify the 177/181 Fremont pile extraction procedure as desired by ARUP:  Based on the revised proposal for unforeseen pile extraction work along 181 Fremont St. from ARUP, BBII (W/O) can agree with revisions as the follows: - Item 6 should read, "BBII will extract the piles with vibratory hammer only as necessary. BBII will use as little vibration as possible to remove the piles from the ground. BBII will perform dewatering enough to be able to connect the hammer to the pile." - Item 8 should read, "BBII will back fill the pile voids using a tremie pipe of minimum length 20ft attached to the concrete bucket. The tremie shall be inserted as far into the pile hole as possible prior to pouring the concrete, and the concrete shall be placed using normal tremie techniques. BBII will make efforts to pour the material into the void as possible, but BBII is not responsible to eliminate void completely." Other items shall remain the same.  Please also clarify that the response from RFI#T-0146.3 stating "Since this work has previously been classified as an "unknown obstruction" paid on force account; if there is damage to the 199 Masonry wall that the cost of repair is considered part of the force account work. BBII is to exert efforts to avoid damage and use the method of pulling the piles that gives least amount of risk for damage to the masonry wall." is this instead, meant to address the property and work related to 177/181 Fremont? If not, please address the question regarding 177/181 address.



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<b>T-0147</b>	<b>301 Mission Wall - Stone Application Detail</b>	<b>Closed</b>	<b>CR</b>	<b>05/19/2011</b>	<b>05/29/2011</b>	<b>05/27/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Attached Sketch			Reference: Attached Sketch			
Please review the attached sketch showing the thinset manufacturer's recommendations for the tile installation at this wall. In reference to the approved submittal detail (attached) an additional layer of cement board will be installed to fur out the substrate so that the materials can be applied to their recommended thickness. In addition, the manufacturer recommends to use Laticrete 254 Platinum thinset material. The stone tiles finished surface will align with the aluminum panel above. Please expedite the review of this RFI.			Please review the attached sketch showing the thinset manufacturer's recommendations for the tile installation at this wall. In reference to the approved submittal detail (attached) an additional layer of cement board will be installed to fur out the substrate so that the materials can be applied to their recommended thickness. In addition, the manufacturer recommends to use Laticrete 254 Platinum thinset material. The stone tiles finished surface will align with the aluminum panel above. Please expedite the review of this RFI.			



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<b>T-0148</b>	<b>BSE - Additional Timber Piles Adjacent 199 Fremont Building Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>05/23/2011</b>	<b>06/02/2011</b>	<b>05/24/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI#T-0146.2		Reference RFI#T-0146.2				
Based on the joint meeting between W/O, BBII and the TJPA on 5/23/2011, BBII would like to confirm the following:		Based on the joint meeting between W/O, BBII and the TJPA on 5/23/2011, BBII would like to confirm the following:				
199 Fremont Street Pile Extraction: 1. BBII will install additional survey control to establish the back of the shoring wall limit. 2. BBII will contact DND Construction to confirm the allowable distance between an existing pile and the back of the shoring wall. 3. BBII will excavate, in the presence of the engineer, 8 piles at one time. 4. BBII and the Engineer will jointly determine the piles that can be left in place with reasonable assurance that they will not impact the shoring wall. 5. BBII will extract the piles with vibratory hammer, with the same stroking procedure without steel casing. BBII will perform dewatering enough to be able to connect the hammer to the pile. 6. BBII will backfill the void with low strength material Central Concrete Mix FOA100CX (RFI #T-0138.1). 7. BBII will backfill the piles and start over with Step 3. 8. All of this work will be tracked and compensated on force account under CR T-010. 9. Similar to the extraction in front of the 301 Mission garage wall, BBII will take every precaution to avoid damaging the adjacent wall; however, due to the proximity of the hammer to the wall, BBII will not guarantee not damaging the wall. If damage to the adjacent wall occurs in any phase of the pile extraction operation described above, BBII will be compensated for repairs under CR T-010 as well.		199 Fremont Street Pile Extraction: 1. BBII will install additional survey control to establish the back of the shoring wall limit. 2. BBII will contact DND Construction to confirm the allowable distance between an existing pile and the back of the shoring wall. 3. BBII will excavate, in the presence of the engineer, 8 piles at one time. 4. BBII and the Engineer will jointly determine the piles that can be left in place with reasonable assurance that they will not impact the shoring wall. 5. BBII will extract the piles with vibratory hammer, with the same stroking procedure without steel casing. BBII will perform dewatering enough to be able to connect the hammer to the pile. 6. BBII will backfill the void with low strength material Central Concrete Mix FOA100CX (RFI #T-0138.1). 7. BBII will backfill the piles and start over with Step 3. 8. All of this work will be tracked and compensated on force account under CR T-010. 9. Similar to the extraction in front of the 301 Mission garage wall, BBII will take every precaution to avoid damaging the adjacent wall; however, due to the proximity of the hammer to the wall, BBII will not guarantee not damaging the wall. If damage to the adjacent wall occurs in any phase of the pile extraction operation described above, BBII will be compensated for repairs under CR T-010 as well.				
Please confirm the above as soon as possible. In addition, BBII requests immediate confirmation of allowable work hours for the work described above.		Please confirm the above as soon as possible. In addition, BBII requests immediate confirmation of allowable work hours for the work described above.				



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<b>T-0148.1</b>	<b>BSE - Additional Timber Piles Adjacent 199 Fremont Building Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>05/23/2011</b>	<b>06/02/2011</b>	<b>06/07/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI#T-0148			Reference RFI#T-0148			
The response RFI T-0148 did not answer for Item 8 and 9. Please respond for Item 8 and Item 9. ----- ----- RFI#T-0148 Question: Reference RFI#T-0146.2			The response RFI T-0148 did not answer for Item 8 and 9. Please respond for Item 8 and Item 9. ----- ----- RFI#T-0148 Question: Reference RFI#T-0146.2			
Based on the joint meeting between W/O, BBII and the TJPA on 5/23/2011, BBII would like to confirm the following:			Based on the joint meeting between W/O, BBII and the TJPA on 5/23/2011, BBII would like to confirm the following:			
199 Fremont Street Pile Extraction: 1. BBII will install additional survey control to establish the back of the shoring wall limit. 2. BBII will contact DND Construction to confirm the allowable distance between an existing pile and the back of the shoring wall. 3. BBII will excavate, in the presence of the engineer, 8 piles at one time. 4. BBII and the Engineer will jointly determine the piles that can be left in place with reasonable assurance that they will not impact the shoring wall. 5. BBII will extract the piles with vibratory hammer, with the same stroking procedure without steel casing. BBII will perform dewatering enough to be able to connect the hammer to the pile. 6. BBII will backfill the void with low strength material Central Concrete Mix FOA100CX (RFI #T-0138.1). 7. BBII will backfill the piles and start over with Step 3. 8. All of this work will be tracked and compensated on force account under CR T-010. 9. Similar to the extraction in front of the 301 Mission garage wall, BBII will take every precaution to avoid damaging the adjacent wall; however, due to the proximity of the hammer to the wall, BBII will not guarantee not damaging the wall. If damage to the adjacent wall occurs in any phase of the pile extraction operation described above, BBII will be compensated for repairs under CR T- 010 as well.			199 Fremont Street Pile Extraction: 1. BBII will install additional survey control to establish the back of the shoring wall limit. 2. BBII will contact DND Construction to confirm the allowable distance between an existing pile and the back of the shoring wall. 3. BBII will excavate, in the presence of the engineer, 8 piles at one time. 4. BBII and the Engineer will jointly determine the piles that can be left in place with reasonable assurance that they will not impact the shoring wall. 5. BBII will extract the piles with vibratory hammer, with the same stroking procedure without steel casing. BBII will perform dewatering enough to be able to connect the hammer to the pile. 6. BBII will backfill the void with low strength material Central Concrete Mix FOA100CX (RFI #T-0138.1). 7. BBII will backfill the piles and start over with Step 3. 8. All of this work will be tracked and compensated on force account under CR T-010. 9. Similar to the extraction in front of the 301 Mission garage wall, BBII will take every precaution to avoid damaging the adjacent wall; however, due to the proximity of the hammer to the wall, BBII will not guarantee not damaging the wall. If damage to the adjacent wall occurs in any phase of the pile extraction operation described above, BBII will be compensated for repairs under CR T-010 as well.			
Please confirm the above as soon as possible. In addition, BBII requests immediate confirmation of allowable work hours for the work described above.			Please confirm the above as soon as possible. In addition, BBII requests immediate confirmation of allowable work hours for the work described above.			





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<b>T-0151</b>	<b>BSE - Buttress Footprint Increase Due to Oversized Casing</b>	<b>Closed</b>	<b>01</b>	<b>05/26/2011</b>	<b>06/05/2011</b>	<b>05/31/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference attached sketch						Reference attached sketch
Becho will be utilizing a 2200mm OD temporary casing for the Buttress Pile Installation. Becho requests that the spacing between tangent piles remain at 4" minimum and the secant piles overlap remain 1'-6". This will approximately increase the Buttress footprint by approximately 4'-4" to the east and 1'-9" to the south.						Becho will be utilizing a 2200mm OD temporary casing for the Buttress Pile Installation. Becho requests that the spacing between tangent piles remain at 4" minimum and the secant piles overlap remain 1'-6". This will approximately increase the Buttress footprint by approximately 4'-4" to the east and 1'-9" to the south.
Please confirm this is acceptable.						Please confirm this is acceptable.
<b>T-0152</b>	<b>BSE - Additional Timber Piles Adjacent 199 Fremont Building</b>	<b>Closed</b>	<b>01</b>	<b>05/26/2011</b>	<b>06/05/2011</b>	<b>06/07/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Sheet GT-2103 and RFI#T-0148						Reference Sheet GT-2103 and RFI#T-0148
In regards to item #4 in the response to RFI T-0148; field investigations of the curvature in first few piles removed along 199 Freemont, BBII feels that at a minimum it is necessary to remove all piles that's top is within 12" of the "neat line" 36" wide CDSM wall.						In regards to item #4 in the response to RFI T-0148; field investigations of the curvature in first few piles removed along 199 Freemont, BBII feels that at a minimum it is necessary to remove all piles that's top is within 12" of the "neat line" 36" wide CDSM wall.
Please confirm that removal of these piles to the limits described above, in addition to any associated damage to adjacent structures caused by the extraction will be reimbursed under CR T-010.						Please confirm that removal of these piles to the limits described above, in addition to any associated damage to adjacent structures caused by the extraction will be reimbursed under CR T-010.
Item 4: 4. BBII and TJPA will jointly determine the piles that can be left in place with reasonable assurance that they will not impact the shoring wall.						Item 4: 4. BBII and TJPA will jointly determine the piles that can be left in place with reasonable assurance that they will not impact the shoring wall.
<b>T-0153</b>	<b>BSE - Additional Timber Piles Adjacent 177/181 Fremont Building</b>	<b>Closed</b>	<b>01</b>	<b>05/26/2011</b>	<b>06/05/2011</b>	<b>06/07/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Sheet GT-2103 and RFI#T-0146.2						Reference Sheet GT-2103 and RFI#T-0146.2
In regards to item #4 in the response to RFI T-0146.2; field						In regards to item #4 in the response to RFI T-0146.2;





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	<p>investigations of the curvature in first few piles removed along 199 Fremont, BBII feels that at a minimum it is necessary to remove all piles that's top is within 12" of the "neat line" 36" wide CDSM wall.</p> <p>Please confirm that removal of these piles to the limits described above, in addition to any associated damage to adjacent structures caused by the extraction will be reimbursed under CR T-010.</p> <p>Also, please confirm allowable work hours, since 199 extractions have already begun.</p> <p>Item 4: 4. BBII and TJPA will jointly determine the piles that can be left in place with reasonable assurance that they will not impact the shoring wall.</p>					<p>field investigations of the curvature in first few piles removed along 199 Fremont, BBII feels that at a minimum it is necessary to remove all piles that's top is within 12" of the "neat line" 36" wide CDSM wall.</p> <p>Please confirm that removal of these piles to the limits described above, in addition to any associated damage to adjacent structures caused by the extraction will be reimbursed under CR T-010.</p> <p>Also, please confirm allowable work hours, since 199 extractions have already begun.</p> <p>Item 4: 4. BBII and TJPA will jointly determine the piles that can be left in place with reasonable assurance that they will not impact the shoring wall.</p>
T-0154	BSE - Becho Tremie Placement Process	Closed	01	05/26/2011	05/26/2011	05/31/2011
	<p>From: Webcor Construction LP                      Nhi Tran</p> <p>REQUEST:</p> <p>Reference Specification Section 31 63 29, 3.5.G.4.K</p> <p>SS31.63.29.3.5.G.4.k states "The tremie discharge end shall be immersed at least 25' in concrete at all times after starting the flow of concrete."</p> <p>Becho requests concrete tremie embedment to be reduced to 10ft minimum for all piles and 5ft minimum tremie embedment at the secondary pile transition zones between structural and CLSM mix pushing the minimum contaminated structural/CLSM concrete zone at sub grade to +5 foot above sub grade elevation.</p> <p>Please confirm this is acceptable.</p>					<p>ANSWER:</p> <p>Reference Specification Section 31 63 29, 3.5.G.4.K</p> <p>SS31.63.29.3.5.G.4.k states "The tremie discharge end shall be immersed at least 25' in concrete at all times after starting the flow of concrete."</p> <p>Becho requests concrete tremie embedment to be reduced to 10ft minimum for all piles and 5ft minimum tremie embedment at the secondary pile transition zones between structural and CLSM mix pushing the minimum contaminated structural/CLSM concrete zone at sub grade to +5 foot above sub grade elevation.</p> <p>Please confirm this is acceptable.</p>
T-0155	BSE - Primary Concrete Mix Tolerance	Closed	01	05/31/2011	06/10/2011	06/03/2011
	<p>From: Webcor Construction LP                      Nhi Tran</p> <p>REQUEST:</p>					<p>ANSWER:</p>





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	Reference Specification Section 03 30 01, 1.5.F					
	BBII, Becho, Central Concrete, W/O, ARUP and Adamson Associates met on Tuesday 5/24/2011 to discuss the results of Buttress Primary Concrete Mix Trial Batches. During this meeting, Central Concrete expressed concern about variability in the Buttress Primary Concrete mix due to slight variations in material and batching. The Buttress Primary Concrete Mix is a very high performance mix and even small variations in the mix constituents can result in significant changes in strength. Please advise how much of a working tolerance is acceptable for the primary buttress concrete mix.					
	Reference Specification Section 03 30 01, 1.5.F					
	BBII, Becho, Central Concrete, W/O, ARUP and Adamson Associates met on Tuesday 5/24/2011 to discuss the results of Buttress Primary Concrete Mix Trial Batches. During this meeting, Central Concrete expressed concern about variability in the Buttress Primary Concrete mix due to slight variations in material and batching. The Buttress Primary Concrete Mix is a very high performance mix and even small variations in the mix constituents can result in significant changes in strength. Please advise how much of a working tolerance is acceptable for the primary buttress concrete mix.					
<b>T-0156</b>	<b>BSE - Primary Concrete Mix 90-Day Compressive Strength</b>	<b>Closed</b>	<b>01</b>	<b>05/31/2011</b>	<b>06/10/2011</b>	<b>06/03/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>						
Reference Specification Section 03 30 01, 1.5.F						
Per Specification Section 03 30 01 - 1.5F Trial Batches: "The mixes shall be proportioned to develop a compressive strength of 2,000 psi at 28 days." Per the response to Question TG0300-0262, "The rate of strength gain can be reduced so that the design strength is reached after 28 days but less than 91 days".						
Please confirm that the Buttress Primary Shaft Concrete may take up to 90 days to achieve 2,000 psi.						
<b>ANSWER:</b>						
Reference Specification Section 03 30 01, 1.5.F						
Per Specification Section 03 30 01 - 1.5F Trial Batches: "The mixes shall be proportioned to develop a compressive strength of 2,000 psi at 28 days." Per the response to Question TG0300-0262, "The rate of strength gain can be reduced so that the design strength is reached after 28 days but less than 91 days".						
Please confirm that the Buttress Primary Shaft Concrete may take up to 90 days to achieve 2,000 psi.						



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<b>T-0156.1</b>	<b>BSE - 120 Day Acceptability of Buttress Primary Shaft Concrete</b>	<b>Closed</b>	<b>01</b>	<b>04/16/2012</b>	<b>04/26/2012</b>	<b>04/19/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: 4/12/12 Central Letter			Reference: 4/12/12 Central Letter			
BBII requests that in the event that the Buttress Primary Mix test specimens do not meet the 2,000 psi specified strength of 2,000 psi at 90 days (reference Response to previous RFIs #T-0157.2, and #T-0156), additional cylinders are to be taken and tested at 120 days. During this cooler climate, initial temperature may be impeding overall strength at the required time. Although only a few specimens are suspect of low strengths, Central Concrete is confident that at 120 days, the specimens in question will reach the required strength. If this criteria can be accepted for all test specimens at 120 days, this can mitigate any future concerns of suspect low strength.			BBII requests that in the event that the Buttress Primary Mix test specimens do not meet the 2,000 psi specified strength of 2,000 psi at 90 days (reference Response to previous RFIs #T-0157.2, and #T-0156), additional cylinders are to be taken and tested at 120 days. During this cooler climate, initial temperature may be impeding overall strength at the required time. Although only a few specimens are suspect of low strengths, Central Concrete is confident that at 120 days, the specimens in question will reach the required strength. If this criteria can be accepted for all test specimens at 120 days, this can mitigate any future concerns of suspect low strength.			
<b>T-0157</b>	<b>BSE - Primary Concrete Mix 500 PSI At 7-Days</b>	<b>Closed</b>	<b>01</b>	<b>05/31/2011</b>	<b>06/10/2011</b>	<b>06/03/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification Section 03 30 01, 2.2.E			Reference Specification Section 03 30 01, 2.2.E			
BBII, Becho, Central Concrete, W/O, ARUP and Adamson Associates met on Tuesday 5/24/2011 to discuss the results of Buttress Primary Concrete Mix Trial Batches. One of the concerns for the Buttress Primary Concrete is to provide a mix that is able to consistently achieve both 500 psi at 7 days and 2,000 psi at 28 days. The Buttress Primary Concrete Mix is a very high performance mix and even small variations in the mix constituents can result in significant changes in strength. Please advise if it acceptable to allow a working tolerance for the 500 psi requirement at 7 days.			BBII, Becho, Central Concrete, W/O, ARUP and Adamson Associates met on Tuesday 5/24/2011 to discuss the results of Buttress Primary Concrete Mix Trial Batches. One of the concerns for the Buttress Primary Concrete is to provide a mix that is able to consistently achieve both 500 psi at 7 days and 2,000 psi at 28 days. The Buttress Primary Concrete Mix is a very high performance mix and even small variations in the mix constituents can result in significant changes in strength. Please advise if it acceptable to allow a working tolerance for the 500 psi requirement at 7 days.			
<b>T-0157.1</b>	<b>BSE - PSI Schedule for Buttress Shaft Primary Mix</b>	<b>Closed</b>	<b>01</b>	<b>01/13/2012</b>	<b>01/23/2012</b>	<b>01/18/2012</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Kirk Nielsen						
<b>REQUEST:</b>			<b>ANSWER:</b>			

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T-0157.2	BSE - PSI Schedule for Buttress Shaft Primary Mix	Closed	CR	01/18/2012	01/28/2012	01/18/2012
From: Webcor/Obayashi Joint Venture      Kirk Nielsen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
To date there are multiple RFI responses that address the scheduled PSI requirements for the primary shaft mix which is resulting in confusion and unnecessary Vela issues. For clarification sake please confirm the following schedule is correct: 1. 300 psi at 7 days pursuant to RFI response T-0157. 2. 2000 psi based on an arithmetic average of tests on or before 90 days pursuant to RFI response T-0155 and T-0156.		To date there are multiple RFI responses that address the scheduled PSI requirements for the primary shaft mix which is resulting in confusion and unnecessary Vela issues. For clarification sake please confirm the following schedule is correct: 1. 300 psi at 7 days pursuant to RFI response T-0157. 2. 2000 psi based on an arithmetic average of tests on or before 90 days pursuant to RFI response T-0155 and T-0156.				



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<b>T-0157.3</b>	<b>BSE - PSI Schedule for Buttress Shaft Primary Mix</b>	<b>Closed</b>	<b>01</b>	<b>01/19/2012</b>	<b>01/29/2012</b>	<b>01/23/2012</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Kirk Nielsen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
To date there are multiple RFI responses that address the scheduled PSI requirements for the primary shaft mix which is resulting in confusion and unnecessary Vela issues. For clarification sake please confirm the following schedule is correct:			To date there are multiple RFI responses that address the scheduled PSI requirements for the primary shaft mix which is resulting in confusion and unnecessary Vela issues. For clarification sake please confirm the following schedule is correct:			
1. 300 psi at 7 days pursuant to RFI response T-0157.			1. 300 psi at 7 days pursuant to RFI response T-0157.			
2. 2000 psi based on an arithmetic average of tests on or before 90 days pursuant to RFI response T-0155 and T-0156.			2. 2000 psi based on an arithmetic average of tests on or before 90 days pursuant to RFI response T-0155 and T-0156.			
<b>T-0158</b>	<b>301 Mission Wall - Architect of Record</b>	<b>Closed</b>	<b>CR</b>	<b>06/01/2011</b>	<b>06/11/2011</b>	<b>06/06/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please clarify who is the registered Architect of Record, for the 301 Mission Interim Screen Wall Project.			Please clarify who is the registered Architect of Record, for the 301 Mission Interim Screen Wall Project.			



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T-0159	BSE - Unforeseen Obstruction - Timber Piles Within Pre-Trench Limits Zone 3	Closed	01	06/02/2011	06/12/2011	06/06/2011
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet D-2212, Specification Section 02 41 01, attached sketch and photo		Reference Sheet D-2212, Specification Section 02 41 01, attached sketch and photo				
During Pre-trench, BBII found additional unforeseen timber piles within the pre-trench limits along gridline A, between gridlines 24 & 25. Per Contract Drawing D-2212 (attached), there should only be a single row of timber piles in conflict with the CDSM wall, although when the area was exposed there are three rows within the CDSM wall limits (see attached photo). These will have to be removed and will be considered extra work.		During Pre-trench, BBII found additional unforeseen timber piles within the pre-trench limits along gridline A, between gridlines 24 & 25. Per Contract Drawing D-2212 (attached), there should only be a single row of timber piles in conflict with the CDSM wall, although when the area was exposed there are three rows within the CDSM wall limits (see attached photo). These will have to be removed and will be considered extra work.				
Please advise.		Please advise.				
T-0159.1	BSE - Unforeseen Obstruction - Timber Piles Within Pre-Trench Limits Zone 3	Closed	01	06/08/2011	06/18/2011	06/27/2011
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI#T-0159, Sheet D-2212, Specification Section 02 41 19, and attached photos		Reference RFI#T-0159, Sheet D-2212, Specification Section 02 41 19, and attached photos				
The Response to RFI#T-0159, appears to have misunderstood the question. Therefore BBII is providing additional information.		The Response to RFI#T-0159, appears to have misunderstood the question. Therefore BBII is providing additional information.				
BBII contends that the lower and smaller diameter piles as indicated in the attached sketch were not shown in either the contract drawings or the reference documents, therefore BBII was un-able to account for the removal of these piles in their bid item prices. These piles meet the general conditions article 3.05A.2 definition of an unforeseen condition, because that quantity of piles encountered exceeds that shown in the bid docs.		BBII contends that the lower and smaller diameter piles as indicated in the attached sketch were not shown in either the contract drawings or the reference documents, therefore BBII was un-able to account for the removal of these piles in their bid item prices. These piles meet the general conditions article 3.05A.2 definition of an unforeseen condition, because that quantity of piles encountered exceeds that shown in the bid docs.				
Please confirm the removal of the "unforeseen" timber piles in excess of those shown in the drawings, will be tracked and paid under a Force account contract change order similarly as done for Zone 4 pre-trench obstructions.		Please confirm the removal of the "unforeseen" timber piles in excess of those shown in the drawings, will be tracked and paid under a Force account contract change order similarly as done for Zone 4 pre-trench obstructions.				



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<b>T-0160</b>	<b>BSE - Timber Piles Not Extracted In Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>06/03/2011</b>	<b>06/13/2011</b>	<b>06/16/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference CR T-010 and attached summary and sketch		Reference CR T-010 and attached summary and sketch				
BBII continues to remove unforeseen timber piles along 199 Fremont Street in Zone 4 and soon will commence extraction along 181 Fremont Street.		BBII continues to remove unforeseen timber piles along 199 Fremont Street in Zone 4 and soon will commence extraction along 181 Fremont Street.				
As of May 31, 2011, BBII has left 7 piles in place as they were estimated to be more than 12" away from the limits of the CDSM shoring wall. In addition, 5 piles were broken during extraction a portion of which were left in place due to their proximity to the adjacent building walls. While these piles also appear to be more than 12" outside the limits of the CDSM shoring wall, due to possible undulations and alignment changes underground, the possibility of these piles encroaching into the CDSM shoring wall area exist.		As of May 31, 2011, BBII has left 7 piles in place as they were estimated to be more than 12" away from the limits of the CDSM shoring wall. In addition, 5 piles were broken during extraction a portion of which were left in place due to their proximity to the adjacent building walls. While these piles also appear to be more than 12" outside the limits of the CDSM shoring wall, due to possible undulations and alignment changes underground, the possibility of these piles encroaching into the CDSM shoring wall area exist.				
These piles are not shown on the contract plans and are extracted with extreme caution under the TJPA's direction and prescribed methods, taking the integrity of the adjacent buildings in consideration. Please confirm that it is the TJPA's intention to leave these piles in place.		These piles are not shown on the contract plans and are extracted with extreme caution under the TJPA's direction and prescribed methods, taking the integrity of the adjacent buildings in consideration. Please confirm that it is the TJPA's intention to leave these piles in place.				
<b>T-0161</b>	<b>BSE - CDSM Wall Soldier Pile Installation</b>	<b>Closed</b>	<b>01</b>	<b>06/03/2011</b>	<b>06/13/2011</b>	<b>06/06/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification Section 31 56 13, 3.13 and attached detail sketch		Reference Specification Section 31 56 13, 3.13 and attached detail sketch				
Is it acceptable to cut a 1.5" diameter hole, 16" from the bottom tip, in the web of the soldier beam pile beams? The purpose of the hole is to aid in securing the tail of the beam to the "dolly" that DND will use to raise the beams into a vertical position.		Is it acceptable to cut a 1.5" diameter hole, 16" from the bottom tip, in the web of the soldier beam pile beams? The purpose of the hole is to aid in securing the tail of the beam to the "dolly" that DND will use to raise the beams into a vertical position.				
<b>T-0162</b>	<b>BSE - Buttress Concrete Test Cylinders</b>	<b>Closed</b>	<b>01</b>	<b>06/03/2011</b>	<b>06/13/2011</b>	<b>06/08/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						



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	<b>REQUEST:</b>  Reference Specification Section 03 30 01 and attached summary of test results  BBII, Becho, Central Concrete, W/O, ARUP and Adamson Associates met on Tuesday 5/24/2011 to discuss the results of Buttress Primary Concrete Mix Trial Batches (please refer to the attachment for a summary of the test results). The 28-day test results for the 4x8 test cylinders were on average 57% of the core 4" diameter core test results. The 28-day test results for the 6x12 test cylinders were on average 88% of the 4" diameter core test results. The test samples were extracted from the same concrete batches, at the same time and cured in the same manner. BBII believes the difference in compressive strength between the test results may be attributed to the sample size & the resultant heat of hydration which drives the concrete cure rate. BBII also believes that the concrete cores may be more indicative of the actual in-situ concrete strength than the concrete test cylinders.  The Specification Section 03 30 01 - 1.5 F Trial Batches references "concrete cylinders", however it does not specify 4x8 or 6x12 test cylinders.  During the course of the meeting, it was generally agreed upon that 6x12 test cylinders appeared to be a more representative and consistent measure of the Primary Buttress Concrete strength relative to the core samples. BBII has confirmed through CTS that there should be no additional cost in sampling and testing a 4x8 cylinder relative to a 6x12 cylinder.  Therefore, BBII proposes that the 6x12 test cylinders should be used as the basis of acceptance testing both for the Trial Batches and also for future Field Quality Control and Testing for the Primary Buttress Concrete; 4x8 test cylinders should only be used for informational purposes only. Please confirm.					
	<b>ANSWER:</b>  Reference Specification Section 03 30 01 and attached summary of test results  BBII, Becho, Central Concrete, W/O, ARUP and Adamson Associates met on Tuesday 5/24/2011 to discuss the results of Buttress Primary Concrete Mix Trial Batches (please refer to the attachment for a summary of the test results). The 28-day test results for the 4x8 test cylinders were on average 57% of the core 4" diameter core test results. The 28-day test results for the 6x12 test cylinders were on average 88% of the 4" diameter core test results. The test samples were extracted from the same concrete batches, at the same time and cured in the same manner. BBII believes the difference in compressive strength between the test results may be attributed to the sample size & the resultant heat of hydration which drives the concrete cure rate. BBII also believes that the concrete cores may be more indicative of the actual in-situ concrete strength than the concrete test cylinders.  The Specification Section 03 30 01 - 1.5 F Trial Batches references "concrete cylinders", however it does not specify 4x8 or 6x12 test cylinders.  During the course of the meeting, it was generally agreed upon that 6x12 test cylinders appeared to be a more representative and consistent measure of the Primary Buttress Concrete strength relative to the core samples. BBII has confirmed through CTS that there should be no additional cost in sampling and testing a 4x8 cylinder relative to a 6x12 cylinder.  Therefore, BBII proposes that the 6x12 test cylinders should be used as the basis of acceptance testing both for the Trial Batches and also for future Field Quality Control and Testing for the Primary Buttress Concrete; 4x8 test cylinders should only be used for informational purposes only. Please confirm.					



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T-0163	BSE - Hazardous Material Removed From Site Zone 2	Closed	01	06/03/2011	06/13/2011	06/06/2011
<b>From:</b> Webcor Construction LP Nhi Tran						
<b>REQUEST:</b> <p>Reference Specification Section 00 03 35, 1.2</p> <p>During Investigation of Zone 2, BBII discovered potential lead based material existing on site. The specific area of concern is the pedestals on First Street.</p> <p>Please confirm that all contaminated material (specifically the referenced pedestals) as specified in the specification section 00 03 35 Article 1.2 has been removed and abated by the Demolition Contractor.</p> <p>BBII is scheduled to remove these pedestals next week and cannot proceed with this critical work until it is confirmed that the site is cleared of lead based materials as required by the Specifications.</p> <p>The TJPA's attention is directed to the following Section of the Specifications:</p> <p>SECTION 00 03 35 - EXISTING CONDITIONS: HAZARDOUS MATERIALS</p> <p>"1.2 HAZARDOUS MATERIALS REPORTS A. The TJPA's environmental consultants have surveyed the facility for the presence of various hazardous materials. Materials investigated may include asbestos, lead, PCB ballasts, mercury containing lamps, contaminated soils, underground storage tanks, and other hazardous materials. The demolition contractor for the Demolition project (Evans Brothers Inc.) is responsible for removing and abating products containing asbestos, lead, or PCB ballast, and mercury-containing lamps."</p>		<b>ANSWER:</b> <p>Reference Specification Section 00 03 35, 1.2</p> <p>During Investigation of Zone 2, BBII discovered potential lead based material existing on site. The specific area of concern is the pedestals on First Street.</p> <p>Please confirm that all contaminated material (specifically the referenced pedestals) as specified in the specification section 00 03 35 Article 1.2 has been removed and abated by the Demolition Contractor.</p> <p>BBII is scheduled to remove these pedestals next week and cannot proceed with this critical work until it is confirmed that the site is cleared of lead based materials as required by the Specifications.</p> <p>The TJPA's attention is directed to the following Section of the Specifications:</p> <p>SECTION 00 03 35 - EXISTING CONDITIONS: HAZARDOUS MATERIALS</p> <p>"1.2 HAZARDOUS MATERIALS REPORTS A. The TJPA's environmental consultants have surveyed the facility for the presence of various hazardous materials. Materials investigated may include asbestos, lead, PCB ballasts, mercury containing lamps, contaminated soils, underground storage tanks, and other hazardous materials. The demolition contractor for the Demolition project (Evans Brothers Inc.) is responsible for removing and abating products containing asbestos, lead, or PCB ballast, and mercury-containing lamps."</p>				





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T-0164	BSE - Timber Piles Adjacent 177/181 Fremont Building South Zone 4	Closed	01	06/06/2011	06/16/2011	06/06/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference RFI@T-0146.1 [BBI 0104] and attached photo		Reference RFI@T-0146.1 [BBI 0104] and attached photo				
Per [RFI #T-0146.1] RFI 104 Response, BBII inserted a metal sheet behind the timber piles required to be removed, in the location between 199 and 181 Fremont. The sheet is to hold back the soil in the alley. Due to the close proximity of the timber piles, the sheet location is too close to the timber piles required to be removed from the CDSM Wall Location. The sheet is too close for the pile extractor to attach to the tops of the pile. See Attached Photo.		Per [RFI #T-0146.1] RFI 104 Response, BBII inserted a metal sheet behind the timber piles required to be removed, in the location between 199 and 181 Fremont. The sheet is to hold back the soil in the alley. Due to the close proximity of the timber piles, the sheet location is too close to the timber piles required to be removed from the CDSM Wall Location. The sheet is too close for the pile extractor to attach to the tops of the pile. See Attached Photo.				
Please Advise in detail.		Please Advise in detail.				
T-0165	BSE - High pH Water Found In Zone 3 Pre-Trenching	Closed	01	06/07/2011	06/17/2011	06/10/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference Specification Section 00 08 13, 1.9.C		Reference Specification Section 00 08 13, 1.9.C				
BBI found high pH water while digging an exploratory hole in the Fremont St. side of Zone 3. This was confirmed by Peter Cusack from Treadwell & Rollo. Specification Section 00.08.13.1.9.C states that "Should the existing wastewater be contaminated, or should it be uncontaminated but subsequently become contaminated as a result of conditions other than the Contractor's operations, a Change Order will be issued..".		BBI found high pH water while digging an exploratory hole in the Fremont St. side of Zone 3. This was confirmed by Peter Cusack from Treadwell & Rollo. Specification Section 00.08.13.1.9.C states that "Should the existing wastewater be contaminated, or should it be uncontaminated but subsequently become contaminated as a result of conditions other than the Contractor's operations, a Change Order will be issued..".				
Please consider this as a Notice of Existing Contaminated Wastewater as defined by SS00.08.13.1.9.C. Please advise on how to proceed.		Please consider this as a Notice of Existing Contaminated Wastewater as defined by SS00.08.13.1.9.C. Please advise on how to proceed.				
T-0166	BSE - Unknown Concrete Structure at 199 Fremont Zone 4 (Gridline 33-30)	Closed	01	06/07/2011	06/17/2011	06/22/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference RFI#T-0144 (BBI RFI 0103), Specification Section 31 56 13, and attached Turner Field Condition		Reference RFI#T-0144 (BBI RFI 0103), Specification Section 31 56 13, and attached Turner Field Condition				



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	<p>Report 056 and photos</p> <p>BBII demolished the Unforeseen Concrete Structure along 199 Fremont St., and associated curb per RFI #103 [RFI#T-0144] response. During the process, due to the previous contractor's construction means, the curb inadvertently damaged the metal flashing, and possibly the waterproofing beside it.</p> <p>Along with the curb, the fence panel was built on top of the Unforeseen Concrete Structure, so when the structure was removed, the fence came down too.</p> <p>See attached pictures and Turner Field Condition Report (5/24/11)</p> <p>BBII requests immediate direction from the TJPA on this issue.</p>					
<b>T-0166.1</b>	<b>BSE - Unknown Concrete Structure at 199 Fremont Zone 4 (Gridline 33-30)</b>	<b>Closed</b>	<b>01</b>	<b>07/20/2011</b>	<b>07/30/2011</b>	<b>07/26/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>						
Reference RFI #T-0144, RFI #T-0166 and Specification 31 56 13						
Per the response to RFI#T-0166 (BBI RFI 103.1), please provide an acceptable repair procedure for the 199 Fremont building. Also, please confirm that the repair work will be included in CR T-010.						
<b>ANSWER:</b>						
Reference RFI #T-0144, RFI #T-0166 and Specification 31 56 13						
Per the response to RFI#T-0166 (BBI RFI 103.1), please provide an acceptable repair procedure for the 199 Fremont building. Also, please confirm that the repair work will be included in CR T-010.						



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<b>T-0167</b>	<b>Survey Grid Control Documents</b>	<b>Closed</b>	<b>CR</b>	<b>06/08/2011</b>	<b>06/10/2011</b>	<b>06/20/2011</b>
<b>From:</b> Webcor Construction LP      Tim Maxwell						
<b>REQUEST:</b>  Reference RFI T-0112.1 and drawing GT-0100  As requested by Ed Sum in today's (6/8/11) OAC meeting we submit the following question:  Please confirm that gridlines as established from the GT-0100 and as confirmed on Chaudhary & Associates Survey Grid Control Documents (Ref: RFI T-0112.1) can be used for all future construction elements (i.e., CDSM wall, etc). Please confirm by 6/10/11.		<b>ANSWER:</b>  Reference RFI T-0112.1 and drawing GT-0100  As requested by Ed Sum in today's (6/8/11) OAC meeting we submit the following question:  Please confirm that gridlines as established from the GT-0100 and as confirmed on Chaudhary & Associates Survey Grid Control Documents (Ref: RFI T-0112.1) can be used for all future construction elements (i.e., CDSM wall, etc). Please confirm by 6/10/11.				
<b>T-0167.1</b>	<b>Survey Grid Control Documents</b>	<b>Closed</b>	<b>CR</b>	<b>07/01/2011</b>	<b>07/11/2011</b>	<b>07/05/2011</b>
<b>From:</b> Webcor Construction LP      Daniel Foudy						
<b>REQUEST:</b>  Please provide City Survey of property lines with a translation to grid for our use.		<b>ANSWER:</b>  Please provide City Survey of property lines with a translation to grid for our use.				
<b>T-0168</b>	<b>BSE - Soil Classification Data</b>	<b>Closed</b>	<b>01</b>	<b>06/08/2011</b>	<b>06/18/2011</b>	<b>06/22/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 01 13 50  The Class 1 and Class 2 Disposal site does not want to use the old "PSI for Caltrans" Reports in the Soil Profile, due to the lack of necessary tests, missing pages in the report, and age.  The Disposal site recommends the use of the Treadwell & Rollo reports from 2008 and 2009, and to dismiss the "PSI for Caltrans" reports.  Please Advise.		<b>ANSWER:</b>  Reference Specification Section 01 13 50  The Class 1 and Class 2 Disposal site does not want to use the old "PSI for Caltrans" Reports in the Soil Profile, due to the lack of necessary tests, missing pages in the report, and age.  The Disposal site recommends the use of the Treadwell & Rollo reports from 2008 and 2009, and to dismiss the "PSI for Caltrans" reports.  Please Advise.				
<b>T-0169</b>	<b>BSE - Disposal of Drilling Spoils</b>	<b>Closed</b>	<b>01</b>	<b>06/09/2011</b>	<b>06/19/2011</b>	<b>07/07/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						





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	<p>D-5103 along with the response to Pre-Bid RFI #TG0300-014 describe the finish grades and subsequent quantities of crushed 3" minus concrete to be left on site for the BSE package. In summary, Zone 4 was to be left with a depression as shown on GT-1303 and Zone 1-3 were to be left no higher than existing ground elevations.</p> <p>Previous discussions between BBII, W/O, EBI and TJPA were made to accommodate BBII's early access into Zones 1-3 for pre-trenching. At the time of these discussions EBI indicated they were short approximately 7000 cy of balancing the site and that they would not be able to get that remaining 7000 cy until the existing ramps were demolished. As a result of the short term shortage and in exchange for access to zone 1-3 BBII agreed to:</p> <ul style="list-style-type: none"><li>- Allow EBI to leave Zone 3 low of the Existing elevations</li><li>- Allow EBI to set up Crusher in Zone 2 for ramp demolition</li><li>- Allow EBI to leave the 7000 cy shortage in a stockpile in Zone 2, for our later use.</li></ul> <p>BBII appreciated the partnering agreement however the current size of the stockpile is far greater than BBII ever expected. BBII surveyed the stockpile and the Zone 3 depression on 6/7/11 after they completed their export to zone 4 and BBII estimates the size of the concrete stockpile to be in excess of 11,000 cy (this does not include the asphalt stockpile that was created after the survey).</p> <p>Based on BBII's calculations (see attached topo) Zone 3 was left approximately 2000 cy short of existing grade and 5000 cy were taken from the stockpile to Zone 4. As a result BBII requests the current stockpile be removed in its entirety from the site, as it is in excess of the contractual amount to be removed by the BSE contract.</p> <p>However, If acceptable to TJPA, BBII would be interested in taking 2000 cy of the crushed concrete if it could be delivered and stockpiled in an mutually agreeable staging area. BBII suggests Lot S. This material would then be used as need for excavation stabilization throughout the BSE contract.</p>					
	<p>and D-5103 along with the response to Pre-Bid RFI #TG0300-014 describe the finish grades and subsequent quantities of crushed 3" minus concrete to be left on site for the BSE package. In summary, Zone 4 was to be left with a depression as shown on GT-1303 and Zone 1-3 were to be left no higher than existing ground elevations.</p> <p>Previous discussions between BBII, W/O, EBI and TJPA were made to accommodate BBII's early access into Zones 1-3 for pre-trenching. At the time of these discussions EBI indicated they were short approximately 7000 cy of balancing the site and that they would not be able to get that remaining 7000 cy until the existing ramps were demolished. As a result of the short term shortage and in exchange for access to zone 1-3 BBII agreed to:</p> <ul style="list-style-type: none"><li>- Allow EBI to leave Zone 3 low of the Existing elevations</li><li>- Allow EBI to set up Crusher in Zone 2 for ramp demolition</li><li>- Allow EBI to leave the 7000 cy shortage in a stockpile in Zone 2, for our later use.</li></ul> <p>BBII appreciated the partnering agreement however the current size of the stockpile is far greater than BBII ever expected. BBII surveyed the stockpile and the Zone 3 depression on 6/7/11 after they completed their export to zone 4 and BBII estimates the size of the concrete stockpile to be in excess of 11,000 cy (this does not include the asphalt stockpile that was created after the survey).</p> <p>Based on BBII's calculations (see attached topo) Zone 3 was left approximately 2000 cy short of existing grade and 5000 cy were taken from the stockpile to Zone 4. As a result BBII requests the current stockpile be removed in its entirety from the site, as it is in excess of the contractual amount to be removed by the BSE contract.</p> <p>However, If acceptable to TJPA, BBII would be interested in taking 2000 cy of the crushed concrete if it could be delivered and stockpiled in an mutually agreeable staging area. BBII suggests Lot S. This</p>					



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material would then be used as need for excavation stabilization throughout the BSE contract.						
<b>T-0171</b>	<b>BSE - Concrete Section Protruding Into CDSM Shoring Wall Area Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>06/13/2011</b>	<b>06/23/2011</b>	<b>06/17/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference attached photo			Reference attached photo			
While excavating a pile next to 181 Fremont Street, a section of concrete that was protruding into the CDSM shoring wall area fell from the foundation wall of 181 Fremont. Please advise on how to proceed.			While excavating a pile next to 181 Fremont Street, a section of concrete that was protruding into the CDSM shoring wall area fell from the foundation wall of 181 Fremont. Please advise on how to proceed.			
<b>T-0172</b>	<b>LEED Submittal Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>06/13/2011</b>	<b>06/23/2011</b>	<b>06/21/2011</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref Spec Section 01 81 13 Section 1.5:			Ref Spec Section 01 81 13 Section 1.5:			
According to spec section 018113.1.5, LEED submittals shall be submitted in addition to other submittal requirements specified elsewhere. If a submitted item is identical to an item submitted to comply with other requirements, a duplicate copy is to be submitted. In effort to minimize duplicate submittals, please confirm it is acceptable to issue one submittal package to cover both the technical spec. and LEED spec section requirements.			According to spec section 018113.1.5, LEED submittals shall be submitted in addition to other submittal requirements specified elsewhere. If a submitted item is identical to an item submitted to comply with other requirements, a duplicate copy is to be submitted. In effort to minimize duplicate submittals, please confirm it is acceptable to issue one submittal package to cover both the technical spec. and LEED spec section requirements.			



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<b>T-0173</b>	<b>BSE - Enhanced Trial Batch Testing</b>	<b>Closed</b>	<b>01</b>	<b>06/13/2011</b>	<b>06/23/2011</b>	<b>06/15/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>						
Reference Specification Section 03 30 01, 2.2.E and attached mix designs						
BBII, Becho, Central Concrete, W/O, ARUP and Adamson Associates met on Tuesday 5/24/2011 to discuss the results of Buttress Primary Concrete Mix Trial Batches. Based upon the preliminary results of the 2nd Trial Batch, BBII proposes to submit the following three mixes for approval for use on the Buttress Primary Shaft Concrete: 1. Mix 1: 85AEC3B6 2. Mix 5: 86AEC3A6 3. Mix 7: 87AEC3A6						
BBII believes that having additional mixes available for use as the Buttress Primary Concrete would be of great benefit to the Project. BBII proposes "enhanced testing" of these three mixes as well as three additional hybrids of each mix for a total of nine mixes (please see attached for mix designs). The intent of the enhanced testing is to further refine the information we currently have on all three of the above three mixes, as well develop additional mixes for future use as Primary Shaft Concrete.						
One of the concerns of 1st and 2nd Trial Batches was potentially accelerated curing due to the Styrofoam insulated boxes in which the trial batch "cubes" were cast. BBII proposes a 3rd trial batch using all of the same methodology of the approved trial batch method placing, the only exception being that the concrete will be cast into +/- 5'x5'x4' deep excavations in lieu of the Styrofoam insulated forms. Each mix would be placed in an individual excavation, lined with plastic to retain moisture. All other aspects of the proposed trial batch methodology would be as previously submitted & approved.						
The results of the "enhanced testing" would be evaluated and possibly submitted for approval as additional Buttress Primary Shaft Concrete Mixes.						
Please confirm that this is acceptable.						
<b>ANSWER:</b>						
Reference Specification Section 03 30 01, 2.2.E and attached mix designs						
BBII, Becho, Central Concrete, W/O, ARUP and Adamson Associates met on Tuesday 5/24/2011 to discuss the results of Buttress Primary Concrete Mix Trial Batches. Based upon the preliminary results of the 2nd Trial Batch, BBII proposes to submit the following three mixes for approval for use on the Buttress Primary Shaft Concrete: 1. Mix 1: 85AEC3B6 2. Mix 5: 86AEC3A6 3. Mix 7: 87AEC3A6						
BBII believes that having additional mixes available for use as the Buttress Primary Concrete would be of great benefit to the Project. BBII proposes "enhanced testing" of these three mixes as well as three additional hybrids of each mix for a total of nine mixes (please see attached for mix designs). The intent of the enhanced testing is to further refine the information we currently have on all three of the above three mixes, as well develop additional mixes for future use as Primary Shaft Concrete.						
One of the concerns of 1st and 2nd Trial Batches was potentially accelerated curing due to the Styrofoam insulated boxes in which the trial batch "cubes" were cast. BBII proposes a 3rd trial batch using all of the same methodology of the approved trial batch method placing, the only exception being that the concrete will be cast into +/- 5'x5'x4' deep excavations in lieu of the Styrofoam insulated forms. Each mix would be placed in an individual excavation, lined with plastic to retain moisture. All other aspects of the proposed trial batch methodology would be as previously submitted & approved.						
The results of the "enhanced testing" would be evaluated and possibly submitted for approval as additional Buttress Primary Shaft Concrete Mixes.						
Please confirm that this is acceptable.						





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<b>T-0174</b>	<b>301 Mission Wall - New Curb Detail</b>	<b>Closed</b>	<b>CR</b>	<b>06/14/2011</b>	<b>06/24/2011</b>	<b>06/20/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: Attached sheet C-5000  The required curb details are not clearly defined. Is new curb set atop finish pavers, onto topping slab, or set all the way down to structural slab. Additionally, provide all applicable rebar details to match condition.					<b>ANSWER:</b>  Reference: Attached sheet C-5000  The required curb details are not clearly defined. Is new curb set atop finish pavers, onto topping slab, or set all the way down to structural slab. Additionally, provide all applicable rebar details to match condition.	
<b>T-0175</b>	<b>301 Mission Wall - Concrete Mix for Curb Around Existing Manhole Covers</b>	<b>Closed</b>	<b>CR</b>	<b>06/15/2011</b>	<b>06/25/2011</b>	<b>06/20/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference drawing C-2000  The existing curb around the manholes at the east and west ends of the 301 Mission Wall is unknown. Design documents do not provide information as to the specs of this concrete mixture. The existing concrete appears to have a color added to the mix design. Please provide a mix design and color specification (if necessary) to use at these locations.					<b>ANSWER:</b>  Reference drawing C-2000  The existing curb around the manholes at the east and west ends of the 301 Mission Wall is unknown. Design documents do not provide information as to the specs of this concrete mixture. The existing concrete appears to have a color added to the mix design. Please provide a mix design and color specification (if necessary) to use at these locations.	
<b>T-0176</b>	<b>301 Mission Wall - Fill Pour Back and New Curbs</b>	<b>Closed</b>	<b>CR</b>	<b>06/15/2011</b>	<b>06/25/2011</b>	<b>06/20/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Should the concrete mix design for the fill pour back and 9"x12" curbs along the north side of the 301 Mission wall be the same mix that is used for the new curb around the manhole? The mix design for curbs around the existing manhole was requested in RFI T-0175. Please advise.					<b>ANSWER:</b>  Should the concrete mix design for the fill pour back and 9"x12" curbs along the north side of the 301 Mission wall be the same mix that is used for the new curb around the manhole? The mix design for curbs around the existing manhole was requested in RFI T-0175. Please advise.	
<b>T-0177</b>	<b>BSE - Alternate Method Of Pile Removal Along 181 Fremont</b>	<b>Closed</b>	<b>01</b>	<b>06/15/2011</b>	<b>06/25/2011</b>	<b>06/16/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference attached procedure, photos, and sketch					<b>ANSWER:</b>  Reference attached procedure, photos, and sketch	





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	<p>During the extraction of unforeseen piles along 181 Fremont, two piles located inside the proposed CDSM wall broke and are now too deep to extract under using the current extraction method. During the attempted extraction of pile 151, the pile continued to break. The top of this pile is approximately 9' below the base of the foundation wall. Considering the length of the adjacent removed piles, there is approximately 6' left to be removed. Pile 105 is approximately 6' below the base of the foundation wall leaving approximately 12'-14' to be removed. Further excavation to expose these piles is not reasonable. BBII proposes to drill the remainder of each pile out. See below the proposed procedure as per committee meeting and consultation with Viking Drillers Inc. on 6-15-11. It was agreed that this work will be charged to CR T-010. Also attached are photos and a drawing indicating the location of both broken piles (105 and 151).</p> <p>Please provide direction.</p>					
T-0178	<b>BSE - Connector Wall Layout</b>	Closed	01	06/16/2011	06/26/2011	06/21/2011
	<b>From:</b> Webcor Construction LP      Nhi Tran					
	<b>REQUEST:</b>  Reference RFI#T-0151 and Sheets GT-2103 and GT-2201  Per the Engineer's response to RFI#T-0151, it is acceptable to expand the overall Buttress 4'-4" to the east. Please advise if the CDSM connector columns can still be installed per contract drawings GT-2103 and GT-2201.					
						<b>ANSWER:</b>  Reference RFI#T-0151 and Sheets GT-2103 and GT-2201  Per the Engineer's response to RFI#T-0151, it is acceptable to expand the overall Buttress 4'-4" to the east. Please advise if the CDSM connector columns can still be installed per contract drawings GT-2103 and GT-2201.



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T-0179	301 Mission Wall - Detail at Steel Baseplates on South Side	Closed	CR	06/21/2011	07/01/2011	07/11/2011
From: Webcor Construction LP                      David Hungerford						
REQUEST:  "Reference drawing D/A-6000 and attached sketch  Detail D/A-6000 does not provide a plywood panel termination detail at the steel baseplate locations along the south side of the 301 Mission wall. At the locations of the steel baseplates, use of sealant and backer rod would leave the steel baseplate exposed (see attached sketch). Please advise."						ANSWER:  "Reference drawing D/A-6000 and attached sketch  Detail D/A-6000 does not provide a plywood panel termination detail at the steel baseplate locations along the south side of the 301 Mission wall. At the locations of the steel baseplates, use of sealant and backer rod would leave the steel baseplate exposed (see attached sketch). Please advise."
T-0180	BSE - CDSM Wall Tolerance	Closed	01	06/22/2011	07/02/2011	06/22/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:  Reference Specification Section 31 56 13  As requested by the TJPA, DND submits this request to modify the horizontal tolerance for the CDSM shoring wall. The new goal is to set the wall 2" outside of the original planned centerline of shoring wall. This solution has been proposed by the TJPA in order to not encroach into the structure at the bottom of the train box.  DND respectfully requests the maximum soldier pile & CDSM wall tolerances be revised to 0 inches into the trainbox & up to 5 inches outside the trainbox.  There will be no additional excavation and/or bracing costs associated with this increase in tolerance from BBI. However; there may be future additional cost impacts to the Structural Concrete & Waterproofing that are to be handled in future trade packages.  Please confirm, if this is acceptable.						ANSWER:  Reference Specification Section 31 56 13  As requested by the TJPA, DND submits this request to modify the horizontal tolerance for the CDSM shoring wall. The new goal is to set the wall 2" outside of the original planned centerline of shoring wall. This solution has been proposed by the TJPA in order to not encroach into the structure at the bottom of the train box.  DND respectfully requests the maximum soldier pile & CDSM wall tolerances be revised to 0 inches into the trainbox & up to 5 inches outside the trainbox.  There will be no additional excavation and/or bracing costs associated with this increase in tolerance from BBI. However; there may be future additional cost impacts to the Structural Concrete & Waterproofing that are to be handled in future trade packages.  Please confirm, if this is acceptable.
T-0180.1	BSE - CDSM Wall Tolerance	Closed	01	06/24/2011	07/04/2011	07/07/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:						ANSWER:



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	Reference Response to RFI#T-0180  Please delete the first sentence "TJPA did not request this RFI" of the response for RFI T-0180, because it is the wrong statement. Emilio Cruz, PMPC, requested to submit this RFI at the Schedule Review Meeting on 6/14/2011 at W-O JV Office Conference Room, 183 Fremont St.					Reference Response to RFI#T-0180  Please delete the first sentence "TJPA did not request this RFI" of the response for RFI T-0180, because it is the wrong statement. Emilio Cruz, PMPC, requested to submit this RFI at the Schedule Review Meeting on 6/14/2011 at W-O JV Office Conference Room, 183 Fremont St.
<b>T-0181</b>	<b>BSE - CDSM Pile Tolerance Measurement Location</b>	<b>Closed</b>	<b>01</b>	<b>06/22/2011</b>	<b>07/02/2011</b>	<b>07/01/2011</b>
	<b>From:</b> Webcor Construction LP      Nhi Tran  <b>REQUEST:</b>  Reference Specification Section 31 56 13  BBII's subcontractor DND would like to confirm the exact location of the soldier pile, where the pile tolerance is to be measured. Please find below DND's question:  "It is our understanding that the tolerance of the soldier pile beams is to be measured at the plan top of pile elevation. Is this correct?"  Please confirm that DND's interpretation of the pile tolerance measurement is correct.					<b>ANSWER:</b>  Reference Specification Section 31 56 13  BBII's subcontractor DND would like to confirm the exact location of the soldier pile, where the pile tolerance is to be measured. Please find below DND's question:  "It is our understanding that the tolerance of the soldier pile beams is to be measured at the plan top of pile elevation. Is this correct?"  Please confirm that DND's interpretation of the pile tolerance measurement is correct.



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<b>T-0181.1</b>	<b>BSE - CDSM Tolerances</b>	<b>Closed</b>	<b>01</b>	<b>07/21/2011</b>	<b>07/31/2011</b>	<b>07/26/2011</b>
<b>From:</b> Webcor Construction LP Nhi Tran						
<b>REQUEST:</b> <p>Reference RFIs #T-180, #T-0180.1, #T-0181 and Specification Section 31 56 13</p> <p>Previous RFIs T-180, T-180.1, and T-181 have all addressed CDSM shoring wall tolerances. Below is BBII's interpretation of the responses:</p> <p>1. Horizontal Tolerance:  a) CDSM Columns: 0" in towards the train box, 2" maximum away from the train box - measured relative to the "plan" CDSM shoring wall centerline located at the ground surface (original grade) at the start of drilling (W/O comment - Reference Specification Section 31 56 13, 3.3.A)</p> <p>b) Steel Soldier Pile: 0" in towards the train box, 4" maximum away from the trainbox - measured relative to the "plan" CDSM shoring wall centerline located at the ground surface (original grade) at the start of drilling (W/O comment - Reference Specification Section 31 56 13, 3.13.B.8)</p> <p>2. Vertical Tolerance:  a) CDSM Columns: Inclination deviation no more than 1:150 (horizontal to vertical)  (W/O comment - Same as stated in Specification Section 31 56 13, 3.4.A)</p> <p>b) Steel Soldier Pile: Inclination no more than 1:200 (horizontal to vertical)  (W/O comment - Same as stated in Specification Section 31 56 13, 3.13.B.9)</p> <p>Please confirm this is acceptable</p>		<b>ANSWER:</b> <p>Reference RFIs #T-180, #T-0180.1, #T-0181 and Specification Section 31 56 13</p> <p>Previous RFIs T-180, T-180.1, and T-181 have all addressed CDSM shoring wall tolerances. Below is BBII's interpretation of the responses:</p> <p>1. Horizontal Tolerance:  a) CDSM Columns: 0" in towards the train box, 2" maximum away from the train box - measured relative to the "plan" CDSM shoring wall centerline located at the ground surface (original grade) at the start of drilling (W/O comment - Reference Specification Section 31 56 13, 3.3.A)</p> <p>b) Steel Soldier Pile: 0" in towards the train box, 4" maximum away from the trainbox - measured relative to the "plan" CDSM shoring wall centerline located at the ground surface (original grade) at the start of drilling (W/O comment - Reference Specification Section 31 56 13, 3.13.B.8)</p> <p>2. Vertical Tolerance:  a) CDSM Columns: Inclination deviation no more than 1:150 (horizontal to vertical)  (W/O comment - Same as stated in Specification Section 31 56 13, 3.4.A)</p> <p>b) Steel Soldier Pile: Inclination no more than 1:200 (horizontal to vertical)  (W/O comment - Same as stated in Specification Section 31 56 13, 3.13.B.9)</p> <p>Please confirm this is acceptable</p>				



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<b>T-0182</b>	<b>BSE - Inclinometer Locations Within The CDSM Wall</b>	<b>Closed</b>	<b>01</b>	<b>06/23/2011</b>	<b>07/03/2011</b>	<b>06/24/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheets GT-1301, GT-1302, Specification Section 31 56 13, and Transmittal No. 140-01802 (attached)		Reference Sheets GT-1301, GT-1302, Specification Section 31 56 13, and Transmittal No. 140-01802 (attached)				
Please refer to the Instrumentation Plan within the contract drawings GT-1301 & GT-1302, which depicts the rough locations of the 15 inclinometers (IW-1 through IW-15) that are to be installed through the CDSM shoring wall. Please notify BBII of the exact locations of those inclinometers by utilizing the soldier pile numbers 1 through 681, sent in Transmittal No. 140-01802 (attached).		Please refer to the Instrumentation Plan within the contract drawings GT-1301 & GT-1302, which depicts the rough locations of the 15 inclinometers (IW-1 through IW-15) that are to be installed through the CDSM shoring wall. Please notify BBII of the exact locations of those inclinometers by utilizing the soldier pile numbers 1 through 681, sent in Transmittal No. 140-01802 (attached).				
<b>T-0182.1</b>	<b>BSE - Connector Wall Inclinometer Locations</b>	<b>Closed</b>	<b>01</b>	<b>06/30/2011</b>	<b>07/10/2011</b>	<b>07/05/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI#T-0182, Transmittal No. 140-01802, and Specification Section 31 56 13		Reference RFI#T-0182, Transmittal No. 140-01802, and Specification Section 31 56 13				
BBII is in receipt of the Engineer's response to RFI T-0182, which lists the fourteen pile numbers where the inclinometers will be installed. Please note that pile # 443 was already installed on 06/18/2011, as part of the CDSM test panel.		BBII is in receipt of the Engineer's response to RFI T-0182, which lists the fourteen pile numbers where the inclinometers will be installed. Please note that pile # 443 was already installed on 06/18/2011, as part of the CDSM test panel.				
Can the inclinometer casing be installed at pile # 446, instead of pile # 443?		Can the inclinometer casing be installed at pile # 446, instead of pile # 443?				
<b>T-0183</b>	<b>BSE - Connector Wall Shift</b>	<b>Closed</b>	<b>01</b>	<b>06/23/2011</b>	<b>07/03/2011</b>	<b>06/27/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI#T-0178, Sheets GT-2201, GT-5101, and attached sketch		Reference RFI#T-0178, Sheets GT-2201, GT-5101, and attached sketch				
Per the Engineer's response to RFI T-0178, it is acceptable to shift the CDSM Connector Columns to the east and to add additional columns to provide CDSM material for the full width of the Buttress. Please confirm that it is acceptable to shift the lower three rows of the		Per the Engineer's response to RFI T-0178, it is acceptable to shift the CDSM Connector Columns to the east and to add additional columns to provide CDSM material for the full width of the Buttress. Please confirm that it is acceptable to shift the lower				



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	<p>CDSM Connector Columns approximately 3'-6" to the east and add two more columns to the top row. Additionally, please confirm that the CDSM Shoring Wall between Gridlines 26 and 30 can still be installed per GT-2201 and Table 16/GT-5101.</p>					<p>three rows of the CDSM Connector Columns approximately 3'-6" to the east and add two more columns to the top row. Additionally, please confirm that the CDSM Shoring Wall between Gridlines 26 and 30 can still be installed per GT-2201 and Table 16/GT-5101.</p>
T-0183.1	<p><b>BSE - Connector Wall Shift</b></p> <p>From: Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference RFI#T-0151, RFI#T-0178, RFI#T-0183, Specification Sections 31 63 29 and 31 56 13, and attached drawing</p> <p>Please refer to the Engineer's response to RFI # T-0151, which accepted the expansion of the Buttress 4'-4" to the east. Please also refer to the Engineer's response to RFI No. T-#0178, where the designer required the connector columns be shifted and/or supplemented with additional columns to provide CDSM material for the full width of the buttress. BBII suggests to revise the connector column layout per the attached drawing and install two additional connector columns at Grid "A" and "30" intersection.</p> <p>Please confirm, if the proposed revision of the CDSM connector columns according to the attached drawing fulfills the design requirement.</p> <p>Also, please issue revised construction drawings that would reflect the changes made to the Buttress and the CDSM connector walls.</p>	Closed	01	06/30/2011	07/10/2011	07/11/2011
						<p><b>ANSWER:</b></p> <p>Reference RFI#T-0151, RFI#T-0178, RFI#T-0183, Specification Sections 31 63 29 and 31 56 13, and attached drawing</p> <p>Please refer to the Engineer's response to RFI # T-0151, which accepted the expansion of the Buttress 4'-4" to the east. Please also refer to the Engineer's response to RFI No. T-#0178, where the designer required the connector columns be shifted and/or supplemented with additional columns to provide CDSM material for the full width of the buttress. BBII suggests to revise the connector column layout per the attached drawing and install two additional connector columns at Grid "A" and "30" intersection.</p> <p>Please confirm, if the proposed revision of the CDSM connector columns according to the attached drawing fulfills the design requirement.</p> <p>Also, please issue revised construction drawings that would reflect the changes made to the Buttress and the CDSM connector walls.</p>



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<b>T-0184</b>	<b>BSE - CIDH Pile Rebar Cage Hoop Size</b>	<b>Closed</b>	<b>01</b>	<b>06/27/2011</b>	<b>07/07/2011</b>	<b>06/28/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Sheet GT-5202, Specification Section 03 20 01, attached sketch, and approved Shop Drawings from Package TA2010-032001A05  Drawing 12/GT-5202 shows 5" clearance between the hoop OD and the inside diameter of a 7' +/- 2" shaft. Per discussions with Becho, at least 3" of clearance is needed between the rebar spacers and the ID of the casing to facilitate proper installation of the rebar cages inside the casing.  BBII would like to propose 7 1/4" minimum clearance in lieu of the 5" clearance (shown on 12/GT-5202) between the hoops and the inside diameter of the hole. Changing the clearance from 5" to 7 1/4" would give Becho the 3" of clearance that they need between the spacers and casing ID.  Note that the approved rebar shop drawings show 5" clearance to the hoops as per 12/GT-5202. BBII will submit for your records only revised shop drawings showing the proposed 7 1/4" minimum clearance.						
						<b>ANSWER:</b>  Reference Sheet GT-5202, Specification Section 03 20 01, attached sketch, and approved Shop Drawings from Package TA2010-032001A05  Drawing 12/GT-5202 shows 5" clearance between the hoop OD and the inside diameter of a 7' +/- 2" shaft. Per discussions with Becho, at least 3" of clearance is needed between the rebar spacers and the ID of the casing to facilitate proper installation of the rebar cages inside the casing.  BBII would like to propose 7 1/4" minimum clearance in lieu of the 5" clearance (shown on 12/GT-5202) between the hoops and the inside diameter of the hole. Changing the clearance from 5" to 7 1/4" would give Becho the 3" of clearance that they need between the spacers and casing ID.  Note that the approved rebar shop drawings show 5" clearance to the hoops as per 12/GT-5202. BBII will submit for your records only revised shop drawings showing the proposed 7 1/4" minimum clearance.
<b>T-0185</b>	<b>Division 01 specifications issued for the TG08.1 package</b>	<b>Closed</b>	<b>CR</b>	<b>06/29/2011</b>	<b>07/09/2011</b>	<b>07/13/2011</b>
<b>From:</b> Webcor Construction LP      Tim Maxwell						
<b>REQUEST:</b>  Confirm if any of all of the Specification Sections 00 01 10, 00 01 15, 00 01 16, 00 03 50, 01 10 20 / APH, 01 10 30, 01 10 30 / APA, and 01 80 50 issued for the TG08.1 bid documents are to be incorporated into the overall project specifications. If so, the specifications should be issued to W/O by Field Order or Change Order.						
						<b>ANSWER:</b>  Confirm if any of all of the Specification Sections 00 01 10, 00 01 15, 00 01 16, 00 03 50, 01 10 20 / APH, 01 10 30, 01 10 30 / APA, and 01 80 50 issued for the TG08.1 bid documents are to be incorporated into the overall project specifications. If so, the specifications should be issued to W/O by Field Order or Change Order.
<b>T-0186</b>	<b>BSE - Hazardous Materials Removed From 564 &amp; 568 Howard Street</b>	<b>Closed</b>	<b>01</b>	<b>06/30/2011</b>	<b>07/10/2011</b>	<b>07/07/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Final Pre-Demolition Hazardous Materials						
						<b>ANSWER:</b>  Reference Final Pre-Demolition Hazardous Materials







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<b>T-0188</b>	<b>BSE - Timber Piles Minna Street</b>	<b>Closed</b>	<b>01</b>	<b>07/01/2011</b>	<b>07/11/2011</b>	<b>07/05/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b> Reference D-2211 and D-5101. During the pre-trenching operation on Minna Street between Gridlines 9-17, BBII discovered unknown timber piles. The timber piles are not shown on the BSE drawings. See attached BSE drawing D-2211, D-5101. The attached pictures indicate timber piles to be approx 2ft from the centerline of the CDSM wall. These piles meet the general conditions set out in article 3.05A.2. The piles encountered were not outlined in the bid documents. Please confirm the removal of the "unforeseen" timber piles, tracking and paid under a Force account contract change order similarly as done for Zone 4 pre-trench obstructions.		<b>ANSWER:</b> Reference D-2211 and D-5101. During the pre-trenching operation on Minna Street between Gridlines 9-17, BBII discovered unknown timber piles. The timber piles are not shown on the BSE drawings. See attached BSE drawing D-2211, D-5101. The attached pictures indicate timber piles to be approx 2ft from the centerline of the CDSM wall. These piles meet the general conditions set out in article 3.05A.2. The piles encountered were not outlined in the bid documents. Please confirm the removal of the "unforeseen" timber piles, tracking and paid under a Force account contract change order similarly as done for Zone 4 pre-trench obstructions.				
<b>T-0188.1</b>	<b>BSE - Timber Piles Minna Street</b>	<b>Closed</b>	<b>01</b>	<b>07/07/2011</b>	<b>07/17/2011</b>	<b>07/12/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b> Reference RFI T-0188, Drawing D-2211 and D-5101.  Further to the TJP A response RFI # 188, this response did not address the mentioned timber pile removal method. Please see the attached cross section showing timber pile location in relationship to the existing utilities and structures. Due to the pile location, in relation to the shoring box BBII proposes direct extraction as done on A line in Zone 3. Please confirm this removal method is acceptable for the entire length of Minna Street.		<b>ANSWER:</b> Reference RFI T-0188, Drawing D-2211 and D-5101.  Further to the TJP A response RFI # 188, this response did not address the mentioned timber pile removal method. Please see the attached cross section showing timber pile location in relationship to the existing utilities and structures. Due to the pile location, in relation to the shoring box BBII proposes direct extraction as done on A line in Zone 3. Please confirm this removal method is acceptable for the entire length of Minna Street.				
<b>T-0188.2</b>	<b>BSE - Timber Piles Minna Street</b>	<b>Closed</b>	<b>01</b>	<b>07/13/2011</b>	<b>07/23/2011</b>	<b>07/14/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b> Reference response to RFI#T-0188.1 and RFI#T-0146.4		<b>ANSWER:</b> Reference response to RFI#T-0188.1 and RFI#T-0146.4				



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<hr/>						
<p>As discussed at the TG03 BSE Design Team meeting on 7/13/2011, sand shall be used for back fillings instead of the low strength material described in RFI#T-0146.4. Also, TJPB representative shall observe the extraction and instruct the extraction method in the field, if necessary.</p> <p>Please confirm.</p>						
<p>As discussed at the TG03 BSE Design Team meeting on 7/13/2011, sand shall be used for back fillings instead of the low strength material described in RFI#T-0146.4. Also, TJPB representative shall observe the extraction and instruct the extraction method in the field, if necessary.</p> <p>Please confirm.</p>						
<b>T-0188.3</b>	<b>BSE - Timber Piles Minna Street</b>	<b>Closed</b>	<b>01</b>	<b>07/18/2011</b>	<b>07/28/2011</b>	<b>07/26/2011</b>
<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI#T-0188.2 and attached photos			Reference RFI#T-0188.2 and attached photos			
BBII has concerns for the integrity of the adjacent street and utilities, as a result of the pile extraction being performed on Minna Street in accordance with the response to RFI#T-0188.2. BBII has observed undermining and adjacent settlement during the extraction process (see attached photos).			BBII has concerns for the integrity of the adjacent street and utilities, as a result of the pile extraction being performed on Minna Street in accordance with the response to RFI#T-0188.2. BBII has observed undermining and adjacent settlement during the extraction process (see attached photos).			
Please advise an acceptable method of pile extraction that will allow this work to continue			Please advise an acceptable method of pile extraction that will allow this work to continue			



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<b>T-0189</b>	<b>BSE - CDSM Spoils - Initial Off Haul</b>	<b>Closed</b>	<b>01</b>	<b>07/01/2011</b>	<b>07/11/2011</b>	<b>07/05/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b>  Per our meeting on 6-23-11 with the TJPA, PMPC, T&R, TCCO and W/O, this RFI is to confirm the initial off haul of the CDSM spoils to be classified as Class 2 non-hazardous waste and will be paid under bid item #38 due to lack of soil testing data required by the landfill and risk of cross contamination. BBII is currently in talks with various local landfills and their Consultant with the advice of Treadwell Rollo for the acceptance of the spoil to be classified under "clean soil" (not Class 2). Please confirm.		<b>ANSWER:</b>  Per our meeting on 6-23-11 with the TJPA, PMPC, T&R, TCCO and W/O, this RFI is to confirm the initial off haul of the CDSM spoils to be classified as Class 2 non-hazardous waste and will be paid under bid item #38 due to lack of soil testing data required by the landfill and risk of cross contamination. BBII is currently in talks with various local landfills and their Consultant with the advice of Treadwell Rollo for the acceptance of the spoil to be classified under "clean soil" (not Class 2). Please confirm.				
<b>T-0190</b>	<b>BSE - Connector Wall Daily As Built Requirement</b>	<b>Closed</b>	<b>01</b>	<b>07/01/2011</b>	<b>07/11/2011</b>	<b>07/13/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b>  Reference Specification Section 31 56 13 1.4F.  To satisfy the Section 31 56 13 1.4F requirement, BBII will continue to submit the "DND Daily Construction Report" on a daily basis along with the attached as-built drawing within 24 hours of column installation.  Please confirm that this will satisfy the Section 1.4F requirement: "submit as-built drawings within 24 hours of column installation."		<b>ANSWER:</b>  Reference Specification Section 31 56 13 1.4F.  To satisfy the Section 31 56 13 1.4F requirement, BBII will continue to submit the "DND Daily Construction Report" on a daily basis along with the attached as-built drawing within 24 hours of column installation.  Please confirm that this will satisfy the Section 1.4F requirement: "submit as-built drawings within 24 hours of column installation."				
<b>T-0191</b>	<b>BSE - Connector Wall Final As Built Requirement</b>	<b>Closed</b>	<b>01</b>	<b>07/01/2011</b>	<b>07/11/2011</b>	<b>07/12/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b>  Reference Specification Section 31 56 13 3.3B.  To satisfy the Section 31 56 13 3.3B requirement, BBII proposes to submit as built drawings prepared by a California licensed surveyor at the approximate completion of each Zone.  Please confirm that this will satisfy the Section 3.3B requirement: "Following CDSM wall construction, the		<b>ANSWER:</b>  Reference Specification Section 31 56 13 3.3B.  To satisfy the Section 31 56 13 3.3B requirement, BBII proposes to submit as built drawings prepared by a California licensed surveyor at the approximate completion of each Zone.  Please confirm that this will satisfy the Section 3.3B requirement: "Following CDSM wall construction, the				



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<div>Contractor shall submit as-built drawings prepared by a California licensed surveyor indicating the location of the CDSM walls relative to the excavation alignment."</div> <div>Contractor shall submit as-built drawings prepared by a California licensed surveyor indicating the location of the CDSM walls relative to the excavation alignment."</div>						
T-0191.1	BSE - CDSM Connector Wall Final As Built Requirement	Closed	01	07/27/2011	08/06/2011	08/03/2011
<div>From: Webcor Construction LP</div> <div>Nhi Tran</div>						
REQUEST:			ANSWER:			
Reference RFI#T-0191 and Specification Section 31 56 13			Reference RFI#T-0191 and Specification Section 31 56 13			
BBII disagrees with TJPA's interpretation of the requirements of the Specifications in its Response to RFI T-0191.			BBII disagrees with TJPA's interpretation of the requirements of the Specifications in its Response to RFI T-0191.			
Article 1.4F, Section 31 56 13 of the Specifications state: Record Documents 1. Submit as-built drawings within 24 hours of column installation. 2. Note and submit immediately to the TJPA's Representative unusual conditions encountered, including amounts of cement grout overpours during construction.			Article 1.4F, Section 31 56 13 of the Specifications state: Record Documents 1. Submit as-built drawings within 24 hours of column installation. 2. Note and submit immediately to the TJPA's Representative unusual conditions encountered, including amounts of cement grout overpours during construction.			
Article 3.11D2, Section 31 56 13 of the Specifications state: The Daily Quality Control Report shall include as a minimum the results of the following QC parameter monitoring for each column: a. Rig number b. Type of mixing tool c. Date and time (start and finish) of column construction d. Column diameter e. Column top and bottom elevations f. Grout mix design designation g. Slurry specific gravity measurements (obtained from the Testing Agency) h. Description of obstructions, interruptions, or other difficulties during installation and how they were resolved i. Surveyed as-built of previous day's work in relation to grid			Article 3.11D2, Section 31 56 13 of the Specifications state: The Daily Quality Control Report shall include as a minimum the results of the following QC parameter monitoring for each column: a. Rig number b. Type of mixing tool c. Date and time (start and finish) of column construction d. Column diameter e. Column top and bottom elevations f. Grout mix design designation g. Slurry specific gravity measurements (obtained from the Testing Agency) h. Description of obstructions, interruptions, or other difficulties during installation and how they were resolved i. Surveyed as-built of previous day's work in relation			
Article 3.3B, Section 31 56 13 of the Specifications state: (emphasis added) Following CDSM wall construction, the Contractor shall			i. Surveyed as-built of previous day's work in relation			



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	<p>submit as-built drawings prepared by a California licensed surveyor indicating the location of the CDSM walls relative to the excavation alignment.</p> <p>Article 3.3B of the above provides the only requirement for a survey performed by California licensed surveyor. BBII's proposal in RFI T-0191 exceeded the requirements of Article 3.3B by proposing to submit as-built drawings prepared by a California licensed surveyor at the completion of the CDSM wall at each Zone, rather than at the completion of the entire CDSM scope as the Specifications require.</p> <p>Please confirm that submitting as-built drawings prepared by BBII/DND's project staff within 24 hours of installation and as-builts of each zone at the completion of the zone by a licensed surveyor is acceptable. BBII will perform additional survey by a licensed surveyor if necessary at areas of concern, to ensure conformance with the project requirements.</p>					
T-0192	<p><b>BSE - Unforeseen Tank on Gridline 35</b></p> <p><b>From:</b> Webcor Construction LP      Masashi Kojima</p> <p><b>REQUEST:</b></p> <p>BBII discovered an unforeseen tank structure during the pre-trenching operation along Gridline 35 between Gridline A-J that is not shown on the contract plans. The tank contains liquid substance; the odor from the excavation around the tank, it is assumed this is a fuel liquid. This tank needs to be removed to allow the continuation of the pre-trenching operation. Please advise as soon as possible.</p>	Closed	01	07/06/2011	07/16/2011	07/08/2011
						<p><b>ANSWER:</b></p> <p>BBII discovered an unforeseen tank structure during the pre-trenching operation along Gridline 35 between Gridline A-J that is not shown on the contract plans. The tank contains liquid substance; the odor from the excavation around the tank, it is assumed this is a fuel liquid. This tank needs to be removed to allow the continuation of the pre-trenching operation. Please advise as soon as possible.</p>
T-0192.1	<p><b>BSE - Unforeseen Tank on Gridline 35</b></p>	Closed	01	07/11/2011	07/21/2011	08/01/2011

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From: Webcor Construction LP	Nhi Tran					
<b>REQUEST:</b>						
Reference RFI#T-0192 and attached photo						
The unforeseen tank discovered during the pre-trench operation on Beale Street contains liquid. The liquid has spilled and is present in the surrounding soil, visible from the surface. The response to RFI#T-0192 does not address the soil surrounding the tank. BBII suspects this soil is contaminated with hydrocarbons in excess of the current approved Class 1 profile.						
Please advise on the classification, limits and disposal methods for the contaminated soil surrounding the tank.						
<b>T-0192.2</b>	<b>BSE - Unforeseen Tank on Gridline 35</b>	<b>Closed</b>	<b>01</b>	<b>08/02/2011</b>	<b>08/12/2011</b>	<b>08/15/2011</b>
From: Webcor Construction LP	Nhi Tran					
<b>REQUEST:</b>						
Reference RFI#T-0192.1						
The Analytical Report for the sample taken from the soil around the Underground Storage Tank (UST) has been sent to BBII. The soil classification that has been determined was not listed in the response, nor the Analytical Report. Please advise on the classification of the soil.						
<b>ANSWER:</b>						
Reference RFI#T-0192.1						
The Analytical Report for the sample taken from the soil around the Underground Storage Tank (UST) has been sent to BBII. The soil classification that has been determined was not listed in the response, nor the Analytical Report. Please advise on the classification of the soil.						









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<b>T-0195</b>	<b>BSE - Unknown Utility on Beale Street West Side</b>	<b>Closed</b>	<b>01</b>	<b>07/13/2011</b>	<b>07/23/2011</b>	<b>07/14/2011</b>
<b>From:</b> Webcor Construction LP Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference attached photos and drawing		Reference attached photos and drawing				
BBI discovered an 8" utility line during the installation of the wheel wash on the west side of Beale Street. The utility indicated in the attached pictures is not shown on the BSE contract drawings. The alignment (North to South direction) of this utility appears in conflict with the CDSM wall. On 7/12/2011, BBI was able to confirm that this utility is not active. This utility will need to be removed during the pre-trenching operation, to avoid conflict with the CDSM.		BBI discovered an 8" utility line during the installation of the wheel wash on the west side of Beale Street. The utility indicated in the attached pictures is not shown on the BSE contract drawings. The alignment (North to South direction) of this utility appears in conflict with the CDSM wall. On 7/12/2011, BBI was able to confirm that this utility is not active. This utility will need to be removed during the pre-trenching operation, to avoid conflict with the CDSM.				
Please advise on the method for removal of this utility line.		Please advise on the method for removal of this utility line.				
<b>T-0196</b>	<b>BSE - CDSM Shoring Wall Installation Sequence Zone 4 North of A-Line</b>	<b>Closed</b>	<b>01</b>	<b>07/20/2011</b>	<b>07/30/2011</b>	<b>07/26/2011</b>
<b>From:</b> Webcor Construction LP Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet GT-2201 and Specification Section 31 56 13		Reference Sheet GT-2201 and Specification Section 31 56 13				
See Note 1 on Sheet GT-2201. DND is concerned that if the row of buttress connector columns (A/26.5 - A/30) immediately adjacent to the shoring wall is installed prior to the shoring wall, the shoring wall will not meet verticality and tolerance specifications due to a difference in strength of the soil on one side and the CDSM on the other side. BBII believes that it will be possible to install the buttress connector columns after the shoring wall without hitting the shoring wall beams.		See Note 1 on Sheet GT-2201. DND is concerned that if the row of buttress connector columns (A/26.5 - A/30) immediately adjacent to the shoring wall is installed prior to the shoring wall, the shoring wall will not meet verticality and tolerance specifications due to a difference in strength of the soil on one side and the CDSM on the other side. BBII believes that it will be possible to install the buttress connector columns after the shoring wall without hitting the shoring wall beams.				
Is it acceptable to install the shoring wall prior to the immediately adjacent buttress connector columns?		Is it acceptable to install the shoring wall prior to the immediately adjacent buttress connector columns?				
<b>T-0197</b>	<b>BSE - Maximum Allowable Vibration</b>	<b>Closed</b>	<b>01</b>	<b>07/20/2011</b>	<b>07/30/2011</b>	<b>08/12/2011</b>
<b>From:</b> Webcor Construction LP Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification Sections 31 09 13 and 01 35 65		Reference Specification Sections 31 09 13 and 01 35 65				



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
<div><p>According to the Final FEIS/EIR, specified in the Specification 01 35 65 as the reference document, the Vibration Impact Criteria, which is the base criteria for the analysis, is shown in the table 5.21-8 (refer to BBI RFI for table).</p><p>The vibration impact criteria used in the Final FEIS/EIR contradicts the Maximum Allowable Movement for the vibration (PPV) specified in Specification 31 09 13. In this specification section, the maximum allowable movement for vibration and the action trigger level is described in Table 1 (refer to BBI RFI for table).</p><p>Please clarify where within the project site the vibration impact criteria for fragile structures are applicable (according to Specification 01 35 65), and where the maximum allowable movement for vibration of 1 inch per second is applicable (according to Specification 31 09 13).</p></div> <div><p>According to the Final FEIS/EIR, specified in the Specification 01 35 65 as the reference document, the Vibration Impact Criteria, which is the base criteria for the analysis, is shown in the table 5.21-8 (refer to BBI RFI for table).</p><p>The vibration impact criteria used in the Final FEIS/EIR contradicts the Maximum Allowable Movement for the vibration (PPV) specified in Specification 31 09 13. In this specification section, the maximum allowable movement for vibration and the action trigger level is described in Table 1 (refer to BBI RFI for table).</p><p>Please clarify where within the project site the vibration impact criteria for fragile structures are applicable (according to Specification 01 35 65), and where the maximum allowable movement for vibration of 1 inch per second is applicable (according to Specification 31 09 13).</p></div>						
<b>T-0197.1</b>	<b>BSE - Maximum Allowable Vibration</b>	<b>Closed</b>	<b>01</b>	<b>07/20/2011</b>	<b>07/30/2011</b>	<b>09/12/2011</b>
<b>From:</b> Turner Construction Company      Gary Krutsch						
<b>REQUEST:</b> Refer to RFI #T-0197			<b>ANSWER:</b> Refer to RFI #T-0197			



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<b>T-0197.2</b>	<b>BSE - Maximum Allowable Vibration - VOID</b>	<b>Closed</b>	<b>01</b>	<b>09/12/2011</b>	<b>09/22/2011</b>	<b>09/12/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI #T-0197, Specification Section 01 35 65 & 31 09 13, and attached map			Reference RFI #T-0197, Specification Section 01 35 65 & 31 09 13, and attached map			
BBII recognizes and agrees Table 5.12-8 is in error, and BBII will refer to FTA Table 12-3 as the correct table. However, BBII believes the TJPA's response provides information that is in conflict with the specifications as well as between the two separate responses provided. BBII requests the following clarifications and confirmations:			BBII recognizes and agrees Table 5.12-8 is in error, and BBII will refer to FTA Table 12-3 as the correct table. However, BBII believes the TJPA's response provides information that is in conflict with the specifications as well as between the two separate responses provided. BBII requests the following clarifications and confirmations:			
1. BBII has applied FTA Table 12-3 per [RFI #T-0197] (BBI RFI 147) to the attached map. The attached map indicates PPV values for continuous construction events, based on the surrounding buildings. Please review and verify this interpretation. Please note that this table, as also indicated in ARUP's response, applies to "continuous construction events".			1. BBII has applied FTA Table 12-3 per [RFI #T-0197] (BBI RFI 147) to the attached map. The attached map indicates PPV values for continuous construction events, based on the surrounding buildings. Please review and verify this interpretation. Please note that this table, as also indicated in ARUP's response, applies to "continuous construction events".			
2. As also stated in ARUP's response, BBII's interpretation of Section 31 09 13 is that the limits provided in this section apply to "transient construction events". Therefore, contrary to URS' response, the values provided in this section are applicable to transient construction events.			2. As also stated in ARUP's response, BBII's interpretation of Section 31 09 13 is that the limits provided in this section apply to "transient construction events". Therefore, contrary to URS' response, the values provided in this section are applicable to transient construction events.			
In addition, BBII will apply Table 1 in Specification Section 31 09 13 for transient construction events to all structures around the site. Table 1 indicates the Action Trigger Level for vibration (PPV) is 1/2 inch per second and Maximum Allowable Movement for vibration (PPV) is 1 inch per second.			In addition, BBII will apply Table 1 in Specification Section 31 09 13 for transient construction events to all structures around the site. Table 1 indicates the Action Trigger Level for vibration (PPV) is 1/2 inch per second and Maximum Allowable Movement for vibration (PPV) is 1 inch per second.			
Please confirm the vibration Peak Particle Velocity (PPV) values indicated above are acceptable for continuous and transient construction events.			Please confirm the vibration Peak Particle Velocity (PPV) values indicated above are acceptable for continuous and transient construction events.			



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<b>T-0198</b>	<b>BSE - Demolition Drawings in South-West Corner of Zone 1</b>	<b>Closed</b>	<b>01</b>	<b>07/28/2011</b>	<b>08/08/2011</b>	<b>08/25/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 02 41 01  BBII is requesting a copy of the added scope demolition drawings issued to EBI, for the South-West corner of Zone 1.					<b>ANSWER:</b>  Reference Specification Section 02 41 01  BBII is requesting a copy of the added scope demolition drawings issued to EBI, for the South-West corner of Zone 1.	
<b>T-0199</b>	<b>BSE - Pile Extraction Method For Grid Line 35.2</b>	<b>Closed</b>	<b>01</b>	<b>08/01/2011</b>	<b>08/11/2011</b>	<b>08/15/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference RFI#T-0188.2  After exposing piles at grid line 35.2 east of Beale Street, BBII intends on extracting these piles as per the method described in RFI#T-0188.2 (BBI 0139.2). This involves backfilling any voids with sand. Please confirm this method is acceptable.					<b>ANSWER:</b>  Reference RFI#T-0188.2  After exposing piles at grid line 35.2 east of Beale Street, BBII intends on extracting these piles as per the method described in RFI#T-0188.2 (BBI 0139.2). This involves backfilling any voids with sand. Please confirm this method is acceptable.	
<b>T-0200</b>	<b>BSE - Unforeseen Buried Obstructions - Zone 4 A Line (Gridline 27-34)</b>	<b>Closed</b>	<b>01</b>	<b>08/02/2011</b>	<b>08/12/2011</b>	<b>08/12/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 31 56 13, attached photos, and sketch  On Saturday, July 30th 2011, DND's CDSM drill rig encountered unidentified buried obstructions during the installation of the CDSM Shoring wall panel identified by the pile numbers 285-286 at Zone 4 "A" line between Grid "27 & 28". The newly found obstructions are deeper than the previously excavated timber piles.  DND construction initially attempted to drill through the buried obstructions without success. The drill rig was subsequently moved to further east to drill the next available panel. Between 10:30 am and 3:30 pm, DND made eight drilling attempts along the "A" line between pile numbers # 285 and # 300. All eight drill attempts failed due to the similar obstructions encountered within the 13' - 17' depth range below grade. Consequently, the CDSM shoring wall installation along grid line "A" at Zone					<b>ANSWER:</b>  Reference Specification Section 31 56 13, attached photos, and sketch  On Saturday, July 30th 2011, DND's CDSM drill rig encountered unidentified buried obstructions during the installation of the CDSM Shoring wall panel identified by the pile numbers 285-286 at Zone 4 "A" line between Grid "27 & 28". The newly found obstructions are deeper than the previously excavated timber piles.  DND construction initially attempted to drill through the buried obstructions without success. The drill rig was subsequently moved to further east to drill the next available panel. Between 10:30 am and 3:30 pm, DND made eight drilling attempts along the "A" line between pile numbers # 285 and # 300. All eight drill attempts failed due to the similar obstructions encountered within the 13' - 17' depth range below	



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T-0201	<b>BSE - Buttress Shift To South</b>  From: Webcor Construction LP      Nhi Tran  <b>REQUEST:</b>  Reference Sheet GT-2201, RFI#T-0151, and attached sketch  Per response to RFI T-0151, the Buttress can expand to the east as long as it doesn't shift to the south. Per discussions with Arup in last week's TG03 BSE Design Team Coordination Meeting (7/27/2011), it is acceptable for the Buttress to shift to the south per the attached sketch. Please confirm.	Closed	01	08/02/2011	08/12/2011	08/08/2011
						<b>ANSWER:</b>  Reference Sheet GT-2201, RFI#T-0151, and attached sketch  Per response to RFI T-0151, the Buttress can expand to the east as long as it doesn't shift to the south. Per discussions with Arup in last week's TG03 BSE Design Team Coordination Meeting (7/27/2011), it is acceptable for the Buttress to shift to the south per the attached sketch. Please confirm.



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<b>T-0202</b>	<b>BSE - Pile Extraction Method For Grid Line 33.5</b>	<b>Closed</b>	<b>01</b>	<b>08/04/2011</b>	<b>08/14/2011</b>	<b>08/12/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI#T-0146.2		Reference RFI#T-0146.2				
After exposing 5 piles at gridline 33.5 west of Beale Street, BBII intends on extracting these piles as per the accepted method described in RFI # T-0146 2,		After exposing 5 piles at gridline 33.5 west of Beale Street, BBII intends on extracting these piles as per the accepted method described in RFI # T-0146 2,				
"6. BBII will extract the wood piles with vibratory hammer, with the same stroking procedure without steel casing. BBII will perform dewatering enough to be able to connect the hammer to the pile. 7. BBII will backfill the void with low strength material Central Concrete Mix FOA100CX (RFI #T-0138.1). 8. BBII will backfill the piles.		"6. BBII will extract the wood piles with vibratory hammer, with the same stroking procedure without steel casing. BBII will perform dewatering enough to be able to connect the hammer to the pile. 7. BBII will backfill the void with low strength material Central Concrete Mix FOA100CX (RFI #T-0138.1). 8. BBII will backfill the piles.				
Answer: Per Brian Dykes, this work is authorized to proceed. Allowable work hours will be established after 199 Fremont pile extraction begins."		Answer: Per Brian Dykes, this work is authorized to proceed. Allowable work hours will be established after 199 Fremont pile extraction begins."				
This involves backfilling any voids with 1 sack sand. The attached drawing indicates the location and quantity of piles to be extracted. Please confirm that this method is acceptable. Also, please advise if any work hour restrictions apply.		This involves backfilling any voids with 1 sack sand. The attached drawing indicates the location and quantity of piles to be extracted. Please confirm that this method is acceptable. Also, please advise if any work hour restrictions apply.				
<b>T-0203</b>	<b>BSE - Clearance From Verticals For CSL Tubes</b>	<b>Closed</b>	<b>01</b>	<b>08/04/2011</b>	<b>08/14/2011</b>	<b>08/09/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet GT-5202, Specification Section 31 63 29, and attached photo		Reference Sheet GT-5202, Specification Section 31 63 29, and attached photo				
In the Phase 1 DFOW Buttress Rebar QC Meeting at Harris-Salinas Rebar's yard in Livermore on 8/01/2011, ARUP suggested moving the adjacent vertical bars away from the CSL tubes to allow for approximately 4" of concrete cover along the entire length of the shaft. Please confirm.		In the Phase 1 DFOW Buttress Rebar QC Meeting at Harris-Salinas Rebar's yard in Livermore on 8/01/2011, ARUP suggested moving the adjacent vertical bars away from the CSL tubes to allow for approximately 4" of concrete cover along the entire length of the shaft. Please confirm.				
<b>T-0204</b>	<b>BSE - Tie Backs Along 535 Mission Street - Vacant Lot</b>	<b>Closed</b>	<b>01</b>	<b>08/04/2011</b>	<b>08/14/2011</b>	<b>08/10/2011</b>



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**From:** Webcor Construction LP      Nhi Tran

**REQUEST:**

Reference GT-2102 & Detail 8 - GT-5103

BBII cannot locate the tie backs in the area of the vacant lot on Minna St. described in the Detail 8 on Contract Drawing GT-5103. The BBII crew went to a depth of 17 feet along the Pre-Trench and was unable to locate the tie backs. This was an additional foot more than the specified 15'-0" +/- 1'-0" depth. BBII believes the tie backs do not extend into the Pre-Trench limits and plans to move forward. Please advise if there is information to the contrary.

**ANSWER:**

Reference GT-2102 & Detail 8 - GT-5103

BBII cannot locate the tie backs in the area of the vacant lot on Minna St. described in the Detail 8 on Contract Drawing GT-5103. The BBII crew went to a depth of 17 feet along the Pre-Trench and was unable to locate the tie backs. This was an additional foot more than the specified 15'-0" +/- 1'-0" depth. BBII believes the tie backs do not extend into the Pre-Trench limits and plans to move forward. Please advise if there is information to the contrary.



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T-0205	BSE - Testing Weld On Hoops	Closed	01	08/05/2011	08/15/2011	08/09/2011
From: Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet GT-5202 and Specification Section 31 63 29		Reference Sheet GT-5202 and Specification Section 31 63 29				
Per SS03.20.01.3.3.B.4, "Inspect welding as required by Code for compliance with AWS D1.4."		Per SS03.20.01.3.3.B.4, "Inspect welding as required by Code for compliance with AWS D1.4."				
Per AWS D1.4.2, "Other welding processes may be used when approved by the Engineer, provided that any special qualification test requirements not covered here are met to ensure that welds are satisfactory for the intended application will be obtained."		Per AWS D1.4.2, "Other welding processes may be used when approved by the Engineer, provided that any special qualification test requirements not covered here are met to ensure that welds are satisfactory for the intended application will be obtained."				
As of this writing, the AWS does not cover Resistance Welding which is the type of welding that Harris-Salinas Rebar is using for the hoops. Caltrans has a written specification for Resistance Welding. Per Caltrans Standard Specifications Section 52, four (4) samples out of a lot of one hundred fifty (150) are taken to the lab for testing. If three (3) or more samples comply with the requirements, the whole lot is accepted. If only two (2) samples comply, one (1) additional test of four (4) samples out of the same lot is allowed. If any of the four (4) fail, the whole lot is rejected.		As of this writing, the AWS does not cover Resistance Welding which is the type of welding that Harris-Salinas Rebar is using for the hoops. Caltrans has a written specification for Resistance Welding. Per Caltrans Standard Specifications Section 52, four (4) samples out of a lot of one hundred fifty (150) are taken to the lab for testing. If three (3) or more samples comply with the requirements, the whole lot is accepted. If only two (2) samples comply, one (1) additional test of four (4) samples out of the same lot is allowed. If any of the four (4) fail, the whole lot is rejected.				
It was agreed upon in the DFOW meeting this week (8/1/2011) that it is acceptable to test the lots per Caltrans Standard Specifications. Please confirm.		It was agreed upon in the DFOW meeting this week (8/1/2011) that it is acceptable to test the lots per Caltrans Standard Specifications. Please confirm.				





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T-0206	BSE - Smart Hoops For CSL Tubes	Closed	01	08/05/2011	08/15/2011	08/09/2011
<div> <div> <b>From:</b> Webcor Construction LP Nhi Tran </div> <div> <b>REQUEST:</b> <p>Reference Sheet GT-5202, Specification Section 31 63 29, attached photo and sketch</p> <p>Drawing GT-5202 shows four (4ea) 4" CSL tubes equally spaced around the perimeter of the shaft tied to reinforced steel.</p> <p>Approved rebar shop drawing shows a square spider designed to serve two purposes:</p> <ol style="list-style-type: none"> <li>1. To allow the tremie pipe to pass through.</li> <li>2. To keep the CSL tubes equally spaced around the perimeter per Drawing GT-5202.</li> </ol> <p>In subsequent discussions the engineer suggested orientating the CSL tubes at a 23 degree angle from the longitudinal center of pile. In the Phase 1 DFOW Buttress Rebar QC Meeting on 8/1/2011 Harris-Salinas Rebar suggested using "smart hoops" to keep the CSL tubes in place and symmetrical around the perimeter at 23 degrees since the square spider could no longer be utilized for CSL tube alignment. This suggestion was well received by meeting attendees. Please confirm that the 23 degree CSL spacing is required. If so, please advise if the added "smart hoop" CSL alignment bars are acceptable?</p> </div> <div> <b>ANSWER:</b> <p>Reference Sheet GT-5202, Specification Section 31 63 29, attached photo and sketch</p> <p>Drawing GT-5202 shows four (4ea) 4" CSL tubes equally spaced around the perimeter of the shaft tied to reinforced steel.</p> <p>Approved rebar shop drawing shows a square spider designed to serve two purposes:</p> <ol style="list-style-type: none"> <li>1. To allow the tremie pipe to pass through.</li> <li>2. To keep the CSL tubes equally spaced around the perimeter per Drawing GT-5202.</li> </ol> <p>In subsequent discussions the engineer suggested orientating the CSL tubes at a 23 degree angle from the longitudinal center of pile. In the Phase 1 DFOW Buttress Rebar QC Meeting on 8/1/2011 Harris-Salinas Rebar suggested using "smart hoops" to keep the CSL tubes in place and symmetrical around the perimeter at 23 degrees since the square spider could no longer be utilized for CSL tube alignment. This suggestion was well received by meeting attendees. Please confirm that the 23 degree CSL spacing is required. If so, please advise if the added "smart hoop" CSL alignment bars are acceptable?</p> </div> </div>						



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<b>T-0207</b>	<b>BSE - Unknown Fiber Optic on Fremont Street</b>	<b>Closed</b>	<b>01</b>	<b>08/09/2011</b>	<b>08/19/2011</b>	<b>08/12/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Specification Section 02 41 01						Reference Specification Section 02 41 01
PG&E was scheduled to have all the utilities and structures confirmed dead on the East side of Fremont Street 8/07/2011 as part of the phase 1 PG&E relocation work. On 8/08/2011, W/O and PG&E conducted a USAR walk-through on Fremont Street to sign off and confirm that all PG&E utilities and structures have been confirmed de-energized and abandoned. PG&E discovered a live fiber optic cable between vaults 1675-1670. This fiber optic cable is in conflict with and causing delays to the CDSM wall and Buttress work commencement.						PG&E was scheduled to have all the utilities and structures confirmed dead on the East side of Fremont Street 8/07/2011 as part of the phase 1 PG&E relocation work. On 8/08/2011, W/O and PG&E conducted a USAR walk-through on Fremont Street to sign off and confirm that all PG&E utilities and structures have been confirmed de-energized and abandoned. PG&E discovered a live fiber optic cable between vaults 1675-1670. This fiber optic cable is in conflict with and causing delays to the CDSM wall and Buttress work commencement.
Please provide a date this fiber will be confirmed de-energized.						Please provide a date this fiber will be confirmed de-energized.
<b>T-0208</b>	<b>BSE - Long Term Seismic Loading</b>	<b>Closed</b>	<b>01</b>	<b>08/09/2011</b>	<b>08/19/2011</b>	<b>08/12/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Sheet GT-1110 and Specification Section 31 55 00						Reference Sheet GT-1110 and Specification Section 31 55 00
Note 7 on sheet GT-1110 states that "Seismic Increment Loads shall be considered to be long term loading." Per conversation at the 8/03/11 TG03 Design Team Coordination meeting, BBII understands that this note applies only to the lower level struts at the 301 Mission buttress case. Please confirm.						Note 7 on sheet GT-1110 states that "Seismic Increment Loads shall be considered to be long term loading." Per conversation at the 8/03/11 TG03 Design Team Coordination meeting, BBII understands that this note applies only to the lower level struts at the 301 Mission buttress case. Please confirm.
<b>T-0209</b>	<b>BSE - Abutment Bearing On CDSM Wall</b>	<b>Closed</b>	<b>01</b>	<b>08/11/2011</b>	<b>08/21/2011</b>	<b>08/19/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Specification 01 53 13						Reference Specification 01 53 13
During previous discussions with URS, ARUP, and DPW it has been expressed that the temporary bridge abutments should not bear on the CDSM shoring wall. The temporary bridges spec section 01 53 13, however, specifically states						During previous discussions with URS, ARUP, and DPW it has been expressed that the temporary bridge abutments should not bear on the CDSM shoring wall. The temporary bridges spec section 01 53 13,



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	that "abutments for bridges shall be supported by the CDSM shoring wall." Please advise if this statement still applies.					however, specifically states that "abutments for bridges shall be supported by the CDSM shoring wall." Please advise if this statement still applies.
T-0209.1	<b>BSE - Abutment Bearing On CDSM Wall</b>	Closed	01	09/02/2011	09/12/2011	09/09/2011
	<b>From:</b> Webcor Construction LP                      Nhi Tran					
	<b>REQUEST:</b>					<b>ANSWER:</b>
	Reference RFI#T-0209, Specification Section 01 53 13, and attached sheets					Reference RFI#T-0209, Specification Section 01 53 13, and attached sheets
	Included with this RFI are loading conditions for CDSM supported abutments. Please confirm that the shoring wall as currently designed can accommodate the loading.					Included with this RFI are loading conditions for CDSM supported abutments. Please confirm that the shoring wall as currently designed can accommodate the loading.
T-0209.2	<b>BSE - Abutment Bearing On CDSM Wall - Follow-Up</b>	Closed	01	09/13/2011	09/23/2011	09/16/2011
	<b>From:</b> Webcor Construction LP                      Nhi Tran					
	<b>REQUEST:</b>					<b>ANSWER:</b>
	Reference RFI #T-0209.2, Specification Section 01 53 13, and attached sheets					Reference RFI #T-0209.2, Specification Section 01 53 13, and attached sheets
	As requested by ARUP, please see the attached loads placed on each individual CDSM soldier beam beneath the proposed temporary bridge abutment. The loads include both the bracing self weight and the combined dead and live loads of the temporary bridges.					As requested by ARUP, please see the attached loads placed on each individual CDSM soldier beam beneath the proposed temporary bridge abutment. The loads include both the bracing self weight and the combined dead and live loads of the temporary bridges.
	BBII requests confirmation from the CDSM shoring wall EOR that these imposed loads do not exceed the assumed vertical loads used during original design analysis.					BBII requests confirmation from the CDSM shoring wall EOR that these imposed loads do not exceed the assumed vertical loads used during original design analysis.



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<b>T-0209.3</b>	<b>BSE - Abutment Bearing On CDSM Wall - Follow-Up</b>	<b>Closed</b>	<b>01</b>	<b>09/13/2011</b>	<b>09/23/2011</b>	<b>09/28/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI #T-0209.2, Specification Section 01 53 13, and attached sheets		Reference RFI #T-0209.2, Specification Section 01 53 13, and attached sheets				
As requested by ARUP, please see the attached loads placed on each individual CDSM soldier beam beneath the proposed temporary bridge abutment. The loads include both the bracing self weight and the combined dead and live loads of the temporary bridges.		As requested by ARUP, please see the attached loads placed on each individual CDSM soldier beam beneath the proposed temporary bridge abutment. The loads include both the bracing self weight and the combined dead and live loads of the temporary bridges.				
BBII requests confirmation from the CDSM shoring wall EOR that these imposed loads do not exceed the assumed vertical loads used during original design analysis.		BBII requests confirmation from the CDSM shoring wall EOR that these imposed loads do not exceed the assumed vertical loads used during original design analysis.				
<b>T-0209.4</b>	<b>BSE - Abutment Bearing On CDSM Wall - Follow-Up</b>	<b>Closed</b>	<b>CR</b>	<b>01/09/2012</b>	<b>01/19/2012</b>	<b>01/16/2012</b>
<b>From:</b> Webcor Construction LP                      Kirk Nielsen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference T-0209.3, Specification Section 01 53 13		Reference T-0209.3, Specification Section 01 53 13				
Contrary to RFI response T-0209.3, subsequent to the test pile loading CR T-025 during which there was little to no movement please confirm the revised direction to install the bridge abutment atop the CDSM wall at all streets pursuant to specification section 01 53 13.1.2.A.		Contrary to RFI response T-0209.3, subsequent to the test pile loading CR T-025 during which there was little to no movement please confirm the revised direction to install the bridge abutment atop the CDSM wall at all streets pursuant to specification section 01 53 13.1.2.A.				
<b>T-0210</b>	<b>BSE - Pile #498 Top Of Pile Elevation Issue</b>	<b>Closed</b>	<b>01</b>	<b>08/16/2011</b>	<b>08/26/2011</b>	<b>08/19/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference W/O NOTICE0010 (attached), Sheet GT-5101, and Specification Section 31 56 13		Reference W/O NOTICE0010 (attached), Sheet GT-5101, and Specification Section 31 56 13				
Please address the following information request from BBII's subcontractor DND:		Please address the following information request from BBII's subcontractor DND:				
"The specifications do not specify an allowable tolerance with regard to the vertical position of the beam tip relative to the plan drawings (GT-5101, Note 16). Please clarify		"The specifications do not specify an allowable tolerance with regard to the vertical position of the beam tip relative to the plan drawings (GT-5101, Note				



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T-0211	<p>the allowable tolerance for the beam tip elevation.</p> <p>For example, beam 498 (BBII ID #287) was set slightly high. The beam was measured prior to setting to be 97'-5 1/2" long. It was set to a top elevation of approximately +16'-11" which calculates a tip elevation of approximately - 80.63'. Specified tip elevation is -81-0" in this wall section (J/27-33.5)."</p> <p><b>Easement Information</b></p> <p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference Email "Fencing Plan at CDSM Wall Radius R2-1 and X1-1" from Turner on 8/10/2011 and attached documents</p> <p>W/O received the enclosed email "Fencing Plan at CDSM Wall Radius R2-1 and X1-1" and it's attachments from Turner on 8/10/2011, listed below:</p> <ul style="list-style-type: none"><li>- 3192 OR 151 easement.pdf</li><li>- Parcel F BNDY-ALTA_AB3721_15A_Rev 1.pdf</li><li>- CASFRA_2007 00369409.pdf</li><li>- Eminent Domain Fencing Plan .pdf</li></ul> <p>The information contained in the above documents differs from and/or does not exist in the current contract documents. Please provide a direction on what W/O and our Trade Subcontractors are to do with this easement information. In addition please indicate what requirements the TJPA expects Webcor Obayashi to now comply with.</p>	Closed	01	08/11/2011	08/21/2011	08/23/2011
						<p><b>ANSWER:</b></p> <p>Reference Email "Fencing Plan at CDSM Wall Radius R2-1 and X1-1" from Turner on 8/10/2011 and attached documents</p> <p>W/O received the enclosed email "Fencing Plan at CDSM Wall Radius R2-1 and X1-1" and it's attachments from Turner on 8/10/2011, listed below:</p> <ul style="list-style-type: none"><li>- 3192 OR 151 easement.pdf</li><li>- Parcel F BNDY-ALTA_AB3721_15A_Rev 1.pdf</li><li>- CASFRA_2007 00369409.pdf</li><li>- Eminent Domain Fencing Plan .pdf</li></ul> <p>The information contained in the above documents differs from and/or does not exist in the current contract documents. Please provide a direction on what W/O and our Trade Subcontractors are to do with this easement information. In addition please indicate what requirements the TJPA expects Webcor Obayashi to now comply with.</p>



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<b>T-0212</b>	<b>BSE - Unforeseen Timber Piles At Grid Line 33.5 J</b>	<b>Closed</b>	<b>01</b>	<b>08/15/2011</b>	<b>08/25/2011</b>	<b>08/16/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI#T-0148.1, Sheet D-2213, attached photos and sketch		Reference RFI#T-0148.1, Sheet D-2213, attached photos and sketch				
BBII exposed 24 piles at gridline 33.5 J close to Beale Street in Zone 4, as shown in the attached photographs. However, drawing D-2213 indicates five piles inside the CDSM wall limits. BBII intends to extract these piles using the method approved in RFI # T-0148 1. Please confirm that it is acceptable to continue tracking this unforeseen work as CR-T-010, as was practiced in this area previously.		BBII exposed 24 piles at gridline 33.5 J close to Beale Street in Zone 4, as shown in the attached photographs. However, drawing D-2213 indicates five piles inside the CDSM wall limits. BBII intends to extract these piles using the method approved in RFI # T-0148 1. Please confirm that it is acceptable to continue tracking this unforeseen work as CR-T-010, as was practiced in this area previously.				
<b>T-0213</b>	<b>BSE - Pile Extraction Method For Concrete Piles Between GL 5-10 at Natoma St</b>	<b>Closed</b>	<b>01</b>	<b>08/15/2011</b>	<b>08/25/2011</b>	<b>08/19/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI #T-0188.1, Specification Section 02 41 19, and attached sketch		Reference RFI #T-0188.1, Specification Section 02 41 19, and attached sketch				
BBII intends on extracting the existing concrete piles located between gridlines 5 and 10 on the south side, using the method approved in RFI#T-0188.1. This involves extracting piles using the vibratory hammer without a steel casing and backfilling the void with structural pre-trench sand. Attached is a drawing indicating the locations of the piles obstructing the CDSM wall. Please confirm that this is acceptable.		BBII intends on extracting the existing concrete piles located between gridlines 5 and 10 on the south side, using the method approved in RFI#T-0188.1. This involves extracting piles using the vibratory hammer without a steel casing and backfilling the void with structural pre-trench sand. Attached is a drawing indicating the locations of the piles obstructing the CDSM wall. Please confirm that this is acceptable.				
<b>T-0214</b>	<b>BSE - Instrumentation Protection Slab Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>08/16/2011</b>	<b>08/26/2011</b>	<b>08/23/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet GT-5102 and attached shop drawing and BBI sketches		Reference Sheet GT-5102 and attached shop drawing and BBI sketches				
BBII is proposing to pour a 2' thick instrument slab per the attached BBII drawings in lieu of the 1' thick concrete slab shown on Drawing GT-5102 to match the overall thickness of the Buttress Temporary Work Platform Concrete Cap. Approved 6000 psi Central Mix #960PC3Z3 (Submittal Item #TZ1010-033001A10) will be used for the instrument		BBII is proposing to pour a 2' thick instrument slab per the attached BBII drawings in lieu of the 1' thick concrete slab shown on Drawing GT-5102 to match the overall thickness of the Buttress Temporary Work Platform Concrete Cap. Approved 6000 psi Central Mix #960PC3Z3 (Submittal				



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	protection slab. Please confirm that this is acceptable.					Item #TZ1010-033001A10) will be used for the instrument protection slab. Please confirm that this is acceptable.
<b>T-0215</b>	<b>BSE - Diagonally Cut Unforeseen Piles at Grid Line 33.5 J</b>	<b>Closed</b>	<b>01</b>	<b>08/17/2011</b>	<b>08/27/2011</b>	<b>08/17/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Sheet GT-2103, Specification Section 02 41 19, and attached photos  BBII has extracted four (4) unforeseen piles at GL 33.5 J. Three (3) piles had an average length of 45' long. However, one (1) of these piles appeared to have 20' diagonally cut out of it at the bottom (see attached Photo 3). Another pile was only 23' long and appeared to have broken off underground (see attached Photo 1). BBII has concerns that lengths of pile may still remain in ground and will be an obstruction to the CDSM shoring wall installation. Please advise on how to proceed.						<b>ANSWER:</b>  Reference Sheet GT-2103, Specification Section 02 41 19, and attached photos  BBII has extracted four (4) unforeseen piles at GL 33.5 J. Three (3) piles had an average length of 45' long. However, one (1) of these piles appeared to have 20' diagonally cut out of it at the bottom (see attached Photo 3). Another pile was only 23' long and appeared to have broken off underground (see attached Photo 1). BBII has concerns that lengths of pile may still remain in ground and will be an obstruction to the CDSM shoring wall installation. Please advise on how to proceed.
<b>T-0215.1</b>	<b>BSE - Diagonally Cut Unforeseen Piles at GL 33.5 J</b>	<b>Closed</b>	<b>01</b>	<b>08/23/2011</b>	<b>09/02/2011</b>	<b>08/30/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference RFI #T-0215 and RFI #T-0177, Sheet GT-2103 and Specification Section 02 41 19  As the top of the broken pile is 33' below ground, further trenching to remove this pile is not practical. BBII proposes following the procedure approved by RFI T-0177 (BBII 0126) to extract this pile. In the future, BBII proposes this to be the standard procedure when a broken or lost pile presents an obstruction to the CDSM Shoring Wall installation and needs to be extracted.  Please confirm.						<b>ANSWER:</b>  Reference RFI #T-0215 and RFI #T-0177, Sheet GT-2103 and Specification Section 02 41 19  As the top of the broken pile is 33' below ground, further trenching to remove this pile is not practical. BBII proposes following the procedure approved by RFI T-0177 (BBII 0126) to extract this pile. In the future, BBII proposes this to be the standard procedure when a broken or lost pile presents an obstruction to the CDSM Shoring Wall installation and needs to be extracted.  Please confirm.





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<b>T-0216</b>	<b>BSE - Revised Buttress Shop Drawings For Record Only</b>	<b>Closed</b>	<b>01</b>	<b>08/18/2011</b>	<b>08/28/2011</b>	<b>08/19/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference attached revised CIDH Rebar Shop Drawings, RFI#T-0184, T-0203, T-0205 and T-0206		Reference attached revised CIDH Rebar Shop Drawings, RFI#T-0184, T-0203, T-0205 and T-0206				
Per discussions at the TG03 BSE Design Team meeting on 8/17/2011, it was agreed by Adamson and ARUP to confirm the finalized buttress rebar cage shop drawings via RFI because the shop drawings have already been approved in a previous submittal TG0300-320 / TA1020-032001A05.		Per discussions at the TG03 BSE Design Team meeting on 8/17/2011, it was agreed by Adamson and ARUP to confirm the finalized buttress rebar cage shop drawings via RFI because the shop drawings have already been approved in a previous submittal TG0300-320 / TA1020-032001A05.				
Attached are the revised shop drawings that incorporate all the changes that were agreed upon in the referenced RFIs. Please confirm that these shop drawings accurately reflects all changes made.		Attached are the revised shop drawings that incorporate all the changes that were agreed upon in the referenced RFIs. Please confirm that these shop drawings accurately reflects all changes made.				
<b>T-0217</b>	<b>BSE - Buttress Shift To The East</b>	<b>Closed</b>	<b>01</b>	<b>08/24/2011</b>	<b>09/03/2011</b>	<b>08/30/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI #T-0183.1, Sheet GT-2201, Specification Section 31 63 29, and attached sketch		Reference RFI #T-0183.1, Sheet GT-2201, Specification Section 31 63 29, and attached sketch				
The sketch that was included in the Engineer's response to RFI T-0183.1 shows Buttress rows S, T, U, V, and W, shifting 4" to the west. Per discussions with the Engineer in the 8/17/2011 TG03 BSE Design Team Meeting, all parties agreed that the 4" shift is not needed. Please confirm that the 4" shift is not necessary and that it is acceptable to install the Buttress shafts per the attached drawing.		The sketch that was included in the Engineer's response to RFI T-0183.1 shows Buttress rows S, T, U, V, and W, shifting 4" to the west. Per discussions with the Engineer in the 8/17/2011 TG03 BSE Design Team Meeting, all parties agreed that the 4" shift is not needed. Please confirm that the 4" shift is not necessary and that it is acceptable to install the Buttress shafts per the attached drawing.				
<b>T-0217.1</b>	<b>BSE - Maximum Allowable Spacing Between Buttress Shafts</b>	<b>Closed</b>	<b>01</b>	<b>03/23/2012</b>	<b>04/02/2012</b>	<b>03/23/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Becho requests for ARUP to provide the maximum allowed spacing between the tangent shafts East of P-Line and West of C-Line. Allowing such changes could possibly		Becho requests for ARUP to provide the maximum allowed spacing between the tangent shafts East of P-Line and West of C-Line. Allowing such changes could				





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	help mitigate Buttress Shaft schedule.				possibly help mitigate Buttress Shaft schedule.	
<b>T-0217.2</b>	<b>BSE - Increased Spacing Between Buttress Shafts east of P-line</b>	<b>Closed</b>	<b>01</b>	<b>04/12/2012</b>	<b>04/22/2012</b>	<b>04/19/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc. Ural Yal						
<b>REQUEST:</b>  Reference: BBII Spacing Sketch  Per the Engineer's response to RFI T-0217.1, "The tangential spacing of the Buttress shafts may be increased from 4" to 8" east of P-line and west of C-line." Please confirm that the revised Buttress footprint and coordinates shown on the attached sketch is acceptable.						
			<b>ANSWER:</b>  Reference: BBII Spacing Sketch  Per the Engineer's response to RFI T-0217.1, "The tangential spacing of the Buttress shafts may be increased from 4" to 8" east of P-line and west of C-line." Please confirm that the revised Buttress footprint and coordinates shown on the attached sketch is acceptable.			
<b>T-0218</b>	<b>BSE - Timber Lagging Underneath Instrument Protection Slab</b>	<b>Closed</b>	<b>01</b>	<b>08/29/2011</b>	<b>09/08/2011</b>	<b>08/31/2011</b>
<b>From:</b> Webcor Construction LP Nhi Tran						
<b>REQUEST:</b>  Reference RFI #T-0214, Sheet GT-5102, and Specification Section 31 56 13  Contract drawing GT-5102 indicates timber lagging being installed underneath the 2' section of the concrete instrumentation protection slab between grids 27 and 30. The original construction sequence foresaw the instrumentation protection slab being installed prior to the adjacent buttress work platform. BBII is planning on pouring the instrumentation slab and the adjacent buttress work platform monolithically on Wednesday 8/31/2011, which makes the timber lagging support redundant.  Please confirm that the timber lagging shown on contract drawing GT-5102 is not required to be installed. Your prompt response is highly appreciated.						
			<b>ANSWER:</b>  Reference RFI #T-0214, Sheet GT-5102, and Specification Section 31 56 13  Contract drawing GT-5102 indicates timber lagging being installed underneath the 2' section of the concrete instrumentation protection slab between grids 27 and 30. The original construction sequence foresaw the instrumentation protection slab being installed prior to the adjacent buttress work platform. BBII is planning on pouring the instrumentation slab and the adjacent buttress work platform monolithically on Wednesday 8/31/2011, which makes the timber lagging support redundant.  Please confirm that the timber lagging shown on contract drawing GT-5102 is not required to be installed. Your prompt response is highly appreciated.			
<b>T-0219</b>	<b>BSE - Abutments At Temporary Bridges</b>	<b>Closed</b>	<b>01</b>	<b>08/29/2011</b>	<b>09/08/2011</b>	<b>09/15/2011</b>
<b>From:</b> Webcor Construction LP Nhi Tran						





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<b>T-0220</b>	<b>BSE - Pile Extraction Method For The Remaining Timber Piles At GL 33.5 J</b>	<b>Closed</b>	<b>01</b>	<b>08/29/2011</b>	<b>09/08/2011</b>	<b>09/02/2011</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Nhi Tran</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI#T-0188.1, Specification Section 02 41 19, and attached sketch			Reference RFI#T-0188.1, Specification Section 02 41 19, and attached sketch			
BBII intends on extracting the remainder of the existing timber piles located at gridline 33.5J/Beale St., using the method approved in T-0188.1, as the piles are located a considerable distance from the 199 Fremont building. This involves extracting piles using the vibratory hammer without a steel casing and backfilling the void with structural pre trench sand. Attached is a drawing indicating the locations of the piles obstructing the CDSM wall. Please confirm that this is acceptable.			BBII intends on extracting the remainder of the existing timber piles located at gridline 33.5J/Beale St., using the method approved in T-0188.1, as the piles are located a considerable distance from the 199 Fremont building. This involves extracting piles using the vibratory hammer without a steel casing and backfilling the void with structural pre trench sand. Attached is a drawing indicating the locations of the piles obstructing the CDSM wall. Please confirm that this is acceptable.			





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T-0222	BSE - Temporary Bridge Pier Locations	Closed	01	08/29/2011	09/08/2011	09/01/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference Specification Section 01 53 13 and Submittal TG0300-201 Item TZ1030-015313A09 response comments (attached)		Reference Specification Section 01 53 13 and Submittal TG0300-201 Item TZ1030-015313A09 response comments (attached)				
Temporary bridge review comments (Submittal TZ1030-015313A09, package TG0300-201) call for the end piers on all three bridges to be relocated to avoid interrupting chamfer rebar (see attached markups). With the information provided to BBII in the plans and specifications, there was no indication that this reinforcement must be avoided, nor was there a required clear zone from the shoring wall to the first pier. Please advise if these piers absolutely need to move, or if their current locations can be accommodated. Increasing the span between the abutments and the first pier will have commercial impacts.		Temporary bridge review comments (Submittal TZ1030-015313A09, package TG0300-201) call for the end piers on all three bridges to be relocated to avoid interrupting chamfer rebar (see attached markups). With the information provided to BBII in the plans and specifications, there was no indication that this reinforcement must be avoided, nor was there a required clear zone from the shoring wall to the first pier. Please advise if these piers absolutely need to move, or if their current locations can be accommodated. Increasing the span between the abutments and the first pier will have commercial impacts.				
T-0223	BSE - Temporary Bridge Pedestrian Barrier Height	Closed	01	08/30/2011	09/09/2011	09/27/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference Specification Section 01 53 13 and Submittal TG0300-201 response comments (attached)		Reference Specification Section 01 53 13 and Submittal TG0300-201 response comments (attached)				
DPW review of the temporary bridges includes comment #42 that calls for the pedestrian barrier to be designed as a combination railing with a minimum height of 4'-6" while the specifications only call for a 3'-6" barrier. Please advise if the minimum height must be increased to 4'-6".		DPW review of the temporary bridges includes comment #42 that calls for the pedestrian barrier to be designed as a combination railing with a minimum height of 4'-6" while the specifications only call for a 3'-6" barrier. Please advise if the minimum height must be increased to 4'-6".				
T-0224	BSE - Temporary Bridge Deflection and Suspended Utilities	Closed	01	08/30/2011	09/09/2011	09/09/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
Reference Specification Section 01 53 13 and attached cut sheets		Reference Specification Section 01 53 13 and attached cut sheets				
Where utilities transition from direct bury to hanging under the temporary bridges, BBII believes there must be some		Where utilities transition from direct bury to hanging under the temporary bridges, BBII believes there must				



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T-0224.1	<b>BSE - Temporary Bridge Deflection and Suspended Utilities</b>  <b>From:</b> Webcor Construction LP      Nhi Tran  <b>REQUEST:</b>  allowance for deflection to prevent damage to the conduits during a seismic event. Attached are cut sheets for an expansion fitting and deflection fitting that BBII has seen used in combination at bridge transitions. Watertight flexible steel conduit may be an option as well. Please confirm that all Phase 2 utilities to be suspended below the temporary bridges will include some means of handling bridge deflection.  The response to RFI T-0224 requested additional information about bridge movements. This information was provided by email to AECOM on 9/9/11. Follow on questions were answered on 9/15/11. Please see the attached email string.  Please provide the make, model, location and quantity per conduit run for all the utilities supported by the bridge	Closed	01	09/23/2011	10/03/2011	09/27/2011
	<b>ANSWER:</b>  be some allowance for deflection to prevent damage to the conduits during a seismic event. Attached are cut sheets for an expansion fitting and deflection fitting that BBII has seen used in combination at bridge transitions. Watertight flexible steel conduit may be an option as well. Please confirm that all Phase 2 utilities to be suspended below the temporary bridges will include some means of handling bridge deflection.					
T-0224.2	<b>BSE - Temporary Bridge Deflection and Suspended Utilities</b>  <b>From:</b> Webcor Construction LP      Masashi Kojima  <b>REQUEST:</b>  Reference RFI #T-0224, Specification Section 01 53 30, and attached e-mails  The response to RFI T-224, 224.1, CR T-017 and Specification Section 01 53 30  The response to RFI T-0224.1 The 4" EX model is not readily available (8 week lead time), however the very similar AX is. Please see the attached data sheets for each model and advise if this revised material is acceptable.	Closed	01	10/05/2011	10/15/2011	10/12/2011
	<b>ANSWER:</b>  Reference RFI T-224, 224.1, CR T-017 and Specification Section 01 53 30  The response to RFI T-0224.1 The 4" EX model is not readily available (8 week lead time), however the very similar AX is. Please see the attached data sheets for each model and advise if this revised material is acceptable.					



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<b>T-0224.3</b>	<b>BSE - Temporary Bridge Deflection and Suspended Utilities</b>	<b>Closed</b>	<b>01</b>	<b>10/24/2011</b>	<b>11/03/2011</b>	<b>11/08/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference CR T-017R1 and Response to RFI#T-0224.2			Reference CR T-017R1 and Response to RFI#T-0224.2			
BBII have been advise that only 1 deflection fitting is required on per rigid conduit run, between gridline A and J.			BBII have been advise that only 1 deflection fitting is required on per rigid conduit run, between gridline A and J.			
The PG&E construction drawings attached, indicate (highlighted in yellow) 2 locations A and J line; request expansion fitting to be used.			The PG&E construction drawings attached, indicate (highlighted in yellow) 2 locations A and J line; request expansion fitting to be used.			
It is not clear from the drawings attached if PG&E require 1 deflection fitting per conduit run as previous stated in RFI # T-0224.2. Please confirm only 1 deflection fitting per conduit run between GL A-J is required by PG&E.			It is not clear from the drawings attached if PG&E require 1 deflection fitting per conduit run as previous stated in RFI # T-0224.2. Please confirm only 1 deflection fitting per conduit run between GL A-J is required by PG&E.			
Please provide a drawing showing, the deflection fitting configuration for individual conduit runs.			Please provide a drawing showing, the deflection fitting configuration for individual conduit runs.			
<b>T-0225</b>	<b>BSE - CDSM Alignment Conflict With Existing Utilities GL 1-J</b>	<b>Closed</b>	<b>01</b>	<b>08/31/2011</b>	<b>09/10/2011</b>	<b>08/31/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Sheet D-2231, Specification Section 31 56 13, and attached photo			Reference Sheet D-2231, Specification Section 31 56 13, and attached photo			
BBII laid out centerline of the CDSM on Gridline 1 and Gridline J. The centerline of the shoring indicates that the existing utilities PG&E/Water is in direct conflict with the location of the CDSM shoring wall. These utilities appear to be capped east of the centerline.			BBII laid out centerline of the CDSM on Gridline 1 and Gridline J. The centerline of the shoring indicates that the existing utilities PG&E/Water is in direct conflict with the location of the CDSM shoring wall. These utilities appear to be capped east of the centerline.			
Drawing D-2231 BSE contract states "Unless specified otherwise all utilities have been cut and capped outside the limits of the work by Transbay Transit Centre program relocation of utilities"... Please see photos attached.			Drawing D-2231 BSE contract states "Unless specified otherwise all utilities have been cut and capped outside the limits of the work by Transbay Transit Centre program relocation of utilities"... Please see photos attached.			
Please confirm the status on the relocation of these utilities.			Please confirm the status on the relocation of these utilities.			



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T-0225.1	BSE - CDSM Alignment Conflict With Existing Utilities GL 1-J	Closed	01	08/31/2011	09/10/2011	09/09/2011
<div> <div> <b>From:</b> Webcor Construction LP Nhi Tran </div> <div> <b>REQUEST:</b>  Reference RFI#T-0225   The response received for RFI #T-0225 does not provide the requested information.  -----  Question from RFI#T-0225   Reference Sheet D-2231, Specification Section 31 56 13, and attached photo   BBII laid out centerline of the CDSM on Gridline 1 and Gridline J. The centerline of the shoring indicates that the existing utilities PG&amp;E/Water is in direct conflict with the location of the CDSM shoring wall. These utilities appear to be capped east of the centerline.   Drawing D-2231 BSE contract states "Unless specified otherwise all utilities have been cut and capped outside the limits of the work by Transbay Transit Centre program relocation of utilities"... Please see photos attached.   Please confirm the status on the relocation of these utilities. </div> <div> <b>ANSWER:</b>  Reference RFI#T-0225   The response received for RFI #T-0225 does not provide the requested information.  -----  Question from RFI#T-0225   Reference Sheet D-2231, Specification Section 31 56 13, and attached photo   BBII laid out centerline of the CDSM on Gridline 1 and Gridline J. The centerline of the shoring indicates that the existing utilities PG&amp;E/Water is in direct conflict with the location of the CDSM shoring wall. These utilities appear to be capped east of the centerline.   Drawing D-2231 BSE contract states "Unless specified otherwise all utilities have been cut and capped outside the limits of the work by Transbay Transit Centre program relocation of utilities"... Please see photos attached.   Please confirm the status on the relocation of these utilities. </div> </div>						





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<b>T-0225.2</b>	<b>BSE - CDSM Alignment Conflict GL 1-J - PG&amp;E Vault Utility Conflict on Natoma</b>	<b>Closed</b>	<b>01</b>	<b>09/12/2011</b>	<b>09/22/2011</b>	<b>09/14/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI #T-0017, #T-0225.1, Sheet U-1110, and Specification Section 31 56 13			Reference RFI #T-0017, #T-0225.1, Sheet U-1110, and Specification Section 31 56 13			
Please refer to RFI No. T-0017, which revised the southwest corner of the CDSM shoring wall alignment. Your attention is also directed to the utility drawing U-1110, which depicts the utilities to be abandoned and the ones to be protected in place with respect to the old CDSM wall alignment. According to U-1110, the PG&E vault on Natoma Street shall be protected in place. However, based on the field layout, the PG&E vault on Natoma St. is in conflict with the southwest corner of the CDSM wall alignment, which was revised per RFI No. T-0017.			Please refer to RFI No. T-0017, which revised the southwest corner of the CDSM shoring wall alignment. Your attention is also directed to the utility drawing U-1110, which depicts the utilities to be abandoned and the ones to be protected in place with respect to the old CDSM wall alignment. According to U-1110, the PG&E vault on Natoma Street shall be protected in place. However, based on the field layout, the PG&E vault on Natoma St. is in conflict with the southwest corner of the CDSM wall alignment, which was revised per RFI No. T-0017.			
Based on BBII's field measurements, the clearance between the PG&E vault on Natoma St. and the centerline of the CDSM wall is 29", which is less than the 36" typical distance required by the contract plans as the minimum clearance between the demarcation lines and the CDSM wall alignment.			Based on BBII's field measurements, the clearance between the PG&E vault on Natoma St. and the centerline of the CDSM wall is 29", which is less than the 36" typical distance required by the contract plans as the minimum clearance between the demarcation lines and the CDSM wall alignment.			
BBII requests the PG&E vault on Natoma St. to be relocated to a safe distance outside the work limits of the revised CDSM wall alignment.			BBII requests the PG&E vault on Natoma St. to be relocated to a safe distance outside the work limits of the revised CDSM wall alignment.			



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T-0225.3	BSE - CDSM Alignment Conflict GL 1-J - PG&E Vault Utility Conflict on Natoma	Closed	01	10/03/2011	10/13/2011	10/20/2011
From: Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI #T-0225.2, Sheet D-2231 and ASI-015, Specification Section 31 56 13, and attached photos and sketch		Reference RFI #T-0225.2, Sheet D-2231 and ASI-015, Specification Section 31 56 13, and attached photos and sketch				
BBII in discussions with DND will be able to work adjacent to PG&E vault #1348, referenced in RFI #T-0225.2.		BBII in discussions with DND will be able to work adjacent to PG&E vault #1348, referenced in RFI #T-0225.2.				
BBII is currently considering removing the concrete over pour on the vault, de-energizing the power in the vault and installing CDSM Shoring Wall without relocating the vault.		BBII is currently considering removing the concrete over pour on the vault, de-energizing the power in the vault and installing CDSM Shoring Wall without relocating the vault.				
Please confirm it is acceptable to remove any concrete over pour within 20" from the centerline of CDSM wall.		Please confirm it is acceptable to remove any concrete over pour within 20" from the centerline of CDSM wall.				
Also, please confirm it is acceptable to install CDSM Wall at the location close to the PG&E vault #1348 without potential damages.		Please confirm it is acceptable to remove any concrete over pour within 20" from the centerline of CDSM wall.				
Please refer to the attached photos		Also, please confirm it is acceptable to install CDSM Wall at the location close to the PG&E vault #1348 without potential damages.				
		Please refer to the attached photos				
T-0226	BSE - Revised Instrument Protection Slab	Closed	01	09/02/2011	09/12/2011	09/06/2011
From: Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference RFI #T-0214 and attached sketch		Reference RFI #T-0214 and attached sketch				
Per discussion with the engineer, it is acceptable to install the Instrument Protection Slab per the attached sketch and the following revisions to RFI T-0214:		Per discussion with the engineer, it is acceptable to install the Instrument Protection Slab per the attached sketch and the following revisions to RFI T-0214:				
1. W-beams cut so that the top mat will be resting on them.		1. W-beams cut so that the top mat will be resting on them.				
2. #6 rebar thru the W-beam, tie-wired to the top mat in lieu of Nelson Studs.		2. #6 rebar thru the W-beam, tie-wired to the top mat in lieu of Nelson Studs.				
Please confirm.		Please confirm.				
T-0227	BSE - Buttress Anti-Washout Admixture	Closed	01	09/02/2011	09/12/2011	09/08/2011



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T-0228	From: Webcor Construction LP Nhi Tran	Closed	01	09/02/2011	09/12/2011	09/27/2011
	REQUEST:  Reference Specification Section 03 30 01 and attached Rheomac product data  Per the recommendations from both Becho and Central Concrete, BBII would like to propose the use of an Anti-Washout Admixture, Rheomac UW 540 in all submitted and approved Buttress Primary and Secondary Shaft Concrete. Please review and confirm that this is acceptable.					
T-0228	From: Webcor Construction LP Nhi Tran	Closed	01	09/02/2011	09/12/2011	09/27/2011
	REQUEST:  Reference Specification Section 01 53 13 and attached sketches  During a temporary bridge traffic coordination meeting on 8/29/11, SFMTA suggested the use of a 6" elevated sidewalk curb in lieu of the crash rated pedestrian barrier. The crash rated barrier would be relocated to the outside edge of the temporary bridge.  BBII believes this layout has numerous advantages and resolves some concerns as well: - SFMTA brought up the obvious concern of damage to side mirrors with tall barriers directly adjacent to the traveled lanes. To compensate for this, drivers will shy away from barriers in already tight lanes. Moving the barrier alleviates this problem on one side of the road. - A barrier between the sidewalk and traveled lanes has a blunt ends that pose a hazard (see sketch). Relocating the barrier eliminates this hazard. - The area formerly occupied by the pedestrian barrier (approx 14" in width) can be used as extra traveled width for vehicles (distributed per SFMTA's discretion) - An elevated sidewalk curb will make trestle crossings feel like a typical street crossing, especially for the visually impaired. As such, pedestrians will be more likely to treat the trestle intersection as a true signalized intersection.  SFMTA has indicated that the elevated sidewalk is preferred over a pedestrian barrier. Attached are several					
T-0228	From: Webcor Construction LP Nhi Tran	Closed	01	09/02/2011	09/12/2011	09/27/2011
	ANSWER:  Reference Specification Section 03 30 01 and attached Rheomac product data  Per the recommendations from both Becho and Central Concrete, BBII would like to propose the use of an Anti-Washout Admixture, Rheomac UW 540 in all submitted and approved Buttress Primary and Secondary Shaft Concrete. Please review and confirm that this is acceptable.					
T-0228	From: Webcor Construction LP Nhi Tran	Closed	01	09/02/2011	09/12/2011	09/27/2011
	ANSWER:  Reference Specification Section 01 53 13 and attached sketches  During a temporary bridge traffic coordination meeting on 8/29/11, SFMTA suggested the use of a 6" elevated sidewalk curb in lieu of the crash rated pedestrian barrier. The crash rated barrier would be relocated to the outside edge of the temporary bridge.  BBII believes this layout has numerous advantages and resolves some concerns as well: - SFMTA brought up the obvious concern of damage to side mirrors with tall barriers directly adjacent to the traveled lanes. To compensate for this, drivers will shy away from barriers in already tight lanes. Moving the barrier alleviates this problem on one side of the road. - A barrier between the sidewalk and traveled lanes has a blunt ends that pose a hazard (see sketch). Relocating the barrier eliminates this hazard. - The area formerly occupied by the pedestrian barrier (approx 14" in width) can be used as extra traveled width for vehicles (distributed per SFMTA's discretion) - An elevated sidewalk curb will make trestle crossings feel like a typical street crossing, especially for the visually impaired. As such, pedestrians will be more likely to treat the trestle intersection as a true signalized intersection.  SFMTA has indicated that the elevated sidewalk is preferred over a pedestrian barrier. Attached are several					



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	sketches of the proposed layout - please confirm this is acceptable.				several sketches of the proposed layout - please confirm this is acceptable.	
<b>T-0229</b>	<b>BSE - Concrete Time of Discharge Requirement</b>	<b>Closed</b>	<b>01</b>	<b>09/06/2011</b>	<b>09/16/2011</b>	<b>09/08/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Specification 03 30 01						Reference Specification 03 30 01
Per SS 03 30 00, 3.3.D, "Discharge of concrete shall be completed within 1½ hours or before the drum has revolved 300 revolutions, whichever comes first, after the introduction of the mixing water to the cement and aggregates or the introduction of the cement to the aggregates."						Per SS 03 30 00, 3.3.D, "Discharge of concrete shall be completed within 1½ hours or before the drum has revolved 300 revolutions, whichever comes first, after the introduction of the mixing water to the cement and aggregates or the introduction of the cement to the aggregates."
Per ACI 301 (Section 4.1.2.9), "Time of discharge - When it is desired to exceed the maximum time for discharge of concrete permitted by ASTM C 94C/ 94M, submit a request along with a description of the precautions to be taken."						Per ACI 301 (Section 4.1.2.9), "Time of discharge - When it is desired to exceed the maximum time for discharge of concrete permitted by ASTM C 94C/ 94M, submit a request along with a description of the precautions to be taken."
BBII is planning for discharging concrete with the following precautions: As concrete hydration can be controlled for a maximum of 10 hours, BBII suggests discharge of concrete shall not be restricted to 1½ hours. In order to sustain the requirements of Becho, BBII purposes to replace the 1½ hour time restriction to 3 hours with an 80° F maximum temperature requirement.						BBII is planning for discharging concrete with the following precautions: As concrete hydration can be controlled for a maximum of 10 hours, BBII suggests discharge of concrete shall not be restricted to 1½ hours. In order to sustain the requirements of Becho, BBII purposes to replace the 1½ hour time restriction to 3 hours with an 80° F maximum temperature requirement.
Please confirm that this discharging plan is acceptable for Buttress Concrete per ACI 301.						Please confirm that this discharging plan is acceptable for Buttress Concrete per ACI 301.



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<b>T-0230</b>	<b>BSE - Concrete Sampling Location</b>	<b>Closed</b>	<b>01</b>	<b>09/12/2011</b>	<b>09/22/2011</b>	<b>09/16/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Specification Section 03 30 01						Reference Specification Section 03 30 01
Per the Pre-Construction Buttress Shoring Phase 1 DFOW Meeting on 8/30/2011, BBII proposes to conduct concrete sampling of Central Concrete Trucks in Lot P in lieu of Zone 4 due to site congestion and safety concerns. In order to sustain the requirements of Becho and to provide safe disposal of concrete for sampling, BBII purposes Lot P for all concrete sample inspections.						Per the Pre-Construction Buttress Shoring Phase 1 DFOW Meeting on 8/30/2011, BBII proposes to conduct concrete sampling of Central Concrete Trucks in Lot P in lieu of Zone 4 due to site congestion and safety concerns. In order to sustain the requirements of Becho and to provide safe disposal of concrete for sampling, BBII purposes Lot P for all concrete sample inspections.
Please confirm that this is acceptable.						Please confirm that this is acceptable.
<b>T-0231</b>	<b>BSE - 24-Hour Inspection of Buttress Shoring Shaft</b>	<b>Closed</b>	<b>01</b>	<b>09/12/2011</b>	<b>09/22/2011</b>	<b>09/12/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Specification Section 03 30 01						Reference Specification Section 03 30 01
Per the Pre-Construction Buttress Shoring Phase 1 DFOW Meeting on 8/30/2011, Becho requests that a TJPA representative be available to observe the 24 hour Buttress Shoring drilling operation and to perform any/all specified inspections. This includes: verticality of shaft, shaft cleanliness, verification of bed rock, concrete and rebar. In addition, Becho requests that a TJPA representative be available 24 hours of the day to provide Becho/BBII with full support and contact information of all available representatives.						Per the Pre-Construction Buttress Shoring Phase 1 DFOW Meeting on 8/30/2011, Becho requests that a TJPA representative be available to observe the 24 hour Buttress Shoring drilling operation and to perform any/all specified inspections. This includes: verticality of shaft, shaft cleanliness, verification of bed rock, concrete and rebar. In addition, Becho requests that a TJPA representative be available 24 hours of the day to provide Becho/BBII with full support and contact information of all available representatives.
Please confirm that this is acceptable.						Please confirm that this is acceptable.
<b>T-0232</b>	<b>BSE - Buttress Red Color Concrete</b>	<b>Closed</b>	<b>01</b>	<b>09/15/2011</b>	<b>09/25/2011</b>	<b>09/16/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Specification Section 03 30 01 and Sheet GT-2201						Reference Specification Section 03 30 01 and Sheet GT-2201
Per discussion with the Engineer, it is acceptable to place red color concrete in Secondary Buttress Shafts C3 and						Per discussion with the Engineer, it is acceptable to place red color concrete in Secondary Buttress Shafts



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	C5 in lieu of Primary Buttress Shafts C2, C4, and C6.  Please confirm this is acceptable.				C3 and C5 in lieu of Primary Buttress Shafts C2, C4, and C6.  Please confirm this is acceptable.	
<b>T-0233</b>	<b>BSE - Internal Bracing Design Coordination with Structural Design</b>	<b>Closed</b>	<b>01</b>	<b>09/20/2011</b>	<b>09/30/2011</b>	<b>09/23/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Masashi Kojima						
<b>REQUEST:</b>  Reference Specification Section 31 55 00  The BSE submittal TG0300-542.1 Internal Bracing Design was approved by TJPA and the fabrication will start as soon as permission is issued by the City. Please confirm the design was acceptable to permanent structural designer (Thornton Tomasetti) and incorporated into their design for future trade packages.			<b>ANSWER:</b>  Reference Specification Section 31 55 00  The BSE submittal TG0300-542.1 Internal Bracing Design was approved by TJPA and the fabrication will start as soon as permission is issued by the City. Please confirm the design was acceptable to permanent structural designer (Thornton Tomasetti) and incorporated into their design for future trade packages.			





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<b>T-0233.2</b>	<b>BSE - Internal Bracing Design Coordination with Structural Design</b>	<b>Closed</b>	<b>01</b>	<b>10/05/2011</b>	<b>10/15/2011</b>	<b>10/10/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI #T-0233, T-0233.1, Submittal TG0300-542 and TJPB Transmittal No.140-02321.			Reference RFI #T-0233, T-0233.1, Submittal TG0300-542 and TJPB Transmittal No.140-02321.			
W/O is in receipt of TJPB Submittal Package #TG0300-542 for the internal bracing from which W/O is proceeding per specification section 01 13 00. W/O is aware the design team did not review and comment on Transmittal #140-02321 (DBI's comments) to Submittal Package #TG0300-542. Please confirm no design team changes or comments will be made to Submittal Package #TG0300-542 rather future trade packages.			W/O is in receipt of TJPB Submittal Package #TG0300-542 for the internal bracing from which W/O is proceeding per specification section 01 13 00. W/O is aware the design team did not review and comment on Transmittal #140-02321 (DBI's comments) to Submittal Package #TG0300-542. Please confirm no design team changes or comments will be made to Submittal Package #TG0300-542 rather future trade packages.			
----- RFI #T-0233.1 Response ----- TT is currently reviewing the Internal Bracing Design Documents, which was received by TT on 09/29/2011. TT's comments to this document will be marked up on the Internal Bracing Design Document.			----- RFI #T-0233.1 Response ----- TT is currently reviewing the Internal Bracing Design Documents, which was received by TT on 09/29/2011. TT's comments to this document will be marked up on the Internal Bracing Design Document.			
----- RFI #T-0233.1 Question ----- The SFDBI-approved Internal Bracing drawings and related calculations was sent to W/O on 9/22/2011 as TJPB Transmittal No. 140-02321 - Approved Internal Bracing for Shoring Wall Permit Drawings, and available in Constructware.			----- RFI #T-0233.1 Question ----- The SFDBI-approved Internal Bracing drawings and related calculations was sent to W/O on 9/22/2011 as TJPB Transmittal No. 140-02321 - Approved Internal Bracing for Shoring Wall Permit Drawings, and available in Constructware.			
----- RFI #T-0233 Response ----- Thornton Tomasetti's response is pending receipt and review of revised internal bracing submittal.			----- RFI #T-0233 Response ----- Thornton Tomasetti's response is pending receipt and review of revised internal bracing submittal.			
----- RFI #T-0233 Question ----- The BSE submittal TG0300-542.1 Internal Bracing Design was approved by TJPB and the fabrication will start as soon as permission is issued by the City. Please confirm the design was acceptable to permanent structural designer (Thornton Tomasetti) and incorporated into their design for future trade packages.			----- RFI #T-0233 Question ----- The BSE submittal TG0300-542.1 Internal Bracing Design was approved by TJPB and the fabrication will start as soon as permission is issued by the City. Please confirm the design was acceptable to permanent structural designer (Thornton Tomasetti) and incorporated into their design for future trade packages.			





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<b>T-0233.3</b>	<b>BSE - Internal Bracing Design Coordination with Structural Design</b>	<b>Closed</b>	<b>CR</b>	<b>10/10/2011</b>	<b>10/20/2011</b>	<b>10/10/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI #T-0233, T-0233.1, T-0233.2, Submittal TG0300-542 and TJPB Transmittal No.140-02321.			Reference RFI #T-0233, T-0233.1, T-0233.2, Submittal TG0300-542 and TJPB Transmittal No.140-02321.			
This RFI shall not be closed until the information / confirmation received from the Design team.			This RFI shall not be closed until the information / confirmation received from the Design team.			
----- RFI #T-0233.2 Response ----- Thornton Tomasetti will be issuing comments to Transmittal #140-02321.			----- RFI #T-0233.2 Response ----- Thornton Tomasetti will be issuing comments to Transmittal #140-02321.			
----- RFI #T-0233.2 Question ----- W/O is in receipt of TJPB Submittal Package #TG0300-542 for the internal bracing from which W/O is proceeding per specification section 01 13 00. W/O is aware the design team did not review and comment on Transmittal #140-02321 (DBI's comments) to Submittal Package #TG0300-542. Please confirm no design team changes or comments will be made to Submittal Package #TG0300-542 rather future trade packages.			----- RFI #T-0233.2 Question ----- W/O is in receipt of TJPB Submittal Package #TG0300-542 for the internal bracing from which W/O is proceeding per specification section 01 13 00. W/O is aware the design team did not review and comment on Transmittal #140-02321 (DBI's comments) to Submittal Package #TG0300-542. Please confirm no design team changes or comments will be made to Submittal Package #TG0300-542 rather future trade packages.			
----- RFI #T-0233.1 Response ----- TT is currently reviewing the Internal Bracing Design Documents, which was received by TT on 09/29/2011. TT's comments to this document will be marked up on the Internal Bracing Design Document.			----- RFI #T-0233.1 Response ----- TT is currently reviewing the Internal Bracing Design Documents, which was received by TT on 09/29/2011. TT's comments to this document will be marked up on the Internal Bracing Design Document.			
----- RFI #T-0233.1 Question ----- The SFDBI-approved Internal Bracing drawings and related calculations was sent to W/O on 9/22/2011 as TJPB Transmittal No. 140-02321 - Approved Internal Bracing for Shoring Wall Permit Drawings, and available in Constructware.			----- RFI #T-0233.1 Question ----- The SFDBI-approved Internal Bracing drawings and related calculations was sent to W/O on 9/22/2011 as TJPB Transmittal No. 140-02321 - Approved Internal Bracing for Shoring Wall Permit Drawings, and available in Constructware.			
----- RFI #T-0233 Response ----- Thornton Tomasetti's response is pending receipt and review of revised internal bracing submittal.			----- RFI #T-0233 Response ----- Thornton Tomasetti's response is pending receipt and review of revised internal bracing submittal.			
----- RFI #T-0233 Question ----- The BSE submittal TG0300-542.1 Internal Bracing Design was approved by TJPB and the fabrication will start as soon as permission is issued by the City. Please confirm the design was acceptable to permanent structural designer (Thornton Tomasetti) and incorporated into their design for future trade packages.			----- RFI #T-0233 Question ----- The BSE submittal TG0300-542.1 Internal Bracing Design was approved by TJPB and the fabrication will start as soon as permission is issued by the City. Please confirm the design was acceptable to permanent structural designer (Thornton Tomasetti) and incorporated into their design for future trade			



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						packages.
T-0233.4	BSE - Internal Bracing Design Coordination with Structural Design	Closed	01	10/10/2011	10/20/2011	10/11/2011
From: Webcor Construction LP Masashi Kojima						
REQUEST:						ANSWER:
Reference RFI #T-0233, T-0233.1, T-0233.2, Submittal TG0300-542 and TJPA Transmittal No.140-02321.						Reference RFI #T-0233, T-0233.1, T-0233.2, Submittal TG0300-542 and TJPA Transmittal No.140-02321.
When will the Design team provide the information / confirmation for RFI #T-0233?						When will the Design team provide the information / confirmation for RFI #T-0233?
----- RFI #T-0233.3 Response ----- This RFI contains a statement, not a question and is inappropriate for the RFI process. RFI T-0233.2 will remain closed but unresolved until the requested information is provided.						----- RFI #T-0233.3 Response ----- This RFI contains a statement, not a question and is inappropriate for the RFI process. RFI T-0233.2 will remain closed but unresolved until the requested information is provided.
----- RFI #T-0233.3 Question ----- This RFI shall not be closed until the information / confirmation received from the Design team.						----- RFI #T-0233.3 Question ----- This RFI shall not be closed until the information / confirmation received from the Design team.
----- RFI #T-0233.2 Response ----- Thornton Tomasetti will be issuing comments to Transmittal #140-02321.						----- RFI #T-0233.2 Response ----- Thornton Tomasetti will be issuing comments to Transmittal #140-02321.
----- RFI #T-0233.2 Question ----- W/O is in receipt of TJPA Submittal Package #TG0300-542 for the internal bracing from which W/O is proceeding per specification section 01 13 00. W/O is aware the design team did not review and comment on Transmittal #140-02321 (DBI's comments) to Submittal Package #TG0300-542. Please confirm no design team changes or comments will be made to Submittal Package #TG0300-542 rather future trade packages.						----- RFI #T-0233.2 Question ----- W/O is in receipt of TJPA Submittal Package #TG0300-542 for the internal bracing from which W/O is proceeding per specification section 01 13 00. W/O is aware the design team did not review and comment on Transmittal #140-02321 (DBI's comments) to Submittal Package #TG0300-542. Please confirm no design team changes or comments will be made to Submittal Package #TG0300-542 rather future trade packages.
----- RFI #T-0233.1 Response ----- TT is currently reviewing the Internal Bracing Design Documents, which was received by TT on 09/29/2011. TT's comments to this document will be marked up on the Internal Bracing Design Document.						----- RFI #T-0233.1 Response ----- TT is currently reviewing the Internal Bracing Design Documents, which was received by TT on 09/29/2011. TT's comments to this document will be marked up on the Internal Bracing Design Document.
----- RFI #T-0233.1 Question -----						



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	<p>The SFDBI-approved Internal Bracing drawings and related calculations was sent to W/O on 9/22/2011 as TJPA Transmittal No. 140-02321 - Approved Internal Bracing for Shoring Wall Permit Drawings, and available in Constructware.</p> <p>----- RFI #T-0233 Response ----- Thornton Tomasetti's response is pending receipt and review of revised internal bracing submittal.</p> <p>----- RFI #T-0233 Question ----- The BSE submittal TG0300-542.1 Internal Bracing Design was approved by TJPA and the fabrication will start as soon as permission is issued by the City. Please confirm the design was acceptable to permanent structural designer (Thornton Tomasetti) and incorporated into their design for future trade packages.</p>					
<b>T-0233.5</b>	<b>BSE - Internal Bracing Design Coordination with Structural Design</b>	<b>Closed</b>	<b>01</b>	<b>10/17/2011</b>	<b>10/27/2011</b>	<b>10/18/2011</b>
	<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference RFI #T-0233, T-0233.1, T-0233.2, T-0233.3, T-0233.4, Submittal TG0300-542 and TJPA Transmittal No.140-02321.</p> <p>Per response to RFI#T-0233.4, comments from the design team were to be received by October 14, 2011.</p> <p>Please provide the design team comments and confirmation for RFI #T-0233.</p> <p>----- RFI #T-0233.4 Response ----- Comments will be returned by 14 October 2011.</p> <p>----- RFI #T-0233.4 Question ----- Reference RFI #T-0233, T-0233.1, T-0233.2, Submittal TG0300-542 and TJPA Transmittal No.140-02321.</p> <p>When will the Design team provide the information / confirmation for RFI #T-0233?</p>					
	<p>----- RFI #T-0233.1 Question ----- The SFDBI-approved Internal Bracing drawings and related calculations was sent to W/O on 9/22/2011 as TJPA Transmittal No. 140-02321 - Approved Internal Bracing for Shoring Wall Permit Drawings, and available in Constructware.</p> <p>----- RFI #T-0233 Response ----- Thornton Tomasetti's response is pending receipt and review of revised internal bracing submittal.</p> <p>----- RFI #T-0233 Question ----- The BSE submittal TG0300-542.1 Internal Bracing Design was approved by TJPA and the fabrication will start as soon as permission is issued by the City. Please confirm the design was acceptable to permanent structural designer (Thornton Tomasetti) and incorporated into their design for future trade packages.</p>					
	<p><b>ANSWER:</b></p> <p>Reference RFI #T-0233, T-0233.1, T-0233.2, T-0233.3, T-0233.4, Submittal TG0300-542 and TJPA Transmittal No.140-02321.</p> <p>Per response to RFI#T-0233.4, comments from the design team were to be received by October 14, 2011.</p> <p>Please provide the design team comments and confirmation for RFI #T-0233.</p> <p>----- RFI #T-0233.4 Response ----- Comments will be returned by 14 October 2011.</p> <p>----- RFI #T-0233.4 Question ----- Reference RFI #T-0233, T-0233.1, T-0233.2, Submittal TG0300-542 and TJPA Transmittal No.140-02321.</p> <p>When will the Design team provide the information /</p>					



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	<p>----- RFI #T-0233.3 Response ----- This RFI contains a statement, not a question and is inappropriate for the RFI process. RFI T-0233.2 will remain closed but unresolved until the requested information is provided.</p> <p>----- RFI #T-0233.3 Question ----- This RFI shall not be closed until the information / confirmation received from the Design team.</p> <p>----- RFI #T-0233.2 Response ----- Thornton Tomasetti will be issuing comments to Transmittal #140-02321.</p> <p>----- RFI #T-0233.2 Question ----- W/O is in receipt of TJPA Submittal Package #TG0300-542 for the internal bracing from which W/O is proceeding per specification section 01 13 00. W/O is aware the design team did not review and comment on Transmittal #140-02321 (DBI's comments) to Submittal Package #TG0300-542.</p> <p>Please confirm no design team changes or comments will be made to Submittal Package #TG0300-542 rather future trade packages.</p> <p>----- RFI #T-0233.1 Response ----- TT is currently reviewing the Internal Bracing Design Documents, which was received by TT on 09/29/2011. TT's comments to this document will be marked up on the Internal Bracing Design Document.</p> <p>----- RFI #T-0233.1 Question ----- The SFDBI-approved Internal Bracing drawings and related calculations was sent to W/O on 9/22/2011 as TJPA Transmittal No. 140-02321 - Approved Internal Bracing for Shoring Wall Permit Drawings, and available in Constructware.</p> <p>----- RFI #T-0233.0 Response ----- Thornton Tomasetti's response is pending receipt and</p>					
	<p>confirmation for RFI #T-0233?</p> <p>----- RFI #T-0233.3 Response ----- This RFI contains a statement, not a question and is inappropriate for the RFI process. RFI T-0233.2 will remain closed but unresolved until the requested information is provided.</p> <p>----- RFI #T-0233.3 Question ----- This RFI shall not be closed until the information / confirmation received from the Design team.</p> <p>----- RFI #T-0233.2 Response ----- Thornton Tomasetti will be issuing comments to Transmittal #140-02321.</p> <p>----- RFI #T-0233.2 Question ----- W/O is in receipt of TJPA Submittal Package #TG0300-542 for the internal bracing from which W/O is proceeding per specification section 01 13 00. W/O is aware the design team did not review and comment on Transmittal #140-02321 (DBI's comments) to Submittal Package #TG0300-542.</p> <p>Please confirm no design team changes or comments will be made to Submittal Package #TG0300-542 rather future trade packages.</p> <p>----- RFI #T-0233.1 Response ----- TT is currently reviewing the Internal Bracing Design Documents, which was received by TT on 09/29/2011. TT's comments to this document will be marked up on the Internal Bracing Design Document.</p> <p>----- RFI #T-0233.1 Question ----- The SFDBI-approved Internal Bracing drawings and related calculations was sent to W/O on 9/22/2011 as TJPA Transmittal No. 140-02321 - Approved Internal Bracing for Shoring Wall Permit Drawings, and available in Constructware.</p> <p>----- RFI #T-0233.0 Response -----</p>					



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	<p>review of revised internal bracing submittal.</p> <p>----- RFI #T-0233.0 Question ----- Reference Specification Section 31 55 00 The BSE submittal TG0300-542.1 Internal Bracing Design was approved by TJPA and the fabrication will start as soon as permission is issued by the City.</p> <p>Please confirm the design was acceptable to permanent structural designer (Thornton Tomasetti) and incorporated into their design for future trade packages.</p>					<p>Thornton Tomasetti's response is pending receipt and review of revised internal bracing submittal.</p> <p>----- RFI #T-0233.0 Question ----- Reference Specification Section 31 55 00 The BSE submittal TG0300-542.1 Internal Bracing Design was approved by TJPA and the fabrication will start as soon as permission is issued by the City.</p> <p>Please confirm the design was acceptable to permanent structural designer (Thornton Tomasetti) and incorporated into their design for future trade packages.</p>
<b>T-0234</b>	<b>BSE - Buttress Shaft Post Pour Settlement</b>	<b>Closed</b>	<b>01</b>	<b>09/20/2011</b>	<b>09/30/2011</b>	<b>09/22/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Sheet GT-2201 and Specification Section 31 63 29						Reference Sheet GT-2201 and Specification Section 31 63 29
Please be informed that an uncontrolled settlement was observed at Buttress shaft C2, which was poured on Sunday 9/18/2011. The settlement led to the formation of a 13' deep unstable hole on the buttress working pad. After consulting with ARUP representative and W/O's field personnel, BBII/Becho Inc. decided to fill the newly formed hole with concrete to mitigate the settlement risk of the working pad. Additional concrete was poured into the 13' deep hole on Monday 9/19/2011.						Please be informed that an uncontrolled settlement was observed at Buttress shaft C2, which was poured on Sunday 9/18/2011. The settlement led to the formation of a 13' deep unstable hole on the buttress working pad. After consulting with ARUP representative and W/O's field personnel, BBII/Becho Inc. decided to fill the newly formed hole with concrete to mitigate the settlement risk of the working pad. Additional concrete was poured into the 13' deep hole on Monday 9/19/2011.
Please confirm that pouring additional concrete/CLSM will be considered as an acceptable method, if such settlements will occur during the future installation of the upcoming buttress shafts.						Please confirm that pouring additional concrete/CLSM will be considered as an acceptable method, if such settlements will occur during the future installation of the upcoming buttress shafts.



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<b>T-0235</b>	<b>BSE - Unforeseen Reinforced Concrete Slab at GL 7.5 J</b>	<b>Closed</b>	<b>01</b>	<b>09/20/2011</b>	<b>09/30/2011</b>	<b>09/27/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet D-2210, Specification Section 31 56 13, attached photos and sketch		Reference Sheet D-2210, Specification Section 31 56 13, attached photos and sketch				
While excavating a pre trench at gridline 7.5J close to Natoma Street, BBII uncovered an unforeseen reinforced concrete slab. This slab is 3ft thick, uncovered at grade and a section of it is in the direct line of the proposed CDSM wall. Indicated at this location in drawing D-2210 are grade beams and pile caps which BBII assumes will be encountered under this mat slab. However, this slab is not indicated on contract drawing D-2210. The concrete shown in contract survey sheet 5 appears to be a concrete driveway and it does not indicate the 3ft thick concrete slab that BBII are encountering. Measurements taken in the field also indicate a larger area than this. The attached photos and drawing indicate the scale of this obstruction. It is required to be removed.		While excavating a pre trench at gridline 7.5J close to Natoma Street, BBII uncovered an unforeseen reinforced concrete slab. This slab is 3ft thick, uncovered at grade and a section of it is in the direct line of the proposed CDSM wall. Indicated at this location in drawing D-2210 are grade beams and pile caps which BBII assumes will be encountered under this mat slab. However, this slab is not indicated on contract drawing D-2210. The concrete shown in contract survey sheet 5 appears to be a concrete driveway and it does not indicate the 3ft thick concrete slab that BBII are encountering. Measurements taken in the field also indicate a larger area than this. The attached photos and drawing indicate the scale of this obstruction. It is required to be removed.				
Please advise if this is acceptable.		Please advise if this is acceptable.				
<b>T-0236</b>	<b>BSE - Unforeseen Concrete Section Found at Grid Line 1E</b>	<b>Closed</b>	<b>01</b>	<b>09/22/2011</b>	<b>10/02/2011</b>	<b>09/26/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet D-2210 (attached), Specification Section 31 56 13, and attached photos		Reference Sheet D-2210 (attached), Specification Section 31 56 13, and attached photos				
While DND were drilling at panel 28 and 29 on grid line 1E at the locations of piles 839-843, an unknown section of concrete was encountered. The concrete was found at a depth of 9.5ft. The quantity of concrete is unknown at this point. The concrete is not indicated on contract drawing D-2210. It is in direct conflict with the CDSM shoring wall and must be removed. Shown below [attached] are photos of the debris removed from the excavation.		While DND were drilling at panel 28 and 29 on grid line 1E at the locations of piles 839-843, an unknown section of concrete was encountered. The concrete was found at a depth of 9.5ft. The quantity of concrete is unknown at this point. The concrete is not indicated on contract drawing D-2210. It is in direct conflict with the CDSM shoring wall and must be removed. Shown below [attached] are photos of the debris removed from the excavation.				
Please advise on how to proceed.		Please advise on how to proceed.				
<b>T-0237</b>	<b>BSE - Bridge Welding Code</b>	<b>Closed</b>	<b>01</b>	<b>09/26/2011</b>	<b>10/06/2011</b>	<b>10/03/2011</b>





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<div><div>From: Webcor Construction LP</div><div>Nhi Tran</div></div>						
REQUEST:			ANSWER:			
Reference Specification 01 53 30			Reference Specification 01 53 30			
<p>Temporary Bridge Specification 01 53 13 (1.6H) requires the welding qualifications for the bridges to be in accordance with AWS D1.5 "Bridge Welding Code", however BBII's design was based on AWS D1.1 "Structural Welding Code" as specified in General note 3.2-A4.2 of Sheet SH-0100. BBII and their designer felt AWS D1.1 is more applicable for the temporary bridge structure for the following reasons:</p> <ul style="list-style-type: none"><li>- The members that make up BBII's temporary bridge consists of readily available standard grade mill rolled shapes, comprised of a variety of base metals (A36, A53, A572, A992, A500, and A252) which are joined by simple prequalified joints (fillets). D1.1 provides the flexibility to weld all of these base metals in any combination utilizing prequalified procedures, since they are all in the same base metal group. D1.5 only allows prequalified welding of A709 plate material only.</li><li>- BBII's temporary bridge structure contains structural tubing (piers and rails), which D1.5 does not cover tubing</li><li>- The bridge as designed has short spans and very simple welded connections. All welds shown are fillet welds (mostly single pass). Additionally there are no complete penetration welds as are typically seen on steel plate girder bridges.</li><li>- The life span of these temporary bridges are less than 5 years</li><li>- The temporary bridge's intended use and the site specific geometry restraints led to a steel framing design much more similar to a structural steel building than to a typical Highway bridge. The steel columns with angle cross-bracing, and the girders and cap beams as detailed are similar to building with columns and floor beams.</li></ul> <p>The submittal review did not take exception to the general note specifying D1.1. therefore please confirm it is acceptable to submit weld procedures and welder qualifications per AWS D1.1 as specified by the bridge's Engineer of Record.</p>			<p>Temporary Bridge Specification 01 53 13 (1.6H) requires the welding qualifications for the bridges to be in accordance with AWS D1.5 "Bridge Welding Code", however BBII's design was based on AWS D1.1 "Structural Welding Code" as specified in General note 3.2-A4.2 of Sheet SH-0100. BBII and their designer felt AWS D1.1 is more applicable for the temporary bridge structure for the following reasons:</p> <ul style="list-style-type: none"><li>- The members that make up BBII's temporary bridge consists of readily available standard grade mill rolled shapes, comprised of a variety of base metals (A36, A53, A572, A992, A500, and A252) which are joined by simple prequalified joints (fillets). D1.1 provides the flexibility to weld all of these base metals in any combination utilizing prequalified procedures, since they are all in the same base metal group. D1.5 only allows prequalified welding of A709 plate material only.</li><li>- BBII's temporary bridge structure contains structural tubing (piers and rails), which D1.5 does not cover tubing</li><li>- The bridge as designed has short spans and very simple welded connections. All welds shown are fillet welds (mostly single pass). Additionally there are no complete penetration welds as are typically seen on steel plate girder bridges.</li><li>- The life span of these temporary bridges are less than 5 years</li><li>- The temporary bridge's intended use and the site specific geometry restraints led to a steel framing design much more similar to a structural steel building than to a typical Highway bridge. The steel columns with angle cross-bracing, and the girders and cap beams as detailed are similar to building with columns and floor beams.</li></ul> <p>The submittal review did not take exception to the general note specifying D1.1. therefore please confirm it is acceptable to submit weld procedures and welder</p>			



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qualifications per AWS D1.1 as specified by the bridge's Engineer of Record.						
<b>T-0237.1</b>	<b>BSE - Bridge Welding Code</b>	<b>Closed</b>	<b>01</b>	<b>10/03/2011</b>	<b>10/13/2011</b>	<b>10/03/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI #T-0237 and Specification Section 01 53 30			Reference RFI #T-0237 and Specification Section 01 53 30			
RFI #T-0237 was returned to W/O with two responses regarding the temporary bridge welding. Please clarify which is the governing response or provide one coordinated response.			RFI #T-0237 was returned to W/O with two responses regarding the temporary bridge welding. Please clarify which is the governing response or provide one coordinated response.			





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<b>T-0238</b>	<b>BSE - Zone 1 CDSM Crossing Over Existing Wall</b>	<b>Closed</b>	<b>01</b>	<b>09/26/2011</b>	<b>10/06/2011</b>	<b>09/29/2011</b>
<div><div><p><b>From:</b> Webcor Construction LP      Nhi Tran</p><p><b>REQUEST:</b></p><p>Reference Sheet GT-5101, Specification Section 31 56 13, attached photos and sketch</p><p>Please address the following information request from BBII's sub contractor DND:</p><p>"The new CDSM shoring wall crosses an existing CDSM wall at 2 locations. Following CR T-005B, both of these crossings are perpendicular to the existing CDSM wall, as shown in Note 1 on GT-5101. Note 1 shows the new wall making a jog to avoid hitting the beams of the existing CDSM wall. The detail shown on contract plan GT-5101 is constructible only if the existing CDSM wall was built exactly as shown, without any room for construction tolerances for both the new and existing wall. Instead of trying to install this section of the CDSM wall according to the detail shown on GT-5101, which would potentially cause damage to the CDSM equipment, DND proposes to remove the existing CDSM beams that are in conflict. The contract plan GT-5101 shows two CDSM panels to jog around the existing beam and one offset panel parallel to the new wall.</p><p>DND's proposed solution would eliminate the 2 panels in the jog but still maintain the additional offset panel parallel to the wall line. This additional offset panel would act as insurance so a seal is maintained through any deflection caused by the hard in-situ soil mix. This would present a potential cost savings to the project (due to 2 less panels being installed), providing the conflicting beams can be successfully removed.</p><p>DND has mobilized a drill rig with an auger to this area to pre-drill the wall prior to the removal of beams. This will substantially reduce the amount of vibration that will be required to remove the beams. DND proposes to utilize the same method at the other wall crossing near Natoma Street. Is this proposed method of removing the existing beams and soil mixing through the existing CDSM wall acceptable?"</p></div><div><p><b>ANSWER:</b></p><p>Reference Sheet GT-5101, Specification Section 31 56 13, attached photos and sketch</p><p>Please address the following information request from BBII's sub contractor DND:</p><p>"The new CDSM shoring wall crosses an existing CDSM wall at 2 locations. Following CR T-005B, both of these crossings are perpendicular to the existing CDSM wall, as shown in Note 1 on GT-5101. Note 1 shows the new wall making a jog to avoid hitting the beams of the existing CDSM wall. The detail shown on contract plan GT-5101 is constructible only if the existing CDSM wall was built exactly as shown, without any room for construction tolerances for both the new and existing wall. Instead of trying to install this section of the CDSM wall according to the detail shown on GT-5101, which would potentially cause damage to the CDSM equipment, DND proposes to remove the existing CDSM beams that are in conflict. The contract plan GT-5101 shows two CDSM panels to jog around the existing beam and one offset panel parallel to the new wall.</p><p>DND's proposed solution would eliminate the 2 panels in the jog but still maintain the additional offset panel parallel to the wall line. This additional offset panel would act as insurance so a seal is maintained through any deflection caused by the hard in-situ soil mix. This would present a potential cost savings to the project (due to 2 less panels being installed), providing the conflicting beams can be successfully removed.</p><p>DND has mobilized a drill rig with an auger to this area to pre-drill the wall prior to the removal of beams. This will substantially reduce the amount of vibration that will be required to remove the beams. DND proposes to utilize the same method at the other wall crossing near Natoma Street. Is this proposed method of removing the existing beams and soil mixing through the existing CDSM wall acceptable?"</p></div></div>						
<b>T-0239</b>	<b>BSE - Rebar Cages for Deeper Buttress Shafts</b>	<b>Closed</b>	<b>01</b>	<b>09/28/2011</b>	<b>10/08/2011</b>	<b>10/03/2011</b>



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	<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference Sheet GT-5202 Detail 12, RFI T-0216, and Approved Rebar Shop Drawings</p> <p>The approved rebar cages per RFI T-0216 are sized for 241' deep shafts. Rebar cages for shafts C-1 and M-1 have already been released and fabricated. Note that the depth after airlifting of shafts C-2 and M-2 have been 247' and 252.7' respectively. Please advise on how to proceed with the installation of the cages for shafts C-1 and M-1 and with the fabrication of the rest of the cages assuming these shafts extend beyond planned depth.</p>					
	<p><b>ANSWER:</b></p> <p>Reference Sheet GT-5202 Detail 12, RFI T-0216, and Approved Rebar Shop Drawings</p> <p>The approved rebar cages per RFI T-0216 are sized for 241' deep shafts. Rebar cages for shafts C-1 and M-1 have already been released and fabricated. Note that the depth after airlifting of shafts C-2 and M-2 have been 247' and 252.7' respectively. Please advise on how to proceed with the installation of the cages for shafts C-1 and M-1 and with the fabrication of the rest of the cages assuming these shafts extend beyond planned depth.</p>					
<b>T-0240</b>	<b>BSE - Demo AT&amp;T Duct on Natoma at Second</b>	<b>Closed</b>	<b>01</b>	<b>09/29/2011</b>	<b>10/09/2011</b>	<b>10/07/2011</b>
	<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference Sheets U-1110, D-2231, ASI-015, Specification Section 31 56 13, attached email and BBI RFI 222</p> <p>It was discovered on 9/27/2011 while performing the utility demo for the revised shoring wall alignment (TG03 BSE CR T-005B) issued in ASI 15 that the abandoned AT&amp;T line servicing the demolished buildings on Natoma was never fully abandoned by AT&amp;T. According to the attached email from Huan Huynh of AT&amp;T, AT&amp;T was never notified that these lines needed to be abandoned due to the revised shoring wall alignment of the Transbay Project.</p> <p>Please confirm when CDSM Shoring Wall can be installed in the area. Currently, BBII is installing the CDSM Shoring Wall on line 1 and the confirmation of the line abandonment is required as quickly as possible to avoid any project delay.</p> <p>Please also refer to the attached BBI RFI 0222 for this issue</p>					
	<p><b>ANSWER:</b></p> <p>Reference Sheets U-1110, D-2231, ASI-015, Specification Section 31 56 13, attached email and BBI RFI 222</p> <p>It was discovered on 9/27/2011 while performing the utility demo for the revised shoring wall alignment (TG03 BSE CR T-005B) issued in ASI 15 that the abandoned AT&amp;T line servicing the demolished buildings on Natoma was never fully abandoned by AT&amp;T. According to the attached email from Huan Huynh of AT&amp;T, AT&amp;T was never notified that these lines needed to be abandoned due to the revised shoring wall alignment of the Transbay Project.</p> <p>Please confirm when CDSM Shoring Wall can be installed in the area. Currently, BBII is installing the CDSM Shoring Wall on line 1 and the confirmation of the line abandonment is required as quickly as possible to avoid any project delay.</p> <p>Please also refer to the attached BBI RFI 0222 for this issue</p>					
<b>T-0241</b>	<b>BSE - Brick Wall at GL 2, J Line In Conflict With The CDSM Wall</b>	<b>Closed</b>	<b>01</b>	<b>09/29/2011</b>	<b>10/09/2011</b>	<b>10/07/2011</b>
	<p><b>From:</b> Webcor Construction LP                      Nhi Tran</p>					



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	<b>REQUEST:</b>  Reference Specification Section 31 56 13 and attached meeting minutes and photos  The brick wall remaining from the 580 Howard building, at grid line 2 J, is protruding into the CDSM wall limits, as noted in BBII's previous RFI #203 (The question was responded by TCCO at the job site meeting on 9/6/2011. Refer to the attached meeting minutes). While attempting to remove, BBII has discovered that the fence and patio pavement are founded on this remaining portion of brick wall. This condition does not allow for the removal of the wall without damage to the fence and patio.  Please provide direction on how to proceed.					
	<b>ANSWER:</b>  Reference Specification Section 31 56 13 and attached meeting minutes and photos  The brick wall remaining from the 580 Howard building, at grid line 2 J, is protruding into the CDSM wall limits, as noted in BBII's previous RFI #203 (The question was responded by TCCO at the job site meeting on 9/6/2011. Refer to the attached meeting minutes). While attempting to remove, BBII has discovered that the fence and patio pavement are founded on this remaining portion of brick wall. This condition does not allow for the removal of the wall without damage to the fence and patio.  Please provide direction on how to proceed.					
<b>T-0242</b>	<b>BSE - Becho's Request For Rock Classification Data</b>	<b>Closed</b>	<b>01</b>	<b>09/29/2011</b>	<b>10/09/2011</b>	<b>10/11/2011</b>
	<b>From:</b> Webcor Construction LP Nhi Tran					
	<b>REQUEST:</b>  Reference Sheet GT-2201, Specification Section 31 63 29, and attached letter from Becho  Please find attached BBII's sub-contractor Becho's letter that requests the following information:  "... during the drilling of buttress shaft M4 rock socket, at a depth of approximately 250 feet below ground level, Becho encountered rock formations of unmeasured hardness. At a depth of 250 feet, Becho's steel grab, used for rock drilling, fractured under the increased stress. Please see attached photos. The incident occurred between the hours of 9.30 am and 10.00 am on Wednesday, 09.28.11. BBII immediately notified W/O and called for an emergency meeting to discuss the hardness of the rock formation and the status of drilling. During the meeting, Arup confirmed and accepted the 250 foot depth to be adequate and sufficient to stop the rock socket drilling. Immediately, following Arup's confirmation at 11.09 am, Becho proceeded to clean the remaining rock debris from the bottom of the shaft and prep for air lifting operation. The total down time recorded as a result of the incident is 68 minutes, not including adjustments of airlift, tremie pipe and repair of grab.					
	<b>ANSWER:</b>  Reference Sheet GT-2201, Specification Section 31 63 29, and attached letter from Becho  Please find attached BBII's sub-contractor Becho's letter that requests the following information:  "... during the drilling of buttress shaft M4 rock socket, at a depth of approximately 250 feet below ground level, Becho encountered rock formations of unmeasured hardness. At a depth of 250 feet, Becho's steel grab, used for rock drilling, fractured under the increased stress. Please see attached photos. The incident occurred between the hours of 9.30 am and 10.00 am on Wednesday, 09.28.11. BBII immediately notified W/O and called for an emergency meeting to discuss the hardness of the rock formation and the status of drilling. During the meeting, Arup confirmed and accepted the 250 foot depth to be adequate and sufficient to stop the rock socket drilling. Immediately, following Arup's confirmation at 11.09 am, Becho proceeded to clean the remaining rock debris from the bottom of the shaft and prep for air lifting operation. The total down time recorded as a result of the incident is 68 minutes, not including adjustments of					



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T-0243	<p><b>BSE - Emergency Exit at 530 Howard GL 10 J</b></p> <p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference Specification Section 31 56 13 and attached sketch</p> <p>Pre-trenching and CDSM wall installation at the rear of the 530 Howard building will have an impact on the accessibility to the emergency exit at that location. In order for the pre trench and the CDSM wall installation to safely proceed past this location, the rear exit must be closed for 1-2 days for each operation. The attached drawing indicates the location of the emergency exit and its proximity to the CDSM wall.</p> <p>Please confirm if this is acceptable. BBII is available to meet with the property owner to coordinate this work.</p>	Closed	01	09/29/2011	10/09/2011	10/10/2011
	<p><b>ANSWER:</b></p> <p>Reference Specification Section 31 56 13 and attached sketch</p> <p>Pre-trenching and CDSM wall installation at the rear of the 530 Howard building will have an impact on the accessibility to the emergency exit at that location. In order for the pre trench and the CDSM wall installation to safely proceed past this location, the rear exit must be closed for 1-2 days for each operation. The attached drawing indicates the location of the emergency exit and its proximity to the CDSM wall.</p> <p>Please confirm if this is acceptable. BBII is available to meet with the property owner to coordinate this work.</p>					



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0244</b>	<b>BSE - Request for Additional Geotechnical Data Pertaining To Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>09/29/2011</b>	<b>10/09/2011</b>	<b>10/11/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Sheet GT-2201 and Specification Section 31 63 29			Reference Sheet GT-2201 and Specification Section 31 63 29			
Please address the following information request from BBII's sub contractor Becho Inc.:			Please address the following information request from BBII's sub contractor Becho Inc.:			
"... for each of the shafts completed and under construction, Becho has excavated deeper than the elevations shown for boring logs. Becho is requesting soil samples, boring logs, torque requirements, skin friction values, and rock strengths be provided for these depths. (Currently 254 ft below elevation +14.00).			"... for each of the shafts completed and under construction, Becho has excavated deeper than the elevations shown for boring logs. Becho is requesting soil samples, boring logs, torque requirements, skin friction values, and rock strengths be provided for these depths. (Currently 254 ft below elevation +14.00).			
The requested information is similar to what was provided up to the depths of 234 and 237.5 feet in the "Final Geotechnical Data Report" prepared by Arup dated February 2010, and "Prototype Test Program and Monitoring During Construction of Drilled Shafts" prepared by Arup dated May 2010. Becho requests this information for drilling beyond the depths specified in the Geotechnical Report."			The requested information is similar to what was provided up to the depths of 234 and 237.5 feet in the "Final Geotechnical Data Report" prepared by Arup dated February 2010, and "Prototype Test Program and Monitoring During Construction of Drilled Shafts" prepared by Arup dated May 2010. Becho requests this information for drilling beyond the depths specified in the Geotechnical Report."			
<b>T-0244.1</b>	<b>BSE - Becho Request for Buttress Field Logs</b>	<b>Closed</b>	<b>CR</b>	<b>03/23/2012</b>	<b>04/02/2012</b>	<b>04/24/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
BECHO formally requests to obtain the Daily Field Logs from every ARUP field engineer/geotech/geologist, TJPA representative involved with the Buttress Shaft work. More specifically, field notes/logs from engineers and TJPA representatives involved with the field data collection, sample collection and inspection process. Becho requests the Daily Field Logs for the following dates: - September 12th 2011 through October 20th 2011 - February 22nd 2012 through Today			BECHO formally requests to obtain the Daily Field Logs from every ARUP field engineer/geotech/geologist, TJPA representative involved with the Buttress Shaft work. More specifically, field notes/logs from engineers and TJPA representatives involved with the field data collection, sample collection and inspection process. Becho requests the Daily Field Logs for the following dates: - September 12th 2011 through October 20th 2011 - February 22nd 2012 through Today			



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<b>T-0244.2</b>	<b>BSE - Becho Request for Buttress Field Logs Follow-Up</b>	<b>Closed</b>	<b>01</b>	<b>04/18/2012</b>	<b>04/28/2012</b>	<b>04/24/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b> After reviewing Constructware as directed in RFI T-0244.1; W/O is unable to locate ARUP field reports for the dates between 9/12/11-9/30/11. Please advise as to the location of the aforementioned documents.						<b>ANSWER:</b> After reviewing Constructware as directed in RFI T-0244.1; W/O is unable to locate ARUP field reports for the dates between 9/12/11-9/30/11. Please advise as to the location of the aforementioned documents.
<b>T-0244.3</b>	<b>Becho's 3rd Request for Arup's Field Logs</b>	<b>Closed</b>	<b>01</b>	<b>07/24/2012</b>	<b>08/03/2012</b>	<b>08/01/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ernie Cortez						
<b>REQUEST:</b> Becho formally requests to obtain any and all documentation Arup has for logging and documenting soil samples retrieved from the Buttress shafts starting 9/12/2011 thru 10/1/2011, including all documentation pertaining to quality control as specified in section 31.63.29.3.8.B.  Reference attached Becho Letter BI-0244.						<b>ANSWER:</b> Becho formally requests to obtain any and all documentation Arup has for logging and documenting soil samples retrieved from the Buttress shafts starting 9/12/2011 thru 10/1/2011, including all documentation pertaining to quality control as specified in section 31.63.29.3.8.B.  Reference attached Becho Letter BI-0244.
<b>T-0245</b>	<b>BSE - Ground Conduits detail for PG&amp;E phase 2 works on First Street</b>	<b>Closed</b>	<b>01</b>	<b>10/05/2011</b>	<b>10/15/2011</b>	<b>10/12/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b> Reference: CR No. T-017 - BSE - First Street Phase 2 Utility Relocation  For the installation of the PGE 6" and PGE 4" GRS conduit between the CDSM walls, is grounding of the PGE conduits required? If so, please provide grounding details/requirements.						<b>ANSWER:</b> Reference: CR No. T-017 - BSE - First Street Phase 2 Utility Relocation  For the installation of the PGE 6" and PGE 4" GRS conduit between the CDSM walls, is grounding of the PGE conduits required? If so, please provide grounding details/requirements.
<b>T-0246</b>	<b>BSE - PG&amp;E Sweep Radius Requirements</b>	<b>Closed</b>	<b>01</b>	<b>10/10/2011</b>	<b>10/20/2011</b>	<b>10/11/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b> Reference CR T-017.  (The attached drawings provided at the PG&E / BBII / Verizon Coordination Meeting on 9/29/2011) refer to 10ft						<b>ANSWER:</b> Reference CR T-017.  (The attached drawings provided at the PG&E / BBII / Verizon Coordination Meeting on 9/29/2011) refer to



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## PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

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	radius elbows and bends. PG&E standards refer require 6ft radius elbows and bends. Please confirm radius requirements for 6" conduit installation for the Phase 2 utility on First Street.					10ft radius elbows and bends. PG&E standards refer require 6ft radius elbows and bends. Please confirm radius requirements for 6" conduit installation for the Phase 2 utility on First Street.
<b>T-0247</b>	<b>BSE - Proposed Corrective Action Plan for Sunken CDSM Soldier Piles</b>	<b>Closed</b>	<b>01</b>	<b>10/10/2011</b>	<b>10/10/2011</b>	<b>10/12/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Specification Section 31 56 13						Reference Specification Section 31 56 13
Please address the following information request from BBII's sub contractor DND: "As of to date, the following three soldier piles have sunk below grade during their placement into the CDSM wall. - Beam # 154 installed on 09.08.11 - Beam # 631, installed on 09.29.11 - Beam # 602, installed on 10.01.11  DND was unable to recover those piles and set them to their plan elevations without disturbing the adjacent beams that were already in place. To mitigate this issue, DND proposes to conduct the below course of remedial action: 1) Wait until mass excavation commences. Excavate with caution the locations, and determine the top elevation of the sunken beams. 2) Provide this information to the Engineer for evaluation. 3) Implement corrective action based on Engineer's evaluation. Possible corrective measures are: a. No action necessary. The strength of the CDSM material may be sufficient to support the unreinforced depth. b. Install lagging between the adjacent beams above the top of the sunken beam. c. Splice a beam on the top of the sunken beam and backfill with low strength concrete.  Please advise, if the proposed course of remedial action and/or any of the three possible corrective measures are acceptable."						Please address the following information request from BBII's sub contractor DND: "As of to date, the following three soldier piles have sunk below grade during their placement into the CDSM wall. - Beam # 154 installed on 09.08.11 - Beam # 631, installed on 09.29.11 - Beam # 602, installed on 10.01.11  DND was unable to recover those piles and set them to their plan elevations without disturbing the adjacent beams that were already in place. To mitigate this issue, DND proposes to conduct the below course of remedial action: 1) Wait until mass excavation commences. Excavate with caution the locations, and determine the top elevation of the sunken beams. 2) Provide this information to the Engineer for evaluation. 3) Implement corrective action based on Engineer's evaluation. Possible corrective measures are: a. No action necessary. The strength of the CDSM material may be sufficient to support the unreinforced depth. b. Install lagging between the adjacent beams above the top of the sunken beam. c. Splice a beam on the top of the sunken beam and backfill with low strength concrete.  Please advise, if the proposed course of remedial action and/or any of the three possible corrective measures are acceptable."





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T-0247.1	<b>BSE - Proposed Corrective Plan for the following Sunken Solider Piles</b>  <b>From:</b> Webcor/Obayashi Joint Venture      Kirk Nielsen  <b>REQUEST:</b>  Reference: Attached Corrective Action Plan  Message: Please find attached BBII's proposed corrective plan for the following sunken solider piles: 1. Pile #59, Notice #47, Vela Issue #J-00007. 2. Pile #154, Vela Issue #J-00001. 3. Pile #602, Vela Issue #J-00008. Please approve and or comment.	Closed	CR	01/10/2012	01/20/2012	01/12/2012
	<b>ANSWER:</b>  Reference: Attached Corrective Action Plan  Message: Please find attached BBII's proposed corrective plan for the following sunken solider piles: 1. Pile #59, Notice #47, Vela Issue #J-00007. 2. Pile #154, Vela Issue #J-00001. 3. Pile #602, Vela Issue #J-00008. Please approve and or comment.					
T-0248	<b>BSE - First St. Verizon Utilities Relocation</b>  <b>From:</b> Webcor Construction LP      Masashi Kojima  <b>REQUEST:</b>  Reference Specification Section 01 53 13  Attached is an as-built sketch of Verizon utilities potholed and located along First St. on 10/4/10. These utilities were originally scheduled to be relocated during phase two to allow for CDSM installation and subsequently temporary bridge construction. BBII has learned that in an effort to save time, the TJPA is considering leaving the utilities in their current locations and working around them. As shown on the attached section of the First St. temporary bridge, the Verizon utilities will be in direct conflict with the temporary bridge structure. Please confirm these utilities will be relocated as planned to allow for installation of the CDSM shoring wall and temporary bridge.	Closed	01	10/10/2011	10/20/2011	01/04/2012
	<b>ANSWER:</b>  Reference Specification Section 01 53 13  Attached is an as-built sketch of Verizon utilities potholed and located along First St. on 10/4/10. These utilities were originally scheduled to be relocated during phase two to allow for CDSM installation and subsequently temporary bridge construction. BBII has learned that in an effort to save time, the TJPA is considering leaving the utilities in their current locations and working around them. As shown on the attached section of the First St. temporary bridge, the Verizon utilities will be in direct conflict with the temporary bridge structure. Please confirm these utilities will be relocated as planned to allow for installation of the CDSM shoring wall and temporary bridge.					
T-0249	<b>BSE - Pavement lights at the rear of 580 Howard</b>  <b>From:</b> Webcor Construction LP      Masashi Kojima  <b>REQUEST:</b>  Reference Specification Section 31 56 13 and CR T-005B.  There are two lights located on the ground inside the boundary fence at the rear of 580 Howard. The lights are located 4ft away from the brick wall (which is due to be	Closed	01	10/10/2011	10/20/2011	10/12/2011
	<b>ANSWER:</b>  Reference Specification Section 31 56 13 and CR T-005B.  There are two lights located on the ground inside the boundary fence at the rear of 580 Howard. The lights					





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T-0250	demolished) as shown the attached photos. A preliminary investigation indicates that the lights are de-energized. Please confirm that access to the property's electrical system will be available to confirm that the lights are de-energized.	Closed	01	10/13/2011	10/23/2011	11/03/2011
	are located 4ft away from the brick wall (which is due to be demolished) as shown the attached photos. A preliminary investigation indicates that the lights are de-energized. Please confirm that access to the property's electrical system will be available to confirm that the lights are de-energized.					
<b>T-0250</b>	<b>BSE - Soil Classification of South West Area of the Work Site</b>	<b>Closed</b>	<b>01</b>	<b>10/13/2011</b>	<b>10/23/2011</b>	<b>11/03/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 01 13 50 and Treadwell & Rollo site maps (attached)  BBII needs the soil classification listed and mapped for the lot between Natoma Street and Howard Street, and between Gridline A to Gridline 10. Please see the attached Treadwell & Rollo's Site Mitigation Map of the Soil Classification for the area in question.						
<b>ANSWER:</b>  Reference Specification Section 01 13 50 and Treadwell & Rollo site maps (attached)  BBII needs the soil classification listed and mapped for the lot between Natoma Street and Howard Street, and between Gridline A to Gridline 10. Please see the attached Treadwell & Rollo's Site Mitigation Map of the Soil Classification for the area in question.						
<b>T-0251</b>	<b>BSE - Drawings To Coordinate Trestle Pile Locations</b>	<b>Closed</b>	<b>01</b>	<b>10/13/2011</b>	<b>10/23/2011</b>	<b>10/14/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Masashi Kojima						
<b>REQUEST:</b>  During the 10/12/11 trestle submittal review meeting, statements were repeatedly made with regard to incrementally complete underground drawings in which to coordinate trestle pile locations. As of 10/13/11, W/O has not received any future package documents accompanied with the direction to coordinate with the TG03 documents. If such documents are available please make available the entire series to include, however not limited to, A, S, M, E, & P.						
<b>ANSWER:</b>  During the 10/12/11 trestle submittal review meeting, statements were repeatedly made with regard to incrementally complete underground drawings in which to coordinate trestle pile locations. As of 10/13/11, W/O has not received any future package documents accompanied with the direction to coordinate with the TG03 documents. If such documents are available please make available the entire series to include, however not limited to, A, S, M, E, & P.						
<b>T-0251.1</b>	<b>BSE - Drawings To Coordinate Trestle Pile Locations</b>	<b>Closed</b>	<b>01</b>	<b>10/14/2011</b>	<b>10/24/2011</b>	<b>11/03/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Nhi Tran						



**ANSWER:**

RFI T-0251 original inquiry:  
During the 10/12/11 trestle submittal review meeting, statements were repeatedly made with regard to incrementally complete underground drawings in which to coordinate trestle pile locations. As of 10/13/11, W/O has not received any future package documents accompanied with the direction to coordinate with the TG03 documents. If such documents are available please make available the entire series to include, however not limited to, A, S, M, E, & P.

RFI T-0251.1 Clarification to RFI T-0251:  
The TG03 package was executed with limited documents in which to coordinate future packages with. Please provide all documents the TJPA requests BBII coordinate the TG03 package with and to.

As it pertains to structural columns (round/pill/rectangle/etc.) please provide the minimum clear distance to trestle pile penetrations in the mat slab so BBII may coordinate.

Should there remain any ambiguity in the inquiry above please indicate the nature of misunderstanding.



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<b>T-0251.3</b>	<b>BSE - Drawings To Coordinate Trestle Pile Locations - "No Pin Pile Zone" at Lower Level</b>	<b>Closed</b>	<b>01</b>	<b>11/28/2011</b>	<b>12/08/2011</b>	<b>12/13/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference RFI #T-0251.2  So W/O may coordinate as requested in RFI response T-0251.2 please provide a drawing that depicts the column configurations, dimensions, and minimum clearance requirements, for both the platform and concourse levels. This information is required to locate trestle piles and internal bracing struts.						<b>ANSWER:</b>  Reference RFI #T-0251.2  So W/O may coordinate as requested in RFI response T-0251.2 please provide a drawing that depicts the column configurations, dimensions, and minimum clearance requirements, for both the platform and concourse levels. This information is required to locate trestle piles and internal bracing struts.
<b>T-0252</b>	<b>BSE - Buttress Rebar Cage Length Adjustment</b>	<b>Closed</b>	<b>01</b>	<b>10/19/2011</b>	<b>10/29/2011</b>	<b>10/24/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference RFI #T-0216, #T-0239, Sheet GT-2201, Specification Section 31 63 29, and attached sketch  Per the response to RFI T-0239, BBII needs to extend the length of rebar cages to accommodate buttress shafts that are deeper than 240'. The exact length of the rebar cage cannot be known until the drilling of the adjacent shaft. Due to this uncertainty, and the long lead time required to fabricate cages with varying lengths, BBII proposes to fabricate all rebar cages to a pre-extended length of 260'.  Once the depth of the adjacent shaft is known, the final length of the rebar cage will be adjusted by cutting the top of the rebar cage and the CSL tubes to the desired length. The length of the bottom "structural cage" section that consists of 24 Ea. vertical rebars will remain unchanged at 186'. The length of the top "setting cage" section that consists of 8 Ea. vertical rebars will be adjusted as described above. Please refer to the attached documents and the original shop drawings for the "structural cage" and the "setting cage" details.  BBII proposes to accommodate this change at no additional cost to TJPB beyond the bid item quantity payment per drilled shaft lengths.  Please advise, if it is acceptable.						<b>ANSWER:</b>  Reference RFI #T-0216, #T-0239, Sheet GT-2201, Specification Section 31 63 29, and attached sketch  Per the response to RFI T-0239, BBII needs to extend the length of rebar cages to accommodate buttress shafts that are deeper than 240'. The exact length of the rebar cage cannot be known until the drilling of the adjacent shaft. Due to this uncertainty, and the long lead time required to fabricate cages with varying lengths, BBII proposes to fabricate all rebar cages to a pre-extended length of 260'.  Once the depth of the adjacent shaft is known, the final length of the rebar cage will be adjusted by cutting the top of the rebar cage and the CSL tubes to the desired length. The length of the bottom "structural cage" section that consists of 24 Ea. vertical rebars will remain unchanged at 186'. The length of the top "setting cage" section that consists of 8 Ea. vertical rebars will be adjusted as described above. Please refer to the attached documents and the original shop drawings for the "structural cage" and the "setting cage" details.  BBII proposes to accommodate this change at no additional cost to TJPB beyond the bid item quantity payment per drilled shaft lengths.  Please advise, if it is acceptable.



**ANSWER:**

Reference Attachment 3 of Exhibit A of the TG03 Bid Package and attached memo from PB&A

Pursuant to the trestle design meeting held on October 12, 2011, Balfour Beatty Infrastructure Inc.' (BBII) requests clarification regarding their interpreted design criteria of the Temporary Access Trestle

As the only Contract document regarding the Trestle, Attachment 3 of Exhibit A of the TG030 Bid Manual has the following instructions:

In the second sentence of the second paragraph, the following statement is made, "For the design criteria for the Access Trestle, the Contract Documents and applicable standard shall be referred to." The next sentence states, "All requirements in the Temporary Bridge Specification in the Contract Documents, Section 01 53 13, shall apply to the Access Trestle."

Attachment 3 goes on further to provide very specific design load conditions and structural elements (i.e. Deck & barrier) that contradict the requirements of the Temporary bridge Spec Section 01 53 13. Based on the more "Trestle Specific" requirements of Attachment 3 and the interpreted function, being for construction use and not public use, of this type of temporary works structure, BBII and its Engineering Team arrived at the criteria /(basis of design) described in the attached memo from PB&A. This document was included with BBII's original design submittal; however for this RFI BBII has expanded some of the explanations.

Please review the provided information and confirm whether or not BBII's design criteria is appropriate for the Temporary Access Trestle.



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<b>T-0253.1</b>	<b>BSE - Trestle Design Criteria Follow-Up</b>	<b>Closed</b>	<b>01</b>	<b>11/21/2011</b>	<b>12/01/2011</b>	<b>12/02/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI#T-0253, Attachment 3 of Exhibit A of the TG03 BSE Bid Package, Specification Section 01 53 13, and attached memo from PB&A			Reference RFI#T-0253, Attachment 3 of Exhibit A of the TG03 BSE Bid Package, Specification Section 01 53 13, and attached memo from PB&A			
Follow up to RFI T-0253 and the meeting held 11/16/11			Follow up to RFI T-0253 and the meeting held 11/16/11			
As noted in the 11/16/11 meeting, the cross lot bracing "struts" are supported by the Trestle substructure and analysis requires limiting trestle deformations to be compatible with the allowable strut deflections (approximately 2"). As a result the "push over" analysis as required by the AASHTO Seismic Design Criteria "SDC" (requirement of bridge spec 01 53 13) is not applicable. This was discussed in detail during the 11/16 meeting and it was concurred that due to unique structural configuration and deflection requirements, an alternate analysis method other than the SDC would be required. Discussions were had that a site specific elastic analysis using the 475 year seismic loads that is controlled by the deflection limits of the cross lot bracing would be necessary. Please confirm that a "push over" type analysis of SDC will not be required for the trestle and that the attached detailed Design Criteria (and analysis method) is acceptable.			As noted in the 11/16/11 meeting, the cross lot bracing "struts" are supported by the Trestle substructure and analysis requires limiting trestle deformations to be compatible with the allowable strut deflections (approximately 2"). As a result the "push over" analysis as required by the AASHTO Seismic Design Criteria "SDC" (requirement of bridge spec 01 53 13) is not applicable. This was discussed in detail during the 11/16 meeting and it was concurred that due to unique structural configuration and deflection requirements, an alternate analysis method other than the SDC would be required. Discussions were had that a site specific elastic analysis using the 475 year seismic loads that is controlled by the deflection limits of the cross lot bracing would be necessary. Please confirm that a "push over" type analysis of SDC will not be required for the trestle and that the attached detailed Design Criteria (and analysis method) is acceptable.			
(W/O added clarification) BBII believes the site specific analysis would demonstrate the trestle substructure will not deform greater than 2" however the trestle superstructure will deform greater than 2".			(W/O added clarification) BBII believes the site specific analysis would demonstrate the trestle substructure will not deform greater than 2" however the trestle superstructure will deform greater than 2".			



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<b>T-0254</b>	<b>BSE - Modified CDSM Installation Plan for Verizon Lines at First St.</b>	<b>Closed</b>	<b>01</b>	<b>10/20/2011</b>	<b>10/30/2011</b>	<b>11/01/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Specification Section 31 56 13 and attached sketches from PMPC  W/O received the modified CDSM Installation plan for Verizon lines at First St. without the relocation of the lines from PMPC as the attached. Please confirm the plan is acceptable for CDSM Shoring Wall Designer (ARUP).						<b>ANSWER:</b>  Reference Specification Section 31 56 13 and attached sketches from PMPC  W/O received the modified CDSM Installation plan for Verizon lines at First St. without the relocation of the lines from PMPC as the attached. Please confirm the plan is acceptable for CDSM Shoring Wall Designer (ARUP).
<b>T-0255</b>	<b>BSE - Verizon Spacing Requirement on First Street (Phase 2 Utility Installation)</b>	<b>Closed</b>	<b>01</b>	<b>10/21/2011</b>	<b>10/31/2011</b>	<b>10/31/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference  BBII have commenced the PG&E Phase 2 installation on First Street, in order to co-ordinate the PG&E utility locations and the future Verizon phase 2 utility indicated on the attached drawing. The attached drawing was issue to BBII in the field, please confirm this drawing has been co-ordinated with the PG&E construction drawings.  BBII require the following: - Provide a profile/section drawing indicating accurate clearances between PG&E and Verizon, - Include (Verizon) Trench dimensions, on First Street for the phase 2 installation. - Site meeting with Verizon representative to discuss Verizon configuration.						<b>ANSWER:</b>  Reference  BBII have commenced the PG&E Phase 2 installation on First Street, in order to co-ordinate the PG&E utility locations and the future Verizon phase 2 utility indicated on the attached drawing. The attached drawing was issue to BBII in the field, please confirm this drawing has been co-ordinated with the PG&E construction drawings.  BBII require the following: - Provide a profile/section drawing indicating accurate clearances between PG&E and Verizon, - Include (Verizon) Trench dimensions, on First Street for the phase 2 installation. - Site meeting with Verizon representative to discuss Verizon configuration.
<b>T-0256</b>	<b>BSE - CR T-018 Design Omissions</b>	<b>Closed</b>	<b>01</b>	<b>10/21/2011</b>	<b>10/31/2011</b>	<b>11/03/2011</b>
<b>From:</b> Webcor Construction LP      Masashi Kojima						
<b>REQUEST:</b>  Reference CR T-018  Neither the original albeit incomplete CR T-018 dated 9/21/11 or the flurry of subsequent email clarifications furnished the following design omissions required to						<b>ANSWER:</b>  Reference CR T-018  Neither the original albeit incomplete CR T-018 dated 9/21/11 or the flurry of subsequent email clarifications furnished the following design omissions required to



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	<div><div>complete the CR T-018: 1. Emergency egress signage requirements? 2. Lighting: Location, lumen, schedule, and if emergency lighting is required? 3. Gates &amp; crash bar requirements? 4. Although the driveway design was not provided until 10/20/11, no dimensions were provided and there are proximity conflict(s) with the fire hydrant relative to the vent &amp; DI.  Please provide and or remove from scope so the contractor may complete the work.</div></div>					
	<div><div>complete the CR T-018: 1. Emergency egress signage requirements? 2. Lighting: Location, lumen, schedule, and if emergency lighting is required? 3. Gates &amp; crash bar requirements? 4. Although the driveway design was not provided until 10/20/11, no dimensions were provided and there are proximity conflict(s) with the fire hydrant relative to the vent &amp; DI.  Please provide and or remove from scope so the contractor may complete the work.</div></div>					
T-0257	BSE - Request to Sonic Caliper 20 feet from Projected Bottom of Rock Socket	Closed	01	10/24/2011	11/03/2011	10/31/2011
	<div><div>From: Webcor Construction LP</div><div>Nhi Tran</div><div>REQUEST:  Please address the following information request from BBII's sub contractor Becho Inc.:  "... Becho would like to start performing Sonic Caliper analyses within 20 feet of the projected final bottom elevation of the shaft(s) to expedite the "Drill, Place, Pour" process. In order to continue the Buttress Drilling Operation without interruptions, Becho would like to utilize the hours between 1am - 6am to perform the Sonic Caliper test. For example, if Becho anticipates the completion of shaft at 10am, it would be beneficial to perform the Sonic Caliper test during the hours of 1am - 6am. This allows crews to prep, setup and perform the airlift process without having to wait for Becho engineers to test the shaft(s) during normal hours of operation, thus expediting the "Drill, Place, Pour" process.  Please advise, if it is acceptable.</div></div>					
	<div><div>ANSWER:  Please address the following information request from BBII's sub contractor Becho Inc.:  "... Becho would like to start performing Sonic Caliper analyses within 20 feet of the projected final bottom elevation of the shaft(s) to expedite the "Drill, Place, Pour" process. In order to continue the Buttress Drilling Operation without interruptions, Becho would like to utilize the hours between 1am - 6am to perform the Sonic Caliper test. For example, if Becho anticipates the completion of shaft at 10am, it would be beneficial to perform the Sonic Caliper test during the hours of 1am - 6am. This allows crews to prep, setup and perform the airlift process without having to wait for Becho engineers to test the shaft(s) during normal hours of operation, thus expediting the "Drill, Place, Pour" process.  Please advise, if it is acceptable.</div></div>					
T-0258	BSE - Demolition Status of Pile Cap at GL 33.5	Closed	01	10/27/2011	11/06/2011	12/09/2011
	<div><div>From: Webcor Construction LP</div><div>Nhi Tran</div><div>REQUEST:</div></div>					
	<div><div>ANSWER:</div></div>					





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<b>T-0260</b>	<b>BSE - D.I. Installation at Natoma Street and First Street</b>	<b>Closed</b>	<b>01</b>	<b>11/01/2011</b>	<b>11/11/2011</b>	<b>11/08/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Sheet U-3012 and attached sketch			Reference Sheet U-3012 and attached sketch			
BBII carried out an investigation of the active catch basin around the perimeter of the BSE project; and has a concern regarding the street elevation relative to the flow line on Natoma Street between GL 10-17.			BBII carried out an investigation of the active catch basin around the perimeter of the BSE project; and has a concern regarding the street elevation relative to the flow line on Natoma Street between GL 10-17.			
The flow line directs surface water in a North East direction towards First Street. The only active catch basin at the intersection of Natoma and First Street is CB #305, which is approximately +8.5" higher than the currently decommissioned CB located at the intersection of Natoma St and First St (see sketch attached).			The flow line directs surface water in a North East direction towards First Street. The only active catch basin at the intersection of Natoma and First Street is CB #305, which is approximately +8.5" higher than the currently decommissioned CB located at the intersection of Natoma St and First St (see sketch attached).			
Noted during the last rain fall, surface water was directed to the decommissioned catch basin at the North East corner of Natoma Street and First Street intersection, BBII recorded approximately 6" of standing rain water accumulating at First Street and Natoma intersection. Please note that existing catch basin was decommissioned during the new sewer installation on First Street (see attached mark up drawing).			Noted during the last rain fall, surface water was directed to the decommissioned catch basin at the North East corner of Natoma Street and First Street intersection, BBII recorded approximately 6" of standing rain water accumulating at First Street and Natoma intersection. Please note that existing catch basin was decommissioned during the new sewer installation on First Street (see attached mark up drawing).			
BBII recommends 2 options to control rain water from outside the BSE work area: A) modify the flow line on Natoma Street to direct the flow toward CB # 305, B) Install a new catch basin and connect it to the existing lateral connection CB # 305 to the combine sewer system, or connect directly to the existing MH.			BBII recommends 2 options to control rain water from outside the BSE work area: A) modify the flow line on Natoma Street to direct the flow toward CB # 305, B) Install a new catch basin and connect it to the existing lateral connection CB # 305 to the combine sewer system, or connect directly to the existing MH.			
Please advise on TJPA method to prevent water collecting on First Street.			Please advise on TJPA method to prevent water collecting on First Street.			





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<b>T-0262</b>	<b>BSE - CAD File for trestle/pin pile exclusion zones</b>	<b>Closed</b>	<b>01</b>	<b>11/09/2011</b>	<b>11/19/2011</b>	<b>11/17/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference RFI#T-0251.1 and Specification Section 01 53 13  The response to RFI T-0251.1 included a set of sketches showing hatched "exclusion zones" where trestle/pin pile placement is not allowed. Please provide the CAD file for these sketches for BBII use in coordinating pile locations.		<b>ANSWER:</b>  Reference RFI#T-0251.1 and Specification Section 01 53 13  The response to RFI T-0251.1 included a set of sketches showing hatched "exclusion zones" where trestle/pin pile placement is not allowed. Please provide the CAD file for these sketches for BBII use in coordinating pile locations.				
<b>T-0262.1</b>	<b>BSE - CAD File for Micropile Exclusion Zones</b>	<b>Closed</b>	<b>01</b>	<b>05/17/2012</b>	<b>05/27/2012</b>	<b>05/29/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>  Reference: Specification 31 63 33 RFI T-0262  Please provide the CAD file for Micropile "Exclusion Zones," if they differ from the exclusion zones subjected to RFI # T-262.		<b>ANSWER:</b>  Reference: Specification 31 63 33 RFI T-0262  Please provide the CAD file for Micropile "Exclusion Zones," if they differ from the exclusion zones subjected to RFI # T-262.				
<b>T-0263</b>	<b>BSE - Strut Conflicts to Thornton Tomasetti's comments on the approved Internal</b>	<b>Closed</b>	<b>01</b>	<b>11/09/2011</b>	<b>11/19/2011</b>	<b>11/17/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference RFI #T-0251.1 and Transmittal No. 140-02329  Subsequent to W/O's receipt of an approved 100% internal bracing submittal and procurement, Thornton Tomasetti's comments in the plans transmitted via Transmittal #140-02329 added both columns & dimensions and revised column configurations relative to the location of the internal bracing struts not otherwise included in the base contract BSE documents. So as W/O may accurately coordinate strut locations in order to mitigate conflicts, please provide the minimum allowable dimension from column to strut.		<b>ANSWER:</b>  Reference RFI #T-0251.1 and Transmittal No. 140-02329  Subsequent to W/O's receipt of an approved 100% internal bracing submittal and procurement, Thornton Tomasetti's comments in the plans transmitted via Transmittal #140-02329 added both columns & dimensions and revised column configurations relative to the location of the internal bracing struts not otherwise included in the base contract BSE documents. So as W/O may accurately coordinate strut locations in order to mitigate conflicts, please provide the minimum allowable dimension from column to strut.				



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<b>T-0264</b>	<b>BSE - Bridge / Trestle Piles in Exclusion Zones</b>	<b>Closed</b>	<b>01</b>	<b>11/09/2011</b>	<b>11/19/2011</b>	<b>11/18/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference RFI#T-0251.1 and Specification Section 01 53 13  BBII is in receipt of the drawings included in RFI T-251.1 that illustrate trestle pile "exclusion zones" where piles cannot penetrate the mat slab. Of the 24 piles that are currently in conflict with the pile exclusion zones, 20 of them can be relocated with relatively minor member changes. The other 4 as indicated in the attached drawings will require significant redesign and re-procurement, especially at the bridges. Can an exception be made at these four locations?						<b>ANSWER:</b>  Reference RFI#T-0251.1 and Specification Section 01 53 13  BBII is in receipt of the drawings included in RFI T-251.1 that illustrate trestle pile "exclusion zones" where piles cannot penetrate the mat slab. Of the 24 piles that are currently in conflict with the pile exclusion zones, 20 of them can be relocated with relatively minor member changes. The other 4 as indicated in the attached drawings will require significant redesign and re-procurement, especially at the bridges. Can an exception be made at these four locations?
<b>T-0264.1</b>	<b>BSE - Beale St Bridge Pile Conflict (Follow up to RFI T-264)</b>	<b>Closed</b>	<b>01</b>	<b>01/26/2012</b>	<b>02/05/2012</b>	<b>02/03/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Shad Gardner						
<b>REQUEST:</b>  Reference: BBI Marked-Up SKS-0135, SH-3103  The previous response to RFI T-264 requested BBII move one of the Beale St. Bridge piles 3' west to avoid mat slab reinforcing congestion. BBII has investigated this request and found that the cap beam already has a significant cantilever on the east side of the pile in question. In order to comply with the request to move the pile, we would have to extend the cap beam and support it off the CDSM wall as shown on the attached sketch. Please advise if this is acceptable, otherwise the pile will need to remain in its current position.						<b>ANSWER:</b>  Reference: BBI Marked-Up SKS-0135, SH-3103  The previous response to RFI T-264 requested BBII move one of the Beale St. Bridge piles 3' west to avoid mat slab reinforcing congestion. BBII has investigated this request and found that the cap beam already has a significant cantilever on the east side of the pile in question. In order to comply with the request to move the pile, we would have to extend the cap beam and support it off the CDSM wall as shown on the attached sketch. Please advise if this is acceptable, otherwise the pile will need to remain in its current position.
<b>T-0264.2</b>	<b>Beale St Bridge Pile Conflict (Follow up to RFI T-264.1)</b>	<b>Closed</b>	<b>CR</b>	<b>02/08/2012</b>	<b>02/18/2012</b>	<b>02/16/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Shad Gardner						
<b>REQUEST:</b>  The response to RFI T-264.1 requested BBII provide the loading that would placed onto the CDSM wall. This response leads us to believe that the option to leave the pile in the current location was unacceptable. Please confirm that the pile must be moved and provide a						<b>ANSWER:</b>  The response to RFI T-264.1 requested BBII provide the loading that would placed onto the CDSM wall. This response leads us to believe that the option to leave the pile in the current location was unacceptable.











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	<p>Per TCCO Request RFI being submitted in lieu of a submittal:</p> <p>Based on the response to Webcor Submittal No. TG0300-206.1, BBII has shifted the bridge superstructure west between the grid lines 34 and 34.8 beams as directed. This necessitates the installation of 2 rows of 5 bridge columns as shown in the attached drawings. The west row will be located 7' east of GL 34 and the east row will be located a further 25' east as shown. All 10 columns have been positioned clear of the internal bracing. The sidewalk will be located in Lot N.</p> <p>There are two options for the location of east bridge column 3 as shown in the attached layout drawing.</p> <p>- Option 1 is the preferred option. This is located on E line 10'-6" west of Grid line 35 (Pile exclusion zone penetration approved via response to RFI 264.3).</p> <p>- Option 2 is located a further 5' west of option 1 to the location on the TG-06 drawing. The impacts of option 2 to the superstructure are not known at this time. The irregular alignment of the eastern row of piles in option 2 will create local stress concentration in both the diaphragm and superstructure in the longitudinal seismic analysis. This is not a preferable configuration.</p> <p>Please confirm the location of the superstructure and the piles. Advise on the location of east bridge pile 3.</p>					
	<p>Per TCCO Request RFI being submitted in lieu of a submittal:</p> <p>Based on the response to Webcor Submittal No. TG0300-206.1, BBII has shifted the bridge superstructure west between the grid lines 34 and 34.8 beams as directed. This necessitates the installation of 2 rows of 5 bridge columns as shown in the attached drawings. The west row will be located 7' east of GL 34 and the east row will be located a further 25' east as shown. All 10 columns have been positioned clear of the internal bracing. The sidewalk will be located in Lot N.</p> <p>There are two options for the location of east bridge column 3 as shown in the attached layout drawing.</p> <p>- Option 1 is the preferred option. This is located on E line 10'-6" west of Grid line 35 (Pile exclusion zone penetration approved via response to RFI 264.3).</p> <p>- Option 2 is located a further 5' west of option 1 to the location on the TG-06 drawing. The impacts of option 2 to the superstructure are not known at this time. The irregular alignment of the eastern row of piles in option 2 will create local stress concentration in both the diaphragm and superstructure in the longitudinal seismic analysis. This is not a preferable configuration.</p> <p>Please confirm the location of the superstructure and the piles. Advise on the location of east bridge pile 3.</p>					
T-0265	BSE - TG03 BSE CDSM Cut-off Wall	Closed	01	11/09/2011	11/19/2011	11/17/2011
	From: Webcor Construction LP					
	Nhi Tran					
	REQUEST:					
	Reference Drawings GT-2102, GT-2103, QBD TG0300-0098					
	Balfour Beatty Infrastructure, Inc. (BBII) is planning to start dewatering and excavation without installing cut-off walls and sectionalized dewatering. According to the response					
	ANSWER:					
	Reference Drawings GT-2102, GT-2103, QBD TG0300-0098					
	Balfour Beatty Infrastructure, Inc. (BBII) is planning to start dewatering and excavation without installing cut-off walls and sectionalized dewatering. According to					





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	for QBD TG0300-0098, BBII can eliminate cut-off walls as their means and methods although contract drawings/specifications indicate cut-off walls. Please confirm.					the response for QBD TG0300-0098, BBII can eliminate cut-off walls as their means and methods although contract drawings/specifications indicate cut-off walls. Please confirm.
T-0266	BSE - Moratorium Conflict With Phase 2 Utilities In 1st Street	Closed	01	11/23/2011	11/23/2011	12/06/2011
	From: Webcor Construction LP Manuel Saldana					
	REQUEST: BBII is in receipt of the moratorium waiver expire date of 12-09-2011. BBII/PEC will not be able to complete the Phase II utility work by 12/9/11 without accelerating the schedule. Our original request for extension was December 19, 2011. A 12/9/11 completion date may be achievable if PEC is allowed to work 10 hr shifts during the day beginning 11/28 through 12/2 as well as working on 12/3 and 12/4. In addition, we propose to have a separate night crew to work near / around the Minna Street intersection to alleviate impacts to heavy demand of day traffic. The majority, if not all, of the demolition can occur during the dday to mitigate noise at night. The night work would need to begin on 11/28 and run through 12/2. Please keep in mind that implenting an accelerated schedule may also impact PG&E. We have no control over their work and the completion of the utlity tie-ins and Mandral testing is contingent on PG&E's availability per the new adjusted completion date.  In summary we are requesting direction for the following items to meet the 12/9/11 moratorium deadline: 1) W/O to permit BBII / PEC to work the extended hours, and night shift i.e. 10 Hours Days and Night work operations, 2) Permit from MTA to extend working hours (closure times) during the day 3) Permit from MTA and DPW to work at night within lane closures 4) Permit from TJPA to work in Zones 1 & 2 at night 5) Agreement / Approval for compensation of additional cost (premium time and or shift rate) BBII will have magnitude of cost for the Monday morning discussion					ANSWER: BBII is in receipt of the moratorium waiver expire date of 12-09-2011. BBII/PEC will not be able to complete the Phase II utility work by 12/9/11 without accelerating the schedule. Our original request for extension was December 19, 2011. A 12/9/11 completion date may be achievable if PEC is allowed to work 10 hr shifts during the day beginning 11/28 through 12/2 as well as working on 12/3 and 12/4. In addition, we propose to have a separate night crew to work near / around the Minna Street intersection to alleviate impacts to heavy demand of day traffic. The majority, if not all, of the demolition can occur during the dday to mitigate noise at night. The night work would need to begin on 11/28 and run through 12/2. Please keep in mind that implenting an accelerated schedule may also impact PG&E. We have no control over their work and the completion of the utility tie-ins and Mandral testing is contingent on PG&E's availability per the new adjusted completion date.  In summary we are requesting direction for the following items to meet the 12/9/11 moratorium deadline: 1) W/O to permit BBII / PEC to work the extended hours, and night shift i.e. 10 Hours Days and Night work operations, 2) Permit from MTA to extend working hours (closure times) during the day 3) Permit from MTA and DPW to work at night within lane closures 4) Permit from TJPA to work in Zones 1 & 2 at night 5) Agreement / Approval for compensation of additional cost (premium time and or shift rate) BBII



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	We respectively request a meeting with W/O on Monday morning (11-28-2011) to discuss direction regarding the above items.				will have magnitude of cost for the Monday morning discussion	
					We respectively request a meeting with W/O on Monday morning (11-28-2011) to discuss direction regarding the above items.	
T-0269	BSE - Mass Excavation Pile Extraction Clarification	Closed	01	12/13/2011	12/23/2011	12/27/2011
	From: Webcor Construction LP                      David Fields					
	REQUEST:				ANSWER:	
	Reference: 31 00 00 1.4 C.2 and Attached Sketch				Reference: 31 00 00 1.4 C.2 and Attached Sketch	
	31 00 00 1.4.C.2 Pile Extraction - To occur in two (2) stages per Zone. Stage 1 extraction will remove the piles within the footprint of the trestle the middle 60' of the work zone, dewatering wells and piles that are in conflict with the bracing pin pile locations. Piles will be removed using a non ground deformation control method and be removed full length to be utilized for offsite LEED projects and to help achieve sustainability for this material. Trestle piles will be installed after Stage 1 pile extraction and concurrently with Stage 2 pile extraction.				31 00 00 1.4.C.2 Pile Extraction - To occur in two (2) stages per Zone. Stage 1 extraction will remove the piles within the footprint of the trestle the middle 60' of the work zone, dewatering wells and piles that are in conflict with the bracing pin pile locations. Piles will be removed using a non ground deformation control method and be removed full length to be utilized for offsite LEED projects and to help achieve sustainability for this material. Trestle piles will be installed after Stage 1 pile extraction and concurrently with Stage 2 pile extraction.	
	Stage 2 extraction will remove the piles within the 50' +- area adjacent to the CDSM walls along A and J lines. Piles will be extracted using a ground deformation control method as per Section 02 41 19 - 3.1.B of the specifications utilizing both casing and backfilling of the void or removal by means of cutting the pile off at the grade of each level of excavation as the work proceeds. Please reference the attached drawing for details of the above procedure.				Stage 2 extraction will remove the piles within the 50' +- area adjacent to the CDSM walls along A and J lines. Piles will be extracted using a ground deformation control method as per Section 02 41 19 - 3.1.B of the specifications utilizing both casing and backfilling of the void or removal by means of cutting the pile off at the grade of each level of excavation as the work proceeds. Please reference the attached drawing for details of the above procedure.	
	The 80 Natoma shoring wall will be removed in stages coinciding with the stages of excavation.				The 80 Natoma shoring wall will be removed in stages coinciding with the stages of excavation.	
	Please confirm this method of pile extraction during mass excavation is acceptable.				Please confirm this method of pile extraction during mass excavation is acceptable.	





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<b>T-0269.2</b>	<b>BSE - Zone 2 Free Pull Pile Extraction Test Section</b>	<b>Closed</b>	<b>01</b>	<b>05/01/2012</b>	<b>05/11/2012</b>	<b>05/04/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: BBII 4/30/12 Ground Deformation Control Drawing			Reference: BBII 4/30/12 Ground Deformation Control Drawing			
BBII are proposing to perform "free pull" pile extraction on a "test section" in Zone 2. The proposed piles will be extracted near GL14, close to CDSM wall on the north side using a "non ground deformation control method" by free pulling each pile without using steel casing. Inclinator (I-011) located close to GL 14 will be monitored during the test. This test section will give the information needed to determine: 1) If free pulling the piles using a non ground deformation control method affects the CDSM wall by causing movement. 2) If it is a suitable method to adopt for removing the remainder of the piles in Zone 2 located outside the "trestle area".			BBII are proposing to perform "free pull" pile extraction on a "test section" in Zone 2. The proposed piles will be extracted near GL14, close to CDSM wall on the north side using a "non ground deformation control method" by free pulling each pile without using steel casing. Inclinator (I-011) located close to GL 14 will be monitored during the test. This test section will give the information needed to determine: 1) If free pulling the piles using a non ground deformation control method affects the CDSM wall by causing movement. 2) If it is a suitable method to adopt for removing the remainder of the piles in Zone 2 located outside the "trestle area".			
The attached drawing conveys the test section in green. Please advise on the suitability of this test to determine if free pulling can be used outside the trestle zone.			The attached drawing conveys the test section in green. Please advise on the suitability of this test to determine if free pulling can be used outside the trestle zone.			
<b>T-0269.3</b>	<b>BSE - Zone 2 Pile Extraction Test Section</b>	<b>Closed</b>	<b>01</b>	<b>06/15/2012</b>	<b>06/25/2012</b>	<b>06/21/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
BBII completed the timber pile extraction test section in zone 2 on 06/12/2012. Based on the data recorded by ARUP inclinometers, please advise if BBII can continue with the timber pile extraction in Zone 2 using non ground deformation control methods ("free pull").			BBII completed the timber pile extraction test section in zone 2 on 06/12/2012. Based on the data recorded by ARUP inclinometers, please advise if BBII can continue with the timber pile extraction in Zone 2 using non ground deformation control methods ("free pull").			
<b>T-0269.4</b>	<b>BSE Zones 3/4 Pile Extraction Methodology</b>	<b>Closed</b>	<b>01</b>	<b>09/27/2012</b>	<b>10/07/2012</b>	<b>10/05/2012</b>
<b>From:</b> Webcor Construction LP      Kirk Nielsen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please confirm ARUP's 9/25/12 verbal revision to RFI response T-0269.3, to employ the originally specified ground deformation control method (not free pull) when pulling timber piles between: Soldier piles 251 and 276 &			Please confirm ARUP's 9/25/12 verbal revision to RFI response T-0269.3, to employ the originally specified ground deformation control method (not free pull) when pulling timber piles between: Soldier piles 251			



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<b>T-0269.7</b>	<b>BSE - Timber pile extraction method in the footprint of the Zone-4 trestle</b>	<b>Closed</b>	<b>CR</b>	<b>04/11/2013</b>	<b>04/21/2013</b>	<b>04/16/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>  Ref: GT-2102, GT-2103  Please confirm ARUP's 4/10/13 verbal comment that the contractor may the use non-ground deformation method (free pulling) for Zone-4 timber piles in the footprint of the trestle.						<b>ANSWER:</b>  Ref: GT-2102, GT-2103  Please confirm ARUP's 4/10/13 verbal comment that the contractor may the use non-ground deformation method (free pulling) for Zone-4 timber piles in the footprint of the trestle.
<b>T-0270</b>	<b>BSE - Clarification for Existing Ground Water Elevation</b>	<b>Closed</b>	<b>CR</b>	<b>12/28/2011</b>	<b>01/07/2012</b>	<b>12/30/2011</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>  Reference: 31-23-29 and Attached Document  As discussed during the meeting on 12/22/11, to help obtain an accurate dewatering model, BBII is requesting the recent piezometer data for Zones 1 and 2. In addition, BBII has reviewed the data for piezometers 1182, 1229 and 1255 located adjacent to 301 Mission St (see attachment) and would like to clarify the initial ground water level to use in the model for Zone 4. Based on our review, the existing natural groundwater condition fluctuates between 1.6 E.L and -8.1 E.L in this area. BBII would like to agree upon a starting groundwater elevation of -5.0 E.L for Zone 4. Also, BBII would like clarification as to the base groundwater level to use for Zones 1, 2 and 3 based on the project data.						<b>ANSWER:</b>  Reference: 31-23-29 and Attached Document  As discussed during the meeting on 12/22/11, to help obtain an accurate dewatering model, BBII is requesting the recent piezometer data for Zones 1 and 2. In addition, BBII has reviewed the data for piezometers 1182, 1229 and 1255 located adjacent to 301 Mission St (see attachment) and would like to clarify the initial ground water level to use in the model for Zone 4. Based on our review, the existing natural groundwater condition fluctuates between 1.6 E.L and -8.1 E.L in this area. BBII would like to agree upon a starting groundwater elevation of -5.0 E.L for Zone 4. Also, BBII would like clarification as to the base groundwater level to use for Zones 1, 2 and 3 based on the project data.
<b>T-0271</b>	<b>BSE - CRT-021 Gate Fence Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>01/05/2012</b>	<b>01/05/2012</b>	<b>01/10/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>  In regards to the Proposed Driveway shown on the CRT#021 drawing and outlined in Bullets #1 and #2 in the Scope of Work, please clarify the following:  -Per the location of the 18ft Gate, a 10ft fence would need to be constructed to connect the existing 9ft tall fence to the Proposed Driveway gate location (see 1/4/12 Photo						<b>ANSWER:</b>  In regards to the Proposed Driveway shown on the CRT#021 drawing and outlined in Bullets #1 and #2 in the Scope of Work, please clarify the following:  -Per the location of the 18ft Gate, a 10ft fence would need to be constructed to connect the existing 9ft tall fence to the Proposed Driveway gate location (see



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	<p>attached). Please confirm the 10ft fence should be included in this CRT-021. -Should the 24'-10" section of the existing 6ft tall fence (see 1/4/12 Photo attached) be replaced?</p> <p>Confirm Howard St shown on the CRT#021 attached drawing should read "Folsom St"</p> <p>Confirm that Bullet #3 under the "Scope of Work" refers to Gate #1 in the CRT#021 attached drawing.</p>					<p>1/4/12 Photo attached). Please confirm the 10ft fence should be included in this CRT-021. -Should the 24'-10" section of the existing 6ft tall fence (see 1/4/12 Photo attached) be replaced?</p> <p>Confirm Howard St shown on the CRT#021 attached drawing should read "Folsom St"</p> <p>Confirm that Bullet #3 under the "Scope of Work" refers to Gate #1 in the CRT#021 attached drawing.</p>
<b>T-0272</b>	<b>BSE - D1 Casing Recovery Inquiries</b>	<b>Closed</b>	<b>01</b>	<b>01/27/2012</b>	<b>02/02/2012</b>	<b>01/27/2012</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b> BBII is requesting the following to complete its D1 casing retrieval plan: 1. Condition specific engineering calculations to mitigate earth and water heave from the bottom of the casing. 2. Condition specific engineering calculations to substantiate no casing buckling. 3. Condition specific plan engineering calculations for dewatering, specifically expected water quantity.  Note - This RFI is high priority and an expedited review/response is necessary.						<b>ANSWER:</b> BBII is requesting the following to complete its D1 casing retrieval plan: 1. Condition specific engineering calculations to mitigate earth and water heave from the bottom of the casing. 2. Condition specific engineering calculations to substantiate no casing buckling. 3. Condition specific plan engineering calculations for dewatering, specifically expected water quantity.  Note - This RFI is high priority and an expedited review/response is necessary.





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<b>T-0272.1</b>	<b>BSE - D1 Casing Recovery Inquiries</b>	<b>Closed</b>	<b>CR</b>	<b>01/27/2012</b>	<b>02/06/2012</b>	<b>01/27/2012</b>
<b>From:</b> Webcor Construction LP      Kirk Nielsen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
BBII is requesting the following to complete its D1 casing retrieval plan: 1. Condition specific engineering calculations to mitigate earth and water heave from the bottom of the casing. 2. Condition specific engineering calculations to substantiate no casing buckling. 3. Condition specific plan engineering calculations for dewatering, specifically expected water quantity.  Note - This RFI is high priority and an expedited review/response is necessary.			BBII is requesting the following to complete its D1 casing retrieval plan: 1. Condition specific engineering calculations to mitigate earth and water heave from the bottom of the casing. 2. Condition specific engineering calculations to substantiate no casing buckling. 3. Condition specific plan engineering calculations for dewatering, specifically expected water quantity.  Note - This RFI is high priority and an expedited review/response is necessary.			
<b>T-0273</b>	<b>BSE - Clarification for Driveway Desgin at 540 Howard CR -018R2</b>	<b>Closed</b>	<b>01</b>	<b>01/30/2012</b>	<b>02/09/2012</b>	<b>02/06/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Attached BBI Sketch CRT-018RI directs BBII to complete a 12ft driveway at the 540 Howard Street. The existing conditions/location of the curb, USPS facilities and water fire hydrant prevents the driveway from being installed within compliance with the DPW and ADA standards. DPW/Tumer/W/0 and BBII discussed various solutions to bring the driveway into confrmance with ADA and DPW standards at the field meeting held on January 17th 2012 and again 01/24//2012. Pursuant to the field meeting and direction of CRT-018R2, BBII is requesting detailed plans to allow for construction of a compliant driveway at 540 Howard Street. BBII has been directed in the field by W /0/Tumer, to complete modification to the driveway at 540 Howard Street. Per our field meeting please refer to the attached drawing, indicating BBII understanding on the modifications required. Please confirm the modification per the attached drawing is compliant with City and ADA driveway standards.			Reference: Attached BBI Sketch CRT-018RI directs BBII to complete a 12ft driveway at the 540 Howard Street. The existing conditions/location of the curb, USPS facilities and water fire hydrant prevents the driveway from being installed within compliance with the DPW and ADA standards. DPW/Tumer/W/0 and BBII discussed various solutions to bring the driveway into confrmance with ADA and DPW standards at the field meeting held on January 17th 2012 and again 01/24//2012. Pursuant to the field meeting and direction of CRT-018R2, BBII is requesting detailed plans to allow for construction of a compliant driveway at 540 Howard Street. BBII has been directed in the field by W /0/Tumer, to complete modification to the driveway at 540 Howard Street. Per our field meeting please refer to the attached drawing, indicating BBII understanding on the modifications required. Please confirm the modification per the attached drawing is compliant with City and ADA driveway standards.			







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T-0276	<p>BSE - Request to Change Buttress Concrete Slump Requirements</p> <p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Emre Erzen</p> <p><b>REQUEST:</b></p> <p>Reference: 31 63 29</p> <p>Currently, the primary and the secondary shafts utilize a superplasticizer to achieve slump as the water content of the mixes is low. Typically, mixes that utilize a superplasticizer are intended for slump ranges between 9" and 12," however, project specifications require an 8" +/- 1" slump. Unfortunately, the addition of the superplasticizer has made it difficult to achieve slump as specified. BBII and Central Concrete are requesting an 8" + 1" - 2" slump (giving a range of 6" to 9") in lieu of the specified 8" +/- 1". There will be no adverse effect to the strength as slump is achieved through chemical admixtures and not by adding water. Please advise.</p>	Closed	01	02/16/2012	02/26/2012	02/17/2012
	<p><b>ANSWER:</b></p> <p>Reference: 31 63 29</p> <p>Currently, the primary and the secondary shafts utilize a superplasticizer to achieve slump as the water content of the mixes is low. Typically, mixes that utilize a superplasticizer are intended for slump ranges between 9" and 12," however, project specifications require an 8" +/- 1" slump. Unfortunately, the addition of the superplasticizer has made it difficult to achieve slump as specified. BBII and Central Concrete are requesting an 8" + 1" - 2" slump (giving a range of 6" to 9") in lieu of the specified 8" +/- 1". There will be no adverse effect to the strength as slump is achieved through chemical admixtures and not by adding water. Please advise.</p>					



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<b>T-0277</b>	<b>BSE - Request for Buttress Shaft Design Documentation</b>	<b>Closed</b>	<b>CR</b>	<b>02/16/2012</b>	<b>02/26/2012</b>	<b>02/23/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Emre Erzen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please address the following information request from BBII's sub contractor Becho Inc.: " ... Becho requests to obtain all and any documentation used in the design of the Buttress Shafts. Documentation should include, but is not limited to, submitted and approved calculations, sketches, preliminary designs and calculations, conceptual drawings, all site investigation, and all other work documents and -work papers that were utilized to develop the buttress shaft design in addition to -what's provided in the contract documents and specifications. "  Please advise, if it is acceptable.			Please address the following information request from BBII's sub contractor Becho Inc.: " ... Becho requests to obtain all and any documentation used in the design of the Buttress Shafts. Documentation should include, but is not limited to, submitted and approved calculations, sketches, preliminary designs and calculations, conceptual drawings, all site investigation, and all other work documents and -work papers that were utilized to develop the buttress shaft design in addition to -what's provided in the contract documents and specifications. "  Please advise, if it is acceptable.			
<b>T-0277.1</b>	<b>BSE - Becho's 2nd Request for Buttress Design Doc</b>	<b>Closed</b>	<b>CR</b>	<b>03/23/2012</b>	<b>04/02/2012</b>	<b>03/28/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Becho requests to obtain all work documents, sketches, preliminary calculations and approved calculations which show how the designer arrived the final skin friction values used in the design of the buttress shafts as well as the buttress shafts minimum 10 feet embedment into bedrock.			Becho requests to obtain all work documents, sketches, preliminary calculations and approved calculations which show how the designer arrived the final skin friction values used in the design of the buttress shafts as well as the buttress shafts minimum 10 feet embedment into bedrock.			
<b>T-0277.2</b>	<b>BSE - Request for Buttress Shaft Design Documentation</b>	<b>Closed</b>	<b>01</b>	<b>04/04/2012</b>	<b>04/14/2012</b>	<b>04/11/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per the agreement at the 4/4/12 TCCO Progress Meeting BSE Buttress Shoring and Excavation please find Becho's Request for additional design documentation below:  Becho is in receipt of RFI # T-0277.1 regarding the Buttress Shaft Design Documentation. As per the TJPA response, Becho more specifically requests the Reference Shoring Design work documents pertinent to zone 4.			Per the agreement at the 4/4/12 TCCO Progress Meeting BSE Buttress Shoring and Excavation please find Becho's Request for additional design documentation below:  Becho is in receipt of RFI # T-0277.1 regarding the Buttress Shaft Design Documentation. As per the TJPA response, Becho more specifically requests the Reference Shoring Design work documents pertinent to zone 4.			



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<b>T-0278</b>	<b>BSE - Access Trestle Bump Out Coordination</b>	<b>Closed</b>	<b>01</b>	<b>02/16/2012</b>	<b>02/26/2012</b>	<b>02/24/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Attached BII Sketch Due to the deletion of the "Natoma Finger" portion of the access trustle BBII is proposing to install additional "bump outs" (per the attached sketch). For coordination purposes, please provide "no fly" zone information for these locations.			Reference: Attached BII Sketch Due to the deletion of the "Natoma Finger" portion of the access trustle BBII is proposing to install additional "bump outs" (per the attached sketch). For coordination purposes, please provide "no fly" zone information for these locations.			



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T-0279	BSE - Trestle Welding Code Compatibility	Closed	01	02/27/2012	03/08/2012	03/20/2012
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Shad Gardner						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: ASHTO/AWSS D1.5M/D1.5:2008 SH-0200		Reference: ASHTO/AWSS D1.5M/D1.5:2008 SH-0200				
The Temporary Access Trestle Design submitted in December specified AWS 01.1 as the required welding code. During the review process the reviewers requested that the welding code be changed to AWS 01.5- Bridge Welding Code. This request was complied with by revising general note A5.2 on the conformed trestle drawings.		The Temporary Access Trestle Design submitted in December specified AWS 01.1 as the required welding code. During the review process the reviewers requested that the welding code be changed to AWS 01.5- Bridge Welding Code. This request was complied with by revising general note A5.2 on the conformed trestle drawings.				
Since issuing these documents, BBII has been informed by both our shop and field welding inspectors that a compatibility discrepancy exists between the 01.5 welding code and base metals/ member shapes originally specified in the trestle design.		Since issuing these documents, BBII has been informed by both our shop and field welding inspectors that a compatibility discrepancy exists between the 01.5 welding code and base metals/ member shapes originally specified in the trestle design.				
D1.5 is specifically intended for use on bridges and it is not intended for use on "structures composed of structural tubing" as noted in section 1.1.1 attached. This causes a discrepancy because unlike most bridges, our trestle contains a substructure completely comprised of structural steel tubing. (ie Pipe pile, lateral and longitudinal X-bracing).		D1.5 is specifically intended for use on bridges and it is not intended for use on "structures composed of structural tubing" as noted in section 1.1.1 attached. This causes a discrepancy because unlike most bridges, our trestle contains a substructure completely comprised of structural steel tubing. (ie Pipe pile, lateral and longitudinal X-bracing).				
In addition to the pipe incompatibility, there is also an incompatibility between the specified base metals. 01.5 requires base metals to be ASTM A709 and the trestle design specified a variety of different base metals depending on their structural shape as shown in general note 2.28 also attached Since Article 1.1.1 of 01.5 permits the Engineer to choose to reference an alternate applicable welding standard when fabrication or structure components are not specifically addressed within its sections, BBII proposes keeping AWS 01.1 as the specified welding code because of its base metal compatibility, but adding a supplemental trestle specific welding specification written by the EOR that increases the quality control to a level equal to that of 01.5. This supplemental specification will include applicable portions of 01.5 section 3 "Workmanship" and section 3 "Inspection" when the requirements are greater than that of 01.1. (ie: fit-up tolerances, NOT frequency, etc).		In addition to the pipe incompatibility, there is also an incompatibility between the specified base metals. 01.5 requires base metals to be ASTM A709 and the trestle design specified a variety of different base metals depending on their structural shape as shown in general note 2.28 also attached Since Article 1.1.1 of 01.5 permits the Engineer to choose to reference an alternate applicable welding standard when fabrication or structure components are not specifically addressed within its sections, BBII proposes keeping AWS 01.1 as the specified welding code because of its base metal compatibility, but adding a supplemental trestle specific welding specification written by the EOR that increases the quality control to a level equal to that of 01.5. This supplemental specification will include applicable portions of 01.5 section 3 "Workmanship" and section 3				



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<p>Please advise if the proposed resolution is acceptable. Upon concurrence, BBII will submit the EOR's Trestle Welding specification for review.</p> <p>"Inspection" when the requirements are greater than that of 01.1. (ie: fit-up tolerances, NOT frequency, etc).</p> <p>Please advise if the proposed resolution is acceptable. Upon concurrence, BBII will submit the EOR's Trestle Welding specification for review.</p>						
<b>T-0279.1</b>	<b>BSE - Trestle Welding Code Compatibility</b>	<b>Closed</b>	<b>CR</b>	<b>03/28/2012</b>	<b>04/07/2012</b>	<b>04/09/2012</b>
<p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Shad Gardner</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: BBII Demarcation Sketch PB&A Trestle Welding Inspection Plan			Reference: BBII Demarcation Sketch PB&A Trestle Welding Inspection Plan			
The response to RFI T-279 provided a method of dealing with the trestle welding code compatibility issues that would be difficult to enforce, track and document. BBII proposes making a clear demarcation line at the bottom the cap beam that will clearly differentiate the two welding codes.			The response to RFI T-279 provided a method of dealing with the trestle welding code compatibility issues that would be difficult to enforce, track and document. BBII proposes making a clear demarcation line at the bottom the cap beam that will clearly differentiate the two welding codes.			
Additionally the RFI response appears to infer that the Temporary Bridge Specification 01-53-13 requires full compliance with AWS D1.5 as described in the third and last paragraph. 01-53-13 Paragraph 1.6.H (revB) only requires Welding Qualifications (procedures and personnel) to be performed in accordance with AWS D1.5.			Additionally the RFI response appears to infer that the Temporary Bridge Specification 01-53-13 requires full compliance with AWS D1.5 as described in the third and last paragraph. 01-53-13 Paragraph 1.6.H (revB) only requires Welding Qualifications (procedures and personnel) to be performed in accordance with AWS D1.5.			
Therefore in order to comply with the project specifications and the appropriate welding codes, BBII will Perform all welding below the demarcation line (substructure) with weld procedures and welder qualifications in conformance with AWS D1.1 since the members are predominately comprised of tubular material.			Therefore in order to comply with the project specifications and the appropriate welding codes, BBII will Perform all welding below the demarcation line (substructure) with weld procedures and welder qualifications in conformance with AWS D1.1 since the members are predominately comprised of tubular material.			
Perform all welding above the demarcation line (superstructure) with weld procedures and welder qualifications conformance with AWS D1.5 since the main members are Wide flange beam.			Perform all welding above the demarcation line (superstructure) with weld procedures and welder qualifications conformance with AWS D1.5 since the main members are Wide flange beam.			
Inspection will be performed by the project special						



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	<div>inspector in accordance with recommendations of the EOR attached.</div> <div>Please confirm this is acceptable.</div>					<div>Inspection will be performed by the project special inspector in accordance with recommendations of the EOR attached.</div> <div>Please confirm this is acceptable.</div>
T-0280	<div>BSE - Request to shorten depth on shaft D/1</div> <div>From: Webcor Construction LPJoanne Filipas</div>	Closed	01	02/29/2012	03/10/2012	03/02/2012
	<div>REQUEST:</div> <div>Ref - Attached RFI from BBI/Becho</div> <div>Due to the blowout conditions previously encountered on Buttress Shaft D1, BECHO requests to install Shaft D1 to a depth of 180 feet as previously proposed by ARUP. BECHO believes the blowout condition still exists and thus would like to proceed with caution to prevent another occurrence. Alternatively, if ARUP feels this is no longer an option, BECHO requests that ARUP increase the maximum spacing allowed between the tangent shafts, in event to mitigate possible schedule delay, and/or re-break of casing while advancing D1. By allowing such changes will help mitigate Buttress shaft schedule.</div> <div>W/O acknowledges that BBII has yet to demonstrate that a "blowout" condition has in fact occurred. W/O would request the design team consider short pouring D-1 due to drilling difficulties encountered. Alternatively, W/O would request the spacing revision described above.</div>					<div>ANSWER:</div> <div>Ref - Attached RFI from BBI/Becho</div> <div>Due to the blowout conditions previously encountered on Buttress Shaft D1, BECHO requests to install Shaft D1 to a depth of 180 feet as previously proposed by ARUP. BECHO believes the blowout condition still exists and thus would like to proceed with caution to prevent another occurrence. Alternatively, if ARUP feels this is no longer an option, BECHO requests that ARUP increase the maximum spacing allowed between the tangent shafts, in event to mitigate possible schedule delay, and/or re-break of casing while advancing D1. By allowing such changes will help mitigate Buttress shaft schedule.</div> <div>W/O acknowledges that BBII has yet to demonstrate that a "blowout" condition has in fact occurred. W/O would request the design team consider short pouring D-1 due to drilling difficulties encountered. Alternatively, W/O would request the spacing revision described above.</div>



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<b>T-0281</b>	<b>BSE - Survey Site Drawing and Certificate Submittal</b>	<b>Closed</b>	<b>CR</b>	<b>03/06/2012</b>	<b>03/16/2012</b>	<b>03/09/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Danny Walsh						
<b>REQUEST:</b>			<b>ANSWER:</b>			
BBII is unclear on what is required for the "site drawing and certificate" submittal listed in section 01 10 50 1.3B. As the first contractor working on the construction of the terminal, no previous work is in place. Please confirm that the requirement is intended for future trade packages (to verify the work already completed by previous trade subcontractors), or provide additional clarification on what is required of BBII to complete this submittal requirement.			BBII is unclear on what is required for the "site drawing and certificate" submittal listed in section 01 10 50 1.3B. As the first contractor working on the construction of the terminal, no previous work is in place. Please confirm that the requirement is intended for future trade packages (to verify the work already completed by previous trade subcontractors), or provide additional clarification on what is required of BBII to complete this submittal requirement.			
<b>T-0282</b>	<b>BSE - News/Advertisement Stand Removal</b>	<b>Closed</b>	<b>CR</b>	<b>03/16/2012</b>	<b>03/26/2012</b>	<b>03/19/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The unused news/advertisement stand on the Westside of Fremont Street needs to be removed to accommodate the Buttress drilling on shafts A & B. BBII intends to modify the sidewalk at this current location to provide 3 - 11ft lanes on Fremont Street per specification section 01-15-70. (see attached sketch)			The unused news/advertisement stand on the Westside of Fremont Street needs to be removed to accommodate the Buttress drilling on shafts A & B. BBII intends to modify the sidewalk at this current location to provide 3 - 11ft lanes on Fremont Street per specification section 01-15-70. (see attached sketch)			
Please provide direction to relocate or remove these stands.			Please provide direction to relocate or remove these stands.			
<b>T-0283</b>	<b>BSE - Backfill Material For Pre-Trench</b>	<b>Closed</b>	<b>CR</b>	<b>03/15/2012</b>	<b>03/25/2012</b>	<b>03/20/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Jeff Molloy						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Proposed 1 sack sand mix design			Reference: Proposed 1 sack sand mix design			
BBII is not able to achieve the required compaction per SFDPW requirements due to inclement weather conditions. We have been advised from suppliers that the sand backfill material is saturated, and from past experience will not achieve the required compaction.			BBII is not able to achieve the required compaction per SFDPW requirements due to inclement weather conditions. We have been advised from suppliers that the sand backfill material is saturated, and from past experience will not achieve the required compaction.			
If the weather persists as forecasted BBII is proposing to backfill with 1 sack sand as a substitute to dry material. This will allow us to maintain the scheduled CDSM wall			If the weather persists as forecasted BBII is proposing to backfill with 1 sack sand as a substitute to dry material. This will allow us to maintain the scheduled			





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T-0283.1	<p>installation on 3/23/2012, and maintain the DPW compaction standards. Note sand slurry is only required in the street or public right of way.</p> <p>Note: According to BBII this will not impact DND/Malcolm in the installation of the CDSM wall.</p> <p><b>BSE - Backfill for Pretrenching</b></p> <p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal</p> <p><b>REQUEST:</b></p> <p>As a supplement to RFI 283 regarding the use of a CDF mix for backfill of the pre-trench at A-line across First Street, BBII is submitting the attached mix design for review and acceptance. The previously submitted mix design was not pumpable and due to the nature of the pile extraction and backfill operation a pumpable mix is required so backfill compaction can be achieved. The attached mix will allow us to achieve the DPW compaction requirements and also allow for the installation of the CDSM wall.</p> <p>The use of this mix design is scheduled for this afternoon in order to maintain the CDSM installation schedule for this weekend. BBII would much appreciate an expedited review and acceptance of this mix design.</p>	Closed	CR	03/29/2012	04/08/2012	03/30/2012
	<p><b>ANSWER:</b></p> <p>As a supplement to RFI 283 regarding the use of a CDF mix for backfill of the pre-trench at A-line across First Street, BBII is submitting the attached mix design for review and acceptance. The previously submitted mix design was not pumpable and due to the nature of the pile extraction and backfill operation a pumpable mix is required so backfill compaction can be achieved. The attached mix will allow us to achieve the DPW compaction requirements and also allow for the installation of the CDSM wall.</p> <p>The use of this mix design is scheduled for this afternoon in order to maintain the CDSM installation schedule for this weekend. BBII would much appreciate an expedited review and acceptance of this mix design.</p>					
T-0284	<p><b>BSE - Request to Borehole Coordinates TTB-07 TTB-09</b></p> <p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal</p> <p><b>REQUEST:</b></p> <p>After further review of the Geotechnical Report produced by ARUP it has come to BECHO's attention that Boreholes TTB-07 and TTB-09 were not surveyed. BECHO respectfully requests to obtain Northing and Easting coordinates for TTB-07 and TTB-09.</p>	Closed	01	03/21/2012	03/31/2012	03/23/2012
	<p><b>ANSWER:</b></p> <p>After further review of the Geotechnical Report produced by ARUP it has come to BECHO's attention that Boreholes TTB-07 and TTB-09 were not surveyed. BECHO respectfully requests to obtain Northing and Easting coordinates for TTB-07 and TTB-09.</p>					
T-0285	<p><b>BSE - Buttress Rebar Cage Length Adjustment</b></p>	Closed	01	03/21/2012	03/31/2012	03/26/2012





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T-0287	Temporary Bridge specification 01-53-13 (1.3B) requires the bridge design to include a 3000 lb/lf allowance for hanging utilities below the bridge. Extensive coordination between the RUP designers and the utility owners, BBII has attained the exact location and actual weight of the utilities to be supported by the bridge structures. These weights are shown in the attached document and have been used in the design of the bridge structure as well as the utility hangers. Through our coordination efforts we also know that future utilities will not be added until the temporary bridges are removed. Please confirm that use of the actual utility weights in our design is acceptable.	Closed	01	04/04/2012	04/14/2012	04/12/2012
	Temporary Bridge specification 01-53-13 (1.3B) requires the bridge design to include a 3000 lb/lf allowance for hanging utilities below the bridge. Extensive coordination between the RUP designers and the utility owners, BBII has attained the exact location and actual weight of the utilities to be supported by the bridge structures. These weights are shown in the attached document and have been used in the design of the bridge structure as well as the utility hangers. Through our coordination efforts we also know that future utilities will not be added until the temporary bridges are removed. Please confirm that use of the actual utility weights in our design is acceptable.					
T-0287	<b>BSE - Drain Inlet at the Northwest Corner of Minna and First street</b>  From: Balfour Beatty Infrastructure, Inc. Shad Gardner  <b>REQUEST:</b> Reference: TG0300-210.1 TG0300-205.2 City Planning/KCA Emails  In order to comply with city standards BBII intended to install a standard city drain inlet on the north west corner of the Minna and First street intersection as required by our site civil drainage plan (submittal TG0300-205.2, TZ1030-01513A08.2 see also submittal TZ1030-015313A04.1 package TG0300-210.1 for product data). When potholing where this drain inlet is to be located, it was discovered that it would be in conflict with an existing gas line. BBII's design engineer KCA contacted the city planning department and got pre approval of the attached catch basin per the attached email and details. Please confirm that it is acceptable for us to install this catch basin in lieu of what was submitted in the aforementioned submittals.					
T-0288	Temporary Bridge specification 01-53-13 (1.3B) requires the bridge design to include a 3000 lb/lf allowance for hanging utilities below the bridge. Extensive coordination between the RUP designers and the utility owners, BBII has attained the exact location and actual weight of the utilities to be supported by the bridge structures. These weights are shown in the attached document and have been used in the design of the bridge structure as well as the utility hangers. Through our coordination efforts we also know that future utilities will not be added until the temporary bridges are removed. Please confirm that use of the actual utility weights in our design is acceptable.	Closed	01	04/04/2012	04/14/2012	04/12/2012
	Temporary Bridge specification 01-53-13 (1.3B) requires the bridge design to include a 3000 lb/lf allowance for hanging utilities below the bridge. Extensive coordination between the RUP designers and the utility owners, BBII has attained the exact location and actual weight of the utilities to be supported by the bridge structures. These weights are shown in the attached document and have been used in the design of the bridge structure as well as the utility hangers. Through our coordination efforts we also know that future utilities will not be added until the temporary bridges are removed. Please confirm that use of the actual utility weights in our design is acceptable.					
T-0288	<b>BSE - Request to Relocate Rathole to D9</b>  From: Balfour Beatty Infrastructure, Inc. Ural Yal  <b>ANSWER:</b> Reference: TG0300-210.1 TG0300-205.2 City Planning/KCA Emails  In order to comply with city standards BBII intended to install a standard city drain inlet on the north west corner of the Minna and First street intersection as required by our site civil drainage plan (submittal TG0300-205.2, TZ1030-01513A08.2 see also submittal TZ1030-015313A04.1 package TG0300-210.1 for product data). When potholing where this drain inlet is to be located, it was discovered that it would be in conflict with an existing gas line. BBII's design engineer KCA contacted the city planning department and got pre approval of the attached catch basin per the attached email and details. Please confirm that it is acceptable for us to install this catch basin in lieu of what was submitted in the aforementioned submittals.					

<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
	<p><b>REQUEST:</b></p> <p>Attached please find Becho's request to relocate existing rathole to Shaft D9 where it will remain until Buttress work is complete. Below is Becho's exact wording:</p> <p>"Due to the upcoming bridge construction on Fremont Street, Becho will be losing the existing location of the rathole. Becho requests that the existing rathole be relocated to Shaft D9 where it will remain for the duration of the Buttress Shaft Work. Becho proposes to pour Shaft D9 30 to 35 feet short from grade to accommodate the new rathole. Please advise if this is acceptable."</p>					
	<p><b>ANSWER:</b></p> <p>Attached please find Becho's request to relocate existing rathole to Shaft D9 where it will remain until Buttress work is complete. Below is Becho's exact wording:</p> <p>"Due to the upcoming bridge construction on Fremont Street, Becho will be losing the existing location of the rathole. Becho requests that the existing rathole be relocated to Shaft D9 where it will remain for the duration of the Buttress Shaft Work. Becho proposes to pour Shaft D9 30 to 35 feet short from grade to accommodate the new rathole. Please advise if this is acceptable."</p>					
<b>T-0289</b>	<b>BSE - Becho Requesting 9-20-2011 Meeting Minutes</b>	<b>Closed</b>	<b>01</b>	<b>04/11/2012</b>	<b>04/21/2012</b>	<b>05/08/2012</b>
	<p><b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal</p> <p><b>REQUEST:</b></p> <p>"On September 20th, 2011 a meeting was held in the TJPA's office to discuss Noise Issues, Coring thru the Concrete Slab and Buttress Work. Present in the meeting where the following key representatives: Brian Dykes, Maria Ayerdi-Kaplan, Rebecca Armenta, and Steven Rule. Please request the meeting minutes for the meeting on 9/20/2011."</p>					
	<p><b>ANSWER:</b></p> <p>"On September 20th, 2011 a meeting was held in the TJPA's office to discuss Noise Issues, Coring thru the Concrete Slab and Buttress Work. Present in the meeting where the following key representatives: Brian Dykes, Maria Ayerdi-Kaplan, Rebecca Armenta, and Steven Rule. Please request the meeting minutes for the meeting on 9/20/2011."</p>					



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<b>T-0292</b>	<b>BSE - First St Bridge Pier 1 Relocation</b>	<b>Closed</b>	<b>CR</b>	<b>05/02/2012</b>	<b>05/12/2012</b>	<b>05/03/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Revised Drawings and Calculations for Revised Pier 1 Location			Reference: Revised Drawings and Calculations for Revised Pier 1 Location			
The western Pier 1 CIDH pile was rejected due to an anomaly. The corrective action is to replace it with a new pile 6'-0" south. Attached is the revised Bridge Drawings and the revised calculations. This package was emailed to the Bridge Design reviewers on 4-24-12 for expedited review. Please confirm that the new pier 1 location does not cause conflicts with the future structure.			The western Pier 1 CIDH pile was rejected due to an anomaly. The corrective action is to replace it with a new pile 6'-0" south. Attached is the revised Bridge Drawings and the revised calculations. This package was emailed to the Bridge Design reviewers on 4-24-12 for expedited review. Please confirm that the new pier 1 location does not cause conflicts with the future structure.			
<b>T-0292.1</b>	<b>BSE - First St Bridge Pier 1 Relocation</b>	<b>Closed</b>	<b>CR</b>	<b>05/03/2012</b>	<b>05/13/2012</b>	<b>05/04/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: SH-2100 SH-2101			Reference: SH-2100 SH-2101			
Detail: The western Pier 1 CIDH pile was rejected due to an anomaly. The corrective action is to replace it with a new pile 6'-0" south. Attached are the revised Bridge Drawings showing new pile locations. Please confirm that the new pier 1 location does not cause conflicts with the future structure. Please note the revised design documents were emailed to the Bridge Design reviewers on 4-24-12 for expedited review.			Detail: The western Pier 1 CIDH pile was rejected due to an anomaly. The corrective action is to replace it with a new pile 6'-0" south. Attached are the revised Bridge Drawings showing new pile locations. Please confirm that the new pier 1 location does not cause conflicts with the future structure. Please note the revised design documents were emailed to the Bridge Design reviewers on 4-24-12 for expedited review.			
<b>T-0293</b>	<b>BSE - First Street Natoma blind spot hazard</b>	<b>Closed</b>	<b>01</b>	<b>06/05/2012</b>	<b>06/15/2012</b>	<b>06/15/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Regarding the temporary first street bridge. Contract specification section 01 53 13-1.3.A.4 requires us to provide a "8' -high solid barrier system" consisting of 1" plywood which does not allow viewing through the barrier. This is creating a blind turn hazard for traffic entering First street from Natoma street on the south side of First street. Please advise on how you would like to mitigate/fix this			Regarding the temporary first street bridge. Contract specification section 01 53 13-1.3.A.4 requires us to provide a "8' -high solid barrier system" consisting of 1" plywood which does not allow viewing through the barrier. This is creating a blind turn hazard for traffic entering First street from Natoma street on the south side of First street. Please advise on how you would			





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	hazard.					like to mitigate/fix this hazard.
<b>T-0293.1</b>	<b>BSE - First Street and Natoma blind spot hazard.</b>	<b>Closed</b>	<b>01</b>	<b>06/29/2012</b>	<b>07/09/2012</b>	<b>07/09/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b> Please find attached sketch SK-0293 for proposed pedestrian barrier at the First st. bridge. Please confirm this is acceptable in lieu of previously installed plywood barrier.						<b>ANSWER:</b> Please find attached sketch SK-0293 for proposed pedestrian barrier at the First st. bridge. Please confirm this is acceptable in lieu of previously installed plywood barrier.
<b>T-0293.2</b>	<b>BSE - Blind Spots at Fremont St. and Beale Street Bridges</b>	<b>Closed</b>	<b>CR</b>	<b>08/13/2012</b>	<b>08/23/2012</b>	<b>08/21/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference: RFI T-0293.1 RFI T-0293  Blind spots similiar to the those in RFI T-0293 at First street and Natoma street exist at the following locations:  Fremont Street - Northwest & Southwest Corners (Cars exiting from 301 Mission and 400 Howard) Beale Street - Southwest Corner (Cars exiting from 199 Fremont and 301 Mission)  Please confirm that similiar fencing as per response to RFI T-0293.1 should be installed at these locations.						<b>ANSWER:</b> Reference: RFI T-0293.1 RFI T-0293  Blind spots similiar to the those in RFI T-0293 at First street and Natoma street exist at the following locations:  Fremont Street - Northwest & Southwest Corners (Cars exiting from 301 Mission and 400 Howard) Beale Street - Southwest Corner (Cars exiting from 199 Fremont and 301 Mission)  Please confirm that similiar fencing as per response to RFI T-0293.1 should be installed at these locations.



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<b>T-0293.3</b>	<b>BSE Blind Spots at Fremont St. and Beale Street Bridges</b>	<b>Closed</b>	<b>01</b>	<b>08/28/2012</b>	<b>09/07/2012</b>	<b>08/29/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference: RFI T-0293.1 RFI T-0293.2  In RFI T-0293.2 there was an error in requesting confirmation for fencing in the Northwest corner when it was meant to request fencing in the Northeast corner.  Please confirm that fencing as per response to RFI T-0293.1 should be installed on Fremont Street on the Northeast corner rather than the Northwest corner.		<b>ANSWER:</b>  Reference: RFI T-0293.1 RFI T-0293.2  In RFI T-0293.2 there was an error in requesting confirmation for fencing in the Northwest corner when it was meant to request fencing in the Northeast corner.  Please confirm that fencing as per response to RFI T-0293.1 should be installed on Fremont Street on the Northeast corner rather than the Northwest corner.				
<b>T-0293.4</b>	<b>BSE - Blind Spots at Beale Street Bridge</b>	<b>Closed</b>	<b>01</b>	<b>04/08/2013</b>	<b>04/18/2013</b>	<b>04/11/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>  Reference: RFI #T-0293.2  Blind spots similar to what was alleviated at First & Fremont Streets, the result of the originally specified "8'-high solid barrier system", exist on Beale St. at the following locations:  1. Making a right at the Southwest corner exiting 199 Fremont's garage. 2. Making a right at the Northwest corner exiting 301 Mission's garage (the concern being if someone is coming down Beale the wrong way.)  Please confirm if and where chain link, similar to what was specified in RFI response #T-0293.1, is required and what CR # to reference.		<b>ANSWER:</b>  Reference: RFI #T-0293.2  Blind spots similar to what was alleviated at First & Fremont Streets, the result of the originally specified "8'-high solid barrier system", exist on Beale St. at the following locations:  1. Making a right at the Southwest corner exiting 199 Fremont's garage. 2. Making a right at the Northwest corner exiting 301 Mission's garage (the concern being if someone is coming down Beale the wrong way.)  Please confirm if and where chain link, similar to what was specified in RFI response #T-0293.1, is required and what CR # to reference.				
<b>T-0294</b>	<b>BSE - Expected CDSM wall deflection</b>	<b>Closed</b>	<b>01</b>	<b>06/14/2012</b>	<b>06/24/2012</b>	<b>07/02/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>  BBII requests the anticipated deflection values for the CDSM wall obtained in ARUP's design of the shoring wall		<b>ANSWER:</b>  BBII requests the anticipated deflection values for the CDSM wall obtained in ARUP's design of the shoring				



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T-0295	<p>and used to determine appropriate action trigger levels specified in section 31 09 13.</p> <p><b>BSE - 301 Mission drive way</b></p> <p><b>From:</b> Webcor Construction LP      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Per conversation in previous coordination meeting between Balfour Beatty Webcor, Turner, TJPA and 301 Mission's management. We are confirming direction to extend the sidewalk past the limits shown in our grading and drainage submittal through the limits of the 301 Mission drive way. It is also our understanding that we are directed to match the color of the existing black sidewalk in this area. Please confirm.</p>	Closed	01	06/19/2012	06/29/2012	06/24/2012
	<p>wall and used to determine appropriate action trigger levels specified in section 31 09 13.</p> <p><b>ANSWER:</b></p> <p>Per conversation in previous coordination meeting between Balfour Beatty Webcor, Turner, TJPA and 301 Mission's management. We are confirming direction to extend the sidewalk past the limits shown in our grading and drainage submittal through the limits of the 301 Mission drive way. It is also our understanding that we are directed to match the color of the existing black sidewalk in this area. Please confirm.</p>					







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<b>T-0297</b>	<b>BSE - Phase 3 Utilities on Beale Street</b>  <b>From:</b> Webcor Construction LP      Joanne Filipas  <b>REQUEST:</b>  Reference attached sketch.  The BSE subcontractor is proposing to relocate the Beale Street temporary bridge to the east; similar to the attached sketch. Please confirm if this will impact any future utilities, i.e. PG&E phase 3 on Beale Street.	<b>Closed</b>	<b>01</b>	<b>06/28/2012</b>	<b>07/08/2012</b>	<b>07/10/2012</b>
					<b>ANSWER:</b>  Reference attached sketch.  The BSE subcontractor is proposing to relocate the Beale Street temporary bridge to the east; similar to the attached sketch. Please confirm if this will impact any future utilities, i.e. PG&E phase 3 on Beale Street.	
<b>T-0297.2</b>	<b>BGP - Injection Hose Testing Criteria</b>  <b>From:</b> Webcor Construction LP      Claude Titché  <b>REQUEST:</b>  Per conference call with design team, please confirm that it is acceptable to test the waterstop injection hoses with water as recommended by manufacturer.	<b>Void</b>	<b>01</b>	<b>02/18/2014</b>	<b>02/28/2014</b>	
					<b>ANSWER:</b>  Per conference call with design team, please confirm that it is acceptable to test the waterstop injection hoses with water as recommended by manufacturer.	
<b>T-0298</b>	<b>BSE -Timber Pile Extraction at grid line 19 to 20 and 24 to 25</b>  <b>From:</b> Webcor Construction LP      Robert Kjome  <b>REQUEST:</b>  BBII completed the timber pile extraction test section in zone 2 on 06/12/2012. Based on the data recorded by ARUP inclinometers, please advise if BBII can continue with the timber pile extraction at grid line 19 to 20 and grid line 24 to 25 using non ground deformation control methods ("free pull").  The attached drawings (D-21 02 and D-21 03) for reference.  Please advise.	<b>Closed</b>	<b>01</b>	<b>06/29/2012</b>	<b>06/29/2012</b>	<b>07/02/2012</b>
					<b>ANSWER:</b>  BBII completed the timber pile extraction test section in zone 2 on 06/12/2012. Based on the data recorded by ARUP inclinometers, please advise if BBII can continue with the timber pile extraction at grid line 19 to 20 and grid line 24 to 25 using non ground deformation control methods ("free pull").  The attached drawings (D-21 02 and D-21 03) for reference.  Please advise.	
<b>T-0299</b>	<b>Micropile Performance Testing</b>  <b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal  <b>REQUEST:</b>  Reference Part 3.2 "Performance And Proof Testing" of Specification Section 31 63 33  In order to expedite the Micropile Performance Testing	<b>Closed</b>	<b>01</b>	<b>07/16/2012</b>	<b>07/26/2012</b>	<b>07/30/2012</b>
					<b>ANSWER:</b>  Reference Part 3.2 "Performance And Proof Testing" of Specification Section 31 63 33  In order to expedite the Micropile Performance Testing	



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
T-0300	<p>review period, BBII is requesting to conduct the performance testing of micropiles prior to excavating Level 5, at approximately -32' Elevation, concurrent with the installation of Level "0" struts. See attached sketch for details.Please confirm that it is acceptable.</p> <p><b>Micropile Performance Test Pile Relocations</b></p> <p><b>From:</b> Balfour Beatty Infrastructure, Inc. Yuriy Stryzheus</p> <p><b>REQUEST:</b></p> <p>Please refer to BBII's micropile layout submittal and RFI T-262 that references IFB- Below Grade package for coordination of micropile layouts.</p> <p>Based on the information provided within BBII's Micropile layout drawing and Below Grade package drawings S1-2023 through S1-2027, the four micropiles subjected to performance testing are labeled as: W411, W396, E383, and E401.</p> <p>BBII requests to conduct the performance test in Zone 1 at pile No. W604 instead of pile No. W411, which is located underneath Struts No. 6 &amp; 7.</p> <p>Similarly, BBII requests to test the piles numbered as W473, E477, &amp; E599, instead of the piles numbered as W396, E383, &amp; E401, which are located underneath the trestle.</p> <p>Please confirm that it is acceptable.</p>	Closed	01	07/17/2012	07/27/2012	07/26/2012
	<p><b>ANSWER:</b></p> <p>Please refer to BBII's micropile layout submittal and RFI T-262 that references IFB- Below Grade package for coordination of micropile layouts.</p> <p>Based on the information provided within BBII's Micropile layout drawing and Below Grade package drawings S1-2023 through S1-2027, the four micropiles subjected to performance testing are labeled as: W411, W396, E383, and E401.</p> <p>BBII requests to conduct the performance test in Zone 1 at pile No. W604 instead of pile No. W411, which is located underneath Struts No. 6 &amp; 7.</p> <p>Similarly, BBII requests to test the piles numbered as W473, E477, &amp; E599, instead of the piles numbered as W396, E383, &amp; E401, which are located underneath the trestle.</p> <p>Please confirm that it is acceptable.</p>					

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T-0301	Trestle Piles in Exclusion Zones (Zone 4)	Closed	01	07/23/2012	08/02/2012	07/30/2012
<div> <div> <b>From:</b> Webcor Construction LP           Robert Kjome         </div> <div> <b>REQUEST:</b>            Review comments on submittal package TG0300-284 directed BBII to shift two trestle piles (#69 &amp; #72) out of pile exclusion zones (provided by Thornton Tomasetti in response to RFI T-0251.1). BBII worked to avoid these zones to the extent possible. However, in zone 4 the additional buttress shafts created further limitations on trestle pile locations and it was infeasible to completely avoid both the permanent structure and buttress. BBII is aware of the possibility of eliminating some of these additional buttress shafts but this will not resolve these specific conflicts. Due to the congestion in Zone 4 with both the pile exclusion zones and added buttress shafts, BBII requests an exception for trestle piles #69 and #72.         </div> </div>						
<div> <div> <b>ANSWER:</b>            Review comments on submittal package TG0300-284 directed BBII to shift two trestle piles (#69 &amp; #72) out of pile exclusion zones (provided by Thornton Tomasetti in response to RFI T-0251.1). BBII worked to avoid these zones to the extent possible. However, in zone 4 the additional buttress shafts created further limitations on trestle pile locations and it was infeasible to completely avoid both the permanent structure and buttress. BBII is aware of the possibility of eliminating some of these additional buttress shafts but this will not resolve these specific conflicts. Due to the congestion in Zone 4 with both the pile exclusion zones and added buttress shafts, BBII requests an exception for trestle piles #69 and #72.         </div> </div>						
T-0302	ISI Low Compression Strength for CLSM	Closed	01	07/31/2012	08/10/2012	08/10/2012
<div> <div> <b>From:</b> Balfour Beatty Infrastructure, Inc.           Ural Yal         </div> <div> <b>REQUEST:</b>            Please confirm the low compression strengths for the CLSM, in the ISI test results (attached), are acceptable. The CLSM was used for pre-trench backfill on Gridline A, First St. and Fremont St.             Please see attached ISI Test reports:            55606 Compression Test Report on A line between 18-19 lines, sampled 3/29/2012            55607 Compression Test Report on A line between 19-20 lines, sampled 3/30/2012            55608 Compressive Test Report on A line between 19-20 lines, sampled 4/4/2012            51399 Compression Test Report on A line between 19-20 lines, sampled 3/28/2012            56162 Compressive Test Report on A line between 25.2 - 25.5 lines, sampled 4/2/2012         </div> </div>						
<div> <div> <b>ANSWER:</b>            Please confirm the low compression strengths for the CLSM, in the ISI test results (attached), are acceptable. The CLSM was used for pre-trench backfill on Gridline A, First St. and Fremont St.             Please see attached ISI Test reports:            55606 Compression Test Report on A line between 18-19 lines, sampled 3/29/2012            55607 Compression Test Report on A line between 19-20 lines, sampled 3/30/2012            55608 Compressive Test Report on A line between 19-20 lines, sampled 4/4/2012            51399 Compression Test Report on A line between 19-20 lines, sampled 3/28/2012            56162 Compressive Test Report on A line between 25.2 -25.5 lines, sampled 4/2/2012         </div> </div>						
T-0303	BSE - Verizon Duct Bank at the First St Bridge	Closed	01	08/07/2012	08/17/2012	08/08/2012
<div> <div> <b>From:</b> Webcor Construction LP           Kirk Nielsen         </div> <div> <b>REQUEST:</b> </div> </div>						
<div> <div> <b>ANSWER:</b> </div> </div>						





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	<p>Reference: Attached Photo</p> <p>Despite providing Verizon surveying, staking, and cutsheets, the Verizon duct bank at the North side of the First St. bridge was installed by others at the incorrect elevation (too low). Please confirm if additional utility supports will be required of TG03 or if others will be performing the additional utility supports required for the Verizon duct bank.</p>					
<b>T-0304</b>	<b>BSE - Inquiries with Regard to Proposed Beale St Bridge Atop East CDSM Wall</b>	<b>Closed</b>	<b>01</b>	<b>08/23/2012</b>	<b>09/02/2012</b>	<b>08/27/2012</b>
	<p><b>From:</b> Webcor Construction LP                      Kirk Nielsen</p> <p><b>REQUEST:</b></p> <p>On 8/22/12 Beale St. bridge submittal #TG0300-206 was returned to W/O marked not reviewed (Exhibit-A). Upon W/O's review of BBII's Beale St. bridge design W/O encountered the following inquiries relative to the CDSM wall:</p> <p>1. BBII's bridge design relies on ARUP's RFI response #T-0209.3 (Exhibit-B). Please confirm ARUP's RFI response #T-0209.3 (Exhibit-C) is applicable as the basis of the design for the Beale St. bridge, given unlike First and Fremont Streets, the length of the Beale St. bridge is resting atop the East CDSM wall.</p> <p>2. The decision to allow the North and South bridge abutments to be located atop the CDSM wall was predicated on the CR #T-025 load testing reference RFI #T-0209.4 (Exhibit-D). Given the testing was performed on different soldier piles (by others) and differing soil conditions between Zone-1 and Zone-4, is the load capacity derived from the CR #T-025 testing applicable given the different bridge location and configuration?</p> <p>3. BBII's Beale St. bridge design relies on resting the length of the Beale St. bridge atop the East CDSM wall. As the</p>					<p><b>ANSWER:</b></p> <p>On 8/22/12 Beale St. bridge submittal #TG0300-206 was returned to W/O marked not reviewed (Exhibit-A). Upon W/O's review of BBII's Beale St. bridge design W/O encountered the following inquiries relative to the CDSM wall:</p> <p>1. BBII's bridge design relies on ARUP's RFI response #T-0209.3 (Exhibit-B). Please confirm ARUP's RFI response #T-0209.3 (Exhibit-C) is applicable as the basis of the design for the Beale St. bridge, given unlike First and Fremont Streets, the length of the Beale St. bridge is resting atop the East CDSM wall.</p> <p>2. The decision to allow the North and South bridge abutments to be located atop the CDSM wall was predicated on the CR #T-025 load testing reference RFI #T-0209.4 (Exhibit-D). Given the testing was performed on different soldier piles (by others) and differing soil conditions between Zone-1 and Zone-4, is the load capacity derived from the CR #T-025 testing applicable given the different bridge location and configuration?</p> <p>3. BBII's Beale St. bridge design relies on resting the</p>



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	designer of the CDSM wall, does ARUP endorse further loading of the East CDSM wall with the forces imposed by the Beale St. bridge?					length of the Beale St. bridge atop the East CDSM wall. As the designer of the CDSM wall, does ARUP endorse further loading of the East CDSM wall with the forces imposed by the Beale St. bridge?
<b>T-0304.1</b>	<b>BSE - Inquiries with Regard to Proposed Beale St Bridge Follow-Up</b>	<b>Closed</b>	<b>01</b>	<b>08/29/2012</b>	<b>09/08/2012</b>	<b>08/31/2012</b>
<b>From:</b> Webcor Construction LP      Kirk Nielsen						
<b>REQUEST:</b>  In follow-up to RFI T-0304:  - From the response to question #2 of RFI T-0304 it is understood that ARUP's design recommendations were not informed solely by the load testing results. However the original question remains, is the load capacity derived from the CR #T-025 testing applicable given the different bridge location and configuration?  - So the contractor can understand the parameters of what we are submitting, was the Shoring Wall Designed to withstand the loads imposed by the proposed Beale St. bridge?						<b>ANSWER:</b>  In follow-up to RFI T-0304:  - From the response to question #2 of RFI T-0304 it is understood that ARUP's design recommendations were not informed solely by the load testing results. However the original question remains, is the load capacity derived from the CR #T-025 testing applicable given the different bridge location and configuration?  - So the contractor can understand the parameters of what we are submitting, was the Shoring Wall Designed to withstand the loads imposed by the proposed Beale St. bridge?





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<b>T-0307</b>	<b>Re - Bracing Drawings</b>	<b>Closed</b>	<b>01</b>	<b>08/23/2012</b>	<b>09/02/2012</b>	<b>08/24/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference: Spec. Section 31 55 00 Drawing S1-1112  In order to design the re-bracing BBII requests drawings for the Below Grade Package. Please provide these drawings on a CD in AutoCAD and PDF format.						<b>ANSWER:</b>  Reference: Spec. Section 31 55 00 Drawing S1-1112  In order to design the re-bracing BBII requests drawings for the Below Grade Package. Please provide these drawings on a CD in AutoCAD and PDF format.
<b>T-0308</b>	<b>BSE - Phase 2 Extension During the Service Life of the Beale St. Bridge</b>	<b>Closed</b>	<b>01</b>	<b>08/27/2012</b>	<b>09/06/2012</b>	<b>08/29/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>  On 8/22/12 Beale St. Bridge submittal TG0300-206 was returned to W/O marked not reviewed.  In lieu of piers the proposed Beale St. Bridge relies on the eastern shoring wall for structural support. As a result of this configuration the eastern shoring wall located along grid line 35.25 will have to remain in place throughout the entire life of the bridge. Multiple contract documents including S1-2027 (Exhibit-A) elude to a "Phase 2" which extends the underground portion of the structure to the east of the existing shoring wall. Please confirm the verbal direction that the "Phase 2" package will not be constructed during the life of the Beale St. bridge.						<b>ANSWER:</b>  On 8/22/12 Beale St. Bridge submittal TG0300-206 was returned to W/O marked not reviewed.  In lieu of piers the proposed Beale St. Bridge relies on the eastern shoring wall for structural support. As a result of this configuration the eastern shoring wall located along grid line 35.25 will have to remain in place throughout the entire life of the bridge. Multiple contract documents including S1-2027 (Exhibit-A) elude to a "Phase 2" which extends the underground portion of the structure to the east of the existing shoring wall. Please confirm the verbal direction that the "Phase 2" package will not be constructed during the life of the Beale St. bridge.
<b>T-0309</b>	<b>BSE - Traffic Control During the Construction of the Beale St. Bridge</b>	<b>Closed</b>	<b>01</b>	<b>08/27/2012</b>	<b>09/06/2012</b>	<b>08/29/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>  At the 8/27/12 TJPB Traffic Coordination meeting Balfour Beatty presented a construction plan for the proposed Beale St. bridge. In violation of Specification Section 01 15 70-2 the construction plan included reducing Beale St. down to two available traffic lanes for an approximately six week duration. Please confirm if this is acceptable.						<b>ANSWER:</b>  At the 8/27/12 TJPB Traffic Coordination meeting Balfour Beatty presented a construction plan for the proposed Beale St. bridge. In violation of Specification Section 01 15 70-2 the construction plan included reducing Beale St. down to two available traffic lanes for an approximately six week duration. Please confirm if this is acceptable.



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T-0310	Clarification on Sump Pit Location	Closed	01	08/28/2012	09/07/2012	09/07/2012
From: Webcor Construction LP Robert Kjome						
REQUEST: RFI Ref: T-0251.3 Spec. Ref: 31 00 00 Drawing/Detail Ref: GT 2101, 2102, 2103  The current coordination drawing for sump pit locations, received in RFI response T-0251.3 (12/13/2011) do not correspond with the BSE contract drawing GT 2101, 2102, 2103. Please confirm the correct sump pit location.		ANSWER: RFI Ref: T-0251.3 Spec. Ref: 31 00 00 Drawing/Detail Ref: GT 2101, 2102, 2103  The current coordination drawing for sump pit locations, received in RFI response T-0251.3 (12/13/2011) do not correspond with the BSE contract drawing GT 2101, 2102, 2103. Please confirm the correct sump pit location.				
T-0311	Subgrade French Drains Along CDSM Wall	Closed	01	08/31/2012	09/10/2012	09/07/2012
From: Balfour Beatty Infrastructure, Inc. Ural Yal						
REQUEST: Spec. Reference: 31 00 00  In order to control surface water at final subgrade, Balfour Beatty would like the option of installing (a) trench drain(s) per the attached drawing as necessary around the perimeter of the excavation just prior to or once final subgrade is established. These trench drains will be filled with ¾" drain rock in accordance with specification section 31 00 00-3.16.A. These trench drains will be left in place during micro-pile installation and remain below the mud slab. Water will be pumped out of these trench drains using sump pumps and/or routed to dewatering wells in accordance with specification section 31 23 19. Please confirm that this is acceptable.		ANSWER: Spec. Reference: 31 00 00  In order to control surface water at final subgrade, Balfour Beatty would like the option of installing (a) trench drain(s) per the attached drawing as necessary around the perimeter of the excavation just prior to or once final subgrade is established. These trench drains will be filled with ¾" drain rock in accordance with specification section 31 00 00-3.16.A. These trench drains will be left in place during micro-pile installation and remain below the mud slab. Water will be pumped out of these trench drains using sump pumps and/or routed to dewatering wells in accordance with specification section 31 23 19. Please confirm that this is acceptable.				
T-0311.1	Modified French Drains Zones 3 and 4	Closed	01	03/04/2014	03/14/2014	
From: Webcor Construction LP Claude Titcher						
REQUEST: Due to the varying dimensions between the edge of mud slab and the face of the CDSM wall, WOJV requests that the currently specified width (24 inches) of the French drain be maintained as a minimum at all locations for Zones 3 and 4. As a result, the width of the drain may be		ANSWER: Due to the varying dimensions between the edge of mud slab and the face of the CDSM wall, WOJV requests that the currently specified width (24 inches) of the French drain be maintained as a minimum at all locations for Zones 3 and 4. As a result, the width of				



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	up to 30 inches at the widest locations.  Please confirm this is acceptable.					the drain may be up to 30 inches at the widest locations.  Please confirm this is acceptable.
T-0312	Proximity Inquiry as to Beale St. Bridge Pile Location	Closed	01	09/19/2012	09/29/2012	09/20/2012
From: Webcor Construction LP                      Kirk Nielsen						
<b>REQUEST:</b>  BBII's sheet 1/SH-2105 (BBII submittal TZ1030-015313A31.1) calls for the 48" diameter CIDH column to be located 21'-6" off 35-line along E-line. As per sheet A1-2817 (TG06) the proposed location would obstruct, requiring redesign of the reinforcement, the construction of the structural wall separating the (2) deep pits depicted on 1/S1-3007 (TG06) in room B2761. The location of the pits and the wall separating the (2) pits were always depicted on S1-2027 (TG03). May the aforementioned CIDH column be located as proposed?						<b>ANSWER:</b>  BBII's sheet 1/SH-2105 (BBII submittal TZ1030-015313A31.1) calls for the 48" diameter CIDH column to be located 21'-6" off 35-line along E-line. As per sheet A1-2817 (TG06) the proposed location would obstruct, requiring redesign of the reinforcement, the construction of the structural wall separating the (2) deep pits depicted on 1/S1-3007 (TG06) in room B2761. The location of the pits and the wall separating the (2) pits were always depicted on S1-2027 (TG03). May the aforementioned CIDH column be located as proposed?



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<b>T-0313</b>	<b>Micropile Layout</b>	<b>Closed</b>	<b>01</b>	<b>09/13/2012</b>	<b>09/23/2012</b>	<b>09/20/2012</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Documents Specification Section: 31 63 33 Drawings: ASI #0097			Reference Documents Specification Section: 31 63 33 Drawings: ASI #0097			
Per 9/12/12 Turner BSE Progress Meeting, Adamsons Associates(AA) requested BBII to submit a RFI requesting distance tolerances for the proposed micropile layout relocations. Please see BBII's verbage below in response.			Per 9/12/12 Turner BSE Progress Meeting, Adamsons Associates(AA) requested BBII to submit a RFI requesting distance tolerances for the proposed micropile layout relocations. Please see BBII's verbage below in response.			
The response comments provided to submittal TA1020-316333A12.2 (TG0300-622.2) for micropile stated that the submitted micropile layout was unacceptable, but that the micropile locations shown in the TG0600 (ASI 0097) documents are acceptable. The attached marked up coordination drawings show the locations of the TG0600 documents micropile locations compared to various overhead horizontal and vertical obstructions. The obstructions considered in this comparison include trestle pile and bracing; internal bracing struts, supports, and pin piles; bridge piles; and the buttress walls. The submitted micropile locations are also shown.			The response comments provided to submittal TA1020-316333A12.2 (TG0300-622.2) for micropile stated that the submitted micropile layout was unacceptable, but that the micropile locations shown in the TG0600 (ASI 0097) documents are acceptable. The attached marked up coordination drawings show the locations of the TG0600 documents micropile locations compared to various overhead horizontal and vertical obstructions. The obstructions considered in this comparison include trestle pile and bracing; internal bracing struts, supports, and pin piles; bridge piles; and the buttress walls. The submitted micropile locations are also shown.			
The equipment that will be used to install the micropiles require 2.5 feet clearance from the center of the micropile hole to surrounding obstructions. The circles and arrows on the attached drawing indicate which micropiles do not have the required clearance and which direction of shift is preferred. The maximum shift is 4 feet, which occurs when a micropile is located directly below an internal bracing strut.			The equipment that will be used to install the micropiles require 2.5 feet clearance from the center of the micropile hole to surrounding obstructions. The circles and arrows on the attached drawing indicate which micropiles do not have the required clearance and which direction of shift is preferred. The maximum shift is 4 feet, which occurs when a micropile is located directly below an internal bracing strut.			
Please confirm that the micropile locations shown on the TG0600 documents are to be used for the micropile layout, and that a shift of up to 4 feet in the directions shown on the attached drawings is acceptable.			Please confirm that the micropile locations shown on the TG0600 documents are to be used for the micropile layout, and that a shift of up to 4 feet in the directions shown on the attached drawings is acceptable.			
As an alternative, BBII would prefer to use the submitted layout which has fewer conflicts. Micropiles would be eliminated or added per notes 2 and 3 respectively on sheet ML-1 of the returned submittal. The submitted micropile layout contains 1858 each micropiles. The TG0600 documents contain 1860 each micropiles. By eliminating piles per comments 2 and adding piles per comment 3, the total quantity would be approximately the quantity in the TG0600 documents.			As an alternative, BBII would prefer to use the submitted layout which has fewer conflicts. Micropiles would be eliminated or added per notes 2 and 3 respectively on sheet ML-1 of the returned submittal. The submitted micropile layout contains 1858 each micropiles. The TG0600 documents contain 1860 each micropiles. By eliminating piles per comments 2			



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T-0314	<b>Permit Clarification</b>  From: Webcor Construction LP Robert Kjome  <b>REQUEST:</b> Reference Specification: 01 14 10 - 2 1.2A  Pursuant to specification section 01 14 10 - 2 1.2A, the Contractor is directed to obtain permits from the San Francisco Department of Building Inspection(DBI) for work including, but not limited to: Excavation, Structural, Architectural, Mechanical, Plumbing, and Electrical.  To date TJPA has been acting as the permitting authority, and has distributed permits for work contracturally required to be authorized by the DBI.  Please confirm that W/O is to obtain these permits through the TJPA, not the DBI.	Closed	01	09/14/2012	09/24/2012	09/19/2012
						<b>ANSWER:</b> Reference Specification: 01 14 10 - 2 1.2A  Pursuant to specification section 01 14 10 - 2 1.2A, the Contractor is directed to obtain permits from the San Francisco Department of Building Inspection(DBI) for work including, but not limited to: Excavation, Structural, Architectural, Mechanical, Plumbing, and Electrical.  To date TJPA has been acting as the permitting authority, and has distributed permits for work contracturally required to be authorized by the DBI.  Please confirm that W/O is to obtain these permits through the TJPA, not the DBI.





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<b>T-0315</b>	<b>Performance Test Micropile Layout</b>	<b>Closed</b>	<b>01</b>	<b>09/17/2012</b>	<b>09/27/2012</b>	<b>09/27/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification: 31 63 33 Reference Drawing: S1-2022			Reference Specification: 31 63 33 Reference Drawing: S1-2022			
Drawing S1-2022 shows the Zone 1 performance test micropile on gridline E near gridline 2. BBII proposes to locate the Zone 1 test piles per the attached sketch. More than 1 test pile will be installed at this location. The additional test piles are to be installed at BBII's option for verification of design assumptions. They will be installed at no additional cost and will not take the place of any other test piles in other zones. Please confirm that it is acceptable to install the performance test micropiles at the locations shown on the attached drawing.			Drawing S1-2022 shows the Zone 1 performance test micropile on gridline E near gridline 2. BBII proposes to locate the Zone 1 test piles per the attached sketch. More than 1 test pile will be installed at this location. The additional test piles are to be installed at BBII's option for verification of design assumptions. They will be installed at no additional cost and will not take the place of any other test piles in other zones. Please confirm that it is acceptable to install the performance test micropiles at the locations shown on the attached drawing.			
<b>T-0316</b>	<b>Becho's Request for Modification of Shafts T3.5 and T4.5</b>	<b>Closed</b>	<b>01</b>	<b>09/20/2012</b>	<b>09/30/2012</b>	<b>09/21/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ernie Cortez						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Specification Reference: 31.63.29 Drawing Reference: GT-2201			Specification Reference: 31.63.29 Drawing Reference: GT-2201			
Reference attached Becho Letter BI-0271.			Reference attached Becho Letter BI-0271.			
Becho recognized that the shaft installed on 9/13/12 (believed to be T3.5) was poured in the location of Buttress shaft T2.5. Attached is Becho's proposal to rectify the installation of Buttress Shaft T2.5.			Becho recognized that the shaft installed on 9/13/12 (believed to be T3.5) was poured in the location of Buttress shaft T2.5. Attached is Becho's proposal to rectify the installation of Buttress Shaft T2.5.			
Please confirm that Becho's proposal is acceptable.			Please confirm that Becho's proposal is acceptable.			
<b>T-0317</b>	<b>Demolition and Excavation Limit Associated with the Sub Grade</b>	<b>Closed</b>	<b>01</b>	<b>09/21/2012</b>	<b>10/01/2012</b>	<b>09/27/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Joe Chapman						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification: 31-00-00 Reference Drawings: GT-2101, D-5100, S1-2022, M1-2022			Reference Specification: 31-00-00 Reference Drawings: GT-2101, D-5100, S1-2022, M1-2022			
Drawings D-5100 shows the demolition depth of the Test Buttress Shaft to EL -41.5', and the demolition depth of the			Drawings D-5100 shows the demolition depth of the Test Buttress Shaft to EL -41.5', and the demolition			



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	80 Natoma Piles to EL -44.5'. Please confirm that these elevations are sufficient for future trades, and slab depressions.					depth of the 80 Natoma Piles to EL -44.5'. Please confirm that these elevations are sufficient for future trades, and slab depressions.
T-0317.1	BSE -Demolition and Excavation Limit Associated with the Sub Grade Follow-Up	Closed	01	10/01/2012	10/01/2012	10/09/2012
From: Webcor Construction LP David Fields						
REQUEST:						ANSWER:
BSE Drawing M-0006 states that GHEX piping loops will be installed 12" below the mud slab.						BSE Drawing M-0006 states that GHEX piping loops will be installed 12" below the mud slab.
Below Grade Drawing M-0006 (Issued with FO T-00010 R2) states that GHEX piping loops shall be installed 24" below the mud slab, drop in elevation with the contours of any depressions while maintaining 24" of depth, and offset where required around Micropiles and Trestle Piles.						Below Grade Drawing M-0006 (Issued with FO T-00010 R2) states that GHEX piping loops shall be installed 24" below the mud slab, drop in elevation with the contours of any depressions while maintaining 24" of depth, and offset where required around Micropiles and Trestle Piles.
BSE Drawing D-5100 dictates a specific demolition depth of - 41.5' for the Drilled Shaft Prototype and - 44.5' for the 80 Natoma shoring wall.						BSE Drawing D-5100 dictates a specific demolition depth of - 41.5' for the Drilled Shaft Prototype and - 44.5' for the 80 Natoma shoring wall.
Given the disparity above and the revision to pit locations within FO-00010 R2 W/O has detected the following conflicts to Geothermal Piping Loops:						Given the disparity above and the revision to pit locations within FO-00010 R2 W/O has detected the following conflicts to Geothermal Piping Loops:
- 80 Natoma Shoring wall with Pit location at Gridline H-2 ( - 44' - 9' Final Subgrade Elevation)						- 80 Natoma Shoring wall with Pit location at Gridline H-2 ( - 44' - 9' Final Subgrade Elevation)
- Drilled Shaft Prototype ( - 41' - 5" Final Subgrade Elevation)						- Drilled Shaft Prototype ( - 41' - 5" Final Subgrade Elevation)
Please specify a specific grade to demolish the aforementioned obstructions in order to avoid the GHEX piping loops and advise as to any additional conflicts.						Please specify a specific grade to demolish the aforementioned obstructions in order to avoid the GHEX piping loops and advise as to any additional conflicts.
T-0317.2	BSE - Buttress Demolition Limits Relative to Sub Grade Elevations	Closed	01	10/15/2012	10/25/2012	10/19/2012
From: Balfour Beatty Infrastructure, Inc. Joe Chapman						



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	<p><b>REQUEST:</b></p> <p>During the 10/10/12 MRP meeting ARUP indicated channels, pursuant to RFI response #T-0319, were not required at the Northwest corner levels C&amp;D. Please confirm.</p>					
	<p><b>ANSWER:</b></p> <p>During the 10/10/12 MRP meeting ARUP indicated channels, pursuant to RFI response #T-0319, were not required at the Northwest corner levels C&amp;D. Please confirm.</p>					
<b>T-0320</b>	<b>BSE - Ground Level Structural Beams at Gridlines 34 and 34.8</b>	<b>Closed</b>	<b>01</b>	<b>09/25/2012</b>	<b>10/05/2012</b>	<b>10/02/2012</b>
	<p><b>From:</b> Webcor Construction LP                      David Fields</p> <p><b>REQUEST:</b></p> <p>Reference: 100% Superstructure Package Drawings S1-2307, 1/S1-3206</p> <p>To coordinate the location of the Beale St. Bridge with future work please provide the dimensions for the Ground Level structural beams located at Gridlines 34 and 34.8.</p>					
	<p><b>ANSWER:</b></p> <p>Reference: 100% Superstructure Package Drawings S1-2307, 1/S1-3206</p> <p>To coordinate the location of the Beale St. Bridge with future work please provide the dimensions for the Ground Level structural beams located at Gridlines 34 and 34.8.</p>					





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T-0321.1	Additional Excavation and Bracing Constraints at A Line and 301 Mission	Closed	01	10/10/2012	10/20/2012	10/19/2012
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Dean Wallahan						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>BBII would like to confirm the following direction received at TCCO's weekly meeting on October 10, 2012 in regards to the limits of the berm and sequence of work referenced in the response to RFI T-0321.</p> <p>The following work will take place between CDSM beams 251 and 276 along the A-Line and the southern edge of Zone 3's trestle.</p> <p>Demolition: Upon completion of the demolition and removal of the basement walls and footings BBII is to place an earth berm to elevation +10.00, extending 25 feet from the face of the CDSM wall into the excavation and having a 3:1 slope at the southern hinge point of the berm.</p> <p>Bracing: Walers 24 and 48 as well as Struts 49 and 50 will be installed within a 6 working day window to address ARUP's concern of overexposure from the Millennium's Building's foundation pressure on the CDSM wall. During the installation of these walers and struts the berm as described in the demolition section above will remain between CDSM beams 260 and 271 until completion of the bracing of walers 24 and 48 and struts 49-50. The sequence will be repeated for installation of walers 25 and 49 as well as struts 51 and 52 with the exception of the earth berm easterly limit will be CDSM beam 276 (centerline of buttress A line pile).</p>			<p>BBII would like to confirm the following direction received at TCCO's weekly meeting on October 10, 2012 in regards to the limits of the berm and sequence of work referenced in the response to RFI T-0321.</p> <p>The following work will take place between CDSM beams 251 and 276 along the A-Line and the southern edge of Zone 3's trestle.</p> <p>Demolition: Upon completion of the demolition and removal of the basement walls and footings BBII is to place an earth berm to elevation +10.00, extending 25 feet from the face of the CDSM wall into the excavation and having a 3:1 slope at the southern hinge point of the berm.</p> <p>Bracing: Walers 24 and 48 as well as Struts 49 and 50 will be installed within a 6 working day window to address ARUP's concern of overexposure from the Millennium's Building's foundation pressure on the CDSM wall. During the installation of these walers and struts the berm as described in the demolition section above will remain between CDSM beams 260 and 271 until completion of the bracing of walers 24 and 48 and struts 49-50. The sequence will be repeated for installation of walers 25 and 49 as well as struts 51 and 52 with the exception of the earth berm easterly limit will be CDSM beam 276 (centerline of buttress A line pile).</p>			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
T-0321.1R	Additional Excavation and Bracing Constraints at A Line and 301 Mission REVISIO Closed		01	10/26/2012	11/05/2012	10/26/2012
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Dean Wallahan						
<b>REQUEST:</b>			<b>ANSWER:</b>			
BBII would like to confirm the following direction received at TCCO's weekly meeting on October 10, 2012 in regards to the limits of the berm and sequence of work referenced in the response to RFI T-0321.			BBII would like to confirm the following direction received at TCCO's weekly meeting on October 10, 2012 in regards to the limits of the berm and sequence of work referenced in the response to RFI T-0321.			
The following work will take place between CDSM beams 251 and 276 along the A-Line and the southern edge of Zone 3's trestle.			The following work will take place between CDSM beams 251 and 276 along the A-Line and the southern edge of Zone 3's trestle.			
Demolition: Upon completion of the demolition and removal of the basement walls and footings BBII is to place an earth berm to elevation +10.00, extending 25 feet from the face of the CDSM wall into the excavation and having a 3:1 slope at the southern hinge point of the berm.			Demolition: Upon completion of the demolition and removal of the basement walls and footings BBII is to place an earth berm to elevation +10.00, extending 25 feet from the face of the CDSM wall into the excavation and having a 3:1 slope at the southern hinge point of the berm.			
Bracing: Walers 24 and 48 as well as Struts 49 and 50 will be installed within a 6 working day window to address ARUP's concern of overexposure from the Millennium's Building's foundation pressure on the CDSM wall. During the installation of these walers and struts the berm as described in the demolition section above will remain between CDSM beams 260 and 271 until completion of the bracing of walers 24 and 48 and struts 49-50. The sequence will be repeated for installation of walers 25 and 49 as well as struts 51 and 52 with the exception of the earth berm easterly limit will be CDSM beam 276 (centerline of buttress A line pile).			Bracing: Walers 24 and 48 as well as Struts 49 and 50 will be installed within a 6 working day window to address ARUP's concern of overexposure from the Millennium's Building's foundation pressure on the CDSM wall. During the installation of these walers and struts the berm as described in the demolition section above will remain between CDSM beams 260 and 271 until completion of the bracing of walers 24 and 48 and struts 49-50. The sequence will be repeated for installation of walers 25 and 49 as well as struts 51 and 52 with the exception of the earth berm easterly limit will be CDSM beam 276 (centerline of buttress A line pile).			





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0322</b>	<b>BSE - Dewatering Pipe Termination at System Removal</b>	<b>Closed</b>	<b>01</b>	<b>10/03/2012</b>	<b>10/13/2012</b>	<b>10/08/2012</b>
<div><div><b>From:</b> Webcor Construction LP      David Fields</div><div><b>REQUEST:</b> Upon system removal, specification 31 23 19 (BSE Documents) requires the contractor to fill dewatering pipes with grout, cut, and cap to an elevation 36" below subgrade. Sheet A1-8711 (Below Grade Documents) shows in detail the final configuration of the dewatering pipes and requires that they are capped at 8" below Top of Mat Slab elevation.  Will Cutting and Capping of the dewatering pipes be required at 36" below subgrade?  Assuming the dewatering pipes will be cut and capped at 8" below Top of Mat Slab elevation: Is it acceptable to have a void space in the abandoned dewatering pipes between the grout terminating 36" below subgrade elevation to the Bentonite at 14" below top of mat slab?</div><div><b>ANSWER:</b> Upon system removal, specification 31 23 19 (BSE Documents) requires the contractor to fill dewatering pipes with grout, cut, and cap to an elevation 36" below subgrade. Sheet A1-8711 (Below Grade Documents) shows in detail the final configuration of the dewatering pipes and requires that they are capped at 8" below Top of Mat Slab elevation.  Will Cutting and Capping of the dewatering pipes be required at 36" below subgrade?  Assuming the dewatering pipes will be cut and capped at 8" below Top of Mat Slab elevation: Is it acceptable to have a void space in the abandoned dewatering pipes between the grout terminating 36" below subgrade elevation to the Bentonite at 14" below top of mat slab?</div></div>						
<b>T-0322.1</b>	<b>BSE - Dewatering Pipe Termination at System Removal Follow-Up</b>	<b>Closed</b>	<b>CR</b>	<b>10/08/2012</b>	<b>10/18/2012</b>	<b>10/10/2012</b>
<div><div><b>From:</b> Webcor Construction LP      David Fields</div><div><b>REQUEST:</b> In follow up to RFI T-0322:  Upon dewatering system removal BSE Specification 31 23 19 3.9 requires that abandoned piping be filled with grout to an elevation of 36" below subgrade elevation consistent with the originally specified cut and cap elevation. Below Grade Drawing A-8711 does not specify a grout requirement for the dewatering pipes.  Is it acceptable to have a void space in the abandoned dewatering pipes between the grout terminating 36" below subgrade elevation to the Bentonite at 14" below top of mat slab consistent with the current contract documents?</div><div><b>ANSWER:</b> In follow up to RFI T-0322:  Upon dewatering system removal BSE Specification 31 23 19 3.9 requires that abandoned piping be filled with grout to an elevation of 36" below subgrade elevation consistent with the originally specified cut and cap elevation. Below Grade Drawing A-8711 does not specify a grout requirement for the dewatering pipes.  Is it acceptable to have a void space in the abandoned dewatering pipes between the grout terminating 36" below subgrade elevation to the Bentonite at 14" below top of mat slab consistent with the current contract documents?</div></div>						
<b>T-0323</b>	<b>Modification of E-line Due to Shortened Shaft E3</b>	<b>Closed</b>	<b>01</b>	<b>10/03/2012</b>	<b>10/13/2012</b>	<b>10/03/2012</b>



# 30100 - Transbay Transit Center Project



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0323.4</b>	<b>BSE - Confirmation of Buttress E-4 Installation</b>	<b>Closed</b>	<b>01</b>	<b>01/17/2013</b>	<b>01/27/2013</b>	<b>01/18/2013</b>
<b>From:</b> Webcor Construction LP      Kirk Nielsen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Drawing: GT2201			Reference Drawing: GT2201			
Please confirm the verbal direction given after the 1/17/13 8:30 Buttress Meeting that shaft E-4 is to be tangential rather than secant as described in RFI response #T-0323.1.			Please confirm the verbal direction given after the 1/17/13 8:30 Buttress Meeting that shaft E-4 is to be tangential rather than secant as described in RFI response #T-0323.1.			



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-0324	BSE - Field Order T-00010R2 - Clouded Revisions	Closed	01	10/04/2012	10/14/2012	10/15/2012
From: Webcor Construction LP                      Joanne Filipas						

**REQUEST:**

Reference: Field Order T-00010R2, TJP A CADD Standards Manual dated 15Nov10 and Sheet A1-8711 attached.

Field Order T-00010R2 included the TG06 Below Grade IFC drawings and specifications. It is unclear what revisions are to be incorporated by the TG03 contractor as the revised drawings do not include revision blocks and clouds consistent with the TJP A CADD Standards. For example, sheet A1-8711 (attached) was Issued For Construction with the TG03 BSE package. The revisions to this drawing through the design development and issuance with the TG06 bid/construction set are not clouded and the revision block does not include all previous revision descriptions. The revision block on the final Issued for Construction drawing should read as follows and all changes from the Rev 0 IFC issuance should be clouded in accordance with the TJP A CADD Standards:

No	Date	Description
Ä0	12/10/2010	Issued For Construction - Buttress/Shoring/Excavation
ÄÄ	4/18/2012	Issued for Bid - Below Grade Package
ÄB	8/17/2012	Issued for Bid - Below Grade Package - Addendum #2
Ä1	8/30/2012	Issued for Construction- Below Grade Package

Please confirm any previously issued IFC drawings that have since been revised will be re-issued consistent with the TJP A CADD standards. Also, please confirm all packages going forward will be in accordance with the TJP A CADD standards revision provisions.

**ANSWER:**

Reference: Field Order T-00010R2, TJP A CADD Standards Manual dated 15Nov10 and Sheet A1-8711 attached.

Field Order T-00010R2 included the TG06 Below Grade IFC drawings and specifications. It is unclear what revisions are to be incorporated by the TG03 contractor as the revised drawings do not include revision blocks and clouds consistent with the TJP A CADD Standards. For example, sheet A1-8711 (attached) was Issued For Construction with the TG03 BSE package. The revisions to this drawing through the design development and issuance with the TG06 bid/construction set are not clouded and the revision block does not include all previous revision descriptions. The revision block on the final Issued for Construction drawing should read as follows and all changes from the Rev 0 IFC issuance should be clouded in accordance with the TJP A CADD Standards:

No	Date	Description
Ä0	12/10/2010	Issued For Construction - Buttress/Shoring/Excavation
ÄÄ	4/18/2012	Issued for Bid - Below Grade Package
ÄB	8/17/2012	Issued for Bid - Below Grade Package - Addendum #2
Ä1	8/30/2012	Issued for Construction- Below Grade Package

Please confirm any previously issued IFC drawings that have since been revised will be re-issued consistent with the TJP A CADD standards. Also, please confirm all packages going forward will be in accordance with the TJP A CADD standards revision provisions.



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0324.1</b>	<b>Field Order T-00010R2 - Clouded Revisions</b>	<b>Closed</b>	<b>01</b>	<b>10/17/2012</b>	<b>10/27/2012</b>	<b>10/23/2012</b>
<b>From:</b> Webcor Construction LP      Kirk Nielsen						
<b>REQUEST:</b> In follow up to RFI response #T-0324 and the 10/17/12 BSE meeting it was clarified by AAI that what W/O was requesting in RFI #T-0324 was actually a "revision set for TG03". Please provide.						<b>ANSWER:</b> In follow up to RFI response #T-0324 and the 10/17/12 BSE meeting it was clarified by AAI that what W/O was requesting in RFI #T-0324 was actually a "revision set for TG03". Please provide.
<b>T-0325</b>	<b>BSE - Excavation Sequence Relative to Installation of Struts 10 &amp; 11</b>	<b>Closed</b>	<b>CR</b>	<b>10/05/2012</b>	<b>10/15/2012</b>	<b>10/11/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b> In an effort to expedite the installation of struts 10 & 11 at level D to help reduce eastward movement of the A-line wall, BBII proposes the following:  Excavate to level D for struts STD-10 and STD-11, and notch along the wall so that waler WD-05 may be installed, leaving the berm present beyond the notch. Excavate on the south side to the end of waler WD-67. Excavation to install strut STD-12 will proceed once enough struts have been installed at level C to advance the level D excavation to strut STD-12 per the specifications.  A sketch has been attached for reference. Please confirm this is acceptable.						<b>ANSWER:</b> In an effort to expedite the installation of struts 10 & 11 at level D to help reduce eastward movement of the A-line wall, BBII proposes the following:  Excavate to level D for struts STD-10 and STD-11, and notch along the wall so that waler WD-05 may be installed, leaving the berm present beyond the notch. Excavate on the south side to the end of waler WD-67. Excavation to install strut STD-12 will proceed once enough struts have been installed at level C to advance the level D excavation to strut STD-12 per the specifications.  A sketch has been attached for reference. Please confirm this is acceptable.
<b>T-0326</b>	<b>Available Power Source for First Street Traffic Signal</b>	<b>Closed</b>	<b>01</b>	<b>10/15/2012</b>	<b>10/25/2012</b>	<b>10/19/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> First Street Bridge Temporary Traffic Signal is due to be activated on 10/24/2012. The traffic signal controller cabinet requires electrical power to activate the signal. Being that BBII will not be drawing power off site temporary power (Skids 3 and 4), please advise and provide direction for the use of an available power source.						<b>ANSWER:</b> First Street Bridge Temporary Traffic Signal is due to be activated on 10/24/2012. The traffic signal controller cabinet requires electrical power to activate the signal. Being that BBII will not be drawing power off site temporary power (Skids 3 and 4), please advise and provide direction for the use of an available power source.



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0327</b>	<b>GRD - Ground Rod Placement</b>	<b>Closed</b>	<b>01</b>	<b>10/16/2012</b>	<b>10/26/2012</b>	<b>10/31/2012</b>
<b>From:</b> BASS Electric Jerry Brys						
<b>REQUEST:</b>  There are no dimensions shown for placement of the ground rods. Should we scale off the drawings or will a drawing be produced reflecting dimensions?						<b>ANSWER:</b>  There are no dimensions shown for placement of the ground rods. Should we scale off the drawings or will a drawing be produced reflecting dimensions?
<b>T-0328</b>	<b>BSE - Re-Bracing Elevations</b>	<b>Closed</b>	<b>01</b>	<b>10/17/2012</b>	<b>10/17/2012</b>	<b>11/01/2012</b>
<b>From:</b> Webcor Construction LP David Fields						
<b>REQUEST:</b>  Drawing GT-1112 stage 13 shows a maximum of 16' or 17' between level B struts and the lower level of rebracing. For Case West, level B supports are at elevation -3 ', resulting in the lower level of rebracing supports at elevation -20'. Internal bracing drawing sheet SH-4000 shows W21 strut support members on the underside of level C bracing. In order to install the lower level rebracing and accommodate the existing level C bracing, the lower level bracing will need to be installed at elevation -22'.  Similarly, the top level of rebracing is called out in stage 15 to be 3' below level A bracing. Top level rebracing will need to be 5' below level A bracing in order for struts to be clear of the overhead strut supports.  Please confirm that the 17' and 16' maximum dimensions in stage 13 and 3' maximum dimension in stage 15 will not be required if the rebracing design calculations show that it is acceptable.						<b>ANSWER:</b>  Drawing GT-1112 stage 13 shows a maximum of 16' or 17' between level B struts and the lower level of rebracing. For Case West, level B supports are at elevation -3 ', resulting in the lower level of rebracing supports at elevation -20'. Internal bracing drawing sheet SH-4000 shows W21 strut support members on the underside of level C bracing. In order to install the lower level rebracing and accommodate the existing level C bracing, the lower level bracing will need to be installed at elevation -22'.  Similarly, the top level of rebracing is called out in stage 15 to be 3' below level A bracing. Top level rebracing will need to be 5' below level A bracing in order for struts to be clear of the overhead strut supports.  Please confirm that the 17' and 16' maximum dimensions in stage 13 and 3' maximum dimension in stage 15 will not be required if the rebracing design calculations show that it is acceptable.
<b>T-0329</b>	<b>BGP - Proposed Construction Joint Layout</b>	<b>Closed</b>	<b>01</b>	<b>10/24/2012</b>	<b>11/03/2012</b>	<b>10/31/2012</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  Reference sketches: SCCI #1, SCCI #2 Reference Drawing: S-0007 Reference Specification: 03 30 20  Per note CJ-2 on sheet S-0007 No horizontal construction joints will be permitted unless specifically shown in the drawings or approved in writing. Please confirm that the						<b>ANSWER:</b>  Reference sketches: SCCI #1, SCCI #2 Reference Drawing: S-0007 Reference Specification: 03 30 20  Per note CJ-2 on sheet S-0007 No horizontal construction joints will be permitted unless specifically shown in the drawings or approved in writing. Please



<i>Number</i>	<i>Subject</i>	<i>Status</i>	<i>Choice Required</i>	<i>Date Created</i>	<i>Date Required</i>	<i>Date Answered</i>
T-0330	longitudinal construction joint shown between gridlines G and K is acceptable as it follows the micropile construction sequence and it will help the schedule with re-bracing in the Southwest Corner.	Closed	01	10/30/2012	11/09/2012	11/09/2012
	confirm that the longitudinal construction joint shown between gridlines G and K is acceptable as it follows the micropile construction sequence and it will help the schedule with re-bracing in the Southwest Corner.					
T-0331	<b>BSE - Mud Slab Vapor Retarder</b>	Closed	01	10/30/2012	11/09/2012	11/09/2012
	<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Reference Drawing : A1-8711 S1-3003 Reference Specification: 03 30 00</p> <p>Specification 03 30 00.3 .I.E, Vapor Retarder Placement:: See Division 7, Thermal and Moisture Protection, describes installation of vapor retarder. Specification 03 30 00.3 .4.A.13 states "Place vapor retarder directly below slabs on grade as specified in contract documents."</p> <p>Vapor retarder is not referenced on Detail 5, Mud Slab Detail, on sheet S1-3003, or on any of the slab penetration details on sheets A1-8711 and S1-3003 .</p> <p>Please verify whether or not vapor retarder is required.</p>					
T-0331	<b>BGP - Geothermal Maximum Horizontal Loop or Ground Loop Zone Length</b>	Closed	01	10/31/2012	11/10/2012	11/05/2012
	<p><b>From:</b> Webcor Construction LP                      David Fields</p> <p><b>REQUEST:</b></p> <p>Reference: 23 57 34</p> <p>Please confirm that there is no restriction on GHEX Horizontal Loop or Ground Loop Zone length.</p>					
	<p><b>ANSWER:</b></p> <p>Reference Drawing : A1-8711 S1-3003 Reference Specification: 03 30 00</p> <p>Specification 03 30 00.3 .I.E, Vapor Retarder Placement:: See Division 7, Thermal and Moisture Protection, describes installation of vapor retarder. Specification 03 30 00.3 .4.A.13 states "Place vapor retarder directly below slabs on grade as specified in contract documents."</p> <p>Vapor retarder is not referenced on Detail 5, Mud Slab Detail, on sheet S1-3003, or on any of the slab penetration details on sheets A1-8711 and S1-3003 .</p> <p>Please verify whether or not vapor retarder is required.</p>					
	<p><b>ANSWER:</b></p> <p>Reference: 23 57 34</p> <p>Please confirm that there is no restriction on GHEX Horizontal Loop or Ground Loop Zone length.</p>					



Webcor/Obayashi Joint Venture

PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

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<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
<b>T-0332</b>	<b>BSE - Micropile W203 Relocation</b>	<b>Closed</b>	<b>01</b>	<b>11/01/2012</b>	<b>11/11/2012</b>	<b>11/02/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Micropile 203 as laid out is too close to a piezometer well. BBII proposes moving pile W203 East 4'-9.5" and South 1'-.75". See attached sketch.  Please confirm this is acceptable.		<b>ANSWER:</b>  Micropile 203 as laid out is too close to a piezometer well. BBII proposes moving pile W203 East 4'-9.5" and South 1'-.75". See attached sketch.  Please confirm this is acceptable.				
<b>T-0333</b>	<b>BSE - Utilization of the Mat Slab for Re-Bracing Reactions</b>	<b>Closed</b>	<b>01</b>	<b>11/01/2012</b>	<b>11/11/2012</b>	<b>11/07/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>  Is utilizing the mat slab for re-bracing reactions (via rackers) acceptable provided it meets the provisions set forth within 31 55 00 1.5 Q in regards to connections, penetrations, imbeds, and restoration?		<b>ANSWER:</b>  Is utilizing the mat slab for re-bracing reactions (via rackers) acceptable provided it meets the provisions set forth within 31 55 00 1.5 Q in regards to connections, penetrations, imbeds, and restoration?				
<b>T-0333.1</b>	<b>BSE - BSE - Utilization of the Mat Slab for Re-Bracing Reactions Follow-Up</b>	<b>Closed</b>	<b>01</b>	<b>11/07/2012</b>	<b>11/17/2012</b>	<b>11/13/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>  Response to RFI T-0333 stipulates that the contractor:  "..submit further information on this proposed alternative.."  This statement implies that the utilization of the mat slab for rebracing reactions is a deviation from what is required by contract. Please identify the primary method the rebracing design is to employ in order resist gravity, seismic, or other additional loading to be resisted and/or provide restraint against buckling, torsion, or other function as necessary per the design to provide required capacities of elements.		<b>ANSWER:</b>  Response to RFI T-0333 stipulates that the contractor:  "..submit further information on this proposed alternative.."  This statement implies that the utilization of the mat slab for rebracing reactions is a deviation from what is required by contract. Please identify the primary method the rebracing design is to employ in order resist gravity, seismic, or other additional loading to be resisted and/or provide restraint against buckling, torsion, or other function as necessary per the design to provide required capacities of elements.				
<b>T-0333.2</b>	<b>BSE - Utilization of the Mat Slab for Re-Bracing Reactions Follow-Up</b>	<b>Closed</b>	<b>01</b>	<b>11/09/2012</b>	<b>11/19/2012</b>	<b>11/20/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>  RFI T-0333 inquired if utilizing the mat slab for re-bracing		<b>ANSWER:</b>  RFI T-0333 inquired if utilizing the mat slab for re-				





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>reactions was acceptable provided it meets the provisions set forth within 31 55 00 1.5 Q in regards to connections, penetrations, imbeds, and restoration.</p> <p>The response stated that structurally it was acceptable provided the contractor outline if there will be a cost and schedule reduction pursuant to the specification requirements for "Proposed Alternatives". W/O is unable to locate a specification provision for "Proposed Alternatives" in the TG03 or TG06 contract documents.</p> <p>Is utilizing the mat slab for rebracing reactions acceptable pursuant to the TG03 or TG06 contract documents?</p> <p>If acceptable, please identify the specification section for "Proposed Alternatives" within the TG03 or TG06 documents so cost and schedule reduction proposals can be provided pursuant to the applicable requirements.</p> <p>Additionally, please identify the TG03 and/or TG06 contract requirements for secondary bracing (31 55 00 1.3D) geometry.</p>					
<b>T-0334</b>	<b>BGP - Catch Basin Elevation at Gridlines 14 and B.3.</b>	<b>Closed</b>	<b>01</b>	<b>11/01/2012</b>	<b>11/11/2012</b>	<b>11/02/2012</b>
<b>From:</b> Webcor Construction LP      David Fields						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: A1-2814		Reference: A1-2814				
Please provide the elevation for the catch basin located along gridlines 14 and B.3.		Please provide the elevation for the catch basin located along gridlines 14 and B.3.				



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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

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T-0335	BGP - Contract Bury Bar for Support	Closed	01	11/05/2012	11/15/2012	11/10/2012
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference Specification: 03 30 00		Reference Specification: 03 30 00				
Please confirm it is acceptable to displace a top mat 4th layer contract reinforcing bar and a bottom mat 2nd layer contract reinforcing bar one bar diameter every 6' - 0" +/- oc to support the mat reinforcing. A sketch is attached for reference and to graphically represent the proposed bar configuration.		Please confirm it is acceptable to displace a top mat 4th layer contract reinforcing bar and a bottom mat 2nd layer contract reinforcing bar one bar diameter every 6' - 0" +/- oc to support the mat reinforcing. A sketch is attached for reference and to graphically represent the proposed bar configuration.				
T-0336	BGP - Wall Dowels Standard Hooks	Closed	01	11/05/2012	11/15/2012	11/10/2012
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference Specification: 03 20 00 Reference Drawings: SI-3201		Reference Specification: 03 20 00 Reference Drawings: SI-3201				
Contract drawing S1-3201, Section 1 depicts the #11 vertical wall dowels with a terminator, typ. embedded into the mat foundation rather than a standard hook. Shimmick is requesting the option to utilize a #11 standard hook (1' - 7") orientated inward or a terminator as shown at these locations. Please verify that either option is acceptable for use.		Contract drawing S1-3201, Section 1 depicts the #11 vertical wall dowels with a terminator, typ. embedded into the mat foundation rather than a standard hook. Shimmick is requesting the option to utilize a #11 standard hook (1' - 7") orientated inward or a terminator as shown at these locations. Please verify that either option is acceptable for use.				
T-0337	BGP - Bottom Mat Reinforcing Clear Cover to Edge	Closed	01	11/06/2012	11/16/2012	11/12/2012
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference Specification: 03 30 00 Reference Drawings: S1-3201		Reference Specification: 03 30 00 Reference Drawings: S1-3201				
Contract drawing S1-3201 depicts the clear cover to the mat reinforcing as 6" along the edge. Please verify it is acceptable to extend the bottom mat reinforcing closer to the edge such that the clear cover along the edge is 2", the same as it is for the vertical wall reinforcement.		Contract drawing S1-3201 depicts the clear cover to the mat reinforcing as 6" along the edge. Please verify it is acceptable to extend the bottom mat reinforcing closer to the edge such that the clear cover along the edge is 2", the same as it is for the vertical wall reinforcement.				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0338</b>	<b>BGP - Mat Reinforcing Clear Cover, Exterior Face Wall Vertical Clear Cover.</b>	<b>Closed</b>	<b>01</b>	<b>11/06/2012</b>	<b>11/16/2012</b>	<b>11/10/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Specification: 03 20 00 Reference Drawing: S1-3001 / S1-3201  Please confirm the clear cover to the bottom mat reinforcing is 3" as called out on contract drawing sheet S1-3001, typical detail 5. Additionally please verify if the outside face vertical reinforcing bars can be lifted such that clear cover to this bar is 6" from the concrete below as it is for the inside face vertical bar as depicted on contract drawing S1-3201.		<b>ANSWER:</b>  Reference Specification: 03 20 00 Reference Drawing: S1-3001 / S1-3201  Please confirm the clear cover to the bottom mat reinforcing is 3" as called out on contract drawing sheet S1-3001, typical detail 5. Additionally please verify if the outside face vertical reinforcing bars can be lifted such that clear cover to this bar is 6" from the concrete below as it is for the inside face vertical bar as depicted on contract drawing S1-3201.				
<b>T-0339</b>	<b>BGP - Wall Reinforcing Clear Cover</b>	<b>Closed</b>	<b>01</b>	<b>11/06/2012</b>	<b>11/16/2012</b>	<b>11/15/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Specification: 03 20 00 Reference Drawing: S1-3201  Contract drawing sheet S1-3201 depicts extent lines showing the 2" clear cover to the vertical wall reinforcing bars. Please confirm that the cross ties will infringe on the 2" clear cover and that the design intent is to maintain the clear cover to the main vertical reinforcing.		<b>ANSWER:</b>  Reference Specification: 03 20 00 Reference Drawing: S1-3201  Contract drawing sheet S1-3201 depicts extent lines showing the 2" clear cover to the vertical wall reinforcing bars. Please confirm that the cross ties will infringe on the 2" clear cover and that the design intent is to maintain the clear cover to the main vertical reinforcing.				
<b>T-0340</b>	<b>BGP - IDEA Machine</b>	<b>Closed</b>	<b>01</b>	<b>11/06/2012</b>	<b>11/16/2012</b>	<b>11/15/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Specification: 03 20 00 Reference Drawings: N/A  Shimmick would like to request the use of the Schnell IDEA Machine. The IDEA Machine pre-assembles grade beam, columns or other "boundary" type elements by a process of resistance welding three (3) 1/4" ASTM 82 wires to the ASTM A706 reinforcing ties. This process provides a more secure and accurate tie configuration with a more effective and timely installation. Attached is CRSI's engineering data report #53 which provides an in-depth explanation of the process as well as their acceptance of		<b>ANSWER:</b>  Reference Specification: 03 20 00 Reference Drawings: N/A  Shimmick would like to request the use of the Schnell IDEA Machine. The IDEA Machine pre-assembles grade beam, columns or other "boundary" type elements by a process of resistance welding three (3) 1/4" ASTM 82 wires to the ASTM A706 reinforcing ties. This process provides a more secure and accurate tie configuration with a more effective and timely installation. Attached is CRSI's engineering data report #53 which provides an in-depth				



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	<p>the same. Also attached is a reference list of recent projects where this process has been approved and utilized as well as a testing report from Christensen Materials Engineering. Please confirm if this is acceptable.</p>					<p>explanation of the process as well as their acceptance of the same. Also attached is a reference list of recent projects where this process has been approved and utilized as well as a testing report from Christensen Materials Engineering. Please confirm if this is acceptable.</p>
<b>T-0341</b>	<b>BGP - One Piece Ties</b>	<b>Closed</b>	<b>01</b>	<b>11/06/2012</b>	<b>11/16/2012</b>	<b>11/10/2012</b>
<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Shimmick would like to request the use of the "one-piece" or "serpentine" ties at this project. These ties are made by an automatic bender that bends a column or boundary element tie from one continuous piece of rebar. The end result is the same perimeter and cross tie configuration as the design in the contract documents. Please confirm if this procedure is acceptable.</p>			<p>Shimmick would like to request the use of the "one-piece" or "serpentine" ties at this project. These ties are made by an automatic bender that bends a column or boundary element tie from one continuous piece of rebar. The end result is the same perimeter and cross tie configuration as the design in the contract documents. Please confirm if this procedure is acceptable.</p>			
<b>T-0341.1</b>	<b>BGP - Type D8 Column Serpentine Ties</b>	<b>Closed</b>	<b>CR</b>	<b>12/04/2013</b>	<b>12/11/2013</b>	<b>12/10/2013</b>
<p><b>From:</b> Webcor Construction LP                      Jackson Tukuafu</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Please refer to drawing S1-3305 and RFI T-0341.</p> <p>The response to RFI T-0341 accepted the use of one-piece/serpentine ties for the columns as proposed by Gerdau. Gerdau has found that the fabrication of a single piece serpentine tie for the type D8 column could pose safety risks. Therefore, Gerdau is proposing to fabricate the type D8 column ties with two pieces of serpentine ties. See the attached Gerdau sketch SK- T-0341.1 for details.</p> <p>Please confirm the alternate serpentine ties as shown in the attached sketch is acceptable.</p>			<p>Please refer to drawing S1-3305 and RFI T-0341.</p> <p>The response to RFI T-0341 accepted the use of one-piece/serpentine ties for the columns as proposed by Gerdau. Gerdau has found that the fabrication of a single piece serpentine tie for the type D8 column could pose safety risks. Therefore, Gerdau is proposing to fabricate the type D8 column ties with two pieces of serpentine ties. See the attached Gerdau sketch SK- T-0341.1 for details.</p> <p>Please confirm the alternate serpentine ties as shown in the attached sketch is acceptable.</p>			



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<b>T-0342</b>	<b>BGP - Mat Slab Reinforcing and Lap Ratio</b>	<b>Closed</b>	<b>01</b>	<b>11/06/2012</b>	<b>11/16/2012</b>	<b>11/20/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  In follow up to the 10/31/12 Value Engineering prosal meeting, please confirm it is acceptable to change the grade 60 #11 bars to grade 75 #10 bars in the mat slab reinforcing.  Please provide the increased lap ratio required for the change in grade and bar size.		<b>ANSWER:</b>  In follow up to the 10/31/12 Value Engineering prosal meeting, please confirm it is acceptable to change the grade 60 #11 bars to grade 75 #10 bars in the mat slab reinforcing.  Please provide the increased lap ratio required for the change in grade and bar size.				
<b>T-0343</b>	<b>BSE - Micropile W072 Relocation</b>	<b>Closed</b>	<b>01</b>	<b>11/09/2012</b>	<b>11/19/2012</b>	<b>11/12/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Micropile W072 as laid out is too close to overhead strut support BA-29. BBII Proposes moving pile W072 East 1' to provide adequate clearance See attached sketch.  Please confirm this is acceptable.		<b>ANSWER:</b>  Micropile W072 as laid out is too close to overhead strut support BA-29. BBII Proposes moving pile W072 East 1' to provide adequate clearance See attached sketch.  Please confirm this is acceptable.				
<b>T-0344</b>	<b>Micropile W073 and W074 Relocation</b>	<b>Closed</b>	<b>01</b>	<b>11/12/2012</b>	<b>11/22/2012</b>	<b>11/13/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Micropiles W073 and W074 as laid out are too close to overhead strut support BA-28. BBII proposes moving pile W073 West 2' and North 0.5' and pile W074 East 2' and South 0.5' to provide adequate clearance. See attached sketch.  Please confirm this is acceptable.		<b>ANSWER:</b>  Micropiles W073 and W074 as laid out are too close to overhead strut support BA-28. BBII proposes moving pile W073 West 2' and North 0.5' and pile W074 East 2' and South 0.5' to provide adequate clearance. See attached sketch.  Please confirm this is acceptable.				
<b>T-0345</b>	<b>BSE - CDSM Wall Parallel Stiffness for Bridge Design</b>	<b>Closed</b>	<b>01</b>	<b>11/13/2012</b>	<b>11/23/2012</b>	<b>11/15/2012</b>
<b>From:</b> Balfour Beatty Infrastructure, Inc.      Ural Yal						
<b>REQUEST:</b>  Specification Reference: 01 53 13.1.3D  BBII has recently received information in a non bridge design related correspondence that could impact the		<b>ANSWER:</b>  Specification Reference: 01 53 13.1.3D  BBII has recently received information in a non bridge design related correspondence that could impact the				



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	<p>already designed, permitted, and constructed First and Fremont St. Bridges. In an ARUP memo dated October 5th, 2012 bullet #2 states:</p> <p>"The Contractor has verbally attested that they designed the diagonal corner braces using an interpretation of Note 11 on sheet GT-1111 which yielded a key assumption which is that the CDSM wall is infinitely strong and infinitely stiff. This assumption is an inaccurate and unreasonable interpretation of this note and in no way does the note infer this".</p> <p>Although this comment is in reference to the internal bracing design, it also relates to the temporary bridge design. As noted on page 156 of the First and Fremont St Bridge structural calculations (attached), this same interpretation of note 11 on GT-1111 was used for the abutment shear key design. The Bridges have been designed, reviewed and approved by DPW under with the assumption that no additional deformation occurs at the base of the abutments. If in fact the CDSM wall is truly NOT infinitely strong or infinitely stiff parallel to the wall, BBII requests a value from the CDSM engineer of record that can used in our re-evaluation of the First and Fremont Bridges to ensure the existing design remains in compliance with the design criteria. Additionally this value would be used in the re-design of the Beale St. Bridge.</p>					
	<p>already designed, permitted, and constructed First and Fremont St. Bridges. In an ARUP memo dated October 5th, 2012 bullet #2 states:</p> <p>"The Contractor has verbally attested that they designed the diagonal corner braces using an interpretation of Note 11 on sheet GT-1111 which yielded a key assumption which is that the CDSM wall is infinitely strong and infinitely stiff. This assumption is an inaccurate and unreasonable interpretation of this note and in no way does the note infer this".</p> <p>Although this comment is in reference to the internal bracing design, it also relates to the temporary bridge design. As noted on page 156 of the First and Fremont St Bridge structural calculations (attached), this same interpretation of note 11 on GT-1111 was used for the abutment shear key design. The Bridges have been designed, reviewed and approved by DPW under with the assumption that no additional deformation occurs at the base of the abutments. If in fact the CDSM wall is truly NOT infinitely strong or infinitely stiff parallel to the wall, BBII requests a value from the CDSM engineer of record that can used in our re-evaluation of the First and Fremont Bridges to ensure the existing design remains in compliance with the design criteria. Additionally this value would be used in the re-design of the Beale St. Bridge.</p>					
<b>T-0346</b>	<b>BGP - Mat Slab Maximum Aggregate Size</b>	<b>Closed</b>	<b>01</b>	<b>11/15/2012</b>	<b>11/25/2012</b>	<b>11/21/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						
Specification Reference 30 30 20						
Shimmick is requesting approval of 1 inch nominal maximum aggregate size in lieu of the 3/4 inch nominal maximum aggregate size for the Mat Slab concrete. Shimmick's backup data indicates that concrete made with larger aggregate size (1 inch instead of 3/4 inch) produces lower drying shrinkage values mainly due to a reduction in the water consumption of the mix and a reduction in paste content.						
<b>ANSWER:</b>						
Specification Reference 30 30 20						
Shimmick is requesting approval of 1 inch nominal maximum aggregate size in lieu of the 3/4 inch nominal maximum aggregate size for the Mat Slab concrete. Shimmick's backup data indicates that concrete made with larger aggregate size (1 inch instead of 3/4 inch) produces lower drying shrinkage values mainly due to a reduction in the water consumption of the mix and a reduction in paste						



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content.						
<b>T-0347</b>	<b>Trim Steel Requirements for the Mud Slab</b>	<b>Closed</b>	<b>01</b>	<b>11/19/2012</b>	<b>11/29/2012</b>	<b>11/29/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Specification: 03 20 01 Reference Drawing: S1-3003  Please confirm that trim steel will not be required. If trim steel is required, provide the details for trim in the 4" mudslab where the #4 bars @ 18" are interrupted. Please reference the attached sketch.		<b>ANSWER:</b>  Reference Specification: 03 20 01 Reference Drawing: S1-3003  Please confirm that trim steel will not be required. If trim steel is required, provide the details for trim in the 4" mudslab where the #4 bars @ 18" are interrupted. Please reference the attached sketch.				
<b>T-0347.1</b>	<b>BSE - Mud Slab Trim Rebar</b>	<b>Closed</b>	<b>01</b>	<b>12/12/2012</b>	<b>12/22/2012</b>	<b>12/18/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Submittal: TG0300-340 Reference Sketch: 12B035_SK-1  Upon further review of contract requirements subsequent to the approval of the mud slab rebar shop drawings (TG0300-340) it does not appear that trim steel is required for penetrations in the mud slab.  Please confirm that trim steel at penetrations in the mud slab will not be required pending submission of a follow up "For Record Only" mud slab shop drawing submittal.		<b>ANSWER:</b>  Reference Submittal: TG0300-340 Reference Sketch: 12B035_SK-1  Upon further review of contract requirements subsequent to the approval of the mud slab rebar shop drawings (TG0300-340) it does not appear that trim steel is required for penetrations in the mud slab.  Please confirm that trim steel at penetrations in the mud slab will not be required pending submission of a follow up "For Record Only" mud slab shop drawing submittal.				





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<b>T-0348</b>	<b>BSE - Micropile W235 Relocation</b>	<b>Closed</b>	<b>01</b>	<b>11/20/2012</b>	<b>11/30/2012</b>	<b>11/20/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Micropile W235 as laid out cannot be effectively installed from the Trestle. BBII proposes moving pile W235 North 2' to provide adequate clearance. See attached sketch.  Please confirm this is acceptable.						<b>ANSWER:</b>  Micropile W235 as laid out cannot be effectively installed from the Trestle. BBII proposes moving pile W235 North 2' to provide adequate clearance. See attached sketch.  Please confirm this is acceptable.
<b>T-0349</b>	<b>BGP - Construction Joint Layout</b>	<b>Closed</b>	<b>01</b>	<b>11/20/2012</b>	<b>11/30/2012</b>	<b>11/21/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Specification: 03 30 20.3.2.A.3  Per specification 033020.3.2.A.3 the maximum construction joint spacing in the mat slab is 120 feet (E/W direction), 3.2.A.4 maximum construction joint spacing in the foundation wall, lower concourse slab, ramp slab, interior walls, and the ground floor concrete slab is 60 feet. Foundation wall, lower concourse floor slab, and ground floor construction joints shall align with the location of the mat slab joint below and 3.2.B.1 construction joints in floor slab shall be located within the central third of the span. Due to the moment frames along grid lines V, W, and X being angled Shimmick see's the attached drawings as the only viable construction joint layout to comply with all set forth specifications. Please advise if the mat slab, foundation wall, and lower concourse construction joint layout is acceptable?						<b>ANSWER:</b>  Reference Specification: 03 30 20.3.2.A.3  Per specification 033020.3.2.A.3 the maximum construction joint spacing in the mat slab is 120 feet (E/W direction), 3.2.A.4 maximum construction joint spacing in the foundation wall, lower concourse slab, ramp slab, interior walls, and the ground floor concrete slab is 60 feet. Foundation wall, lower concourse floor slab, and ground floor construction joints shall align with the location of the mat slab joint below and 3.2.B.1 construction joints in floor slab shall be located within the central third of the span. Due to the moment frames along grid lines V, W, and X being angled Shimmick see's the attached drawings as the only viable construction joint layout to comply with all set forth specifications. Please advise if the mat slab, foundation wall, and lower concourse construction joint layout is acceptable?
<b>T-0349.1</b>	<b>BGP - Construction Joint Layout</b>	<b>Closed</b>	<b>01</b>	<b>11/26/2012</b>	<b>12/06/2012</b>	<b>12/07/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Per specification 03 30 20.3.2.A.3 the maximum construction joint spacing in the mat slab is 120 feet (E/W direction), 3.2.A.4 maximum construction joint spacing in the foundation wall, lower concourse slab, ramp slab, interior walls, and the ground floor concrete slab is 60 feet. Foundation wall, lower concourse floor slab, and ground floor construction joints shall align with the location of the						<b>ANSWER:</b>  Per specification 03 30 20.3.2.A.3 the maximum construction joint spacing in the mat slab is 120 feet (E/W direction), 3.2.A.4 maximum construction joint spacing in the foundation wall, lower concourse slab, ramp slab, interior walls, and the ground floor concrete slab is 60 feet. Foundation wall, lower concourse floor slab, and ground floor construction joints shall align





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	<p>mat slab joint below and 3.2.B.1 construction joints in floor slab shall be located within the central third of the span.</p> <p>Due to the beam configurations at the South West radius of the train box the following deviations from the aforementioned requirements will be required:</p> <p>1.) A construction joint will need to pass through a Moment Frame Beam along Grid Line X near Grid Line H in the attached sketch.</p> <p>2.) Slab construction joints at two locations will not align with the mat slab or wall construction joints along the radius wall between Grid Line Wand Grid Line 5.</p> <p>Please confirm these proposed deviations would be acceptable pending evaluation of a full contract joint location submittal.</p>					
	<p>with the location of the mat slab joint below and 3.2.B.1 construction joints in floor slab shall be located within the central third of the span.</p> <p>Due to the beam configurations at the South West radius of the train box the following deviations from the aforementioned requirements will be required:</p> <p>1.) A construction joint will need to pass through a Moment Frame Beam along Grid Line X near Grid Line H in the attached sketch.</p> <p>2.) Slab construction joints at two locations will not align with the mat slab or wall construction joints along the radius wall between Grid Line Wand Grid Line 5.</p> <p>Please confirm these proposed deviations would be acceptable pending evaluation of a full contract joint location submittal.</p>					
<b>T-0350</b>	<b>BGP - Mat Slab Penetration Waterproofing</b>	<b>Closed</b>	<b>01</b>	<b>11/21/2012</b>	<b>12/01/2012</b>	<b>11/28/2012</b>
	<p><b>From:</b> Webcor Construction LP      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Specifiction Reference: 07 12 10 Drawing Reference: A1-8711</p> <p>Please reference Drawing Sheet A1-8711, Laurenco E-Mail and Stamped Shop Drawing Details. Penetration details on drawing sheet A1-8711 call for 4 inch wide butyl tape to wrap around the mat slab penetrations prior to pouring of the mud slab. The specifications call for all shop drawings to bear the manufacturer's stamp of approval. Laurenco (manufacturer) has indicated that they require the butyl tape to extend 4 inches minimum past the top of the mud slab. Please review and advise as this does not match the as bid details.</p>					
	<p><b>ANSWER:</b></p> <p>Specifiction Reference: 07 12 10 Drawing Reference: A1-8711</p> <p>Please reference Drawing Sheet A1-8711, Laurenco E-Mail and Stamped Shop Drawing Details. Penetration details on drawing sheet A1-8711 call for 4 inch wide butyl tape to wrap around the mat slab penetrations prior to pouring of the mud slab. The specifications call for all shop drawings to bear the manufacturer's stamp of approval. Laurenco (manufacturer) has indicated that they require the butyl tape to extend 4 inches minimum past the top of the mud slab. Please review and advise as this does not match the as bid details.</p>					
<b>T-0350.1</b>	<b>BGP - Mat Slab Penetration Waterproofing</b>	<b>Closed</b>	<b>01</b>	<b>12/06/2012</b>	<b>12/16/2012</b>	<b>12/13/2012</b>
	<p><b>From:</b> Webcor Construction LP      Robert Kjome</p>					



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T-0350.2	<b>BGP - Mat Slab Penetration Waterproofing</b>  <b>From:</b> Webcor Construction LP Robert Kjome	Closed	01	12/20/2012	12/30/2012	12/21/2012
<b>REQUEST:</b> Reference Specification:07 12 10 Reference Documents: A1-8711  Futher to the engineers response to RFI T-0350, the extension of the butyl tape conflicts with the casings that are required around the dewatering wells, trestle piles, bridge piles, and pin piles. Please provide revised details at each of the aforementioned locations to accommodate the extended butyl tape.			<b>ANSWER:</b> Reference Specification:07 12 10 Reference Documents: A1-8711  Futher to the engineers response to RFI T-0350, the extension of the butyl tape conflicts with the casings that are required around the dewatering wells, trestle piles, bridge piles, and pin piles. Please provide revised details at each of the aforementioned locations to accommodate the extended butyl tape.			
<b>REQUEST:</b> Reference Drawing: A1-8711  SCCI would like to confirm conversations concerning the Butyl tape and Mud Slab Penetrations. From the meeting held 12/19/2012, the design Engineer mentioned that the Butyl tape at the Mud Slab Penetrations does not serve as a waterproofing purpose, but rather a bond breaker between the concrete and the steel penetrating through the mud slab. Because of this, the Engineer stated the Butyl tape did not need to be extended above the Mud Slab and could stop at the penetration slab.  Please confirm.			<b>ANSWER:</b> Reference Drawing: A1-8711  SCCI would like to confirm conversations concerning the Butyl tape and Mud Slab Penetrations. From the meeting held 12/19/2012, the design Engineer mentioned that the Butyl tape at the Mud Slab Penetrations does not serve as a waterproofing purpose, but rather a bond breaker between the concrete and the steel penetrating through the mud slab. Because of this, the Engineer stated the Butyl tape did not need to be extended above the Mud Slab and could stop at the penetration slab.  Please confirm.			



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T-0351	BGP - Grace Eclipse Floor 200	Closed	01	11/26/2012	12/06/2012	11/26/2012
From: Webcor Construction LP Robert Kjome						
REQUEST: Reference Specification: 03 30 20  Eclipse Floor and Eclipse Plus admixtures were replaced by a new generation of drying shrinkage reducing admixtures Eclipse Floor 200 and Eclipse 4500. This new family of admixtures is equivalent to BASF Tetraguard and based on our experience we should be able to achieve project specifications on drying shrinkage. CEMEX has been using the two new products for more than two years with excellent results. Attached, please find the communication from Grace Construction Products about the two new shrinkage reducing admixture products. Please verify these eclipse products are acceptable for use on this project.		ANSWER: Reference Specification: 03 30 20  Eclipse Floor and Eclipse Plus admixtures were replaced by a new generation of drying shrinkage reducing admixtures Eclipse Floor 200 and Eclipse 4500. This new family of admixtures is equivalent to BASF Tetraguard and based on our experience we should be able to achieve project specifications on drying shrinkage. CEMEX has been using the two new products for more than two years with excellent results. Attached, please find the communication from Grace Construction Products about the two new shrinkage reducing admixture products. Please verify these eclipse products are acceptable for use on this project.				
T-0352	BGP - Commissioning of Ground Loop Heat Exchanger	Closed	01	11/26/2012	11/26/2012	11/30/2012
From: Webcor Construction LP David Fields						
REQUEST: Reference: 23 57 34 3.5 Please confirm that commissioning will not be required for the Ground Loop Heat Exchanger.		ANSWER: Reference: 23 57 34 3.5 Please confirm that commissioning will not be required for the Ground Loop Heat Exchanger.				
T-0352.1	BGP - Commissioning of Ground Loop Heat Exchanger Follow-Up	Closed	CR	11/30/2012	12/10/2012	12/07/2012
From: Webcor Construction LP David Fields						
REQUEST: In reviewing the issued for construction documents W/O is unable to locate specification section "01 91 00 General Commissioning Requirements" or "23 08 00 HVAC Systems Commissioning". Please advise.		ANSWER: In reviewing the issued for construction documents W/O is unable to locate specification section "01 91 00 General Commissioning Requirements" or "23 08 00 HVAC Systems Commissioning". Please advise.				
T-0353	BSE - Micropile W107 Relocation	Closed	01	12/04/2012	12/14/2012	12/11/2012
From: Webcor Construction LP Robert Kjome						
REQUEST: Micropile W107 as laid out is in conflict with Pin-pile #15.		ANSWER: Micropile W107 as laid out is in conflict with Pin-pile #15.				





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<b>T-0355.1</b>	<b>BSE - Zone 4 Instrumentation Pad Demolition</b>	<b>Closed</b>	<b>CR</b>	<b>01/30/2013</b>	<b>02/09/2013</b>	<b>02/07/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b> Per conversation in previous MRP meetings after RFI T-0355 was answered, BBII noted that the instrumentation protection slab which is to remain in place as seen on section 10 of contract drawing GT-5102 as directed by RFI T-0355 response prevents survey markers from being placed on the top of soldier piles. Concerns were raised from the owner's design team suggesting that this slab may have to be removed. Please confirm that RFI T-0355 was answered correctly and the instrumentation protection slab is to remain in place.		<b>ANSWER:</b> Per conversation in previous MRP meetings after RFI T-0355 was answered, BBII noted that the instrumentation protection slab which is to remain in place as seen on section 10 of contract drawing GT-5102 as directed by RFI T-0355 response prevents survey markers from being placed on the top of soldier piles. Concerns were raised from the owner's design team suggesting that this slab may have to be removed. Please confirm that RFI T-0355 was answered correctly and the instrumentation protection slab is to remain in place.				
<b>T-0356</b>	<b>BGP - GEOTHERMAL - Loop Soil Compaction</b>	<b>Closed</b>	<b>01</b>	<b>12/11/2012</b>	<b>12/21/2012</b>	<b>12/17/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Specifications: 31 23 34 3.3 F 23 57 34 3.1 D 23 57 34 1.2 A.3  Per Specification 31 23 34, page 6, paragraph 3.3, F., soil is to be compacted to 95% maximum dry density. Specification 23 57 34, page 4, paragraph 3.1, D., S3H is to backfill per IGSHPA with loose soil.  Please confirm S3H is to backfill the geothermal loop per IGSHPA standard section 23 57 34, page 1, 1.2, A. 3.		<b>ANSWER:</b> Reference Specifications: 31 23 34 3.3 F 23 57 34 3.1 D 23 57 34 1.2 A.3  Per Specification 31 23 34, page 6, paragraph 3.3, F., soil is to be compacted to 95% maximum dry density. Specification 23 57 34, page 4, paragraph 3.1, D., S3H is to backfill per IGSHPA with loose soil.  Please confirm S3H is to backfill the geothermal loop per IGSHPA standard section 23 57 34, page 1, 1.2, A. 3.				
<b>T-0356.1</b>	<b>BGP - GEOTHERMAL - Loop Soil Compaction Conflict in Specifications</b>	<b>Closed</b>	<b>CR</b>	<b>01/22/2013</b>	<b>02/01/2013</b>	<b>01/29/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b> Please refer to attached excerpts from spec section 23 57 34, 31 23 34 and RFI response to T-0356.  The RFI response to T-0356 (SCI-017) requires the backfill of the trenches to meet specifications section 23 57 34 and 31 23 34. However, the two sections are in conflict with one another. Section 23 57 34-3.1, D,		<b>ANSWER:</b> Please refer to attached excerpts from spec section 23 57 34, 31 23 34 and RFI response to T-0356.  The RFI response to T-0356 (SCI-017) requires the backfill of the trenches to meet specifications section 23 57 34 and 31 23 34. However, the two sections are in conflict with one another. Section 23 57 34-3.1, D,				





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<b>T-0358</b>	<b>BGP - Geothermal Ground Temperature Probe Sleeve</b>	<b>Closed</b>	<b>01</b>	<b>12/11/2012</b>	<b>12/21/2012</b>	<b>12/19/2012</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Drawings: M1-5002  Detail A on M1-5002 shows the 2" ground temperature probe sleeve terminating at grade. Detail 5 on M1-5002 shows the same pipe terminating at the same elevation as GLS/GLR piping.  Please provide an elevation drawing for the temperature probe pipe sleeve.		<b>ANSWER:</b>  Reference Drawings: M1-5002  Detail A on M1-5002 shows the 2" ground temperature probe sleeve terminating at grade. Detail 5 on M1-5002 shows the same pipe terminating at the same elevation as GLS/GLR piping.  Please provide an elevation drawing for the temperature probe pipe sleeve.				
<b>T-0358.1</b>	<b>BGP - Geothermal Temperature Probe Elevations</b>	<b>Closed</b>	<b>01</b>	<b>02/25/2014</b>	<b>03/07/2014</b>	<b>03/04/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Per field conversations with the geothermal EOR(WSP) the elevation of the four(4) temperature probe stub outs has been revised. Please provide the revised elevations of these temperature probe stub outs.  Note that two(2) of the temperature probes have already been installed to an elevation consistent with the response to seer RFI 18(T-0358).		<b>ANSWER:</b>  Per field conversations with the geothermal EOR(WSP) the elevation of the four(4) temperature probe stub outs has been revised. Please provide the revised elevations of these temperature probe stub outs.  Note that two(2) of the temperature probes have already been installed to an elevation consistent with the response to seer RFI 18(T-0358).				
<b>T-0359</b>	<b>BGP - Water Treatment for Geothermal</b>	<b>Closed</b>	<b>01</b>	<b>12/18/2012</b>	<b>12/18/2012</b>	<b>12/21/2012</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b>  Reference Specification 23 57 34 Sub Section 3.4  During the TG06 IFB process section 3.4 was added to the Ground Loop Heat Exchanger specifications. We believe this requirement is intended for a future bid package during the commissioning of the system. Please confirm.		<b>ANSWER:</b>  Reference Specification 23 57 34 Sub Section 3.4  During the TG06 IFB process section 3.4 was added to the Ground Loop Heat Exchanger specifications. We believe this requirement is intended for a future bid package during the commissioning of the system. Please confirm.				
<b>T-0360</b>	<b>BSE - Mud Slab Welded Wire Reinforcement</b>	<b>Closed</b>	<b>01</b>	<b>12/21/2012</b>	<b>12/28/2012</b>	<b>01/03/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						

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T-0362	BGP - Wall Vertical Reinforcement at 3rd Level Bracing	Closed	01	01/07/2013	01/17/2013	01/11/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:		ANSWER:				
Reference Drawing: S1-3201 Reference Specification: 03 30 01		Reference Drawing: S1-3201 Reference Specification: 03 30 01				
Please reference attached sketch of the shoring wall section and CD S1-3201.		Please reference attached sketch of the shoring wall section and CD S1-3201.				
To allow required access and sequencing for installation of the wall waterproofing and reinforcing steel, an additional row of type 2 mechanical couplers will be required on the back face walls directly below 3rd level of bracing.		To allow required access and sequencing for installation of the wall waterproofing and reinforcing steel, an additional row of type 2 mechanical couplers will be required on the back face walls directly below 3rd level of bracing.				
This will allow the following: 1. "Blocking out" the waterproofing at the waler beam packing locations will be avoided. 2. Provide required access for waterproofing installation. 3. Reduce the time installed waterproofing is exposed on wall before concrete pours.		This will allow the following: 1. "Blocking out" the waterproofing at the waler beam packing locations will be avoided. 2. Provide required access for waterproofing installation. 3. Reduce the time installed waterproofing is exposed on wall before concrete pours.				
Please provide your approval of this additional row of couplers.		Please provide your approval of this additional row of couplers.				
T-0363	BGP - Slab Penetration Sleeve Thickness	Closed	CR	01/09/2013	01/19/2013	01/18/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:		ANSWER:				
Reference Drawings: A1-8711 and S1-3003		Reference Drawings: A1-8711 and S1-3003				
Plan sheet A1-8711 details all of the slab penetration sleeves to be fabricated of 3/8" steel. Plan sheet S1-3003 details only the pin pile, trestle pile, and 48" bridge pier sleeves to be fabricated of 1/2" steel. Please confirm that it is acceptable to fabricate all penetration sleeves of 3/8" steel like that shown on A1-8711.		Plan sheet A1-8711 details all of the slab penetration sleeves to be fabricated of 3/8" steel. Plan sheet S1-3003 details only the pin pile, trestle pile, and 48" bridge pier sleeves to be fabricated of 1/2" steel. Please confirm that it is acceptable to fabricate all penetration sleeves of 3/8" steel like that shown on A1-8711.				
T-0364	BGP - WPM-1 ASTM 6769 & Blindside Waterproofing Application	Closed	01	01/15/2013	01/24/2013	01/25/2013
From: Webcor Construction LP                      Jackson Tukuafu						
REQUEST:		ANSWER:				



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	<p>Please refer to Specification 07 12 10 3.3 and Drawing 1/A1-8710.</p> <p>Section 3.3 of the specifications require that all work be performed in accordance with ASTM D6769 (Application of Fully Adhered, Cold-Applied, Prefabricated Reinforced Modified Bituminous Membrane Waterproofing Systems). The WPM-1 vertical application (071210-1.1, A.2) is a blind-side WP application; however, the ASTM D6769 is written to address positive-side WP application.</p> <p>1. Please confirm the blind -side WP application is covered under the ASTM D6769 requirement or provide the applicable ASTM requirient to perform the blind-side application.</p> <p>2. Please confirm which section of the ASTM D6769 requirement is applicable to blind-side WP application.</p> <p>3. The ASTM D6769 section 11.7 requirement to "backfill vertical waterproofing installation within 24 h of protective board installation..." isn't feasible due to the extensive work sequence to install concrete reinforcement, form and place the foundation wall. Please confirm this section of the ASTM requirement is not applicable to blind-side WP applications.</p>					
<b>T-0365</b>	<b>BSE - Micropile W127 Relocation</b>	<b>Closed</b>	<b>01</b>	<b>01/15/2013</b>	<b>01/25/2013</b>	<b>01/17/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>						
Ref: Specification 31 63 33						
Micropile W127 (5'-5 3/4" West of G.L. 3 and 74'-0 3/4" South of G.L. J) is located in an area that is not accessible to drilling equipment. BBII proposes to eliminate this micropile. Please confirm this is acceptable.						
	<p>Please refer to Specification 07 12 10 3.3 and Drawing 1/A1-8710.</p> <p>Section 3.3 of the specifications require that all work be performed in accordance with ASTM D6769 (Application of Fully Adhered, Cold-Applied, Prefabricated Reinforced Modified Bituminous Membrane Waterproofing Systems). The WPM-1 vertical application (071210-1.1, A.2) is a blind-side WP application; however, the ASTM D6769 is written to address positive-side WP application.</p> <p>1. Please confirm the blind -side WP application is covered under the ASTM D6769 requirement or provide the applicable ASTM requirient to perform the blind-side application.</p> <p>2. Please confirm which section of the ASTM D6769 requirement is applicable to blind-side WP application.</p> <p>3. The ASTM D6769 section 11.7 requirement to "backfill vertical waterproofing installation within 24 h of protective board installation..." isn't feasible due to the extensive work sequence to install concrete reinforcement, form and place the foundation wall. Please confirm this section of the ASTM requirement is not applicable to blind-side WP applications.</p>					
<b>T-0366</b>	<b>BGP - WPM-1 - Adhesive Between Bottom Ply Waterproofing Membrane and Mud ǂ Closed</b>	<b>Closed</b>	<b>01</b>	<b>01/22/2013</b>	<b>01/22/2013</b>	<b>01/24/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>ANSWER:</b>						
Ref: Specification 31 63 33						
Micropile W127 (5'-5 3/4" West of G.L. 3 and 74'-0 3/4" South of G.L. J) is located in an area that is not accessible to drilling equipment. BBII proposes to eliminate this micropile. Please confirm this is acceptable.						



CR	02/01/2013	02/11/2013	02/05/2013
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T-0367	BGP - REBAR - Vertical Pit - Two Piece Bar	Closed	CR	01/17/2013	01/27/2013	01/25/2013
<div><div><div>From: Webcor Construction LP</div><div>Robert Kjome</div></div><div><div>REQUEST:</div><div>Please refer to Specification Section 03 20 00-3.1, E, attached drawing S1-3004, S1-3006 and Gerdau sketch SK-RFI014.</div><div>Concrete reinforcement details around the mat slab pit sections shown on drawing SI-3004 and SI-3006 depict a continuous vertical "Z" bar around the pit slab edge. Please confirm the proposed lap splice detail and requirements as shown in the attached Gerdau sketch SK-RFI014 is acceptable.</div></div><div><div>ANSWER:</div><div>Please refer to Specification Section 03 20 00-3.1, E, attached drawing S1-3004, S1-3006 and Gerdau sketch SK-RFI014.</div><div>Concrete reinforcement details around the mat slab pit sections shown on drawing SI-3004 and SI-3006 depict a continuous vertical "Z" bar around the pit slab edge. Please confirm the proposed lap splice detail and requirements as shown in the attached Gerdau sketch SK-RFI014 is acceptable.</div></div></div>						
T-0368	BGP - Hub and Spigot Type Pipe Support Spacing	Closed	01	01/17/2013	01/27/2013	02/01/2013
<div><div><div>From: Webcor Construction LP</div><div>Robert Kjome</div></div><div><div>REQUEST:</div><div>Reference Specification: 22 13 01 , 3.2 Reference Drawings: P1-6001</div><div>In Section 3.2 C, Supports, the support spacing for all horizontal cast iron no-hub pipe is specified to be 10 feet maximum, and within 6 inches at each side of each joint; however, the support spacing for all horizontal cast iron hub and spigot type pipe is not provided.</div><div>Please provide the required support spacing for the horizontal cast iron hub and spigot type pipe.</div></div><div><div>ANSWER:</div><div>Reference Specification: 22 13 01 , 3.2 Reference Drawings: P1-6001</div><div>In Section 3.2 C, Supports, the support spacing for all horizontal cast iron no-hub pipe is specified to be 10 feet maximum, and within 6 inches at each side of each joint; however, the support spacing for all horizontal cast iron hub and spigot type pipe is not provided.</div><div>Please provide the required support spacing for the horizontal cast iron hub and spigot type pipe.</div></div></div>						
T-0369	BGP - REBAR - Headed Steel Bar Shear Conflict in Mat Slab	Closed	CR	01/21/2013	01/31/2013	01/25/2013
<div><div><div>From: Webcor Construction LP</div><div>Jackson Tukuafu</div></div><div><div>REQUEST:</div><div>Please refer to attached drawing S1-3005 and S1-2022.</div><div>Detail 3 on sheet S1-3005 depicts the full size T-head bars as they interface with the mat reinforcement. The same detail includes additional reinforcement depicted at column locations. The reinforcement (open circles) is shown between the typical main mat reinforcement and others</div></div><div><div>ANSWER:</div><div>Please refer to attached drawing S1-3005 and S1-2022.</div><div>Detail 3 on sheet S1-3005 depicts the full size T-head bars as they interface with the mat reinforcement. The same detail includes additional reinforcement depicted at column locations. The reinforcement</div></div></div>						



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	<p>are aligned with above layers one and two of the main mat reinforcement as defined in note 4 and 6 on sheet S1-2002. As a result, the clearances created by the #10 main mat reinforcement being spaced at 8" O.C. and the 3" square heads at the ends of the #8 T-heads (refer 2/S1-3005) do not allow enough of a clearance to install the headed bars into position. Refer to the annotations in the attached drawings.</p> <p>Please advise.</p>					<p>(open circles) is shown between the typical main mat reinforcement and others are aligned with above layers one and two of the main mat reinforcement as defined in note 4 and 6 on sheet S1-2002. As a result, the clearances created by the #10 main mat reinforcement being spaced at 8" O.C. and the 3" square heads at the ends of the #8 T-heads (refer 2/S1-3005) do not allow enough of a clearance to install the headed bars into position. Refer to the annotations in the attached drawings.</p> <p>Please advise.</p>
T-0370	BGP - WPM-1 - Mud Slab Finish for Waterproofing	Closed	01	01/22/2013	02/01/2013	01/25/2013
	<p>From: Webcor Construction LP Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Specification Section 07 12 10, 3.2</p> <p>The concrete surface profile (CSP) required by the waterproofing manufacturer Laurenc0, ranges between a CSP level of 2 and 4 as defined by the International Concrete Repair Institute (ICRI) of technical guide "Selecting and Specifying Concrete Surface Preparation for Sealers, Coatings, and Polymer Overlays." The ICRI defines the levels of CSP as 1 (nearly flat) to CSP Level 9 (very rough). The Laurenc0 waterproofing system requires "a good wood screed or broom finish...often referred to as a 'sidewalk' finish..Do not use a steel trowel finish." See attached excerpt of the manufacturer specification.</p> <p>1. Please confirm the ICRI CSP requirements as it relates to surface finish, flatness and levelness are to supersede the varying ASTM F-value requirements setforth in specification section 033000-3.6, B1 or provide a revised specification section 033000 incorporating the ICRI requirement.</p> <p>2. Please confirm a wood screed or broom finish is accpetable for the mud slab.</p>					<p><b>ANSWER:</b></p> <p>Specification Section 07 12 10, 3.2</p> <p>The concrete surface profile (CSP) required by the waterproofing manufacturer Laurenc0, ranges between a CSP level of 2 and 4 as defined by the International Concrete Repair Institute (ICRI) of technical guide "Selecting and Specifying Concrete Surface Preparation for Sealers, Coatings, and Polymer Overlays." The ICRI defines the levels of CSP as 1 (nearly flat) to CSP Level 9 (very rough). The Laurenc0 waterproofing system requires "a good wood screed or broom finish...often referred to as a 'sidewalk' finish..Do not use a steel trowel finish." See attached excerpt of the manufacturer specification.</p> <p>1. Please confirm the ICRI CSP requirements as it relates to surface finish, flatness and levelness are to supersede the varying ASTM F-value requirements setforth in specification section 033000-3.6, B1 or provide a revised specification section 033000 incorporating the ICRI requirement.</p> <p>2. Please confirm a wood screed or broom finish is accpetable for the mud slab.</p>



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<b>T-0371</b>	<b>BSE - Micropile W154 &amp; W236 Bent After Install</b>	<b>Closed</b>	<b>01</b>	<b>01/22/2013</b>	<b>02/01/2013</b>	<b>01/29/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification: 31 63 33 Reference Drawings: Sheet ML-1 (Approved Micropile Layout submittal.)		Reference Specification: 31 63 33 Reference Drawings: Sheet ML-1 (Approved Micropile Layout submittal.)				
The top 5ft of micropile W154 is out of plumb by approximately 8% and micropile W236 is out of plumb 2.5%. It appears that the piles have been hit by a piece of equipment and bent near subgrade. BBII recommends the piles should be left as-is. Please confirm this is acceptable.		The top 5ft of micropile W154 is out of plumb by approximately 8% and micropile W236 is out of plumb 2.5%. It appears that the piles have been hit by a piece of equipment and bent near subgrade. BBII recommends the piles should be left as-is. Please confirm this is acceptable.				
BBII will take steps to ensure this does not happened again. The importance of taking special care to avoid damaging permanent work will be an emphasized topic in tool-box talks for crews running equipment near micropiles.		BBII will take steps to ensure this does not happened again. The importance of taking special care to avoid damaging permanent work will be an emphasized topic in tool-box talks for crews running equipment near micropiles.				
In the event that a micropile becomes bent in the future, please provide the design teams percentage of tolerance that the micropile can be out of plumb.		In the event that a micropile becomes bent in the future, please provide the design teams percentage of tolerance that the micropile can be out of plumb.				
<b>T-0371.1</b>	<b>BSE - Micropile W154 &amp; W236 Bent After Install</b>	<b>Closed</b>	<b>CR</b>	<b>02/04/2013</b>	<b>02/14/2013</b>	<b>02/06/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref: Specification Section 31 63 33 3.2.L		Ref: Specification Section 31 63 33 3.2.L				
BBII is in receipt of reply to RFI T-0371, which suggests that piles which have experienced an impact be rejected and re-drilled. Acceptance of piles is based on specification 31 63 33 3.2.L. Per the recommendation of the Micropile Engineer (Drill Tech's Steve McCullough) and the anchor bar manufacturer (DSI), the piles were bent back to plumb and retested on 02/01/2013. Attached are the passing proof test results for the piles in question. Please confirm that these piles are accepted.		BBII is in receipt of reply to RFI T-0371, which suggests that piles which have experienced an impact be rejected and re-drilled. Acceptance of piles is based on specification 31 63 33 3.2.L. Per the recommendation of the Micropile Engineer (Drill Tech's Steve McCullough) and the anchor bar manufacturer (DSI), the piles were bent back to plumb and retested on 02/01/2013. Attached are the passing proof test results for the piles in question. Please confirm that these piles are accepted.				
<b>T-0371.2</b>	<b>BSE - Micropile W154 &amp; W236 Bent After Install</b>	<b>Closed</b>	<b>CR</b>	<b>02/08/2013</b>	<b>02/18/2013</b>	<b>02/11/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						











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T-0377	BGP - Two Piece Oval Hoop Columns A1, A2, & A3	Closed	01	01/24/2013	02/03/2013	01/29/2013
From: Webcor Construction LP                      Joanne Filipas						
REQUEST: Reference S1-3304  Please confirm it is acceptable to use a two-piece oval tie in lieu of the single-piece oval tie, as depicted on contract drawing S1-3304, for columns A1, A2 and A3. Gerdau proposes to use a lap splice along the flat sides of the oval to connect either side of the hoop.						ANSWER: Reference S1-3304  Please confirm it is acceptable to use a two-piece oval tie in lieu of the single-piece oval tie, as depicted on contract drawing S1-3304, for columns A1, A2 and A3. Gerdau proposes to use a lap splice along the flat sides of the oval to connect either side of the hoop.
T-0378	BGP - Drainage Catch Basin Clarification	Closed	01	01/24/2013	02/03/2013	02/01/2013
From: Webcor Construction LP                      Joanne Filipas						
REQUEST: Reference P1-2022  There are two (clouded) sump pits attached that are not connected to any of the drainage system called out on PI-2022. Please confirm that there are no drainage lines connected to these two sump pits.						ANSWER: Reference P1-2022  There are two (clouded) sump pits attached that are not connected to any of the drainage system called out on PI-2022. Please confirm that there are no drainage lines connected to these two sump pits.
T-0379	BGP - Geothermal Pipe Fusion Butt Weld	Closed	01	01/24/2013	01/24/2013	01/29/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST: Reference Specification: 23 57 34  The response to Submittal TG0601-008 commented that only socket fittings and electrofusion fittings are allowed. This insinuates that Butt Fusion welds are not allowable. However, per Specification Section 23 57 34, Butt Fusion welding does not seem to be precluded. The butt fusion method is acceptable per the IGSHP A.  Please confirm that butt fusion welding is acceptable under this contract for the geothermal piping.						ANSWER: Reference Specification: 23 57 34  The response to Submittal TG0601-008 commented that only socket fittings and electrofusion fittings are allowed. This insinuates that Butt Fusion welds are not allowable. However, per Specification Section 23 57 34, Butt Fusion welding does not seem to be precluded. The butt fusion method is acceptable per the IGSHP A.  Please confirm that butt fusion welding is acceptable under this contract for the geothermal piping.
T-0379.1	BGP - Geothermal Pipe Fusion Butt Weld	Void	01	05/13/2014	05/23/2014	
From: Webcor Construction LP                      Claude Titcher						

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
<b>REQUEST:</b>						
See attached RFI T-0379						
Please confirm it is acceptable to repair the punctured geothermal piping in geofield 14R Area 16 by using a fusion butt weld. The butt fusion method of heat fusing ground loop piping is acceptable per the IGSHP A.						
<b>T-0380</b>	<b>BSE - K9 Buttress shaft CSL Tubes</b>	Closed	01	01/28/2013	02/07/2013	02/14/2013
<b>From:</b> Webcor Construction LP	Lynn Kowallis					
<b>REQUEST:</b>						
Reference attached sketch and spreadsheet.						
We were informed by Harris-Salinas that they are short of CSL tubes for the last rebar cage K9. Since K8/K9 interface will not be CSL tested, per the agreed upon list of shaft interfaces (generated by Arup and BBII), it is in BBII's opinion that it would be more beneficial to the shaft if it is installed without CSL tubes. The benefits include the following: There would be no need to grout the holes; no voids; and there would be more concrete in the shaft. If CSL tubes are required, we are proposing to install them per the attached drawing.						
Please advise.						
<b>ANSWER:</b>						
See attached RFI T-0379						
Please confirm it is acceptable to repair the punctured geothermal piping in geofield 14R Area 16 by using a fusion butt weld. The butt fusion method of heat fusing ground loop piping is acceptable per the IGSHP A.						
<b>ANSWER:</b>						
Reference attached sketch and spreadsheet.						
We were informed by Harris-Salinas that they are short of CSL tubes for the last rebar cage K9. Since K8/K9 interface will not be CSL tested, per the agreed upon list of shaft interfaces (generated by Arup and BBII), it is in BBII's opinion that it would be more beneficial to the shaft if it is installed without CSL tubes. The benefits include the following: There would be no need to grout the holes; no voids; and there would be more concrete in the shaft. If CSL tubes are required, we are proposing to install them per the attached drawing.						
Please advise.						



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T-0381	BGP - PLUMBING Floor Cleanout Requirement	Closed	01	01/28/2013	02/07/2013	02/01/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference Specification: 22 13 01 2.3 A.3 Reference Drawing: P1-0051, P1-2022		Reference Specification: 22 13 01 2.3 A.3 Reference Drawing: P1-0051, P1-2022				
Drawing P1-0051 specifies a Fig. Number of "MIFAB C-100-R/S" with remarks of "STAINLESS STEEL COVER AND PLUG, HEAVY DUTY, ANCHOR FLANGE". This item differs from the floor cleanout required in Spec section 22 13 01-2.3.A.3 which calls for "Extra heavy duty cast iron cleanout with round adjustable galvanized cast iron top, vandal proof screws, plastic plug or bronze gasketed plug, spigot outlet; 'No. 4220-G Series' by J.R. Smith, 'No. ZI400-G-VP Series' by Zurn Industries, Inc., Mifab C1100-RI3-6 or equal."		Drawing P1-0051 specifies a Fig. Number of "MIFAB C-100-R/S" with remarks of "STAINLESS STEEL COVER AND PLUG, HEAVY DUTY, ANCHOR FLANGE". This item differs from the floor cleanout required in Spec section 22 13 01-2.3.A.3 which calls for "Extra heavy duty cast iron cleanout with round adjustable galvanized cast iron top, vandal proof screws, plastic plug or bronze gasketed plug, spigot outlet; 'No. 4220-G Series' by J.R. Smith, 'No. ZI400-G-VP Series' by Zurn Industries, Inc., Mifab C1100-RI3-6 or equal."				
Please confirm which type of floor cleanout is required.		Please confirm which type of floor cleanout is required.				
T-0382	BSE - Eliminate CSL Tubes from Shaft D1	Closed	01	01/31/2013	02/10/2013	02/07/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference attached Arup email dated 1/29/2013.		Reference attached Arup email dated 1/29/2013.				
Please confirm that Shaft D1 can be installed without the need for CSL tubes. At Arup's direction, and at no extra cost to the owner, BBII will provide a QC core hole that extends into native soil.		Please confirm that Shaft D1 can be installed without the need for CSL tubes. At Arup's direction, and at no extra cost to the owner, BBII will provide a QC core hole that extends into native soil.				
T-0383	BGP - Drainage Flow Lines	Closed	01	01/31/2013	02/10/2013	02/07/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference Specification: 22 13 01 Reference Drawing: P1-2022 & P1-2030		Reference Specification: 22 13 01 Reference Drawing: P1-2022 & P1-2030				
Please reference contract drawings P1-2022 thru P1-2030. There is a discrepancy between the called out elevations of the pipe inverts and the flow grades between		Please reference contract drawings P1-2022 thru P1-2030. There is a discrepancy between the called out elevations of the pipe inverts and the flow grades between				



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T-0384	<b>BSE - Dry Excavation of Buttress Shaft D1</b>  From: Webcor Construction LP Lynn Kowallis  <b>REQUEST:</b> Reference attached Arup email dated 1/30/2013.  Becho will proceed on excavating Shaft D1 dry as per Arup's email.  Please confirm this is still acceptable.	Closed	CR	02/01/2013	02/11/2013	02/12/2013
	the sump pits and catch basins. All pipe inverts at the catch basins are to be set to El. -36.83' and pipe inverts at the sump pits are at either El. -37.50' or -37'-25'.  At the long pipe runs the flow grade matches to 1% as called out on the plans. However, on the short pipe runs, this grade is up to 18%.  Please clarify which details governs, and whether the 18% slope is acceptable.					
	between the sump pits and catch basins. All pipe inverts at the catch basins are to be set to El. -36.83' and pipe inverts at the sump pits are at either El. -37.50' or -37'-25'.  At the long pipe runs the flow grade matches to 1% as called out on the plans. However, on the short pipe runs, this grade is up to 18%.  Please clarify which details governs, and whether the 18% slope is acceptable.					



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<b>T-0385</b>	<b>BSE - Micropile Moves in NW Corner W013, W031, W047, W198.</b>	<b>Closed</b>	<b>CR</b>	<b>02/05/2013</b>	<b>02/15/2013</b>	<b>02/06/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: Submittal Pakage TG0601-009.1 - 235734-003.1						Ref: Submittal Pakage TG0601-009.1 - 235734-003.1
Upon staking layout of micropiles in Northwest corner of Zone 1, BBII discovered two micropiles that require relocation.						Upon staking layout of micropiles in Northwest corner of Zone 1, BBII discovered two micropiles that require relocation.
1. Pile W013 is too close to installed dewatering well. BBII proposes moving this pile 4' Southwest. This does not appear to conflict with Ghex shop drawings revision date 02/04/13.						1. Pile W013 is too close to installed dewatering well. BBII proposes moving this pile 4' Southwest. This does not appear to conflict with Ghex shop drawings revision date 02/04/13.
2. Pipe W198 is too close to overhead struts and strut supports. BBII proposes moving this pile 2' Northwest. This appears to eliminate the need for a "jog" in the Ghex piping as shown on Ghex shop drawings revision date 02/04/13.						2. Pipe W198 is too close to overhead struts and strut supports. BBII proposes moving this pile 2' Northwest. This appears to eliminate the need for a "jog" in the Ghex piping as shown on Ghex shop drawings revision date 02/04/13.
Upon drilling two piles in the NW corner of Zone 1, Drill Tech discovered unforeseen obstructions below grade (reference COM1741 sent 02/04/2013). Relocation of these micropiles is required.						Upon drilling two piles in the NW corner of Zone 1, Drill Tech discovered unforeseen obstructions below grade (reference COM1741 sent 02/04/2013). Relocation of these micropiles is required.
1. Pile W031 encountered an obstruction below grade which did not allow installation of the anchor bar in the drilled hole. After discovery of the obstruction, the pile was relocated 2' Northwest of its planned location. Installation of the micropile was completed on 02/01/2013. This does not appear to conflict with Ghex shop drawings revision date 02/04/13.						1. Pile W031 encountered an obstruction below grade which did not allow installation of the anchor bar in the drilled hole. After discovery of the obstruction, the pile was relocated 2' Northwest of its planned location. Installation of the micropile was completed on 02/01/2013. This does not appear to conflict with Ghex shop drawings revision date 02/04/13.
2. Pile W047 encountered an obstruction below grade which did not allow the micropile hole to be drilled past approximately 12'. BBII proposes to relocate this pile 2.8' Southwest. This appears to conflict with Ghex piping shown in Ghex shop drawings revision date 02/04/13 and may require the addition of a "jog".						2. Pile W047 encountered an obstruction below grade which did not allow the micropile hole to be drilled past approximately 12'. BBII proposes to relocate this pile 2.8' Southwest. This appears to conflict with Ghex piping shown in Ghex shop drawings revision date 02/04/13 and may require the addition of a "jog".
Please confirm these changes are acceptable.						Please confirm these changes are acceptable.



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T-0386	BSE - Elevator Pit Dimensions	Closed	CR	02/05/2013	02/15/2013	02/07/2013
From: Webcor Construction LPLynn Kowallis						
REQUEST: Ref: SI-2024 and Detail 3/S1-3008  The slab depression between Gridlines 15 &16 Between Gridlines B & C does not contain enough dimensions to construct. Detail 3/S1-3008 Note 2 states "For extent of thickened mat see plan." Plan sheet S1-2024 revision 2 dated 11/27/2012 provides width of the pit in the North-South direction, but does not provide the length of the pit in the East-West direction. Please provide these dimensions.		ANSWER: Ref: SI-2024 and Detail 3/S1-3008  The slab depression between Gridlines 15 &16 Between Gridlines B & C does not contain enough dimensions to construct. Detail 3/S1-3008 Note 2 states "For extent of thickened mat see plan." Plan sheet S1-2024 revision 2 dated 11/27/2012 provides width of the pit in the North-South direction, but does not provide the length of the pit in the East-West direction. Please provide these dimensions.				
T-0387	BGP - Geothermal Loop Compaction Requirements	Closed	CR	02/07/2013	02/17/2013	02/15/2013
From: Webcor Construction LPLynn Kowallis						
REQUEST: Reference Specification: 31 23 34 3.3 F  Per Specification Section 31 23 34, Section 3.3, Part F, the trench is required to be compacted to 95% . To acheive 95% compaction, the surrounding soil must have an equal or greater compaction. Please confirm.		ANSWER: Reference Specification: 31 23 34 3.3 F  Per Specification Section 31 23 34, Section 3.3, Part F, the trench is required to be compacted to 95% . To acheive 95% compaction, the surrounding soil must have an equal or greater compaction. Please confirm.				
T-0388	BGP - Temperature Probe Sleeve Penetration	Closed	01	02/08/2013	02/18/2013	02/14/2013
From: Webcor Construction LPLynn Kowallis						
REQUEST: Ref: TG06.1 Bid Package - 5/M1-5002 and TG06.0 - 5/M1-5002  The TG06.1 bid package, M1-5002 drawing does not show a temperature probe sleeve in Detail 5. Is the temperature probe sleeve to penetrate through the wall like it is shown in the TG06.0 M1-5002, Detail 5 or is it not to penetrate through the wall like the TG06.1 documents? Please advise.		ANSWER: Ref: TG06.1 Bid Package - 5/M1-5002 and TG06.0 - 5/M1-5002  The TG06.1 bid package, M1-5002 drawing does not show a temperature probe sleeve in Detail 5. Is the temperature probe sleeve to penetrate through the wall like it is shown in the TG06.0 M1-5002, Detail 5 or is it not to penetrate through the wall like the TG06.1 documents? Please advise.				



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<b>T-0389</b>	<b>BGP - Cast-in-place Concrete Shrinkage</b>	<b>Closed</b>	<b>01</b>	<b>02/11/2013</b>	<b>02/21/2013</b>	<b>02/22/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: Specification Section 03 30 20 1.7.F.3.i			Ref: Specification Section 03 30 20 1.7.F.3.i			
Please reference attached ACTM C 157, pages from SEONC San Francisco Bay Area Concrete Aggregate Report 2008, and Specification Section 03 30 20- 1.7.F.3.i. ASTM 157 section 4.3 states that if the condition of mixing, curing sampling and storage other than specified in the test method are required, they shall be reported but are not to be considered as standard conditions of this test method. In section 6. Sampling, it requires samples from batches made in the laboratory and the Note 2 states that field cast specimens can show up to twice as much drying shrinkage as laboratory cast specimens from the same materials and proportions. Furthermore, SEONC 2008 states that "actual shrinkage of the concrete in service and in field-cured tests will not necessarily correlate closely with the trial batch test results." For these reasons SCCI believes that shrinkage tests from samples at the job site can not verify the specified shrinkage limit and can not be compared with the laboratory tests.			Please reference attached ACTM C 157, pages from SEONC San Francisco Bay Area Concrete Aggregate Report 2008, and Specification Section 03 30 20- 1.7.F.3.i. ASTM 157 section 4.3 states that if the condition of mixing, curing sampling and storage other than specified in the test method are required, they shall be reported but are not to be considered as standard conditions of this test method. In section 6. Sampling, it requires samples from batches made in the laboratory and the Note 2 states that field cast specimens can show up to twice as much drying shrinkage as laboratory cast specimens from the same materials and proportions. Furthermore, SEONC 2008 states that "actual shrinkage of the concrete in service and in field-cured tests will not necessarily correlate closely with the trial batch test results." For these reasons SCCI believes that shrinkage tests from samples at the job site can not verify the specified shrinkage limit and can not be compared with the laboratory tests.			
Please confirm that shrinkage results from the samples taken in the field will not be directly compared to laboratory tests, and consequently used as basis for rejection of material.			Please confirm that shrinkage results from the samples taken in the field will not be directly compared to laboratory tests, and consequently used as basis for rejection of material.			



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<b>T-0390</b>	<b>BGP - Floor Drain FD-1 Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>02/12/2013</b>	<b>02/22/2013</b>	<b>02/20/2013</b>
<b>From:</b> Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: P1-0051						Ref: P1-0051
The "Drains and Cleanout Schedule" on drawing P1-0051 calls Floor Drain FD-1 to be Mifab F-1000-S with a grate size of 6" in diameter. Per the manufacturer, F-1000-S has a square grate.						The "Drains and Cleanout Schedule" on drawing P1-0051 calls Floor Drain FD-1 to be Mifab F-1000-S with a grate size of 6" in diameter. Per the manufacturer, F-1000-S has a square grate.
1. Is the floor drain grate to be round with a 6" diameter or square?						1. Is the floor drain grate to be round with a 6" diameter or square?
2. If it is square, then what are the dimensions of the square grate?						2. If it is square, then what are the dimensions of the square grate?
3. The remarks for FD-1 specifies a "Clamping Device." Is the "Clamping Device" referring to a membrane clamp?						3. The remarks for FD-1 specifies a "Clamping Device." Is the "Clamping Device" referring to a membrane clamp?
Please advise						Please advise
<b>T-0391</b>	<b>BGP - Zone 2 Sump Pit Depth</b>	<b>Closed</b>	<b>CR</b>	<b>02/13/2013</b>	<b>02/23/2013</b>	<b>02/19/2013</b>
<b>From:</b> Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: A1-9215 and S1-3006						Ref: A1-9215 and S1-3006
Please confirm in drawing AI-9215, the call outs "SP TOC -42'-4"" and "SP TOC -46'-4"" for the sump pits between grid lines C-D and 4-5 are referring to the elevation for the bottom of sump pits, as illustrated in the attached markup of SI-3006.						Please confirm in drawing AI-9215, the call outs "SP TOC -42'-4"" and "SP TOC -46'-4"" for the sump pits between grid lines C-D and 4-5 are referring to the elevation for the bottom of sump pits, as illustrated in the attached markup of SI-3006.
<b>T-0392</b>	<b>BGP - CMU Partition Walls</b>	<b>Closed</b>	<b>CR</b>	<b>02/15/2013</b>	<b>02/25/2013</b>	<b>02/20/2013</b>
<b>From:</b> Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference A-2224 and A-0022						Reference A-2224 and A-0022
Sheet A-2224 shows future CMU partition walls as type .6. Per the masonry partition schedule there is no .6 type.						Sheet A-2224 shows future CMU partition walls as type .6. Per the masonry partition schedule there is no .6 type.
Please confirm these walls are partition type 6.						Please confirm these walls are partition type 6.





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<b>T-0393</b>	<b>BGP - Reinforcement anchoring stagger and clearance for "addl bottom bars"</b>	<b>Closed</b>	<b>01</b>	<b>02/15/2013</b>	<b>02/25/2013</b>	<b>02/27/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b> Reference 3/S1-3006  1. Confirm there is no stagger for the reinforcement anchoring. 2. Provide the minimum clearance for the reinforcement anchoring to the "addl bottom bars".					<b>ANSWER:</b> Reference 3/S1-3006  1. Confirm there is no stagger for the reinforcement anchoring. 2. Provide the minimum clearance for the reinforcement anchoring to the "addl bottom bars".	
<b>T-0394</b>	<b>BSE - Micropile Relocations at Beale Street</b>	<b>Closed</b>	<b>01</b>	<b>02/19/2013</b>	<b>03/01/2013</b>	<b>02/22/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Specification: 31 63 33  Eight micropiles will be in conflict with the Beale Street Bridge Piles; BBII proposes relocating these micropiles to provide adequate clearance. See attached chart and drawings for proposed relocation information.  Please confirm these relocations are acceptable.					<b>ANSWER:</b> Reference Specification: 31 63 33  Eight micropiles will be in conflict with the Beale Street Bridge Piles; BBII proposes relocating these micropiles to provide adequate clearance. See attached chart and drawings for proposed relocation information.  Please confirm these relocations are acceptable.	
<b>T-0395</b>	<b>BGP - Floor Sink FSK-2 Clarification</b>	<b>Closed</b>	<b>01</b>	<b>02/19/2013</b>	<b>03/01/2013</b>	<b>03/05/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b> Ref: P 1-0051  The "Drains and Cleanout Schedule" on drawing P 1-0051 calls for Floor Sink FSK-2 to be Mifab FS 1700-1 -FLC-5. This model is not available per discussions between SCCI and the manufacturer.  Please confirm required floor sink model.					<b>ANSWER:</b> Ref: P 1-0051  The "Drains and Cleanout Schedule" on drawing P 1-0051 calls for Floor Sink FSK-2 to be Mifab FS 1700-1 -FLC-5. This model is not available per discussions between SCCI and the manufacturer.  Please confirm required floor sink model.	
<b>T-0396</b>	<b>BGP - Curb Frame Steel and Anchor Clip Requirements</b>	<b>Closed</b>	<b>01</b>	<b>02/19/2013</b>	<b>03/01/2013</b>	<b>02/28/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>					<b>ANSWER:</b>	



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Ref: 8/P1-6001	Detail 8 on drawing PI-6001 calls out a "Heavy duty galvanized steel custom made curb frame embedded in concrete." Please provide the following information:  1. Thickness of steel for curb frame. 2. Anchor clip details (size, spacing, connection to curb frame).					Ref: 8/P1-6001  Detail 8 on drawing PI-6001 calls out a "Heavy duty galvanized steel custom made curb frame embedded in concrete." Please provide the following information:  1. Thickness of steel for curb frame. 2. Anchor clip details (size, spacing, connection to curb frame).
T-0396.1	BGP - Drainage Pits Embedded Frame Details and Curb Frame Steel and Anchor C Closed		01	03/04/2013	03/14/2013	03/08/2013
From: Webcor Construction LP	Lynn Kowallis					
REQUEST:						ANSWER:
Ref: 8/P1-6001, DS-0001, RFI # 396	Detail 8 on P1-6001 does not specify the thickness of the frame material, nor any of the Specs and Addendums. Based on RFI 396, Designer specified for the frames to be 5/8" thick. However SCCI believes that 1/4" thick frame is adequate to satisfy "heavy duty requirement". SCCI's has estimated the Work to fabricate the embedded grate frame out of the stock angles (2x2x1/4" and 3x2x1/4"), per attached SCCI's drawing DS-0001. Further to RFI 396, please answer the following:  1. Can stock angle sizes noted above be used for construction of the embedded frames? 2. Could Nelson studs be used in lieu of the anchor clips, as noted on the attached drawing?  Please note that increase of the material size consequently increases the cost of furnished material, and therefore will constitute a compensable change.					Ref: 8/P1-6001, DS-0001, RFI # 396  Detail 8 on P1-6001 does not specify the thickness of the frame material, nor any of the Specs and Addendums. Based on RFI 396, Designer specified for the frames to be 5/8" thick. However SCCI believes that 1/4" thick frame is adequate to satisfy "heavy duty requirement". SCCI's has estimated the Work to fabricate the embedded grate frame out of the stock angles (2x2x1/4" and 3x2x1/4"), per attached SCCI's drawing DS-0001. Further to RFI 396, please answer the following:  1. Can stock angle sizes noted above be used for construction of the embedded frames? 2. Could Nelson studs be used in lieu of the anchor clips, as noted on the attached drawing?  Please note that increase of the material size consequently increases the cost of furnished material, and therefore will constitute a compensable change.



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<b>T-0396.2</b>	<b>BGP - Drainage Pits Embedded Frame Grates</b>	<b>Closed</b>	<b>01</b>	<b>03/22/2013</b>	<b>04/01/2013</b>	<b>04/01/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification: 23 13 01 Reference Drawings: P1-6001 Reference RFIs: T-0396, T-0396.1		Reference Specification: 23 13 01 Reference Drawings: P1-6001 Reference RFIs: T-0396, T-0396.1				
Detail 8 on CD P1-6001 does not provide enough details for assembly and fabrication of the embedded frames. SCCI's drawing attachment in the RFI 396 series provides such details.		Detail 8 on CD P1-6001 does not provide enough details for assembly and fabrication of the embedded frames. SCCI's drawing attachment in the RFI 396 series provides such details.				
As per our discussion with the SER(Structural Engineer of Record) on 3/21/2013, see attached revised SCCI's drawings of the embedded grate assemblies. As discussed SCCI has revised the weld detail between the two angles to be used to fabricate the embedded frames. Weld is changed to T-joint, PJP double bevel groove weld per AWS D1.1 (references 8-56, table 8-2 from AISC Steel Manual 13th ED.)		As per our discussion with the SER(Structural Engineer of Record) on 3/21/2013, see attached revised SCCI's drawings of the embedded grate assemblies. As discussed SCCI has revised the weld detail between the two angles to be used to fabricate the embedded frames. Weld is changed to T-joint, PJP double bevel groove weld per AWS D1.1 (references 8-56, table 8-2 from AISC Steel Manual 13th ED.)				
Is it acceptable to construct the embedded grate frames per attached detail?		Is it acceptable to construct the embedded grate frames per attached detail?				
<b>T-0397</b>	<b>BGP - RCW Dimension Clarification</b>	<b>Closed</b>	<b>01</b>	<b>02/21/2013</b>	<b>03/03/2013</b>	<b>02/28/2013</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference A1-2123 and attached.		Reference A1-2123 and attached.				
Between gridlines 12-13 and G-H there appears to be two conflicting dimensions. Please confirm the 7'3" dimension is from gridline H to the work the point.		Between gridlines 12-13 and G-H there appears to be two conflicting dimensions. Please confirm the 7'3" dimension is from gridline H to the work the point.				
<b>T-0398</b>	<b>BGP - Vehicle Ramp Concrete Corbel Dimension</b>	<b>Closed</b>	<b>01</b>	<b>02/21/2013</b>	<b>03/03/2013</b>	<b>02/28/2013</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference A1-2310 and S1-2251		Reference A1-2310 and S1-2251				
Please provide dimension for the continuous concrete corbel at the vehicle/bike ramp.		Please provide dimension for the continuous concrete corbel at the vehicle/bike ramp.				



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<b>T-0399</b>	<b>BGP - Polystyrene Void Fill Material</b>	<b>Closed</b>	<b>01</b>	<b>02/21/2013</b>	<b>03/03/2013</b>	<b>03/05/2013</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b> Reference A1-7404 and 03 30 20  Please provide the PSI and specification for the Polystyrene void fill called out in detail D on A1-7404.					<b>ANSWER:</b> Reference A1-7404 and 03 30 20  Please provide the PSI and specification for the Polystyrene void fill called out in detail D on A1-7404.	
<b>T-0400</b>	<b>BGP - Seismic Joint Detail</b>	<b>Closed</b>	<b>01</b>	<b>02/21/2013</b>	<b>03/03/2013</b>	<b>03/06/2013</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b> Reference S1-3010, A1-8881, & A1-8882  The detail 4/S1-3010 does not appear to be coordinated with the details shown on A1-8881 and A1-8882. Please revise accordingly.					<b>ANSWER:</b> Reference S1-3010, A1-8881, & A1-8882  The detail 4/S1-3010 does not appear to be coordinated with the details shown on A1-8881 and A1-8882. Please revise accordingly.	
<b>T-0401</b>	<b>BGP - Dimension Clarification between Column and Slab at Ramp</b>	<b>Closed</b>	<b>01</b>	<b>02/21/2013</b>	<b>03/03/2013</b>	<b>02/28/2013</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b> Reference 5/S1-3502 and attached.  Please provide the dimension between the vehicle ramp and column.					<b>ANSWER:</b> Reference 5/S1-3502 and attached.  Please provide the dimension between the vehicle ramp and column.	
<b>T-0402</b>	<b>BGP - Dimension at slab and parapet wall footing detail</b>	<b>Closed</b>	<b>01</b>	<b>02/21/2013</b>	<b>03/03/2013</b>	<b>02/28/2013</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b> Reference 4/S1-3210 and attached.  Please provide dimension between the ground level slab and parapet wall footing.					<b>ANSWER:</b> Reference 4/S1-3210 and attached.  Please provide dimension between the ground level slab and parapet wall footing.	
<b>T-0403</b>	<b>BSE - Mud Slab Flatness and Levelness Testing</b>	<b>Closed</b>	<b>01</b>	<b>02/21/2013</b>	<b>03/03/2013</b>	<b>02/27/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>					<b>ANSWER:</b>	



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	Reference: 03 30 00 3.6.C.1.d  In follow up to the Turner's request, please confirm this specification section does not apply to the mud slab and no flatness or level testing is required.				Reference: 03 30 00 3.6.C.1.d  In follow up to the Turner's request, please confirm this specification section does not apply to the mud slab and no flatness or level testing is required.	
<b>T-0404</b>	<b>BGP - Replacement of Lap Splice with Mechanical Couplers</b>  From: Webcor Construction LP Lynn Kowallis	<b>Closed</b>	<b>01</b>	<b>02/22/2013</b>	<b>03/04/2013</b>	<b>03/06/2013</b>
<b>REQUEST:</b>  Ref: S1/3201  Please verify that it is acceptable to replace a lap splice with an approved mechanical coupler (500 series coupler) as needed to support the means and methods of construction. The current location being considered is the outside face wall vertical lap splice between the dowel extending from the mat slab and the typical wall vertical reinforcing at the bottom of the wall. See attached plan sheet S1-3201 to reference the proposed location. Should this be acceptable please verify:  1. The mechanical coupler can infringe upon the 2" clearance as the diameter of the coupler is greater then that of the actual reinforcing. 2. Also verify that the couplers can be installed at one typical elevation similar to that of the other couplers depicted on the inside face wall curtain.		<b>ANSWER:</b>  Ref: S1/3201  Please verify that it is acceptable to replace a lap splice with an approved mechanical coupler (500 series coupler) as needed to support the means and methods of construction. The current location being considered is the outside face wall vertical lap splice between the dowel extending from the mat slab and the typical wall vertical reinforcing at the bottom of the wall. See attached plan sheet S1-3201 to reference the proposed location. Should this be acceptable please verify:  1. The mechanical coupler can infringe upon the 2" clearance as the diameter of the coupler is greater then that of the actual reinforcing. 2. Also verify that the couplers can be installed at one typical elevation similar to that of the other couplers depicted on the inside face wall curtain.				



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<b>T-0404.1</b>	<b>BGP - Replacement of Lap Splice with Mechanical Couplers</b>	<b>Closed</b>	<b>01</b>	<b>02/22/2013</b>	<b>03/04/2013</b>	<b>03/27/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: S1/3201						Ref: S1/3201
Number 2 of the RFI T-0404 was not answered.						Number 2 of the RFI T-0404 was not answered.
Please verify that it is acceptable to replace a lap splice with an approved mechanical coupler (500 series coupler) as needed to support the means and methods of construction. The current location being considered is the outside face wall vertical lap splice between the dowel extending from the mat slab and the typical wall vertical reinforcing at the bottom of the wall. See attached plan sheet S1-3201 to reference the proposed location. Should this be acceptable. please verify.						Please verify that it is acceptable to replace a lap splice with an approved mechanical coupler (500 series coupler) as needed to support the means and methods of construction. The current location being considered is the outside face wall vertical lap splice between the dowel extending from the mat slab and the typical wall vertical reinforcing at the bottom of the wall. See attached plan sheet S1-3201 to reference the proposed location. Should this be acceptable. please verify.
1. Answered in RFI T-0404						1. Answered in RFI T-0404
2. Verify that the couplers can be installed at one typical elevation similar to that of the other couplers depicted on the inside face wall curtain.						2. Verify that the couplers can be installed at one typical elevation similar to that of the other couplers depicted on the inside face wall curtain.
<b>T-0405</b>	<b>BSE - Required Percent of Maximum Dry Density Required at Areas of Over Excav: Closed</b>		<b>01</b>	<b>02/22/2013</b>	<b>03/04/2013</b>	<b>03/01/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: Specification Section 31 00 00.3.15.C.1						Ref: Specification Section 31 00 00.3.15.C.1
Specification Section 31 00 00.3.15.C.1 states. C.Percentage of Maximum Dry Density Requirements: Compact soil to not less than the following percentages of maximum dry density according to ASTM D1557: 1.Under structures, building slabs, foundations and steps, fill deeper than five feet, shall be placed in lifts as defined above and compacted to at least 95 percent dry density.						Specification Section 31 00 00.3.15.C.1 states. C.Percentage of Maximum Dry Density Requirements: Compact soil to not less than the following percentages of maximum dry density according to ASTM D1557: 1.Under structures, building slabs, foundations and steps, fill deeper than five feet, shall be placed in lifts as defined above and compacted to at least 95 percent dry density.
Does the 95 percent dry density requirement apply only when fill is deeper than five feet and/or under structures, building slabs, foundations and steps?						Does the 95 percent dry density requirement apply only when fill is deeper than five feet and/or under structures, building slabs, foundations and steps?



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<b>T-0406</b>	<b>BSE - Micropile W434 Relocation</b>	<b>Closed</b>	<b>01</b>	<b>02/22/2013</b>	<b>02/22/2013</b>	<b>02/27/2013</b>
<b>From:</b> Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: Submittal TG0300-622.4						Ref: Submittal TG0300-622.4
Micropile W434 as laid out is in conflict with a dewatering well. BBII proposes moving Micropile W434 North 5' to provide adequate clearance. See attached sketch.						Micropile W434 as laid out is in conflict with a dewatering well. BBII proposes moving Micropile W434 North 5' to provide adequate clearance. See attached sketch.
Please confirm this is acceptable.						Please confirm this is acceptable.
<b>T-0407</b>	<b>BSE - Micropile W327 Installed 2' South</b>	<b>Closed</b>	<b>CR</b>	<b>02/22/2013</b>	<b>03/04/2013</b>	<b>02/27/2013</b>
<b>From:</b> Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: Submittal TG0300-622.4						Ref: Submittal TG0300-622.4
Micropile W327 was installed 2' South of original location. BBII proposes leaving the installed pile as is. See attached sketch.						Micropile W327 was installed 2' South of original location. BBII proposes leaving the installed pile as is. See attached sketch.
Please confirm this is acceptable.						Please confirm this is acceptable.
<b>T-0408</b>	<b>BGP - Open Stirrup with a Cap for Frame Beam Sections</b>	<b>Closed</b>	<b>01</b>	<b>02/25/2013</b>	<b>03/07/2013</b>	<b>03/01/2013</b>
<b>From:</b> Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: 5/S1-3600						Ref: 5/S1-3600
Detail 5 on sheet S1-3600 depicts beam configurations Type SI through S5 all of which graphically depict a closed stirrup. Please confirm that it is acceptable to utilize an open stirrup with a cap. The cap would maintain a 135 degree hook on one side and 90 degree hook on the other and placed in an alternating fashion.						Detail 5 on sheet S1-3600 depicts beam configurations Type SI through S5 all of which graphically depict a closed stirrup. Please confirm that it is acceptable to utilize an open stirrup with a cap. The cap would maintain a 135 degree hook on one side and 90 degree hook on the other and placed in an alternating fashion.
<b>T-0409</b>	<b>BSE - Micropile W226 Relocation (Due to Overhead Obstruction)</b>	<b>Closed</b>	<b>01</b>	<b>02/27/2013</b>	<b>03/09/2013</b>	<b>03/04/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	<p>Reference Specification: 31 63 33 Reference Dwg: Attached sketch</p> <p>Micropile W226 as laid out does not have adequate overhead clearance to be installed. BBII proposes moving Micropile W226 North 12' to provide adequate clearance. An alternate relocation position for Micropile W226 could be 4' East and 4' North.</p> <p>W/O recommends relocating the micropile North in order to avoid conflict with geothermal.</p> <p>Please confirm this is acceptable.</p>					
<b>T-0409.1</b>	<b>BSE - Micropile W226 Relocation (Due to Overhead Obstruction)</b>	<b>Closed</b>	<b>01</b>	<b>03/04/2013</b>	<b>03/14/2013</b>	<b>03/05/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						
<p>Specification Reference: 31 63 33 Specification Drawings: Attached BBII sketch</p> <p>Micropile W226 as laid out does not have adequate overhead clearance to be installed. BBII previously asked to move the pile 12' North. BBII understands that this location would be too close to pile W227 which is already installed. BBII now proposes to move the pile 10' North and 1' West. This does not appear to conflict with geothermal piping.</p> <p>Please confirm this is acceptable.</p>						
<b>ANSWER:</b>						
<p>Specification Reference: 31 63 33 Specification Drawings: Attached BBII sketch</p> <p>Micropile W226 as laid out does not have adequate overhead clearance to be installed. BBII previously asked to move the pile 12' North. BBII understands that this location would be too close to pile W227 which is already installed. BBII now proposes to move the pile 10' North and 1' West. This does not appear to conflict with geothermal piping.</p> <p>Please confirm this is acceptable.</p>						





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<b>T-0410</b>	<b>BGP - Lower Concourse Top of Slab between Gridlines 3-9</b>	<b>Closed</b>	<b>01</b>	<b>02/27/2013</b>	<b>03/09/2013</b>	<b>03/05/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Drawings: S1-3201 (BSE Drawings) S1-2202 (BGP Drawings) S1-2203 (BGP Drawings)						Reference Drawings: S1-3201 (BSE Drawings) S1-2202 (BGP Drawings) S1-2203 (BGP Drawings)
12/10/10 Issued for construction BSE drawing S1-3201 shows lower concourse top of slab to be 8'-8" between gridlines 3-9. 11/27/12 Issued for construction per ASI 100 BGP Drawings S1-2202 and S1-2203 shows lower concourse top of slab to be 5'-5" between gridlines 3-5 & 8-9. Gridlines 5-8 shows top of slab at 5'-10".						12/10/10 Issued for construction BSE drawing S1-3201 shows lower concourse top of slab to be 8'-8" between gridlines 3-9. 11/27/12 Issued for construction per ASI 100 BGP Drawings S1-2202 and S1-2203 shows lower concourse top of slab to be 5'-5" between gridlines 3-5 & 8-9. Gridlines 5-8 shows top of slab at 5'-10".
Please verify the elevation of the lower concourse top of slab between gridlines 3-9.						Please verify the elevation of the lower concourse top of slab between gridlines 3-9.
<b>T-0411</b>	<b>BGP - Welding for Penetration Sleeves</b>	<b>Closed</b>	<b>01</b>	<b>02/28/2013</b>	<b>03/10/2013</b>	<b>03/08/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Specification: 05 50 10 Reference Submittal No: TG0600-036						Reference Specification: 05 50 10 Reference Submittal No: TG0600-036
Per the Submittal TG0600-036 comments, the intermediate ring, 3" horizontal weld must have a removable backer bar. Is it acceptable to have a double beveled groove weld replace the single bevel groove with a back bar? Eliminating the backer bar in this weld and having a double beveled groove instead is more efficient.						Per the Submittal TG0600-036 comments, the intermediate ring, 3" horizontal weld must have a removable backer bar. Is it acceptable to have a double beveled groove weld replace the single bevel groove with a back bar? Eliminating the backer bar in this weld and having a double beveled groove instead is more efficient.
Secondly, the Submittal TG0600-036 comments address the field welding of the penetration sleeve collars to be conducted after the trestle pile is to be removed. Is it acceptable to weld a full collar with cap in the shop? The assembly would arrive onsite to be welded in place as originally intended by the designer.						Secondly, the Submittal TG0600-036 comments address the field welding of the penetration sleeve collars to be conducted after the trestle pile is to be removed. Is it acceptable to weld a full collar with cap in the shop? The assembly would arrive onsite to be welded in place as originally intended by the designer.
Please advise.						Please advise.



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<b>T-0412</b>	<b>BGP - Dewatering Well &amp; Piezometer Penetration Sleeve Anchors</b>	<b>Closed</b>	<b>CR</b>	<b>02/28/2013</b>	<b>03/10/2013</b>	<b>03/05/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Specification Section: 05 50 10 Specification Submittal: TG0600-036		Specification Section: 05 50 10 Specification Submittal: TG0600-036				
Per the Metal Fabrication Submittal for the pipe/pile penetration sleeves, TG0600-36, the number of anchor holes per ring were arbitrary for the submittal. Is it acceptable to have 4 equally spaced 1/2" holes to fit 3/8" wedge anchors for the anchorage of the dewatering well & piezometer penetration sleeves?		Per the Metal Fabrication Submittal for the pipe/pile penetration sleeves, TG0600-36, the number of anchor holes per ring were arbitrary for the submittal. Is it acceptable to have 4 equally spaced 1/2" holes to fit 3/8" wedge anchors for the anchorage of the dewatering well & piezometer penetration sleeves?				
Please advise.		Please advise.				
<b>T-0413</b>	<b>BGP - Bulkhead Formwork Material</b>	<b>Closed</b>	<b>01</b>	<b>02/28/2013</b>	<b>03/10/2013</b>	<b>03/13/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification: 031000 Reference Drawings: Sketches attached		Reference Specification: 031000 Reference Drawings: Sketches attached				
SCCI is planning to use Stayform for the construction of various bulkheads and blockouts in concrete structure. Reference attached sketches of the Mat slab bulkhead forms as an example. Stayform material shall be kept within 1.5" of all exposed concrete surfaces. Is it acceptable to use Stayform?		SCCI is planning to use Stayform for the construction of various bulkheads and blockouts in concrete structure. Reference attached sketches of the Mat slab bulkhead forms as an example. Stayform material shall be kept within 1.5" of all exposed concrete surfaces. Is it acceptable to use Stayform?				
<b>T-0414</b>	<b>BGP - Cast Iron Supports</b>	<b>Closed</b>	<b>01</b>	<b>02/28/2013</b>	<b>03/10/2013</b>	<b>03/11/2013</b>
<b>From:</b> Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref: 7/P1-6001		Ref: 7/P1-6001				
Please reference attached drawing and Detail 7 on Contract Drawing P1-6001. Detail 7 does not specify any imensions of the pipe support assembly. SCCI interprets that detail 7 is purely conceptual and proposes that the pipe support assemblies ("goal posts") to be constructed per the attached drawings.		Please reference attached drawing and Detail 7 on Contract Drawing P1-6001. Detail 7 does not specify any imensions of the pipe support assembly. SCCI interprets that detail 7 is purely conceptual and proposes that the pipe support assemblies ("goal posts") to be constructed per the attached drawings.				
Is this acceptable?		Is this acceptable?				



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<b>T-0414.2</b>	<b>BGP - Cast Iron Pipe Support</b>	<b>Closed</b>	<b>01</b>	<b>05/02/2013</b>	<b>05/15/2013</b>	<b>05/13/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: RFI T-0414 and T-0414.1		Reference: RFI T-0414 and T-0414.1				
Per the response to RFI T-0414.1, the Designer states that the protection slab will be sloped with a 4" maximum slab thickness. SCCI does not plan to pour the protection slab with a slope. SCCI plans to pour the protection slab level and keep the protection slab consistently 4" thick.		Per the response to RFI T-0414.1, the Designer states that the protection slab will be sloped with a 4" maximum slab thickness. SCCI does not plan to pour the protection slab with a slope. SCCI plans to pour the protection slab level and keep the protection slab consistently 4" thick.				
The Designer suggests using a pipe support with a grouted plate for scenarios where the drilled holes may get too close to the membrane. It would appear that grouted plate would still require some type of embedded anchor. By adding the grout, the manufacturer's embedment depth for the anchor is shortened and the tensile (pull-out) strength will be reduced.		The Designer suggests using a pipe support with a grouted plate for scenarios where the drilled holes may get too close to the membrane. It would appear that grouted plate would still require some type of embedded anchor. By adding the grout, the manufacturer's embedment depth for the anchor is shortened and the tensile (pull-out) strength will be reduced.				
An alternative method to anchoring the pipe supports would be the use of 1/2" short drop-in anchors (see attached Red Head Multi-Set II information) which requires 1" of embedment into concrete. The holes would be drilled using a Depth Charge drill bit which is a 1" long bit with a shoulder to prevent over drilling.		An alternative method to anchoring the pipe supports would be the use of 1/2" short drop-in anchors (see attached Red Head Multi-Set II information) which requires 1" of embedment into concrete. The holes would be drilled using a Depth Charge drill bit which is a 1" long bit with a shoulder to prevent over drilling.				
Confirm if this is acceptable.		Confirm if this is acceptable.				
<b>T-0415</b>	<b>BGP - Wall and Coupler Modifications in Zone 1 Train Box</b>	<b>Closed</b>	<b>01</b>	<b>02/28/2013</b>	<b>03/10/2013</b>	<b>03/13/2013</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Field Order T-00011 and SKA-2438 R2 attached		Reference: Field Order T-00011 and SKA-2438 R2 attached				
SKA-2438 includes proposed relocations and additions of rooms at the train platform level, specifically between gridlines 1 and 3.		SKA-2438 includes proposed relocations and additions of rooms at the train platform level, specifically between gridlines 1 and 3.				
1. Please confirm these proposed locations are final. 2. Please provide dimensions for these rooms.		1. Please confirm these proposed locations are final. 2. Please provide dimensions for these rooms.				
<b>T-0416</b>	<b>BGP - Geothermal Loop Pneumatic Testing Pressure</b>	<b>Closed</b>	<b>01</b>	<b>03/01/2013</b>	<b>03/11/2013</b>	<b>03/06/2013</b>



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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-0417	<div>From: Webcor Construction LP</div> <div>Lynn Kowallis</div> <div>REQUEST: Per specification 23 57 34, 3.2, C  Per specification 23 57 34, 3.2, C, all individual loops shall be pressure tested at 100 PSI for 30 minutes before installation. The moisture content from the Hydrostatic test of the loop can compromise the fusion weld. As discussed in the DFOW meeting, S3H is recommending pneumatic testing of the pipe at 80 PSI. Upon complete installation of the loops and header piping, the complete sub group will be hydrostatically tested as specified. Please see attached documentation from Manufacturer regarding Pneumatic Testing and confirm pneumatic testing is acceptable.</div>	Closed	01	03/04/2013	03/14/2013	03/12/2013
	<div>ANSWER: Per specification 23 57 34, 3.2, C  Per specification 23 57 34, 3.2, C, all individual loops shall be pressure tested at 100 PSI for 30 minutes before installation. The moisture content from the Hydrostatic test of the loop can compromise the fusion weld. As discussed in the DFOW meeting, S3H is recommending pneumatic testing of the pipe at 80 PSI. Upon complete installation of the loops and header piping, the complete sub group will be hydrostatically tested as specified. Please see attached documentation from Manufacturer regarding Pneumatic Testing and confirm pneumatic testing is acceptable.</div>					
T-0418	<div>BSE - Micropile W434.5 Addition per Contract Drawings</div> <div>From: Webcor Construction LP</div> <div>Lynn Kowallis</div> <div>REQUEST: Ref: S1-2031 and Submittal TG0300-622.4  Micropile W434.5 was installed on 2/28/2013 per contract drawing S1-2031. However, W434.5 was not included in approved submittal TG0300-622.4.  Please confirm Micropile W434.5 is required.</div>	Closed	01	03/05/2013	03/15/2013	03/06/2013
	<div>ANSWER: Ref: S1-2031 and Submittal TG0300-622.4  Micropile W434.5 was installed on 2/28/2013 per contract drawing S1-2031. However, W434.5 was not included in approved submittal TG0300-622.4.  Please confirm Micropile W434.5 is required.</div>					
T-0419	<div>BGP - Revit CAD Files Confirmation</div> <div>From: Webcor Construction LP</div> <div>Robert Kjome</div> <div>REQUEST: Reference Document: Email attached.  Pursuant to the direction received from the TJPA, please confirm the CAD files which are to be extracted from the Revit model are consistent with the Contract Documents.</div>	Closed	01	03/07/2013	03/17/2013	03/12/2013
	<div>ANSWER: Reference Document: Email attached.  Pursuant to the direction received from the TJPA, please confirm the CAD files which are to be extracted from the Revit model are consistent with the Contract Documents.</div>					
T-0419	BGP - Foundation Walls Formwork Ties	Closed	01	03/07/2013	03/17/2013	03/12/2013



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*PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG*  
 30100 - Transbay Transit Center Project









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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

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Time: 07:01 AM  
Job: 30100

Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-0423	BSE -Subgrade pit dimensions per comments to TG0300-340.1	Closed	01	03/07/2013	03/17/2013	03/20/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference Drawings: S1-2024,S1-2027, 7/S1-3010, A1-2817 Reference Specificaiton: 31 00 00 Reference Submittal: TG0300-340.1		Reference Drawings: S1-2024,S1-2027, 7/S1-3010, A1-2817 Reference Specificaiton: 31 00 00 Reference Submittal: TG0300-340.1				
The response to Mud Slab Rebar Shop Drawings Submittal TG0300-340.1/TA1020-32001A06.1 provided new dimensions for depressions in the trainbox subgrade. Per 00 07 00 Part 6.02.A, BBII would like to clarify which dimensions are to be used for construction.		The response to Mud Slab Rebar Shop Drawings Submittal TG0300-340.1/TA1020-32001A06.1 provided new dimensions for depressions in the trainbox subgrade. Per 00 07 00 Part 6.02.A, BBII would like to clarify which dimensions are to be used for construction.				
1. Sheet MS-4 of submittal shows subgrade depression between Grid lines 18 & 19 between Gridlines B & C having dimension of 20 '-0' x 40'-4". This is consistent with the dimensions provided on sheet S1-2024 Revision 2 dated 11/27/2013. The review comment by TT revises the 20'-0" dimension to 20'-3". Please confirm which dimension is to be used.		1. Sheet MS-4 of submittal shows subgrade depression between Grid lines 18 & 19 between Gridlines B & C having dimension of 20 '-0' x 40'-4". This is consistent with the dimensions provided on sheet S1-2024 Revision 2 dated 11/27/2013. The review comment by TT revises the 20'-0" dimension to 20'-3". Please confirm which dimension is to be used.				
2. Sheet MS-7 of submittal shows subgrade depression at Gridline 35 between Gridlines B & C as having dimensions of 22'-1 3/4" x 18'-6 3/4". This geometry is base on the size of the pit shown on A1-2817 Revision 1 dated 11/27/2012 and 7/S1-3010 Revision 0 dated 08/30/2012. The Submittal response comments provided show a new overall dimension of 19' -9" and a specific offset to Gridline 35. Please confirm which dimensions are to be used.		2. Sheet MS-7 of submittal shows subgrade depression at Gridline 35 between Gridlines B & C as having dimensions of 22'-1 3/4" x 18'-6 3/4". This geometry is base on the size of the pit shown on A1-2817 Revision 1 dated 11/27/2012 and 7/S1-3010 Revision 0 dated 08/30/2012. The Submittal response comments provided show a new overall dimension of 19' -9" and a specific offset to Gridline 35. Please confirm which dimensions are to be used.				
3. Sheet MS-7 of submittal shows subgrade depression between Gridlines 34 & 35 at Gridline E. TT comment calls out 3'-0" from eastern limit of depression to Gridline 35. This dimension was not provided on sheet S1-2027 Revision 2 dated 11/27/2012. Please confirm this dimension is to be used.		3. Sheet MS-7 of submittal shows subgrade depression between Gridlines 34 & 35 at Gridline E. TT comment calls out 3'-0" from eastern limit of depression to Gridline 35. This dimension was not provided on sheet S1-2027 Revision 2 dated 11/27/2012. Please confirm this dimension is to be used.				
4. BBII understands that dimensions provided on this submittal are to bottom of Mat Slab concrete, and that each dimension should be increased to account for thickness of protection slab and waterproofing. Please confirm that an additional 0'-7" is the correct dimension for this adjustment.		4. BBII understands that dimensions provided on this submittal are to bottom of Mat Slab concrete, and that each dimension should be increased to account for thickness of protection slab and waterproofing. Please confirm that an additional 0'-7" is the correct dimension for this adjustment.				



T-0426	BGP - Welded Wire Mesh in Sump and Elevator Pits	Closed	01	03/11/2013	03/21/2013	03/26/2013
From: Webcor Construction LP		Robert Kijome				

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<b>REQUEST:</b> Reference Drawing: S1-3004  SCCI is requesting to use welded wire mesh (specification attached) at the sloped surfaces of the sump and elevator pits. The welded wire mesh will inhibit concrete settlement towards the bottom of the pits during placement. Please advise if this is acceptable.					
	<b>ANSWER:</b> Reference Drawing: S1-3004  SCCI is requesting to use welded wire mesh (specification attached) at the sloped surfaces of the sump and elevator pits. The welded wire mesh will inhibit concrete settlement towards the bottom of the pits during placement. Please advise if this is acceptable.					
<b>T-0427</b>	<b>BSE - Back of CDSM wall allowable friction value.</b>	<b>Closed</b>	<b>01</b>	<b>03/12/2013</b>	<b>03/22/2013</b>	<b>03/27/2013</b>
	<b>From:</b> Webcor Construction LP                      Lynn Kowallis					
	<b>REQUEST:</b> WOJV is preparing details to connect and reinforce the Zone-4 walers. Please provide the value of back of CDSM wall allowable friction.					
	<b>ANSWER:</b> WOJV is preparing details to connect and reinforce the Zone-4 walers. Please provide the value of back of CDSM wall allowable friction.					
<b>T-0428</b>	<b>BGP - Geothermal Manifold Valves</b>	<b>Closed</b>	<b>01</b>	<b>03/11/2013</b>	<b>03/25/2013</b>	<b>03/22/2013</b>
	<b>From:</b> Webcor Construction LP                      Ian Corcorran					
	<b>REQUEST:</b> Reference Specification: 23 57 34 2.1.B  In addition to the keystone valves submitted for the geothermal manifold, S3H is requesting to also install Nibco valves (specification attached) as allowed under specification 23 57 34 2.1.B. There are currently procurement issues with the submitted Keystone valves. All manifolds will be installed with similar valves. Please confirm this is acceptable.					
	<b>ANSWER:</b> Reference Specification: 23 57 34 2.1.B  In addition to the keystone valves submitted for the geothermal manifold, S3H is requesting to also install Nibco valves (specification attached) as allowed under specification 23 57 34 2.1.B. There are currently procurement issues with the submitted Keystone valves. All manifolds will be installed with similar valves. Please confirm this is acceptable.					



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0429</b>	<b>BGP - Contract Limit Lines</b>	<b>Closed</b>	<b>01</b>	<b>03/11/2013</b>	<b>03/25/2013</b>	<b>03/22/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b> Reference Drawing: S1-3206, S1-3201  Contract Drawing Sheet S1-3206 Section 4 depicts an elevation of the knockout walls along the West end of the structure. Within this elevation the bold limit line for the contract TG0600 is shown well above the top wall CJ which does not align with Note 1 and the typical wall section on sheet S1-3201. Please clarify the proper location of the contract package TG0600 limit line on sheet S1-3206 Section 4.		<b>ANSWER:</b> Reference Drawing: S1-3206, S1-3201  Contract Drawing Sheet S1-3206 Section 4 depicts an elevation of the knockout walls along the West end of the structure. Within this elevation the bold limit line for the contract TG0600 is shown well above the top wall CJ which does not align with Note 1 and the typical wall section on sheet S1-3201. Please clarify the proper location of the contract package TG0600 limit line on sheet S1-3206 Section 4.				
<b>T-0430</b>	<b>BGP - Trainbox Shear Wall STD Hook</b>	<b>Closed</b>	<b>01</b>	<b>03/11/2013</b>	<b>03/22/2013</b>	<b>03/20/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b> Reference Drawings: S1-3260  Detail 2 of S1-3260 depicts standard hook reinforcement between the horizontal ties in the shearwall above the lower concourse; however, it is not clear if the standard hooks are required in the shearwall below the lower concourse. Please confirm if standard hook reinforcement is required in between the center shear wall ties. If standard hooks are required, please provide detail for the layout of the standard hooks in between the center shear wall ties.		<b>ANSWER:</b> Reference Drawings: S1-3260  Detail 2 of S1-3260 depicts standard hook reinforcement between the horizontal ties in the shearwall above the lower concourse; however, it is not clear if the standard hooks are required in the shearwall below the lower concourse. Please confirm if standard hook reinforcement is required in between the center shear wall ties. If standard hooks are required, please provide detail for the layout of the standard hooks in between the center shear wall ties.				
<b>T-0431</b>	<b>BGP - Knockout Wall, Top of Wall T-Head</b>	<b>Closed</b>	<b>01</b>	<b>03/12/2013</b>	<b>03/26/2013</b>	<b>03/22/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b> Reference Drawing: S1-3206 Reference Specification: 03 20 00  Dwg. Sheet S1-3206 Section 4 depicts the vertical reinforcing at the top of wall without a T-headed bar. Please confirm that a T-headed bar is not required at the top of the vertical bars throughout the knockout wall area.		<b>ANSWER:</b> Reference Drawing: S1-3206 Reference Specification: 03 20 00  Dwg. Sheet S1-3206 Section 4 depicts the vertical reinforcing at the top of wall without a T-headed bar. Please confirm that a T-headed bar is not required at the top of the vertical bars throughout the knockout wall area.				



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-0432</b>	<b>BGP - Shear Wall Layout</b>	<b>Closed</b>	<b>01</b>	<b>03/12/2013</b>	<b>03/26/2013</b>	<b>03/19/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>  Reference Drawings: S1-2250, S1-2030  The Northern-most shear wall when laid out based on the details (angle = 38.4 degrees from GL H) and dimensions (30'-5 7 /8") per contract drawing sheet S1-2030 do not conform with the dimensions provided on contract sheet S1-2250 Section 1. Please confirm which layout is correct and directions how to proceed.						<b>ANSWER:</b>  Reference Drawings: S1-2250, S1-2030  The Northern-most shear wall when laid out based on the details (angle = 38.4 degrees from GL H) and dimensions (30'-5 7 /8") per contract drawing sheet S1-2030 do not conform with the dimensions provided on contract sheet S1-2250 Section 1. Please confirm which layout is correct and directions how to proceed.
<b>T-0433</b>	<b>BGP - Columns Within the Shear Wall</b>	<b>Closed</b>	<b>01</b>	<b>03/12/2013</b>	<b>03/22/2013</b>	<b>03/21/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Specification: 03 20 00 Reference Drawing: S1-2250, S1-3306  The two columns C19 and column C38 depicted on contract drawings SI-2250, Section 1 all appear to be located adjacent to the opening and per the plan view are graphically represented as diamond shaped. When referencing contract drawing sheet S1-3306 these columns are graphically and dimensionally represented as square and not diamond shaped. Please confirm the geometry of these columns matches that as shown on S1-3306.						<b>ANSWER:</b>  Reference Specification: 03 20 00 Reference Drawing: S1-2250, S1-3306  The two columns C19 and column C38 depicted on contract drawings SI-2250, Section 1 all appear to be located adjacent to the opening and per the plan view are graphically represented as diamond shaped. When referencing contract drawing sheet S1-3306 these columns are graphically and dimensionally represented as square and not diamond shaped. Please confirm the geometry of these columns matches that as shown on S1-3306.
<b>T-0434</b>	<b>BSE - Micropile W603 Installed 1' South (Below ground obstruction)</b>	<b>Closed</b>	<b>CR</b>	<b>03/13/2013</b>	<b>03/23/2013</b>	<b>03/15/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>  Ref:Submittal TG0300-622.4  Micropile W603 was relocated 1' South of original location after encountering grout from the adjacent pin pile. See attached sketch.  Please confirm this is acceptable.						<b>ANSWER:</b>  Ref:Submittal TG0300-622.4  Micropile W603 was relocated 1' South of original location after encountering grout from the adjacent pin pile. See attached sketch.  Please confirm this is acceptable.



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0435</b>	<b>BGP - Flame Cutting of Reinforcement</b>	<b>Closed</b>	<b>01</b>	<b>03/11/2013</b>	<b>03/25/2013</b>	<b>03/22/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Specification: 03 20 00-3.1.6.A			Reference Specification: 03 20 00-3.1.6.A			
Project specification section 03 20 00-3 .1.6 states "Do not heat or flame cut bars;" however, this statement is a subpart to section 03 20 00-3.1.6. "Bend bars cold." It is unclear if the statement regarding to heating and flame-cutting of bars exclusively applies to bending of bars. Please confirm that heating and flame-cutting for purposes other than that of bending of bars is permitted.			Project specification section 03 20 00-3 .1.6 states "Do not heat or flame cut bars;" however, this statement is a subpart to section 03 20 00-3.1.6. "Bend bars cold." It is unclear if the statement regarding to heating and flame-cutting of bars exclusively applies to bending of bars. Please confirm that heating and flame-cutting for purposes other than that of bending of bars is permitted.			
Also, please refer to the attached section from CRSI which states that flame-cutting of bars have no adverse effects on reinforcement.			Also, please refer to the attached section from CRSI which states that flame-cutting of bars have no adverse effects on reinforcement.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0435.1</b>	<b>BGP - Flame Cutting Follow-Up to RFI 435</b>	<b>Closed</b>	<b>01</b>	<b>05/02/2013</b>	<b>05/10/2013</b>	<b>05/14/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: RFI T-0435, 03 20 00-3.1.6.A		Reference: RFI T-0435, 03 20 00-3.1.6.A				
The response to RFI T-0435 indicated that heating and flame cutting of reinforcing is prohibited unless approved by the EOR and per further discussion about this matter with the engineer it was requested that specific applications be submitted for further review. The following is a list of those applications:		The response to RFI T-0435 indicated that heating and flame cutting of reinforcing is prohibited unless approved by the EOR and per further discussion about this matter with the engineer it was requested that specific applications be submitted for further review. The following is a list of those applications:				
1. Penetrations in Slabs, Walls or Decks. Torch used to cut opening into reinforcing based on final asbuilt layout of penetration.		1. Penetrations in Slabs, Walls or Decks. Torch used to cut opening into reinforcing based on final asbuilt layout of penetration.				
2. Support Bar. Torch used to trim or remove support/give-away bar due to conflict or other project need.		2. Support Bar. Torch used to trim or remove support/give-away bar due to conflict or other project need.				
3. Column Rack/Crush Bar Removal. Torch used to remove rack and crush bars from columns to allow for tremie insertion and additional open space through center of column after column erected into place.		3. Column Rack/Crush Bar Removal. Torch used to remove rack and crush bars from columns to allow for tremie insertion and additional open space through center of column after column erected into place.				
4.Unforeseen Conflicts. Project conflicts that are identified during the course of work and require trimming or removal of reinforcing steel to correct condition. Example CDSM pile conflict.		4.Unforeseen Conflicts. Project conflicts that are identified during the course of work and require trimming or removal of reinforcing steel to correct condition. Example CDSM pile conflict.				
Please confirm the use of a torch/flame is allowable for the applications listed above.		Please confirm the use of a torch/flame is allowable for the applications listed above.				
<b>T-0436</b>	<b>BGP - Elevator Rail Support Width</b>	<b>Closed</b>	<b>01</b>	<b>03/13/2013</b>	<b>03/23/2013</b>	<b>03/21/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Drawing: Section A of 4/S1-7630		Reference Drawing: Section A of 4/S1-7630				
Please confirm that the dimension from the left end of the 1/2" embedded plate to the center of the welded headed stud is 3".		Please confirm that the dimension from the left end of the 1/2" embedded plate to the center of the welded headed stud is 3".				





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0437</b>	<b>BGP - Geothermal Riser Conflict with Soldier Pile</b>	<b>Closed</b>	<b>01</b>	<b>03/13/2013</b>	<b>03/23/2013</b>	<b>03/25/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Specification:23 57 34  As laid out per the approved shop drawings, the GLS/GLR Riser for the geothermal loops is in conflict with the soldier pile in the field. Please confirm that the riser can be relocated to the next CDSM wall panel to the West.		<b>ANSWER:</b> Reference Specification:23 57 34  As laid out per the approved shop drawings, the GLS/GLR Riser for the geothermal loops is in conflict with the soldier pile in the field. Please confirm that the riser can be relocated to the next CDSM wall panel to the West.				
<b>T-0438</b>	<b>BGP - Knockout Wall CJ</b>	<b>Closed</b>	<b>01</b>	<b>03/12/2013</b>	<b>03/26/2013</b>	<b>03/21/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b> Reference Drawing: 4/S1-3206  Reference Dwg. S1-3206 Section 4 - knockout wall section details. Since knockout walls are to be constructed independent of the rest of the structure, SCCI intention is to construct the knockout walls in two lifts. SCCI suggests eliminating bottom horizontal CJ of the knockout walls, as shown on the attached marked up drawing.  Is this acceptable?		<b>ANSWER:</b> Reference Drawing: 4/S1-3206  Reference Dwg. S1-3206 Section 4 - knockout wall section details. Since knockout walls are to be constructed independent of the rest of the structure, SCCI intention is to construct the knockout walls in two lifts. SCCI suggests eliminating bottom horizontal CJ of the knockout walls, as shown on the attached marked up drawing.  Is this acceptable?				
<b>T-0439</b>	<b>BGP - Mat Slab Elevator Opening Embeds</b>	<b>Closed</b>	<b>CR</b>	<b>03/13/2013</b>	<b>03/23/2013</b>	<b>03/27/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b> Ref: Drawings S1-2052 through S1-2061, 1/S1-7004, 12/S1-7602, 3/S1-3006, S1-3004, S1 -3008.  Please reference attached drawings of Mat Slab openings and Embeds. Drawings S1-2052 through S1-2061 show the locations of openings in the Mat Slab. At gridlines 1.8-E on drawing S1-2052 there is an elevator opening. Detail 1 on Drawing S1-7004 is the elevator opening from S1-2052 and shows the opening having two L8x4xl/2 full length embeds at the Mat Slab. See Detail 12 on attached drawing S1-7602 for embed. S1-2052 and detail 1 on S1-7004 both have cut lines referencing Detail 3 on S1-3006 showing the Mat Slab Pit details at this location. There are		<b>ANSWER:</b> Ref: Drawings S1-2052 through S1-2061, 1/S1-7004, 12/S1-7602, 3/S1-3006, S1-3004, S1 -3008.  Please reference attached drawings of Mat Slab openings and Embeds. Drawings S1-2052 through S1-2061 show the locations of openings in the Mat Slab. At gridlines 1.8-E on drawing S1-2052 there is an elevator opening. Detail 1 on Drawing S1-7004 is the elevator opening from S1-2052 and shows the opening having two L8x4xl/2 full length embeds at the Mat Slab. See Detail 12 on attached drawing S1-7602 for embed. S1-2052 and detail 1 on S1-7004 both have cut lines referencing Detail 3 on S1-3006				





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	<p>additional elevator openings on drawings S1-2054, S1-2055 and S1-2057. These" openings reference drawings S1-3004 and S1 -3008. Detail I on S1-7004 does not correspond to the openings on S1-2054, S1-2055 and S1-2057. Therefore, the only elevator opening that has L8x4xl/2 full length embeds on the Mat Slab is located at gridline 1.8-E.</p> <p>Please advise if this is correct.</p>					<p>showing the Mat Slab Pit details at this location. There are additional elevator openings on drawings S1-2054, S1-2055 and S1-2057. These" openings reference drawings S1-3004 and S1 -3008. Detail I on S1-7004 does not correspond to the openings on S1-2054, S1-2055 and S1-2057. Therefore, the only elevator opening that has L8x4xl/2 full length embeds on the Mat Slab is located at gridline 1.8-E.</p> <p>Please advise if this is correct.</p>
<b>T-0439.1</b>	<b>BGP - Mat Slab Elevator Opening Embeds</b>	<b>Closed</b>	<b>01</b>	<b>03/29/2013</b>	<b>04/08/2013</b>	<b>04/09/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b> Ref: RFI T-0439, SKS-0184  Please reference attached drawing. The response to WOJV RFI T -0439 modifies the continuous embedded assemblies to be four L8" x 4" x W' x 1 '-2" elevator post bases as depicted on Contract Drawing S 1-7600 Detail 11. The RFI response does not show the location and spacing of the embedded assemblies. Please provide locations and spacing.						<b>ANSWER:</b> Ref: RFI T-0439, SKS-0184  Please reference attached drawing. The response to WOJV RFI T -0439 modifies the continuous embedded assemblies to be four L8" x 4" x W' x 1 '-2" elevator post bases as depicted on Contract Drawing S 1-7600 Detail 11. The RFI response does not show the location and spacing of the embedded assemblies. Please provide locations and spacing.
<b>T-0439.2</b>	<b>BGP - Mat Slab Elevator Opening Embed Dimensions</b>	<b>Closed</b>	<b>01</b>	<b>05/10/2013</b>	<b>05/24/2013</b>	<b>05/15/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b> Ref. RFI T-0439.1  TJPA's response to RFI T-0439.1 stated "Final elevator post locations shall be coordinated with elevator manufacturer." The response has a second option to use a continuous L8x4x1/2 in lieu of the 1'-2" base. Please provide the elevator post locations if an elevator manufacturer has been selected? If not, SCCI is requesting to use continuous embeds. Please advise if this is acceptable.						<b>ANSWER:</b> Ref. RFI T-0439.1  TJPA's response to RFI T-0439.1 stated "Final elevator post locations shall be coordinated with elevator manufacturer." The response has a second option to use a continuous L8x4x1/2 in lieu of the 1'-2" base. Please provide the elevator post locations if an elevator manufacturer has been selected? If not, SCCI is requesting to use continuous embeds. Please advise if this is acceptable.





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T-0441	BSE - Micropile W638 Relocation (Dewatering Well Conflict)	Closed	01	03/14/2013	03/24/2013	03/19/2013
From: Webcor Construction LP                      Lynn Kowallis						
REQUEST:		ANSWER:				
Ref: Submittal TG0300 - 622.4		Ref: Submittal TG0300 - 622.4				
Micropile W638 as laid out is in conflict with a dewatering well. BBII proposes moving Micropile W638 East 2' to provide adequate clearance. This Micropile is located south of J-Line and the Geothermal piping area. See attached sketch.		Micropile W638 as laid out is in conflict with a dewatering well. BBII proposes moving Micropile W638 East 2' to provide adequate clearance. This Micropile is located south of J-Line and the Geothermal piping area. See attached sketch.				
Please confirm this is acceptable.		Please confirm this is acceptable.				
T-0442	BGP - Geothermal Riser Bracket Details	Closed	01	03/14/2013	03/24/2013	03/18/2013
From: Webcor Construction LP                      Lynn Kowallis						
REQUEST:		ANSWER:				
As requested in the Geothermal Meeting with the TJP A and Turner, please confirm that the attached details for the geothermal pipe riser brackets are acceptable. These details clarify the offset from the face of the CDSM wall required to avoid conflict with the water proofing membranes.		As requested in the Geothermal Meeting with the TJP A and Turner, please confirm that the attached details for the geothermal pipe riser brackets are acceptable. These details clarify the offset from the face of the CDSM wall required to avoid conflict with the water proofing membranes.				
T-0442.1	BGP - Geothermal Riser Bracket Details	Closed	01	03/21/2013	03/31/2013	03/29/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:		ANSWER:				
As requested in the Geothermal Meeting with the TJPA and Turner, please confirm that the attached details for the geothermal pipe riser brackets are acceptable. These details clarify the offset from the face of the CDSM wall required to avoid conflict with the water proofing membranes.		As requested in the Geothermal Meeting with the TJPA and Turner, please confirm that the attached details for the geothermal pipe riser brackets are acceptable. These details clarify the offset from the face of the CDSM wall required to avoid conflict with the water proofing membranes.				
T-0443	BGP - C Channel Confilct with Geothermal Pipe Riser	Closed	01	03/12/2013	03/26/2013	03/21/2013
From: Webcor Construction LP                      Ian Corcorran						
REQUEST:		ANSWER:				
Reference Specification: 23 57 34 Reference Photo: Attached		Reference Specification: 23 57 34 Reference Photo: Attached				





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T-0448	CDSM Soldier Pile Encroachment	Closed	01	03/19/2013	03/29/2013	03/27/2013
From: Webcor Construction LP                      Kirk Nielsen						
REQUEST:		ANSWER:				
Reference Documents: Exhibits A-H		Reference Documents: Exhibits A-H				
In follow up to the 3/13/13 meeting with AAI and TT regarding the CDSM soldier pile (SP) encroachment WOJV's proposal for mat slab area #1 (Exhibit-A) is as follows:		In follow up to the 3/13/13 meeting with AAI and TT regarding the CDSM soldier pile (SP) encroachment WOJV's proposal for mat slab area #1 (Exhibit-A) is as follows:				
Marked up sheets SH-2000 (Exhibit-B) and SH-2001 (Exhibit-C) depict the location of the encroaching SPs and the degree in which they are encroaching.		Marked up sheets SH-2000 (Exhibit-B) and SH-2001 (Exhibit-C) depict the location of the encroaching SPs and the degree in which they are encroaching.				
Predicated on SE stamped detail A/SLC.1 (Exhibit-D):		Predicated on SE stamped detail A/SLC.1 (Exhibit-D):				
A. At (4) SPs 753, 761, 765, & 787, WOJV is proposing to decrease the wall thickness to 34-1/2" with #11 rebar spacing to 6" o.c. between the centerline of the (2) adjacent piles. For example, as depicted in SK-T-0448.1 (Exhibit-E) SP #753 encroaches 1-1/4". WOJV would reduce the wall thickness while reducing the rebar spacing to compensate for the reduced wall thickness to clear the encroaching SP as depicted in SK-T-0448.2 (Exhibit-F).		A. At (4) SPs 753, 761, 765, & 787, WOJV is proposing to decrease the wall thickness to 34-1/2" with #11 rebar spacing to 6" o.c. between the centerline of the (2) adjacent piles. For example, as depicted in SK-T-0448.1 (Exhibit-E) SP #753 encroaches 1-1/4". WOJV would reduce the wall thickness while reducing the rebar spacing to compensate for the reduced wall thickness to clear the encroaching SP as depicted in SK-T-0448.2 (Exhibit-F).				
B. At SP 819 WOJV is proposing to decrease the wall thickness to 33 3/16" with #11 rebar spacing to 6" o.c. between the centerline of the (2) adjacent piles. Similar to above, as depicted in SK-T-0448.3 (Exhibit-G) SP #753 encroaches 2-3/16". WOJV would reduce the wall thickness while reducing the rebar spacing to compensate for the reduced wall thickness to clear the encroaching SP as depicted in SK-T-0448.4 (Exhibit-H).		B. At SP 819 WOJV is proposing to decrease the wall thickness to 33 3/16" with #11 rebar spacing to 6" o.c. between the centerline of the (2) adjacent piles. Similar to above, as depicted in SK-T-0448.3 (Exhibit-G) SP #753 encroaches 2-3/16". WOJV would reduce the wall thickness while reducing the rebar spacing to compensate for the reduced wall thickness to clear the encroaching SP as depicted in SK-T-0448.4 (Exhibit-H).				
WOJV did review the possibility of cutting the W21x201 flanges to accommodate the encroachment however, this high risk remedy was ruled out as it could jeopardize the project shoring system.		WOJV did review the possibility of cutting the W21x201 flanges to accommodate the encroachment however, this high risk remedy was ruled out as it could jeopardize the project shoring system.				
Please advise.		Please advise.				
T-0448.1	BGP - CDSM Soldier Pile Encroachment, mat areas 1&2 all levels (Exhibit-A).	Closed	01	04/26/2013	05/06/2013	04/26/2013
From: Webcor Construction LP                      Lynn Kowallis						



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<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref: T-0448, SH-2001, SH-2000		Ref: T-0448, SH-2001, SH-2000				
<p>Previous RFI response #T-0448 (Exhibit-A) only addressed the impact of the encroaching CDSM soldier piles (SPs) on the first or bottom wall segments. This RFI address the encroaching SPs in mat slab areas 1&amp;2 (Exhibit-B) at all levels of wall. This RFI shall supersede previous RFI response #T-0448.</p> <p>Marked up sheet SH-2001 (Exhibit-C) depicts the location of the encroaching SPs and the degree in which they are encroaching.</p> <p>1. SP #753 in mat area #2 encroaches 1-1/4" at elevation -34.12. WOJV is proposing to decrease the specified 36" wall thickness to 34-3/4" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>2. SP #761 in mat area #1 encroaches 7/8" at elevation -34.12. WOJV is proposing to decrease the specified 36" wall thickness to 35-1/8" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>3. SPs #765-770, vary in the degree of encroachment the worst of which is SP #765 in mat area #1 which encroaches 1-7/8" at elevation 25.10. WOJV is proposing to decrease the specified 36" wall thickness to 34-1/8" to clear the encroaching SPs. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>4. SP #787 in mat area #1 encroaches 7/8" at elevation -34.42. WOJV is proposing to decrease the specified 36" wall thickness to 35-1/8" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by</p>		<p>Previous RFI response #T-0448 (Exhibit-A) only addressed the impact of the encroaching CDSM soldier piles (SPs) on the first or bottom wall segments. This RFI address the encroaching SPs in mat slab areas 1&amp;2 (Exhibit-B) at all levels of wall. This RFI shall supersede previous RFI response #T-0448.</p> <p>Marked up sheet SH-2001 (Exhibit-C) depicts the location of the encroaching SPs and the degree in which they are encroaching.</p> <p>1. SP #753 in mat area #2 encroaches 1-1/4" at elevation -34.12. WOJV is proposing to decrease the specified 36" wall thickness to 34-3/4" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>2. SP #761 in mat area #1 encroaches 7/8" at elevation -34.12. WOJV is proposing to decrease the specified 36" wall thickness to 35-1/8" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>3. SPs #765-770, vary in the degree of encroachment the worst of which is SP #765 in mat area #1 which encroaches 1-7/8" at elevation 25.10. WOJV is proposing to decrease the specified 36" wall thickness to 34-1/8" to clear the encroaching SPs. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>4. SP #787 in mat area #1 encroaches 7/8" at elevation -34.42. WOJV is proposing to decrease the specified 36" wall</p>				





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	<p>supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>Marked up sheet SH-2000 (Exhibit-E) depicts the location of the encroaching SPs and the degree in which they are encroaching.</p> <p>1. SP #819 in mat area #1 encroaches 2-3/16" at elevation -34.24. WOJV is proposing to decrease the specified 36" wall thickness to 33-13/16" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p>					<p>thickness to 35-1/8" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>Marked up sheet SH-2000 (Exhibit-E) depicts the location of the encroaching SPs and the degree in which they are encroaching.</p> <p>1. SP #819 in mat area #1 encroaches 2-3/16" at elevation -34.24. WOJV is proposing to decrease the specified 36" wall thickness to 33-13/16" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p>
<b>T-0448.2</b>	<b>BSE - CDSM Soldier Pile Encroachment</b>	<b>Closed</b>	<b>01</b>	<b>04/29/2013</b>	<b>05/09/2013</b>	<b>04/26/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: T-0448, SH-2001, SH-2000						Ref: T-0448, SH-2001, SH-2000
<p>Previous RFI response #T-0448 (Exhibit-A) only addressed the impact of the encroaching CDSM soldier piles (SPs) on the first or bottom wall segments. This RFI address the encroaching SPs in mat slab areas 1&amp;2 (Exhibit-B) at all levels of wall. This RFI shall supersede previous RFI response #T-0448.</p> <p>Marked up sheet SH-2001 (Exhibit-C) depicts the location of the encroaching SPs and the degree in which they are encroaching.</p> <p>1. SP #753 in mat area #2 encroaches 1-1/4" at elevation -34.12. WOJV is proposing to decrease the specified 36" wall thickness to 34-3/4" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at</p>						<p>Previous RFI response #T-0448 (Exhibit-A) only addressed the impact of the encroaching CDSM soldier piles (SPs) on the first or bottom wall segments. This RFI address the encroaching SPs in mat slab areas 1&amp;2 (Exhibit-B) at all levels of wall. This RFI shall supersede previous RFI response #T-0448.</p> <p>Marked up sheet SH-2001 (Exhibit-C) depicts the location of the encroaching SPs and the degree in which they are encroaching.</p> <p>1. SP #753 in mat area #2 encroaches 1-1/4" at elevation -34.12. WOJV is proposing to decrease the specified 36" wall thickness to 34-3/4" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract</p>





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	<p>the level of encroachment.</p> <p>2. SP #761 in mat area #1 encroaches 7/8" at elevation -34.12. WOJV is proposing to decrease the specified 36" wall thickness to 35-1/8" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>3. SPs #765-770, vary in the degree of encroachment the worst of which is SP #765 in mat area #1 which encroaches 1-7/8" at elevation 25.10. WOJV is proposing to decrease the specified 36" wall thickness to 34-1/8" to clear the encroaching SPs. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>4. SP #787 in mat area #1 encroaches 7/8" at elevation -34.42. WOJV is proposing to decrease the specified 36" wall thickness to 35-1/8" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>Marked up sheet SH-2000 (Exhibit-E) depicts the location of the encroaching SPs and the degree in which they are encroaching.</p> <p>1. SP #819 in mat area #1 encroaches 2-3/16" at elevation -34.24. WOJV is proposing to decrease the specified 36" wall thickness to 33-13/16" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>Please confirm that WOJV's proposed solutions are acceptable.</p>					<p>#11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>2. SP #761 in mat area #1 encroaches 7/8" at elevation -34.12. WOJV is proposing to decrease the specified 36" wall thickness to 35-1/8" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>3. SPs #765-770, vary in the degree of encroachment the worst of which is SP #765 in mat area #1 which encroaches 1-7/8" at elevation 25.10. WOJV is proposing to decrease the specified 36" wall thickness to 34-1/8" to clear the encroaching SPs. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>4. SP #787 in mat area #1 encroaches 7/8" at elevation -34.42. WOJV is proposing to decrease the specified 36" wall thickness to 35-1/8" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>Marked up sheet SH-2000 (Exhibit-E) depicts the location of the encroaching SPs and the degree in which they are encroaching.</p> <p>1. SP #819 in mat area #1 encroaches 2-3/16" at elevation -34.24. WOJV is proposing to decrease the specified 36" wall thickness to 33-13/16" to clear the encroaching SP. WOJV would reduce the wall thickness while compensating by supplementing the base contract #11 bars @ 8" o.c. with intermediate #7 bars (Option #3 Exhibit-D) exclusively at the level of encroachment.</p> <p>Please confirm that WOJV's proposed solutions are</p>



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acceptable.						
<b>T-0448.3</b>	<b>BGP - CDSM Soldier Pile Encroachment, mat areas 1&amp;2 all levels.</b>	<b>Closed</b>	<b>01</b>	<b>05/03/2013</b>	<b>05/17/2013</b>	<b>04/26/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Previous RFI #T-0448, Related RFI #T-0530.		Reference: Previous RFI #T-0448, Related RFI #T-0530.				
Previous RFI response #T-0448 only addressed the impact of the encroaching CDSM soldier piles (SPs) on the first or bottom wall segments. This RFI addresses the encroaching SPs in mat slab areas 1&2 at all levels of wall. This RFI shall supersede previous RFI response #T-0448.		Previous RFI response #T-0448 only addressed the impact of the encroaching CDSM soldier piles (SPs) on the first or bottom wall segments. This RFI addresses the encroaching SPs in mat slab areas 1&2 at all levels of wall. This RFI shall supersede previous RFI response #T-0448.				
Please see attachment SK-1 for RFI T-0448.3 questions.		Please see attachment SK-1 for RFI T-0448.3 questions.				
<b>T-0448.4</b>	<b>CDSM Soldier Pile Encroachment</b>	<b>Closed</b>	<b>01</b>	<b>05/09/2013</b>	<b>05/19/2013</b>	<b>05/24/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Previous RFI #T-0448, Related RFI #T-0530.		Reference: Previous RFI #T-0448, Related RFI #T-0530.				
Previous RFI response #T-0448 only addressed the impact of the encroaching CDSM soldier piles (SPs) on the first or bottom wall segments. This RFI addresses the encroaching SPs in mat slab areas 1&2 at all levels of wall. This RFI shall supersede previous RFI response #T-0448.		Previous RFI response #T-0448 only addressed the impact of the encroaching CDSM soldier piles (SPs) on the first or bottom wall segments. This RFI addresses the encroaching SPs in mat slab areas 1&2 at all levels of wall. This RFI shall supersede previous RFI response #T-0448.				
Please see attachment SK-1 for RFI T-0448.4 questions.		Please see attachment SK-1 for RFI T-0448.4 questions.				







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T-0452	<b>BGP - Concrete Beam Under Slab</b>  <b>From:</b> Webcor Construction LP      Robert Kjome  <b>REQUEST:</b>  Reference Specification: 033020 Reference Drawing: S1-2251, S1-3205, S1-3400  Please reference the Vehicle/Bike ramp framing plans on S1-2251. Detail 1 calls for a 36" x 48" concrete beam below the ramp slab. However, this concrete beam is not indicated in section detail 7 on S1-3205. The beam size and specifications as described on S1-2251 does not match a beam listed in the beam schedule on sheet SI-3400. The plan on SI-2251 does not clearly show where this beam begins and ends.  Please provide additional information and clarification regarding this 36" x 48" concrete beam.	Closed	01	03/19/2013	03/29/2013	03/25/2013
	<b>ANSWER:</b>  Reference Specification: 033020 Reference Drawing: S1-2251, S1-3205, S1-3400  Please reference the Vehicle/Bike ramp framing plans on S1-2251. Detail 1 calls for a 36" x 48" concrete beam below the ramp slab. However, this concrete beam is not indicated in section detail 7 on S1-3205. The beam size and specifications as described on S1-2251 does not match a beam listed in the beam schedule on sheet SI-3400. The plan on SI-2251 does not clearly show where this beam begins and ends.  Please provide additional information and clarification regarding this 36" x 48" concrete beam.					



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<b>T-0453</b>	<b>BGP - Angle Steel Beam Connections</b>	<b>Closed</b>	<b>01</b>	<b>03/19/2013</b>	<b>03/29/2013</b>	<b>03/29/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Specification:032000 Reference Drawings:S1-3411, S1-2251  Please reference the Vehicle/Bike beam end support detail 1 on S1-3411. The L8x8 connections appear to be shown to be fabricated at a 90 deg angle between the foundation wall and the Vehicle/Bike beams. Per Detail 1 on sheet S1-2251, the beams are shown to be intersecting the foundation wall at varying angles. SCCI requests further clarification/details at the beam locations for the fabrication of the L8x8 connections.		<b>ANSWER:</b>  Reference Specification:032000 Reference Drawings:S1-3411, S1-2251  Please reference the Vehicle/Bike beam end support detail 1 on S1-3411. The L8x8 connections appear to be shown to be fabricated at a 90 deg angle between the foundation wall and the Vehicle/Bike beams. Per Detail 1 on sheet S1-2251, the beams are shown to be intersecting the foundation wall at varying angles. SCCI requests further clarification/details at the beam locations for the fabrication of the L8x8 connections.				
<b>T-0453.1</b>	<b>BGP - Vehicle/Bike Beam End Supports</b>	<b>Closed</b>	<b>01</b>	<b>04/11/2013</b>	<b>04/21/2013</b>	<b>04/22/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>  Ref: RFI T-0453, AI-7401, SK-115  Please reference attached drawings. RFI T-0453 stated that the L8x8x1 1/8" shall be bent to match the angle at which the Vehicle/Bike ramp beams meet the wall. At the western most beam the acute angle at which the beam meets the wall is 56 degrees and the obtuse angle is 124 degrees. See attached marked up Contract Drawing AI-7401 for angle measurements. Bending the 1 1/8" thick legs of the L8x8 is not feasible and would structually stress the member. SCCI proposes to weld two 1 1/8" plates together to fabricate the angles. See attached drawing SK-115 for details. The additional two beam members shall be fabricated per the measured angles.  Please advise if this is acceptable.		<b>ANSWER:</b>  Ref: RFI T-0453, AI-7401, SK-115  Please reference attached drawings. RFI T-0453 stated that the L8x8x1 1/8" shall be bent to match the angle at which the Vehicle/Bike ramp beams meet the wall. At the western most beam the acute angle at which the beam meets the wall is 56 degrees and the obtuse angle is 124 degrees. See attached marked up Contract Drawing AI-7401 for angle measurements. Bending the 1 1/8" thick legs of the L8x8 is not feasible and would structually stress the member. SCCI proposes to weld two 1 1/8" plates together to fabricate the angles. See attached drawing SK-115 for details. The additional two beam members shall be fabricated per the measured angles.  Please advise if this is acceptable.				
<b>T-0453.2</b>	<b>BGP - Clarification of Vehicle/Bike Beam End Support</b>	<b>Closed</b>	<b>CR</b>	<b>10/02/2013</b>	<b>10/12/2013</b>	<b>10/16/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached drawing S1-2251, S1-3411. A1-7401 and SCCI sketch SK-115.		<b>ANSWER:</b>  Please refer to attached drawing S1-2251, S1-3411. A1-7401 and SCCI sketch SK-115.				



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T-0454	<b>BGP - Steel Cap Collar Weld Location</b>  <b>From:</b> Webcor Construction LP      Robert Kjome  <b>REQUEST:</b> Reference Specification: 055010 Reference Drawings: S1-3003, A1-8711, Submittal No. TG0600-036  Please reference attached Contract Drawing SI-3003 and AI-8711 along with approved as noted dewatering pipe sleeve shop drawing. The 3 dewatering sleeve drawings depict conflicting weld locations for the 5/16" fillet weld of the steel cap collar to sleeve connection (see highlighted drawings).  Please clarify/confirm the location of this weld.	Closed	01	03/19/2013	03/29/2013	03/22/2013
						<b>ANSWER:</b> Reference Specification: 055010 Reference Drawings: S1-3003, A1-8711, Submittal No. TG0600-036  Please reference attached Contract Drawing SI-3003 and AI-8711 along with approved as noted dewatering pipe sleeve shop drawing. The 3 dewatering sleeve drawings depict conflicting weld locations for the 5/16" fillet weld of the steel cap collar to sleeve connection (see highlighted drawings).  Please clarify/confirm the location of this weld.





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<b>T-0455</b>	<b>BGP - Out of Plumb Dewatering Casing</b>	<b>Closed</b>	<b>01</b>	<b>03/19/2013</b>	<b>03/29/2013</b>	<b>03/27/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification:055010-3.2.C Reference Drawings: S1-3003 Reference Photo: attached		Reference Specification:055010-3.2.C Reference Drawings: S1-3003 Reference Photo: attached				
Please reference Sheet S1-3003 of the Contract Drawings and Spec Section 055010-3.2.C SCCI spot checked two of the existing dewatering wells for plumbness and found them both to be approximately 3/4" over 48" out of plumb (see attached photos). With this existing condition, SCCI can not adhere to the plumbness tolerance (1/16") for installation and maintain the required 1/2" maximum gap between sleeve and casing per Section 2 of Sheet S1-3003. SCCI suggests increasing the diameter of the sleeve and granting a variance on the 1/2" gap tolerance per Sheet S1-3003. SCCI will maintain adherence to the installation tolerances in Spec Section 05 50 10.		Please reference Sheet S1-3003 of the Contract Drawings and Spec Section 055010-3.2.C SCCI spot checked two of the existing dewatering wells for plumbness and found them both to be approximately 3/4" over 48" out of plumb (see attached photos). With this existing condition, SCCI can not adhere to the plumbness tolerance (1/16") for installation and maintain the required 1/2" maximum gap between sleeve and casing per Section 2 of Sheet S1-3003. SCCI suggests increasing the diameter of the sleeve and granting a variance on the 1/2" gap tolerance per Sheet S1-3003. SCCI will maintain adherence to the installation tolerances in Spec Section 05 50 10.				
<b>T-0455.1</b>	<b>BGP - Dewatering Well Above Grade PVC Pipe</b>	<b>Closed</b>	<b>01</b>	<b>03/29/2013</b>	<b>04/08/2013</b>	<b>04/02/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Drawings: A1-8711		Reference Drawings: A1-8711				
Per discussion in the pre-installation and preparatory DFOW meetings for the metal fabrication penetration sleeves, the PVC dewatering casing above the mud slab can be cut just above or at top of mud slab elevation to avoid varying diameter issues. Without the dewatering casing present above mud slab grade, the varying casing diameter issues and plumbness issues are solved. The to avoid debris entering the dewatering casing, the casing would not be cut until the penetration sleeve is to be installed. Please confirm per the discussions in the meeting that cutting the casing is acceptable. Please note that the grouting back of the dewatering casing shortly after the decommissioning of the dewatering pump will be uniform (without segregation) for both below mudslab elevation and above.		Per discussion in the pre-installation and preparatory DFOW meetings for the metal fabrication penetration sleeves, the PVC dewatering casing above the mud slab can be cut just above or at top of mud slab elevation to avoid varying diameter issues. Without the dewatering casing present above mud slab grade, the varying casing diameter issues and plumbness issues are solved. The to avoid debris entering the dewatering casing, the casing would not be cut until the penetration sleeve is to be installed. Please confirm per the discussions in the meeting that cutting the casing is acceptable. Please note that the grouting back of the dewatering casing shortly after the decommissioning of the dewatering pump will be uniform (without segregation) for both below mudslab elevation and above.				
<b>T-0456</b>	<b>BGP - Mass Concrete Temperature Monitoring Equipment Installation in MAT Slab Closed</b>		<b>01</b>	<b>03/25/2013</b>	<b>03/25/2013</b>	<b>04/03/2013</b>







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T-0458	BGP - Concourse Slab CJ Layout	Closed	01	03/26/2013	04/05/2013	04/04/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference Specification: 03 30 20 Reference Drawings: CJ-05 and CJ-22		Reference Specification: 03 30 20 Reference Drawings: CJ-05 and CJ-22				
In order to meet the Joints in Concrete specifications (03 30 20-3.2), SCCI's revision of Construction Joint (CJ) Layout Submittal requires the CJ between concourse slabs D116 and D117 (see atttached reference drawing CJ-22) to be 2'-10" outside of the required center third of the span (reference 03 30 20- 3.2.B.1 ). Please advise if this is acceptable.		In order to meet the Joints in Concrete specifications (03 30 20-3.2), SCCI's revision of Construction Joint (CJ) Layout Submittal requires the CJ between concourse slabs D116 and D117 (see attached reference drawing CJ-22) to be 2'-10" outside of the required center third of the span (reference 03 30 20- 3.2.B.1 ). Please advise if this is acceptable.				
If the above is not acceptable, then SCCI proposes to move the CJ line (between D116 and D117) 2'-10" to the East. Since mat slab S108 (see attached reference drawing CJ-05) is currently 120'-0" wide, it will be increased to 122 '-1 0" wide. This would be wider than the maximum width of 120' -0" as specified in 03 30 20- 3.2.A.3. Please advise if this alternative is acceptable.		If the above is not acceptable, then SCCI proposes to move the CJ line (between D116 and D117) 2'-10" to the East. Since mat slab S108 (see attached reference drawing CJ-05) is currently 120'-0" wide, it will be increased to 122 '-1 0" wide. This would be wider than the maximum width of 120' -0" as specified in 03 30 20-3.2.A.3. Please advise if this alternative is acceptable.				

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T-0459	BGP - Waterproofing and CJ Concourse Slab Layout Conflict	Closed	CR	03/27/2013	04/06/2013	04/01/2013
<b>From:</b> Webcor Construction LP                  Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification: 07 12 10 Reference Drawings: A1-2203 and S1-3201		Reference Specification: 07 12 10 Reference Drawings: A1-2203 and S1-3201				
Please reference AI-2203 and SI-3201 of the Contract Plans and the attached drawings. The current elevation at the bottom of the 2nd level bracing lookouts is at approximately -5.13, WEST of Grid 9 (see concourse slab drawing). The proposed top of concourse slab elevation is to be -5.42, WEST of Grid 9. Per the WPM-1 waterproofing system, the minimum overall tie-in dimension needed for the succeeding lift is approximately 1 '-11" (see attached waterproofing drawing).		Please reference AI-2203 and SI-3201 of the Contract Plans and the attached drawings. The current elevation at the bottom of the 2nd level bracing lookouts is at approximately -5.13, WEST of Grid 9 (see concourse slab drawing). The proposed top of concourse slab elevation is to be -5.42, WEST of Grid 9. Per the WPM-1 waterproofing system, the minimum overall tie-in dimension needed for the succeeding lift is approximately 1 '-11" (see attached waterproofing drawing).				
The current elevation at the bottom of the 2nd level bracing lookouts is at approximately -6.15, EAST of Grid 9 (see concourse slab drawing). The proposed top of concourse slab elevation CJ is to be -7.67, EAST of Grid 9. Per the WPM-1 waterproofing system, the minimum overall tie-in dimension needed for the succeeding lift is approximately 1 '-11" (see attached waterproofing drawing).		The current elevation at the bottom of the 2nd level bracing lookouts is at approximately -6.15, EAST of Grid 9 (see concourse slab drawing). The proposed top of concourse slab elevation CJ is to be -7.67, EAST of Grid 9. Per the WPM-1 waterproofing system, the minimum overall tie-in dimension needed for the succeeding lift is approximately 1 '-11" (see attached waterproofing drawing).				
In both locations, the minimum required dimension (1 '-11") to tie-in to the next lift of waterproofing can not be reached with the current location of the 2nd level bracing lookouts and the proposed concourse slab elevations. SCCI is restricted in location for the CJ due to the absolute concourse slab location and elevation.		In both locations, the minimum required dimension (1 '-11") to tie-in to the next lift of waterproofing can not be reached with the current location of the 2nd level bracing lookouts and the proposed concourse slab elevations. SCCI is restricted in location for the CJ due to the absolute concourse slab location and elevation.				
Furthermore, a similar conflict exists in the 1st foundation wall lift and the 3rd level of bracing lookouts (see 1st wall lift drawing). With SCCI's current location of the CJ, there is virtually no room to allow for the waterproofing overlap to occur. SCCI fully understands its freedom to manipulate the location of the CJ's by lowering it approximately 2'. This will potentially change BBII's rebracing plans.		Furthermore, a similar conflict exists in the 1st foundation wall lift and the 3rd level of bracing lookouts (see 1st wall lift drawing). With SCCI's current location of the CJ, there is virtually no room to allow for the waterproofing overlap to occur. SCCI fully understands its freedom to manipulate the location of the CJ's by lowering it approximately 2'. This will potentially change BBII's rebracing plans.				
Please advise.		Please advise.				
T-0460	BGP - Waterproofing and CJ at Mat Slab Conflict	Closed	01	03/27/2013	04/06/2013	04/01/2013
<b>From:</b> Webcor Construction LP                  Lynn Kowallis						





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<b>T-0462</b>	<b>BGP - Grounding Wire Penetrations in Mud &amp; Protection Slab</b>	<b>Closed</b>	<b>01</b>	<b>03/28/2013</b>	<b>04/07/2013</b>	<b>04/10/2013</b>
<b>From:</b> Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>  Ref: 5/A1-8710 and Submittal Package TG0600-023.1 sheet 5.07, Specification Section 26 05 27  The contract plans and specifications call for the grounding wire to be bare copper. At the locations in which this grounding wire penetrates the mud & protection slab, the waterproofing supplier (Laurenco) requires the ground wire penetration to be solid metal or a rod. Laurenco has stated that if the electrical grounding penetration through the slab is wire as shown in the plans and specifications, the waterproofing system will leak. Please advise.		<b>ANSWER:</b>  Ref: 5/A1-8710 and Submittal Package TG0600-023.1 sheet 5.07, Specification Section 26 05 27  The contract plans and specifications call for the grounding wire to be bare copper. At the locations in which this grounding wire penetrates the mud & protection slab, the waterproofing supplier (Laurenco) requires the ground wire penetration to be solid metal or a rod. Laurenco has stated that if the electrical grounding penetration through the slab is wire as shown in the plans and specifications, the waterproofing system will leak. Please advise.				
<b>T-0463</b>	<b>BSE - Micropiles W400 &amp; 417 Relocation</b>	<b>Closed</b>	<b>01</b>	<b>03/28/2013</b>	<b>03/29/2013</b>	<b>04/01/2013</b>
<b>From:</b> Webcor Construction LP                      Ian Corcorran						
<b>REQUEST:</b>  Reference Specification: 31 63 33  Micropiles W400 and W417 cannot be installed as laid out due to an overhead obstruction (Geotechnical Instrumentation Pipes).  BBII proposes moving W400 South 5' and W417 South 3' to provide adequate clearance. The proposed locations for Micropile W400 and W417 will be within the geothermal area; however, the proposed locations do not appear to impact geothermal piping.  See attached sketch.  Please confirm this is acceptable.		<b>ANSWER:</b>  Reference Specification: 31 63 33  Micropiles W400 and W417 cannot be installed as laid out due to an overhead obstruction (Geotechnical Instrumentation Pipes).  BBII proposes moving W400 South 5' and W417 South 3' to provide adequate clearance. The proposed locations for Micropile W400 and W417 will be within the geothermal area; however, the proposed locations do not appear to impact geothermal piping.  See attached sketch.  Please confirm this is acceptable.				
<b>T-0464</b>	<b>BGP - Clarification of Curing and Thermal Protection Methods</b>	<b>Closed</b>	<b>01</b>	<b>03/28/2013</b>	<b>04/07/2013</b>	<b>04/09/2013</b>
<b>From:</b> Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>  Ref: Specification Section 033020.2.11.D, and 033020.3.7.A.c Ref: Submittal TG0600-201.1		<b>ANSWER:</b>  Ref: Specification Section 033020.2.11.D, and 033020.3.7.A.c Ref: Submittal TG0600-201.1				



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	<p>Reference Mass Concrete Plan prepared by CTL Group, Submittal Package TG0600-201.1, item 033000-011.1, page 10 - Curing. This paragraph state " ... slab placement will be cured in a method that does not artificially excessively decrease the surface temperature of the concrete placement. This means that continuous wetting of the concrete should be avoided."</p> <p>Reference specification section 033020.3.11.D. This paragraph states "Thermal Blankets are required to insure minimal thermal cracking."</p> <p>Reference specification section 033020.3.7.A.C. This paragraph calls for "Moist curing of the Mat Slab by means of continuously covering the slab with water."</p> <p>Reference attached letter " Wet Curing Mat Slab", author Bob Foley, CEMEX QC Manager Bay Area.</p> <p>SCCI intends to cure the Mat slab per recommendations of CLT Group Mass Concrete Plan. Due to the high risk of thermal cracking, SCCI intends to utilize impermeable insulated curing blankets and not cover the slab with water.</p> <p>Is this acceptable?</p>					<p>Reference Mass Concrete Plan prepared by CTL Group, Submittal Package TG0600-201.1, item 033000-011.1, page 10 - Curing. This paragraph state " ... slab placement will be cured in a method that does not artificially excessively decrease the surface temperature of the concrete placement. This means that continuous wetting of the concrete should be avoided."</p> <p>Reference specification section 033020.3.11.D. This paragraph states "Thermal Blankets are required to insure minimal thermal cracking."</p> <p>Reference specification section 033020.3.7.A.C. This paragraph calls for "Moist curing of the Mat Slab by means of continuously covering the slab with water."</p> <p>Reference attached letter " Wet Curing Mat Slab", author Bob Foley, CEMEX QC Manager Bay Area.</p> <p>SCCI intends to cure the Mat slab per recommendations of CLT Group Mass Concrete Plan. Due to the high risk of thermal cracking, SCCI intends to utilize impermeable insulated curing blankets and not cover the slab with water.</p> <p>Is this acceptable?</p>
<b>T-0464.1</b>	<b>BGP - Mat Slab Curing Techniques</b>	<b>Closed</b>	<b>01</b>	<b>04/26/2013</b>	<b>05/06/2013</b>	<b>05/08/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Specification: 03 30 20						Reference Specification: 03 30 20
Please reference TG06.0 Contract Specifications 03 30 20.3.7.5.b.3 and Project Meeting with Thornton Tomasetti (SER), held Thursday, April 25.						Please reference TG06.0 Contract Specifications 03 30 20.3.7.5.b.3 and Project Meeting with Thornton Tomasetti (SER), held Thursday, April 25.
SCCI intends to Moist cure the Mat Foundation Slab using the above referenced method found in the contract specifications and discussed in the above mentioned Project meeting.						SCCI intends to Moist cure the Mat Foundation Slab using the above referenced method found in the contract specifications and discussed in the above mentioned Project meeting.



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	Please confirm this method is acceptable.					Please confirm this method is acceptable.
<b>T-0465</b>	<b>BGP - Relocation of Geothermal Risers Due to Leaking CDSM Wall</b>	<b>Closed</b>	<b>01</b>	<b>03/28/2013</b>	<b>04/07/2013</b>	<b>04/04/2013</b>
	<b>From:</b> Webcor Construction LP                      Robert Kjome					
	<b>REQUEST:</b>  Reference photo: Attached  As seen in the picture attached, water is leaking through the surface of not only the CDSM panel that the geothermal riser is laid out on, but the various adjacent CDSM panels as well.  Please confirm that SCCI can move the Field 1 risers location between Piles 35 & 36 and the Field 2 risers location between Piles 38 & 39. Both of these new locations appear to be leaking less than the original riser locations.					<b>ANSWER:</b>  Reference photo: Attached  As seen in the picture attached, water is leaking through the surface of not only the CDSM panel that the geothermal riser is laid out on, but the various adjacent CDSM panels as well.  Please confirm that SCCI can move the Field 1 risers location between Piles 35 & 36 and the Field 2 risers location between Piles 38 & 39. Both of these new locations appear to be leaking less than the original riser locations.
<b>T-0466</b>	<b>BGP - Ground Rod for SFPUC</b>	<b>Closed</b>	<b>01</b>	<b>03/29/2013</b>	<b>04/08/2013</b>	<b>04/10/2013</b>
	<b>From:</b> Webcor Construction LP                      Joanne Filipas					
	<b>REQUEST:</b>  In follow up to the 3/28/2013 OAC, PCP informed us that the SF PUC requires a ground rod to be installed. Please provide all necessary information including but not limited to rod type, length, and location.					<b>ANSWER:</b>  In follow up to the 3/28/2013 OAC, PCP informed us that the SF PUC requires a ground rod to be installed. Please provide all necessary information including but not limited to rod type, length, and location.





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<b>T-0466.1</b>	<b>BGP - Ground Rod for SFPUC</b>	<b>Closed</b>	<b>01</b>	<b>04/11/2013</b>	<b>04/21/2013</b>	<b>04/23/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref: RFI T-0466, RFI T-0442		Ref: RFI T-0466, RFI T-0442				
SCCI is in receipt of the response to RFI T -0466 concerning the addition of SFPUC grounding rods/grids. In order to price this change SCCI and its electrical subcontractor need the following information:		SCCI is in receipt of the response to RFI T -0466 concerning the addition of SFPUC grounding rods/grids. In order to price this change SCCI and its electrical subcontractor need the following information:				
On Drawing SKE-021, Note 8, please provide a location on where to terminate each of the four 4/0 cables at the lower concourse slab. A revised SKE-024 drawing showing the exact stub up locations and dimensions is needed to accurately price and construct this change.		On Drawing SKE-021, Note 8, please provide a location on where to terminate each of the four 4/0 cables at the lower concourse slab. A revised SKE-024 drawing showing the exact stub up locations and dimensions is needed to accurately price and construct this change.				
On Drawing SKE-022, Note 3, please again advise where to terminate the four 4/0 cables at the lower concourse slab. A revised SKE-024 drawing showing the exact stub up locations and dimensions is needed to accurately price and construct this change.		On Drawing SKE-022, Note 3, please again advise where to terminate the four 4/0 cables at the lower concourse slab. A revised SKE-024 drawing showing the exact stub up locations and dimensions is needed to accurately price and construct this change.				
Please confirm that the details from the RFI T -442 response will apply to these penetrations.		Please confirm that the details from the RFI T -442 response will apply to these penetrations.				
Please confirm that there only two areas (detailed on SKE-021 & SKE-022) that will require the additional SFPUC grounding.		Please confirm that there only two areas (detailed on SKE-021 & SKE-022) that will require the additional SFPUC grounding.				
<b>T-0466.2</b>	<b>BGP - Requesting Detail 2 on drawing E1-6006</b>	<b>Closed</b>	<b>01</b>	<b>04/19/2013</b>	<b>04/29/2013</b>	<b>04/23/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref: RFI T-0466, Drawing E1-6006		Ref: RFI T-0466, Drawing E1-6006				
Reference is made to RFI T -0466 and the attached sketches. Note I on SKE-022, Note A on SKE-023 and the first note below (Top of Slab -35'-8") references a detail on Contract Drawing E1-6006 for the added SFPUC Ground Rods. The current drawing E1-6006 does not have the noted detail. SCCI requests an updated E1-6006 drawing with the new detail.		Reference is made to RFI T -0466 and the attached sketches. Note I on SKE-022, Note A on SKE-023 and the first note below (Top of Slab -35'-8") references a detail on Contract Drawing E1-6006 for the added SFPUC Ground Rods. The current drawing E1-6006 does not have the noted detail. SCCI requests an updated E1-6006 drawing with the new detail.				
<b>T-0467</b>	<b>BGP - Lower Concourse Conflicts</b>	<b>Closed</b>	<b>01</b>	<b>03/28/2013</b>	<b>03/28/2013</b>	<b>04/01/2013</b>





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<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Reference Drawings: SH-5002, SH-2007, SH-2008, SH-3001</p> <p>SCCI is in discovery that the W21x101 and W14x30 support beams and lookouts at the shoring level B are encroaching into the lower concourse slab between GL 1 and 9.5. TOC for the concourse slab is at EL. -5.42' (GL 1 thru 9.5); Bottom of W21x101 support beams and W14x30 lookouts are at EL. -6.25' and -5.67' respectively.</p> <p>Please confirm that these will be removed prior to construction of the lower concourse level. If these struts supports are to remain throughout construction of the lower concourse please provide detailed drawings showing incorporation (or blockout) of these W21x101 support beams and W14x30 lookouts into the lower concourse slab.</p>						<p><b>ANSWER:</b></p> <p>Reference Drawings: SH-5002, SH-2007, SH-2008, SH-3001</p> <p>SCCI is in discovery that the W21x101 and W14x30 support beams and lookouts at the shoring level B are encroaching into the lower concourse slab between GL 1 and 9.5. TOC for the concourse slab is at EL. - 5.42' (GL 1 thru 9.5); Bottom of W21x101 support beams and W14x30 lookouts are at EL. -6.25' and - 5.67' respectively.</p> <p>Please confirm that these will be removed prior to construction of the lower concourse level. If these struts supports are to remain throughout construction of the lower concourse please provide detailed drawings showing incorporation (or blockout) of these W21x101 support beams and W14x30 lookouts into the lower concourse slab.</p>
<b>T-0468</b>	<b>BGP - Geothermal Pipe Riser in CDSM Wall Excavation Specification</b>	<b>Closed</b>	<b>01</b>	<b>03/29/2013</b>	<b>04/08/2013</b>	<b>04/08/2013</b>
<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Reference Specification: 23 57 34, 31 23 34</p> <p>Per discussions with the designer (ARUP), the CDSM wall will continue to move until the mat slab has been placed. With the geothermal pipe riser being installed much ahead of the mat slab, there is a good chance that the riser chase pour back will be jeopardized by the wall movement. Per specification 31 23 34, 3.2, B, the geothermal riser pipe chase cannot remain open for longer than 10 calendar days. Is it acceptable to extend this duration to account for the wall movement until the mat slab is poured?</p> <p>Please advise.</p>						<p><b>ANSWER:</b></p> <p>Reference Specification: 23 57 34, 31 23 34</p> <p>Per discussions with the designer (ARUP), the CDSM wall will continue to move until the mat slab has been placed. With the geothermal pipe riser being installed much ahead of the mat slab, there is a good chance that the riser chase pour back will be jeopardized by the wall movement. Per specification 31 23 34, 3.2, B, the geothermal riser pipe chase cannot remain open for longer than 10 calendar days. Is it acceptable to extend this duration to account for the wall movement until the mat slab is poured?</p> <p>Please advise.</p>
<b>T-0469</b>	<b>BGP - Embed Nail Holes</b>	<b>Closed</b>	<b>01</b>	<b>04/01/2013</b>	<b>04/11/2013</b>	<b>04/11/2013</b>
<p><b>From:</b> Webcor Construction LP                      Lynn Kowallis</p> <p><b>REQUEST:</b></p>						<p><b>ANSWER:</b></p>





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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

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<b>T-0470.1</b>	<b>BGP - Concourse Penetrations Discrepancies</b>	<b>Closed</b>	<b>01</b>	<b>07/16/2013</b>	<b>07/16/2013</b>	<b>07/29/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Reference attached sketch and RFI T-0470.  Note GR-9 on S-0005 raises a non constructability issue with the concourse slab penetration blockout. If the GR-9 is followed the minimal clear cover over couplers on the lower concourse slab will not conform to the specifications. Please provide rebar details for the concourse slab penetrations that conform to the specifications.						<b>ANSWER:</b>  Reference attached sketch and RFI T-0470.  Note GR-9 on S-0005 raises a non constructability issue with the concourse slab penetration blockout. If the GR-9 is followed the minimal clear cover over couplers on the lower concourse slab will not conform to the specifications. Please provide rebar details for the concourse slab penetrations that conform to the specifications.
<b>T-0471</b>	<b>BGP - Galvanizing Testing</b>	<b>Closed</b>	<b>01</b>	<b>04/02/2013</b>	<b>04/12/2013</b>	<b>04/05/2013</b>
<b>From:</b> Webcor Construction LP Lynn Kowallis						
<b>REQUEST:</b>  Ref: Specification Section 05 05 15 3.6 A  Section 3.6 A of 05 05 15 -Hot Dip Galvanizing calls for "the contractor's testing laboratory shall perform inspection and testing of zinc coatings under the guidelines outlined in the American Galvanizer's Association (AGA)." Per the hot dip galvanizing pre-installation meeting, SCCI plans to use AZZ Galvanizing Services and their independent testing agency for shop testing and inspection and to fulfill all requirements described in 05 05 15-3.6 -Testing. Personnel qualifications are available upon request.  Please confirm this is acceptable.						<b>ANSWER:</b>  Ref: Specification Section 05 05 15 3.6 A  Section 3.6 A of 05 05 15 -Hot Dip Galvanizing calls for "the contractor's testing laboratory shall perform inspection and testing of zinc coatings under the guidelines outlined in the American Galvanizer's Association (AGA)." Per the hot dip galvanizing pre-installation meeting, SCCI plans to use AZZ Galvanizing Services and their independent testing agency for shop testing and inspection and to fulfill all requirements described in 05 05 15-3.6 -Testing. Personnel qualifications are available upon request.  Please confirm this is acceptable.
<b>T-0472</b>	<b>BGP - Future Train Platform Wall Conflict with Trestle Pile Opening</b>	<b>Closed</b>	<b>01</b>	<b>04/02/2013</b>	<b>04/16/2013</b>	<b>04/15/2013</b>
<b>From:</b> Webcor Construction LP Ian Corcorran						
<b>REQUEST:</b>  Ref. Dwg: S1-2054, S1-2055, 1/S1-3205  Dwg sheets S1-2054 and S1-2055 depict the future walls for the train platform which per detail 1/S1-3205 receive #7 dowels E.F. at 8" O.C. with a formsaver coupler positioned at the top of the mat slab. When referencing S1-2054 and S1-2055 it is noted that in 14 locations the openings for the trestle pile are shown directly on top of this future wall						<b>ANSWER:</b>  Ref. Dwg: S1-2054, S1-2055, 1/S1-3205  Dwg sheets S1-2054 and S1-2055 depict the future walls for the train platform which per detail 1/S1-3205 receive #7 dowels E.F. at 8" O.C. with a formsaver coupler positioned at the top of the mat slab. When referencing S1-2054 and S1-2055 it is noted that in 14 locations the openings for the trestle pile are shown





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<b>T-0475</b>	<b>BGP - Mat Slab Drainage Sloping</b>	<b>Closed</b>	<b>01</b>	<b>04/03/2013</b>	<b>04/17/2013</b>	<b>04/04/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref. Spec: 03 30 20.3.6.B.1.b		Ref. Spec: 03 30 20.3.6.B.1.b				
Contract specification section 03 30 20.3.6.B.1.b, states "Slope surfaces uniformly to drains where required."		Contract specification section 03 30 20.3.6.B.1.b, states "Slope surfaces uniformly to drains where required."				
However, the contract plans for the below grade package (TG06.0), does not show drainage slope for the Mat Slab. SCCI intends to uniformly place top of Mat Slab at -35' - 8" as shown on contract drawings. If sloping of the Mat Slab is required, please provide drainage plan for top of Mat Slab.		However, the contract plans for the below grade package (TG06.0), does not show drainage slope for the Mat Slab. SCCI intends to uniformly place top of Mat Slab at -35' - 8" as shown on contract drawings. If sloping of the Mat Slab is required, please provide drainage plan for top of Mat Slab.				
<b>T-0476</b>	<b>BSE - Zone 4 Waler Connection Criteria</b>	<b>Closed</b>	<b>01</b>	<b>04/03/2013</b>	<b>04/13/2013</b>	<b>04/05/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
BBII has received COM1902 directing BBII to re-design the east end shoring utilizing similar waler connections provided in the attached sketches.		BBII has received COM1902 directing BBII to re-design the east end shoring utilizing similar waler connections provided in the attached sketches.				
Prior to commencing re-design, BBII requests the following information from the Shoring wall EOR so our Bracing EOR can properly evaluate the interaction between the Bracing system and wall.		Prior to commencing re-design, BBII requests the following information from the Shoring wall EOR so our Bracing EOR can properly evaluate the interaction between the Bracing system and wall.				
Will it be permissible to shed the bracing loads from the transverse end wall (line 35) into the longitudinal CDSM wall (A&J Lines)? If this is acceptable please indicate if there are any limitations, restrictions, or design assumptions regarding the amount of load that can be shed over a given length of wall.		Will it be permissible to shed the bracing loads from the transverse end wall (line 35) into the longitudinal CDSM wall (A&J Lines)? If this is acceptable please indicate if there are any limitations, restrictions, or design assumptions regarding the amount of load that can be shed over a given length of wall.				
<b>T-0477</b>	<b>BSE - Multiple Micropile Relocation (Below Grade Obstruction)</b>	<b>Closed</b>	<b>01</b>	<b>04/03/2013</b>	<b>04/13/2013</b>	<b>04/04/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref: Submittal TG0300-622.4		Ref: Submittal TG0300-622.4				
While installing Micropile W454 as laid out in the approved submittal, BBII encountered a concrete obstruction 8'		While installing Micropile W454 as laid out in the approved submittal, BBII encountered a concrete				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>below grade and was unable to continue drilling at that location. Even though the micropile layout was approved in submittal TG0300-622.4, BBII suspects the drill rig encountered the CDSM Prototype wall as approximately shown in the attached drawing. BBII suggests relocating the micropiles as shown in the attached drawing to avoid the obstruction. The proposed locations for the micropile relocations will be within the geothermal area; however, the proposed locations do not appear to impact geothermal piping. See attached sketch. Please confirm this is acceptable.</p>					
	<p>obstruction 8' below grade and was unable to continue drilling at that location. Even though the micropile layout was approved in submittal TG0300-622.4, BBII suspects the drill rig encountered the CDSM Prototype wall as approximately shown in the attached drawing. BBII suggests relocating the micropiles as shown in the attached drawing to avoid the obstruction. The proposed locations for the micropile relocations will be within the geothermal area; however, the proposed locations do not appear to impact geothermal piping. See attached sketch. Please confirm this is acceptable.</p>					
<b>T-0478</b>	<b>BGP - Shear Reinforcement Clear Cover at Pits</b>	<b>Closed</b>	<b>01</b>	<b>04/03/2013</b>	<b>04/17/2013</b>	<b>04/10/2013</b>
<p><b>From:</b> Webcor Construction LP      Ian Corcorran</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref. Dwg. 2/S1-3005, 3/S1-3008, and S1-2063			Ref. Dwg. 2/S1-3005, 3/S1-3008, and S1-2063			
Sheet S1-3005/Detail 2 specifies the typical top clear cover for the headed shear reinforcement to be 0.75" and for overall length of the headed shear reinforcement to be 57" long. It is not clear if the same clear cover of 0.75" applies to headed shear reinforcement that is within a pit as shown in Sheet S1-3008/Detail 3. Note that typical rebar around the pits are called out to be 3" as shown on sheet SI -2063.			Sheet S1-3005/Detail 2 specifies the typical top clear cover for the headed shear reinforcement to be 0.75" and for overall length of the headed shear reinforcement to be 57" long. It is not clear if the same clear cover of 0.75" applies to headed shear reinforcement that is within a pit as shown in Sheet S1-3008/Detail 3. Note that typical rebar around the pits are called out to be 3" as shown on sheet SI -2063.			
Please confirm top clear cover for headed shear reinforcement that is within a pit.			Please confirm top clear cover for headed shear reinforcement that is within a pit.			



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-0479	BGP - Trestle/Pin pile in MAT Depressions	Closed	CR	04/03/2013	04/17/2013	04/17/2013
<div><div>From: Webcor Construction LP</div><div>Ian Corcorran</div></div>						
REQUEST:			ANSWER:			
Ref. Dwg. S1-2022, S1-2027, S1-3004, S1-3006			Ref. Dwg. S1-2022, S1-2027, S1-3004, S1-3006			
Please reference Sheets S1-2022, S1-2027, S1-3004, and S1-3006 of the Contract Plans. The trestle pile at Gridline D.4 between 4 and 5 is located in the sloped section of the mat slab depression (see highlighted S1-2022). The mat slab depression section plans (S1-3006) do not incorporate this type of sloped pipe penetration. Furthermore, the pin pile between Gridline F.7 and G, just east of 34 is located in the sloped section of the mat slab depression (see highlighted S1-2027). The mat slab depression section plans (S1-3004) do not incorporate this type of sloped pipe penetration. Also, Sheet S1-3003 depicts all pipe penetrations on a horizontal surface only. Please provide a trestle/pin pile penetration detail located on an angle in a mat slab depression incorporating a revised waterproofing detail.			Please reference Sheets S1-2022, S1-2027, S1-3004, and S1-3006 of the Contract Plans. The trestle pile at Gridline D.4 between 4 and 5 is located in the sloped section of the mat slab depression (see highlighted S1-2022). The mat slab depression section plans (S1-3006) do not incorporate this type of sloped pipe penetration. Furthermore, the pin pile between Gridline F.7 and G, just east of 34 is located in the sloped section of the mat slab depression (see highlighted S1-2027). The mat slab depression section plans (S1-3004) do not incorporate this type of sloped pipe penetration. Also, Sheet S1-3003 depicts all pipe penetrations on a horizontal surface only. Please provide a trestle/pin pile penetration detail located on an angle in a mat slab depression incorporating a revised waterproofing detail.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0479.1</b>	<b>BGP - Trestle and Pin Pile in MAT Depression Clarification</b>	<b>Closed</b>	<b>01</b>	<b>05/28/2013</b>	<b>06/07/2013</b>	<b>06/10/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Robert Kjome</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Response to RFI T-0479 provides SKA-2676 and SKA-2677 which apply to two trestle piles in conflict with sloped portions of sump pits. BBII has identified several other pit locations which appear to have trestle piles, pin piles, or bridge piers located so that there is not 18" clear horizontal for waterproofing. Please clarify if the following slab penetration locations require the 18" clear horizontal for waterproofing. If so, please confirm that the details issued in RFI T-0479 can be used for the following locations:			Response to RFI T-0479 provides SKA-2676 and SKA-2677 which apply to two trestle piles in conflict with sloped portions of sump pits. BBII has identified several other pit locations which appear to have trestle piles, pin piles, or bridge piers located so that there is not 18" clear horizontal for waterproofing. Please clarify if the following slab penetration locations require the 18" clear horizontal for waterproofing. If so, please confirm that the details issued in RFI T-0479 can be used for the following locations:			
1.) First St. Bridge Pier #5 at pit between Gridlines 17/18 at Gridline H			1.) First St. Bridge Pier #5 at pit between Gridlines 17/18 at Gridline H			
2.) Trestle Piles #53, #54, and #55 at pit between Gridlines 22.5/23.5 and D/F			2.) Trestle Piles #53, #54, and #55 at pit between Gridlines 22.5/23.5 and D/F			
3.) Fremont St. Bridge Pier #8 at pit between Gridlines 26/27 at E			3.) Fremont St. Bridge Pier #8 at pit between Gridlines 26/27 at E			
4.) Trestle Pile #74 at pit between Gridlines 30/30.5 and D/E.			4.) Trestle Pile #74 at pit between Gridlines 30/30.5 and D/E.			
5.) Trestle Pile #80 at pit between Gridlines 32.5/33 and D/E			5.) Trestle Pile #80 at pit between Gridlines 32.5/33 and D/E			
6.) Beale St. Bridge Piers #3 and #8 at pit between Gridlines 34/35 at Gridline E			6.) Beale St. Bridge Piers #3 and #8 at pit between Gridlines 34/35 at Gridline E			
7.) Pin Pile # 6 between Gridlines 4/5			7.) Pin Pile # 6 between Gridlines 4/5			
8.) Pin Pile #14 between Gridlines 34/35 and F.7/H			8.) Pin Pile #14 between Gridlines 34/35 and F.7/H			
<b>T-0479.2</b>	<b>BGP - Trestle and Pin Pile in MAT Depression Clarification</b>	<b>Closed</b>	<b>01</b>	<b>07/18/2013</b>	<b>07/28/2013</b>	<b>07/24/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Robert Kjome</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI T-0479 & T-0479.1			Reference RFI T-0479 & T-0479.1			
Grace requires that there be a minimum 8" clear horizontal to allow for the waterproofing membrane transition.			Grace requires that there be a minimum 8" clear horizontal to allow for the waterproofing membrane transition.			
For trestle piles and pin piles located at slab depressions at the edge of the slope or on the face off the slope, please confirm that the flat mud slab can be lowered to provide 8" clear horizontal to allow waterproof membrane transition in lieu of the 18" described in RFI T-0479 and T-0479.1			For trestle piles and pin piles located at slab depressions at the edge of the slope or on the face off the slope, please confirm that the flat mud slab can be lowered to provide 8" clear horizontal to allow waterproof membrane transition in lieu of the 18" described in RFI T-0479 and T-0479.1			





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0480</b>	<b>BGP - Future Train Platform Wall Dimension Conflict</b>	<b>Closed</b>	<b>01</b>	<b>04/03/2013</b>	<b>04/17/2013</b>	<b>04/16/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>  Ref. Dwg. S1-2054, S1-2055  Drawing S1-2054, at Grid line E/13 calls out Future Train Platform Room Walls to be 1'- 2" Typ, UNO. Drawing S1-2055, at Grid line C/22 calls out Future Train Platform RM Walls to be 1'- 0" Typ. UNO. Please clarify the proper dimension of the Future Train Platform RM Walls.		<b>ANSWER:</b>  Ref. Dwg. S1-2054, S1-2055  Drawing S1-2054, at Grid line E/13 calls out Future Train Platform Room Walls to be 1'- 2" Typ, UNO. Drawing S1-2055, at Grid line C/22 calls out Future Train Platform RM Walls to be 1'- 0" Typ. UNO. Please clarify the proper dimension of the Future Train Platform RM Walls.				
<b>T-0481</b>	<b>BGP - Concourse Slab Penetration Sleeves</b>	<b>Closed</b>	<b>01</b>	<b>04/08/2013</b>	<b>04/18/2013</b>	<b>04/12/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>  Ref Dwg. A1-2842-2851  In SCCI's experience unanticipated modifications and adjustments to the plumbing system are inevitable. Because of this SCCI requests not installing vertical block out sleeves in the concourse level for plumbing prior to slab placement. SCCI shall core penetrations after the slab is placed. The slab shall be scanned for rebar prior to coring to avoid unnecessary rebar strikes. This will allow for any unforeseen modifications or adjustments and ensure there are no unnecessary or extra penetrations in the concourse slab. Please advise if this is acceptable.		<b>ANSWER:</b>  Ref Dwg. A1-2842-2851  In SCCI's experience unanticipated modifications and adjustments to the plumbing system are inevitable. Because of this SCCI requests not installing vertical block out sleeves in the concourse level for plumbing prior to slab placement. SCCI shall core penetrations after the slab is placed. The slab shall be scanned for rebar prior to coring to avoid unnecessary rebar strikes. This will allow for any unforeseen modifications or adjustments and ensure there are no unnecessary or extra penetrations in the concourse slab. Please advise if this is acceptable.				
<b>T-0482</b>	<b>BGP - Partition Wall Pier Height</b>	<b>Closed</b>	<b>01</b>	<b>04/05/2013</b>	<b>04/15/2013</b>	<b>04/17/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Drawing: S1-9050  Please reference attached sheets S1-9050, A1-9216 and A1-9217 regarding partition wall piers. Detail 9 on S1-9050 shows an h max of 24'8" for wall piers. Detail A on sheet A1-9216, and detail B on sheet A1-9217 appear to be showing piers at a height of 27'2" and 28'11" respectfully. SCCI is requesting clarification with pier height regarding reinforcement as well as opening width allowed.		<b>ANSWER:</b>  Reference Drawing: S1-9050  Please reference attached sheets S1-9050, A1-9216 and A1-9217 regarding partition wall piers. Detail 9 on S1-9050 shows an h max of 24'8" for wall piers. Detail A on sheet A1-9216, and detail B on sheet A1-9217 appear to be showing piers at a height of 27'2" and 28'11" respectfully. SCCI is requesting clarification with pier height regarding reinforcement as well as opening width allowed.				



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-0483</b>	<b>BGP - Request for reinstatement of a smaller high congestion mock-up.</b>	<b>Closed</b>	<b>01</b>	<b>04/05/2013</b>	<b>04/15/2013</b>	<b>04/17/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: S1-3202, S1-2204, S1-3201, S1-3208			Ref: S1-3202, S1-2204, S1-3201, S1-3208			
Via CCO #0035 the TJPA unilaterally deleted Bid Item #14 the high congestion mock-up and disposal. WOJV maintains that the inclusion of a mock-up for areas of high congestion (Exhibit-A) is not only good construction practice but will mitigate if not eviscerate the unquantifiable liability. WOJV recommends, at a minimum, reinstating a high congestion mock-up configured as follows:			Via CCO #0035 the TJPA unilaterally deleted Bid Item #14 the high congestion mock-up and disposal. WOJV maintains that the inclusion of a mock-up for areas of high congestion (Exhibit-A) is not only good construction practice but will mitigate if not eviscerate the unquantifiable liability. WOJV recommends, at a minimum, reinstating a high congestion mock-up configured as follows:			
1. The area to mock-up is indicated on marked up sheet S1-3202 (Exhibit-B).			1. The area to mock-up is indicated on marked up sheet S1-3202 (Exhibit-B).			
2. The mock-up is representative of the location marked up on sheet S1-2204 (Exhibit-C) and configured as indicated on marked up sheet S1-3201 (Exhibit-D).			2. The mock-up is representative of the location marked up on sheet S1-2204 (Exhibit-C) and configured as indicated on marked up sheet S1-3201 (Exhibit-D).			
3. The mock-up is dimensioned as indicated on marked up sheet S1-3208 (Exhibit-E).			3. The mock-up is dimensioned as indicated on marked up sheet S1-3208 (Exhibit-E).			
Please issue drawings for a smaller high congestion mock-up that the TJPA deems appropriate, if not indicated on the attached sheets.			Please issue drawings for a smaller high congestion mock-up that the TJPA deems appropriate, if not indicated on the attached sheets.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0484</b>	<b>BGP - Water Welding Test</b>	<b>Closed</b>	<b>01</b>	<b>04/05/2013</b>	<b>04/15/2013</b>	<b>04/18/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Specification:05 50 10- 2.5.C.2  Per the discussions held at the Metal Fabrications Preparatory DFOW meeting, SCCI is requesting a variance from Spec Section 05 50 10 - 2.5.C.2. This Spec. is feasible in a shop environment prior to galvanization and an effective means to dry and remove water upon completion of testing. These sleeves will be continuously welded in the field both before and after the horizontal waterproofing is installed (depending on the type of sleeve), therefore making it very difficult to seal and handle the water upon completion of the test. Discussions were held regarding leaving the water between the sleeve and pile and evaporating over time. SCCI sees this as a concern due to the backside of the weld and the heat-affected zone will not be galvanized and will potentially become a point of corrosion. SCCI requests 100% visual inspection on both the root and cover passes in lieu of filling the sleeve gap with water. Is this request and variance acceptable?						<b>ANSWER:</b> Reference Specification:05 50 10- 2.5.C.2  Per the discussions held at the Metal Fabrications Preparatory DFOW meeting, SCCI is requesting a variance from Spec Section 05 50 10 - 2.5.C.2. This Spec. is feasible in a shop environment prior to galvanization and an effective means to dry and remove water upon completion of testing. These sleeves will be continuously welded in the field both before and after the horizontal waterproofing is installed (depending on the type of sleeve), therefore making it very difficult to seal and handle the water upon completion of the test. Discussions were held regarding leaving the water between the sleeve and pile and evaporating over time. SCCI sees this as a concern due to the backside of the weld and the heat-affected zone will not be galvanized and will potentially become a point of corrosion. SCCI requests 100% visual inspection on both the root and cover passes in lieu of filling the sleeve gap with water. Is this request and variance acceptable?
<b>T-0485</b>	<b>BGP - SCCI Issued Drawings</b>	<b>Closed</b>	<b>01</b>	<b>04/08/2013</b>	<b>04/18/2013</b>	<b>05/02/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference spreadsheet: See attached.  Please reference attached spreadsheet of SCCI missing contract drawings. Based on drawing index sheets G-0000 - G-0006 issued F.O. 15 there are 33 sheets that have a more current revision than what we have been issue. Please issue the updated drawings referenced.						<b>ANSWER:</b> Reference spreadsheet: See attached.  Please reference attached spreadsheet of SCCI missing contract drawings. Based on drawing index sheets G-0000 - G-0006 issued F.O. 15 there are 33 sheets that have a more current revision than what we have been issue. Please issue the updated drawings referenced.
<b>T-0486</b>	<b>BGP - Extended Time for Concrete Delivery</b>	<b>Closed</b>	<b>01</b>	<b>04/08/2013</b>	<b>04/18/2013</b>	<b>04/16/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Specification: 03 30 20						<b>ANSWER:</b> Reference Specification: 03 30 20





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0486.2</b>	<b>BGP - Extended Time for Concrete Delivery - Mat Slab</b>	<b>Closed</b>	<b>01</b>	<b>05/28/2013</b>	<b>06/07/2013</b>	<b>06/03/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference RFI: T-0486  Pursuant TJPA's response to RFI T-0486 please reference attached letter from SCCI's concrete supplier Cemex.  Cemex has performed the set time test to evaluate the time at which the onset of hydration occurs for mix #1557204 (Mat Slab Mix).  For the mix referenced herein, is it acceptable to extend the concrete delivery times to 2 hours?		<b>ANSWER:</b>  Reference RFI: T-0486  Pursuant TJPA's response to RFI T-0486 please reference attached letter from SCCI's concrete supplier Cemex.  Cemex has performed the set time test to evaluate the time at which the onset of hydration occurs for mix #1557204 (Mat Slab Mix).  For the mix referenced herein, is it acceptable to extend the concrete delivery times to 2 hours?				
<b>T-0487</b>	<b>BGP - Structural Pier Reinforcement Detail</b>	<b>Closed</b>	<b>01</b>	<b>04/08/2013</b>	<b>04/18/2013</b>	<b>04/22/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>  Ref: A1-9215, 9/S1-9050  Please confirm that the vertical rebar size and spacing of #7@ 8" OC EF (as shown on Detail 9 of S1-9050) applies to the structural pier between GL 4 and 5 which is dimensioned as 2'-0" x 2'-0"(A1-9215).		<b>ANSWER:</b>  Ref: A1-9215, 9/S1-9050  Please confirm that the vertical rebar size and spacing of #7@ 8" OC EF (as shown on Detail 9 of S1-9050) applies to the structural pier between GL 4 and 5 which is dimensioned as 2'-0" x 2'-0"(A1-9215).				
<b>T-0488</b>	<b>BGP - Handling HVFA Test Cylinders- Mat Slab</b>	<b>Closed</b>	<b>01</b>	<b>04/08/2013</b>	<b>04/18/2013</b>	<b>04/17/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>  Ref: Specification Section 03 30 20 1.7 F 3 j 2 ASTM C 31  ASTM C 31 Identifies that concrete cylinders should not be transported until at least 8 hours after final set. Per ASTM C 31, Allowable field curing is 48 hours maximum. Typically test cylinders are transported within 24 to 48 hours after casting. Some of the mix designs approved for this project include High volume of Flyash (HVFA) and high dose of Shrinkage Reducing Admixture (SRA). This combination provides a concrete mix with retarded set and slow strength gain. In the interest of providing reliable test results, SCCI and CEMEX requests that transporting of		<b>ANSWER:</b>  Ref: Specification Section 03 30 20 1.7 F 3 j 2 ASTM C 31  ASTM C 31 Identifies that concrete cylinders should not be transported until at least 8 hours after final set. Per ASTM C 31, Allowable field curing is 48 hours maximum. Typically test cylinders are transported within 24 to 48 hours after casting. Some of the mix designs approved for this project include High volume of Flyash (HVFA) and high dose of Shrinkage Reducing Admixture (SRA). This combination provides a concrete mix with retarded set and slow strength gain. In the interest of providing reliable test results,				





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0490</b>	<b>BSE - Multiple Micropile Relocation (Trestle Overhead Obstruction)</b>	<b>Closed</b>	<b>01</b>	<b>04/09/2013</b>	<b>04/19/2013</b>	<b>04/16/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b> Ref: TG0300-622.4  Multiple micropiles underneath the trestle cannot be installed as laid out due to an overhead strut support obstruction. BBII suggests relocating these micropiles south to provide 4' of clearance from the overhead strut support to each micropile. The proposed micropile locations will be within the geothermal area; however, they do not appear to impact geothermal piping. See attached sketch.  Please confirm this is acceptable.						<b>ANSWER:</b> Ref: TG0300-622.4  Multiple micropiles underneath the trestle cannot be installed as laid out due to an overhead strut support obstruction. BBII suggests relocating these micropiles south to provide 4' of clearance from the overhead strut support to each micropile. The proposed micropile locations will be within the geothermal area; however, they do not appear to impact geothermal piping. See attached sketch.  Please confirm this is acceptable.
<b>T-0492</b>	<b>BGP - Backfill of Geothermal Pipe</b>	<b>Closed</b>	<b>CR</b>	<b>04/11/2013</b>	<b>04/21/2013</b>	<b>04/23/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b> Per discussions following the Turner BSE Progress Meeting with the geothermal designer, it is acceptable to backfill and compact the continuous loop after having been installed in the trench. This backfill is contingent upon the ends of the loop being left exposed for the loop welds to the manifold. Backfill over these welded joints and manifold will not be completed until the 100 psi hydro test is complete.  Please confirm this is acceptable.						<b>ANSWER:</b> Per discussions following the Turner BSE Progress Meeting with the geothermal designer, it is acceptable to backfill and compact the continuous loop after having been installed in the trench. This backfill is contingent upon the ends of the loop being left exposed for the loop welds to the manifold. Backfill over these welded joints and manifold will not be completed until the 100 psi hydro test is complete.  Please confirm this is acceptable.
<b>T-0493</b>	<b>BGP - Geothermal Loop Spacing Tolerances</b>	<b>Closed</b>	<b>01</b>	<b>04/11/2013</b>	<b>04/21/2013</b>	<b>04/16/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b> Ref: RFI T-0473  Per the Engineer response to a WOJV RFI, the geothermal loop spacing cannot exceed 4'. Per discussions after the progress meeting today (4/10/ 13), the 5th and 6th loops in field 1 are acceptable with a spacing of 20". This exception is for this location only and all further exceptions are to be submitted under a seperate RFI at the time of the layout.						<b>ANSWER:</b> Ref: RFI T-0473  Per the Engineer response to a WOJV RFI, the geothermal loop spacing cannot exceed 4'. Per discussions after the progress meeting today (4/10/ 13), the 5th and 6th loops in field 1 are acceptable with a spacing of 20". This exception is for this location only and all further exceptions are to be submitted under a seperate RFI at the time of the





<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>Please confirm that this 20" spacing for Field 1 loops 5 &amp; 6 is acceptable at 20".</p>					<p>layout. Please confirm that this 20" spacing for Field 1 loops 5 &amp; 6 is acceptable at 20".</p>
<b>T-0494</b>	<b>BGP - Formwork- Form Release Compatability Certification</b>	<b>Closed</b>	<b>01</b>	<b>04/11/2013</b>	<b>04/21/2013</b>	<b>04/16/2013</b>
	<p><b>From:</b> Webcor Construction LP                      Lynn Kowallis</p> <p><b>REQUEST:</b></p> <p>Ref: A1-9601 through A1-9606 Specifications Section 03 10 00.1.3.B.6</p> <p>Please reference specifications section 03 10 00.1.3.B.6. Section states contractor shall submit for record a written statement certifying that form release agent used is compatable with susequent architectural finish materials applied to concrete surfaces. Drawings A1-9601 through A1-9606, is the room finish schedule, however the TG06.0 drawing package does not include the above mentioned finish schedule drawings. Without knowledge of the subsequent architectural finish, Shimmick Construction cannot comply with the above mentioned specification.</p> <p>Please provide a room finish schedule so that Shimmnick Construction can comply with the above mentioned specification.</p>					<p><b>ANSWER:</b></p> <p>Ref: A1-9601 through A1-9606 Specifications Section 03 10 00.1.3.B.6</p> <p>Please reference specifications section 03 10 00.1.3.B.6. Section states contractor shall submit for record a written statement certifying that form release agent used is compatable with susequent architectural finish materials applied to concrete surfaces. Drawings A1-9601 through A1-9606, is the room finish schedule, however the TG06.0 drawing package does not include the above mentioned finish schedule drawings. Without knowledge of the subsequent architectural finish, Shimmick Construction cannot comply with the above mentioned specification.</p> <p>Please provide a room finish schedule so that Shimmnick Construction can comply with the above mentioned specification.</p>
<b>T-0494.1</b>	<b>BGP - Architectural Finish Schedule</b>	<b>Closed</b>	<b>01</b>	<b>06/03/2013</b>	<b>06/13/2013</b>	<b>06/10/2013</b>
	<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Per attached RFI response T-0494, please provide SCCI with an architectural fin ish schedule.</p>					<p><b>ANSWER:</b></p> <p>Per attached RFI response T-0494, please provide SCCI with an architectural fin ish schedule.</p>





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0495</b>	<b>BGP - Foundation Wall Concrete Inserts</b>	<b>Closed</b>	<b>CR</b>	<b>04/12/2013</b>	<b>04/22/2013</b>	<b>04/24/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: A1-2812, A1-2821 A1-2842, A1-2843, A1-6231			Ref: A1-2812, A1-2821 A1-2842, A1-2843, A1-6231			
Please reference the attached drawings regarding foundation wall concrete inserts. SCCI is requesting details clarifying the locations and scope of the horizontal concrete inserts on the mat slab level foundation walls and vertical concrete inserts on the lower concourse level foundation walls. The following issues have been discovered in the drawings:			Please reference the attached drawings regarding foundation wall concrete inserts. SCCI is requesting details clarifying the locations and scope of the horizontal concrete inserts on the mat slab level foundation walls and vertical concrete inserts on the lower concourse level foundation walls. The following issues have been discovered in the drawings:			
1. A1-2843 has specified two contradicting lengths for the continuous vertical wall inserts as shown in the clouded sections.			1. A1-2843 has specified two contradicting lengths for the continuous vertical wall inserts as shown in the clouded sections.			
2. A1-2812 and A1-2842 appear to indicate inserts along the West wall however there is no information declaring lengths and scope.			2. A1-2812 and A1-2842 appear to indicate inserts along the West wall however there is no information declaring lengths and scope.			
3. A1-2821 references detail 3 / A1-6231 which is not a detail that is shown on A1-6231.			3. A1-2821 references detail 3 / A1-6231 which is not a detail that is shown on A1-6231.			



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<b>T-0496</b>	<b>BGP - Deneef Swellseal at Micropile Boots</b>	<b>Closed</b>	<b>01</b>	<b>04/11/2013</b>	<b>04/25/2013</b>	<b>04/26/2013</b>
<b>From:</b> Webcor Construction LP                      Ian Corcorran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref Dwg. 2/A1-8711		Ref Dwg. 2/A1-8711				
Please reference Detail 2 of A1-8711 of the Contract Drawings and the attached letter from Deneef/Grace. Detail 2 of A1-8711 calls for a 6" diameter, 18 ga. galvanized steel boot to be adhered with trowelable grade adhesive and filled with urethane sealant. Submittal #TG0600-024 approved the use of Deneef Swellseal WA which is the product called out in Spec Section 07 12 10.		Please reference Detail 2 of A1-8711 of the Contract Drawings and the attached letter from Deneef/Grace. Detail 2 of A1-8711 calls for a 6" diameter, 18 ga. galvanized steel boot to be adhered with trowelable grade adhesive and filled with urethane sealant. Submittal #TG0600-024 approved the use of Deneef Swellseal WA which is the product called out in Spec Section 07 12 10.				
The attached Deneef/Grace technical letter dated 04/05/13, states that filling the entire boot with Deneef Swellseal is excessive and state that filling the entire boot with Swellseal WA is more than necessary and affect the curing capability.		The attached Deneef/Grace technical letter dated 04/05/13, states that filling the entire boot with Deneef Swellseal is excessive and state that filling the entire boot with Swellseal WA is more than necessary and affect the curing capability.				
Deneef/Grace suggests that the material be installed 2-3" deep and topped with a non-shrink grout such as "Rapid Set CT Construction Grout" or "Rapid Set Cement All" to contain it in the boot. The manufacturer states that the waterproofing ability of the material in this configuration would not be compromised. Please review and advise.		Deneef/Grace suggests that the material be installed 2-3" deep and topped with a non-shrink grout such as "Rapid Set CT Construction Grout" or "Rapid Set Cement All" to contain it in the boot. The manufacturer states that the waterproofing ability of the material in this configuration would not be compromised. Please review and advise.				
<b>T-0497</b>	<b>BGP - C29 Column Detail Clarification</b>	<b>Closed</b>	<b>01</b>	<b>04/17/2013</b>	<b>04/27/2013</b>	<b>04/22/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Specification: 03 20 00 Reference Drawing: S1-3300, A1-2850, S1-2030, S1-3303		Reference Specification: 03 20 00 Reference Drawing: S1-3300, A1-2850, S1-2030, S1-3303				
Contract drawing S1-3300 refers to detail 1/S 1-3303 for the rebar elevation detail of column C29. Detail I/SI-3303 appears to be for columns that pass through the ramp and based on drawing AI-2853 column C29 does not pass through the ramp.		Contract drawing S1-3300 refers to detail 1/S 1-3303 for the rebar elevation detail of column C29. Detail I/SI-3303 appears to be for columns that pass through the ramp and based on drawing AI-2853 column C29 does not pass through the ramp.				
Please confirm if Detail 1/SI-3303 is the correct elevation detail for column C29.		Please confirm if Detail 1/SI-3303 is the correct elevation detail for column C29.				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0498</b>	<b>BGP - Waterproofing Mock Up</b>	<b>Closed</b>	<b>01</b>	<b>04/18/2013</b>	<b>04/18/2013</b>	<b>04/25/2013</b>
<b>From:</b> Webcor Construction LP                      Kody Cooper						
<b>REQUEST:</b>  Reference Specification: 07 12 10 - 1.6.C.2  The waterproofing manufacturer's field representative/installer are to construct a 10'x10' on site mock up of the full waterproofing assembly. Upon completion of the mock up (excluding any major waterproofing deficiencies), SCCI intends to utilize it as part of the permanent structure. Is this acceptable?					<b>ANSWER:</b>  Reference Specification: 07 12 10 - 1.6.C.2  The waterproofing manufacturer's field representative/installer are to construct a 10'x10' on site mock up of the full waterproofing assembly. Upon completion of the mock up (excluding any major waterproofing deficiencies), SCCI intends to utilize it as part of the permanent structure. Is this acceptable?	
<b>T-0499</b>	<b>BGP - Geothermal Manifold Location for Fields1 &amp; 2</b>	<b>Closed</b>	<b>01</b>	<b>04/18/2013</b>	<b>04/28/2013</b>	<b>04/25/2013</b>
<b>From:</b> Webcor Construction LP                      Kody Cooper						
<b>REQUEST:</b>  Reference Drawing: SK-3  Per the contract drawing, the manifold is to be located at an elevation no greater than 14' below finish grade (street) elevation. Per conversations in the preparatory DFOV meeting and other coordination meetings, the Engineer planned to have the manifold in a specific location. Attached is an elevation drawing for Field 1 & 2 Manifolds. Please confirm that the attached elevation details work with the designer's intent for the manifold locations for Field 1 & 2.					<b>ANSWER:</b>  Reference Drawing: SK-3  Per the contract drawing, the manifold is to be located at an elevation no greater than 14' below finish grade (street) elevation. Per conversations in the preparatory DFOV meeting and other coordination meetings, the Engineer planned to have the manifold in a specific location. Attached is an elevation drawing for Field 1 & 2 Manifolds. Please confirm that the attached elevation details work with the designer's intent for the manifold locations for Field 1 & 2.	
<b>T-0500</b>	<b>BSE - Micropile Blockouts in Mud Slab</b>	<b>Closed</b>	<b>01</b>	<b>04/18/2013</b>	<b>04/28/2013</b>	<b>05/01/2013</b>
<b>From:</b> Webcor Construction LP                      Kody Cooper						
<b>REQUEST:</b>  Reference Specification: 03 30 00  In mud slab pour 1, micropiles W154, W154R1, W127, W236, and W236R1 are all blocked out. BBII would like the option to pour back the blockouts with 4,000psi neat grout (mix approved for installation of micropiles) or the approved 2,500psi concrete.  Please confirm that either option is acceptable.					<b>ANSWER:</b>  Reference Specification: 03 30 00  In mud slab pour 1, micropiles W154, W154R1, W127, W236, and W236R1 are all blocked out. BBII would like the option to pour back the blockouts with 4,000psi neat grout (mix approved for installation of micropiles) or the approved 2,500psi concrete.  Please confirm that either option is acceptable.	



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T-0501	BGP - Slide Bearing Connection details	Closed	01	04/18/2013	04/28/2013	04/30/2013
From: Webcor Construction LP                      Kody Cooper						
REQUEST:		ANSWER:				
Reference Drawings: S1-3204 and S1-3205		Reference Drawings: S1-3204 and S1-3205				
The two drawings detail the slide bearing assemblies at the east wall and vehicle/bike ramp. Detail 9-A on S1-3204 does not detail how the 10 gauge carbon steel plate is connected to the bottom support. Similarly, Details 2,3,6 and 7 on S1-3205 do not detail how the assemblies are connected to the embedded plates. Please provide details on connections between slide bearing assemblies and support/embedded plates.		The two drawings detail the slide bearing assemblies at the east wall and vehicle/bike ramp. Detail 9-A on S1-3204 does not detail how the 10 gauge carbon steel plate is connected to the bottom support. Similarly, Details 2,3,6 and 7 on S1-3205 do not detail how the assemblies are connected to the embedded plates. Please provide details on connections between slide bearing assemblies and support/embedded plates.				
T-0502	BGP - Slide Bearing Weld Details	Closed	01	04/18/2013	04/28/2013	04/29/2013
From: Webcor Construction LP                      Kody Cooper						
REQUEST:		ANSWER:				
Reference Drawing: S1-3205, S1-3210 and S1-3211		Reference Drawing: S1-3205, S1-3210 and S1-3211				
The details call for various pieces of the slide bearing assemblies to be continuously and tack welded to plates. See clouded callouts on attached drawings. No welding details are provided with the callouts. Please provide details for continuous welds and spacing for tack welds.		The details call for various pieces of the slide bearing assemblies to be continuously and tack welded to plates. See clouded callouts on attached drawings. No welding details are provided with the callouts. Please provide details for continuous welds and spacing for tack welds.				
T-0503	BGP - Geothermal Pipe Loop Bends	Closed	01	04/18/2013	04/18/2013	04/23/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:		ANSWER:				
Per the geothermal pipe manufacturer's (Performance Pipe) recommendations, the geothermal pipe should not be bent in a radius smaller than 25 times the pipe diameter. For the geothermal pipe loops, this equates to a bend radius of 41.5". However, the geothermal design drawings depict the loops to be 60" on center that would leave a large overlap (in theory) of almost 24"/2'. To achieve a 41.5" radius, the trench spacing will have to be increased to 83" between the supply and return trench. Please note, that the pipe manufacturer discourage "bulbing" the end of the loop and recommended resolving the issue away from bending or "bulbing" the end of the		Per the geothermal pipe manufacturer's (Performance Pipe) recommendations, the geothermal pipe should not be bent in a radius smaller than 25 times the pipe diameter. For the geothermal pipe loops, this equates to a bend radius of 41.5". However, the geothermal design drawings depict the loops to be 60" on center that would leave a large overlap (in theory) of almost 24"/2'. To achieve a 41.5" radius, the trench spacing will have to be increased to 83" between the supply and return trench. Please note, that the pipe manufacturer discourage "bulbing" the end of the loop and recommended resolving the issue away from				



bending or "bulbing" the end of the pipe loop.

S3H Inc. is proposing to overlap half of a loop onto another such that the spacing between pipes remains at a 4' minimum (per RFI T-0493). This would create a 8' minimum distance between the supply side of a loop and the return side of a loop. In doing so, a portion of the two overlapping loops would be crossing. Is this acceptable? Please find attached drawing #1 as a reference of the proposed layout. Please note that this proposed method would change the reverse return self balancing configuration of piping. This proposed method also has the possibility of being impacted by various micropile conflicts.

S3H Inc. is also proposing as a fix to field one to install 2 fused - 90 degree elbows at the end of each loop in a U-shape configuration using the current, as installed dimensions between the loops. Please find attached Drawing #2 depicting the 90 degree elbows on the loops This would eliminate the required 83" bend diameter. This is least impact proposal to rectify the already installed field 1, but would be an additional cost.

Please advise as to how to proceed with Field 1 as well as the remaining 14 Fields.

**T-0504** **BGP - Radius Foundation Walls - R=637.63'**

**From:** Webcor Construction LP                      Lynn Kowallis

**REQUEST:**

Ref: Submittal Package T0600-030

SCCI's plan is to construct the R=637.63' foundation walls in 16' chords. Layout of the construction joints shall be per approved as noted CJ layout submittal. R=637.63' foundation wall runs along the Southwest portion of the project, from GL 3 thru GL 16, or SCCI's wall pours W160 thru W174A. See attached sketch of the wall detail for clarification.

Is this acceptable?

<b>Closed</b>	<b>01</b>	<b>04/19/2013</b>	<b>04/29/2013</b>	<b>05/02/2013</b>
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**ANSWER:**

Ref: Submittal Package T0600-030

SCCI's plan is to construct the R=637.63' foundation walls in 16' chords. Layout of the construction joints shall be per approved as noted CJ layout submittal. R=637.63' foundation wall runs along the Southwest portion of the project, from GL 3 thru GL 16, or SCCI's wall pours W160 thru W174A. See attached sketch of the wall detail for clarification.

Is this acceptable?



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0504.1</b>	<b>BGP - Radius Foundation Wall Formwork</b>	<b>Closed</b>	<b>CR</b>	<b>11/19/2013</b>	<b>11/29/2013</b>	<b>11/25/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Please reference RFI T-0504.  SCCI plans to construct the south foundation walls from GL2.75 to GL 12.08 in 8' chords. See attached sketch for clarification. 8' chording of the walls will keep the wall faces within the construction tolerances.  Is this acceptable?						<b>ANSWER:</b>  Please reference RFI T-0504.  SCCI plans to construct the south foundation walls from GL2.75 to GL 12.08 in 8' chords. See attached sketch for clarification. 8' chording of the walls will keep the wall faces within the construction tolerances.  Is this acceptable?
<b>T-0505</b>	<b>BGP - Protection Board on Horizontal Surface of Waterproofing</b>	<b>Closed</b>	<b>01</b>	<b>04/19/2013</b>	<b>05/03/2013</b>	<b>04/29/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>  Ref. Dwg. A1-8710, A1 -8711, S1-3003  Please confirm that there is no protection board installed on top of the waterproofing membrane to receive protection slab. Drawing S1-3003 shows protection board, while A1-8710 & A1-8711 does not.						<b>ANSWER:</b>  Ref. Dwg. A1-8710, A1 -8711, S1-3003  Please confirm that there is no protection board installed on top of the waterproofing membrane to receive protection slab. Drawing S1-3003 shows protection board, while A1-8710 & A1-8711 does not.
<b>T-0506</b>	<b>BGP - Continuous Horizontal Concrete Inserts</b>	<b>Closed</b>	<b>01</b>	<b>04/22/2013</b>	<b>05/02/2013</b>	<b>05/07/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Drawing: A1-6231  Please reference the attached sheets regarding continuous concrete inserts. On the enlarged detail C of A1-6231 SCCI is proposing the layout of the horizontal concrete inserts. Raising the bottom insert 1- 1/2" and lowering the top insert 1" will provide a greater clearance between the inserts and the construction joint. Achieving a greater clearance from the construction joint will reduce the risk of rock pockets or voids. Please confirm these dimensions as acceptable.						<b>ANSWER:</b>  Reference Drawing: A1-6231  Please reference the attached sheets regarding continuous concrete inserts. On the enlarged detail C of A1-6231 SCCI is proposing the layout of the horizontal concrete inserts. Raising the bottom insert 1- 1/2" and lowering the top insert 1" will provide a greater clearance between the inserts and the construction joint. Achieving a greater clearance from the construction joint will reduce the risk of rock pockets or voids. Please confirm these dimensions as acceptable.



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-0507</b>	<b>BGP - Continuous Concrete dobie-mat slab</b>	<b>Closed</b>	<b>01</b>	<b>04/22/2013</b>	<b>05/02/2013</b>	<b>05/06/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b> Please see attached sheets regarding mat slab bulkhead forms. SCCI is proposing the use of a continuous concrete dobie as part of the bulkhead design along the vertical construction joint. The continuous dobie will be installed with the reinforcement mats and will act as a cast-in portion of the formwork. The dobie will become a permanent member and will meet all specifications that the mat slab concrete mix design requires. Please confirm approval of the use of the continuous dobie.						<b>ANSWER:</b> Please see attached sheets regarding mat slab bulkhead forms. SCCI is proposing the use of a continuous concrete dobie as part of the bulkhead design along the vertical construction joint. The continuous dobie will be installed with the reinforcement mats and will act as a cast-in portion of the formwork. The dobie will become a permanent member and will meet all specifications that the mat slab concrete mix design requires. Please confirm approval of the use of the continuous dobie.
<b>T-0508</b>	<b>BGP - Drainage Composite Joint Orientation</b>	<b>Closed</b>	<b>01</b>	<b>04/23/2013</b>	<b>05/03/2013</b>	<b>04/25/2013</b>
<b>From:</b> Webcor Construction LP Kody Cooper						
<b>REQUEST:</b> Reference Specification: 07 12 10-3.2.F  This spec section states "Install drainage composite either vertically or horizontally and lap sheets 1 inch in the direction of water flow." The manufacturer's instructions state "the drainage side laps must be tightly butt joined together so there are no gaps or voids between them." SCCI suggests butt joining the drainage composites per the manufacturer's instructions. Is this acceptable?						<b>ANSWER:</b> Reference Specification: 07 12 10-3.2.F  This spec section states "Install drainage composite either vertically or horizontally and lap sheets 1 inch in the direction of water flow." The manufacturer's instructions state "the drainage side laps must be tightly butt joined together so there are no gaps or voids between them." SCCI suggests butt joining the drainage composites per the manufacturer's instructions. Is this acceptable?
<b>T-0510</b>	<b>BGP - Internal Bracing Pin Pile #8 in conflict with Moment Beam BMATV</b>	<b>Closed</b>	<b>01</b>	<b>04/23/2013</b>	<b>05/03/2013</b>	<b>04/30/2013</b>
<b>From:</b> Webcor Construction LP Lynn Kowallis						
<b>REQUEST:</b> Please reference attached marked up sheet S1-2202.  The location of internal bracing pin pile #8 conflicts with moment beam BMATV. General Note GR-9 on sheet S-0005 precludes blocking out moment frames. Upon submitting for the internal bracing system the TG03 BSE subcontractor was not aware of the location of beam BMATV to coordinate around. WOJV is requesting a variance from note GR-9 and is requesting to block out beam BMATV around pin pile #8.						<b>ANSWER:</b> Please reference attached marked up sheet S1-2202.  The location of internal bracing pin pile #8 conflicts with moment beam BMATV. General Note GR-9 on sheet S-0005 precludes blocking out moment frames. Upon submitting for the internal bracing system the TG03 BSE subcontractor was not aware of the location of beam BMATV to coordinate around. WOJV is requesting a variance from note GR-9 and is requesting to block out beam BMATV around pin pile #8.





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	Please advise.					
			Please advise.			
<b>T-0510.1</b>	<b>BGP - Internal Bracing Pin Pile #8 in conflict with Moment Beam BMATV</b>	<b>Closed</b>	<b>01</b>	<b>05/02/2013</b>	<b>05/14/2013</b>	<b>05/15/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						
Please reference attached marked up sheet S1-2202.						
The location of internal bracing pin pile #8 conflicts with moment beam BMATV. General Note GR-9 on sheet S-0005 precludes blocking out moment frames. Upon submitting for the internal bracing system the TG03 BSE subcontractor was not aware of the location of beam BMATV to coordinate around. On 4/23/13 WOJV submitted RFI #T-0510 requesting a variance from note GR-9 and is requesting to block out beam BMATV around pin pile #8. During the 4/25/13 "WOJV SE Assist Meeting," when the issue was brought up, a PMPC employee suggested prematurely removing strut STA09 because it has diminished load. On 4/30/13 WOJV received RFI response #T-0510 stating pin pile #8 was going to be removed hence WOJV should close the RFI #T-0510. In addition to strut STA09 pin pile #8 supports strut #STB09 which is carrying a load, not that the internal bracing EOR would allow the premature removal of two strut levels. WOJV again requests a variance from note GR-9 and is requesting to block out beam BMATV around pin pile #8.						
Please advise.						
<b>ANSWER:</b>						
Please reference attached marked up sheet S1-2202.						
The location of internal bracing pin pile #8 conflicts with moment beam BMATV. General Note GR-9 on sheet S-0005 precludes blocking out moment frames. Upon submitting for the internal bracing system the TG03 BSE subcontractor was not aware of the location of beam BMATV to coordinate around. On 4/23/13 WOJV submitted RFI #T-0510 requesting a variance from note GR-9 and is requesting to block out beam BMATV around pin pile #8. During the 4/25/13 "WOJV SE Assist Meeting," when the issue was brought up, a PMPC employee suggested prematurely removing strut STA09 because it has diminished load. On 4/30/13 WOJV received RFI response #T-0510 stating pin pile #8 was going to be removed hence WOJV should close the RFI #T-0510. In addition to strut STA09 pin pile #8 supports strut #STB09 which is carrying a load, not that the internal bracing EOR would allow the premature removal of two strut levels. WOJV again requests a variance from note GR-9 and is requesting to block out beam BMATV around pin pile #8.						
Please advise.						





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0511</b>	<b>BGP - Deneef Swellseal at Electrical Grounding System Boots</b>	<b>Closed</b>	<b>01</b>	<b>04/23/2013</b>	<b>05/07/2013</b>	<b>05/09/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference SCCI RFI #130, Detail 5/A1-8710, and the attached letter. Detail 5/A1-8710 calls for 2" diameter, 18 ga galvanized steel boot to be adhered with trowelable grade adhesive and filled with urethane sealant. Submittal #TG0600-024 approved the use of Deneef Swellseal WA which is the product called out in Specifications 07 12 10.			Please reference SCCI RFI #130, Detail 5/A1-8710, and the attached letter. Detail 5/A1-8710 calls for 2" diameter, 18 ga galvanized steel boot to be adhered with trowelable grade adhesive and filled with urethane sealant. Submittal #TG0600-024 approved the use of Deneef Swellseal WA which is the product called out in Specifications 07 12 10.			
The attached letter Deneef/Grace technical letter dated 04/05/13 states that filling the entire boot with Deneef Swellseal is excessive and state that filling the entire boot with Swellseal WA is more than necessary and may affect the curing capability.			The attached letter Deneef/Grace technical letter dated 04/05/13 states that filling the entire boot with Deneef Swellseal is excessive and state that filling the entire boot with Swellseal WA is more than necessary and may affect the curing capability.			
Deneef/Grace suggests that the material be installed 2-3" deep and topped with a non-shrink grout such as "Rapid Set CT Construction Grout" or "Rapid Set Cement All" to contain it in the boot. The Manufacturer states that the waterproofing ability of the material in this configuration would not be compromised.			Deneef/Grace suggests that the material be installed 2-3" deep and topped with a non-shrink grout such as "Rapid Set CT Construction Grout" or "Rapid Set Cement All" to contain it in the boot. The Manufacturer states that the waterproofing ability of the material in this configuration would not be compromised.			
Please review and advise.			Please review and advise.			
<b>T-0512</b>	<b>BGP - Additional Fasteners for Protection Board Installation</b>	<b>Closed</b>	<b>CR</b>	<b>04/23/2013</b>	<b>05/07/2013</b>	<b>04/26/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref. Spec. 07 12 10-3.2.D			Ref. Spec. 07 12 10-3.2.D			
Please reference Spec Section 07 12 10- 3.2.D. Spec Section 07 12 10 - 3.2.D states the following: "Secure 1/4" protection board to flanges of soldier piles with powder driven fasteners and washers spaced 12 inches o.c. Butt vertical joints. Maximum joint width: 1/4" ..."			Please reference Spec Section 07 12 10- 3.2.D. Spec Section 07 12 10 - 3.2.D states the following: "Secure 1/4" protection board to flanges of soldier piles with powder driven fasteners and washers spaced 12 inches o.c. Butt vertical joints. Maximum joint width: 1/4" ..."			
The manufacturer of membrane waterproofing system (Laurenco) has indicated that due to "out of plane" piles, and relaxation of CDSM substrate requirement, they are requiring intermediate fasteners to hold the 1/4" protection board tight to the CDSM wall. Please review and advise.			The manufacturer of membrane waterproofing system (Laurenco) has indicated that due to "out of plane" piles, and relaxation of CDSM substrate requirement, they are requiring intermediate fasteners to hold the 1/4" protection board tight to the CDSM wall. Please review and advise.			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0513</b>	<b>BSE - Steel plate at CDSM piles 738-739</b>	<b>Closed</b>	<b>01</b>	<b>04/24/2013</b>	<b>05/04/2013</b>	<b>05/08/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: Specification Section 31 56 13			Ref: Specification Section 31 56 13			
During leak grouting at level 5 excavation, a section of the CDSM wall panel between soldier piles 738-739 became dislodged, resulting in a high volume leak. In an effort to stabilize the damaged CDSM panel and stop the leak, BBII installed a steel road plate between soldier piles 738-739 and injected grout behind it.			During leak grouting at level 5 excavation, a section of the CDSM wall panel between soldier piles 738-739 became dislodged, resulting in a high volume leak. In an effort to stabilize the damaged CDSM panel and stop the leak, BBII installed a steel road plate between soldier piles 738-739 and injected grout behind it.			
BBII is concerned that removing the plate will likely cause the panel to become destabilized and could reopen the flow of water. BBII surveyed the face of the plate and found that at pile #738, the face of plate is 3' 0-5/8" back from the inside face of concrete wall and at pile #739 the plate is 3' 1-7/8" back from inside face of concrete wall. BBII proposes leaving the steel plate in place to maintain integrity of the CDSM panel. The edges of the plate may be grouted to provide a smooth transition to the CDSM wall for waterproofing.			BBII is concerned that removing the plate will likely cause the panel to become destabilized and could reopen the flow of water. BBII surveyed the face of the plate and found that at pile #738, the face of plate is 3' 0-5/8" back from the inside face of concrete wall and at pile #739 the plate is 3' 1-7/8" back from inside face of concrete wall. BBII proposes leaving the steel plate in place to maintain integrity of the CDSM panel. The edges of the plate may be grouted to provide a smooth transition to the CDSM wall for waterproofing.			
Please confirm this is acceptable			Please confirm this is acceptable			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0513.1</b>	<b>BGP - Steel plate (RFI #T-0513) encroachment between CDSM Piles No. 738 &amp; 739</b>	<b>Closed</b>	<b>01</b>	<b>05/16/2013</b>	<b>05/26/2013</b>	<b>05/24/2013</b>
<b>From:</b> Webcor Construction LP      Kirk Nielsen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
As depicted in attached SK-0153.1, the encroachment of the steel plate is primarily in the mat slab pour. WOJV is proposing to locally adjust the reinforcement in the mat slab pour to achieve the required cover. There will be no change to the reinforcement on the wall width.		As depicted in attached SK-0153.1, the encroachment of the steel plate is primarily in the mat slab pour. WOJV is proposing to locally adjust the reinforcement in the mat slab pour to achieve the required cover. There will be no change to the reinforcement on the wall width.				
As a means of chamfering the offset the result of steel plates edges to the face of CDSM wall: WOJV is proposing to mechanically fasten expanded metal lath to the CDSM beams using powder activated fasteners. Rapid set mortar is then applied to the required depth ensuring all edges of the plates have a gradual slope back to match the existing face of the CDSM wall.		As a means of chamfering the offset the result of steel plates edges to the face of CDSM wall: WOJV is proposing to mechanically fasten expanded metal lath to the CDSM beams using powder activated fasteners. Rapid set mortar is then applied to the required depth ensuring all edges of the plates have a gradual slope back to match the existing face of the CDSM wall.				
Please confirm this is acceptable.		Please confirm this is acceptable.				
<b>T-0514</b>	<b>BGP - Mech Room Slab Finish Elevation and Grate Clarification</b>	<b>Closed</b>	<b>01</b>	<b>04/24/2013</b>	<b>05/04/2013</b>	<b>04/30/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref: P1-2022		Ref: P1-2022				
Drawing P1-2022 details slab elevations "TOC = -35'-8"" and "Future FFE = -35'-5"" Detail C/P1-4001 depicts a section view of the mat slab in the mechanical pump room; however, it is not clear whether both the Future FFE and TOC of mat slab are shown.		Drawing P1-2022 details slab elevations "TOC = -35'-8"" and "Future FFE = -35'-5"" Detail C/P1-4001 depicts a section view of the mat slab in the mechanical pump room; however, it is not clear whether both the Future FFE and TOC of mat slab are shown.				
1. Please confirm if the attached marked up drawing is correct in detailing the two elevations.		1. Please confirm if the attached marked up drawing is correct in detailing the two elevations.				
2. Also, please confirm if the grates shown in Detail C/P1-4001 are part of the TG06 scope of work. If so, then please provide details for the grate.		2. Also, please confirm if the grates shown in Detail C/P1-4001 are part of the TG06 scope of work. If so, then please provide details for the grate.				
<b>T-0514.1</b>	<b>BGP -Mech Room Slab Finish Elevation and Grate Size Clarification</b>	<b>Closed</b>	<b>01</b>	<b>05/03/2013</b>	<b>05/10/2013</b>	<b>05/07/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						



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<b>T-0519</b>	<b>BGP - Waterproofing Detail Clarification at "Pressure Slab" Joints</b>	<b>Closed</b>	<b>01</b>	<b>04/25/2013</b>	<b>05/05/2013</b>	<b>04/29/2013</b>
<b>From:</b> Webcor Construction LP                      Kody Cooper						
<b>REQUEST:</b>						
Please reference Specification Section 07 12 10 - 3.3.G and Detail 4/A1-8710. Detail 4/A1-8710 shows a typical waterproofing detail for cold joints (construction joints) at walls. Spec Section 07 12 10 - 3.3.G states the following: "Apply two 9" wide strips staggered 6 inches and 3 inches centered over the following joints: 1. Under cold joints in the pressure slab. Temporarily protect the exposed side with protection board until the adjacent slab is cast. 2. On protection boards to receive blindside waterproofing."						
1. Please clarify what the "pressure slab" is referring to as there is no reference to "pressure slab" in the Contract Drawings. 2. Please provide a detail for waterproofing for this condition as a detail does not exist in the Contract Documents. Detail 4/A1-8710 does not reflect what is called out in Specifications Section 07 12 10 - 3.3.G for construction joints.						
<b>ANSWER:</b>						
Please reference Specification Section 07 12 10 - 3.3.G and Detail 4/A1-8710. Detail 4/A1-8710 shows a typical waterproofing detail for cold joints (construction joints) at walls. Spec Section 07 12 10 - 3.3.G states the following: "Apply two 9" wide strips staggered 6 inches and 3 inches centered over the following joints: 1. Under cold joints in the pressure slab. Temporarily protect the exposed side with protection board until the adjacent slab is cast. 2. On protection boards to receive blindside waterproofing."						
1. Please clarify what the "pressure slab" is referring to as there is no reference to "pressure slab" in the Contract Drawings. 2. Please provide a detail for waterproofing for this condition as a detail does not exist in the Contract Documents. Detail 4/A1-8710 does not reflect what is called out in Specifications Section 07 12 10 - 3.3.G for construction joints.						
<b>T-0520</b>	<b>BGP - Finish Floor Elevation</b>	<b>Closed</b>	<b>01</b>	<b>04/26/2013</b>	<b>05/10/2013</b>	<b>05/06/2013</b>
<b>From:</b> Webcor Construction LP                      Ian Corcorran						
<b>REQUEST:</b>						
Ref. Dwg. P1-2022 Ref. Spec. 22 13 01						
Contract drawing P1-2022 calls out "Future FFE = -35'-5"" for the Future Finish Floor Elevation. This elevation note does not appear in any of the other mat slab plumbing drawings (P 1-2023 to P 1-2030). Please confirm if the Future Finish Floor Elevation applies to the entire mat slab.						
<b>ANSWER:</b>						
Ref. Dwg. P1-2022 Ref. Spec. 22 13 01						
Contract drawing P1-2022 calls out "Future FFE = - 35'-5"" for the Future Finish Floor Elevation. This elevation note does not appear in any of the other mat slab plumbing drawings (P 1-2023 to P 1-2030). Please confirm if the Future Finish Floor Elevation applies to the entire mat slab.						
<b>T-0521</b>	<b>BGP - 1 in Aggregate in Protection Slab Cast-in-Place Concrete Mix Design</b>	<b>Closed</b>	<b>01</b>	<b>04/29/2013</b>	<b>05/09/2013</b>	<b>05/02/2013</b>
<b>From:</b> Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>						
Ref: Submittal TG0600-200.1						
<b>ANSWER:</b>						
Ref: Submittal TG0600-200.1						



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<b>T-0522</b>	<p>Please reference submittal TG0600-200.1 (cast-in-place concrete mix design - Protection Slab). Per the referenced submittal and submittal response, sent to SCCI April 12, 2013 and returned as "Make Corrections Noted," SCCI intends to use 1" aggregate in the above mentioned cast-in-place concrete mix. In addition, the above mentioned mix design was also reviewed at the TG06.0 Protection Slab Preparatory DFOW meeting, held April19, 2013.</p> <p>Please confirm the use of 1" aggregate in the Protection Slab is acceptable.</p>	<b>Closed</b>	<b>01</b>	<b>04/29/2013</b>	<b>05/09/2013</b>	<b>05/03/2013</b>
	<p><b>From:</b> Webcor Construction LP      Lynn Kowallis</p> <p><b>REQUEST:</b></p> <p>Ref: S1-2023</p> <p>The primary performance test micropile is yet to be installed for Zone 2. Due to sequencing advantages, BBII proposes relocating this pile from the original location shown in S1-2023 to GL 15 between B&amp;C. The relocated micropile location is within the geothermal area; however, it does not appear to impact geothermal piping. See attached sketch.</p> <p>Please confirm this is acceptable.</p>					
	<p>Please reference submittal TG0600-200.1 (cast-in-place concrete mix design - Protection Slab). Per the referenced submittal and submittal response, sent to SCCI April 12, 2013 and returned as "Make Corrections Noted," SCCI intends to use 1" aggregate in the above mentioned cast-in-place concrete mix. In addition, the above mentioned mix design was also reviewed at the TG06.0 Protection Slab Preparatory DFOW meeting, held April19, 2013.</p> <p>Please confirm the use of 1" aggregate in the Protection Slab is acceptable.</p>					
	<p><b>ANSWER:</b></p> <p>Ref: S1-2023</p> <p>The primary performance test micropile is yet to be installed for Zone 2. Due to sequencing advantages, BBII proposes relocating this pile from the original location shown in S1-2023 to GL 15 between B&amp;C. The relocated micropile location is within the geothermal area; however, it does not appear to impact geothermal piping. See attached sketch.</p> <p>Please confirm this is acceptable.</p>					









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<b>T-0523</b>	<b>BGP - Floor Drain Elevation in Foot Traffic Areas</b>	<b>Closed</b>	<b>01</b>	<b>05/01/2013</b>	<b>05/09/2013</b>	<b>05/07/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref: Specification Section 22 13 01 - 3.2 D.3		Ref: Specification Section 22 13 01 - 3.2 D.3				
Contract specification 22 13 01 - 3.2.D.3 have the following criteria for installation of floor drains:		Contract specification 22 13 01 - 3.2.D.3 have the following criteria for installation of floor drains:				
a. Set drain rims flush and level with finished floor in areas subject to foot traffic.		a. Set drain rims flush and level with finished floor in areas subject to foot traffic.				
b. Set drain rims minus 1/8-inch to 1/4-inch from finish floor elevation, so as to provide positive drainage, where drain is not subject to foot traffic.		b. Set drain rims minus 1/8-inch to 1/4-inch from finish floor elevation, so as to provide positive drainage, where drain is not subject to foot traffic.				
Please provide a map of areas which are to be subject to foot traffic.		Please provide a map of areas which are to be subject to foot traffic.				
<b>T-0524</b>	<b>BGP - Protection Slab Minimum Thickness</b>	<b>Closed</b>	<b>01</b>	<b>05/08/2013</b>	<b>05/18/2013</b>	<b>05/08/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Drawing: S1-3201, A1-8710		Reference Drawing: S1-3201, A1-8710				
SCCI will set the top elevation of the protection slab at -40.67' as shown on the attached contract drawing. Protection slab thickness may vary due to mudslab elevation, mudslab heaving or built-up waterproofing membrane, adhesive and flashings.		SCCI will set the top elevation of the protection slab at -40.67' as shown on the attached contract drawing. Protection slab thickness may vary due to mudslab elevation, mudslab heaving or built-up waterproofing membrane, adhesive and flashings.				
Please provide minimum thickness for protection slab.		Please provide minimum thickness for protection slab.				
<b>T-0525</b>	<b>BGP - Asphalt Cement Specification</b>	<b>Closed</b>	<b>01</b>	<b>04/30/2013</b>	<b>05/10/2013</b>	<b>05/03/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref: Specification Section 07 12 10 - 3.2.E.		Ref: Specification Section 07 12 10 - 3.2.E.				
Please reference Specification Section 07 12 10 - 3.2.E. Specification 3.2.E states "Install two plies of asphalt saturated felts over the protection board in walnut sized gobs of asphalt cement sufficiently spaced to hold felts in place." Spec Section 07 12 10 does not specify the type of asphalt cement to be used. SCCI submitted Roofxtender RX-100 Flashing Cement which was rejected. Shimmick is		Please reference Specification Section 07 12 10 - 3.2.E. Specification 3.2.E states "Install two plies of asphalt saturated felts over the protection board in walnut sized gobs of asphalt cement sufficiently spaced to hold felts in place." Spec Section 07 12 10 does not specify the type of asphalt cement to be used. SCCI submitted Roofxtender RX-100 Flashing				



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T-0526	now proposing to use Laurenco recommended AIM # 340 Flashing Cement. Please confirm that this is acceptable.	Closed	01	05/02/2013	05/14/2013	05/07/2013
T-0526	<b>BGP - Replacement of T9 Wall Cross Ties with S3 Open Stirrups</b>	Closed	01	05/02/2013	05/14/2013	05/07/2013
	<b>From:</b> Webcor Construction LP                      Ian Corcorran					
	<b>REQUEST:</b> Ref Dwg. S1-3201  Please confirm that it is acceptable to replace two T9 wall cross-ties, as depicted in detail 1 on S1-3201, with a single S3 open stirrup. Reference the attached sheets depicting the configuration of the T9 crosstie and S3 open stirrup.					
T-0527		Closed	01	05/01/2013	05/11/2013	05/14/2013
T-0527	<b>BSE - Revision to Zone 4 bracing elevations level A-D</b>	Closed	01	05/01/2013	05/11/2013	05/14/2013
	<b>From:</b> Webcor Construction LP                      Lynn Kowallis					
	<b>REQUEST:</b> Ref: Specification section 31 55 00  Please confirm the design team has no exceptions to raising the Zone-4 bracing elevations, all levels of struts/walers and all related strut supports/trestle bracing, 1'-0" so as to facilitate the specified waterproofing lap in relation to the top of wall.					
T-0527		Closed	01	05/01/2013	05/11/2013	05/14/2013



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T-0527.1	BSE -Revision to Zone 4 Bracing Elevations Level A-D	Closed	01	05/10/2013	05/20/2013	05/14/2013
From: Webcor Construction LP                      Ian Corcorran						
REQUEST:			ANSWER:			
As installed and or planned the current elevation of the A-level internal bracing walers conflicts with the TG06 wall termination elevations relative to the waterproofing overlap which was unspecified when the internal bracing was submitted. Please find attached RFI SK-527.1-1, WOJV proposes to:			As installed and or planned the current elevation of the A-level internal bracing walers conflicts with the TG06 wall termination elevations relative to the waterproofing overlap which was unspecified when the internal bracing was submitted. Please find attached RFI SK-527.1-1, WOJV proposes to:			
1. Reduce the TG06 top of wall elevation 2'-0" to an elevation of +7.50' between approx. GL(s) 1 to 16-17.			1. Reduce the TG06 top of wall elevation 2'-0" to an elevation of +7.50' between approx. GL(s) 1 to 16-17.			
2. Reduce the TG06 top of wall elevation 1'-0" to an elevation of +3.50' between GL(s) approx. GL(s) 16-17 to 25-26.			2. Reduce the TG06 top of wall elevation 1'-0" to an elevation of +3.50' between GL(s) approx. GL(s) 16-17 to 25-26.			
3. Reduce the TG06 top of wall elevation .75' to an elevation of +1.50' between GL(s) approx.. GL(s) 25-26 to 35.			3. Reduce the TG06 top of wall elevation .75' to an elevation of +1.50' between GL(s) approx.. GL(s) 25-26 to 35.			
This scope reallocation would exchange concrete rebar and waterproofing from TG06 to TG07 package, which assuming a prompt response, there is still time to do.			This scope reallocation would exchange concrete rebar and waterproofing from TG06 to TG07 package, which assuming a prompt response, there is still time to do.			



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<b>T-0527.2</b>	<b>BSE - Revision to Zone 4 Bracing Elevations Level A-D</b>	<b>Closed</b>	<b>01</b>	<b>05/28/2013</b>	<b>06/07/2013</b>	<b>06/11/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Sketch: SK-5773  Webcor is proposing that the vertical changes in elevation (per RFI T-527.1) from +7.50' to +3.50' at level A gridline 16 - 17 will occur at a distance of 14'4" from gridline 16 and will be located between CDSM piles 164 - 165 on the north wall elevation and between CDSM piles 618 - 619 on the south elevation  Also vertical changes in elevation level A between gridline 25-26 from +3.50 to +1.50 will occur at a distance of 18'4" from gridline 25 and will be located between CDSM piles 265 - 266 on the north wall elevation and between CDSM piles 517 - 518 on the south elevation  Please confirm is this is acceptable						
						<b>ANSWER:</b>  Reference Sketch: SK-5773  Webcor is proposing that the vertical changes in elevation (per RFI T-527.1) from +7.50' to +3.50' at level A gridline 16 - 17 will occur at a distance of 14'4" from gridline 16 and will be located between CDSM piles 164 - 165 on the north wall elevation and between CDSM piles 618 - 619 on the south elevation  Also vertical changes in elevation level A between gridline 25-26 from +3.50 to +1.50 will occur at a distance of 18'4" from gridline 25 and will be located between CDSM piles 265 - 266 on the north wall elevation and between CDSM piles 517 - 518 on the south elevation  Please confirm is this is acceptable
<b>T-0527.3</b>	<b>BGP - Revision to the top of the foundation wall Elevations TG06</b>	<b>Closed</b>	<b>CR</b>	<b>10/25/2013</b>	<b>11/04/2013</b>	<b>10/29/2013</b>
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>  Due to the revision of the Zone 4 internal bracing and the use of the already procured steal sections the lookout installed for level A bracing were installed at a lower elevation than first planned resulting in the need to revise the finished elevation of the foundation wall downwards for the TG06 package. This scope reallocation will now be moved to the TG07.2 work package. See sketch SK01 attached for TG06 foundation wall finish elevations.  Please confirm if this is acceptable.						
						<b>ANSWER:</b>  Due to the revision of the Zone 4 internal bracing and the use of the already procured steal sections the lookout installed for level A bracing were installed at a lower elevation than first planned resulting in the need to revise the finished elevation of the foundation wall downwards for the TG06 package. This scope reallocation will now be moved to the TG07.2 work package. See sketch SK01 attached for TG06 foundation wall finish elevations.  Please confirm if this is acceptable.
<b>T-0528</b>	<b>BSE - Zone 4 Level 2 Excavation</b>	<b>Closed</b>	<b>01</b>	<b>05/02/2013</b>	<b>05/12/2013</b>	<b>05/13/2013</b>
<b>From:</b> Webcor Construction LP      Kody Cooper						
<b>REQUEST:</b>  Per sheet GT-1111, excavation at each level is limited to						
						<b>ANSWER:</b>  Per sheet GT-1111, excavation at each level is limited



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T-0529	<b>BGP - CJ Layout at Gridline J</b>  <b>From:</b> Webcor Construction LP      Ian Corcorran  <b>REQUEST:</b>  Ref. Drawings: S1-2052 Ref. Spec. 03 30 20- 3.2.A.4  Per Contract Specification 03 30 20- 3.2.A.4, foundation wall, lower concourse floor slab, and ground floor construction joints shall align with the location of the mat slab joint below.  SCCI proposes to have a construction joint at grid line J as shown on attached drawing CJ -11; however, the construction joint would end up dividing the knockout wall into 2 pieces. SCCI proposes to install the J-line construction joint through the mat slab and typical foundation walls while omitting the construction joint through the knockout wall.  Please confirm this is acceptable.	Closed	01	05/02/2013	05/14/2013	05/13/2013
	<b>ANSWER:</b>  Ref. Drawings: S1-2052 Ref. Spec. 03 30 20- 3.2.A.4  Per Contract Specification 03 30 20- 3.2.A.4, foundation wall, lower concourse floor slab, and ground floor construction joints shall align with the location of the mat slab joint below.  SCCI proposes to have a construction joint at grid line J as shown on attached drawing CJ -11; however, the construction joint would end up dividing the knockout wall into 2 pieces. SCCI proposes to install the J-line construction joint through the mat slab and typical foundation walls while omitting the construction joint through the knockout wall.  Please confirm this is acceptable.					



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T-0530	BGP - Dimension conflict between space allocated for BGP waterproofing and BGI	Closed	01	05/03/2013	05/12/2013	05/28/2013
From: Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: GT-2101 Detail 1 Specificaion Section 07 12 10			Ref: GT-2101 Detail 1 Specificaion Section 07 12 10			
To include however not limited to, sheet 1/GT-2101 allows 2" for the specified below grade waterproofing. The installed waterproofing system (07 12 10) thickness is 2-1/4". The insulation layer of the waterproofing system is specified at ½" (07 12 10.2.5.E).			To include however not limited to, sheet 1/GT-2101 allows 2" for the specified below grade waterproofing. The installed waterproofing system (07 12 10) thickness is 2-1/4". The insulation layer of the waterproofing system is specified at ½" (07 12 10.2.5.E).			
So as to resolve the proximity conflict may the contractor reduce the specified insulation thickness from ½" to ¼"?			So as to resolve the proximity conflict may the contractor reduce the specified insulation thickness from ½" to ¼"?			



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T-0531	BGP - Waterproofing Detail Clarification for Flashing Penetrations	Closed	01	05/03/2013	05/12/2013	05/14/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:		ANSWER:				
Reference Specification: 07 12 10 Reference Drawings: A1-8710, A1-8711		Reference Specification: 07 12 10 Reference Drawings: A1-8710, A1-8711				
Contract Drawings and approved shop drawings show sheet metal boots over micropile penetrations set in trowel grade adhesive over two plies of Laurenco.		Contract Drawings and approved shop drawings show sheet metal boots over micropile penetrations set in trowel grade adhesive over two plies of Laurenco.				
Spec Section 07 12 10- 3.4 states the following:		Spec Section 07 12 10- 3.4 states the following:				
A Install flashing at terminations and penetrations		A Install flashing at terminations and penetrations				
B. Flash waterproofing with a minimum of 2 plies of woven glass fabric and 3 applications of adhesive. Extend first ply 6 inches onto each membrane and second 8 inches		B. Flash waterproofing with a minimum of 2 plies of woven glass fabric and 3 applications of adhesive. Extend first ply 6 inches onto each membrane and second 8 inches				
C. At penetrations, apply a minimum of 2 plies spirally wrapped and a target patch per Manufacturer's requirements. Where indicated on the drawings provide stainless steel drawbands.		C. At penetrations, apply a minimum of 2 plies spirally wrapped and a target patch per Manufacturer's requirements. Where indicated on the drawings provide stainless steel drawbands.				
Contract Drawing details and approved shop drawing submittal details do not match what is called out in the above Spec section. Please advise as to which detail is to be used (shop drawing or specifications).		Contract Drawing details and approved shop drawing submittal details do not match what is called out in the above Spec section. Please advise as to which detail is to be used (shop drawing or specifications).				



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T-0532	BGP - Sump Pit Grate Requirements	Closed	01	05/07/2013	05/17/2013	05/14/2013
<b>From:</b> Webcor Construction LP                      Ian Corcorran						
<b>REQUEST:</b>  Ref. Dwg. P1-2022 through P1-2027  There are several sumps shown on the referenced Architectural drawings which are not shown and/or defined on the corresponding Plumbing drawings. The Plumbing Drawing Sheet Notes indicate the grating requirements for all other sumps and Catch Basins on the project (reference note No 1 ,2, 14 and 16 on P1-2022 through P1-2027) There are no such notes for grating requirements for the sumps shown on the attached marked-up Contract Drawings. See attached. Please verify that no grating is required for these sumps.						<b>ANSWER:</b>  Ref. Dwg. P1-2022 through P1-2027  There are several sumps shown on the referenced Architectural drawings which are not shown and/or defined on the corresponding Plumbing drawings. The Plumbing Drawing Sheet Notes indicate the grating requirements for all other sumps and Catch Basins on the project (reference note No 1 ,2, 14 and 16 on P1-2022 through P1-2027) There are no such notes for grating requirements for the sumps shown on the attached marked-up Contract Drawings. See attached. Please verify that no grating is required for these sumps.
T-0533	BGP - Mat Slab Drainage System Testing	Closed	CR	05/06/2013	05/16/2013	05/09/2013
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>  Reference Specification Section 22 13 01 3.3 E, Reference 2010 California Plumbing Code article 712.  Article 712.1 Media, of the California plumbing code states that: "The piping of plumbing, drainage, and vent piping systems shall be tested with water or air except that plastic pipe shall not be tested with air."  For testing of the cast iron drainage lines that get embedded in the Mat slab SCCI would like to utilize the air test method.Air test method is specified in the California plumbing code article 712.3, and achieved by: "forcing airinto the system until there is a uniform gauge pressure of five (5) PSI. The pressure shall be held without introduction of additional air for a period of not less than fifteen (15) minutes."  Is this acceptable?						<b>ANSWER:</b>  Reference Specification Section 22 13 01 3.3 E, Reference 2010 California Plumbing Code article 712.  Article 712.1 Media, of the California plumbing code states that: "The piping of plumbing, drainage, and vent piping systems shall be tested with water or air except that plastic pipe shall not be tested with air."  For testing of the cast iron drainage lines that get embedded in the Mat slab SCCI would like to utilize the air test method.Air test method is specified in the California plumbing code article 712.3, and achieved by: "forcing airinto the system until there is a uniform gauge pressure of five (5) PSI. The pressure shall be held without introduction of additional air for a period of not less than fifteen (15) minutes."  Is this acceptable?
T-0534	BGP - Request for Latest Revit Model	Closed	01	05/07/2013	05/16/2013	05/09/2013
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>						<b>ANSWER:</b>





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	Reference Specification: 01 31 26					
	SCCI is requesting access to the latest, most up to date Structural and Architectural Revit models from the designers. The 3D database would be used for reference only and will not be used for construction. SCCI understands that the 3D Database is subject to change as the project design evolves. As a user of this 30 database, SCCI accepts the risk and acknowledge that the data is subject to change. SCCI also acknowledges the terms and conditions outlined in the Transbay Transit Specification Section 01 31 26.					
	Reference Specification: 01 31 26					
	SCCI is requesting access to the latest, most up to date Structural and Architectural Revit models from the designers. The 3D database would be used for reference only and will not be used for construction. SCCI understands that the 3D Database is subject to change as the project design evolves. As a user of this 30 database, SCCI accepts the risk and acknowledge that the data is subject to change. SCCI also acknowledges the terms and conditions outlined in the Transbay Transit Specification Section 01 31 26.					
<b>T-0535</b>	<b>BGP - Elevator Opening Encroachment at Concrete Beam B131</b>	<b>Closed</b>	<b>01</b>	<b>05/07/2013</b>	<b>05/16/2013</b>	<b>05/09/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>						
Reference Drawing: A1-2842, S1-2202, S1-3401						
Please reference attached Contract Drawings A 1-2842, S 1-2202 and S 1-3401. DrawingS 1-2202 calls out concrete beam B131 running east to west between the elevator and Stair openings. The dimensions of concrete beam B131 are 22 inches wide and 36 inches high. See drawing S1-3401 for beam schedule. A1 -2842 calls out the spacing between openings to be 1'-9". This makes the elevator pit encroach 1 inch into concrete beam B131. Shall the elevator opening be relocated 1 inch to the south to accommodate the concrete beam? Please advise.						
<b>ANSWER:</b>						
Reference Drawing: A1-2842, S1-2202, S1-3401						
Please reference attached Contract Drawings A 1-2842, S 1-2202 and S 1-3401. DrawingS 1-2202 calls out concrete beam B131 running east to west between the elevator and Stair openings. The dimensions of concrete beam B131 are 22 inches wide and 36 inches high. See drawing S1-3401 for beam schedule. A1 -2842 calls out the spacing between openings to be 1'-9". This makes the elevator pit encroach 1 inch into concrete beam B131. Shall the elevator opening be relocated 1 inch to the south to accommodate the concrete beam? Please advise.						



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<b>T-0536</b>	<b>BGP - Sump Conflicting with Trestle Pile</b>	<b>Closed</b>	<b>01</b>	<b>05/07/2013</b>	<b>05/06/2013</b>	<b>05/22/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Drawing: A1-2817, S1-2027  Based on the latest BBII trestle model available to SCCI and contract drawing A1-2817, there appears to be a conflict between a sump pit and trestle pile near column line "34" and "E". Please refer to the attached screen shot from SCCIs Revit Model.  8/31/2012 IFC drawings did not show this sump pit as it was added in ASI No. 0099.  Please provide direction on how to proceed		<b>ANSWER:</b> Reference Drawing: A1-2817, S1-2027  Based on the latest BBII trestle model available to SCCI and contract drawing A1-2817, there appears to be a conflict between a sump pit and trestle pile near column line "34" and "E". Please refer to the attached screen shot from SCCIs Revit Model.  8/31/2012 IFC drawings did not show this sump pit as it was added in ASI No. 0099.  Please provide direction on how to proceed				
<b>T-0537</b>	<b>BGP - Sump Pit/Catch Basin Clarification at Gridlines C/19.1</b>	<b>Closed</b>	<b>01</b>	<b>05/07/2013</b>	<b>05/16/2013</b>	<b>05/13/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Specification: 22 13 01 Reference Drawing: A1-2815, S1-2055, P1-2025  The pit near gridlines C/19.1 is identified as a catch basin in drawing A1-2815 but identified as a sump pit in drawing S1-2055. Drawing P1-2025 does not show any piping for this pit. Please confirm if this should be a sump pit or is the piping detail missing?		<b>ANSWER:</b> Reference Specification: 22 13 01 Reference Drawing: A1-2815, S1-2055, P1-2025  The pit near gridlines C/19.1 is identified as a catch basin in drawing A1-2815 but identified as a sump pit in drawing S1-2055. Drawing P1-2025 does not show any piping for this pit. Please confirm if this should be a sump pit or is the piping detail missing?				
<b>T-0538</b>	<b>BGP - Sump Pit Frame Elevation</b>	<b>Closed</b>	<b>01</b>	<b>05/07/2013</b>	<b>05/15/2013</b>	<b>05/10/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b> Ref Dwg. P1-2022, P1-6001  Please reference attached sketch SK-0163 and Contract Drawings P1-2022 and P1-6001. Drawing P1-2022 calls out Top of Concrete = -35'-8" and Finish Floor Elevation= -35'-5". P1-6001 Detail 8 shows top of sump grate frames to be flush with the surface in which it is embedded. It is unclear whether this is top of concrete or top of finish floor. SCCI has not been provided drawings to confirm topping slab extents. There shall be a 3 in vertical edge if sump pit frames are placed flush with top of mat slab concrete and		<b>ANSWER:</b> Ref Dwg. P1-2022, P1-6001  Please reference attached sketch SK-0163 and Contract Drawings P1-2022 and P1-6001. Drawing P1-2022 calls out Top of Concrete = -35'-8" and Finish Floor Elevation= -35'-5". P1-6001 Detail 8 shows top of sump grate frames to be flush with the surface in which it is embedded. It is unclear whether this is top of concrete or top of finish floor. SCCI has not been provided drawings to confirm topping slab extents. There shall be a 3 in vertical edge if sump pit frames				



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T-0539	<b>BGP - ASTM 123 Galvanizing Variance</b>  <b>From:</b> Webcor Construction LP                      Kody Cooper  <b>REQUEST:</b>  Reference Specification: 05 05 15.3.3.B and the attached letter from AZZ Galvanizing  The steel pipe penetration sleeves are to be coated under the Structural Shapes and Plate Material Category with a grade of 100 and 3.9 mils thickness per Tables 1 & 2 of ASTM A123. The first 2 shipments of steel penetration sleeves (approximately 12 pin pile and 17 trestle pile) were coated under the pipe and tubing material category with a Grade 75 per Table 1 of ASTM A123. This coating grade requires 3.0 miles per Table 2 - Coating Thickness Grade. SCCI is requesting that the Grade 75 be allowed for the first two pin pile in Area 1 that are fit and welded to the intermetallic layers having still penetrated the material and per the attached letter, the process used will insure a long service life. The average thickness for the specified pin pile above is 3.2 mils. Is this acceptable?	Closed	01	05/07/2013	05/17/2013	05/07/2013
						<b>ANSWER:</b>  Reference Specification: 05 05 15.3.3.B and the attached letter from AZZ Galvanizing  The steel pipe penetration sleeves are to be coated under the Structural Shapes and Plate Material Category with a grade of 100 and 3.9 mils thickness per Tables 1 & 2 of ASTM A123. The first 2 shipments of steel penetration sleeves (approximately 12 pin pile and 17 trestle pile) were coated under the pipe and tubing material category with a Grade 75 per Table 1 of ASTM A123. This coating grade requires 3.0 miles per Table 2 - Coating Thickness Grade. SCCI is requesting that the Grade 75 be allowed for the first two pin pile in Area 1 that are fit and welded to the intermetallic layers having still penetrated the material and per the attached letter, the process used will insure a long service life. The average thickness for the specified pin pile above is 3.2 mils. Is this acceptable?



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<b>T-0541</b>	<b>BGP - Protection Board Installation at SW Corner</b>	<b>Closed</b>	<b>01</b>	<b>05/09/2013</b>	<b>05/23/2013</b>	<b>05/13/2013</b>
<b>From:</b> Webcor Construction LP                      Ian Corcorran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref Spec. 07 12 10- 3.2.D		Ref Spec. 07 12 10- 3.2.D				
Please reference Specifications Section 07 12 10- 3.2.D. This spec states the following: "Install protection board on vertical surfaces with long dimension vertical and the polyethylene film side facing the soil/cement surfaces. Secure 1 /4" protection board to flanges of soldier piles with powder driven fasteners and washers spaced 12 inches o.c .... "		Please reference Specifications Section 07 12 10- 3.2.D. This spec states the following: "Install protection board on vertical surfaces with long dimension vertical and the polyethylene film side facing the soil/cement surfaces. Secure 1 /4" protection board to flanges of soldier piles with powder driven fasteners and washers spaced 12 inches o.c .... "				
At the SW corner of the project, the soldier beams are spaced greater than the width of the protection board. We suggest rotating the protection board 90-degrees so that the long dimension is horizontal instead of vertical. This would allow for attachment to the soldier piles and the inside corner of protection board to be heated and formed into the corner. We also suggest using this method where pile spacing exceeds 4' on center.		At the SW corner of the project, the soldier beams are spaced greater than the width of the protection board. We suggest rotating the protection board 90-degrees so that the long dimension is horizontal instead of vertical. This would allow for attachment to the soldier piles and the inside corner of protection board to be heated and formed into the corner. We also suggest using this method where pile spacing exceeds 4' on center.				
Please review and advise.		Please review and advise.				
<b>T-0542</b>	<b>BGP - Drainage Mat Installation Clarification</b>	<b>Closed</b>	<b>01</b>	<b>05/09/2013</b>	<b>05/23/2013</b>	<b>05/13/2013</b>
<b>From:</b> Webcor Construction LP                      Ian Corcorran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref. Spec. 07 12 10- 3.2.F.		Ref. Spec. 07 12 10- 3.2.F.				
This spec section states the following: "Install drainage composite in largest practical sizes over the entire area of the felts. Install either vertically or horizontally and lap sheets 1" in direction of flow ... "		This spec section states the following: "Install drainage composite in largest practical sizes over the entire area of the felts. Install either vertically or horizontally and lap sheets 1" in direction of flow ... "				
1. If drainage composite is installed vertically, please confirm that direction of water flow is down vertically towards the mudslab.		1. If drainage composite is installed vertically, please confirm that direction of water flow is down vertically towards the mudslab.				
2. Please confirm that only horizontal joints in the drainage composite will be lapped 1 inch.		2. Please confirm that only horizontal joints in the drainage composite will be lapped 1 inch.				
3. Please confirm that vertical drainage core joints will be butt jointed.		3. Please confirm that vertical drainage core joints will be butt jointed.				





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<b>T-0544</b>	<b>BSE - Micropile Relocation - W990 &amp; W986 (Well Obstructions)</b>	<b>Closed</b>	<b>01</b>	<b>05/10/2013</b>	<b>05/11/2013</b>	<b>05/13/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>  Micropiles W990 and W986 as laid out are both in conflict with dewatering wells. BBII recommends relocating W990 south 3' and W986 north 3'. See attached sketch.  Please confirm this is acceptable.		<b>ANSWER:</b>  Micropiles W990 and W986 as laid out are both in conflict with dewatering wells. BBII recommends relocating W990 south 3' and W986 north 3'. See attached sketch.  Please confirm this is acceptable.				
<b>T-0545</b>	<b>BGP - Embedded Junction Box Details</b>	<b>Closed</b>	<b>01</b>	<b>05/10/2013</b>	<b>05/24/2013</b>	<b>05/24/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>  Ref. Dwg. A1-2842, A1-2850  Please reference Contract Drawings A1-2842 to A1-2850. These drawings contain numerous "EJB" callouts. SCCI's issued drawings do not contain details for embedded junction boxes. SCCI is trying to determine if there will be any conflicts with the EJB locations. Will the EJB's be selected by the future contactor in which this scope is contained? If specific EJB's have been specified already, please provide the detail so SCCI can confirm there are no conflicts with SCCI's scope.		<b>ANSWER:</b>  Ref. Dwg. A1-2842, A1-2850  Please reference Contract Drawings A1-2842 to A1-2850. These drawings contain numerous "EJB" callouts. SCCI's issued drawings do not contain details for embedded junction boxes. SCCI is trying to determine if there will be any conflicts with the EJB locations. Will the EJB's be selected by the future contactor in which this scope is contained? If specific EJB's have been specified already, please provide the detail so SCCI can confirm there are no conflicts with SCCI's scope.				
<b>T-0546</b>	<b>BGP - Shear Reinforcement and Drainage Conflict at Grldlines 4/C</b>	<b>Closed</b>	<b>01</b>	<b>05/09/2013</b>	<b>05/23/2013</b>	<b>05/28/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>  Ref Dwg A1-9215, S1-2022  At gridlines 4/C, the floor clean out and floor sinks (see A1-9215) cannot be installed due to the spacing of the top layer mat slab and shear reinforcement (see S1-2022). Please advise on how to proceed. Reference the attached sketch of conflict.		<b>ANSWER:</b>  Ref Dwg A1-9215, S1-2022  At gridlines 4/C, the floor clean out and floor sinks (see A1-9215) cannot be installed due to the spacing of the top layer mat slab and shear reinforcement (see S1-2022). Please advise on how to proceed. Reference the attached sketch of conflict.				
<b>T-0546.1</b>	<b>BGP - Follow Up to RFI 173- Shear Reinforcement and Drainage Conflict at 4/C</b>	<b>Closed</b>	<b>01</b>	<b>06/28/2013</b>	<b>07/08/2013</b>	<b>07/12/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						















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<b>T-0553</b>	<b>BGP - Examination of Substrate Clarification</b>	<b>Closed</b>	<b>01</b>	<b>05/14/2013</b>	<b>05/28/2013</b>	<b>05/23/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please reference Specification Section 07 12 10- 3.1.A.		Please reference Specification Section 07 12 10- 3.1.A.				
Specification Section 07 12 10 - 3.1.A states the following: "With manufacturer's representative present, examine surfaces to which insulation and waterproofing will be applied prior to beginning work."		Specification Section 07 12 10 - 3.1.A states the following: "With manufacturer's representative present, examine surfaces to which insulation and waterproofing will be applied prior to beginning work."				
Please confirm that this is in reference to CDSM wall and mudslab.		Please confirm that this is in reference to CDSM wall and mudslab.				
<b>T-0554</b>	<b>BGP - Field Quality Control</b>	<b>Closed</b>	<b>01</b>	<b>05/14/2013</b>	<b>05/28/2013</b>	<b>05/25/2013</b>
<b>From:</b> Webcor Construction LP      Ian Corcorran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please reference Specifications Sections 07 12 10 - 3.3- 3.5.		Please reference Specifications Sections 07 12 10 - 3.3-3.5.				
Specifications Section 07 12 10- 3.5.A states the following: "The manufacturer's field representative shall be present before and during installation as specified above."		Specifications Section 07 12 10- 3.5.A states the following: "The manufacturer's field representative shall be present before and during installation as specified above."				
Please confirm that this is in reference to Section 3.3 "Application" and Section 3.4 "Flashing" which are directly above Section 3.5, A on page 07 12 10-8 of the Specifications (attached for reference).		Please confirm that this is in reference to Section 3.3 "Application" and Section 3.4 "Flashing" which are directly above Section 3.5, A on page 07 12 10-8 of the Specifications (attached for reference).				
<b>T-0555</b>	<b>BGP - Waterproofing Asphalt Cement Walnut Sized Gob Spacing</b>	<b>Closed</b>	<b>01</b>	<b>05/16/2013</b>	<b>05/26/2013</b>	<b>05/23/2013</b>
<b>From:</b> Webcor Construction LP      Kody Cooper						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Specification Section 07 12 10, 3.2, E states the following:		Specification Section 07 12 10, 3.2, E states the following:				
"Install two piles of asphalt saturated felts over the protection board in walnut sized gobs of asphalt cement sufficiently spaced to hold felts in place."		"Install two piles of asphalt saturated felts over the protection board in walnut sized gobs of asphalt cement sufficiently spaced to hold felts in place."				
SCCI and Best have been informed that this layer is to act as the shear/slip plane for structural movement. Please provide the spacing requirements of the walnut sized		SCCI and Best have been informed that this layer is to act as the shear/slip plane for structural movement.				



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	gobs.					Please provide the spacing requirements of the walnut sized gobs.
<b>T-0556</b>	<b>BGP - Waterproofing Asphalt Cement Diameter of Walnut Sized Gobs</b>	<b>Closed</b>	<b>01</b>	<b>05/16/2013</b>	<b>05/26/2013</b>	<b>05/20/2013</b>
<b>From:</b> Webcor Construction LP                      Kody Cooper						
<b>REQUEST:</b> Specification Section 07 12 10, 3.2, E states the following:  "Install two piles of asphalt saturated felts over the protection board in walnut sized gobs of asphalt cement sufficiently spaced to hold felts in place."  Please provide approximate diameter of walnut sized gobs (maximum/minimum will suffice).						<b>ANSWER:</b> Specification Section 07 12 10, 3.2, E states the following:  "Install two piles of asphalt saturated felts over the protection board in walnut sized gobs of asphalt cement sufficiently spaced to hold felts in place."  Please provide approximate diameter of walnut sized gobs (maximum/minimum will suffice).
<b>T-0557</b>	<b>BGP - Waterproofing Asphalt Cement with Laps in Felt Layers</b>	<b>Closed</b>	<b>01</b>	<b>05/16/2013</b>	<b>05/26/2013</b>	<b>05/21/2013</b>
<b>From:</b> Webcor Construction LP                      Kody Cooper						
<b>REQUEST:</b> In reference to Specification Section 07 12 10, 3.2, E, the specifications do not mention laps in felt layers needing to be fully sealed in asphalt cement. Please confirm that fully sealed laps are not required.						<b>ANSWER:</b> In reference to Specification Section 07 12 10, 3.2, E, the specifications do not mention laps in felt layers needing to be fully sealed in asphalt cement. Please confirm that fully sealed laps are not required.
<b>T-0557.1</b>	<b>BGP - Waterproofing Asphalt Cement with Laps in Felt Layers</b>	<b>Closed</b>	<b>01</b>	<b>05/31/2013</b>	<b>06/10/2013</b>	<b>06/03/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b> Please reference RFI #T-0557 response and Specifications Section 071210-3.2. RFI #T-0557 response confirms that end laps are not sealed, but does not address the side laps.  Please confirm that this applies to the side laps as well.						<b>ANSWER:</b> Please reference RFI #T-0557 response and Specifications Section 071210-3.2. RFI #T-0557 response confirms that end laps are not sealed, but does not address the side laps.  Please confirm that this applies to the side laps as well.



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<b>T-0558</b>	<b>BGP - Waterproofing Asphalt Cement at Protection Board Transitions</b>	<b>Closed</b>	<b>01</b>	<b>05/16/2013</b>	<b>05/26/2013</b>	<b>05/23/2013</b>
<b>From:</b> Webcor Construction LP                      Kody Cooper						
<b>REQUEST:</b>  In discussions with the TJPA and Designers, the "gaps" where the 2' protection board meets 6" turnout at the base transition, shall be filled with asphalt cement. Is it acceptable to fill these "gaps" with asphalt cement?						<b>ANSWER:</b>  In discussions with the TJPA and Designers, the "gaps" where the 2' protection board meets 6" turnout at the base transition, shall be filled with asphalt cement. Is it acceptable to fill these "gaps" with asphalt cement?
<b>T-0559</b>	<b>BGP - ASI 102 Change Clarification at Elevator Pit Near GL 2-E 2</b>	<b>Closed</b>	<b>01</b>	<b>05/14/2013</b>	<b>05/28/2013</b>	<b>05/23/2013</b>
<b>From:</b> Webcor Construction LP                      Ian Corcorran						
<b>REQUEST:</b>  Ref. Dwg. A1-2812, 1/A1-9214, 3/S1-3006  Revision 0 of A1-2812 previously contained dimensions for the elevator pit near gridlines 4-E.2; however, as a result of ASI 102 revision 1 of A1-2812 no longer contain the dimensions for the elevator pit and the referenced detail 1 of A1-9214 does not either. Please provide the dimensions of the elevator pit.  Also, detail 3 of S1-3006 indicates that there is a change in the thickened section of the elevator but it does not appear that there were any changes made. Please confirm if there are changes to the thickened section.						<b>ANSWER:</b>  Ref. Dwg. A1-2812, 1/A1-9214, 3/S1-3006  Revision 0 of A1-2812 previously contained dimensions for the elevator pit near gridlines 4-E.2; however, as a result of ASI 102 revision 1 of A1-2812 no longer contain the dimensions for the elevator pit and the referenced detail 1 of A1-9214 does not either. Please provide the dimensions of the elevator pit.  Also, detail 3 of S1-3006 indicates that there is a change in the thickened section of the elevator but it does not appear that there were any changes made. Please confirm if there are changes to the thickened section.
<b>T-0560</b>	<b>BGP - Grade 60 ASTM A-615 Conforming Bar In-Lieu of ASTM A-706</b>	<b>Closed</b>	<b>01</b>	<b>05/16/2013</b>	<b>05/29/2013</b>	<b>05/29/2013</b>
<b>From:</b> Webcor Construction LP                      Ian Corcorran						
<b>REQUEST:</b>  Ref. Dwg. RE-2/S-0007  Gerdau proposes to use Grade 60 ASTM A-615 bar in place of Grade 60 ASTM A-706 material in the locations defined within RE-2 on sheet S-0007 which include foundation walls, columns and moment frame beams. The Grade 60 ASTM A-615 bar shall conform to the strength properties published in the attached ASTM specifications.  This is not a request to replace all Grade 60 ASTM A-706 bars with Grade 60 ASTM-615. Is it acceptable to use						<b>ANSWER:</b>  Ref. Dwg. RE-2/S-0007  Gerdau proposes to use Grade 60 ASTM A-615 bar in place of Grade 60 ASTM A-706 material in the locations defined within RE-2 on sheet S-0007 which include foundation walls, columns and moment frame beams. The Grade 60 ASTM A-615 bar shall conform to the strength properties published in the attached ASTM specifications.  This is not a request to replace all Grade 60 ASTM A-



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T-0561	ASTM-615 bars, when available, that would otherwise be wasted during the rebar fabrication process?					
	706 bars with Grade 60 ASTM-615. Is it acceptable to use ASTM-615 bars, when available, that would otherwise be wasted during the rebar fabrication process?					
T-0561	<b>BSE - Standard for Determining Buttress Concrete Strength</b>  From: Webcor Construction LP      Kirk Nielsen  <b>REQUEST:</b> Spec. section 31 63 29.3.9.D states,  "Not less than 28 days after concreting is completed, perform HQ coring over the full depth of 10% of the shafts to verify the quality of concrete and test whether the shafts are free of defects. Provide these cores for inspection by the TJPA Representative. The TJPA's Representative will select the locations where coring shall be performed and will select the cores which will be tested for strength."  The aforementioned language in addition to spec. section 31 63 29.1.6.A which states:  "Perform work in accordance with ACI 301, except where otherwise specified. Specifications herein set minimum results required and references to procedures to establish minimum guidelines."  reads as if ACI 301 would be the specified standard for determining the required buttress concrete strength (specifically ACI 301 section 1.6.6.2) hence acceptance.  Please confirm what if not ACI 301 is the standard for determining the buttress concrete strength hence acceptance.	Closed	01	05/16/2013	05/26/2013	05/20/2013
T-0562	BGP Stair 403 Embed Conflict					
	From: Webcor Construction LP      Robert Kjome  <b>REQUEST:</b>	Closed	01	05/17/2013	05/27/2013	05/24/2013
T-0562						
	<b>ANSWER:</b>					



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	<p>Reference Drawings: S1-7011, SI -7600, S1-7602, and sketch SK-194.</p> <p>Detail 3 on S1-7011 has a callout for 11/S1-7600 and 8/S1-7602. Both of these angles are embedded in the top edge of the stair opening. The locations of embeds overlap at the Northeast and Southeast portions of the opening. See SK-194 for details. The 8" legs of the angles are to be on different surfaces of the concrete causing future stair installation issues.</p> <p>Please provide details on how to proceed.</p>					
T-0562.1	<p><b>BGP - Stair 403 Embed Conflict</b></p> <p><b>From:</b> Webcor/Obayashi Joint Venture      Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>1) Please reference RFI response T-0562. Please confirm the 7' -6" long embed per detail 8 on S I -7602 starts from the western edge of the opening, as shown in the attached sketch.</p> <p>2) Also, please clarify embedded angle conflicts highlighted on attached sketch, where embed as shown on detail 11, S1-7600 and embed as shown on detail 8, S1-7602 are specified to be installed at the same location.</p>	Closed	01	08/13/2013	08/23/2013	08/21/2013





Please confirm this is acceptable.





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<b>T-0564</b>	<b>BGP - Water Treatment for Geothermal</b>	<b>Closed</b>	<b>01</b>	<b>05/21/2013</b>	<b>05/31/2013</b>	<b>06/03/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b> Reference Specification 23 57 34 Sub Section 3.4  During the TG06 IFB process section 3.4 was added to the Ground Loop Heat Exchanger specifications. We believe this requirement is intended for a future bid package during the commissioning of the system. Please confirm.						<b>ANSWER:</b> Reference Specification 23 57 34 Sub Section 3.4  During the TG06 IFB process section 3.4 was added to the Ground Loop Heat Exchanger specifications. We believe this requirement is intended for a future bid package during the commissioning of the system. Please confirm.
<b>T-0565</b>	<b>BGP - Waterstop Injection Hose Boxes</b>	<b>Closed</b>	<b>01</b>	<b>05/22/2013</b>	<b>06/01/2013</b>	<b>05/23/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b> Please reference attached drawing A1-8711. Please confirm all Waterstop Injection Hose Boxes in the Mat Slab are to be mounted as illustrated in the attached drawing (flush@ -35' -8"). With the installation of a future topping slab, mounting these boxes at Mat Slab elevation may render the injection hose system inaccessible at that time.						<b>ANSWER:</b> Please reference attached drawing A1-8711. Please confirm all Waterstop Injection Hose Boxes in the Mat Slab are to be mounted as illustrated in the attached drawing (flush@ -35' -8"). With the installation of a future topping slab, mounting these boxes at Mat Slab elevation may render the injection hose system inaccessible at that time.
<b>T-0566</b>	<b>BSE - Zone 2 A-Line CDSM Embedded Metal Part at Soldier Pile 96</b>	<b>Closed</b>	<b>CR</b>	<b>05/22/2013</b>	<b>06/01/2013</b>	<b>05/24/2013</b>
<b>From:</b> Webcor Construction LP Lynn Kowallis						
<b>REQUEST:</b> Ref: BIM 360 - Field Condition Report (FCR) 000013 Specification Section 31 56 13  Per FCR 000013: "An Embedded Metal part is visible in the CDSM wall between Solder Piles 96 & 97. A Corrective Action Plan must be submitted to remove the object and repair the CDSM wall. Spec 31 56 13." Please see attached BBII proposed Corrective Action Plan.  Please confirm this is acceptable.						<b>ANSWER:</b> Ref: BIM 360 - Field Condition Report (FCR) 000013 Specification Section 31 56 13  Per FCR 000013: "An Embedded Metal part is visible in the CDSM wall between Solder Piles 96 & 97. A Corrective Action Plan must be submitted to remove the object and repair the CDSM wall. Spec 31 56 13." Please see attached BBII proposed Corrective Action Plan.  Please confirm this is acceptable.
<b>T-0567</b>	<b>BGP - Fire Management System</b>	<b>Closed</b>	<b>01</b>	<b>05/23/2013</b>	<b>06/02/2013</b>	<b>06/03/2013</b>
<b>From:</b> Webcor Construction LP Lynn Kowallis						



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T-0568	BGP - Monitoring Instrument Sleeves Detail	Closed	01	05/23/2013	06/02/2013	05/30/2013
<p><b>From:</b> Webcor Construction LP                  Robert Kjome</p>						
<b>REQUEST:</b>						
Reference Drawings: 3/A1-8711, 4/S1-3009						
Detail 4 on SI-3009 does not apply to the monitoring instrument sleeves shown on A1-8711. The sleeves are continuous through the Mat Slab, and thus do not require a blackout.						
Please provide a typical mat slab rebar detail that is applicable to the monitoring instrument sleeves referenced herein.						
<b>ANSWER:</b>						
Ref: 5/E1-6001, E1-2202, E-0006						
Per Plan Sheet E1-6001 , Detail 5. (attached) the embedded conduits for the fire managment system do not extend above or into the lower concourse floor slab. Additionally, on Plan Sheet E1-2202 for the lower concourse level, Sheet Note A (attached) states that the "scope of work on this sheet is limited only to grounding electrode conductors embedded in slab and main grounding bus (MGB) in the main electrical room." From these two notes, it is clear that the fire alarm system scope is limited to the train platfonn level.						
However, on Plan Sheet E-0006 (attached) General Note R., "For fire alarm devices, provide embedded boxes, conduit, and pull strings in the lower concourse slab for service to fire alarm devices for both levels".						
Please proved direction as to whether or not the fire alarm system conduit is to be installed in the lower concourse level slab.						
T-0569	BGP - Reinforced Concrete Wall Clarification	Closed	01	05/23/2013	06/02/2013	05/30/2013
<p><b>From:</b> Webcor Construction LP                  Robert Kjome</p>						
<b>REQUEST:</b>						
<b>ANSWER:</b>						



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	<p>Reference Drawing: A1-2122</p> <p>Per the note on A1-2122, walls called out as "RCW" are reference drawings and not in TG06's scope of work. "RCW" walls are generally illustrated with dotted lines; however, when referencing the walls for the elevator pit and stairs near gridline 2E, the walls are called out as "RCW" but also illustrated with solid lines. Please confirm which walls are part of the TG06 package and which are RCW. Also, please confirm if the entire South wall of the fuel tank room is supposed to be "RCW" or if it is just the wall section as shown.</p>					<p>Reference Drawing: A1-2122</p> <p>Per the note on A1-2122, walls called out as "RCW" are reference drawings and not in TG06's scope of work. "RCW" walls are generally illustrated with dotted lines; however, when referencing the walls for the elevator pit and stairs near gridline 2E, the walls are called out as "RCW" but also illustrated with solid lines. Please confirm which walls are part of the TG06 package and which are RCW. Also, please confirm if the entire South wall of the fuel tank room is supposed to be "RCW" or if it is just the wall section as shown.</p>
<b>T-0570</b>	<b>BGP - Underside of Beam Embed Conflict</b>	<b>Closed</b>	<b>01</b>	<b>05/24/2013</b>	<b>06/03/2013</b>	<b>06/03/2013</b>
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: S1-7011, S1-7900/Detail 9, S1-9100/Detail 2, Attached SK-0201						Ref: S1-7011, S1-7900/Detail 9, S1-9100/Detail 2, Attached SK-0201
Please see attached Contract Drawing S1-7011 and Sketch SK-0201 . Stair opening 403 has stair post plates embedded on the underside of the concrete beams. See S1-7600/D9 for details. The underside of the concourse slab also contains continuous concrete inserts. See S1 - 9100/D2 for details and A1-2844 for locations. The two embeds overlap on the underside of the beams on the north and south sides of stair opening 403. See SK-0201 for drawing of conflicting embeds. This also occurs on the south side of stair opening 501.						Please see attached Contract Drawing S1-7011 and Sketch SK-0201 . Stair opening 403 has stair post plates embedded on the underside of the concrete beams. See S1-7600/D9 for details. The underside of the concourse slab also contains continuous concrete inserts. See S1 -9100/D2 for details and A1-2844 for locations. The two embeds overlap on the underside of the beams on the north and south sides of stair opening 403. See SK-0201 for drawing of conflicting embeds. This also occurs on the south side of stair opening 501.
Please provide details on how to install the two conflicting embeds on the underside of the concourse.						Please provide details on how to install the two conflicting embeds on the underside of the concourse.



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-0571</b>	<b>BGP - New Waterproofing Install Instructions (Additional Adhesive)</b>	<b>Closed</b>	<b>01</b>	<b>05/28/2013</b>	<b>06/07/2013</b>	<b>05/31/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Specifications Section 071210  We are in receipt of Laurencio Waterproofing Products, Inc.'s Installation Instructions revision dated 5/15/13 and have found several discrepancies with what is called out in Specifications Section 071210. Section 8, f, iv, 4, (d), vi calls for an additional layer of adhesive on walls prior to concrete. It calls for a coat of Laurencio Adhesive over COMPLETED membrane and cold joint reinforcement applied after wall ply adhesive has cured at least (3) days and (1) to (3) days in advance of reinforcement steel application for walls. This is added scope as this additional layer is to be applied over the completed membrane and is not called out in the Specifications.  Please confirm that SCCI is to use the manufacturers installation instructions						<b>ANSWER:</b>  Specifications Section 071210  We are in receipt of Laurencio Waterproofing Products, Inc.'s Installation Instructions revision dated 5/15/13 and have found several discrepancies with what is called out in Specifications Section 071210. Section 8, f, iv, 4, (d), vi calls for an additional layer of adhesive on walls prior to concrete. It calls for a coat of Laurencio Adhesive over COMPLETED membrane and cold joint reinforcement applied after wall ply adhesive has cured at least (3) days and (1) to (3) days in advance of reinforcement steel application for walls. This is added scope as this additional layer is to be applied over the completed membrane and is not called out in the Specifications.  Please confirm that SCCI is to use the manufacturers installation instructions
<b>T-0572</b>	<b>BGP - New Waterproofing Install Instructions (3 day Cure)</b>	<b>Closed</b>	<b>01</b>	<b>05/29/2013</b>	<b>06/08/2013</b>	<b>05/30/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Specifications: 071210  We are in receipt of Laurencio Waterproofing Products, Inc.'s Installation Instructions revision dated 5/15/13 and have found several discrepancies with what is called out in Specifications Section 071210. Section 8, f, iv, 5, (d), v calls for a minimum (3) days wait for top ply to firmly adhere before starting the rest of flashing details and placing concrete topping slab. This is not called out in the specifications and may significantly impact the project schedule.  Please confirm that SCCI is to use manufacturers installation instructions.						<b>ANSWER:</b>  Reference Specifications: 071210  We are in receipt of Laurencio Waterproofing Products, Inc.'s Installation Instructions revision dated 5/15/13 and have found several discrepancies with what is called out in Specifications Section 071210. Section 8, f, iv, 5, (d), v calls for a minimum (3) days wait for top ply to firmly adhere before starting the rest of flashing details and placing concrete topping slab. This is not called out in the specifications and may significantly impact the project schedule.  Please confirm that SCCI is to use manufacturers installation instructions.



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T-0573	BGP - Locations of Electrical Outlets, Equipment, and Fixtures	Closed	01	05/29/2013	06/08/2013	06/11/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:						ANSWER:
Reference Specification 26 05 34, 3.2 B.						Reference Specification 26 05 34, 3.2 B.
The exact locations of the electrical equipment are to be provided by the TJPA through the RFI process. With the electrical equipment provided and installed at a later date under a separate contract, please provide the dimensions of the electrical equipment, boxes, and cabinets to allow for accurate electrical riser locations in the concrete slabs. The equipment, boxes, and cabinet dimensions in Zone 1, Area 1 are needed first with the areas to the east to follow.						The exact locations of the electrical equipment are to be provided by the TJPA through the RFI process. With the electrical equipment provided and installed at a later date under a separate contract, please provide the dimensions of the electrical equipment, boxes, and cabinets to allow for accurate electrical riser locations in the concrete slabs. The equipment, boxes, and cabinet dimensions in Zone 1, Area 1 are needed first with the areas to the east to follow.
T-0574	BGP - Field Galvanizing of Mat Slab Sleeve Penetrations	Closed	01	05/31/2013	06/10/2013	06/09/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:						ANSWER:
Reference Specification Section 05 05 15-3.3.B						Reference Specification Section 05 05 15-3.3.B
The shop applied coating thickness for the pin and trestle pile sleeve fabrications is determined to be 3.9 mils per Table 2 in ASTM A 123. Under Section 05 05 15-3.5 the repair/restoration field-applied coating thickness is specified to be 8.0 mils. For field touch-up of damaged areas Section 05 50 10-3.2.D states to apply a thickness of 2.5 to 3.5 mils. For the coating hold back areas for the sleeve field weld joints and for any damage coatings that may arise during installation - is a uniform required minimum field-applied thickness of 3.9 mils acceptable?						The shop applied coating thickness for the pin and trestle pile sleeve fabrications is determined to be 3.9 mils per Table 2 in ASTM A 123. Under Section 05 05 15-3.5 the repair/restoration field-applied coating thickness is specified to be 8.0 mils. For field touch-up of damaged areas Section 05 50 10-3.2.D states to apply a thickness of 2.5 to 3.5 mils. For the coating hold back areas for the sleeve field weld joints and for any damage coatings that may arise during installation - is a uniform required minimum field-applied thickness of 3.9 mils acceptable?
T-0575	BSE - Micropile Relocation - E038 (Overhead Obstructions)	Closed	01	06/03/2013	06/13/2013	08/14/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:						ANSWER:
Reference : Attached Sketch						Reference : Attached Sketch
Micropile E038 as laid out cannot be installed due to an overhead strut obstruction. BBII recommends relocating E038 east 1. .						Micropile E038 as laid out cannot be installed due to an overhead strut obstruction. BBII recommends relocating E038 east 1. .
Please confirm this is acceptable.						Please confirm this is acceptable.



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T-0575.1	BSE - Micropile Relocation - E038 (Overhead Obstructions) Revised	Closed	01	06/04/2013	06/14/2013	06/08/2013
From: Webcor Construction LP                      Lynn Kowallis						
REQUEST:						ANSWER:
Reference : Attached Sketch						Reference : Attached Sketch
This RFI supersedes RFI 375. Micropile E038 as laid out cannot be installed due to an overhead strut obstruction. BBII now recommends relocating E038 east 3'4" to be in line with E037 and E039. See attached sketch.						This RFI supersedes RFI 375. Micropile E038 as laid out cannot be installed due to an overhead strut obstruction. BBII now recommends relocating E038 east 3'4" to be in line with E037 and E039. See attached sketch.
Please confirm this is acceptable.						Please confirm this is acceptable.
T-0576	Wall Alignment on Westside of Zone 1	Closed	01	05/31/2013	06/10/2013	06/11/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:						ANSWER:
Webcor is proposing to change the alignment of the Concrete Foundation wall on the west elevation along gridlines 1 & X1-1.						Webcor is proposing to change the alignment of the Concrete Foundation wall on the west elevation along gridlines 1 & X1-1.
The Concrete wall which runs along gridline 1 would be offset into the structure by 0.2656' (3-1/8") (proposed Face of concrete Foundation wall would now be 15-1/8" of gridline 1). Similarly along gridline X1-1 the wall would also be offset into the structure by 0.1575' (1 7/8") these offsets would enable the contract reinforcement to be installed without the need for further modifications to the reinforcement due to encroachment of the CDSM piles.						The Concrete wall which runs along gridline 1 would be offset into the structure by 0.2656' (3-1/8") (proposed Face of concrete Foundation wall would now be 15-1/8" of gridline 1). Similarly along gridline X1-1 the wall would also be offset into the structure by 0.1575' (1 7/8") these offsets would enable the contract reinforcement to be installed without the need for further modifications to the reinforcement due to encroachment of the CDSM piles.
See sketch SK-1 showing Cross section of concrete Foundation wall between CDSM piles 818 - 822 GL 1 in proposed revised location.						See sketch SK-1 showing Cross section of concrete Foundation wall between CDSM piles 818 - 822 GL 1 in proposed revised location.
Please confirm if this is acceptable.						Please confirm if this is acceptable.
T-0577	BGP - Internal Wall Discrepancies 002	Closed	01	06/03/2013	06/13/2013	06/03/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:						ANSWER:
Reference Drawing: B/A1-9217						Reference Drawing: B/A1-9217



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T-0578	<b>BGP - Micropile Relocation - W916 (Timber Pile Obstruction)</b>  <b>From:</b> Webcor Construction LP                      Robert Kjome  <b>REQUEST:</b>  Reference Drawing: attached.  Micropile W916 encountered a timber pile during installation. It was moved in the field and installed 1' west of plan location. This does not appear to impact geothermal piping.  Please confirm this is acceptable.	Closed	01	06/03/2013	06/13/2013	06/19/2013
						<b>ANSWER:</b>  Reference Drawing: attached.  Micropile W916 encountered a timber pile during installation. It was moved in the field and installed 1' west of plan location. This does not appear to impact geothermal piping.  Please confirm this is acceptable.









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<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: A1-9215, A1-9216, A1-9217			Ref: A1-9215, A1-9216, A1-9217			
Reference attached sketch and CD A1-9215, A1-9216, and A1-9217. Revision 1 of the noted drawings, dated 4/28/2013 , were used to generate this RFI. Elevation views, Detail A on noted CDs A1-9216 and A1-9217 depict discrepant details of the interior wall penetrations between GL 3 and 4.75.			Reference attached sketch and CD A1-9215, A1-9216, and A1-9217. Revision 1 of the noted drawings, dated 4/28/2013 , were used to generate this RFI. Elevation views, Detail A on noted CDs A1-9216 and A1-9217 depict discrepant details of the interior wall penetrations between GL 3 and 4.75.			
Please provide drawings with consistent details. If not able to provide such drawings, please specify which drawing details take precedence.			Please provide drawings with consistent details. If not able to provide such drawings, please specify which drawing details take precedence.			
<b>T-0582</b>	<b>BGP - Use of Laurencio Adhesive and Temporary Fasteners as Alternative</b>	<b>Closed</b>	<b>01</b>	<b>06/05/2013</b>	<b>06/15/2013</b>	<b>06/14/2013</b>
<hr/>						
From: Webcor Construction LP                      Lynn Kowallis						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: RFI #T-0563 and Submittal #TG0600-024			Ref: RFI #T-0563 and Submittal #TG0600-024			
Please reference the response to RFI #T -0563 and Submittal #TG0600-024. The response to Part 3 of the RFI is unclear. Is it the designer's intent to deem temporary fasteners unacceptable with or without the certifications? Or are the temporary fasteners acceptable with the certifications?			Please reference the response to RFI #T -0563 and Submittal #TG0600-024. The response to Part 3 of the RFI is unclear. Is it the designer's intent to deem temporary fasteners unacceptable with or without the certifications? Or are the temporary fasteners acceptable with the certifications?			
Please clarify.			Please clarify.			
Please note, the certifications were submitted and approved on 2/11/13 as part of Submittal Package #TG0600-024.			Please note, the certifications were submitted and approved on 2/11/13 as part of Submittal Package #TG0600-024.			



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<b>T-0583</b>	<b>BGP - BBII Monitoring Instruments/Piezometers</b>	<b>Closed</b>	<b>01</b>	<b>06/06/2013</b>	<b>06/06/2013</b>	<b>06/14/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Drawings: 6/A1-8711, 3/A1-8711  Per coordination discussions with WOJV (email attached), SCCI is to install BBII Piezometer sleeves per Detail 6 of A 1-8711. In this detail, the sleeve is to be blocked out of the Mat Slab and poured back at a later date contrary to the piezometer/monitoring instrument detail 3 on A1-8711. Is it acceptable to eliminate the blockout portion of detail 3/ A 1-8711 and pour the BBII piezometers into the mat slab? The ARUP piezometers will remain operational (per Detail 3/ A1-8711 ) after the mat slab has been poured to monitor the water table levels.						<b>ANSWER:</b>  Reference Drawings: 6/A1-8711, 3/A1-8711  Per coordination discussions with WOJV (email attached), SCCI is to install BBII Piezometer sleeves per Detail 6 of A 1-8711. In this detail, the sleeve is to be blocked out of the Mat Slab and poured back at a later date contrary to the piezometer/monitoring instrument detail 3 on A1-8711. Is it acceptable to eliminate the blockout portion of detail 3/ A 1-8711 and pour the BBII piezometers into the mat slab? The ARUP piezometers will remain operational (per Detail 3/ A1-8711 ) after the mat slab has been poured to monitor the water table levels.
<b>T-0583.1</b>	<b>BGP - Dewatering / Piezometer Clarification</b>	<b>Closed</b>	<b>01</b>	<b>07/11/2013</b>	<b>07/11/2013</b>	<b>07/23/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference RFI T-0583, RFI T-0602 Reference Drawing: A1-8711  In follow up to a phone conversation with George Metzger, detail 3/A1-8711 is to be used for all permanent instruments. Since the piezometers installed by BBII will be removed when the dewatering system is turned off, please confirm which waterproofing detail should be used.  Also, please confirm if any of Arup's instruments will not be permanent. If they are not permanent, please confirm which waterproofing detail should be used.						<b>ANSWER:</b>  Reference RFI T-0583, RFI T-0602 Reference Drawing: A1-8711  In follow up to a phone conversation with George Metzger, detail 3/A1-8711 is to be used for all permanent instruments. Since the piezometers installed by BBII will be removed when the dewatering system is turned off, please confirm which waterproofing detail should be used.  Also, please confirm if any of Arup's instruments will not be permanent. If they are not permanent, please confirm which waterproofing detail should be used.
<b>T-0584</b>	<b>BGP - Dewatering Well and Concrete Wall Conflict</b>	<b>Closed</b>	<b>01</b>	<b>06/05/2013</b>	<b>06/15/2013</b>	<b>06/17/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Please reference the attached drawings S 103.0 and S 104.0 from SCCI's Rev it model. Based on BBII's latest as-built AutoCAD file ("20 13-05-01 BBII Dewatering Well Coordinates.dwg") and SCCI field measurements, it						<b>ANSWER:</b>  Please reference the attached drawings S 103.0 and S 104.0 from SCCI's Rev it model. Based on BBII's latest as-built AutoCAD file ("20 13-05-01 BBII Dewatering Well Coordinates.dwg") and SCCI field



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>appears that de-watering wells #1, #3, #21 and #22 conflict with the concrete partition walls and shear wall.</p> <p>Additional information below:</p> <p>1. BBIs dewatering well layout submittal was approved MCN 5/10/2011</p> <p>2. Dewatering wells in conflict rough installation dates: Dewatering Well # 1 - 1/18/2012 Dewatering Well # 3 - 1/19/2012 Dewatering Well # 21 - 1/24/2012 Dewatering Well # 22 - 1/23/2012</p> <p>3. A1-2122 was issued for construction 8-30-2012 placing concrete walls in conflict with previously approved and installed dewatering wells.</p> <p>Can these walls be blocked out at these locations? Can these walls be moved? Will the dewatering wells need to be relocated (difficult as the mud slab has been already poured in these locations)? Please advise as to how to proceed.</p>					
	<p>measurements, it appears that de-watering wells #1, #3, #21 and #22 conflict with the concrete partition walls and shear wall.</p> <p>Additional information below:</p> <p>1. BBIs dewatering well layout submittal was approved MCN 5/10/2011</p> <p>2. Dewatering wells in conflict rough installation dates: Dewatering Well # 1 - 1/18/2012 Dewatering Well # 3 - 1/19/2012 Dewatering Well # 21 - 1/24/2012 Dewatering Well # 22 - 1/23/2012</p> <p>3. A1-2122 was issued for construction 8-30-2012 placing concrete walls in conflict with previously approved and installed dewatering wells.</p> <p>Can these walls be blocked out at these locations? Can these walls be moved? Will the dewatering wells need to be relocated (difficult as the mud slab has been already poured in these locations)? Please advise as to how to proceed.</p>					
<b>T-0584.1</b>	<b>BSE - Abandoning Dewatering Well #3 at Shearwall</b>	<b>Closed</b>	<b>01</b>	<b>07/25/2013</b>	<b>08/04/2013</b>	<b>07/26/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						
Reference RFI: T-0584 Reference SK-001 and SK-002						
BBI is proposing to abandon the dewatering well #3 at the mudslab level. This will mitigate the impact of shear wall dowels and penetration sleeve in the vicinity of the mat slab penetration created by dewatering well #3, in lieu of creating a blockout in the shear wall and designing rebracing to address structural concerns.						
BBI has discussed this with Viking Drillers and have confirmed they can abandon this well. BBI is proposing to cut the pvc casing flush with the top of mudslab, drill and						
<b>ANSWER:</b>						
Reference RFI: T-0584 Reference SK-001 and SK-002						
BBI is proposing to abandon the dewatering well #3 at the mudslab level. This will mitigate the impact of shear wall dowels and penetration sleeve in the vicinity of the mat slab penetration created by dewatering well #3, in lieu of creating a blockout in the shear wall and designing rebracing to address structural concerns.						
BBI has discussed this with Viking Drillers and have confirmed they can abandon this well. BBI is						



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	<p>epoxy #4 bars 2" down from top of casing with 3" embedment. The bars will be installed in the north, south, east, and west face through the casing and are installed to better ensure the dewatering well cement plug does not upheave. They will use Type II Portland Cement with a 5% bentonite content. Waterproofing will then be installed over the dewatering well, lapping as necessary to the adjacent waterproofing.</p> <p>Please confirm that this is acceptable.</p>					<p>proposing to cut the pvc casing flush with the top of mudslab, drill and epoxy #4 bars 2" down from top of casing with 3" embedment. The bars will be installed in the north, south, east, and west face through the casing and are installed to better ensure the dewatering well cement plug does not upheave. They will use Type II Portland Cement with a 5% bentonite content. Waterproofing will then be installed over the dewatering well, lapping as necessary to the adjacent waterproofing.</p> <p>Please confirm that this is acceptable.</p>
T-0584.2	BGP - Dewatering Well & Concrete Partition Conflict	Closed	01	07/30/2013	08/09/2013	08/08/2013
From: Webcor Construction LP Ryan Burke						
REQUEST:						ANSWER:
Reference: RFI T-0584, Attached sketch						Reference: RFI T-0584, Attached sketch
Please refer to RFI 584 and the attached sketch of the proposed block out in concrete partition walls as referenced in RFI 584. The 28" x 28" blockout in the mat slab will be transferred to the blockout of the wall and be 25" from the mat slab elevation to the top of blockout. This will create 3'-0" from top of penetration sleeve to top of wall blockout. We are proposing to use formsavers and the male ends will extend the length of the blockout.						Please refer to RFI 584 and the attached sketch of the proposed block out in concrete partition walls as referenced in RFI 584. The 28" x 28" blockout in the mat slab will be transferred to the blockout of the wall and be 25" from the mat slab elevation to the top of blockout. This will create 3'-0" from top of penetration sleeve to top of wall blockout. We are proposing to use formsavers and the male ends will extend the length of the blockout.
Please confirm this is acceptable or provide acceptable solution.						Please confirm this is acceptable or provide acceptable solution.



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<b>T-0585</b>	<b>BGP - Mass Concrete Specifications</b>	<b>Closed</b>	<b>01</b>	<b>06/05/2013</b>	<b>06/05/2013</b>	<b>06/13/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Specifications Section: 03 30 20 3.5 & 3.11 Reference attached letter from CTL Group		Specifications Section: 03 30 20 3.5 & 3.11 Reference attached letter from CTL Group				
SCCI is asking for variance to the temperature differential requirements for the mat slab concrete. If granted, this variance would be based on performance based temperature differential limit (PBTDL), which is tailored to both the Project's mass concrete mix design and the placement. Refer to the attached letter from CTL. The intent of this PBTDL is to prevent thermal cracking, and at the same time reduce duration of the thermal control requirement.		SCCI is asking for variance to the temperature differential requirements for the mat slab concrete. If granted, this variance would be based on performance based temperature differential limit (PBTDL), which is tailored to both the Project's mass concrete mix design and the placement. Refer to the attached letter from CTL. The intent of this PBTDL is to prevent thermal cracking, and at the same time reduce duration of the thermal control requirement.				
Is this acceptable?		Is this acceptable?				
<b>T-0586</b>	<b>BGP - Fire Management System and Concourse Slab Electrical Scope</b>	<b>Closed</b>	<b>01</b>	<b>06/05/2013</b>	<b>06/05/2013</b>	<b>06/17/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Drawing: E-0006, E-0000, 6/E-2202 Reference RFI: T-0567		Reference Drawing: E-0006, E-0000, 6/E-2202 Reference RFI: T-0567				
Per the response to RFI T-567, the fire management system conduit is to be installed into the concourse slab per Note on Sheet E-0006 . However, per Plan Sheet E-0000, only a small grounding portion of electrical drawings are in the TG06.0 concourse slab scope. The remaining concourse level electrical drawings are "For Reference Only" and for informational purposes only. Please confirm that the only TG06.0 electrical scopes in the concourse slab are the grounding wire extensions from the mud slab (per Detail 6/E 2202-TG06.2 scope), lighting conduit and boxes for Type "F15" and Exit Signs, and 4" 90 degree elbows per Details 1 & 2 on TE 1-8000. Please confirm that outside of those scopes, all other electrical scopes of work in the concourse slab are to be part of a later electrical scope package as indicated on the E-0000 index and the "for information only" plans.		Per the response to RFI T-567, the fire management system conduit is to be installed into the concourse slab per Note on Sheet E-0006 . However, per Plan Sheet E-0000, only a small grounding portion of electrical drawings are in the TG06.0 concourse slab scope. The remaining concourse level electrical drawings are "For Reference Only" and for informational purposes only. Please confirm that the only TG06.0 electrical scopes in the concourse slab are the grounding wire extensions from the mud slab (per Detail 6/E 2202-TG06.2 scope), lighting conduit and boxes for Type "F15" and Exit Signs, and 4" 90 degree elbows per Details 1 & 2 on TE 1-8000. Please confirm that outside of those scopes, all other electrical scopes of work in the concourse slab are to be part of a later electrical scope package as indicated on the E-0000 index and the "for information only" plans.				



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<b>T-0586.1</b>	<b>BGP - Fire Management System in the Concourse Slab Only</b>	<b>Closed</b>	<b>01</b>	<b>07/12/2013</b>	<b>07/22/2013</b>	<b>07/19/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Per the response to RFI T-567 (attached), please confirm that the only slab with embedded fire system conduit is the concourse slab. All stub ups or risers will either come up out of the concourse slab for the concourse level fire management system or drop down out of the concourse slab for the fire management system on the train platform level. Please confirm that the fire management system is not embedded in the mat slab.		<b>ANSWER:</b>  Per the response to RFI T-567 (attached), please confirm that the only slab with embedded fire system conduit is the concourse slab. All stub ups or risers will either come up out of the concourse slab for the concourse level fire management system or drop down out of the concourse slab for the fire management system on the train platform level. Please confirm that the fire management system is not embedded in the mat slab.				
<b>T-0587</b>	<b>BGP - Future Train Platform Wall Reinforcing Size and Spacing</b>	<b>Closed</b>	<b>01</b>	<b>06/05/2013</b>	<b>06/15/2013</b>	<b>06/16/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference RFI:T-0480  The contractor is to construct the future train platform walls using the thickness of the wall as called out within the specific zone sheet (1'-0" or 1'-2"). When coordinating the wall thickness called out in the Plan with Detail 5 on S1-3205 Future Wall Detail no specific bar size or spacing is called out for the 1'-2" thick walls. Please confirm if the reinforcing required for the 1'-2" walls is the same as that called out for a 12" wall, #6 @ 8" oc.		<b>ANSWER:</b>  Reference RFI:T-0480  The contractor is to construct the future train platform walls using the thickness of the wall as called out within the specific zone sheet (1'-0" or 1'-2"). When coordinating the wall thickness called out in the Plan with Detail 5 on S1-3205 Future Wall Detail no specific bar size or spacing is called out for the 1'-2" thick walls. Please confirm if the reinforcing required for the 1'-2" walls is the same as that called out for a 12" wall, #6 @ 8" oc.				
<b>T-0588</b>	<b>BGP - Future Partition Wall Dowel Size Spacing</b>	<b>Closed</b>	<b>01</b>	<b>06/06/2013</b>	<b>06/06/2013</b>	<b>06/10/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Contract drawing S1-2052 depicts 12" Partition Walls and 12" Future Partition Walls. Contract drawing S 1-9050 provides the reinforcing details for the Partition Walls which depict #7 @ 12" OC reinforcing dowels for a 12" wall. Per S 1-3205 Future 12" Walls receive #6 @8" OC reinforcing dowels. Please confirm the proper bar size for the Future Partition Wall dowels.		<b>ANSWER:</b>  Contract drawing S1-2052 depicts 12" Partition Walls and 12" Future Partition Walls. Contract drawing S 1-9050 provides the reinforcing details for the Partition Walls which depict #7 @ 12" OC reinforcing dowels for a 12" wall. Per S 1-3205 Future 12" Walls receive #6 @8" OC reinforcing dowels. Please confirm the proper bar size for the Future Partition Wall dowels.				





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<b>T-0589</b>	<b>BGP - Epoxy Coating Thickness Over Formsaver Couplers</b>	<b>Closed</b>	<b>01</b>	<b>06/06/2013</b>	<b>06/16/2013</b>	<b>06/17/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: 6/S1-3001, Attached Letter		Reference: 6/S1-3001, Attached Letter				
The response to RFI T 0515 confirmed to coat the form saver couplers for future construction as specified in ASTM A 775. Per ASTM A 775, the standard coating thickness specifies a required thickness range by which different size bars are to be coated 7 to 12 mills for bar sizes 3 to 5 and 7 to 16 mills for bar sizes 6 to 18; however, detail 6 on S1-3001 indicates a 12 mill minimum coating thickness over the couplers. Per the attached letter from Stanley Johnson the Regional Manager for Erico (Lenton) the epoxy coated form-saver couplers specified for use cannot be procured with a guaranteed 12 mill coating but rather an epoxy coating that meets the requirements of the ASTM A 775 standard. Please confirm that supplying an epoxy coated form-saver coupler that meets the ASTM A 775 standard but may contain a mill thickness less than 12 is acceptable.		The response to RFI T 0515 confirmed to coat the form saver couplers for future construction as specified in ASTM A 775. Per ASTM A 775, the standard coating thickness specifies a required thickness range by which different size bars are to be coated 7 to 12 mills for bar sizes 3 to 5 and 7 to 16 mills for bar sizes 6 to 18; however, detail 6 on S1-3001 indicates a 12 mill minimum coating thickness over the couplers. Per the attached letter from Stanley Johnson the Regional Manager for Erico (Lenton) the epoxy coated form-saver couplers specified for use cannot be procured with a guaranteed 12 mill coating but rather an epoxy coating that meets the requirements of the ASTM A 775 standard. Please confirm that supplying an epoxy coated form-saver coupler that meets the ASTM A 775 standard but may contain a mill thickness less than 12 is acceptable.				
<b>T-0590</b>	<b>BGP - Mechanical Room Plumbing Clarifications 002</b>	<b>Closed</b>	<b>01</b>	<b>06/06/2013</b>	<b>06/16/2013</b>	<b>06/12/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Attached Drawing, P1-2022, Spec Section 22 13 01		Reference: Attached Drawing, P1-2022, Spec Section 22 13 01				
Reference attached mechanical room layout drawing P-112. Per the marked up referenced drawing please clarify or provide following: 1. Invert elevations of the piping connecting the sumps. 2. Verify dimensions of the pipes spacing and offsets, per attachment. 3. Size and locations of the equipment pad.		Reference attached mechanical room layout drawing P-112. Per the marked up referenced drawing please clarify or provide following: 1. Invert elevations of the piping connecting the sumps. 2. Verify dimensions of the pipes spacing and offsets, per attachment. 3. Size and locations of the equipment pad.				
<b>T-0591</b>	<b>BGP - Mechanical Room Plumbing Clarifications 001</b>	<b>Closed</b>	<b>01</b>	<b>06/06/2013</b>	<b>06/06/2013</b>	<b>06/11/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Attached Drawing, P1-2022, Spec Section 22		Reference: Attached Drawing, P1-2022, Spec Section				





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13 01	Reference attached drainage layout drawing P-110. Please verify marked up dimensions for the pipe spacing.		22 13 01			Reference attached drainage layout drawing P-110. Please verify marked up dimensions for the pipe spacing.
<b>T-0592</b>	<b>BGP - Mechanical Room Plumbing Clarifications 003</b>	<b>Closed</b>	<b>01</b>	<b>06/06/2013</b>	<b>06/16/2013</b>	<b>06/12/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Attached Drawing, P1-2022, Spec Section 22 13 01			Reference: Attached Drawing, P1-2022, Spec Section 22 13 01			
Reference attached drainage drawing P-113. Please verify marked up dimensions for pipes spacing.			Reference attached drainage drawing P-113. Please verify marked up dimensions for pipes spacing.			
<b>T-0593</b>	<b>BGP - Concrete Clear Cover of Reinforcing Support Bars</b>	<b>Closed</b>	<b>01</b>	<b>06/06/2013</b>	<b>06/16/2013</b>	<b>06/11/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Drawing 5/S1-3001, Spec Section 03 30 20			Reference: Drawing 5/S1-3001, Spec Section 03 30 20			
Gerdau would like to confirm that non-contract reinforcing support (carry) bars are to maintain the required concrete clear cover as specified in detail 5 on S 1-3001 and not encroach upon the designated clear cover limits. See the attached sketch for reference.			Gerdau would like to confirm that non-contract reinforcing support (carry) bars are to maintain the required concrete clear cover as specified in detail 5 on S 1-3001 and not encroach upon the designated clear cover limits. See the attached sketch for reference.			



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<b>T-0594</b>	<b>SSS - Pendulum Bearing Specification</b>	<b>Closed</b>	<b>01</b>	<b>06/07/2013</b>	<b>06/17/2013</b>	<b>06/14/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Specification: 03 20 02 2.6  Spec Section 03 20 02 was issued to W/O on 4/26/13 as part of the TG07.1 IFB set dated 2/19/13 to be issued to existing W/O subcontractors for construction. 03 20 02 2.6 includes Pendulum Bearings . Please provide drawings and details depicting the location and quantity of Pendulum Bearings required.  Please also confirm any placement and attachment details for pendulum bearings and structure.						<b>ANSWER:</b>  Reference Specification: 03 20 02 2.6  Spec Section 03 20 02 was issued to W/O on 4/26/13 as part of the TG07.1 IFB set dated 2/19/13 to be issued to existing W/O subcontractors for construction. 03 20 02 2.6 includes Pendulum Bearings . Please provide drawings and details depicting the location and quantity of Pendulum Bearings required.  Please also confirm any placement and attachment details for pendulum bearings and structure.
<b>T-0595</b>	<b>Geothermal Piping Under Construction Personnel Hoist Concrete Pad</b>	<b>Closed</b>	<b>01</b>	<b>06/10/2013</b>	<b>06/20/2013</b>	<b>06/11/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference: Attached Drawings  Please see the attached drawing of the proposed manlift pad to be installed flushed with the mudslab in Zone 2. Per WSP/Flack & Kurtz the dimension of soil between mudslab and top of geothermal pipe must be maintained at all times. It was stated that the geothermal piping could be installed 12" deeper as long as the rise of the pipe follow the radius loop bend requirments, in the method that the geothermal is installed in the sump pits. Please confirm that this is acceptable.						<b>ANSWER:</b>  Reference: Attached Drawings  Please see the attached drawing of the proposed manlift pad to be installed flushed with the mudslab in Zone 2. Per WSP/Flack & Kurtz the dimension of soil between mudslab and top of geothermal pipe must be maintained at all times. It was stated that the geothermal piping could be installed 12" deeper as long as the rise of the pipe follow the radius loop bend requirments, in the method that the geothermal is installed in the sump pits. Please confirm that this is acceptable.
<b>T-0596</b>	<b>BGP - Sump Pit Grate and Frame at Gridline 19/C</b>	<b>Closed</b>	<b>CR</b>	<b>06/11/2013</b>	<b>06/21/2013</b>	<b>06/20/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  See attached drawing CB-2 of returned submittal package TG0600-710, P1-2025, and A1-2125.  The returned (returned to WOJV/SCCI on 06/07/13) shop drawing submittal (TG0600-710) for catch basin and sump pit grating indicates an additional sump pit grate and frame						<b>ANSWER:</b>  See attached drawing CB-2 of returned submittal package TG0600-710, P1-2025, and A1-2125.  The returned (returned to WOJV/SCCI on 06/07/13) shop drawing submittal (TG0600-710) for catch basin and sump pit grating indicates an additional sump pit



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	<p>at approx. GL 19/C per drawing CB-2. The contract drawing P1-2025 does not have a call-out for a grate and frame at this location. Furthermore, drawing A1-2125 has the sump pit located within an escalator pit in the mat slab level. Per Field Order T-00011, all escalator pits do not receive grates or frames.</p> <p>Please confirm the sump pit at GL 19/C does not have a grate and frame. An expedited response is requested in order to release the full order of frames and grates in a timely manner.</p>					
T-0597	<p><b>BGP - Concourse Deck Capacity for Construction Loads</b></p> <p>From: Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Reference: Attached Documents</p> <p>Please confirm it is acceptable to use a Sky Trak 8042 forklift with an approximate operating weight of 25,365 lbs and rated load capacity of 6,000 lbs on the concourse level deck without temporary shoring in place. The forklift is intended for use on the concourse level deck for the installation of wall reinforcing steel. Should this weight exceed the capacity of the structure please advise as to the structure's load capacities without temporary shoring in place for alternate equipment selection and planning.</p>	Closed	01	06/11/2013	06/21/2013	06/12/2013
	<p><b>ANSWER:</b></p> <p>Reference: Attached Documents</p> <p>Please confirm it is acceptable to use a Sky Trak 8042 forklift with an approximate operating weight of 25,365 lbs and rated load capacity of 6,000 lbs on the concourse level deck without temporary shoring in place. The forklift is intended for use on the concourse level deck for the installation of wall reinforcing steel. Should this weight exceed the capacity of the structure please advise as to the structure's load capacities without temporary shoring in place for alternate equipment selection and planning.</p>					



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<b>T-0597.1</b>	<b>BGP - Concourse Deck Capacity for Construction Loads</b>	<b>Closed</b>	<b>01</b>	<b>06/28/2013</b>	<b>07/08/2013</b>	<b>07/09/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Per response to SCCI's RFI 215 (T-0597) see attached axle loadings for Sky Trak forklift model no. 8042. The forklift is intended for use on the concourse level deck for the installation of wall reinforcing steel.  Please confirm if it is acceptable to use noted forklift on top of concourse slab.						<b>ANSWER:</b>  Per response to SCCI's RFI 215 (T-0597) see attached axle loadings for Sky Trak forklift model no. 8042. The forklift is intended for use on the concourse level deck for the installation of wall reinforcing steel.  Please confirm if it is acceptable to use noted forklift on top of concourse slab.
<b>T-0598</b>	<b>BGP - Fire Management System Class A vs. Class B</b>	<b>Closed</b>	<b>01</b>	<b>06/12/2013</b>	<b>06/22/2013</b>	<b>06/15/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  Reference: Attached Documents, E1-5201  After consulting with Siemens on the fire management system a clarification is needed. The riser diagram on sheet EI-5201 shows Class A conduit routing for the train platform level and the lower concourse level. Using a Class A wiring layout limits the system to 3 or 4 strobe devices per circuit. Under the NFPA 130 6.3.3.2.8 specification, the embedded (note (1) of the specification) fire management conduit protects against the ASTM E119 fire conditions and Class A isn't required per NFP A specification. Is it acceptable to design the fire management conduit system to meet the NFP A 130 specification under Class B requirements and impliment 6 or 7 strobe devices per circuit instead of the 3 or 4 stobe devices per Class A. By implimenting a Class B system, the future fire management system (installed under a future contract) will be less costly all while meeting the the NFP A 130 requirements. Please advise.						<b>ANSWER:</b>  Reference: Attached Documents, E1-5201  After consulting with Siemens on the fire management system a clarification is needed. The riser diagram on sheet EI-5201 shows Class A conduit routing for the train platform level and the lower concourse level. Using a Class A wiring layout limits the system to 3 or 4 strobe devices per circuit. Under the NFPA 130 6.3.3.2.8 specification, the embedded (note (1) of the specification) fire management conduit protects against the ASTM E119 fire conditions and Class A isn't required per NFP A specification. Is it acceptable to design the fire management conduit system to meet the NFP A 130 specification under Class B requirements and impliment 6 or 7 strobe devices per circuit instead of the 3 or 4 stobe devices per Class A. By implimenting a Class B system, the future fire management system (installed under a future contract) will be less costly all while meeting the the NFP A 130 requirements. Please advise.
<b>T-0599</b>	<b>BGP - Continuous Concrete Insert Elevations</b>	<b>Closed</b>	<b>01</b>	<b>06/13/2013</b>	<b>06/23/2013</b>	<b>06/21/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Reference: Attached Documents, Drawing A1-6231						<b>ANSWER:</b>  Reference: Attached Documents, Drawing A1-6231



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	<p>Please reference the attached - clouded, Submittal TG0600- 110 BGP -Concrete Formwork Lift #1 sheet, comment regarding the elevation of the cast-in-place continuous concrete insert. The submittal comment requests an adjustment of the concrete insert elevations. In the attached RFI T-0506 the elevations of the concrete inserts were given to accomplish equal spacing as required by the drawings, as well as incorporate the agreed upon adjustments to the top and bottom insert. SCCI would like to verify that the given elevations of the cast-in members in the clouded section of RFI T-0506 are the correct elevations.</p>					
T-0599.1	<p><b>BGP - Horizontal Cast-In Inserts at EFCO Form Panels</b></p> <p><b>From:</b> Webcor Construction LP                      Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>See attached photo and reference RFI 599.</p> <p>Interior rib of the EFCO form panels lines up with the cast-in insert at EL -27.08'. SCCI intention is to bolt the inserts to the forms and this makes it difficult to properly secure the cast-in insert prior to concrete placement. SCCI proposes to lower or raise this insert 2" in order to properly secure it to the form.</p> <p>Is this acceptable?</p>	Closed	CR	11/19/2013	11/29/2013	11/20/2013



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T-0601	BGP - Internal Wall Discrepancies 004	Closed	01	06/17/2013	06/27/2013	06/24/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:		ANSWER:				
Reference attached marked up CD A 1-9217 detail E.		Reference attached marked up CD A 1-9217 detail E.				
Referenced detail shows openings in the wall near GL5 and GL D. This opening appears to be in conflict with the moment beam that runs along GL D. Please clarify.		Referenced detail shows openings in the wall near GL5 and GL D. This opening appears to be in conflict with the moment beam that runs along GL D. Please clarify.				
T-0602	Arup Monitoring Instruments	Closed	01	07/02/2013	07/12/2013	07/17/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference Drawings: 3/A1-8711 and 6/A1-8711		Reference Drawings: 3/A1-8711 and 6/A1-8711				
BBII's dewatering wells and piezometers are installed per Detail 6 on Sheet A1-8711 which clearly shows how the wells and piezometers will be filled and capped after the dewatering has been decommissioned. Detail 3 on Sheet A1-8711 does not provide any indication that these piezometers will be plugged and/or filled. Does the design team intend on leaving these piezometer holes open after the dewatering is shut off? If not, please provide a revised 3/A1-8711 clarifying the design teams intent.		BBII's dewatering wells and piezometers are installed per Detail 6 on Sheet A1-8711 which clearly shows how the wells and piezometers will be filled and capped after the dewatering has been decommissioned. Detail 3 on Sheet A1-8711 does not provide any indication that these piezometers will be plugged and/or filled. Does the design team intend on leaving these piezometer holes open after the dewatering is shut off? If not, please provide a revised 3/A1-8711 clarifying the design teams intent.				
T-0603	BSE - Beale PG&E Utilities	Closed	01	06/19/2013	06/29/2013	07/01/2013
From: Webcor Construction LP Lynn Kowallis						
REQUEST:		ANSWER:				
Refer RFI T-0286 Specification Section 01 53 13		Refer RFI T-0286 Specification Section 01 53 13				
Please reference W/O RFI T-0286. For First and Fremont street BBII was directed to use a cable weight of 8.2 lb/ft to be used with the 6" conduit. BBII was supplied with a weight of 3 lb/ft for fiber cable used in 4" conduit (not PG&E conduit). BBII does not have a cable weight for 4" PG&E conduit.		Please reference W/O RFI T-0286. For First and Fremont street BBII was directed to use a cable weight of 8.2 lb/ft to be used with the 6" conduit. BBII was supplied with a weight of 3 lb/ft for fiber cable used in 4" conduit (not PG&E conduit). BBII does not have a cable weight for 4" PG&E conduit.				
1. Please confirm that the 6" PG&E conduit on Beale Street will contain a 8.2 lb/ft cable. 2. Please clarify the weight/ft of the cable used in the 4"		1. Please confirm that the 6" PG&E conduit on Beale Street will contain a 8.2 lb/ft cable. 2. Please clarify the weight/ft of the cable used in the				



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	<p>PG&amp;E conduit on Beale Street.</p> <p>This information is necessary to design the utility supports on the Beale street Bridge.</p>					
						<p>4" PG&amp;E conduit on Beale Street.</p> <p>This information is necessary to design the utility supports on the Beale street Bridge.</p>
<b>T-0604</b>	<b>#2 CPH Platform through Mat Slab in Zone 2</b>	<b>Closed</b>	<b>01</b>	<b>06/20/2013</b>	<b>06/30/2013</b>	<b>07/28/2013</b>
	<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Please see the attached shop drawings and layout of the construction personnel hoist (CPH) to be installed in Zone 2. The CPH elevated steel PLATFORM to be installed and later removed and poured back such as the trestle pile penetrations.</p> <p>All work dimensions have been coordinated with structure overhead into future bid packages as well as as-built information of internal bracing in the field.</p> <p>We propose to :</p> <p>1) Lower the geothermal piping an additional 12" to maintain the same 15" deep trench under all concrete. This will be performed the same way they install the piping in the sump pits with correct bend radius.</p> <p>2) Install at 19'-6" x 13'-0" x 16" thickened slab incorporated with the current 4" reinforced mudslab. The thickened slab will contain #5 bars 12" OC EW T&amp;B and we have confirmed that the total load of thickened slab, CPH, and platform will not exceed 500 PSF.</p> <p>3) Install CPH elevated steel platform through the mat slab with 3'-0" of clearance between top of mat slab and bottom of platform deck and beams.</p> <p>4) Waterproof platform legs per detail 5/A1-8711 04/29/13 per ASI 0102 Issued for Construction, Below Grade Package, including galvanized penetration sleeves and waterproofing. Penetration sleeve will be 30" diameter.</p> <p>5) Reinforcing details will be the same as all other reinforcing at pin/trestle pile blockouts.</p>				<p><b>ANSWER:</b></p> <p>Please see the attached shop drawings and layout of the construction personnel hoist (CPH) to be installed in Zone 2. The CPH elevated steel PLATFORM to be installed and later removed and poured back such as the trestle pile penetrations.</p> <p>All work dimensions have been coordinated with structure overhead into future bid packages as well as as-built information of internal bracing in the field.</p> <p>We propose to :</p> <p>1) Lower the geothermal piping an additional 12" to maintain the same 15" deep trench under all concrete. This will be performed the same way they install the piping in the sump pits with correct bend radius.</p> <p>2) Install at 19'-6" x 13'-0" x 16" thickened slab incorporated with the current 4" reinforced mudslab. The thickened slab will contain #5 bars 12" OC EW T&amp;B and we have confirmed that the total load of thickened slab, CPH, and platform will not exceed 500 PSF.</p> <p>3) Install CPH elevated steel platform through the mat slab with 3'-0" of clearance between top of mat slab and bottom of platform deck and beams.</p> <p>4) Waterproof platform legs per detail 5/A1-8711 04/29/13 per ASI 0102 Issued for Construction, Below Grade Package, including galvanized penetration sleeves and waterproofing. Penetration sleeve will be 30" diameter.</p> <p>5) Reinforcing details will be the same as all other</p>	

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-0605	BGP - Plumbing and Electrical Autocad Files	Closed	01	06/21/2013	07/01/2013	06/27/2013
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b> SCCI requesting access to the latest, most up to date Auto cad files for the Plumbing (P1-series) and Electrical (E1-series) drawings from the designers. The files would be used for Reference only and will not be used for construction. SCCI understands that the Autocad files are subject to change as the project design evolves.		<b>ANSWER:</b> SCCI requesting access to the latest, most up to date Auto cad files for the Plumbing (P1-series) and Electrical (E1-series) drawings from the designers. The files would be used for Reference only and will not be used for construction. SCCI understands that the Autocad files are subject to change as the project design evolves.				





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0606</b>	<b>BGP - Mat Slab Pour and Bracing Removal- Area 1 to 4</b>	<b>Closed</b>	<b>01</b>	<b>06/21/2013</b>	<b>07/01/2013</b>	<b>06/28/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Spec Section 01 13 00			Reference: Spec Section 01 13 00			
<p>The latest Webcor's weekly update schedule received by SCCI (Data date 06.17.2013), shows that Balfour Beatty (BBII)'s activity "Bracing Removal- Level D- BBII- Z1 A1", in Zone 1, area 1 cannot commence until the completion of Webcor's activity "Mat Slab Cure- Z1A1 ".The same relationship exists between the two activities for Area 1 to Area 4. Preliminary rough analysis done by SCCI suggests that there is not sufficient sliding resistance to permit the slab in each area to act as effective support for the base of the shoring wall when the lowest level of bracing is removed in that area. The preliminary analysis also suggests that bracing removal level D in Area 1-4 should not commence until the entire mat slab in Area 1-4 are in place.</p> <p>Please confirm that :</p> <ol style="list-style-type: none"><li>1. Webcor has performed a detailed analysis that the relationship as shown in the schedule between the Bracing Removal- Level D and Mat Slab Cure can be performed in each area, independent of any other areas.</li><li>2. SW Comer bracing level D could be removed if only Areas 1 &amp;2 are poured and cured</li><li>3. NW Comer bracing at Level 2 could be removed if only Areas 3&amp;4 are poured and cured</li></ol>			<p>The latest Webcor's weekly update schedule received by SCCI (Data date 06.17.2013), shows that Balfour Beatty (BBII)'s activity "Bracing Removal- Level D- BBII- Z1 A1", in Zone 1, area 1 cannot commence until the completion of Webcor's activity "Mat Slab Cure- Z1A1 ".The same relationship exists between the two activities for Area 1 to Area 4. Preliminary rough analysis done by SCCI suggests that there is not sufficient sliding resistance to permit the slab in each area to act as effective support for the base of the shoring wall when the lowest level of bracing is removed in that area. The preliminary analysis also suggests that bracing removal level D in Area 1-4 should not commence until the entire mat slab in Area 1-4 are in place.</p> <p>Please confirm that :</p> <ol style="list-style-type: none"><li>1. Webcor has performed a detailed analysis that the relationship as shown in the schedule between the Bracing Removal- Level D and Mat Slab Cure can be performed in each area, independent of any other areas.</li><li>2. SW Comer bracing level D could be removed if only Areas 1 &amp;2 are poured and cured</li><li>3. NW Comer bracing at Level 2 could be removed if only Areas 3&amp;4 are poured and cured</li></ol>			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0607</b>	<b>BGP - Bracing Removal Sequence- Area 5-16</b>	<b>Closed</b>	<b>01</b>	<b>06/21/2013</b>	<b>07/01/2013</b>	<b>06/28/2013</b>
<div> <div> <b>From:</b> Shimmick Construction Company, Inc. Ben Gordon </div> <div> <b>REQUEST:</b>  <p>The latest Webcor's weekly update schedule received by SCCI (Data date 06.17.2013), shows that:</p> <ul style="list-style-type: none"> <li>* "Bracing Removal- Level D" (BGSOX-1120) is the driving predecessor to "Wall Waterproofing- 1st lift" (BGSOX-4000)- in each area.</li> <li>* "Bracing Removal- Level E" (BGSOX-41 00) is the predecessor to "Wall Waterproofing- 2nd lift" (BGSOX -4110)- in each area</li> <li>* "Bracing Removal- Level B" (BGSOX-6000) is the predecessor to "Wall Waterproofing- 3rd lift" (BGSOX -6010) in each area</li> </ul> <p>Based on the current schedule logic, the bracing will need to be modified to allow the removal of walers and struts in each area, separately and independently from each other. E.g: Any walers spanning two areas will need to be cut during removal of bracing so seer can proceed with the waterproofing install in that area, without having to wait for the adjacent area. This is applicable to Bracing Removal level B, C and D. Please confirm.</p> </div> <div> <b>ANSWER:</b>  <p>The latest Webcor's weekly update schedule received by SCCI (Data date 06.17.2013), shows that:</p> <ul style="list-style-type: none"> <li>* "Bracing Removal- Level D" (BGSOX-1120) is the driving predecessor to "Wall Waterproofing- 1st lift" (BGSOX-4000)- in each area.</li> <li>* "Bracing Removal- Level E" (BGSOX-41 00) is the predecessor to "Wall Waterproofing- 2nd lift" (BGSOX -4110)- in each area</li> <li>* "Bracing Removal- Level B" (BGSOX-6000) is the predecessor to "Wall Waterproofing- 3rd lift" (BGSOX -6010) in each area</li> </ul> <p>Based on the current schedule logic, the bracing will need to be modified to allow the removal of walers and struts in each area, separately and independently from each other. E.g: Any walers spanning two areas will need to be cut during removal of bracing so seer can proceed with the waterproofing install in that area, without having to wait for the adjacent area. This is applicable to Bracing Removal level B, C and D. Please confirm.</p> </div> </div>						
<b>T-0608</b>	<b>Detail of transition between modified reinforcement to contract reinforcement</b>	<b>Closed</b>	<b>01</b>	<b>06/26/2013</b>	<b>07/06/2013</b>	<b>07/28/2013</b>
<div> <div> <b>From:</b> Webcor Construction LP                      Michael Spillane </div> <div> <b>REQUEST:</b>  <p>Reference Documents: Exhibits A - C, RFI SCI# 236</p> <p>This RFI addresses the transition between modified reinforcement to contract reinforcement at GL6 at the south west corner see Location Plan exhibit - A Exhibit - B (RFI- T-0448.5) proposed the modification of the reinforcement and this detail exhibit C clarifies the exact location and detail where the modified reinforcement changes to the contract reinforcement</p> <p>This detail if approved would be incorporated into the TG06 shop drawings Please confirm if this detail is acceptable</p> </div> <div> <b>ANSWER:</b>  <p>Reference Documents: Exhibits A - C, RFI SCI# 236</p> <p>This RFI addresses the transition between modified reinforcement to contract reinforcement at GL6 at the south west corner see Location Plan exhibit - A Exhibit - B (RFI- T-0448.5) proposed the modification of the reinforcement and this detail exhibit C clarifies the exact location and detail where the modified reinforcement changes to the contract reinforcement</p> <p>This detail if approved would be incorporated into the TG06 shop drawings Please confirm if this detail is acceptable</p> </div> </div>						
<b>T-0608.1</b>	<b>BGP - Revised Spacing to Foundation Wall Vertical Reinforcement in Area 2</b>	<b>Closed</b>	<b>CR</b>	<b>10/10/2013</b>	<b>10/20/2013</b>	<b>10/14/2013</b>





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<b>T-0609</b>	<b>BGP - Clear Cover to the Vertical Reinforcement on the Foundation Wall</b>	<b>Closed</b>	<b>01</b>	<b>07/03/2013</b>	<b>07/13/2013</b>	<b>07/10/2013</b>
<div><div><b>From:</b> Webcor Construction LP      Michael Spillane</div><div><b>REQUEST:</b> Reference: Exhibit A, Attached  At some locations the clear cover to the vertical reinforcement on the foundation wall will be far in excess of the 2" shown on detail 1/S1-3201. Base on the RFI T-180.1 (see Exhibit - A) the clear cover could potentially be up 8" at the interface between the foundation wall at lower mat slab elevation and the waterproofing system.  Existing grade elevation = +25' + (protection slab elevation = -42') = 67' X 1/200 (CDSM pile vertical tolerance) = 4"  4" (CDSM pile vertical tolerance) + 4" (set back Per RFI T-180.1) + 2" (design clear cover to rebar) - 2" (waterproofing thickness subject to change) = 8" clear cover to rebar  Please confirm that this clear cover between the waterproofing system and the vertical reinforcement is acceptable.</div><div><b>ANSWER:</b> Reference: Exhibit A, Attached  At some locations the clear cover to the vertical reinforcement on the foundation wall will be far in excess of the 2" shown on detail 1/S1-3201. Base on the RFI T-180.1 (see Exhibit - A) the clear cover could potentially be up 8" at the interface between the foundation wall at lower mat slab elevation and the waterproofing system.  Existing grade elevation = +25' + (protection slab elevation = -42') = 67' X 1/200 (CDSM pile vertical tolerance) = 4"  4" (CDSM pile vertical tolerance) + 4" (set back Per RFI T-180.1) + 2" (design clear cover to rebar) - 2" (waterproofing thickness subject to change) = 8" clear cover to rebar  Please confirm that this clear cover between the waterproofing system and the vertical reinforcement is acceptable.</div></div>						
<b>T-0610</b>	<b>BGP - Micropiles at CPH #2 Thickened Slab</b>	<b>Closed</b>	<b>01</b>	<b>06/24/2013</b>	<b>07/04/2013</b>	<b>07/01/2013</b>
<div><div><b>From:</b> Webcor Construction LP      Robert Kjome</div><div><b>REQUEST:</b> Reference Sketch: SK-001  There are 4 micropiles within the perimeter of the thickened slab at CPH #2. Hand excavation will occur around these micropiles to keep from damaging the grout columns. The grout columns will be considered penetrations, in the structural design of the thickened mudslab and trim steel will be installed accordingly at each micropile. We will be installing butyl tape around the exposed grout column and onto the micropile, to top of thickened mudslab as a bond breaker. Please confirm this is acceptable.</div><div><b>ANSWER:</b> Reference Sketch: SK-001  There are 4 micropiles within the perimeter of the thickened slab at CPH #2. Hand excavation will occur around these micropiles to keep from damaging the grout columns. The grout columns will be considered penetrations, in the structural design of the thickened mudslab and trim steel will be installed accordingly at each micropile. We will be installing butyl tape around the exposed grout column and onto the micropile, to top of thickened mudslab as a bond breaker. Please confirm this is acceptable.</div></div>						
<b>T-0611</b>	<b>SSS - Grout Hole Diameter and Material</b>	<b>Closed</b>	<b>01</b>	<b>06/24/2013</b>	<b>07/04/2013</b>	<b>07/01/2013</b>



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<hr/>						
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFI: CN-005			Reference RFI: CN-005			
Following up with the response to RFI CN-005 please clarify the following:			Following up with the response to RFI CN-005 please clarify the following:			
1. Please advise if steel pipes intend to be filled with grout or concrete.			1. Please advise if steel pipes intend to be filled with grout or concrete.			
2. If the filler is grout, a 1" hole for venting should work. We do not need a 3" hole for venting.			2. If the filler is grout, a 1" hole for venting should work. We do not need a 3" hole for venting.			
3. If the filler is grout please advise if locations of the grout holes in cast nodes have been reviewed in the 3D model for accessibility in the field after nodes are attached to structural steel.			3. If the filler is grout please advise if locations of the grout holes in cast nodes have been reviewed in the 3D model for accessibility in the field after nodes are attached to structural steel.			
4. If steel pipes are filled with concrete and 3" hole must be patched with partial penetration weld please provide proposed detail and procedure for PJP weld.			4. If steel pipes are filled with concrete and 3" hole must be patched with partial penetration weld please provide proposed detail and procedure for PJP weld.			
5. Please provide procedure for patching the node grout hole.			5. Please provide procedure for patching the node grout hole.			
<b>T-0611.1</b>	<b>SSS - Grout Hole Options</b>	<b>Closed</b>	<b>01</b>	<b>08/19/2013</b>	<b>08/29/2013</b>	<b>08/23/2013</b>
<hr/>						
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Drawings: S1-4002			Reference Drawings: S1-4002			
Reference RFI: T-0611			Reference RFI: T-0611			
Reference Sketch: Sketch 1, Sketch 2			Reference Sketch: Sketch 1, Sketch 2			
Design documents do not specify or provide procedures for filling the steel pipe column with 4,000psi pea gravel. The following two options are proposed, please review and advise.			Design documents do not specify or provide procedures for filling the steel pipe column with 4,000psi pea gravel. The following two options are proposed, please review and advise.			
Option 1 (preferred)			Option 1 (preferred)			
1. Locate 3" grout hole at the back of the pipe to provide access from inside of the building.			1. Locate 3" grout hole at the back of the pipe to provide access from inside of the building.			
2. Locate 3" grout hole about 6" below CJP weld.			2. Locate 3" grout hole about 6" below CJP weld.			
3. Fill out pipe with concrete up to the hole.			3. Fill out pipe with concrete up to the hole.			
4. Use 1" vent / grout hole in the cast node to fill out the upper void with grout (not concrete). If it is not required			4. Use 1" vent / grout hole in the cast node to fill out the upper void with grout (not concrete). If it is not			



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	leave the void to reduce added cost.					
	Option 2 1. Weld a pipe nozzle with threaded end with a valve to 3" grout port. 2. Pump up concrete to completely fill the pipe column including voids in cast nodes. 3. Shut down the valve and wait until concrete sets. 4. Cut the pipe nozzle off. 5. Clean up the nozzle weld, remove extra concrete, weld in the plug, grind to AESS requirements, touch up. 6. Note: this option will be very expensive.					
	required leave the void to reduce added cost.					
	Option 2 1. Weld a pipe nozzle with threaded end with a valve to 3" grout port. 2. Pump up concrete to completely fill the pipe column including voids in cast nodes. 3. Shut down the valve and wait until concrete sets. 4. Cut the pipe nozzle off. 5. Clean up the nozzle weld, remove extra concrete, weld in the plug, grind to AESS requirements, touch up. 6. Note: this option will be very expensive.					
<b>T-0611.2</b>	<b>SSS - Grout Hole Options</b>	<b>Closed</b>	<b>01</b>	<b>08/28/2013</b>	<b>09/07/2013</b>	<b>09/09/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						
Reference RFI: T-0611.1 Reference Sketch: Attached						
The response to RFI T-0611.1 does not address the question. If grouting of the void in the cast node per Option 1 is not permitted, then Option 2 should be applied to completely fill the pipe column and the void in the cast node with concrete. Please confirm this is acceptable.						
<b>ANSWER:</b>						
Reference RFI: T-0611.1 Reference Sketch: Attached						
The response to RFI T-0611.1 does not address the question. If grouting of the void in the cast node per Option 1 is not permitted, then Option 2 should be applied to completely fill the pipe column and the void in the cast node with concrete. Please confirm this is acceptable.						



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<b>T-0612</b>	<b>B2 Electrical Room</b>	<b>Closed</b>	<b>01</b>	<b>06/24/2013</b>	<b>07/04/2013</b>	<b>07/02/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Drawing: A1-9214  Please confirm the pilaster size and whether a control joint is required at the single door opening to B2 Emergency Electrical Room B2880 adjacent to GL C, 1.4 and verify adjacent wall openings.						<b>ANSWER:</b> Reference Drawing: A1-9214  Please confirm the pilaster size and whether a control joint is required at the single door opening to B2 Emergency Electrical Room B2880 adjacent to GL C, 1.4 and verify adjacent wall openings.
<b>T-0612.1</b>	<b>BGP - Revised Plumbing Layout in Emergency Electrical Room B2</b>	<b>Closed</b>	<b>01</b>	<b>08/14/2013</b>	<b>08/24/2013</b>	<b>08/15/2013</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b> Reference: T-0612  The response to RFI T-0612 BGP revised the location of the doors to Emergency Electrical Room B2280. Are there any Mechanical, Electrical or Plumbing revisions required in Below Grade Package, to accommodate equipment layout changes resulting from the modified door locations.						<b>ANSWER:</b> Reference: T-0612  The response to RFI T-0612 BGP revised the location of the doors to Emergency Electrical Room B2280. Are there any Mechanical, Electrical or Plumbing revisions required in Below Grade Package, to accommodate equipment layout changes resulting from the modified door locations.
<b>T-0612.2</b>	<b>BGP - Updated Plumbing Drawing</b>	<b>Closed</b>	<b>01</b>	<b>09/06/2013</b>	<b>09/16/2013</b>	<b>09/09/2013</b>
<b>From:</b> Webcor Construction LP      Marina Rosso						
<b>REQUEST:</b> Please refer to T-0612.1 and drawing P1-3002.  The vent and trap primer lines within the mat slab at Room B2280 were revised in the Foundation Level Zone 02 Plumbing Plan PSK-2022 via RFI T-0612.1. The revised drawing did not include an enlarged plan detail.  Please provide the revised enlarged drawing plan shown on detail 1 of sheet P1-3002 for coordination.						<b>ANSWER:</b> Please refer to T-0612.1 and drawing P1-3002.  The vent and trap primer lines within the mat slab at Room B2280 were revised in the Foundation Level Zone 02 Plumbing Plan PSK-2022 via RFI T-0612.1. The revised drawing did not include an enlarged plan detail.  Please provide the revised enlarged drawing plan shown on detail 1 of sheet P1-3002 for coordination.
<b>T-0613</b>	<b>BSE - Excavation For Zone 4 Timber Pile Survey</b>	<b>Closed</b>	<b>01</b>	<b>06/24/2013</b>	<b>07/04/2013</b>	<b>07/28/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						







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T-0617	<div>From: Webcor Construction LP</div> <div>Robert Kjome</div> <div>REQUEST:</div> <div>Reference: Drawing S1-3001</div> <div>See attached sketches of the mat slab joint between S101 and S102. While performing the layout of the mat slab construction joints SCCI has discovered a conflict between one of the micro piles and the CJ between the two noted mat slab areas. SCCI will not be able to construct the joint as shown Detail 2 on CD S1-3001, with the micro pile in the way. SCCI proposes to modify the mat slab construction joint, to clear the conflicting micro pile, as shown on the attached sketches. Is this acceptable?</div>	Closed	01	06/24/2013	07/04/2013	07/08/2013
	<div>ANSWER:</div> <div>Reference: Drawing S1-3001</div> <div>See attached sketches of the mat slab joint between S101 and S102. While performing the layout of the mat slab construction joints SCCI has discovered a conflict between one of the micro piles and the CJ between the two noted mat slab areas. SCCI will not be able to construct the joint as shown Detail 2 on CD S1-3001, with the micro pile in the way. SCCI proposes to modify the mat slab construction joint, to clear the conflicting micro pile, as shown on the attached sketches. Is this acceptable?</div>					
T-0618	<div>BGP - Catch Basin at the Construction Joint</div> <div>From: Webcor Construction LP</div> <div>Robert Kjome</div> <div>REQUEST:</div> <div>Reference: Drawing A1-2813</div> <div>See attached lift drawings S105.0, S105.4, and CD A1-2813. For construction convenience, SCCI is proposing to move catch basin that falls between GL 8-9 and South of GL J, 24" westward (towards GL 8). Moving noted CB will make this part of the drainage system fall within the S105 mat slab our, and not have CB split between the CJ. Is this acceptable?</div>	Closed	01	06/25/2013	07/05/2013	07/11/2013
	<div>BGP - Mechanical Room Plumbing Clarifications 004</div> <div>From: Webcor Construction LP</div> <div>Jackson Tukuafu</div> <div>REQUEST:</div> <div>Reference: Drawing P1-2022, Spec Section 22 13 01</div> <div>Reference attached marked up CD PI-2022 and the drainage layout drawings. One of the floor sinks is located in the pin pile blockout. This creates a conflict between the added reinforcement in the mat slab and the floor sink. Please provide details for this conflict.</div>					
	<div>ANSWER:</div> <div>Reference: Drawing A1-2813</div> <div>See attached lift drawings S105.0, S105.4, and CD A1-2813. For construction convenience, SCCI is proposing to move catch basin that falls between GL 8-9 and South of GL J, 24" westward (towards GL 8). Moving noted CB will make this part of the drainage system fall within the S105 mat slab our, and not have CB split between the CJ. Is this acceptable?</div>					
	<div>ANSWER:</div> <div>Reference: Drawing P1-2022, Spec Section 22 13 01</div> <div>Reference attached marked up CD PI-2022 and the drainage layout drawings. One of the floor sinks is located in the pin pile blockout. This creates a conflict between the added reinforcement in the mat slab and the floor sink. Please provide details for this conflict.</div>					



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0619</b>	<b>BGP - CDSM Wall Encroachments rebar details- RFI T-0448.5</b>	<b>Closed</b>	<b>01</b>	<b>06/26/2013</b>	<b>07/06/2013</b>	<b>07/02/2013</b>
<b>From:</b> Shimmick Construction Company, Inc. Ben Gordon						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Within the issued response to RFI 448.5 no details were provided to depict the reinforcing configuration at the point in which the wall steps from it's reduced width back to it's original contract width of 36". Please provide a detail depicting the acceptable configuration at both the typical wall section and of the concourse level which includes the spandrel beam/wall interface.			Within the issued response to RFI 448.5 no details were provided to depict the reinforcing configuration at the point in which the wall steps from it's reduced width back to it's original contract width of 36". Please provide a detail depicting the acceptable configuration at both the typical wall section and of the concourse level which includes the spandrel beam/wall interface.			
<b>T-0620</b>	<b>BGP - Strut Bracing Conflicts With Shear Walls and Columns</b>	<b>Closed</b>	<b>01</b>	<b>06/26/2013</b>	<b>07/06/2013</b>	<b>07/15/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Drawing S1-3260, S1-3301, S1-2030  Based on the layout of the shoring and diagonal struts in the West end of the job, shear wall reinforcement (as shown on CD S1-3260) and the diagonal struts are in conflict. CD S1-3260 shows continuous vertical shear wall reinforcement from top of the mat slab to top of concourse deck. To avoid constructability issues SCCI suggests for shear walls to be constructed with horizontal construction joints at the same elevation as the first level of foundation walls. Adding horizontal joints will require modification of the reinforcement. Please confirm this is acceptable.			Reference: Drawing S1-3260, S1-3301, S1-2030  Based on the layout of the shoring and diagonal struts in the West end of the job, shear wall reinforcement (as shown on CD S1-3260) and the diagonal struts are in conflict. CD S1-3260 shows continuous vertical shear wall reinforcement from top of the mat slab to top of concourse deck. To avoid constructability issues SCCI suggests for shear walls to be constructed with horizontal construction joints at the same elevation as the first level of foundation walls. Adding horizontal joints will require modification of the reinforcement. Please confirm this is acceptable.			
<b>T-0621</b>	<b>CDSM Soldier Pile Enchroachment Area 3</b>	<b>Closed</b>	<b>01</b>	<b>06/26/2013</b>	<b>07/06/2013</b>	<b>07/07/2013</b>
<b>From:</b> Webcor Construction LP Michael Spillane						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Documents: Exhibits A - H  This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north wall in slab area 3 as well as all levels of the encroachment into the foundation wall between CDSM piles 1 to 32 as well. Location Plan see exhibit - A  Exhibit - B , C & H depict the location and degree in which the SP are encroaching			Reference Documents: Exhibits A - H  This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north wall in slab area 3 as well as all levels of the encroachment into the foundation wall between CDSM piles 1 to 32 as well. Location Plan see exhibit - A  Exhibit - B , C & H depict the location and degree in which the SP are encroaching			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>Option A</p> <p>Webcor is proposing to change the alignment of the Concrete Foundation wall on the north elevation along gridlines A between gridlines 1 and 5 - 6 (CDSM piles 1 to 50)</p> <p>The Concrete foundation wall which runs along gridline A between gridlines 1 and 5-6 would be offset into the structure by 0.1979' (2-3/8") the proposed Face of concrete Foundation wall would then be 2-3/8" off gridline A, this offset would enable the contract reinforcement to be installed without the need for further modifications to the reinforcement due to encroachment of the CDSM piles in concrete pour Areas 3 &amp; 4. See Exhibit - H</p>					
	<p>Option B</p> <p>WOJV proposal: (See Exhibit - B) Between CDSM piles 1 to 20-21 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear all the encroaching SP. This foundation wall area was originally a WR1 reinforcement area (#11 @8"oc EF vertically) and would change to #11 @6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 see Exhibit - D.</p> <p>Between CDSM piles 20-21 to 22 WOJV is proposing to decrease the specified 36" wall thickness again to 33 5/8" to clear all the encroaching SP, originally this was a WR2 reinforcement area #11 @6"oc vertically and would change to #11 @5"OC the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit - E)</p> <p>Between CDSM piles 22 to 31 the reinforcement would remain unchanged as per the Contract Reinforcement. See Exhibit-G showing a detail of transition between modified reinforcement to contract reinforcement with a non-contact reinforcement lap detail.</p> <p>Either of these options if approved would be incorporated into the TG06 shop drawings</p> <p>Please confirm if either of these options would be acceptable</p>					
	<p>Option A</p> <p>Webcor is proposing to change the alignment of the Concrete Foundation wall on the north elevation along gridlines A between gridlines 1 and 5 - 6 (CDSM piles 1 to 50)</p> <p>The Concrete foundation wall which runs along gridline A between gridlines 1 and 5-6 would be offset into the structure by 0.1979' (2-3/8") the proposed Face of concrete Foundation wall would then be 2-3/8" off gridline A, this offset would enable the contract reinforcement to be installed without the need for further modifications to the reinforcement due to encroachment of the CDSM piles in concrete pour Areas 3 &amp; 4. See Exhibit - H</p>					
	<p>Option B</p> <p>WOJV proposal: (See Exhibit - B) Between CDSM piles 1 to 20-21 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear all the encroaching SP. This foundation wall area was originally a WR1 reinforcement area (#11 @8"oc EF vertically) and would change to #11 @6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 see Exhibit - D.</p> <p>Between CDSM piles 20-21 to 22 WOJV is proposing to decrease the specified 36" wall thickness again to 33 5/8" to clear all the encroaching SP, originally this was a WR2 reinforcement area #11 @6"oc vertically and would change to #11 @5"OC the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit - E)</p> <p>Between CDSM piles 22 to 31 the reinforcement would remain unchanged as per the Contract Reinforcement. See Exhibit-G showing a detail of transition between modified reinforcement to contract reinforcement with a non-contact reinforcement lap detail.</p> <p>Either of these options if approved would be incorporated into the TG06 shop drawings</p> <p>Please confirm if either of these options would be acceptable</p>					







<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
	<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north wall in slab area 4 as well as all levels of the encroachment into the foundation wall between CDSM piles 31 to 60 as well. Location Plan see exhibit - A</p> <p>Exhibit - B ,C &amp; J depict the location and degree in which the SP are encroaching</p> <p>Option A</p> <p>Webcor is proposing to change the alignment of the Concrete Foundation wall on the north elevation along gridlines A between gridlines 1 and 5 - 6 (CDSM piles 1 to 50)</p> <p>The Concrete foundation wall which runs along gridline A between gridlines 1 and 5-6 would be offset into the structure by 0.1979' (2-3/8") the proposed Face of concrete Foundation wall would then be 2-3/8" off gridline A, this offset would enable the contract reinforcement to be installed without the need for further modifications to the reinforcement due to encroachment of the CDSM piles in concrete pour Areas 3 &amp; 4. See Exhibit - J</p> <p>Option B</p> <p>WOJV proposal: (See Exhibit - B) Between CDSM piles 31-32 to 35 and 41-42 to 45-46 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear all the encroaching SP, originally this was a WR2 reinforcement area #11@6"oc EF vertically and would change to #11@5"OC the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit -E) Between CDSM piles 35 to 41-42 and 45-46 to 49 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear all the encroaching SP. This foundation wall area was originally a WR1 reinforcement area (#11@8"oc EF vertically) and would change to #11@6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 ( Exhibit - D). Between CDSM piles 49 to 60 the reinforcement would remain unchanged as per the Contract drawings. See Exhibit-G, H &amp; I showing details of transition between modified reinforcement to contract reinforcement.</p> <p>Either of these options if approved would be incorporated into the TG06 shop drawings</p>					
	<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north wall in slab area 4 as well as all levels of the encroachment into the foundation wall between CDSM piles 31 to 60 as well. Location Plan see exhibit - A</p> <p>Exhibit - B ,C &amp; J depict the location and degree in which the SP are encroaching</p> <p>Option A</p> <p>Webcor is proposing to change the alignment of the Concrete Foundation wall on the north elevation along gridlines A between gridlines 1 and 5 - 6 (CDSM piles 1 to 50)</p> <p>The Concrete foundation wall which runs along gridline A between gridlines 1 and 5-6 would be offset into the structure by 0.1979' (2-3/8") the proposed Face of concrete Foundation wall would then be 2-3/8" off gridline A, this offset would enable the contract reinforcement to be installed without the need for further modifications to the reinforcement due to encroachment of the CDSM piles in concrete pour Areas 3 &amp; 4. See Exhibit - J</p> <p>Option B</p> <p>WOJV proposal: (See Exhibit - B) Between CDSM piles 31-32 to 35 and 41-42 to 45-46 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear all the encroaching SP, originally this was a WR2 reinforcement area #11@6"oc EF vertically and would change to #11@5"OC the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit -E) Between CDSM piles 35 to 41-42 and 45-46 to 49 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear all the encroaching SP. This foundation wall area was originally a WR1 reinforcement area (#11@8"oc EF vertically) and would change to #11@6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 ( Exhibit - D). Between CDSM piles 49 to 60 the reinforcement would remain unchanged as per the Contract drawings. See Exhibit-G, H &amp; I showing details of transition between modified reinforcement to contract</p>					





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## PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

### 30100 - Transbay Transit Center Project

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Job: 30100

<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>Please confirm if either of these options would be acceptable</p>					<p>reinforcement.</p> <p>Either of these options if approved would be incorporated into the TG06 shop drawings Please confirm if either of these options would be acceptable</p>
<b>T-0622.1</b>	<b>BGP - CDSM Soldier Pile Encroachment Area 4</b>	<b>Closed</b>	<b>01</b>	<b>08/13/2013</b>	<b>08/23/2013</b>	<b>08/22/2013</b>
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Documents: Exhibits A & D						Reference Documents: Exhibits A & D
This RFI addresses the previous comments to RFI T-622 see exhibit - D.						This RFI addresses the previous comments to RFI T-622 see exhibit - D.
The contractor preference approach is to use a modified option B originally outlined in RFI T-622						The contractor preference approach is to use a modified option B originally outlined in RFI T-622
Exhibit - A shows the revised Plan view with modifications made. Exhibit - C depict the degree in which the SP are encroaching in area 4.						Exhibit - A shows the revised Plan view with modifications made. Exhibit - C depict the degree in which the SP are encroaching in area 4.
Based on the response to previous RFI's the number of encroaching beams in area 4 has been reduced mainly due to the decreased thickness of the waterproofing system and the contractor willingness to use some of the construction tolerances in an effort to mitigate some of the smaller encroachments. This has resulted in only one area where modified reinforcement will have to be installed; Between CDSM piles 47 to 49 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP number 48. This wall area was originally a WR1 reinforcement area (#11 @8"oc EF vertically) and would change to #11 @6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing.						Based on the response to previous RFI's the number of encroaching beams in area 4 has been reduced mainly due to the decreased thickness of the waterproofing system and the contractor willingness to use some of the construction tolerances in an effort to mitigate some of the smaller encroachments. This has resulted in only one area where modified reinforcement will have to be installed; Between CDSM piles 47 to 49 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP number 48. This wall area was originally a WR1 reinforcement area (#11 @8"oc EF vertically) and would change to #11 @6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing.
In all other locations in area 4 the reinforcement would remain unchanged.						In all other locations in area 4 the reinforcement would remain unchanged.
See Exhibit-B showing details of transition between modified reinforcement to contract reinforcement.						See Exhibit-B showing details of transition between







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Job: 30100

<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
<b>T-0624</b>	<b>BSE - Micropile E231 Relocation - Instrumentation Pipe - Overhead Obstructions</b>	<b>Closed</b>	<b>01</b>	<b>06/28/2013</b>	<b>07/08/2013</b>	<b>07/01/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Attached Drawing, Spec Section 31 63 33		Reference: Attached Drawing, Spec Section 31 63 33				
Micropile E231 under trestle span 3.4 in Zone 3 had to be relocated in field 5.5' north due to overhead obstructions. Blue piping with instrumentation wiring inside was directly in the way of the micropile. See attached drawing for relocation information.		Micropile E231 under trestle span 3.4 in Zone 3 had to be relocated in field 5.5' north due to overhead obstructions. Blue piping with instrumentation wiring inside was directly in the way of the micropile. See attached drawing for relocation information.				
Please confirm this relocation is acceptable.		Please confirm this relocation is acceptable.				
<b>T-0625</b>	<b>BSE - Micropile E137 Relocation - Above Ground Equipment Obstruction</b>	<b>Closed</b>	<b>01</b>	<b>06/28/2013</b>	<b>07/08/2013</b>	<b>07/01/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Attached Drawing, Spec Section 31 63 33		Reference: Attached Drawing, Spec Section 31 63 33				
Micropile E137 in Zone 3 was installed 1' south of original location because it was in conflict with the de-sanding equipment. See attached drawing for relocation information.		Micropile E137 in Zone 3 was installed 1' south of original location because it was in conflict with the de-sanding equipment. See attached drawing for relocation information.				
Please confirm this relocation is acceptable.		Please confirm this relocation is acceptable.				
<b>T-0626</b>	<b>BGP- CDSM Soldier Pile Encroachment Area 5</b>	<b>Closed</b>	<b>01</b>	<b>07/02/2013</b>	<b>07/12/2013</b>	<b>07/10/2013</b>
<b>From:</b> Webcor Construction LP                      Michael Spillane						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Documents: Exhibits A - J		Reference Documents: Exhibits A - J				
This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north & south walls in slab area 5 as well as all levels of the encroachment into the foundation wall between CDSM piles 60 to 81 on the north elevation and 702 to 732 on the south elevation. For Location Plan see exhibit - A.		This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north & south walls in slab area 5 as well as all levels of the encroachment into the foundation wall between CDSM piles 60 to 81 on the north elevation and 702 to 732 on the south elevation. For Location Plan see exhibit - A.				
Exhibit - B, & C depict the location and degree in which the SP are encroaching		Exhibit - B, & C depict the location and degree in which the SP are encroaching				
For this RFI, the combined layers of the water proofing system had been assumed to be 2" thick, which is subject		For this RFI, the combined layers of the water proofing system had been assumed to be 2" thick, which is				



Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-0626.1	<p>to change, this may increase or decrease the number of encroaching piles depending on the thickness of the system used.</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) Between CDSM piles 60 to 62 and 69 to 71 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 61 &amp; 70, originally these were WR1 reinforcement area's #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit - D). Between CDSM piles 76 to 78-42, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 77. This foundation wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit - E).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B &amp; F) Between CDSM piles 704 to 706, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 705, originally this was a WR1 reinforcement area #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM encroachment issues the reinforcement will remain unchanged as per the Contract drawings. See Exhibit-G, H, I &amp; J showing details of transition between modified reinforcement to contract reinforcement. These solutions if approved would be incorporated into the TG06 shop drawings. Please confirm if these solutions would be acceptable.</p>	Closed	01	08/13/2013	08/23/2013	08/23/2013
	<p>subject to change, this may increase or decrease the number of encroaching piles depending on the thickness of the system used.</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) Between CDSM piles 60 to 62 and 69 to 71 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 61 &amp; 70, originally these were WR1 reinforcement area's #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit - D). Between CDSM piles 76 to 78-42, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 77. This foundation wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit - E).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B &amp; F) Between CDSM piles 704 to 706, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 705, originally this was a WR1 reinforcement area #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM encroachment issues the reinforcement will remain unchanged as per the Contract drawings. See Exhibit-G, H, I &amp; J showing details of transition between modified reinforcement to contract reinforcement. These solutions if approved would be incorporated into the TG06 shop drawings. Please confirm if these solutions would be acceptable.</p>					



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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
	<div><div><div>From: Webcor Construction LP</div><div>Michael Spillane</div></div><div><div>REQUEST:</div><div>Reference Documents: Exhibits A &amp; D</div><div>This RFI addresses the previous comments to RFI T-626 see exhibit - D.</div><div>Exhibit - A shows the revised Plan view with modifications made. Exhibit -C depict the degree in which the SP are encroaching in area 5.</div><div>Based on the response to the previous RFI the number of encroaching beams in area 5 has been reduced mainly due to the decreased thickness of the waterproofing system and the contractor willingness to use some of the construction tolerances in an effort to mitigate some of the smaller encroachments. This has resulted in only one area where modified reinforcement will have to be installed this is Between CDSM piles 73-74 to 78 on the north wall elevation WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP number 77. This wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing.</div><div>In all other locations on the north and south walls of area 5 the reinforcement would remain unchanged.</div><div>See Exhibit-B showing details of transition between modified reinforcement to contract reinforcement.</div><div>Please confirm if this solution is acceptable.</div></div></div> <div><div>ANSWER:</div><div>Reference Documents: Exhibits A &amp; D</div><div>This RFI addresses the previous comments to RFI T-626 see exhibit - D.</div><div>Exhibit - A shows the revised Plan view with modifications made. Exhibit -C depict the degree in which the SP are encroaching in area 5.</div><div>Based on the response to the previous RFI the number of encroaching beams in area 5 has been reduced mainly due to the decreased thickness of the waterproofing system and the contractor willingness to use some of the construction tolerances in an effort to mitigate some of the smaller encroachments. This has resulted in only one area where modified reinforcement will have to be installed this is Between CDSM piles 73-74 to 78 on the north wall elevation WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP number 77. This wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing.</div><div>In all other locations on the north and south walls of area 5 the reinforcement would remain unchanged.</div><div>See Exhibit-B showing details of transition between modified reinforcement to contract reinforcement.</div><div>Please confirm if this solution is acceptable.</div></div>					
T-0627	BGP- CDSM Soldier Pile Encroachment Area 6	Closed	01	07/03/2013	07/13/2013	07/11/2013
	<div><div><div>From: Webcor Construction LP</div><div>Michael Spillane</div></div><div><div>REQUEST:</div><div>Reference Documents: Exhibits A - J</div><div>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south walls in slab area 6 as well as all levels of the encroachment into the</div></div></div> <div><div>ANSWER:</div><div>Reference Documents: Exhibits A - J</div><div>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south walls in slab area 6 as well as all levels of the encroachment</div></div>					



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>foundation wall between CDSM piles 81 to 104 on the north elevation and 679 to 703 on the south elevation. For Location Plan see exhibit - A.</p> <p>Exhibit - B, &amp; C depict the location and degree in which the SP are encroaching</p> <p>For this RFI, the combined layers of the water proofing system had being assumed to be 2" thick, which is subject to change this may increase or decrease the number of encroaching piles depending on the thickness of the system used.</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) Between CDSM piles 82 to 84 and 102 to 105 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 83 &amp; 103, originally these were WR1 reinforcement area's #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit - D).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B &amp; Exhibit - F) Between CDSM piles 680 to 683, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 681 &amp; 682, originally this was a WR1 reinforcement area #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 695 to 697, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 696. This foundation wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit -E).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p>					<p>into the foundation wall between CDSM piles 81 to 104 on the north elevation and 679 to 703 on the south elevation. For Location Plan see exhibit - A.</p> <p>Exhibit - B, &amp; C depict the location and degree in which the SP are encroaching</p> <p>For this RFI, the combined layers of the water proofing system had being assumed to be 2" thick, which is subject to change this may increase or decrease the number of encroaching piles depending on the thickness of the system used.</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) Between CDSM piles 82 to 84 and 102 to 105 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 83 &amp; 103, originally these were WR1 reinforcement area's #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit - D).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B &amp; Exhibit - F) Between CDSM piles 680 to 683, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 681 &amp; 682, originally this was a WR1 reinforcement area #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 695 to 697, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 696. This foundation wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit -E).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the</p>



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T-0627.1	<b>BGP - CDSM Soldier Pile Encroachment Area 6</b>  <b>From:</b> Webcor Construction LP      Michael Spillane  <b>REQUEST:</b>  Reference Documents: Exhibits A & D  This RFI addresses the previous comments to RFI T-626 see exhibit - D.  Exhibit - A shows the revised Plan view with modifications made. Exhibit -C depict the degree in which the SP are encroaching in area 6.  Based on the response to the previous RFI the number of encroaching beams in area 6 north elevation has been reduced mainly due to the decreased thickness of the waterproofing system and the contractor willingness to use some of the construction tolerances in an effort to mitigate some of the smaller encroachments. This has resulted in no modifications now required to the contract reinforcement on the north elevation and changes have been made to the south elevation in line with response to the original RFI T-626.  See Exhibit-B & E which shows details of transition between modified reinforcement to contract reinforcement on the south elevations.  Please confirm if this solution is acceptable.	Closed	01	08/13/2013	08/23/2013	08/23/2013
						<b>ANSWER:</b>  Reference Documents: Exhibits A & D  This RFI addresses the previous comments to RFI T-626 see exhibit - D.  Exhibit - A shows the revised Plan view with modifications made. Exhibit -C depict the degree in which the SP are encroaching in area 6.  Based on the response to the previous RFI the number of encroaching beams in area 6 north elevation has been reduced mainly due to the decreased thickness of the waterproofing system and the contractor willingness to use some of the construction tolerances in an effort to mitigate some of the smaller encroachments. This has resulted in no modifications now required to the contract reinforcement on the north elevation and changes have been made to the south elevation in line with response to the original RFI T-626.  See Exhibit-B & E which shows details of transition between modified reinforcement to contract reinforcement on the south elevations.  Please confirm if this solution is acceptable.

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T-0627.2	BGP - CDSM Soldier Pile Encroachment: SP696 & SP104 in Area 6	Closed	CR	10/10/2013	10/20/2013	10/18/2013
<p><b>From:</b> Webcor Construction LP                      Jackson Tukuafu</p>						
<p><b>REQUEST:</b></p> <p>During Shimmick's (SCCI) field layout of the CDSM encroachment in Area 6, the folloWing extent of encroachment has been moved:</p> <p>-For encroachment at SP696, SCCI moved the East extent to SP694, this is due to SP695 encroaching during the buried bar layout. This accounts for 4' additional wall length with 33-5/8" due to CDSM encroachment.</p> <p>- For encroachment at SP104, the west extent of encroachment was moved to SP102. The rebar option 1 for SK1 with #11 rebar @ 6" OC will be used from SK102 to the West Extent of WR2 at Gridline 11</p> <p>Please confirm the deviation from RFI response to T-0627.1 is acceptable.</p>			<p><b>ANSWER:</b></p> <p>During Shimmick's (SCCI) field layout of the CDSM encroachment in Area 6, the folloWing extent of encroachment has been moved:</p> <p>-For encroachment at SP696, SCCI moved the East extent to SP694, this is due to SP695 encroaching during the buried bar layout. This accounts for 4' additional wall length with 33-5/8" due to CDSM encroachment.</p> <p>- For encroachment at SP104, the west extent of encroachment was moved to SP102. The rebar option 1 for SK1 with #11 rebar @ 6" OC will be used from SK102 to the West Extent of WR2 at Gridline 11</p> <p>Please confirm the deviation from RFI response to T-0627.1 is acceptable.</p>			





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<b>T-0628</b>	<b>BGP-CDSM Soldier Pile Encroachment in Area 7</b>	<b>Closed</b>	<b>CR</b>	<b>07/03/2013</b>	<b>07/13/2013</b>	<b>07/11/2013</b>
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Documents: Exhibits A - J			Reference Documents: Exhibits A - J			
This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north & south walls in slab area 7 as well as all levels of the encroachment into the foundation wall between CDSM piles 104 to 134 on the north elevation and 649 to 679 on the south elevation. For Location Plan see Exhibit A.			This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north & south walls in slab area 7 as well as all levels of the encroachment into the foundation wall between CDSM piles 104 to 134 on the north elevation and 649 to 679 on the south elevation. For Location Plan see Exhibit A.			
Exhibit B, & C depict the location and degree in which the SP are encroaching			Exhibit B, & C depict the location and degree in which the SP are encroaching			
For this RFI, the combined layers of the water proofing system had being assumed to be 2" thick, which is subject to change this may increase or decrease the number of encroaching piles depending on the thickness of the system used.			For this RFI, the combined layers of the water proofing system had being assumed to be 2" thick, which is subject to change this may increase or decrease the number of encroaching piles depending on the thickness of the system used.			
WOJV proposal North elevation on gridline A: (See Exhibit B) Between CDSM piles 102 to 105 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 103 & 104, originally these were WR1 reinforcement area #11@8"OC EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit D).			WOJV proposal North elevation on gridline A: (See Exhibit B) Between CDSM piles 102 to 105 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 103 & 104, originally these were WR1 reinforcement area #11@8"OC EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit D).			
WOJV proposal on the South elevation: (See Exhibit B & Exhibit F) Between CDSM piles 657 to 659 & 677 to 680, WOJV is proposing to decrease the specified 36" wall thickness to 33 1/2" & 33 5/8" respectively to clear the encroaching SP 658 & 678, Originally these were a WR1 reinforcement area #11@8"OC EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit D). Between CDSM piles 665 to 667 & 673 to 677, WOJV is proposing to decrease the specified 36" wall thickness to 32 15/16" & 33 5/8" respectively to clear the encroaching SP 666, 674 & 675. This foundation wall area was originally a WR2 reinforcement area (#11@6"OC EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit E).			WOJV proposal on the South elevation: (See Exhibit B & Exhibit F) Between CDSM piles 657 to 659 & 677 to 680, WOJV is proposing to decrease the specified 36" wall thickness to 33 1/2" & 33 5/8" respectively to clear the encroaching SP 658 & 678, Originally these were a WR1 reinforcement area #11@8"OC EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit D). Between CDSM piles 665 to 667 & 673 to 677, WOJV is proposing to decrease the specified 36" wall thickness to 32 15/16" & 33 5/8" respectively to clear the encroaching SP 666, 674 & 675. This foundation wall area was originally a WR2 reinforcement area (#11@6"OC EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped			

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<b>T-0629</b>	<b>BGP - Clear Cover on Concourse Slab</b>	<b>Closed</b>	<b>01</b>	<b>06/28/2013</b>	<b>07/08/2013</b>	<b>07/01/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Drawing S1-3500, Spec Section 03 30 20, 03 20 00		Reference: Drawing S1-3500, Spec Section 03 30 20, 03 20 00				
Detail 1 on S 1-3500 calls out for 3/4" clear cover on top and bottom of the lower concourse slab. ACI codes 301 and 318 specify 3/4" minimum cover for #11 bars and smaller, in slabs that are not exposed to extreme environment. Even though lower concourse is designed to eventually be enclosed with the rest of the superstructure, it will be exposed to the weather elements during the construction of the project. With that said, inadequate cover over rebar can cause plastic settlement cracking. SCCI is concerned that the 3/4" clear cover in the concourse slab could cause this plastic settlement cracking. Please confirm that the clear cover on the lower concourse slab is 3/4" minimum? Please specify what is the maximum clear coverage of the lower concourse reinforcement?		Detail 1 on S 1-3500 calls out for 3/4" clear cover on top and bottom of the lower concourse slab. ACI codes 301 and 318 specify 3/4" minimum cover for #11 bars and smaller, in slabs that are not exposed to extreme environment. Even though lower concourse is designed to eventually be enclosed with the rest of the superstructure, it will be exposed to the weather elements during the construction of the project. With that said, inadequate cover over rebar can cause plastic settlement cracking. SCCI is concerned that the 3/4" clear cover in the concourse slab could cause this plastic settlement cracking. Please confirm that the clear cover on the lower concourse slab is 3/4" minimum? Please specify what is the maximum clear coverage of the lower concourse reinforcement?				
<b>T-0630</b>	<b>BGP - Mat Slab Key Way Waterstops Installation</b>	<b>Closed</b>	<b>01</b>	<b>06/28/2013</b>	<b>07/08/2013</b>	<b>07/04/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Attached Photos, Spec Section 03 30 20		Reference: Attached Photos, Spec Section 03 30 20				
SCCI is proposing to leave the formed key surface in the mat slab as a formed finish to aid in the bonding of the hydrophilic waterstops to the concrete. With a specified 1/4" amplitude on the concrete surface, the bond between the concrete and the waterstop system decreases. The remainder of the construction joint will have stayform in place which generates a roughened surface. Please see attached photos that high light the area which will be formed finish. Please advise if this is acceptable?		SCCI is proposing to leave the formed key surface in the mat slab as a formed finish to aid in the bonding of the hydrophilic waterstops to the concrete. With a specified 1/4" amplitude on the concrete surface, the bond between the concrete and the waterstop system decreases. The remainder of the construction joint will have stayform in place which generates a roughened surface. Please see attached photos that high light the area which will be formed finish. Please advise if this is acceptable?				
<b>T-0631</b>	<b>BGP - Mat Slab Reinforcing Conflict with Micropiles</b>	<b>Closed</b>	<b>01</b>	<b>07/01/2013</b>	<b>07/11/2013</b>	<b>07/12/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>Reference: Drawing S1-2022 Thru S1-2031</p> <p>The typical mat slab reinforcing designed to be installed at 8" O.C.E.W. for the bottom and top mats. The micropile layout also consists of a uniformed spacing and at some locations has been adjusted for conflicts or for other purposes, example RFI 490. Should the typical mat slab reinforcing when laid out at 8" O.C.E.W. or some other reinforcing designed within the mat slab conflict with the micropile asbuilt, is it acceptable to displace the reinforcing from the designed spacing layout such that it is repositioned to either side of the micropile? Additionally, please confirm if reinforcing in direct contact with the micropile is acceptable? Should the displacement of the reinforcing to either side of a micropile not be acceptable please provide direction.</p>					<p>Reference: Drawing S1-2022 Thru S1-2031</p> <p>The typical mat slab reinforcing designed to be installed at 8" O.C.E.W. for the bottom and top mats. The micropile layout also consists of a uniformed spacing and at some locations has been adjusted for conflicts or for other purposes, example RFI 490. Should the typical mat slab reinforcing when laid out at 8" O.C.E.W. or some other reinforcing designed within the mat slab conflict with the micropile asbuilt, is it acceptable to displace the reinforcing from the designed spacing layout such that it is repositioned to either side of the micropile? Additionally, please confirm if reinforcing in direct contact with the micropile is acceptable? Should the displacement of the reinforcing to either side of a micropile not be acceptable please provide direction.</p>
<b>T-0632</b>	<b>BGP - Geothermal Field 7 &amp; 8 Manifold Riser Layout</b>	<b>Closed</b>	<b>01</b>	<b>07/02/2013</b>	<b>07/12/2013</b>	<b>07/09/2013</b>
	<p><b>From:</b> Webcor Construction LP      Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Reference: Attached Photos</p> <p>The initial geothermal riser/manifold layout for Fields 7 &amp; 8 placed the field 7 &amp; field 8 risers between soldier piles 176-177 and 177-178 respectively. To avoid conflicts with the riser install and the temporary 1st bridge, is it acceptable to move the field 7 riser to the CDSM wall panel between piles 172 and 173 and the field 8 riser to the CDSM wall panel between piles 173 and 174? See attached photos. Additionally, SCCI is looking to relocate the temperature probe to the CDSM wall panel between soldier pile beams 171 and 172. Is this acceptable? Please advise.</p>					<p><b>ANSWER:</b></p> <p>Reference: Attached Photos</p> <p>The initial geothermal riser/manifold layout for Fields 7 &amp; 8 placed the field 7 &amp; field 8 risers between soldier piles 176-177 and 177-178 respectively. To avoid conflicts with the riser install and the temporary 1st bridge, is it acceptable to move the field 7 riser to the CDSM wall panel between piles 172 and 173 and the field 8 riser to the CDSM wall panel between piles 173 and 174? See attached photos. Additionally, SCCI is looking to relocate the temperature probe to the CDSM wall panel between soldier pile beams 171 and 172. Is this acceptable? Please advise.</p>

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<b>T-0633</b>	<b>BGP - ASI#104 Clarifications</b>	<b>Closed</b>	<b>01</b>	<b>07/03/2013</b>	<b>07/13/2013</b>	<b>07/26/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: ASI 104s		Reference: ASI 104s				
SCCI is in receipt of ASI #104 on June 25th, 2013 in CR#T-071. Please clarify the following:		SCCI is in receipt of ASI #104 on June 25th, 2013 in CR#T-071. Please clarify the following:				
1) Per Sheet S-2202 to S-2211, the additional internal walls at the concourse are shown to be in solid line, for Zone 2-7, 10-11. Note 7 on S1-2022 refers us to the architectural drawings for CMU and concrete partition layout dimensions, joint locations, and CMU thickness. However, the corresponding Architectural drawings issued in ASI #104 for wall at concourse (A-2222 and A-2223), only depicts changes in Zones 2 and 3.		1) Per Sheet S-2202 to S-2211, the additional internal walls at the concourse are shown to be in solid line, for Zone 2-7, 10-11. Note 7 on S1-2022 refers us to the architectural drawings for CMU and concrete partition layout dimensions, joint locations, and CMU thickness. However, the corresponding Architectural drawings issued in ASI #104 for wall at concourse (A-2222 and A-2223), only depicts changes in Zones 2 and 3.				
a) A-2222 and A-2223 depicts the revised concourse walls to be RCW- please confirm that the internal concourse walls are not in TG-06 scope and additional scope to TG-06 contract will only be the additional couplers for added wall.		a) A-2222 and A-2223 depicts the revised concourse walls to be RCW- please confirm that the internal concourse walls are not in TG-06 scope and additional scope to TG-06 contract will only be the additional couplers for added wall.				
b) Please confirm that there are no internal walls to be constructed in TG06's scope at concours level.		b) Please confirm that there are no internal walls to be constructed in TG06's scope at concours level.				
c) Please confirm that the internal concourse walls shown as solid lines in drawing S-2022 to S-2211 are supposed to be shown as 'dotted' or 'ghost' lines in ASI #104.		c) Please confirm that the internal concourse walls shown as solid lines in drawing S-2022 to S-2211 are supposed to be shown as 'dotted' or 'ghost' lines in ASI #104.				
d) In multiple drawings (e.g: S1-2204), at the middle top of the page, the word "Future" has been deleted. Original drawings show "Future CMU walls, TYP". ASI #104 structural drawings deleted the word "Future" . Please clarify if the CMU walls at the concourse are in TG06's scope.		d) In multiple drawings (e.g: S1-2204), at the middle top of the page, the word "Future" has been deleted. Original drawings show "Future CMU walls, TYP". ASI #104 structural drawings deleted the word "Future" . Please clarify if the CMU walls at the concourse are in TG06's scope.				
e) Please issue revised Architectural drawings for Zone 4-Zone 11 (A2224-2231, revised) with the additional RCWs layout for the wall plan at concourse		e) Please issue revised Architectural drawings for Zone 4-Zone 11 (A2224-2231, revised) with the additional RCWs layout for the wall plan at concourse				
2) Per S1-2210 revised, a new note states: "Coordinate w/ manufacturer shop drawings for extent of beams". Please clarify which manufacturer SCCI is to coordinate with, or provide dimensions		2) Per S1-2210 revised, a new note states: "Coordinate w/ manufacturer shop drawings for extent of beams". Please clarify which manufacturer SCCI is to coordinate with, or provide dimensions				
3) ASI #104 issued new "Slab Edge Plan" A2842, A2843 and A2847 for Zone 2, 3 and 7. The new drawings depict the locations of MEP sleeves that were not shown in		3) ASI #104 issued new "Slab Edge Plan" A2842,				



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	<p>previous drawings.</p> <p>a) SCCI has not received any revisions to concurse plumbing drawings depicting these changes. The original plumbing drawings do not correspond to the location of the sleeves/blackouts shown in the new Architectural drawings A2842, 2843, 2847. Please verify which drawings SCCI needs to utilize to layout the sleeves/openings.</p> <p>b) Please issue Architectural drawings with sleeves/blackouts locations at the reminders of the zones.</p> <p>4) ASI #104 issued revised electrical drawings E1-2202 to E1-2204 that changes the layout of the electrical rooms. TG06 contract scope includes the installation F15 junction boxes to be terminated in designated electrical rooms.</p> <p>a) Please provide revised detailed drawings on the electrical room layout (E1-3201, 3202).</p>					<p>A2843 and A2847 for Zone 2, 3 and 7. The new drawings depict the locations of MEP sleeves that were not shown in previous drawings.</p> <p>a) SCCI has not received any revisions to concurse plumbing drawings depicting these changes. The original plumbing drawings do not correspond to the location of the sleeves/blackouts shown in the new Architectural drawings A2842, 2843, 2847. Please verify which drawings SCCI needs to utilize to layout the sleeves/openings.</p> <p>b) Please issue Architectural drawings with sleeves/blackouts locations at the reminders of the zones.</p> <p>4) ASI #104 issued revised electrical drawings E1-2202 to E1-2204 that changes the layout of the electrical rooms. TG06 contract scope includes the installation F15 junction boxes to be terminated in designated electrical rooms.</p> <p>a) Please provide revised detailed drawings on the electrical room layout (E1-3201, 3202).</p>
T-0633.1	BGP - 100% CD Phase 1 Documentation	Closed	CR	08/27/2013	09/01/2013	09/11/2013
	From: Webcor Construction LP Jackson Tukuafu					
	REQUEST:					ANSWER:
	<p>Please reference CR T-071 - ASI 104 - Below Grade Modifications and RFI T-0633.</p> <p>As per coordination meeting on 08/26/2013, to discuss discrepancies in ASI #104, the architectural drawings for Zone 4 thru Zone 11 (A1-224-2231, A1-2844-2846, A1-2848-2851) are not included in ASI 104. The architectural drawings are critical for SCCI's coordination and pricing of interior wall layout on the concourse level in conjunction with the corresponding structural drawings released in CR T-071 - ASI #104. Although, the design team provided their response to these discrepancies in RFI T-0633 by referencing "100% CD Phase 1 Documentation," the drawings have yet to be released for construction.</p>					<p>Please reference CR T-071 - ASI 104 - Below Grade Modifications and RFI T-0633.</p> <p>As per coordination meeting on 08/26/2013, to discuss discrepancies in ASI #104, the architectural drawings for Zone 4 thru Zone 11 (A1-224-2231, A1-2844-2846, A1-2848-2851) are not included in ASI 104. The architectural drawings are critical for SCCI's coordination and pricing of interior wall layout on the concourse level in conjunction with the corresponding structural drawings released in CR T-071 - ASI #104. Although, the design team provided their response to these discrepancies in RFI T-0633 by referencing "100% CD Phase 1 Documentation," the drawings have yet to be released for construction.</p>



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	<p>1. As per request by the design team, please release the following most-up-to date drawing sheets via this RFI : A1-2224 - 2231, A1-2844 - 2846, A1-2848 - 2851. 2. Please confirm the aforementioned drawings are to supersede current drawings in trade group package TG06.0.</p>					<p>1. As per request by the design team, please release the following most-up-to date drawing sheets via this RFI : A1-2224 - 2231, A1-2844 - 2846, A1-2848 - 2851. 2. Please confirm the aforementioned drawings are to supersede current drawings in trade group package TG06.0.</p>
<b>T-0634</b>	<b>BGP - Mass Concrete Placing Temperature</b>	<b>Closed</b>	<b>01</b>	<b>07/08/2013</b>	<b>07/18/2013</b>	<b>07/18/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: Spec Section 03 30 20, Attached Letter						Reference: Spec Section 03 30 20, Attached Letter
Please reference attached CTL Group letter dated 7.3.2013, Mat Slab Mock-Up thermal monitoring graph, Mat Slab Mock-Up thermal monitoring sensor locations sketch, Mat Slab CEMEX concrete tags and BOP spec section 03 30 20.3.5.B. Shimmick proposes the Maximum concrete placing temperature for Mass Concrete be increased to 80 degrees Farenheit. Is this acceptable?						Please reference attached CTL Group letter dated 7.3.2013, Mat Slab Mock-Up thermal monitoring graph, Mat Slab Mock-Up thermal monitoring sensor locations sketch, Mat Slab CEMEX concrete tags and BOP spec section 03 30 20.3.5.B. Shimmick proposes the Maximum concrete placing temperature for Mass Concrete be increased to 80 degrees Farenheit. Is this acceptable?



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T-0635	BGP - REBAR - Clarification to Maximum Allowable Rebar Clear Cover	Closed	01	07/09/2013	07/19/2013	07/17/2013
From: Webcor/Obayashi Joint Venture      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Drawing S1-3201, Spec Section 03 30 20			Reference: Drawing S1-3201, Spec Section 03 30 20			
RFI T-0608 shows detail of transition between modified reinforcement to contract reinforcement and shows that the internal wall face location of the concrete wall remains as shown in the contract drawing.			RFI T-0608 shows detail of transition between modified reinforcement to contract reinforcement and shows that the internal wall face location of the concrete wall remains as shown in the contract drawing.			
RFI T-0448.5 proposes to decrease the rebar configuration to accomodate the thinnest wall section to be 33-1/8" to clear all the encroaching SPs.			RFI T-0448.5 proposes to decrease the rebar configuration to accomodate the thinnest wall section to be 33-1/8" to clear all the encroaching SPs.			
At some locations, the rebar cover on the vertical wall rebar will exceed 2" Typ as shown in detail 1/S 1- 3201.			At some locations, the rebar cover on the vertical wall rebar will exceed 2" Typ as shown in detail 1/S 1- 3201.			
The worst case scenario in Area 1 & 2 will be at SP 737(lower), where the beam is 3.6" Too Far from the allowable horizontal alignment per TG03's contract Spec 31 56 13-3.3A.			The worst case scenario in Area 1 & 2 will be at SP 737(lower), where the beam is 3.6" Too Far from the allowable horizontal alignment per TG03's contract Spec 31 56 13-3.3A.			
In this case, the rebar cover will be: 2-7/8" (from the difference between 36" and 33-1/8") +2" (allowable rebar cover) +5-3/8" (0.64' offset - 0.1875' allowable waterproofing thickness) = Total cover of 10-1 /4"			In this case, the rebar cover will be: 2-7/8" (from the difference between 36" and 33-1/8") +2" (allowable rebar cover) +5-3/8" (0.64' offset - 0.1875' allowable waterproofing thickness) = Total cover of 10-1 /4"			
Please confirm that the maximum rebar clear cover (unreinforced concrete) of up to 10-1/4" between the CDSM wall and the Vertical Outside Face rebar in Area 1 & 2 is acceptable			Please confirm that the maximum rebar clear cover (unreinforced concrete) of up to 10-1/4" between the CDSM wall and the Vertical Outside Face rebar in Area 1 & 2 is acceptable			



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<b>T-0636</b>	<b>BGP - Micropile and Mat Slab CJ Conflict</b>	<b>Closed</b>	<b>01</b>	<b>07/09/2013</b>	<b>07/19/2013</b>	<b>07/12/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> Reference: Drawing S1-3001, Attached Sketches  See attached sketches of the Mat slab joint between S101 /S103 and S102/S 104. SCCI has discovered conflicts between multiple micro piles and the CJ between noted two mat slab areas. SCCI will not be able to construct the joint as shown Detail 2 on CD S 1-3001 , with the micro piles in the way. SCCI proposes to modify the mat slab construction joint, to clear the conflicting micro piles, as shown on the attached sketches. Is this acceptable?		<b>ANSWER:</b> Reference: Drawing S1-3001, Attached Sketches  See attached sketches of the Mat slab joint between S101 /S103 and S102/S 104. SCCI has discovered conflicts between multiple micro piles and the CJ between noted two mat slab areas. SCCI will not be able to construct the joint as shown Detail 2 on CD S 1-3001 , with the micro piles in the way. SCCI proposes to modify the mat slab construction joint, to clear the conflicting micro piles, as shown on the attached sketches. Is this acceptable?				
<b>T-0637</b>	<b>BGP - CDSM Wall Encroachment Rebar Details at Spandrel and Concourse Neede</b>	<b>Closed</b>	<b>01</b>	<b>07/15/2013</b>	<b>07/25/2013</b>	<b>07/26/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> Please refer to RFI response T-0608 and T-0448.5.  The approved typical CDSM encroachment wall reinforcement detail at the SW corner, West of GL 6 found in RFI T-0608 does not include the concourse level spandrel beam/wall interface.  Please provide a detail depicting an acceptable configuration at the concourse level which includes the spandrel beam/wall interface.		<b>ANSWER:</b> Please refer to RFI response T-0608 and T-0448.5.  The approved typical CDSM encroachment wall reinforcement detail at the SW corner, West of GL 6 found in RFI T-0608 does not include the concourse level spandrel beam/wall interface.  Please provide a detail depicting an acceptable configuration at the concourse level which includes the spandrel beam/wall interface.				
<b>T-0638</b>	<b>BGP - Mat Slab U Bars in Modified WR-2 Reinforcement Areas</b>	<b>Closed</b>	<b>01</b>	<b>07/16/2013</b>	<b>07/26/2013</b>	<b>07/23/2013</b>
<b>From:</b> Webcor Construction LP Michael Spillane						
<b>REQUEST:</b> Reference Documents: Exhibits A - E  The contractor has highlighted a potential conflict with the uses of #11 @5"OC vertically at the areas where CDSM piles at encroaching in WR-2 reinforcement areas.  Exhibit - A is a vertical cross section through the modified WR-2 area		<b>ANSWER:</b> Reference Documents: Exhibits A - E  The contractor has highlighted a potential conflict with the uses of #11 @5"OC vertically at the areas where CDSM piles at encroaching in WR-2 reinforcement areas.  Exhibit - A is a vertical cross section through the				





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	<p>Exhibit - B is a cross section showing the potential conflict with verts @ 5"OC Exhibit - C is a cross section showing the original design with verts @ 6"OC Exhibit - D &amp; E depicts possible solutions</p> <p>One of the proposed solutions Exhibit - D is to have the "U" bars at the contract width of 7.41"(6"+#11 bar dia) and the vertically rebar @ 5" OC and the U bars moves horizontally to avoid any conflicts with the mat slab reinforcement.</p> <p>Another possible solution is to change the "U" bars to a bar with a standard hook "candy cane shaped bar" see Exhibit - E</p> <p>Please confirm if either of these options would be acceptable</p>			<p>modified WR-2 area Exhibit - B is a cross section showing the potential conflict with verts @ 5"OC Exhibit - C is a cross section showing the original design with verts @ 6"OC Exhibit - D &amp; E depicts possible solutions</p> <p>One of the proposed solutions Exhibit - D is to have the "U" bars at the contract width of 7.41"(6"+#11 bar dia) and the vertically rebar @ 5" OC and the U bars moves horizontally to avoid any conflicts with the mat slab reinforcement.</p> <p>Another possible solution is to change the "U" bars to a bar with a standard hook "candy cane shaped bar" see Exhibit - E</p> <p>Please confirm if either of these options would be acceptable</p>		
<b>T-0639</b>	<b>BGP - Weld Access Hole repair</b>	<b>Closed</b>	<b>01</b>	<b>07/16/2013</b>	<b>07/26/2013</b>	<b>07/19/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>						
Reference: S1-3003, Spec Section 05 50 10						
<p>Please reference attached Pile Sleeve pictures, shop drawings, and product data/MSDS for Bituthene Liquid Membrane and Sikaflex Ia. Weld access holes (see photos) allow us to weld the penetration sleeves together in a continuous vertical weld (see shop drawings). SCCI proposes sealing access holes prior to pouring the mat slab.</p> <p>SCCI suggests sealing access holes on the piezometer lower rings (see Photo #1) with Bituthene Liquid Membrane Coating (see attached data) prior to installing the Preprufe Detail Patch per Option C of Grace substitution. SCCI suggests filling all other access holes (typ. trestle piles &amp; monitoring instruments) in the intermediate rings (see Photos #2 &amp; #3) with Sikaflex Ia Premium Sealant (see attached data &amp; MSDS) prior to mat slab pour.</p> <p>Please confirm this is an acceptable solution.</p>						
<b>ANSWER:</b>						
Reference: S1-3003, Spec Section 05 50 10						
<p>Please reference attached Pile Sleeve pictures, shop drawings, and product data/MSDS for Bituthene Liquid Membrane and Sikaflex Ia. Weld access holes (see photos) allow us to weld the penetration sleeves together in a continuous vertical weld (see shop drawings). SCCI proposes sealing access holes prior to pouring the mat slab.</p> <p>SCCI suggests sealing access holes on the piezometer lower rings (see Photo #1) with Bituthene Liquid Membrane Coating (see attached data) prior to installing the Preprufe Detail Patch per Option C of Grace substitution. SCCI suggests filling all other access holes (typ. trestle piles &amp; monitoring instruments) in the intermediate rings (see Photos #2 &amp; #3) with Sikaflex Ia Premium Sealant (see attached data &amp; MSDS) prior to mat slab pour.</p> <p>Please confirm this is an acceptable solution.</p>						







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<b>T-0642</b>	<b>BSE - Steel Plates at CDSM Piles 167-168</b>	<b>Open</b>	<b>01</b>	<b>07/17/2013</b>	<b>07/27/2013</b>	<b>07/18/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference: Spec Section 31 56 13  During leak grouting at level 5 excavation, a section of the CDSM wall panel between soldier piles 167-168 became dislodged, resulting in a high volume leak. In an effort to stabilize the damaged CDSM panel and stop the leak, BBII installed a steel road plate between soldier piles 167-168 and injected grout behind it.  BBII is concerned that removing the plate will likely cause the panel to become destabilized and could reopen the flow of water. BBII surveyed the face of the plate and found that at pile #167, the face of plate is 1/2" out from the theoretical face of pile, and at pile #168 the plate is 1-1/2" out from the theoretical face of pile. BBII proposes leaving the steel plate in place to maintain integrity of the CDSM panel. The edges of the plate may be grouted to provide a smooth transition to the CDSM wall for waterproofing.  Please confirm this is acceptable.		<b>ANSWER:</b> Reference: Spec Section 31 56 13  During leak grouting at level 5 excavation, a section of the CDSM wall panel between soldier piles 167-168 became dislodged, resulting in a high volume leak. In an effort to stabilize the damaged CDSM panel and stop the leak, BBII installed a steel road plate between soldier piles 167-168 and injected grout behind it.  BBII is concerned that removing the plate will likely cause the panel to become destabilized and could reopen the flow of water. BBII surveyed the face of the plate and found that at pile #167, the face of plate is 1/2" out from the theoretical face of pile, and at pile #168 the plate is 1-1/2" out from the theoretical face of pile. BBII proposes leaving the steel plate in place to maintain integrity of the CDSM panel. The edges of the plate may be grouted to provide a smooth transition to the CDSM wall for waterproofing.  Please confirm this is acceptable.				
<b>T-0643</b>	<b>BGP - ASI#104 - A1-2122 Added Line</b>	<b>Closed</b>	<b>01</b>	<b>07/17/2013</b>	<b>07/27/2013</b>	<b>07/19/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b> Reference: Drawing A1-2122, ASI#104  Please find attached A1-2122 issued in ASI#104. Please clarify what do the highlighted lines represent.		<b>ANSWER:</b> Reference: Drawing A1-2122, ASI#104  Please find attached A1-2122 issued in ASI#104. Please clarify what do the highlighted lines represent.				
<b>T-0644</b>	<b>BGP - Plumbing Scope Clarification ASI 104</b>	<b>Closed</b>	<b>01</b>	<b>07/17/2013</b>	<b>07/27/2013</b>	<b>07/26/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b> Reference: Drawing P1-6001, Spec Section 22 13 01  See attached marked up Rev 0 and Rev 1 Drawings P 1-6001. PI-6001 Rev 1 is a revision per AST 104. Rev 1 of the noted drawing does not have any "for reference only" notations in the details.		<b>ANSWER:</b> Reference: Drawing P1-6001, Spec Section 22 13 01  See attached marked up Rev 0 and Rev 1 Drawings P 1-6001. PI-6001 Rev 1 is a revision per AST 104. Rev 1 of the noted drawing does not have any "for reference only" notations in the details.				



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T-0645	<div><div>BGP - Door Opening Size at Emergency Electrical Room</div><div>From: Webcor Construction LP      Jackson Tukuafu</div></div>	Closed	01	07/18/2013	07/28/2013	07/19/2013
<div>Is the intent of the Designers to significantly change the scope of TG06 work?</div> <div>Please clarify the scope of work, i.e. applicable and non applicable details of the CD P1-6001 for the TG06 package.</div>			<div>Is the intent of the Designers to significantly change the scope of TG06 work?</div> <div>Please clarify the scope of work, i.e. applicable and non applicable details of the CD P1-6001 for the TG06 package.</div>			
<b>REQUEST:</b> <div>Reference: SKA-2748, Spec Section 03 30 20</div> <div>A new door opening has been added to the Northeast comer of the Emergency Electrical Room B2280 per drawing "SKA-2748" included with the response to RFI # T-0612. There are no dimensions provided for this new door opening on any of the sheets included in RFI # T - 0612.</div> <div>Please confirm door width to be 3'-5". Reference attached drawing "SKA-2748"</div>			<b>ANSWER:</b> <div>Reference: SKA-2748, Spec Section 03 30 20</div> <div>A new door opening has been added to the Northeast comer of the Emergency Electrical Room B2280 per drawing "SKA-2748" included with the response to RFI # T-0612. There are no dimensions provided for this new door opening on any of the sheets included in RFI # T -0612.</div> <div>Please confirm door width to be 3'-5". Reference attached drawing "SKA-2748"</div>			



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T-0646	BGP - Wall Pier Thickness - 3'5" + 3'5" Openings - Area 3 & 4	Closed	01	07/19/2013	07/29/2013	07/26/2013
<div><div>From: Webcor Construction LP</div><div>Jackson Tukuafu</div></div>						
REQUEST:			ANSWER:			
Reference: Drawing A1-9215, S1-9050, Spec Section 03 30 20			Reference: Drawing A1-9215, S1-9050, Spec Section 03 30 20			
There appears to be conflicting dimensions for the concrete interior wall pier located near gridlines 3.5/C.3 as shown in the attached drawing AI-9215. Contract drawing AI-9215 details the pier to be 2'0" wide by 1 '4" thick. However, based on criteria for wall piers as shown on S1-9050, the wall pier should be 2'0" wide by 1 '6" thick.			There appears to be conflicting dimensions for the concrete interior wall pier located near gridlines 3.5/C.3 as shown in the attached drawing AI-9215. Contract drawing AI-9215 details the pier to be 2'0" wide by 1 '4" thick. However, based on criteria for wall piers as shown on S1-9050, the wall pier should be 2'0" wide by 1 '6" thick.			
Please confirm if the two wall piers identified in the attached A 1-9215 should be 1 '4" thick or 1 '6" thick.			Please confirm if the two wall piers identified in the attached A 1-9215 should be 1 '4" thick or 1 '6" thick.			
If the wall is to be 1 '6" thick, please provide direction as to which side of the wall pier is to be maintained flush with adjacent wall.			If the wall is to be 1 '6" thick, please provide direction as to which side of the wall pier is to be maintained flush with adjacent wall.			





**ANSWER:**



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T-0649	BGP -Area 2 Clear Cover to the Vertical Reinforcement on the Foundation Wall	Closed	01	07/22/2013	08/01/2013	07/31/2013
<div><div><div>From: Webcor Construction LP</div><div>Michael Spillane</div></div><div><div>REQUEST:</div><div>Reference Documents: Exhibits A - G</div><div>Further to response to RFI T-609 (see exhibit - F) this RFI shows the areas of foundation wall in pour area 2, south wall which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A &amp; C</div><div>Exhibit - B &amp; C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement</div><div>Exhibit - D &amp; E (RFI T-448.5 and RFI T-608) which shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in area 2.</div><div>Exhibit - G shows the information on encroaching CDSM pile in this area for your review.</div><div>Please confirm that the clear cover between the waterproofing system and the vertical reinforcement outlined at these locations is acceptable</div></div><div><div>ANSWER:</div><div>Reference Documents: Exhibits A - G</div><div>Further to response to RFI T-609 (see exhibit - F) this RFI shows the areas of foundation wall in pour area 2, south wall which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A &amp; C</div><div>Exhibit - B &amp; C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement</div><div>Exhibit - D &amp; E (RFI T-448.5 and RFI T-608) which shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in area 2.</div><div>Exhibit - G shows the information on encroaching CDSM pile in this area for your review.</div><div>Please confirm that the clear cover between the waterproofing system and the vertical reinforcement outlined at these locations is acceptable</div></div></div>						
T-0650	BGP - Fire Management System Layout Conflicts with Class A Design	Closed	01	07/19/2013	07/29/2013	07/24/2013
<div><div><div>From: Webcor Construction LP</div><div>Jackson Tukuafu</div></div><div><div>REQUEST:</div><div>Reference: Drawing E1-2026, Spec Section 28 30 01, Attached Drawing</div><div>Review of the fire management system device layout appears to not meet the minimum candela rating of the NFPA code; refer to the attached drawing (dwg. #1, shaded) showing the areas of the platform that are deficient. Please confirm the candela rating set forth in the NFPA code are met with the current layout on drawing E1-2026 or provide a new layout that comply with NFPA candela rating requirements.</div></div><div><div>ANSWER:</div><div>Reference: Drawing E1-2026, Spec Section 28 30 01, Attached Drawing</div><div>Review of the fire management system device layout appears to not meet the minimum candela rating of the NFPA code; refer to the attached drawing (dwg. #1, shaded) showing the areas of the platform that are deficient. Please confirm the candela rating set forth in the NFPA code are met with the current layout on drawing E1-2026 or provide a new layout that comply with NFPA candela rating requirements.</div></div></div>						
T-0651	BGP - Area 3 Partition Wall Clarification	Closed	01	07/19/2013	07/29/2013	07/25/2013







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	<div>REQUEST:<div>Reference: Attached Drawing</div><div>Please reference attached CJ Layout for Mat Slab in Zone 1. SCCI requests acceptance to move Mat Slab Control Joints to have a 2' clearance of any pit. Control joints will be returned to their original layout and will tie to Foundation Wall at the submitted CJ locations.</div></div>					<div>ANSWER:<div>Reference: Attached Drawing</div><div>Please reference attached CJ Layout for Mat Slab in Zone 1. SCCI requests acceptance to move Mat Slab Control Joints to have a 2' clearance of any pit. Control joints will be returned to their original layout and will tie to Foundation Wall at the submitted CJ locations.</div></div>
T-0655	BGP - Revised Attached Method of Nelson Studs to the Elevator Pit Embedded Angle	Closed				
	From: Webcor Construction LP	Jackson Tukuafu				
	<div>REQUEST:<div>Reference: Spec Section 05 50 10</div><div>While attaching the 3/4" diameter by 8" Nelson Studs to the 8" X 4" X 1/2" angle it was determined the studs were not fusing to the base metal (angle). To maintain the procurement schedule of this fabrication needed for the Zone 1 - Area 03 Mat Slab placement, our fabricator (Gerlinger Steel) used the fillet weld method performed under the attached Welding Procedure Specifications (WPS) to attach studs to the angle(s). The welding was witnessed by the dispatched (IR #001459) ISI Shop CWI. Attached for the readers information and use are the shop fabrication drawing, the employed WPS, and photographs of the finished fabrication.</div><div>Is the alternate means of attaching the Nelson Studs to the angle, using the fillet weld method in lieu of the fusing method, acceptable?</div></div>					<div>ANSWER:<div>Reference: Spec Section 05 50 10</div><div>While attaching the 3/4" diameter by 8" Nelson Studs to the 8" X 4" X 1/2" angle it was determined the studs were not fusing to the base metal (angle). To maintain the procurement schedule of this fabrication needed for the Zone 1 - Area 03 Mat Slab placement, our fabricator (Gerlinger Steel) used the fillet weld method performed under the attached Welding Procedure Specifications (WPS) to attach studs to the angle(s). The welding was witnessed by the dispatched (IR #001459) ISI Shop CWI. Attached for the readers information and use are the shop fabrication drawing, the employed WPS, and photographs of the finished fabrication.</div><div>Is the alternate means of attaching the Nelson Studs to the angle, using the fillet weld method in lieu of the fusing method, acceptable?</div></div>



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<b>T-0656</b>	<b>BGP - Shear Wall Dowel and Shoring Pipe Bracing Conflict</b>	<b>Closed</b>	<b>01</b>	<b>07/24/2013</b>	<b>08/03/2013</b>	<b>08/07/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Drawing S1-3001, Spec Section 03 30 20		Reference: Drawing S1-3001, Spec Section 03 30 20				
A few potential conflicts exist between the typical shear wall vertical dowels and the 36" OD shoring Pipe Struts in Area 1. See attachment for locations of conflict.		A few potential conflicts exist between the typical shear wall vertical dowels and the 36" OD shoring Pipe Struts in Area 1. See attachment for locations of conflict.				
Based on Detail A shown in S1-3260, the typical shear wall verts will be lap spliced.		Based on Detail A shown in S1-3260, the typical shear wall verts will be lap spliced.				
Per the schedule in Detail 1-S1-3001, the #9 vertical shear wall reinforcement requires a 63" lap splice, which places the top of dowel at elevation -30'-5".		Per the schedule in Detail 1-S1-3001, the #9 vertical shear wall reinforcement requires a 63" lap splice, which places the top of dowel at elevation -30'-5".				
The centerline of Level D diagonal bracing atop Area 1 is shown to be at EL -29'-0" and the bottom of the 36" OD pipe strut at level D is at EL -30'-6".		The centerline of Level D diagonal bracing atop Area 1 is shown to be at EL -29'-0" and the bottom of the 36" OD pipe strut at level D is at EL -30'-6".				
The pipe strut will potentially encroach on the shear wall dowels since the vertical spacing is #9 at 10" OC.		The pipe strut will potentially encroach on the shear wall dowels since the vertical spacing is #9 at 10" OC.				
Please confirm that a 60" lap splice is acceptable at locations where conflicts exist, if not please provide solutions.		Please confirm that a 60" lap splice is acceptable at locations where conflicts exist, if not please provide solutions.				
<b>T-0658</b>	<b>BGP - Embedded Conduits in Mat Slab for the Light Column</b>	<b>Closed</b>	<b>01</b>	<b>07/25/2013</b>	<b>08/03/2013</b>	<b>08/02/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please reference attached drawing E1-2205 and E1-4105.		Please reference attached drawing E1-2205 and E1-4105.				
Per the attached lighting plan drawings, there are no electrical conduits shown to be embedded exclusively for the Light Column on drawing S1-6005.		Per the attached lighting plan drawings, there are no electrical conduits shown to be embedded exclusively for the Light Column on drawing S1-6005.				
Please confirm that there are no conduits required for the light column in both the concourse slab and mat slab or provide the location, route and size of the conduit at each level.		Please confirm that there are no conduits required for the light column in both the concourse slab and mat slab or provide the location, route and size of the conduit at each level.				
<b>T-0659</b>	<b>BGP - Mat Slab Conduits</b>	<b>Closed</b>	<b>01</b>	<b>07/30/2013</b>	<b>08/09/2013</b>	<b>08/13/2013</b>



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T-0660	From: Webcor Construction LP Jackson Tukuafu	Closed	01	07/30/2013	08/09/2013	08/07/2013
	<b>REQUEST:</b> Reference: A1-9204, E1-6001  The electrical conduit details on sheet A1-9204/Detail 1 and Detail 5 on E1-6001 regarding the electrical conduits on the columns are in conflict. Detail 1 on A1 -9204 indicates an embedded junction box in the long portions of the columns at Line D.8 above the Train Platform Level. Detail 5 on E1- 6001 indicates all conduits are to be stubbed up 12" at the face of the column. This Detail 5 shows all conduits (shown dashed) above the 12" stub up in the Mat Slab are to be installed in future phases outside of the TG06.0 contract. The columns are part of the TG06.0 scope.  1. Please clarify if these junction boxes and conduit are to be embedded in the columns or stubbed up through the slab at the face of each column at all four (4) locations..  2. If the conduits and boxes are to be embedded in the columns please provide a revised embedded conduit detail indicating conduits as part of TG06 Below Grade Scope.					
T-0660	BGP - Clear Cover to Mat Reinforcing at CDSM Pile Encroachment	Closed	01	07/30/2013	08/09/2013	08/07/2013
	From: Webcor Construction LP Jackson Tukuafu  <b>REQUEST:</b> Reference: Drawing S1-3201, Spec Section 03 30 20  Per Section 1 on S1-3201, the mat slab reinforcing is shown with 6" of clear cover from the outside face of the concrete wall. When the outside face wall and mat foundation step in and out due to CDSM encroachment, the 6" clear dimension shown on 1/S1-3201 will be encroached upon.  Please confirm this is acceptable. This would apply in any area where the wall thickness is being reduced due to encroaching CDSM Pile.					
T-0661	BSE - Access trestle penetration sleeve	Closed	01	07/30/2013	08/09/2013	08/26/2013





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T-0663	<div><div>BSE - Micropile Tie-Down detail</div><div>From: Webcor Construction LPRobert Kjome</div><div>REQUEST: Reference Drawing: S1-3003 Reference Submittal: TG0300-620.1  Detail 1 on S1-3003 shows a 12"x12"x2" plate under the domed nut on top of the micropile. Note 1 on S1-3003 states that "the contractor is responsible for the design of the pile to meet the design load requirements ... as stated in the project specifications." Submittal No. TG0300-620.1 was returned "No Exceptions Taken" and did not include the plate under the domed nut as it was not a part of BBII's micropile design. Please confirm that it is acceptable to move forward with approved Submittal No. TG0300-620.1 without the 12"x12"x2" plate.</div></div>	Closed	01	08/05/2013	08/15/2013	08/09/2013
						<div><div>upon recognition. See attached sketch #1 for reference.</div><div>3. At the contractor's discretion, he/she may cut the typical contract bar creating a gap in the bar to allow for the clashing micropile. Should this be the selected method to resolve the clash a lap splice bar of the same grade and bar size will be required at either side of the gap. The splice bar may be a non-contact lap splice. See attached sketch #2 for reference.</div></div> <div>ANSWER: Reference Drawing: S1-3003 Reference Submittal: TG0300-620.1  Detail 1 on S1-3003 shows a 12"x12"x2" plate under the domed nut on top of the micropile. Note 1 on S1-3003 states that "the contractor is responsible for the design of the pile to meet the design load requirements ... as stated in the project specifications." Submittal No. TG0300-620.1 was returned "No Exceptions Taken" and did not include the plate under the domed nut as it was not a part of BBII's micropile design. Please confirm that it is acceptable to move forward with approved Submittal No. TG0300-620.1 without the 12"x12"x2" plate.</div>



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T-0664	BGP - Conflict Between Pit Reinforcing & Trestle/Pin Piles	Closed	01	08/05/2013	08/15/2013	08/07/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST: Reference: Drawing S1-2022, Attached Photos  During the installation of the pit reinforcing between GL 1.4-2.3 and D.4-F a conflict was discovered between trestle/pile and the tail of the #11 pit reinforcing that extends beyond the limit of the pit out and into the main mat slab. Gerdau proposes to trim the tails of the conflicting rebar (Flame Cut) such that clearance can be maintained to the sleeve around the piles.  Please confirm this is acceptable or provide direction on how to proceed. This conflict is expected to occur at future pits too.		ANSWER: Reference: Drawing S1-2022, Attached Photos  During the installation of the pit reinforcing between GL 1.4-2.3 and D.4-F a conflict was discovered between trestle/pile and the tail of the #11 pit reinforcing that extends beyond the limit of the pit out and into the main mat slab. Gerdau proposes to trim the tails of the conflicting rebar (Flame Cut) such that clearance can be maintained to the sleeve around the piles.  Please confirm this is acceptable or provide direction on how to proceed. This conflict is expected to occur at future pits too.				
T-0665	BGP - Locations of Electrical Outlets, Equipment, and Fixtures	Closed	01	08/05/2013	08/10/2013	08/07/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST: Reference: Spec Section, 26 05 34  Per Specification Section 26 05 34, 3.2 B., the dimensions of the equipment fixtures and outlets are to be submitted via RFI for clarification pre pour. Attached is the layout for Electrical Room B2221 in the first Mat Slab pour.  Please confirm that these dimensions are acceptable so that the conduit can be laid out correctly.		ANSWER: Reference: Spec Section, 26 05 34  Per Specification Section 26 05 34, 3.2 B., the dimensions of the equipment fixtures and outlets are to be submitted via RFI for clarification pre pour. Attached is the layout for Electrical Room B2221 in the first Mat Slab pour.  Please confirm that these dimensions are acceptable so that the conduit can be laid out correctly.				
T-0665.1	BGP - Electrical Locations of Outlets, Equipment, and Fixtures in Electrical Room	Closed	01	08/23/2013	09/03/2013	08/27/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST: Please refer to drawing A1-9215, 1/E1-3101 and attached sketch SK-SCCI-0204.2.  Please find a revised electrical conduit layout for Electrical Room B2221 as requested in RFI T-0665. Please confirm the conduit layout and outlet, equipment and fixture		ANSWER: Please refer to drawing A1-9215, 1/E1-3101 and attached sketch SK-SCCI-0204.2.  Please find a revised electrical conduit layout for Electrical Room B2221 as requested in RFI T-0665. Please confirm the conduit layout and outlet,				



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	locations shown in the attached sketch SK-.SCCI-0204.2 is acceptable.					equipment and fixture locations shown in the attached sketch SK-.SCCI-0204.2 is acceptable.
<b>T-0665.2</b>	<b>BGP - Locations of Electrical Outlets, Equipment and Fixtures in Electrical Room E Closed</b>		<b>CR</b>	<b>09/12/2013</b>	<b>09/22/2013</b>	<b>09/19/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> Please refer to drawing A1-9215 dated 04/29/2013, E1-3101 dated 05/31/2013 (RFI T-0665) and attached shimmick sketch SK-RFI204.4.  The attached layout for Electrical Room B2221 shows the dimensions of the conduit locations in respect to the interior walls which are lined with 3/4" plywood per RFI T-0665. In addition, the room is located from grid lines, respectively..  Please confirm the layout as shown in the attached Shimmick sketch is acceptable.						<b>ANSWER:</b> Please refer to drawing A1-9215 dated 04/29/2013, E1-3101 dated 05/31/2013 (RFI T-0665) and attached shimmick sketch SK-RFI204.4.  The attached layout for Electrical Room B2221 shows the dimensions of the conduit locations in respect to the interior walls which are lined with 3/4" plywood per RFI T-0665. In addition, the room is located from grid lines, respectively..  Please confirm the layout as shown in the attached Shimmick sketch is acceptable.
<b>T-0665.3</b>	<b>BGP - Locations of Electrical Outlets, Equipment and Fixtures in Electrical Room E Closed</b>		<b>01</b>	<b>09/23/2013</b>	<b>10/03/2013</b>	<b>09/25/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b> Reference E1-3101  Confirm that the conduits for circuits to panelboard LPH-B2-A-12 are not included in the TG06 scope of work.						<b>ANSWER:</b> Reference E1-3101  Confirm that the conduits for circuits to panelboard LPH-B2-A-12 are not included in the TG06 scope of work.



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<b>T-0666</b>	<b>BSE - Elevator Pit Dimensions between GL 1.4 and GL 2</b>	<b>Closed</b>	<b>01</b>	<b>08/05/2013</b>	<b>08/15/2013</b>	<b>08/08/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Drawings: ASI #104, A1-9214 / A1-2122			Reference Drawings: ASI #104, A1-9214 / A1-2122			
Since the elevator manufacturer has not been selected, please confirm that the size of the elevator pit located between GL 1.4 and GL 2 is to be 10'-8" by 8'-10" as depicted in ASI #104 sheet A1-9214.			Since the elevator manufacturer has not been selected, please confirm that the size of the elevator pit located between GL 1.4 and GL 2 is to be 10'-8" by 8'-10" as depicted in ASI #104 sheet A1-9214.			





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<b>T-0666.1</b>	<b>BGP - Mat Slab Clarification to Elevator Pit and Slab Opening Dimensions</b>	<b>Closed</b>	<b>CR</b>	<b>08/21/2013</b>	<b>09/03/2013</b>	<b>08/28/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Webcor/Obayashi (W/O) is in receipt of attached Adamson Associates, Inc. (AAI) response to RFI T-0666 - BSE - Elevator Pit Dimensions Between GL1.4 and GL 2.</p> <p>This response is unacceptable. The Architect has sole responsibility for confirming that the pit dimensions of all elevators and escalators will accommodate the Architect's proposed elevator and escalator systems.</p> <p>Until a 100% IFC set is completed by the Architect, W/O has no definite knowledge of the Architect's proposed elevator and escalator systems. This issue has been discussed verbally for over 2 years, during which the Architect has maintained that they have full responsibility for designing all pits and openings to fit their proposed elevator and escalator systems. W/O is unable to even start the RFQ/Bidding process for hiring sub-contractors until the 100% IFC Contract Drawings are finalized by the Architect and approved by the owner; therefore, it is impossible for W/O to hire/coordinate sub-contractors prior to pouring of the elevator pit in the mat slab.</p> <p>The same applies to all pits and openings throughout the design documents, only the Architect is capable of confirming that these dimensions are acceptable for all of the Architect's proposed elevator/escalator systems.</p> <p>Please confirm all elevator pits and slab openings are acceptable as currently shown on the contract documents.</p>			<p>Webcor/Obayashi (W/O) is in receipt of attached Adamson Associates, Inc. (AAI) response to RFI T-0666 - BSE - Elevator Pit Dimensions Between GL1.4 and GL 2.</p> <p>This response is unacceptable. The Architect has sole responsibility for confirming that the pit dimensions of all elevators and escalators will accommodate the Architect's proposed elevator and escalator systems.</p> <p>Until a 100% IFC set is completed by the Architect, W/O has no definite knowledge of the Architect's proposed elevator and escalator systems. This issue has been discussed verbally for over 2 years, during which the Architect has maintained that they have full responsibility for designing all pits and openings to fit their proposed elevator and escalator systems. W/O is unable to even start the RFQ/Bidding process for hiring sub-contractors until the 100% IFC Contract Drawings are finalized by the Architect and approved by the owner; therefore, it is impossible for W/O to hire/coordinate sub-contractors prior to pouring of the elevator pit in the mat slab.</p> <p>The same applies to all pits and openings throughout the design documents, only the Architect is capable of confirming that these dimensions are acceptable for all of the Architect's proposed elevator/escalator systems.</p> <p>Please confirm all elevator pits and slab openings are acceptable as currently shown on the contract documents.</p>			

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T-0667	BGP - Geothermal Loop Excavation in Zone 4	Closed	01	08/05/2013	08/09/2013	08/07/2013
<div> <div> <b>From:</b> Webcor Construction LP           Jackson Tukuafu         </div> <div> <b>REQUEST:</b>            Reference: Spec Section 31 23 34.             Please refer to attached WOJV and SCCI internal correspondence in RFI #SHIMM000-0038.             SCCi is aware of the CDSM wall excavation required for the geothermal field risers, but is not aware of a geothermal specification requiring buttress shaft demolition for the geothermal loop trenches. Specification 31 23 34, Section 3.2 is very clear in the full scope of the ground excavation in soil and wall riser excavation in the CDSM, but it does not cover trenching in buttress shaft concrete.             Please provide a design defining the geothermal fields within the buttress shafts. Please include slot excavation, back-fill and compaction requirements in the the affected buttress'.         </div> <div> <b>ANSWER:</b>            Reference: Spec Section 31 23 34.             Please refer to attached WOJV and SCCI internal correspondence in RFI #SHIMM000-0038.             SCCi is aware of the CDSM wall excavation required for the geothermal field risers, but is not aware of a geothermal specification requiring buttress shaft demolition for the geothermal loop trenches. Specification 31 23 34, Section 3.2 is very clear in the full scope of the ground excavation in soil and wall riser excavation in the CDSM, but it does not cover trenching in buttress shaft concrete.             Please provide a design defining the geothermal fields within the buttress shafts. Please include slot excavation, back-fill and compaction requirements in the the affected buttress'.         </div> </div>						



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T-0668	BGP - CIDH Temporary Bridge Pier Sleeve Detail	Closed	01	08/05/2013	08/04/2013	08/08/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:		ANSWER:				
Please refer to drawing S1-3003, A1-8711, SCCI RFI #269 with asbuilt information of CIDH Piles at First Street, and ACI 117-90 section 3.4.1.2		Please refer to drawing S1-3003, A1-8711, SCCI RFI #269 with asbuilt information of CIDH Piles at First Street, and ACI 117-90 section 3.4.1.2				
The typical 48" diameter bridge pier detail (6/S1-3003) and waterproofing detail (4/A1-8711, 5/A1-8711 and 6//A1-8711) are designed for a steel assembly i.e. bridge pier, piles for shoring, bracing and trestle columns, pin piles and dewatering wells. As a result, the means of achieving the shown steel pipe sleeve is attainable.		The typical 48" diameter bridge pier detail (6/S1-3003) and waterproofing detail (4/A1-8711, 5/A1-8711 and 6//A1-8711) are designed for a steel assembly i.e. bridge pier, piles for shoring, bracing and trestle columns, pin piles and dewatering wells. As a result, the means of achieving the shown steel pipe sleeve is attainable.				
As per submittal package TG0300-201.3, the 48" temporary bridge piers are designed as CIDH (cast-in-drilled piles) piles and not steel. Specifications for concrete construction tolerances in ACI 117, section 3.4.1.2 allow for horizontal dimension of unformed members cast against soil for greater than 2 ft. but less than 6 ft. allow for +6" and -1/2".		As per submittal package TG0300-201.3, the 48" temporary bridge piers are designed as CIDH (cast-in-drilled piles) piles and not steel. Specifications for concrete construction tolerances in ACI 117, section 3.4.1.2 allow for horizontal dimension of unformed members cast against soil for greater than 2 ft. but less than 6 ft. allow for +6" and -1/2".				
The penetration sleeves for these piles have been fabricated.		The penetration sleeves for these piles have been fabricated.				
Proposed Solutions:		Proposed Solutions:				
1. Remove extra concrete from the outside diameter of the CIDH pile to allow the fabricated 48" penetrations to fit through means of bushing or grinding the concrete as necessary and utilize sleeves as originally intended.		1. Remove extra concrete from the outside diameter of the CIDH pile to allow the fabricated 48" penetrations to fit through means of bushing or grinding the concrete as necessary and utilize sleeves as originally intended.				
2. Please provide a detail drawing with the 48" temporary bridge pier condition as CIDH pile. Please include a sleeve detail allowing for the aforementioned tolerances and waterproofing. Please note, as typical of CIDH piles, the surface profile varies much greater than the 1/2" gap tolerance required for steel assemblies shown in 6/S1-3003.		2. Please provide a detail drawing with the 48" temporary bridge pier condition as CIDH pile. Please include a sleeve detail allowing for the aforementioned tolerances and waterproofing. Please note, as typical of CIDH piles, the surface profile varies much greater than the 1/2" gap tolerance required for steel assemblies shown in 6/S1-3003.				
T-0669	BGP - Foundation Wall Vertical CJ	Closed	01	08/06/2013	08/16/2013	08/09/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:		ANSWER:				



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	<p>Reference: Spec Section 033020, Attached Drawings</p> <p>See attached sketch of the vertical foundation wall CJ.</p> <p>During construction of the high congestion mockup SCCI has discovered a constructibility issue with the construction of the foundation walls, more particularly, the vertical construction joints. Vertical construction joints are to be constructed as prescribed on Detail 2 of the S 1-3001 CD.</p> <p>The designed vertical reinforcement consists of the following:</p> <p>a. WR-1 with #11 vertical bars 8" OC, haunch #10 bars 8" OC, and #4 cross ties 6" or 12" OC.</p> <p>b. WR-2 with #11 vertical bars 8" OC, haunch #10 bars 8" OC, and #4 cross ties 6" or 12" OC.</p> <p>c. WR-2MOD (CDSM Encroachments) with #11 vertical bars 5" OC, haunch #10 bars 8" OC, and #4 cross ties 5".</p> <p>When rebar configurations noted above are implemented, even with ACI allowed tolerances included, it will conflict with construction of the waterstops, hydrophilic hoses and forming of the vertical CJ.</p> <p>As a possible solution to this issue SCCI suggests the following:</p> <p>1. Eliminate a column of cross ties at the construction joints to allow construction of the vertical CJs per Det. 2 on SI-3001</p> <p>2. Reduce the depth of the vertical construction joint to 1.5" (similar to horizontal CJ).</p> <p>Please advise.</p>					
	<p>Reference: Spec Section 033020, Attached Drawings</p> <p>See attached sketch of the vertical foundation wall CJ.</p> <p>During construction of the high congestion mockup SCCI has discovered a constructibility issue with the construction of the foundation walls, more particularly, the vertical construction joints. Vertical construction joints are to be constructed as prescribed on Detail 2 of the S 1-3001 CD.</p> <p>The designed vertical reinforcement consists of the following:</p> <p>a. WR-1 with #11 vertical bars 8" OC, haunch #10 bars 8" OC, and #4 cross ties 6" or 12" OC.</p> <p>b. WR-2 with #11 vertical bars 8" OC, haunch #10 bars 8" OC, and #4 cross ties 6" or 12" OC.</p> <p>c. WR-2MOD (CDSM Encroachments) with #11 vertical bars 5" OC, haunch #10 bars 8" OC, and #4 cross ties 5".</p> <p>When rebar configurations noted above are implemented, even with ACI allowed tolerances included, it will conflict with construction of the waterstops, hydrophilic hoses and forming of the vertical CJ.</p> <p>As a possible solution to this issue SCCI suggests the following:</p> <p>1. Eliminate a column of cross ties at the construction joints to allow construction of the vertical CJs per Det. 2 on SI-3001</p> <p>2. Reduce the depth of the vertical construction joint to 1.5" (similar to horizontal CJ).</p> <p>Please advise.</p>					
T-0670	BGP - Mat Slab Control Joints 2	Closed	01	08/06/2013	08/16/2013	08/20/2013
	From: Webcor Construction LP Jackson Tukuafu					
	REQUEST:					ANSWER:
	Reference: Attached Drawing					Reference: Attached Drawing
	Please see attached drawing of Zone 1 control joints.					Please see attached drawing of Zone 1 control joints.





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T-0672	BGP - Fire Management Device Layout	Closed	01	08/08/2013	08/18/2013	08/14/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:		ANSWER:				
As discussed in the fire management coordination meeting on Monday 8/5, the contract plan device layout does not meet code for current draw. The stub ups from the mat slab to the devices shown on the contract plans at every other column will not be sufficient enough to meet code requirements for the future fully occupied space. If stubbed up at every other column, the consequences are having circuit runs that will end up doubling when the devices are added in the future. Siemens recommends that the stub ups are made at every column which will reduce the total current draw when devices are added in the future.		As discussed in the fire management coordination meeting on Monday 8/5, the contract plan device layout does not meet code for current draw. The stub ups from the mat slab to the devices shown on the contract plans at every other column will not be sufficient enough to meet code requirements for the future fully occupied space. If stubbed up at every other column, the consequences are having circuit runs that will end up doubling when the devices are added in the future. Siemens recommends that the stub ups are made at every column which will reduce the total current draw when devices are added in the future.				
Please advise.		Please advise.				
T-0673	BGP - Displacement of Cap Bar for Support	Closed	01	08/12/2013	08/22/2013	08/13/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:		ANSWER:				
Reference: S1-3600, Attached RFI 069		Reference: S1-3600, Attached RFI 069				
See attached Gerdau's RFI#069		See attached Gerdau's RFI#069				
At the contractors option, Gerdau is requesting to displace one top cap bar every 5' OC within the moment frame beams for support. Allowing the displacement of one top cap bar would reduce congestion near the top of the beam.		At the contractors option, Gerdau is requesting to displace one top cap bar every 5' OC within the moment frame beams for support. Allowing the displacement of one top cap bar would reduce congestion near the top of the beam.				
Please confirm that this is acceptable.		Please confirm that this is acceptable.				
T-0675	BGP - 400 Series HRC Couplers Assembly Procedure	Closed	01	08/12/2013	08/22/2013	08/16/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:		ANSWER:				
Reference: Spec Section 03 30 20		Reference: Spec Section 03 30 20				
SCCI is in receipt of the approval to SCCI's Request for Substitution TG0600-077 .1 to approve the use of HRC 400 Series Couplers at Vertical Walls. The comment on		SCCI is in receipt of the approval to SCCI's Request for Substitution TG0600-077 .1 to approve the use of HRC 400 Series Couplers at Vertical Walls. The				



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	<p>the approved Request for Substitution noted that assembly of the couplers is to be completed using strict adherence to the manufacturer's installation procedures.</p> <p>HRC, the manufacturer of the couplers has provided installation instructions, video footage of performance testing, test result and an operator qualification procedure, all supporting the assembly of the of the 400 serious couplers installation is acceptable with "hand tightened" procedure.</p> <p>Please confirm that the assembly of the 410/420 couplers "hand tight" is acceptable based on this manufacturer's recommendation as it was not directly addressed in the returned submittal comments.</p> <p>Video of the performance testing can be viewed : <a href="http://youtu.be/M5pFkjOgdN8">http://youtu.be/M5pFkjOgdN8</a></p>					<p>comment on the approved Request for Substitution noted that assembly of the couplers is to be completed using strict adherence to the manufacturer's installation procedures.</p> <p>HRC, the manufacturer of the couplers has provided installation instructions, video footage of performance testing, test result and an operator qualification procedure, all supporting the assembly of the of the 400 serious couplers installation is acceptable with "hand tightened" procedure.</p> <p>Please confirm that the assembly of the 410/420 couplers "hand tight" is acceptable based on this manufacturer's recommendation as it was not directly addressed in the returned submittal comments.</p> <p>Video of the performance testing can be viewed : <a href="http://youtu.be/M5pFkjOgdN8">http://youtu.be/M5pFkjOgdN8</a></p>
<b>T-0676</b>	<b>BGP - Mat Slab Construction Joint at 3ft Chamfer</b>	<b>Closed</b>	<b>CR</b>	<b>08/13/2013</b>	<b>08/23/2013</b>	<b>08/22/2013</b>
<p><b>From:</b> Webcor Construction LP      Jackson Tukuafu</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Drawing S1-3201, Spec Section 03 30 20			Reference: Drawing S1-3201, Spec Section 03 30 20			
See attached sketch of the mat slab wall CJ interface, reference Contract Drawing S 1-3201 , and RFI T- 0669. During layout of the bulkhead for the mat slab SCCI has discovered a constructibility issue with the construction ofthe mat slab CJ keyway as depicted on Detail 3 on SI-3001 , at the mat slab interface with the foundation walls.			See attached sketch of the mat slab wall CJ interface, reference Contract Drawing S 1-3201 , and RFI T- 0669. During layout of the bulkhead for the mat slab SCCI has discovered a constructibility issue with the construction ofthe mat slab CJ keyway as depicted on Detail 3 on SI-3001 , at the mat slab interface with the foundation walls.			
Reinforcement bars that are in conflict with the 10" deep keyway are: a.# 4 U-bars as depicted on detail3 on SI-3201. These bars are spaced 6" OC vertically and 5", 6" or 8" OC horizontally with the respect of the type of wall (i.e. WR-1, WR-2, or WR-2MOD) b. 3ft chamfer face bars- #10 at 8" OC per detail 1 on SI-3201			Reinforcement bars that are in conflict with the 10" deep keyway are: a.# 4 U-bars as depicted on detail3 on SI-3201. These bars are spaced 6" OC vertically and 5", 6" or 8" OC horizontally with the respect of the type of wall (i.e. WR-1, WR-2, or WR-2MOD) b. 3ft chamfer face bars- #10 at 8" OC per detail 1 on SI-3201			
When rebar configurations noted above are implemented, even with ACI allowed tolerances included, it will conflict			When rebar configurations noted above are			





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	<p>with construction of the waterstops, hydrophilic hoses and forming of the mat slab CJ.</p> <p>As a possible solution to this issue SCCI suggests the following:</p> <ol style="list-style-type: none"><li>1. Eliminate a section of #4 U-bars and 3' chamfer face bars to allow construction of the vertical CJs per Det. 3 on SI-3001</li><li>2. Transition mat slab keyway to match the foundation wall vertical keyway at 1 1/2" depth (reference RFI T-0669).</li></ol> <p>Please advise.</p>					<p>implemented, even with ACI allowed tolerances included, it will conflict with construction of the waterstops, hydrophilic hoses and forming of the mat slab CJ.</p> <p>As a possible solution to this issue SCCI suggests the following:</p> <ol style="list-style-type: none"><li>1. Eliminate a section of #4 U-bars and 3' chamfer face bars to allow construction of the vertical CJs per Det. 3 on SI-3001</li><li>2. Transition mat slab keyway to match the foundation wall vertical keyway at 1 1/2" depth (reference RFI T-0669).</li></ol> <p>Please advise.</p>
<b>T-0677</b>	<b>BGP - Sand Oil Interceptor and Baffle</b>	<b>Closed</b>	<b>01</b>	<b>08/13/2013</b>	<b>08/23/2013</b>	<b>08/23/2013</b>
	<p><b>From:</b> Webcor Construction LP      Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Reference specification section 22 13 01 2.5, CD PI-6001 Rev 1 (ASI 104), and SCCI's RFI 255. Drawings do not call out nor provide details for the sand oil interceptor and baffle wall that is called out in SP 22 13 01 2.5.</p> <p>Please provide details for the sand oil interceptor and baffle wall.</p>					<p><b>ANSWER:</b></p> <p>Reference specification section 22 13 01 2.5, CD PI-6001 Rev 1 (ASI 104), and SCCI's RFI 255. Drawings do not call out nor provide details for the sand oil interceptor and baffle wall that is called out in SP 22 13 01 2.5.</p> <p>Please provide details for the sand oil interceptor and baffle wall.</p>
<b>T-0678</b>	<b>BGP - Stair 203 Embed Conflict</b>	<b>Closed</b>	<b>01</b>	<b>08/13/2013</b>	<b>08/23/2013</b>	<b>08/27/2013</b>
	<p><b>From:</b> Webcor Construction LP      Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Please see attached SI-2022, SI-7004 and SI-7602. Please confirm full length L8x4xl/2 embed, as shown on detail 2 of S 1-7004 is required. This embed may conflict with future walls as shown on detail 2 of S 1-7004.</p>					<p><b>ANSWER:</b></p> <p>Please see attached SI-2022, SI-7004 and SI-7602. Please confirm full length L8x4xl/2 embed, as shown on detail 2 of S 1-7004 is required. This embed may conflict with future walls as shown on detail 2 of S 1-7004.</p>
<b>T-0679</b>	<b>BGP - CDSM Wall leaks</b>	<b>Closed</b>	<b>01</b>	<b>08/13/2013</b>	<b>08/23/2013</b>	<b>08/27/2013</b>





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<div><div><div>From: Webcor Construction LP</div><div>Jackson Tukuafu</div></div><div><div>REQUEST:</div><div>Reference: Attached Photo, Spec Section 07 12 10</div><div>Please reference the attached photo. CDSM wall leaks above Level D bracing have created standing water on top of the Area 3 protection slab in multiple areas. The ponding water is triggering the Ad cor ES Waterstop (see photo) along the perimeter of the excavation. SCCI has had minimal success shimming the areas of high leakage to help mitigate the water. Please review and provide direction as to how the leaks will be mitigated. As for the repair of the Adcor Waterstop, SCCI suggests cutting and removing the activated waterstop and installing a new strip with a 4" overlap on both sides. Is this acceptable?</div></div><div><div>ANSWER:</div><div>Reference: Attached Photo, Spec Section 07 12 10</div><div>Please reference the attached photo. CDSM wall leaks above Level D bracing have created standing water on top of the Area 3 protection slab in multiple areas. The ponding water is triggering the Ad cor ES Waterstop (see photo) along the perimeter of the excavation. SCCI has had minimal success shimming the areas of high leakage to help mitigate the water. Please review and provide direction as to how the leaks will be mitigated. As for the repair of the Adcor Waterstop, SCCI suggests cutting and removing the activated waterstop and installing a new strip with a 4" overlap on both sides. Is this acceptable?</div></div></div>						
T-0680	BGP -Area 7 Clear Cover to the Vertical Reinforcement on the Foundation Wall	Closed	01	08/14/2013	08/24/2013	08/22/2013
<div><div><div>From: Webcor Construction LP</div><div>Michael Spillane</div></div><div><div>REQUEST:</div><div>Reference Documents: Exhibits A - D</div><div>Further to response to RFI T-609 (see exhibit - D) this RFI shows the areas of foundation wall in pour area 7, on the north &amp; south wall elevations which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A</div><div>Exhibit - B &amp; C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement</div><div>RFI T - 628.1 which shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 7.</div><div>Please confirm that the clear cover between the waterproofing system and the vertical reinforcement as outlined at these locations is acceptable.</div></div><div><div>ANSWER:</div><div>Reference Documents: Exhibits A - D</div><div>Further to response to RFI T-609 (see exhibit - D) this RFI shows the areas of foundation wall in pour area 7, on the north &amp; south wall elevations which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A</div><div>Exhibit - B &amp; C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement</div><div>RFI T - 628.1 which shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 7.</div><div>Please confirm that the clear cover between the waterproofing system and the vertical reinforcement as outlined at these locations is acceptable.</div></div></div>						
T-0681	BGP - Area 6 Clear Cover to the Vertical Reinforcement on the Foundation Wall	Closed	01	08/16/2013	08/26/2013	08/22/2013



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T-0682	BGP -Area 5 Clear Cover to the Vertical Reinforcement on the Foundation Wall	Closed	01	08/16/2013	08/26/2013	08/22/2013
<div> <div> <b>From:</b> Webcor Construction LP Michael Spillane </div> <div> <b>REQUEST:</b> <p>Reference Documents: Exhibits A - D</p> <p>Further to response to RFI T-609 (see exhibit - D) this RFI shows the areas of foundation wall in pour area 5, on the north &amp; south wall elevations which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A</p> <p>Exhibit - B &amp; C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement</p> <p>RFI T - 626.1 shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 5.</p> <p>Please confirm that the clear cover between the waterproofing system and the vertical reinforcement as outlined at these locations is acceptable.</p> </div> <div> <b>ANSWER:</b> <p>Reference Documents: Exhibits A - D</p> <p>Further to response to RFI T-609 (see exhibit - D) this RFI shows the areas of foundation wall in pour area 5, on the north &amp; south wall elevations which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A</p> <p>Exhibit - B &amp; C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement</p> <p>RFI T - 626.1 shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 5.</p> <p>Please confirm that the clear cover between the waterproofing system and the vertical reinforcement as outlined at these locations is acceptable.</p> </div> </div>						







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T-0686	BGP - Drain Line Conflict with Micro Piles	Closed	01	08/22/2013	09/01/2013	09/04/2013
From: Webcor Construction LP Marina Rosso						
REQUEST:  See attached photo and CD PI-2030.  After performed layout of the drainage line system around GL K5 SCCI has discovered that a row of micro piles is in conflict with the 4" cast iron pipe drain line. SCCI suggest shifting the drain line run to clear the micro piles.  Is this acceptable?		ANSWER:  See attached photo and CD PI-2030.  After performed layout of the drainage line system around GL K5 SCCI has discovered that a row of micro piles is in conflict with the 4" cast iron pipe drain line. SCCI suggest shifting the drain line run to clear the micro piles.  Is this acceptable?				
T-0687	BGP - Drain Line Conflict with Reinforcement	Closed	01	08/22/2013	09/01/2013	09/03/2013
From: Webcor Construction LP Marina Rosso						
REQUEST:  See attached photos and CD P 1-2030.  Tails of the bottom rebar mat at the drainage pit are interfering with the construction of drainage lines and catch basin.  SCCI proposes following: 1. Shift the catch basin to where it clears the reinforcement tails. 2. Cut the rebar tails to allow installation of the drainage lines and the catch basin.  Please advise.		ANSWER:  See attached photos and CD P 1-2030.  Tails of the bottom rebar mat at the drainage pit are interfering with the construction of drainage lines and catch basin.  SCCI proposes following: 1. Shift the catch basin to where it clears the reinforcement tails. 2. Cut the rebar tails to allow installation of the drainage lines and the catch basin.  Please advise.				
T-0688	BGP - Pin Pile No 6 Conflict with Future Walls	Closed	CR	08/23/2013	09/02/2013	09/04/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:  Please refer to attached drawing S1-2052 (ASI 102), S1-3205 (ASI 100) and attached photos.  Drawing S1-2052, shows pin pile No. 6 (43"x43 block-out) at GL D.8/4 encroaching the future reinforced concrete wall (RCW). As a result, the couplers shown in detail drawing 5/S1-3205 cannot be installed in the area where the pin pile 43'x43" block-out is located.		ANSWER:  Please refer to attached drawing S1-2052 (ASI 102), S1-3205 (ASI 100) and attached photos.  Drawing S1-2052, shows pin pile No. 6 (43"x43 block-out) at GL D.8/4 encroaching the future reinforced concrete wall (RCW). As a result, the couplers shown in detail drawing 5/S1-3205 cannot be installed in the area where the pin pile 43'x43" block-out is located.				



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T-0689	<b>BSE - Micropiles in Depressions</b>  From: Webcor Construction LP      Robert Kjome  <b>REQUEST:</b>  Reference Sketch: attached Reference Email: attached  The micropile designer has confirmed that it is not necessary to extend the micropile to within 6" of the top of concrete and that the 5' embedment in the sump pit is adequate. Please confirm that this is acceptable to the design team.	Open	01	08/23/2013	09/02/2013	08/30/2013
			<b>ANSWER:</b>  Reference Sketch: attached Reference Email: attached  The micropile designer has confirmed that it is not necessary to extend the micropile to within 6" of the top of concrete and that the 5' embedment in the sump pit is adequate. Please confirm that this is acceptable to the design team.			



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T-0690	SSS - Stainless steel welded to cast iron	Closed	01	08/23/2013	09/02/2013	09/05/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference Drawing: 1/S1-6056		Reference Drawing: 1/S1-6056				
A number of details throughout structural steel drawings indicate stainless steel welded to cast iron or mild steel, see detail 1, 2/S1-6056 as one example. If two metals are fused, cast iron welded to stainless steel results in carbon migration. The chromium in the stainless and carbon in the steel have affinity for each other at elevated temperatures that results in carbon and chromium combining to form chromium carbide. This turns the welded area into hard and brittle material with a potential for rust that overtime has a high possibility to crack and fail.		A number of details throughout structural steel drawings indicate stainless steel welded to cast iron or mild steel, see detail 1, 2/S1-6056 as one example. If two metals are fused, cast iron welded to stainless steel results in carbon migration. The chromium in the stainless and carbon in the steel have affinity for each other at elevated temperatures that results in carbon and chromium combining to form chromium carbide. This turns the welded area into hard and brittle material with a potential for rust that overtime has a high possibility to crack and fail.				
For Det. 1 and 2 on S1-6056 the added tension from cables may contribute to failure. The proposed solutions include:		For Det. 1 and 2 on S1-6056 the added tension from cables may contribute to failure. The proposed solutions include:				
1.Use stainless steel instead of mild steel for the bottom connection plate thus welding stainless steel to stainless steel. Where the bottom plate has to connect to structural steel use bolted connection with thin dielectric isolator between two surfaces.		1.Use stainless steel instead of mild steel for the bottom connection plate thus welding stainless steel to stainless steel. Where the bottom plate has to connect to structural steel use bolted connection with thin dielectric isolator between two surfaces.				
2.Replace welded connection to bolted connection with an isolator.		2.Replace welded connection to bolted connection with an isolator.				
3.Use galvanized and painted plate instead of stainless steel plate.		3.Use galvanized and painted plate instead of stainless steel plate.				
Please advise.		Please advise.				





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<b>T-0691</b>	<b>BGP - FF&amp;FL Values for Mat Slab and Concourse Slab</b>	<b>Closed</b>	<b>01</b>	<b>08/23/2013</b>	<b>09/03/2013</b>	<b>09/03/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
1. Please confirm the contract documetns (TG06.0) do not specify a FF value for the Mat Slab.		1. Please confirm the contract documetns (TG06.0) do not specify a FF value for the Mat Slab.				
2. Also, please reference ACI 302.1R and contract specification 033020.3.6.B. ACI 302.1R does not provide any recommendations on F-numbers for broomed surfaces. Furthermore, table 8.15.3.b of ACI 302.1R (page 46) demonstrates to achieve FF value of 20 for a slab on grade, it must be a smooth, floated surface.		2. Also, please reference ACI 302.1R and contract specification 033020.3.6.B. ACI 302.1R does not provide any recommendations on F-numbers for broomed surfaces. Furthermore, table 8.15.3.b of ACI 302.1R (page 46) demonstrates to achieve FF value of 20 for a slab on grade, it must be a smooth, floated surface.				
Please clarify if the designer intends to have a rough broom/rake finish, or intends to have the concourse slab finished to a value of 20.		Please clarify if the designer intends to have a rough broom/rake finish, or intends to have the concourse slab finished to a value of 20.				
3. Please confirm the concrete finish within the train box.		3. Please confirm the concrete finish within the train box.				
<b>T-0692</b>	<b>BGP - Rebar Configuration at Moment Beam with Incorporation of S-3 vs T-9 Ties</b>	<b>Closed</b>	<b>01</b>	<b>08/23/2013</b>	<b>09/03/2013</b>	<b>08/30/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please refer to attached detail 3 on drawing S1-3603 and attached Gerdau Sketch SK-Gerdau RFI 070.		Please refer to attached detail 3 on drawing S1-3603 and attached Gerdau Sketch SK-Gerdau RFI 070.				
Please confirm it is acceptable to install S-3 stirrups and one T-9 tie as shown in the attached sketch for the vertical ties in the moment frame beam in lieu of installing all T-9 ties as depicted in detail 3/S1-3603. The proposed concrete reinforcement configuration is needed to avoid the constructability issues associated with alternating the hooks under the 1.5" of clear cover beneath the bottom beam bars.		Please confirm it is acceptable to install S-3 stirrups and one T-9 tie as shown in the attached sketch for the vertical ties in the moment frame beam in lieu of installing all T-9 ties as depicted in detail 3/S1-3603. The proposed concrete reinforcement configuration is needed to avoid the constructability issues associated with alternating the hooks under the 1.5" of clear cover beneath the bottom beam bars.				
<b>T-0693</b>	<b>BGP - Conduits in Columns</b>	<b>Closed</b>	<b>01</b>	<b>08/23/2013</b>	<b>09/02/2013</b>	<b>08/27/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
From discussions with the Design Team, we have been		From discussions with the Design Team, we have				



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T-0693.1	<b>BGP - Embedded Conduits in Columns</b>  informed that a number of columns will have post installed steel jackets. Columns with Fire Management and steel jackets will require the conduits and j-boxes to be embedded. It is noted that the jackets will not be full height, so the j-boxes will be flush with the concrete face.  Please provide the locations of the affected columns and a height for the boxes.	Closed	01	09/04/2013	09/14/2013	09/05/2013
<b>From:</b> Webcor Construction LP Marina Rosso						
<b>REQUEST:</b>  In the MEP meeting on 9/4/13, the response to RFI T-0693 was clarified. To confirm conversations with the WSP Electrical Design representative, the only conduits to be embedded in columns per the RFI T-0693 response are to be fire management conduits per the locations depicted in the response. All other conduits (power recepticals etc) are to be stubbed up on the face of the columns and are not to be embedded in the column.		<b>ANSWER:</b>  In the MEP meeting on 9/4/13, the response to RFI T-0693 was clarified. To confirm conversations with the WSP Electrical Design representative, the only conduits to be embedded in columns per the RFI T-0693 response are to be fire management conduits per the locations depicted in the response. All other conduits (power recepticals etc) are to be stubbed up on the face of the columns and are not to be embedded in the column.				



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T-0694	<b>Additional Rebar Conflict for Plumbing Trim at GL2/D.4</b>	Closed	01	08/26/2013	09/03/2013	08/27/2013
<b>From:</b> Webcor Construction LP                      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Please refer to drawings 1/A1-2122, 1/S1-3501 and attached Gerdau sketch SKS-1</p> <p>Due to the density or the typical N-S top mat bars (#10) and additional bars (#11) near the elevator pit at Gridlines 2 and D.4, the additional trim rebar per 1/S1-3501 for interrupting the bars over the plumbing opening cannot be installed to the East of the plumbing opening within 3" of the opening. The alternative solution would be to install the additional steel in a new layer below the top mat; however, due to proximity of the piping to the steel the bars cannot be placed below the top mat. Gerdau proposes the folloing options:</p> <p>A. Omit the additional trim bars to the East of the trimmed opening.</p> <p>B. Relocate the additional trim bars approximately 3'-0" East of the opening where the rebar spacing would allow for additional steel.</p> <p>Please advise if proposed options are acceptable. (see attached SKS-1)</p>			<p>Please refer to drawings 1/A1-2122, 1/S1-3501 and attached Gerdau sketch SKS-1</p> <p>Due to the density or the typical N-S top mat bars (#10) and additional bars (#11) near the elevator pit at Gridlines 2 and D.4, the additional trim rebar per 1/S1-3501 for interrupting the bars over the plumbing opening cannot be installed to the East of the plumbing opening within 3" of the opening. The alternative solution would be to install the additional steel in a new layer below the top mat; however, due to proximity of the piping to the steel the bars cannot be placed below the top mat. Gerdau proposes the folloing options:</p> <p>A. Omit the additional trim bars to the East of the trimmed opening.</p> <p>B. Relocate the additional trim bars approximately 3'-0" East of the opening where the rebar spacing would allow for additional steel.</p> <p>Please advise if proposed options are acceptable. (see attached SKS-1)</p>			







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T-0700	<b>BGP - Vehicle/Bike Beam End Support Embed</b>  From: Webcor Construction LP Jackson Tukuafu  <b>REQUEST:</b> Please reference attached drawing S1-3411 .  The corbel section detail I of sheet S1-3411 calls for a 3/4"x4"x 18" embed plate at the toe of the corbel. However, the embed detail in 1D/S1-3411 shows the embed width at 24".  Please clarify the embed width dimensional discrepancy: 3/4"x4"x18" or 3/4"x4"x24". Please advise.	Open	01	08/27/2013	09/09/2013	08/28/2013
						<b>ANSWER:</b> Please reference attached drawing S1-3411 .  The corbel section detail I of sheet S1-3411 calls for a 3/4"x4"x 18" embed plate at the toe of the corbel. However, the embed detail in 1D/S1-3411 shows the embed width at 24".  Please clarify the embed width dimensional discrepancy: 3/4"x4"x18" or 3/4"x4"x24". Please advise.
T-0702	<b>BGP - Chamfer Bar Top Hook</b>  From: Webcor Construction LP Jackson Tukuafu  <b>REQUEST:</b> See attached Gerdau's RFI#74. See attached SKS-74  In an effort to prevent the chamfer bar from encroaching on the existing shoring waler beams, Gerdau would like to propose over bending the top hook and turning it into a standard 180 degree hook as shown on the attached sketch.  Please advise if this is acceptable	Open	01	08/29/2013	09/08/2013	08/29/2013
						<b>ANSWER:</b> See attached Gerdau's RFI#74. See attached SKS-74  In an effort to prevent the chamfer bar from encroaching on the existing shoring waler beams, Gerdau would like to propose over bending the top hook and turning it into a standard 180 degree hook as shown on the attached sketch.  Please advise if this is acceptable



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<b>T-0703</b>	<b>BGP - Drainage Conflicts with Reinforcement</b>	<b>Closed</b>	<b>01</b>	<b>08/29/2013</b>	<b>09/08/2013</b>	<b>09/05/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  See attached marked up contract drawings PSK-2022 and S1-3005  Some of the drainage lines and fixtures are designed to be constructed in close proximity of the concrete columns, similarly S1-3005 depicts typical mat shar reinforcement schedule and details. Some of these shear reinforcement bars will be interfering with the drainage lines and fixtures. SCCI suggest to displace these shear reinforcement bars where conflicts occur. Displacement would occur laterally, in 8" increments, governed by the grid of the mat slab main reinforcement bars.  Please advise.						<b>ANSWER:</b>  See attached marked up contract drawings PSK-2022 and S1-3005  Some of the drainage lines and fixtures are designed to be constructed in close proximity of the concrete columns, similarly S1-3005 depicts typical mat shar reinforcement schedule and details. Some of these shear reinforcement bars will be interfering with the drainage lines and fixtures. SCCI suggest to displace these shear reinforcement bars where conflicts occur. Displacement would occur laterally, in 8" increments, governed by the grid of the mat slab main reinforcement bars.  Please advise.
<b>T-0704</b>	<b>SSS - Domestic Manufactured W40x503</b>	<b>Closed</b>	<b>01</b>	<b>08/29/2013</b>	<b>09/08/2013</b>	<b>09/03/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Drawings: S1-2505, S1-2506, S1-2507, S1-5050  We have determined that W40x503 is not produced Domestic melted and manufactured. This size girder occurs in 12 locations at the Perimeter Bus Deck Level between grid lines 21 and 33.  This beam is available from import sources, or it can be built up from domestic plate.  6/S1-5050 gives an option for W14 built up columns but does not give an option for W40 columns.  please advise						<b>ANSWER:</b>  Reference Drawings: S1-2505, S1-2506, S1-2507, S1-5050  We have determined that W40x503 is not produced Domestic melted and manufactured. This size girder occurs in 12 locations at the Perimeter Bus Deck Level between grid lines 21 and 33.  This beam is available from import sources, or it can be built up from domestic plate.  6/S1-5050 gives an option for W14 built up columns but does not give an option for W40 columns.  please advise
<b>T-0704.1</b>	<b>SSS - Built Up Plate Fabrication for W40x503</b>	<b>Closed</b>	<b>01</b>	<b>09/16/2013</b>	<b>09/26/2013</b>	<b>09/26/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference RFI: T-0704						<b>ANSWER:</b>  Reference RFI: T-0704

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>The built up beam will consist of 1 9/16" webs and 2 3/4" flanges. Please confirm that these plate sizes are appropriate.</p> <p>The web to flange weld was not addressed on returned RFI T-0704. We would suggest a 3/8" double fillet weld to join the web and flanges.</p> <p>Please confirm or provide an alternate detail.</p>					
<b>T-0705</b>	<b>BGP - Haunch Reinforcement at Double Waler Condition</b>	<b>Closed</b>	<b>01</b>	<b>08/29/2013</b>	<b>09/08/2013</b>	<b>09/02/2013</b>
	<p><b>From:</b> Webcor Construction LP                      Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Please refer to attached drawing 1/S1-3201 attached Photo SCCI-RFI 305.</p> <p>As per field coordination, the double shoring waler condition, where the waler web is lower than that of a single waler, the tail of the #10@8" (reference attached excerpt drawing BM-3t of submittal package TG0600-301.1) haunch reinforcement interferes with the web of the shoring waler.</p> <p>The condition was observed at Grid 2/ A and will likely repeat at other double waler locations. The typical resolution to the condition shall be to adjust the position, where required, so that the interfering tail clears the double waler web. As a result the 1-1/2" clear cover will deviate up to 4-112" of clear cover. The plan location of the tail shall remain as close as possible per the placement drawings. See the attached Photo for further details.</p> <p>The 1-1/2" clear spacing shall remain at locations unaffected by the reduced clearance of the double-wlaer. For pieces not yet fabricated and delivered, please refer to RFI T-0603 as the proposed solution to conform to the 1-1/2" clear cover.</p> <p>Plases confirm the revised haunch reinforcement clear clover as coordinated in the field is acceptable.</p>					
	<p>The built up beam will consist of 1 9/16" webs and 2 3/4" flanges. Please confirm that these plate sizes are appropriate.</p> <p>The web to flange weld was not addressed on returned RFI T-0704. We would suggest a 3/8" double fillet weld to join the web and flanges.</p> <p>Please confirm or provide an alternate detail.</p>					
	<p><b>ANSWER:</b></p> <p>Please refer to attached drawing 1/S1-3201 attached Photo SCCI-RFI 305.</p> <p>As per field coordination, the double shoring waler condition, where the waler web is lower than that of a single waler, the tail of the #10@8" (reference attached excerpt drawing BM-3t of submittal package TG0600-301.1) haunch reinforcement interferes with the web of the shoring waler.</p> <p>The condition was observed at Grid 2/ A and will likely repeat at other double waler locations. The typical resolution to the condition shall be to adjust the position, where required, so that the interfering tail clears the double waler web. As a result the 1-1/2" clear cover will deviate up to 4-112" of clear cover. The plan location of the tail shall remain as close as possible per the placement drawings. See the attached Photo for further details.</p> <p>The 1-1/2" clear spacing shall remain at locations unaffected by the reduced clearance of the double-wlaer. For pieces not yet fabricated and delivered, please refer to RFI T-0603 as the proposed solution to conform to the 1-1/2" clear cover.</p> <p>Plases confirm the revised haunch reinforcement clear clover as coordinated in the field is acceptable.</p>					





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T-0706	BGP - Locations of Electrical Outlets, Equipment, and Fixtures	Closed	01	08/30/2013	09/09/2013	09/13/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST: Per the RFI response, please find attached the revised layout for the Electrical Room B2221. This revised layout shows the dimensions off of the interior walls as requested.  Please advise if it is acceptable.		ANSWER: Per the RFI response, please find attached the revised layout for the Electrical Room B2221. This revised layout shows the dimensions off of the interior walls as requested.  Please advise if it is acceptable.				
T-0707	BGP - Spandrel beam modifications in Area 1 & 2	Closed	01	08/30/2013	09/09/2013	09/10/2013
From: Webcor Construction LP Robert Kjome						
REQUEST: Reference Documents: Exhibits A - C  Further to response to RFI T-637 please find attached proposed changes and details to the spandrel beams in pour area 1 & 2 for location plan see Exhibit - A and B  Exhibit - B shows the extent of the modifications necessary due to the foundation wall offset and changes made to wall reinforcement due to CDSM encroachment.  Exhibit - C shows the transition between modified reinforcement to contract reinforcement at spandrel beam as well as cross sections of the original design and the proposed modified beam detail.  RFI T-448.5 and T-608 shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 1 and 2.  RFI T-576 shows the revised location of the foundation wall on the west elevation of area 1.  Please confirm that these modification as outlined at these locations are acceptable.		ANSWER: Reference Documents: Exhibits A - C  Further to response to RFI T-637 please find attached proposed changes and details to the spandrel beams in pour area 1 & 2 for location plan see Exhibit - A and B  Exhibit - B shows the extent of the modifications necessary due to the foundation wall offset and changes made to wall reinforcement due to CDSM encroachment.  Exhibit - C shows the transition between modified reinforcement to contract reinforcement at spandrel beam as well as cross sections of the original design and the proposed modified beam detail.  RFI T-448.5 and T-608 shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 1 and 2.  RFI T-576 shows the revised location of the foundation wall on the west elevation of area 1.  Please confirm that these modification as outlined at these locations are acceptable.				
T-0708	BGP - Spandrel Beam Modification in Area 3	Closed	01	09/03/2013	09/13/2013	09/11/2013
From: Webcor Construction LP Michael Spillane						



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<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
T-0710	BGP - Haunch Reinforcement Alternative Detail at Dewatering Well in Area 3	Closed	CR	09/03/2013	09/16/2013	09/04/2013
<div> <div> <b>From:</b> Webcor Construction LP           Jackson Tukuafu         </div> <div> <b>REQUEST:</b>            Reference Drawing: S1 -3201            Reference Spec: 03 20 00            Attached Gerdau Sketch: SKS-76.1, SKS-76.2, SKS-76.3             A portion of the #10 @ 8" haunch bars cannot be installed as fabricated due to conflicts with overhead obstructions (shoring walers and struts) and the dewatering well sleeves. Per discussions with Sean McNeil where bars cannot be installed due to the obstructions, a modified #1 0 haunch bar with an HRC 555 head can be installed in place of the typical haunch bar. The attached sketches (SKS-76.1 and SKS-76.2) depict the magnitude of the obstructions at the dewatering wells in Area 3.             Please confirm if this is acceptable.             Additionally, please provide the required embedment length for the headed tail of the modified haunch bar.         </div> <div> <b>ANSWER:</b>            Reference Drawing: S1 -3201            Reference Spec: 03 20 00            Attached Gerdau Sketch: SKS-76.1, SKS-76.2, SKS-76.3             A portion of the #10 @ 8" haunch bars cannot be installed as fabricated due to conflicts with overhead obstructions (shoring walers and struts) and the dewatering well sleeves. Per discussions with Sean McNeil where bars cannot be installed due to the obstructions, a modified #1 0 haunch bar with an HRC 555 head can be installed in place of the typical haunch bar. The attached sketches (SKS-76.1 and SKS-76.2) depict the magnitude of the obstructions at the dewatering wells in Area 3.             Please confirm if this is acceptable.             Additionally, please provide the required embedment length for the headed tail of the modified haunch bar.         </div> </div>						





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
T-0712	<b>BGP - Jitter Bug Finish on Mat Slab Surface</b>  <b>From:</b> Webcor Construction LP      Jackson Tukuafu  <b>REQUEST:</b> Reference Spec Section 033020.3.6.B.I.c. See attached photos for a visual reference.  Please reference TG06.0, BGP contract specifications 033020.3.6.B.I.c. SCCI is proposing to finish the top surface of the Mat foundation Slab, as a "Jitter Bug" finish. All other finishing requirements will remain the same.  Is this acceptable?	Open	01	09/03/2013	09/13/2013	09/04/2013
		<p>original designs that were reviewed. Bradken Atchison can certainly produce the 1 inch radius and manage the effects the 1 inch radius causes, but producing that design will have cost impacts to the casting process.</p> <b>ANSWER:</b> Reference Spec Section 033020.3.6.B.I.c. See attached photos for a visual reference.  Please reference TG06.0, BGP contract specifications 033020.3.6.B.I.c. SCCI is proposing to finish the top surface of the Mat foundation Slab, as a "Jitter Bug" finish. All other finishing requirements will remain the same.  Is this acceptable?				



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<b>T-0713</b>	<b>BGP - Spandrel Beam Modifications in Area 4</b>	<b>Closed</b>	<b>01</b>	<b>09/05/2013</b>	<b>09/15/2013</b>	<b>09/16/2013</b>
<b>From:</b> Webcor Construction LP                      Michael Spillane						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Documents: Exhibits A - B		Reference Documents: Exhibits A - B				
Further to response to RFI T-637 please find attached proposed changes to the spandrel beams in pour Area 4 for location plan see exhibit - A.		Further to response to RFI T-637 please find attached proposed changes to the spandrel beams in pour Area 4 for location plan see exhibit - A.				
Exhibit - B shows the plan view of the modification necessary to the spandrel beam due to the revised reinforcement width of the foundation wall as well as typical cross sections.		Exhibit - B shows the plan view of the modification necessary to the spandrel beam due to the revised reinforcement width of the foundation wall as well as typical cross sections.				
RFI T - 622.1 shows the extent of the modification to the foundation wall on the north elevation of area 4.		RFI T - 622.1 shows the extent of the modification to the foundation wall on the north elevation of area 4.				
Please confirm that this modification as outlined at this location is acceptable.		Please confirm that this modification as outlined at this location is acceptable.				
<b>T-0714</b>	<b>BGP - Area 3- Partition Wall Pier Rebar Conflict With Plumbing Near GL3/C.3</b>	<b>Closed</b>	<b>01</b>	<b>09/03/2013</b>	<b>09/13/2013</b>	<b>09/04/2013</b>
<b>From:</b> Webcor Construction LP                      Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached Gerdau's RFI #078.		See attached Gerdau's RFI #078.				
Near Gridlines 3/C.3, there is a conflict between the partition wall pier dowels and the installed 6" plumbing pipe (8" with insulation). The wall pier currently overlaps with the plumbing pipe by approximately 6". Gerdau proposes to move the wall pier to the East, or West to allow the dowels to clear the pipe.		Near Gridlines 3/C.3, there is a conflict between the partition wall pier dowels and the installed 6" plumbing pipe (8" with insulation). The wall pier currently overlaps with the plumbing pipe by approximately 6". Gerdau proposes to move the wall pier to the East, or West to allow the dowels to clear the pipe.				
Please provide the acceptable direction (East or West) to shift the wall pier.		Please provide the acceptable direction (East or West) to shift the wall pier.				
Please note that there are conduits stub up on the East side that would need to be moved, should the opening is shifted towards the East.		Please note that there are conduits stub up on the East side that would need to be moved, should the opening is shifted towards the East.				



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0715</b>	<b>BGP - Adjustment to CB location</b>	<b>Closed</b>	<b>01</b>	<b>09/03/2013</b>	<b>09/13/2013</b>	<b>09/04/2013</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b> We understand from Design Team small adjustment to the locations of CBs at GL 1.8, J ; GL 7.2, C.3 and GL 10.2, B.5 are required.  Please provide dimensions for the modified locations.						<b>ANSWER:</b> We understand from Design Team small adjustment to the locations of CBs at GL 1.8, J ; GL 7.2, C.3 and GL 10.2, B.5 are required.  Please provide dimensions for the modified locations.
<b>T-0715.1</b>	<b>BGP - Adjustment to CB location</b>	<b>Closed</b>	<b>01</b>	<b>09/04/2013</b>	<b>09/14/2013</b>	<b>09/05/2013</b>
<b>From:</b> Webcor Construction LP      Spencer Sayles						
<b>REQUEST:</b> Please refer to attached drawing SKA-2820 and A1-2812 dated 04/29/2013.  As per design coordination meeting between SCCI, WOJV, AAI and TT, please confirm it is acceptable to omit SKA-2820 provided in RFI T-0715. Due to the timing of the issuance of this change, the Area 3 mat slab pour would be delayed by at least a week because the catch basin is already installed per drawing A1-2812, tested and inspected by DBI.						<b>ANSWER:</b> Please refer to attached drawing SKA-2820 and A1-2812 dated 04/29/2013.  As per design coordination meeting between SCCI, WOJV, AAI and TT, please confirm it is acceptable to omit SKA-2820 provided in RFI T-0715. Due to the timing of the issuance of this change, the Area 3 mat slab pour would be delayed by at least a week because the catch basin is already installed per drawing A1-2812, tested and inspected by DBI.
<b>T-0716</b>	<b>BGP - Haunch Reinforcement Alternative Detail</b>	<b>Closed</b>	<b>01</b>	<b>09/03/2013</b>	<b>09/13/2013</b>	<b>09/03/2013</b>
<b>From:</b> Webcor Construction LP      Marina Rosso						
<b>REQUEST:</b> See attached Gerdau's RFI #79.  The RFI Response to RFI T -0702 stated that the 180 degree hook chamfer bars are acceptable where the bars conflict with the double shoring walers. The intent of the RFI was to request the use of the 180- degree hook for the chamfer bars throughout the structure regardless of whether or not the bars were below a double or single walers.  Please confirm that this is acceptable.						<b>ANSWER:</b> See attached Gerdau's RFI #79.  The RFI Response to RFI T -0702 stated that the 180 degree hook chamfer bars are acceptable where the bars conflict with the double shoring walers. The intent of the RFI was to request the use of the 180- degree hook for the chamfer bars throughout the structure regardless of whether or not the bars were below a double or single walers.  Please confirm that this is acceptable.
<b>T-0716.1</b>	<b>BGP - Haunch Hook Embedment Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>10/08/2013</b>	<b>10/18/2013</b>	<b>10/10/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						







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<b>T-0722</b>	<b>BGP - Haunch Reinforcing Intersection with Dewatering Wells</b>	<b>Closed</b>	<b>01</b>	<b>09/04/2013</b>	<b>09/04/2013</b>	<b>09/04/2013</b>
<b>From:</b> Webcor Construction LP Marina Rosso						
<b>REQUEST:</b>						
Reference drawing: 1/S1-3201 Reference spec: 03 20 00						
Per field coordination with the on-site structural engineer the following conditions are to be confirmed as acceptable:						
1. In Area 3 along Gridline A, the haunch bars have been trimmed at the approximate intersections with the bottom mat. See sketch FC-3						
2. In Area 3 along Gridline 1, (2) haunch bars have been trimmed at the approximate intersection with the top mat with no embedment. See sketch FC-4.						
At future locations where dewatering wells interrupt haunch bars, use detail for bar E in sketches FC-3 or FC-4 if the haunch bars do not have 42" of embedment into the mat slab.						
<b>ANSWER:</b>						
Reference drawing: 1/S1-3201 Reference spec: 03 20 00						
Per field coordination with the on-site structural engineer the following conditions are to be confirmed as acceptable:						
1. In Area 3 along Gridline A, the haunch bars have been trimmed at the approximate intersections with the bottom mat. See sketch FC-3						
2. In Area 3 along Gridline 1, (2) haunch bars have been trimmed at the approximate intersection with the top mat with no embedment. See sketch FC-4.						
At future locations where dewatering wells interrupt haunch bars, use detail for bar E in sketches FC-3 or FC-4 if the haunch bars do not have 42" of embedment into the mat slab.						
<b>T-0723</b>	<b>BGP - Couplers for Future Walls</b>	<b>Pending</b>	<b>01</b>	<b>09/05/2013</b>	<b>09/13/2013</b>	<b>09/05/2013</b>
<b>From:</b> Webcor Construction LP Marina Rosso						
<b>REQUEST:</b>						
Reference Det. 6 on S1-3001 See attached photo of the form savers that are going to be used as couplers for future walls.						
As discussed in area 3 Mat Slab meeting on 9/4/2013 SCCI is proposing to installing all formsavers for future walls in the Mat slab flush with the top of the Mat slab, to EL -35.67'.						
As shown on the attached photo, epoxy coated form savers have tin cap incorporated into coupler's body. This tin cap will protect the rebar until the future construction, and will substitute "tar" shown on Det. 6 on S1-3001.						
Is this acceptable?						
<b>ANSWER:</b>						
Reference Det. 6 on S1-3001 See attached photo of the form savers that are going to be used as couplers for future walls.						
As discussed in area 3 Mat Slab meeting on 9/4/2013 SCCI is proposing to installing all formsavers for future walls in the Mat slab flush with the top of the Mat slab, to EL -35.67'.						
As shown on the attached photo, epoxy coated form savers have tin cap incorporated into coupler's body. This tin cap will protect the rebar until the future construction, and will substitute "tar" shown on Det. 6 on S1-3001.						
Is this acceptable?						



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<b>T-0724</b>	<b>BGP - CDSM Soldier Pile Encroachment Area 8</b>	<b>Closed</b>	<b>01</b>	<b>09/06/2013</b>	<b>09/16/2013</b>	<b>09/17/2013</b>
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Documents: Exhibits A - I			Reference Documents: Exhibits A - I			
<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 8 as well as all levels of the encroachment into the foundation wall between CDSM piles 133 to 164 on the north elevation and 618 to 650 on the south elevation for Location Plan see exhibit - A</p> <p>Exhibit - B, &amp; C depict the location and degree in which the SP are encroaching</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B &amp; Exhibit - F) Between CDSM piles 145 to 147 and 157 to 159 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 146 &amp; 158, originally these were WR1 reinforcement area's #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 159 to 162-163, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 161. This foundation wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit -E).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 631 to 635, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 632. This foundation wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit - E).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p>			<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 8 as well as all levels of the encroachment into the foundation wall between CDSM piles 133 to 164 on the north elevation and 618 to 650 on the south elevation for Location Plan see exhibit - A</p> <p>Exhibit - B, &amp; C depict the location and degree in which the SP are encroaching</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B &amp; Exhibit - F) Between CDSM piles 145 to 147 and 157 to 159 WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 146 &amp; 158, originally these were WR1 reinforcement area's #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 159 to 162-163, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 161. This foundation wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit -E).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 631 to 635, WOJV is proposing to decrease the specified 36" wall thickness to 33 5/8" to clear the encroaching SP 632. This foundation wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on SE stamped Detail A/Sk.3 option 2 (Exhibit - E).</p>			



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See Exhibit- G, H & I shows details of transition between modified reinforcement to contract reinforcement.

These solutions if approved would be incorporated into the TG06 shop drawings.

Please confirm if these solutions would be acceptable.

In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.

See Exhibit- G, H & I shows details of transition between modified reinforcement to contract reinforcement.

These solutions if approved would be incorporated into the TG06 shop drawings.

Please confirm if these solutions would be acceptable.

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<b>T-0725.1</b>	<b>BGP- CDSM Soldier Pile Cut-Off</b>	<b>Closed</b>	<b>CR</b>	<b>09/30/2013</b>	<b>10/10/2013</b>	<b>10/14/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Michael Spillane</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Further, in response to RFI T-725, WOJV requires the cut-off elevations for the 861 CDSM piles around the perimeter of the train box taking into account but not limited to, San Francisco city requirements for beam cut off in a public right of way, the elevation of utilities entering the train box structure, bridge structures and ramps as well as pedestrian stair towers 201A and 201 B and passenger elevator 201 foundations on the west side of Zone 1.</p> <p>Another item which will also need to be taken into account is where the CDSM shoring wall is shared with adjoining Projects i.e. 181 Fremont Street in Zone 4 and 101 1st street in Zone 3.</p> <p>This information once provided will but used as part of the future trade packages TG07.2 Concrete Superstructure and TG12.1 Civil Sitework scopes of work.</p> <p>Please provide in tabular format a list of the final cut-off elevations for each individual CDSM pile around the perimeter of the train box.</p>			<p>Further, in response to RFI T-725, WOJV requires the cut-off elevations for the 861 CDSM piles around the perimeter of the train box taking into account but not limited to, San Francisco city requirements for beam cut off in a public right of way, the elevation of utilities entering the train box structure, bridge structures and ramps as well as pedestrian stair towers 201A and 201 B and passenger elevator 201 foundations on the west side of Zone 1.</p> <p>Another item which will also need to be taken into account is where the CDSM shoring wall is shared with adjoining Projects i.e. 181 Fremont Street in Zone 4 and 101 1st street in Zone 3.</p> <p>This information once provided will but used as part of the future trade packages TG07.2 Concrete Superstructure and TG12.1 Civil Sitework scopes of work.</p> <p>Please provide in tabular format a list of the final cut-off elevations for each individual CDSM pile around the perimeter of the train box.</p>			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0725.2</b>	<b>BGP - CDSM Soldier Pile Cut-off</b>	<b>Closed</b>	<b>01</b>	<b>06/30/2014</b>	<b>07/10/2014</b>	<b>07/16/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Further to response to RFI T-725.1    TG07.2 trade package requires the cut off elevations for the 861 CDSM pile around the perimeter or the train box taking into account but not limited to, San Francisco city requirements for beam cut off in a public right of way, the elevation of utility's entering the train box structure, bridge structures and ramps as well as pedestrian stair towers 201A and 201B and    passenger elevator 201 foundations on the west side of zone 1, Another item which will also need to be take into account is where the CDSM shoring wall is shared with adjoining Projects i.e. 181 Fremont street in Zone 4 and 101 1st street in Zone 3.			Further to response to RFI T-725.1    TG07.2 trade package requires the cut off elevations for the 861 CDSM pile around the perimeter or the train box taking into account but not limited to, San Francisco city requirements for beam cut off in a public right of way, the elevation of utility's entering the train box structure, bridge structures and ramps as well as pedestrian stair towers 201A and 201B and    passenger elevator 201 foundations on the west side of zone 1, Another item which will also need to be take into account is where the CDSM shoring wall is shared with adjoining Projects i.e. 181 Fremont street in Zone 4 and 101 1st street in Zone 3.			
Please provide in Tabular format a list of the final cut-off elevations for each individual CDSM pile.			Please provide in Tabular format a list of the final cut-off elevations for each individual CDSM pile.			
<b>T-0726</b>	<b>BGP- Trestle pile No 6 in conflict with beam at Lower Concourse Level</b>	<b>Open</b>	<b>CR</b>	<b>09/09/2013</b>	<b>09/19/2013</b>	<b>09/20/2013</b>
<b>From:</b> Webcor Construction LP                      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Following a review and discussion on the trestle pile location, it has been noted that trestle pile number 6 (see sketch attached) is in conflict with a beam (B45) at the lower concourse slab elevation between gridline 5-6, E-F. The contractor is proposing to blockout a section of slab as shown on the sketch, this blockout section would then be infilled once the trestle pile has been removed.			Following a review and discussion on the trestle pile location, it has been noted that trestle pile number 6 (see sketch attached) is in conflict with a beam (B45) at the lower concourse slab elevation between gridline 5-6, E-F. The contractor is proposing to blockout a section of slab as shown on the sketch, this blockout section would then be infilled once the trestle pile has been removed.			
The contractor is to insure that the appropriate reinforcement lap splices are present between these concrete pours.			The contractor is to insure that the appropriate reinforcement lap splices are present between these concrete pours.			
Please confirm if this option would be acceptable			Please confirm if this option would be acceptable			







<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0729</b>	<b>BGP - Typical Trim Steel Requirements for Mat Slab per Field Coordination</b>	<b>Closed</b>	<b>CR</b>	<b>09/10/2013</b>	<b>09/20/2013</b>	<b>09/11/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please refer to attached drawing S1-3009 and S1-3501.			Please refer to attached drawing S1-3009 and S1-3501.			
As per field coordination between SCCI, Gerdau, WOJV and TT on 09/09/2013, to help alleviate congestion in the mat reinforcing, and in particular, congestion resulting from add bars due to openings and penetrations, please confirm the following items are acceptable:			As per field coordination between SCCI, Gerdau, WOJV and TT on 09/09/2013, to help alleviate congestion in the mat reinforcing, and in particular, congestion resulting from add bars due to openings and penetrations, please confirm the following items are acceptable:			
1. Details 4 and 7 on Sheet S1-3009 in so far as they apply to trestle piles, pin piles, dewatering wells and piezometric pipes can be relaxed in terms of additional bars. For an even number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars)/2. For an odd number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars +1)/2.			1. Details 4 and 7 on Sheet S1-3009 in so far as they apply to trestle piles, pin piles, dewatering wells and piezometric pipes can be relaxed in terms of additional bars. For an even number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars)/2. For an odd number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars +1)/2.			
2. Detail 1 on Sheet S1-3501, which applies to sinks, can be relaxed in terms of additional bars. For an even number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars)/2. For an odd number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars +1)/2. The minimum requirement of 2 bars on either side of the opening need not apply.			2. Detail 1 on Sheet S1-3501, which applies to sinks, can be relaxed in terms of additional bars. For an even number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars)/2. For an odd number of bars interrupted (typical bars and add bars) the number of bars added on either side of the opening can be (number of interrupted bars +1)/2. The minimum requirement of 2 bars on either side of the opening need not apply.			
3. The number of bars and maintenance of clear spacing will take precedence over 8" or 4" module spacing as to minimize the number of potential bar interruptions (and minimize resulting add bars). Any bar may be displaced to avoid conflict. The maximum center-to-center spacing of any two adjacent bars may be as large as 16". Clear spacing of 1 bar diameter shall be maintained between bars where bar relocation necessarily reduces spacing in the vicinity of relocation. Where bar relocation affects a lap splice, noncontact lap splices will be allowed up to 6" for #10 and #11 bars. This remedy shall apply in particular when seeking to avoid interruptions at small penetrations such as risers, vents, sinks and conduits.			3. The number of bars and maintenance of clear spacing will take precedence over 8" or 4" module spacing as to minimize the number of potential bar interruptions (and minimize resulting add bars). Any bar may be displaced to avoid conflict. The maximum center-to-center spacing of any two adjacent bars may be as large as 16". Clear spacing of 1 bar diameter shall be maintained between bars where bar relocation necessarily reduces spacing in the vicinity of relocation. Where bar relocation affects a lap splice, noncontact lap splices will be allowed up to 6" for #10 and #11 bars. This remedy shall apply in particular when seeking to avoid interruptions at small penetrations such as risers, vents, sinks and conduits.			
4. Clear spacing of 1db minimum shall be maintained in all mat reinforcing except for contact lap splices.						
5. Measures to reduce congestion at other locations such						





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0730.1</b>	<b>BGP - Extended Time for Concrete Delivery for Columns, Foundation Walls, Shear</b> <b>Closed</b>		<b>CR</b>	<b>12/04/2013</b>	<b>12/14/2013</b>	<b>12/11/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please see attached Set-time tests and Letter dated 11/25/2013, authored by Robert Foley, CEMEX QC Manager. The attached Set-time tests are for mixes: #1557205 - Columns, #1557216 - Foundation Walls, and #1558218 - Shear Walls and Concourse Slab.  Is it acceptable to extend the delivery time of the mixes referenced herein to 2 hours?						<b>ANSWER:</b>  Please see attached Set-time tests and Letter dated 11/25/2013, authored by Robert Foley, CEMEX QC Manager. The attached Set-time tests are for mixes: #1557205 - Columns, #1557216 - Foundation Walls, and #1558218 - Shear Walls and Concourse Slab.  Is it acceptable to extend the delivery time of the mixes referenced herein to 2 hours?
<b>T-0731</b>	<b>BGP - Conduit Termination Location for Sump Pumps Between Grid Lines 1 &amp; 12 - Closed</b>		<b>01</b>	<b>09/12/2013</b>	<b>09/22/2013</b>	<b>09/23/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to drawing E1-6001, A1-2102, A1-2103, E1-2023 and E1-2022.  Per Detail 7 on plan sheet E1-6001, the sump pump conduits for the below grade package are to be terminated 12" above the mat slab directly adjacent to the future train platform wall.  1. With the train platform wall beginning at grid line 12 and moving east, please provide the conduit termination location for the sumps installed west of grid line 12 where there is no train platform. Please include a set dimension the conduit should be set away from the sump.  Please note that for the two sumps that have been poured in Area 3, the conduits were placed roughly 9' to the north of each sump opening to avoid the future train tracks. There are 8 total sumps west of grid line 12 with 6 of them left to be placed.						<b>ANSWER:</b>  Please refer to drawing E1-6001, A1-2102, A1-2103, E1-2023 and E1-2022.  Per Detail 7 on plan sheet E1-6001, the sump pump conduits for the below grade package are to be terminated 12" above the mat slab directly adjacent to the future train platform wall.  1. With the train platform wall beginning at grid line 12 and moving east, please provide the conduit termination location for the sumps installed west of grid line 12 where there is no train platform. Please include a set dimension the conduit should be set away from the sump.  Please note that for the two sumps that have been poured in Area 3, the conduits were placed roughly 9' to the north of each sump opening to avoid the future train tracks. There are 8 total sumps west of grid line 12 with 6 of them left to be placed.
<b>T-0732</b>	<b>SSS - Train Box Column Material Specification</b>	<b>Closed</b>	<b>01</b>	<b>09/13/2013</b>	<b>09/23/2013</b>	<b>09/25/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  Reference Drawings: S-0007						<b>ANSWER:</b>  Reference Drawings: S-0007



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	<p>After review of General notes SS-9 F on drawing S-0007 Skanskas fabricator, Thompson Metal Fab, is requesting clarification on the material grade specification for the Train Box Columns.</p> <p>Please review and update the following if needed prior to Thomson Metal Fab's material order.</p> <p>Plate: Grade ASTM A709 H.P.S. 70W Zone 1 All Train Box material to have a Charpy V Notch Impact Test with a Minimum of 25FT Lbs. @ -10 degrees. ASTM A673 Frequency "P", ASTM A6 supplementary requirement S5.</p>					<p>After review of General notes SS-9 F on drawing S-0007 Skanskas fabricator, Thompson Metal Fab, is requesting clarification on the material grade specification for the Train Box Columns.</p> <p>Please review and update the following if needed prior to Thomson Metal Fab's material order.</p> <p>Plate: Grade ASTM A709 H.P.S. 70W Zone 1 All Train Box material to have a Charpy V Notch Impact Test with a Minimum of 25FT Lbs. @ -10 degrees. ASTM A673 Frequency "P", ASTM A6 supplementary requirement S5.</p>
T-0733	SSS - Transfer Girder Material Specifications	Closed	01	09/13/2013	09/23/2013	09/25/2013
	<p>From: Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Reference Drawings: S-0007</p> <p>After review of General notes drawing S-0007 Note our fabricator, Thompson Metal Fab, is requesting clarification on the material grade specification for the Transfer Girders.</p> <p>Please review and update the following if needed prior to their material order.</p> <p>Plate: Grade ASTM A572 GR 50 Zone 1 All Transfer Beam Material to have a Charpy V Notch Impact Test with a Minimum of 20FT Lbs. @ 70 degree F. ASTM A673 Frequency "P", ASTM A6 supplementary requirement S5.</p> <p>Or "AS Noted"</p> <p>ASTM A709 Grade H.P.S. 70 W, Zone 1, CVN 25FT Lbs. @ -10 Deg. F. ASTM Frequency "P", ASTM A6 Supplementary requirement S5.</p>					<p><b>ANSWER:</b></p> <p>Reference Drawings: S-0007</p> <p>After review of General notes drawing S-0007 Note our fabricator, Thompson Metal Fab, is requesting clarification on the material grade specification for the Transfer Girders.</p> <p>Please review and update the following if needed prior to their material order.</p> <p>Plate: Grade ASTM A572 GR 50 Zone 1 All Transfer Beam Material to have a Charpy V Notch Impact Test with a Minimum of 20FT Lbs. @ 70 degree F. ASTM A673 Frequency "P", ASTM A6 supplementary requirement S5.</p> <p>Or "AS Noted"</p> <p>ASTM A709 Grade H.P.S. 70 W, Zone 1, CVN 25FT Lbs. @ -10 Deg. F. ASTM Frequency "P", ASTM A6 Supplementary requirement S5.</p>



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<b>T-0734</b>	<b>SSS - Transfer Girder Elevations</b>	<b>Closed</b>	<b>01</b>	<b>09/13/2013</b>	<b>09/23/2013</b>	<b>09/25/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Documents: S1-2303 thru S1-2307,  Elevations for transfer girders shown on drawings S1-2303 thru S1-2307 are in decimal feet. Once converted to feet/ inches they become 1/16th values.  Please verify if the elevations should be rounded up to the nearest 1/8th of an inch or kept as converted.  See attached specific conversions for each transfer girder locations		<b>ANSWER:</b> Reference Documents: S1-2303 thru S1-2307,  Elevations for transfer girders shown on drawings S1-2303 thru S1-2307 are in decimal feet. Once converted to feet/ inches they become 1/16th values.  Please verify if the elevations should be rounded up to the nearest 1/8th of an inch or kept as converted.  See attached specific conversions for each transfer girder locations				
<b>T-0735</b>	<b>SSS -Clarification of Lateral Bracing Members</b>	<b>Closed</b>	<b>01</b>	<b>09/16/2013</b>	<b>09/26/2013</b>	<b>09/25/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Drawing: S-0007  Please identify what are considered "LATERAL SYSTEM MEMBERS" as called out in the GENERAL NOTES SS-9, B "REGARDLESS OF THICKNESS ALL TRUSSES, LATERAL SYSTEM MEMBERS (INCLUDING COLUMNS, BRACES, ETC.): 20FT-LB @ 70 DEG. F."		<b>ANSWER:</b> Reference Drawing: S-0007  Please identify what are considered "LATERAL SYSTEM MEMBERS" as called out in the GENERAL NOTES SS-9, B "REGARDLESS OF THICKNESS ALL TRUSSES, LATERAL SYSTEM MEMBERS (INCLUDING COLUMNS, BRACES, ETC.): 20FT-LB @ 70 DEG. F."				
<b>T-0736</b>	<b>SSS - PJP Weld Designation at Type 2 Drag Connection</b>	<b>Closed</b>	<b>01</b>	<b>09/16/2013</b>	<b>09/26/2013</b>	<b>09/25/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Drawing: 2/S1-5017 Reference Sketch: SK1  On detail 2/S1-5017 for the Type 2 Drag connection verify at the 2" plates the 1/2" PJP weld is the actual prep or is additional prep required to achieve a 1/2" effective weld requirement (IE; 5/8" prep).		<b>ANSWER:</b> Reference Drawing: 2/S1-5017 Reference Sketch: SK1  On detail 2/S1-5017 for the Type 2 Drag connection verify at the 2" plates the 1/2" PJP weld is the actual prep or is additional prep required to achieve a 1/2" effective weld requirement (IE; 5/8" prep).				
<b>T-0737</b>	<b>SSS - Type 2 Drag Connection Pin Clearance</b>	<b>Closed</b>	<b>01</b>	<b>10/07/2013</b>	<b>10/17/2013</b>	<b>10/09/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						











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	<p>of grade 75 #11 reinforcing where required will be acceptable for use within the typical mat reinforcing installed at 8" O.C.</p> <p>The use of the grade 75 # 11 rebar is expected to supplement the typical #1 0 bar in the following locations, 3rd and 4th layer of Area 6, and 4th layer of Area 7.</p>					<p>implementation of grade 75 #11 reinforcing where required will be acceptable for use within the typical mat reinforcing installed at 8" O.C.</p> <p>The use of the grade 75 # 11 rebar is expected to supplement the typical #1 0 bar in the following locations, 3rd and 4th layer of Area 6, and 4th layer of Area 7.</p>
<b>T-0741</b>	<b>BGP - Pile Location Discrepancy at GL E/34.5 in Zone 4</b>	<b>Closed</b>	<b>CR</b>	<b>09/17/2013</b>	<b>09/27/2013</b>	<b>09/26/2013</b>
	<p><b>From:</b> Webcor Construction LP Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Please reference attached drawing S1-2057 and S1-3007.</p> <p>The bridge pier pile (4'-0" diameter) near grid E/34.5 is shown in SI-2057 to be offset from the typical row of piles shown along gridline 34.7. In addition, detail 1/SI-3007 depicts the pile being located within the pit that is located at gridline E/34.5. However, as per BBI's Beale Street Bridge drawings and as-built conditions, the aforementioned bridge pile is installed in line with the other piles on gridline 34.7.</p> <p>Please confirm the as-built location of the bridge pier is acceptable and the sump pit detail shown in 1/S1-3007 is no longer applicable.</p>					<p><b>ANSWER:</b></p> <p>Please reference attached drawing S1-2057 and S1-3007.</p> <p>The bridge pier pile (4'-0" diameter) near grid E/34.5 is shown in SI-2057 to be offset from the typical row of piles shown along gridline 34.7. In addition, detail 1/SI-3007 depicts the pile being located within the pit that is located at gridline E/34.5. However, as per BBI's Beale Street Bridge drawings and as-built conditions, the aforementioned bridge pile is installed in line with the other piles on gridline 34.7.</p> <p>Please confirm the as-built location of the bridge pier is acceptable and the sump pit detail shown in 1/S1-3007 is no longer applicable.</p>









<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>						
<b>T-0744</b>	<b>BGP - Reinforcement Ties in Knock-Out Corbel and Haunch at SW Corner in Area ' Closed</b>		<b>CR</b>	<b>09/17/2013</b>	<b>09/27/2013</b>	<b>09/18/2013</b>
<p><b>From:</b> Webcor Construction LP      Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Please refer to attached detail drawing 2/S1-2030, excerpt drawing from submittal package TG0600-301.2 and SCCI Sketch SK-RFI-324</p> <p>Per field coordination with TT field engineer, please confirm it is acceptable to omit the pilaster ties of detail 2/S1- 3204 within the body of the haunch provided that:</p> <ul style="list-style-type: none"><li>- The pilaster West corner bar (Bar A in attached sketch) is tied with 135 hooks in both directions</li><li>- Ties shall be #4 bars spaced at 4" o.c.</li><li>- The tie perpendicular to the South wall shall be developed a minimum of 14" into the South wall beyond the haunch</li><li>- The tie parallel to the South wall shall be hooked around the pilaster East corner bar (Bar B in attached sketch)</li><li>- In lieu of two individual ties, it is also acceptable to combine the ties into a single shape with a 90 degree bend at Bar A</li><li>- The extent of the ties shall be from the top of the mat to the top of the haunch, after which Detail 2/S1-3204 will resume</li><li>- The horizontal haunch bars shall terminate with a spliced matching hook</li><li>- The horizontal formsaver bars for the future train tunnel shall be #7 @ 6" O.C. on the inside and outside face of the 3'-0" foundation wall.</li></ul> <p><b>ANSWER:</b></p> <p>Please refer to attached detail drawing 2/S1-2030, excerpt drawing from submittal package TG0600-301.2 and SCCI Sketch SK-RFI-324</p> <p>Per field coordination with TT field engineer, please confirm it is acceptable to omit the pilaster ties of detail 2/S1- 3204 within the body of the haunch provided that:</p> <ul style="list-style-type: none"><li>- The pilaster West corner bar (Bar A in attached sketch) is tied with 135 hooks in both directions</li><li>- Ties shall be #4 bars spaced at 4" o.c.</li><li>- The tie perpendicular to the South wall shall be developed a minimum of 14" into the South wall beyond the haunch</li><li>- The tie parallel to the South wall shall be hooked around the pilaster East corner bar (Bar B in attached sketch)</li><li>- In lieu of two individual ties, it is also acceptable to combine the ties into a single shape with a 90 degree bend at Bar A</li><li>- The extent of the ties shall be from the top of the mat to the top of the haunch, after which Detail 2/S1-3204 will resume</li><li>- The horizontal haunch bars shall terminate with a spliced matching hook</li><li>- The horizontal formsaver bars for the future train tunnel shall be #7 @ 6" O.C. on the inside and outside face of the 3'-0" foundation wall.</li></ul>						
<b>T-0745</b>	<b>BGP - Construction Joint Layout Modifications at Area 6</b>	<b>Closed</b>	<b>CR</b>	<b>09/18/2013</b>	<b>09/28/2013</b>	<b>09/30/2013</b>



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	<p><b>From:</b> Webcor Construction LP                      Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>See attached photos of the construction joint at mat slab area 6 South, near grid line 8.5, and CJ layout drawings.</p> <p>Due to congestion and access SCCI would like to shift the walls and concourse joints at this location 14.5" to the East. This adjustment does not affect any other structure's elements and complies with the CJ parameters outlined in the contract specifications.</p> <p>Please confirm modifying the construction joint layout is acceptable.</p>					
	<p><b>ANSWER:</b></p> <p>See attached photos of the construction joint at mat slab area 6 South, near grid line 8.5, and CJ layout drawings.</p> <p>Due to congestion and access SCCI would like to shift the walls and concourse joints at this location 14.5" to the East. This adjustment does not affect any other structure's elements and complies with the CJ parameters outlined in the contract specifications.</p> <p>Please confirm modifying the construction joint layout is acceptable.</p>					
<b>T-0746</b>	<b>BGP - Plumbing Clarifications to 2" Vent and 3" San Connection in Area 4</b>	<b>Open</b>	<b>CR</b>	<b>09/18/2013</b>	<b>09/28/2013</b>	<b>09/20/2013</b>
	<p><b>From:</b> Webcor Construction LP                      Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Pleaes refer to attached plumbing drawing PSK-2022 (dated 04/26/2013) and IR Report 1633.</p> <p>On 09/10/2013, the SFDBI inspector expressed concern about the installation of the 2" vent and 3" connections in the mat slab area 4 - See IR 1633.</p> <p>Please confirm the 2" vent and 3" connection pipes are to be installed per drawing PSK-2022..</p>					
	<p><b>ANSWER:</b></p> <p>Pleaes refer to attached plumbing drawing PSK-2022 (dated 04/26/2013) and IR Report 1633.</p> <p>On 09/10/2013, the SFDBI inspector expressed concern about the installation of the 2" vent and 3" connections in the mat slab area 4 - See IR 1633.</p> <p>Please confirm the 2" vent and 3" connection pipes are to be installed per drawing PSK-2022..</p>					



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<b>T-0747</b>	<b>SSS - BU Girder Size at Roof GL 28</b>	<b>Closed</b>	<b>01</b>	<b>09/20/2013</b>	<b>09/30/2013</b>	<b>09/25/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Drawing: S1-4114 Reference Sketch: CD RFI 015 SK1 attached.  Reference detail A/S1-4114 which does not indicated the built-up girder size at the Roof Park Level between column lines E.6 and D.4 (see CD RFI-015 SK1 attachment). It appears from the latest Revit model that the BU girder is intended to be BU66x30x1.5x2.25. Please confirm the size provided on the Revit model is accurate or advise the girder size to be used at this location.		<b>ANSWER:</b>  Reference Drawing: S1-4114 Reference Sketch: CD RFI 015 SK1 attached.  Reference detail A/S1-4114 which does not indicated the built-up girder size at the Roof Park Level between column lines E.6 and D.4 (see CD RFI-015 SK1 attachment). It appears from the latest Revit model that the BU girder is intended to be BU66x30x1.5x2.25. Please confirm the size provided on the Revit model is accurate or advise the girder size to be used at this location.				
<b>T-0748</b>	<b>SSS - Type TTT Threadbar Anchor Bolt Embedment</b>	<b>Closed</b>	<b>01</b>	<b>09/20/2013</b>	<b>09/30/2013</b>	<b>09/23/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Drawings: S1-5051  Reference S1-5051 which indicates the embedment depths for Type T and TT threadbar anchors are to be 3'-8" and 2'-8", respectively, while the embedment depth for type TTT threadbar anchors is to be 16'-0". Please verify the embedment depth for Type TTT threadbar anchors is to be 16'-0" as indicated on 4/S1-5051.		<b>ANSWER:</b>  Reference Drawings: S1-5051  Reference S1-5051 which indicates the embedment depths for Type T and TT threadbar anchors are to be 3'-8" and 2'-8", respectively, while the embedment depth for type TTT threadbar anchors is to be 16'-0". Please verify the embedment depth for Type TTT threadbar anchors is to be 16'-0" as indicated on 4/S1-5051.				
<b>T-0749</b>	<b>SSS - Anchor Bolt Finish Requirement</b>	<b>Closed</b>	<b>01</b>	<b>09/20/2013</b>	<b>09/30/2013</b>	<b>09/25/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Drawing: S1-5051, S-0007 Reference Specification: 05 10 00 3.2.P.6  Reference is made to the base plate anchor rod schedule on 7/S1-5051 indicating anchor rods are to conform to either ASTM A615 or A722 standards. While ASTM A615 does not explicitly state finish requirements, A722 calls for all bars to be uncoated. Within the IFC documents, Specification Section 05 10 00 3.2.P.6 and Note SS-10 on S-0007 call for miscellaneous metals and exposed steel to be the hot-dipped galvanized.		<b>ANSWER:</b>  Reference Drawing: S1-5051, S-0007 Reference Specification: 05 10 00 3.2.P.6  Reference is made to the base plate anchor rod schedule on 7/S1-5051 indicating anchor rods are to conform to either ASTM A615 or A722 standards. While ASTM A615 does not explicitly state finish requirements, A722 calls for all bars to be uncoated. Within the IFC documents, Specification Section 05 10 00 3.2.P.6 and Note SS-10 on S-0007 call for miscellaneous metals and exposed steel to be the hot-				





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<b>T-0755</b>	<b>BGP - Area 10 Clear Cover to the Vertical Reinforcement on the Foundation Wall</b>	<b>Closed</b>	<b>01</b>	<b>10/11/2013</b>	<b>10/21/2013</b>	<b>10/18/2013</b>
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Further to response to RFI T-609 this RFI shows the areas of foundation wall/embedded column in pour Area 10, on the north & south wall elevations which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A Exhibit - B & C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement. RFI T - 743 shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 10.  Please confirm that the clear cover between the waterproofing system and the vertical reinforcement as outlined at these locations is acceptable.			Further to response to RFI T-609 this RFI shows the areas of foundation wall/embedded column in pour Area 10, on the north & south wall elevations which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A Exhibit - B & C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement. RFI T - 743 shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 10.  Please confirm that the clear cover between the waterproofing system and the vertical reinforcement as outlined at these locations is acceptable.			
<b>T-0756</b>	<b>BGP - Structural Slurry Primer in Mat Slab</b>	<b>Closed</b>	<b>CR</b>	<b>09/24/2013</b>	<b>10/04/2013</b>	<b>09/25/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please refer to the attached letter, authored by Rober Foley (CEMEX QC), dated September 17, 2013.  With limited site access, many Mat Slab pours will require a larger than normal amount of slick-line. To ensure that no slick-line gets plugged, SCCI is proposing to prime the slick-line with a structural slurry that will reach and exceed the specified design strength for the Mat Slab. A miniscule percentage of this primer will be deposited into the mat slab. This percentage would amount to .01 to .02 percent by volume.  Please confirm the proposed SCCI method of slick-line priming is acceptable.			Please refer to the attached letter, authored by Rober Foley (CEMEX QC), dated September 17, 2013.  With limited site access, many Mat Slab pours will require a larger than normal amount of slick-line. To ensure that no slick-line gets plugged, SCCI is proposing to prime the slick-line with a structural slurry that will reach and exceed the specified design strength for the Mat Slab. A miniscule percentage of this primer will be deposited into the mat slab. This percentage would amount to .01 to .02 percent by volume.  Please confirm the proposed SCCI method of slick-line priming is acceptable.			
<b>T-0757</b>	<b>SSS - HSS Vertical Post Size at Roof Park Level</b>	<b>Closed</b>	<b>01</b>	<b>09/25/2013</b>	<b>10/05/2013</b>	<b>09/26/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			



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T-0758	Reference Drawings: 2/S1-7109, A&C/S1-7136  At grid lines D.4 and E.6, west of grid line 24.9, four HSS vertical posts were added per ASI No. 0105. Please provide the missing HSS vertical post sizes at the indicated locations above the Roof Park Level (reference CD RFI 021 SK1 & SK2).	Closed	01	09/25/2013	10/05/2013	10/11/2013
	Reference Drawings: 2/S1-7109, A&C/S1-7136  At grid lines D.4 and E.6, west of grid line 24.9, four HSS vertical posts were added per ASI No. 0105. Please provide the missing HSS vertical post sizes at the indicated locations above the Roof Park Level (reference CD RFI 021 SK1 & SK2).					
T-0758	SSS - W12 Beam Information at Roof Level GL E.1	Closed	01	09/25/2013	10/05/2013	10/11/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:			ANSWER:			
Reference is made to Drawing S1-2602 regarding the W12x14 beam stubs near grid line E, east of grid line 1. Please verify the following W12x14 beam characteristics as noted on CD RFI 027 SK1:			Reference is made to Drawing S1-2602 regarding the W12x14 beam stubs near grid line E, east of grid line 1. Please verify the following W12x14 beam characteristics as noted on CD RFI 027 SK1:			
1) Please supply the location, length, and elevation for W12x14 beams between grids E.2 and E.6.			1) Please supply the location, length, and elevation for W12x14 beams between grids E.2 and E.6.			
2) Please verify the member sizes for the three areas noted between grids D & E.2 are to be W12x14.			2) Please verify the member sizes for the three areas noted between grids D & E.2 are to be W12x14.			
3) For the same areas indicated in item #2, please supply the beam locations, lengths, and elevations.			3) For the same areas indicated in item #2, please supply the beam locations, lengths, and elevations.			
T-0759	Reference Drawings: 2/S1-7109, A&C/S1-7136  At grid lines D.4 and E.6, west of grid line 24.9, four HSS vertical posts were added per ASI No. 0105. Please provide the missing HSS vertical post sizes at the indicated locations above the Roof Park Level (reference CD RFI 021 SK1 & SK2).	Closed	01	09/25/2013	10/05/2013	10/27/2013
	Reference Drawings: 2/S1-7109, A&C/S1-7136  At grid lines D.4 and E.6, west of grid line 24.9, four HSS vertical posts were added per ASI No. 0105. Please provide the missing HSS vertical post sizes at the indicated locations above the Roof Park Level (reference CD RFI 021 SK1 & SK2).					
T-0759	SSS - Beam Camber Dimensions at Ground Level	Closed	01	09/25/2013	10/05/2013	09/27/2013
From: Webcor Construction LP                      Robert Kjome						
REQUEST:			ANSWER:			
Reference is made to Drawings S1-2303 and S1-2304 near grids F.9 and G.13. Please verify the following:			Reference is made to Drawings S1-2303 and S1-2304 near grids F.9 and G.13. Please verify the following:			
1) S1-2303 indicates the W30x90 beam near grid F.9 is to have a 3 ¼" camber (reference CD RFI 028 SK1). Please verify the camber should be ¾" in lieu of the 3 ¼" dimension indicated.			1) S1-2303 indicates the W30x90 beam near grid F.9 is to have a 3 ¼" camber (reference CD RFI 028 SK1). Please verify the camber should be ¾" in lieu of the 3 ¼" dimension indicated.			
2) S1-2304 indicates that three W24x76 beams between grids F/G & 13/14 are to have a 3 ¼" camber (reference CD RFI 028 SK 2). Please verify the cambers should be ¾" in lieu of the 3 ¼" dimension indicated.			2) S1-2304 indicates that three W24x76 beams between grids F/G & 13/14 are to have a 3 ¼" camber (reference CD RFI 028 SK 2). Please verify the cambers should be ¾" in lieu of the 3 ¼" dimension indicated.			





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	the "haunch."				reinforcement at the "haunch."	
<b>T-0763</b>	<b>SSS - MC10 Link Brace Foul at Roof Perimeter</b>	<b>Closed</b>	<b>01</b>	<b>09/26/2013</b>	<b>10/06/2013</b>	<b>10/02/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference detail 5/S1-4205 showing the link brace detail at the roof along column lines B and H. In the IFC documents, the beam size was increased from W21x55 to W24x55, causing the MC10x41.1 brace (increased from M8x22.6) to foul the beam flange. See CD RFI 020 SK 1 & SK2 attached for reference and advise on resolution for the foul noted.					<b>ANSWER:</b> Reference detail 5/S1-4205 showing the link brace detail at the roof along column lines B and H. In the IFC documents, the beam size was increased from W21x55 to W24x55, causing the MC10x41.1 brace (increased from M8x22.6) to foul the beam flange. See CD RFI 020 SK 1 & SK2 attached for reference and advise on resolution for the foul noted.	
<b>T-0764</b>	<b>SSS - Built-Up Beams - Plate Yield Strength</b>	<b>Closed</b>	<b>01</b>	<b>09/26/2013</b>	<b>10/06/2013</b>	<b>09/30/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Please reference contract sheet S-0007, specifically structural steel note SS-1, which indicates that plates used for flanges in built up beams shall meet the criteria of ASTM Designation A572-50 (UNO) and have a maximum yield point of 58ksi.  The plate mills will not guarantee material that meets the A572-50 criteria and further restricts the yield to a maximum of 58ksi. The plate mills will guarantee material that will yield within the range of 50ksi - 65ksi. Attached you will find correspondence with two major US steel mills for reference.  Please confirm, for the plates used for flanges in built-up members produced from A572-50 material, that a yield range of 50ksi - 65ksi is acceptable.					<b>ANSWER:</b> Please reference contract sheet S-0007, specifically structural steel note SS-1, which indicates that plates used for flanges in built up beams shall meet the criteria of ASTM Designation A572-50 (UNO) and have a maximum yield point of 58ksi.  The plate mills will not guarantee material that meets the A572-50 criteria and further restricts the yield to a maximum of 58ksi. The plate mills will guarantee material that will yield within the range of 50ksi - 65ksi. Attached you will find correspondence with two major US steel mills for reference.  Please confirm, for the plates used for flanges in built-up members produced from A572-50 material, that a yield range of 50ksi - 65ksi is acceptable.	



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<b>T-0766</b>	<b>SSS - Stiffener Requirements at Column Base Detail</b>	<b>Closed</b>	<b>01</b>	<b>09/27/2013</b>	<b>10/07/2013</b>	<b>10/02/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Please confirm the following in reference to the column base details shown on S1-5051.  a) With reference to Drawing S1-5051, please confirm that only the Type I base plates have vertical stiffeners at the column flanges and web, while the Type II and Type III base plates have vertical stiffeners only at the column web.  b) With reference to Details 4 and 6 on Drawing S1-5051, please confirm the base plate type and column indicated in these details are for graphical purposes only and do not indicate the type of base plate to be used with the detailed threadbar anchor.		<b>ANSWER:</b>  Please confirm the following in reference to the column base details shown on S1-5051.  a) With reference to Drawing S1-5051, please confirm that only the Type I base plates have vertical stiffeners at the column flanges and web, while the Type II and Type III base plates have vertical stiffeners only at the column web.  b) With reference to Details 4 and 6 on Drawing S1-5051, please confirm the base plate type and column indicated in these details are for graphical purposes only and do not indicate the type of base plate to be used with the detailed threadbar anchor.				
<b>T-0767</b>	<b>SSS - Herrick RFI 01 - W shapes from BU</b>	<b>Closed</b>	<b>01</b>	<b>09/27/2013</b>	<b>10/07/2013</b>	<b>10/04/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  The following sizes and shapes are not available in the U.S. Please confirm all sizes will be Built up sections from plate.  W40 X 392 Grade A992, W40 X 503 Grade A992, W40 X 593 Grade A992 will be fabricated with A572-50 Plate. W40 X 392 Grade A709, W40 X 503 Grade A709, W40 X 593 Grade A709 at the Bus Deck will be fabricated with A709-50 Plate.  Welding Preparation of Built up sections to follow.		<b>ANSWER:</b>  The following sizes and shapes are not available in the U.S. Please confirm all sizes will be Built up sections from plate.  W40 X 392 Grade A992, W40 X 503 Grade A992, W40 X 593 Grade A992 will be fabricated with A572-50 Plate. W40 X 392 Grade A709, W40 X 503 Grade A709, W40 X 593 Grade A709 at the Bus Deck will be fabricated with A709-50 Plate.  Welding Preparation of Built up sections to follow.				
<b>T-0767.1</b>	<b>SSS - Fillet Weld Sizes for Built up Members</b>	<b>Closed</b>	<b>01</b>	<b>10/18/2013</b>	<b>10/28/2013</b>	<b>10/30/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  The response to Webcor/Obayashi RFI T-0767 (SK RFI 013) confirmed that equivalent built-up sections can be provided in lieu of W shapes not available domestically. All W40 X 503 and W40 X 593 at the Ground level (reference		<b>ANSWER:</b>  The response to Webcor/Obayashi RFI T-0767 (SK RFI 013) confirmed that equivalent built-up sections can be provided in lieu of W shapes not available domestically. All W40 X 503 and W40 X 593 at the				









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<b>T-0769</b>	<b>SSS - Verify Beam Locations at Ground Level East</b>	<b>Closed</b>	<b>01</b>	<b>09/30/2013</b>	<b>10/10/2013</b>	<b>10/02/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Drawings: S1-2305, S1-2306, and S1-2307  As indicated on the sketches attached, there are beams which have not been located on the referenced drawings. The dimensions provided and clouded in red are taken from the latest Revit model. Please verify all clouded dimensions required to located the steel in question.		<b>ANSWER:</b>  Reference Drawings: S1-2305, S1-2306, and S1-2307  As indicated on the sketches attached, there are beams which have not been located on the referenced drawings. The dimensions provided and clouded in red are taken from the latest Revit model. Please verify all clouded dimensions required to located the steel in question.				
<b>T-0769.1</b>	<b>SSS - Verify Beam Locations at Ground Level East</b>	<b>Closed</b>	<b>CR</b>	<b>11/22/2013</b>	<b>12/02/2013</b>	<b>12/13/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  On the response to Webcor RFI # T-0769 (SK RFI # SK 050) & T-0801 (SK RFI # 066) we have reviewed and located most of the beam locations in question using the nearest gridlines, architectural dwg's, partial plans, equal spacing, etc per the noted guidelines in the response. However on drawings S1-2302, S1-2303 & S1-2304 there are still some beam locations that cannot be located and require verification therefore on sketches CD RFI 047.1 SK1 to SK3 please verify all clouded dimensions in RED as noted to close this RFI.		<b>ANSWER:</b>  On the response to Webcor RFI # T-0769 (SK RFI # SK 050) & T-0801 (SK RFI # 066) we have reviewed and located most of the beam locations in question using the nearest gridlines, architectural dwg's, partial plans, equal spacing, etc per the noted guidelines in the response. However on drawings S1-2302, S1-2303 & S1-2304 there are still some beam locations that cannot be located and require verification therefore on sketches CD RFI 047.1 SK1 to SK3 please verify all clouded dimensions in RED as noted to close this RFI.				
<b>T-0770</b>	<b>SSS - Verify Beam Locations at Roof Park Level West</b>	<b>Closed</b>	<b>01</b>	<b>09/30/2013</b>	<b>10/10/2013</b>	<b>10/02/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference Drawings: S1-2602  See attached CD RFI 026 SK1 showing missing dimensions required to locate beams at the Roof Park Level Zone 02. Please verify all dimensions indicated in red, which have been taken from the latest Revit model, are accurate to locate the steel in question.		<b>ANSWER:</b>  Reference Drawings: S1-2602  See attached CD RFI 026 SK1 showing missing dimensions required to locate beams at the Roof Park Level Zone 02. Please verify all dimensions indicated in red, which have been taken from the latest Revit model, are accurate to locate the steel in question.				
<b>T-0770.1</b>	<b>SSS - Verify Additional Beam Locations at Roof Park Level West</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/31/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						



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On the response to Webcor RFI # T-0770 (SK RFI # SK 052) & T-0769 (SK RFI # 050) we have reviewed and located most of the beam locations in question using the nearest gridlines, architectural dwg's, partial plans, equal spacing, etc per the noted guidelines in the response. However, on drawings S1-2602 to S1-2607 there are still some beam locations that cannot be located and require verification; therefore, on sketches CD RFI 026.1 SK1 to SK6 please verify all clouded dimensions in RED as noted to close this RFI.			On the response to Webcor RFI # T-0770 (SK RFI # SK 052) & T-0769 (SK RFI # 050) we have reviewed and located most of the beam locations in question using the nearest gridlines, architectural dwg's, partial plans, equal spacing, etc per the noted guidelines in the response. However, on drawings S1-2602 to S1-2607 there are still some beam locations that cannot be located and require verification; therefore, on sketches CD RFI 026.1 SK1 to SK6 please verify all clouded dimensions in RED as noted to close this RFI.			
T-0771	SSS - Lower Concourse Anchor Bolt Details	Closed	01	09/30/2013	10/10/2013	10/04/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:			ANSWER:			
Please reference Drawing S1-5051 and confirm the following in regards to the column base details:			Please reference Drawing S1-5051 and confirm the following in regards to the column base details:			
1) Confirm it is acceptable to oversize the holes for anchor bolt penetrations through base plate per AISC's 13th Edition Table 14-2 (reference CD RFI 016 SK2 attached).			1) Confirm it is acceptable to oversize the holes for anchor bolt penetrations through base plate per AISC's 13th Edition Table 14-2 (reference CD RFI 016 SK2 attached).			
2) Confirm the hole sizes indicated in Part 1 are acceptable for anchor bolt penetrations through the horizontal column stiffener.			2) Confirm the hole sizes indicated in Part 1 are acceptable for anchor bolt penetrations through the horizontal column stiffener.			
3) Confirm it is acceptable to supply a ½" x 4" x 4" (A36) plate washer above the column stiffener with a 1/16" oversize hole.			3) Confirm it is acceptable to supply a ½" x 4" x 4" (A36) plate washer above the column stiffener with a 1/16" oversize hole.			
4) Confirm it is acceptable to locate the grout holes typically as shown on CD RFI 016 SK3.			4) Confirm it is acceptable to locate the grout holes typically as shown on CD RFI 016 SK3.			
5) To aid in the alignment of the thread bar anchor rods during concrete operations, please confirm it is acceptable to provide one ½" thick anchor plate at the base of the thread bars with size to match the base plate in lieu of four separate ½" x 4" x 4" anchor plates.			5) To aid in the alignment of the thread bar anchor rods during concrete operations, please confirm it is acceptable to provide one ½" thick anchor plate at the base of the thread bars with size to match the base plate in lieu of four separate ½" x 4" x 4" anchor plates.			
6) Confirm the thickness of the stiffener for Type II and Type III column bases is to be 2".			6) Confirm the thickness of the stiffener for Type II and Type III column bases is to be 2".			
7) Confirm an anchor bolt projection of 2.5 x AB dia. above the plate washer on top of the column stiffener is						



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	acceptable. See CD RFI 030 SK3 for reference.					
	8) Confirm an anchor bolt extension of 2.5 x AB dia. below the bottom plate washer is acceptable. See CD RFI 030 SK3 for reference.				7) Confirm an anchor bolt projection of 2.5 x AB dia. above the plate washer on top of the column stiffener is acceptable. See CD RFI 030 SK3 for reference.	
	9) Confirm that the 1" cover as shown on CD RFI 030 SK3 is acceptable.				8) Confirm an anchor bolt extension of 2.5 x AB dia. below the bottom plate washer is acceptable. See CD RFI 030 SK3 for reference.	
	10) Confirm that the anchor bolts shall be installed wrench tight.				9) Confirm that the 1" cover as shown on CD RFI 030 SK3 is acceptable.	
					10) Confirm that the anchor bolts shall be installed wrench tight.	
<b>T-0771.1</b>	<b>SSS - Lower Concourse Anchor Bolt Details at Column Base</b>	<b>Closed</b>	<b>01</b>	<b>10/11/2013</b>	<b>10/21/2013</b>	<b>10/14/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
As per the response to RFI# T-0771 please confirm the following:			As per the response to RFI# T-0771 please confirm the following:			
1. For items 8 & 9 please confirm it is acceptable to have 0" cover at the underside of the concrete beam. See attached sketch SK-1 for clarification.			1. For items 8 & 9 please confirm it is acceptable to have 0" cover at the underside of the concrete beam. See attached sketch SK-1 for clarification.			
2. For item 5 please confirm it is acceptable to use an alignment plate with a 7" diameter center hole to allow for the consolidation of concrete and aid the alignment of the threaded bar. See attached sketch SK-2 for clarification.			2. For item 5 please confirm it is acceptable to use an alignment plate with a 7" diameter center hole to allow for the consolidation of concrete and aid the alignment of the threaded bar. See attached sketch SK-2 for clarification.			



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T-0774	BGP-Pre-cutting of CDSM Soldier Pile	Closed	01	09/30/2013	10/10/2013	10/21/2013
<p>conflicts while maintaining minimum bend radius' and spacing of all 10 loops and was unsuccessful.</p> <p>Please see the attached revised drawing of the Geothermal Piping at Field 12 and confirm it is acceptable.</p>		<p>miscellaneous micropile conflicts while maintaining minimum bend radius' and spacing of all 10 loops and was unsuccessful.</p> <p>Please see the attached revised drawing of the Geothermal Piping at Field 12 and confirm it is acceptable.</p>				
<p><b>From:</b> Webcor Construction LP                      Michael Spillane</p> <p><b>REQUEST:</b></p> <p>Further, in response to RFI T-725, WOJV is proposing to pre-cut the inside flange of the CDSM beams at the required cut off elevations prior to the installation of the waterproofing system see exhibit A for details, This pre-cutting of the CDSM beams would minimize the possibility of heat damage to the waterproofing system. The remainder of the CDSM beam cutting and top section removal will be completed by the TG012.1 Civil Sitework contractor.</p> <p>Please confirm if this would be acceptable.</p>		<p><b>ANSWER:</b></p> <p>Further, in response to RFI T-725, WOJV is proposing to pre-cut the inside flange of the CDSM beams at the required cut off elevations prior to the installation of the waterproofing system see exhibit A for details, This pre-cutting of the CDSM beams would minimize the possibility of heat damage to the waterproofing system. The remainder of the CDSM beam cutting and top section removal will be completed by the TG012.1 Civil Sitework contractor.</p> <p>Please confirm if this would be acceptable.</p>				



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<b>T-0775</b>	<b>BGP-Concrete strength requirement for bracing Removal</b>	<b>Closed</b>	<b>01</b>	<b>09/30/2013</b>	<b>10/10/2013</b>	<b>10/10/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Michael Spillane</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
In accordance with Spec section 31-55-00 1.4 J the contractor is to submit concrete strength results to the design team prior to the removal of internal bracing. In order to fulfill this requirement the contractor has asked the following questions.			In accordance with Spec section 31-55-00 1.4 J the contractor is to submit concrete strength results to the design team prior to the removal of internal bracing. In order to fulfill this requirement the contractor has asked the following questions.			
1. What is the criteria for bracing removal for instant if the average strength of the concrete cylinders tested is calculated to be above the design strength can the internal bracing be removed?			1. What is the criteria for bracing removal for instant if the average strength of the concrete cylinders tested is calculated to be above the design strength can the internal bracing be removed?			
2. Is there any tolerance on the design strength requirement for bracing removal, for example if the concrete has reached 90% of design strength could the bracing be removed? Obviously this could have a positive effect on the construction schedule.			2. Is there any tolerance on the design strength requirement for bracing removal, for example if the concrete has reached 90% of design strength could the bracing be removed? Obviously this could have a positive effect on the construction schedule.			
<b>T-0775.1</b>	<b>BGP-Concrete strength requirement for level D bracing removal</b>	<b>Closed</b>	<b>01</b>	<b>10/09/2013</b>	<b>10/19/2013</b>	<b>10/16/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Michael Spillane</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
To clarify question 2 in RFI T-0775			To clarify question 2 in RFI T-0775			
WOJV is requesting that the level D bracing be removed once the concrete in the mat slab beneath has reached 75% of its design strength.			WOJV is requesting that the level D bracing be removed once the concrete in the mat slab beneath has reached 75% of its design strength.			
Please confirm if this would be acceptable.			Please confirm if this would be acceptable.			



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<b>T-0775.2</b>	<b>BGP-Concrete strength requirement for the level D bracing removal</b>	<b>Closed</b>	<b>01</b>	<b>11/15/2013</b>	<b>11/25/2013</b>	<b>11/20/2013</b>
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>  Further to response to RFI T-0775.1, Please find attached supporting calculations to justify that the concrete in the mat slab is sufficient at 3000 psi to removal the level D bracing.  Please confirm that this is acceptable						<b>ANSWER:</b>  Further to response to RFI T-0775.1, Please find attached supporting calculations to justify that the concrete in the mat slab is sufficient at 3000 psi to removal the level D bracing.  Please confirm that this is acceptable
<b>T-0776</b>	<b>BGP - Mat Slab Construction Joint Between Area 2 and Area 4</b>	<b>Closed</b>	<b>CR</b>	<b>10/01/2013</b>	<b>10/11/2013</b>	<b>10/03/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached excerpt drawing CJ-04 from submittal package TG0600-030.3, Item ID #033000-003.3.  As discussed and coordinated in various Progress Meetings, SCCI plans to combine slab pours S102 and S104 into one pour without bulkhead forms in between. The specifcations do not restrict SCCI from using bulkheads in the east and west directions. The returned construction joint layout shop drawing review comments do not reflect the coordinated revised construction joint.  Please confirm it is acceptable to combine slab placement areas S102 and S104 into one pour without bulkhead forms in between.						<b>ANSWER:</b>  Please refer to attached excerpt drawing CJ-04 from submittal package TG0600-030.3, Item ID #033000-003.3.  As discussed and coordinated in various Progress Meetings, SCCI plans to combine slab pours S102 and S104 into one pour without bulkhead forms in between. The specifcations do not restrict SCCI from using bulkheads in the east and west directions. The returned construction joint layout shop drawing review comments do not reflect the coordinated revised construction joint.  Please confirm it is acceptable to combine slab placement areas S102 and S104 into one pour without bulkhead forms in between.
<b>T-0777</b>	<b>BGP - FF &amp; FL Values for Concourse Slab</b>	<b>Closed</b>	<b>CR</b>	<b>10/02/2013</b>	<b>10/12/2013</b>	<b>10/17/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached RFI T-0691.  This RFI is being submitted in response to RFI response T-0691. As per contract specification section 033020, Section 3.6.B the concrete finish of the lower concourse slab notes an FF value of 20.  Table 8.15.3b of ACI 302.1R (page 46) states that to						<b>ANSWER:</b>  Please refer to attached RFI T-0691.  This RFI is being submitted in response to RFI response T-0691. As per contract specification section 033020, Section 3.6.B the concrete finish of the lower concourse slab notes an FF value of 20.  Table 8.15.3b of ACI 302.1R (page 46) states that to



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T-0778	achieve a surface with an FF value of 20, it must be a smooth floated surface. ACI 302.1R does not provide any recommendations of "F" numbers for broomed surfaces.	Closed	CR	10/02/2013	10/12/2013	10/25/2013
	Please confirm the design intent for the concourse slab finish: 1. To have a rough broom/rake finish or 2. To have the concourse slab finished to an FF value of 20.					
T-0778.1	achieve a surface with an FF value of 20, it must be a smooth floated surface. ACI 302.1R does not provide any recommendations of "F" numbers for broomed surfaces.	Closed	CR	10/28/2013	11/07/2013	10/30/2013
	Please confirm the design intent for the concourse slab finish: 1. To have a rough broom/rake finish or 2. To have the concourse slab finished to an FF value of 20.					
T-0778.2	achieve a surface with an FF value of 20, it must be a smooth floated surface. ACI 302.1R does not provide any recommendations of "F" numbers for broomed surfaces.	Closed	CR	12/20/2013	12/30/2013	12/26/2013
	Please confirm the design intent for the concourse slab finish: 1. To have a rough broom/rake finish or 2. To have the concourse slab finished to an FF value of 20.					









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T-0779.3	<b>BGP - Electrical Equipment and Box Layout in Electrical Room B2461 - Area 8</b>  <b>From:</b> Webcor Construction LP Jackson Tukuafu  <b>REQUEST:</b>  Please reference RFI #T-0779, drawing EI-2024, and Spec Section 26 05 34.  As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."  Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-336.1 for Electrical Room B2461 in Area 08 is acceptable. Please refer to the conduit layout in submittal shop drawing package TG0600-905.  Please note this RFI is being remitted per coordination meeting between AAI, WOJV, SCCI and TCCO on 1/10, to exclude SCCI's version of the RFI which makes reference to cost impacts.	Closed	CR	01/28/2014	02/07/2014	02/10/2014
						<b>ANSWER:</b>  Please reference RFI #T-0779, drawing EI-2024, and Spec Section 26 05 34.  As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."  Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-336.1 for Electrical Room B2461 in Area 08 is acceptable. Please refer to the conduit layout in submittal shop drawing package TG0600-905.  Please note this RFI is being remitted per coordination meeting between AAI, WOJV, SCCI and TCCO on 1/10, to exclude SCCI's version of the RFI which makes reference to cost impacts.



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<b>T-0780</b>	<b>BGP - Electrical Equipment and Box Layout in Electrical Room B2460 - Area 08</b>	<b>Closed</b>	<b>CR</b>	<b>10/02/2013</b>	<b>10/12/2013</b>	<b>10/14/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference drawing E1-2026, A1-2104 and Spec Section 26 05 34.			Please reference drawing E1-2026, A1-2104 and Spec Section 26 05 34.			
As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, please confirm the proposed "...location of outlets, fixtures and equipment..." layout as shown in the attached SCCI sketch SK-RFI-335 for Electrical Room B2460 in Area 08 is acceptable.			As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, please confirm the proposed "...location of outlets, fixtures and equipment..." layout as shown in the attached SCCI sketch SK-RFI-335 for Electrical Room B2460 in Area 08 is acceptable.			
Please advise.			Please advise.			
<b>T-0780.1</b>	<b>BGP - Electrical Equipment and Box Layout in Electrical Room B2460 - Area 08</b>	<b>Closed</b>	<b>CR</b>	<b>10/28/2013</b>	<b>11/07/2013</b>	<b>10/30/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI #T-0780, drawings EI-2026 and AI-2104, and Spec Section 26 05 34.			Please reference RFI #T-0780, drawings EI-2026 and AI-2104, and Spec Section 26 05 34.			
RFI #T - 0780 response proposes layout for electrical equipment and box layout in Electrical Room B2460 - Area 08 in CAD format. See attached.			RFI #T - 0780 response proposes layout for electrical equipment and box layout in Electrical Room B2460 - Area 08 in CAD format. See attached.			
Please confirm that the layout is acceptable.			Please confirm that the layout is acceptable.			
<b>T-0780.2</b>	<b>BGP - Electrical Equipment and Box Layout in Electrical Room B2460 - Area 08</b>	<b>Closed</b>	<b>CR</b>	<b>12/20/2013</b>	<b>12/20/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI #T-0780, drawings EI-2026 and AI-2104, and Spec Section 26 05 34.			Please reference RFI #T-0780, drawings EI-2026 and AI-2104, and Spec Section 26 05 34.			
As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."			As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."			
Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-335.1 for Electrical Room			Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-335.1 for			



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	<div>B2460 in Area 08 is acceptable. Please refer to the conduit layout in submittal shop drawing package TG0600-905.</div>					<div>Electrical Room B2460 in Area 08 is acceptable. Please refer to the conduit layout in submittal shop drawing package TG0600-905.</div>
T-0781	<div>BGP - Electrical Equipment and Box Layout in Electrical Room B2441 - Area 09</div> <div>From: Webcor Construction LP Jackson Tukuafu</div>	Closed	CR	10/02/2013	10/12/2013	10/10/2013
	<div>REQUEST:</div> <div>Please reference drawing E1-2024, A1-2104 and Spec Section 26 05 34.</div> <div>As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, please confirm the proposed "...location of outlets, fixtures and equipment..." layout as shown in the attached SCCI sketch SK-RFI-334 for Electrical Room B2441 in Area 09 is acceptable.</div> <div>Please advise.</div>					<div>ANSWER:</div> <div>Please reference drawing E1-2024, A1-2104 and Spec Section 26 05 34.</div> <div>As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, please confirm the proposed "...location of outlets, fixtures and equipment..." layout as shown in the attached SCCI sketch SK-RFI-334 for Electrical Room B2441 in Area 09 is acceptable.</div> <div>Please advise.</div>
T-0781.1	<div>BGP - Electrical Equipment and Box Layout in Electrical Room B2441 - Area 09</div> <div>From: Webcor Construction LP Jackson Tukuafu</div>	Closed	CR	10/28/2013	11/07/2013	10/30/2013
	<div>REQUEST:</div> <div>Please reference RFI #T-0781, drawings EI-2024 and AI-2104 and Spec Section 26 05 34.</div> <div>RFI #T-0781 response proposes layout for electrical equipment box layout in Electrical Room B2441 - Area 09 in CAD format. See attached.</div> <div>Please confirm that the layout is acceptable.</div>					<div>ANSWER:</div> <div>Please reference RFI #T-0781, drawings EI-2024 and AI-2104 and Spec Section 26 05 34.</div> <div>RFI #T-0781 response proposes layout for electrical equipment box layout in Electrical Room B2441 - Area 09 in CAD format. See attached.</div> <div>Please confirm that the layout is acceptable.</div>



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<b>T-0781.2</b>	<b>BGP - Electrical Equipment and Box Layout in Electrical Room B2441 - Area 09</b>	<b>Closed</b>	<b>CR</b>	<b>12/20/2013</b>	<b>12/30/2013</b>	
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI #T-0781, drawings EI-2024 and AI-2104 and Spec Section 26 05 34.			Please reference RFI #T-0781, drawings EI-2024 and AI-2104 and Spec Section 26 05 34.			
As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."			As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."			
Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-334.1 for Electrical Room B2441 in Area 09 is acceptable. Please refer to the conduit layout in submittal shop drawing package TG0600-905.			Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-334.1 for Electrical Room B2441 in Area 09 is acceptable. Please refer to the conduit layout in submittal shop drawing package TG0600-905.			
<b>T-0781.3</b>	<b>BGP - Electrical Equipment and Box Layout in Electrical Room B2441 - Area 09</b>	<b>Closed</b>	<b>CR</b>	<b>01/28/2014</b>	<b>02/07/2014</b>	<b>02/10/2014</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI #T-0781, drawings EI-2024 and AI-2104 and Spec Section 26 05 34.			Please reference RFI #T-0781, drawings EI-2024 and AI-2104 and Spec Section 26 05 34.			
As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."			As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."			
Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-334.1 for Electrical Room B2441 in Area 09 is acceptable. Please refer to the conduit layout in submittal shop drawing package TG0600-905.			Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-334.1 for Electrical Room B2441 in Area 09 is acceptable. Please refer to the conduit layout in submittal shop drawing package TG0600-905.			
Please note this RFI is being remitted per coordination meeting between AAI, WOJV, SCCI and TCCO on 1/10, to exclude SCCI's version of the RFI which makes reference to cost impacts.			Please note this RFI is being remitted per coordination meeting between AAI, WOJV, SCCI and TCCO on 1/10, to exclude SCCI's version of the RFI which makes reference to cost impacts.			



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T-0782	BGP - Electrical Equipment and Box Layout in Electrical Room B2560 - Area 09	Closed	CR	10/02/2013	10/02/2013	10/14/2013
<div><div>From: Webcor Construction LP</div><div>Jackson Tukuafu</div></div>						
REQUEST:			ANSWER:			
Please reference drawing E1-2025, A1-2105 and Spec Section 26 05 34.			Please reference drawing E1-2025, A1-2105 and Spec Section 26 05 34.			
As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, please confirm the proposed "...location of outlets, fixtures and equipment..." layout as shown in the attached SCCI sketch SK-RFI-333 for Electrical Room B2560 in Area 09 is acceptable.			As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, please confirm the proposed "...location of outlets, fixtures and equipment..." layout as shown in the attached SCCI sketch SK-RFI-333 for Electrical Room B2560 in Area 09 is acceptable.			
Please advise.			Please advise.			
T-0782.1	BGP - Electrical Equipment and Box Layout in Electrical Room B2560 - Area 10	Closed	CR	10/28/2013	11/07/2013	10/31/2013
<div><div>From: Webcor Construction LP</div><div>Jackson Tukuafu</div></div>						
REQUEST:			ANSWER:			
Please reference RFI #T-0782, drawing EI-2025, AI-2105, and Spec Section 26 05 34.			Please reference RFI #T-0782, drawing EI-2025, AI-2105, and Spec Section 26 05 34.			
RFI #T -0782 response proposes layout for electrical equipment and box layout in Electrical Room B2560 - Area I0 in CAD format. See attached.			RFI #T -0782 response proposes layout for electrical equipment and box layout in Electrical Room B2560 - Area I0 in CAD format. See attached.			
Please confirm that the layout is acceptable.			Please confirm that the layout is acceptable.			
T-0782.2	BGP - Electrical Equipment and Box Layout in Electrical Room B2560 - Area 10	Closed	CR	12/20/2013	12/30/2013	
<div><div>From: Webcor Construction LP</div><div>Jackson Tukuafu</div></div>						
REQUEST:			ANSWER:			
Please reference drawing E1-2025, A1-2105 and Spec Section 26 05 34.			Please reference drawing E1-2025, A1-2105 and Spec Section 26 05 34.			
As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."			As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."			
Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-333.1 for Electrical Room			Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-333.1 for			





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T-0782.3	<b>BGP - Electrical Equipment and Box Layout in Electrical Room B2560 - Area 10</b>  <b>From:</b> Webcor Construction LP                      Jackson Tukuafu  <b>REQUEST:</b>  Please reference drawing E1-2025, A1-2105 and Spec Section 26 05 34.  As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."  Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-333.1 for Electrical Room B2560 in Area 10 is acceptable. Please refer to the conduit layout in submittal shop drawing package TG0600-905.  Please note this RFI is being remitted per coordination meeting between AAI, WOJV, SCCI and TCCO on 1/10, to exclude SCCI's version of the RFI which makes reference to cost impacts.	Closed	CR	01/28/2014	02/07/2014	02/10/2014
						<b>ANSWER:</b>  Please reference drawing E1-2025, A1-2105 and Spec Section 26 05 34.  As per spec section requirement 26 05 34 - Raceways and Boxes, Article 3.2 - B, the "...location of outlets, fixtures and equipment is governed by field conditions...verify final location of outlets, fixture and equipment with the TJPA through the RFI process."  Please confirm the coordinated equipment layout with the knee walls per RFI T-0899 as shown in the attached as-built layout SCCI sketch SK-RFI-333.1 for Electrical Room B2560 in Area 10 is acceptable. Please refer to the conduit layout in submittal shop drawing package TG0600-905.  Please note this RFI is being remitted per coordination meeting between AAI, WOJV, SCCI and TCCO on 1/10, to exclude SCCI's version of the RFI which makes reference to cost impacts.





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0783</b>	<b>BGP- CDSM Soldier Pile Encroachment Area 11</b>	<b>Closed</b>	<b>CR</b>	<b>10/18/2013</b>	<b>10/28/2013</b>	<b>10/24/2013</b>
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 11 as well as all levels of the encroachment into the foundation wall between CDSM piles 188 to 236 on the north elevation and 548 to 571 on the south elevation for Location Plan see exhibit - A</p> <p>Exhibit - B &amp; C depict the location and degree in which the SP are encroaching</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) between CDSM pile 234 to 236, WOJV is proposing to decrease the specified 36" wall thickness to 34' to clear the encroaching SP 235. Originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6" OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 548 to 551 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 550, originally this was a WR1 reinforcement areas #11@8" oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p> <p>See Exhibit - E &amp; F showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>			<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 11 as well as all levels of the encroachment into the foundation wall between CDSM piles 188 to 236 on the north elevation and 548 to 571 on the south elevation for Location Plan see exhibit - A</p> <p>Exhibit - B &amp; C depict the location and degree in which the SP are encroaching</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) between CDSM pile 234 to 236, WOJV is proposing to decrease the specified 36" wall thickness to 34' to clear the encroaching SP 235. Originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6" OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 548 to 551 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 550, originally this was a WR1 reinforcement areas #11@8" oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p> <p>See Exhibit - E &amp; F showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p>			





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0784.1</b>	<b>BGP- CDSM Soldier Pile Encroachment Area 12</b>	<b>Closed</b>	<b>01</b>	<b>03/06/2014</b>	<b>03/16/2014</b>	<b>03/13/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Documents: Exhibits A - G			Reference Documents: Exhibits A - G			
<p>This revised RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 12 as well as all levels of the encroachment into the foundation wall between CDSM piles 235 to 265 on the north elevation and 517 to 548 to on the south elevation for location Plan see exhibit - A Exhibit - B, &amp; C depict the location and degree in which the SP are encroaching</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) between CDSM pile 234 to 237-238, 241-242 to 243, 254 to 257 and 262-263 to 270 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 235,242,255,256,263. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 237-238 to 241-242, WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 241. This foundation wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on detail A/Sk.3 option 2 (Exhibit - E).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 530 to 531 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 531, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 531 to 535 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 531, originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar</p>			<p>This revised RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 12 as well as all levels of the encroachment into the foundation wall between CDSM piles 235 to 265 on the north elevation and 517 to 548 to on the south elevation for location Plan see exhibit - A Exhibit - B, &amp; C depict the location and degree in which the SP are encroaching</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) between CDSM pile 234 to 237-238, 241-242 to 243, 254 to 257 and 262-263 to 270 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 235,242,255,256,263. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 237-238 to 241-242, WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 241. This foundation wall area was originally a WR2 reinforcement area (#11@6"oc EF vertically) and would change to #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on detail A/Sk.3 option 2 (Exhibit - E).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 530 to 531 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 531, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p>			



Webcor/Obayashi Joint Venture  
*PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG*  
30100 - Transbay Transit Center Project

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Date: 02/11/2015  
Time: 07:01 AM  
Job: 30100

<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
	<p>spacing predicated on Detail A/Sk.3 option 2 (Exhibit - E).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings. See Exhibit - E, F &amp; G showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>					<p>clear the encroaching SP 531, originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit - E). In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings. See Exhibit - E, F &amp; G showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>
<b>T-0785</b>	<b>BGP - Column Type C31/D22 Vertical Coupler Layout</b>	<b>Closed</b>	<b>CR</b>	<b>10/03/2013</b>	<b>10/03/2013</b>	<b>10/08/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> <p>Please refer to drawing 1/S1-3300, S1-3301, S1-3306 and attached Sketch SK-90.</p> <p>Detail 1/S1-3301 requires the couplers for the adjacent column vertical bars be staggered with a vertical distance of 24" or more; however, due to the pattern and spacing of vertical bars for the type C31/D22 detailed on S1-3306, the condition cannot be met. Attached is Gerdau sketch SK-90 - C31/C22 Column Vert Layout with a proposed pattern for the vertical bars in the type C1/D22 columns.</p> <p>Please confirm the proposed concrete reinforcement detail shown in the attached sketch is acceptable for type C31/D22 columns.</p>						<b>ANSWER:</b> <p>Please refer to drawing 1/S1-3300, S1-3301, S1-3306 and attached Sketch SK-90.</p> <p>Detail 1/S1-3301 requires the couplers for the adjacent column vertical bars be staggered with a vertical distance of 24" or more; however, due to the pattern and spacing of vertical bars for the type C31/D22 detailed on S1-3306, the condition cannot be met. Attached is Gerdau sketch SK-90 - C31/C22 Column Vert Layout with a proposed pattern for the vertical bars in the type C1/D22 columns.</p> <p>Please confirm the proposed concrete reinforcement detail shown in the attached sketch is acceptable for type C31/D22 columns.</p>
<b>T-0785.1</b>	<b>BGP - Type C8 &amp; C9 Coupler Stagger Revised Pattern</b>	<b>Closed</b>	<b>CR</b>	<b>01/17/2014</b>	<b>01/27/2014</b>	<b>01/27/2014</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p><b>REQUEST:</b></p> <p>Reference: RFI T-0785 and drawings s1-3300, S1-3301 and S1-3305.</p> <p>Detail 1/S1-3301 requires the couplers for the adjacent column vertical bars be staggered with a vertical distance of 24" or more; however, due to the pattern and spacing of vertical bars for the type C8/D9 detailed on S1-3305, the condition cannot be met. The attached SCCI sketch SK-RFI418, is the proposed pattern for the vertical bars in the type C8/D9 columns, please confirm if it is acceptable.</p>					
	<p><b>ANSWER:</b></p> <p>Reference: RFI T-0785 and drawings s1-3300, S1-3301 and S1-3305.</p> <p>Detail 1/S1-3301 requires the couplers for the adjacent column vertical bars be staggered with a vertical distance of 24" or more; however, due to the pattern and spacing of vertical bars for the type C8/D9 detailed on S1-3305, the condition cannot be met. The attached SCCI sketch SK-RFI418, is the proposed pattern for the vertical bars in the type C8/D9 columns, please confirm if it is acceptable.</p>					
<b>T-0786</b>	<b>SSS - Light Column Clevis Pin Material</b>	<b>Closed</b>	<b>01</b>	<b>10/04/2013</b>	<b>10/14/2013</b>	<b>10/11/2013</b>
	<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Reference Drawing: S1-6006</p> <p>Note on drawing S1-6006 states "ALL CLEVIS PINS AISI 5160 STEEL, OIL QUENCHED FROM 830C, 650C TEMPER OR DIN 34 CRNIMO 6 + QT CODE EN 10083". The pin manufacturer, Dyson Corp., indicates this material is not available and suggests a substitution to ASTM-A540 grade 823, class 5 (see attachment).</p>					
	<p><b>ANSWER:</b></p> <p>Reference Drawing: S1-6006</p> <p>Note on drawing S1-6006 states "ALL CLEVIS PINS AISI 5160 STEEL, OIL QUENCHED FROM 830C, 650C TEMPER OR DIN 34 CRNIMO 6 + QT CODE EN 10083". The pin manufacturer, Dyson Corp., indicates this material is not available and suggests a substitution to ASTM-A540 grade 823, class 5 (see attachment).</p>					





Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
	<p>thickness less than or equal to 2" and plates less than or equal to 2" in thickness shall have a minimum CVN toughness of 25 ft-lb at 70°F.</p> <p>The testing is in accordance with ASTM A673. The frequency is H.</p> <p>o For "Heavy" rolled shapes, as defined by 05 10 00, test to be in accordance with ASTM A6/A6M, supplementary requirement S30, CVN impact test for structural shapes ¿ alternate core location. The testing is in accordance with ASTM A673. The testing frequency is H.</p> <p>o For "Heavy" built-up shapes, as defined by 05 10 00, test to be in accordance with ASTM A6/A6M, supplementary requirement S5, CVN test. The testing is in accordance with ASTM A673. The testing frequency is P.</p> <p>- Confirm that the exception noted in the response to SK RFI 020 (W/O T-0732) for the built-up train box columns still applies which states that for the built-up heavy plates of the train box columns frequency H testing is acceptable.</p> <p>Please confirm that this RFI, and its response, shall be the governing document for all CVN testing for the structural steel shapes, plates and bars, and that no further testing, beyond what is explicitly stated within the RFI and its response is required.</p>					
	<p>thickness exceeding 2" and plates exceeding 2" in thickness shall have a minimum CVN toughness of 30 ft-lb at 70°F.</p> <p>o ASTM A709 hot rolled shapes with a flange thickness less than or equal to 2" and plates less than or equal to 2" in thickness shall have a minimum CVN toughness of 25 ft-lb at 70°F.</p> <p>The testing is in accordance with ASTM A673. The frequency is H.</p> <p>o For "Heavy" rolled shapes, as defined by 05 10 00, test to be in accordance with ASTM A6/A6M, supplementary requirement S30, CVN impact test for structural shapes ¿ alternate core location. The testing is in accordance with ASTM A673. The testing frequency is H.</p> <p>o For "Heavy" built-up shapes, as defined by 05 10 00, test to be in accordance with ASTM A6/A6M, supplementary requirement S5, CVN test. The testing is in accordance with ASTM A673. The testing frequency is P.</p> <p>- Confirm that the exception noted in the response to SK RFI 020 (W/O T-0732) for the built-up train box columns still applies which states that for the built-up heavy plates of the train box columns frequency H testing is acceptable.</p> <p>Please confirm that this RFI, and its response, shall be the governing document for all CVN testing for the structural steel shapes, plates and bars, and that no further testing, beyond what is explicitly stated within the RFI and its response is required.</p>					
T-0788	<p><b>BGP - Areas 5 and 6 EW Top Mat Reinforcing at South Wall Radius</b></p> <p><b>From:</b> Webcor Construction LP Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Refer to the attached sketch 131003_S105-S106 South Radius.</p> <p>In Areas S105 and S106, EW top mat reinforcing makes an increasingly acute angle with the south wall. This</p>	Closed	CR	10/04/2013	10/14/2013	10/04/2013
	<p><b>ANSWER:</b></p> <p>Refer to the attached sketch 131003_S105-S106 South Radius.</p> <p>In Areas S105 and S106, EW top mat reinforcing makes an increasingly acute angle with the south wall.</p>					



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	<p>eventually prevents the reinforcing from penetrating the haunch and wall reinforcing curtains to reach the edge of the mat.</p> <p>Per field coordination, please confirm it is acceptable to terminate EW top mat reinforcing in a hook prior to reaching the edge of the mat slab provided the following provisions are as followed:</p> <ul style="list-style-type: none"><li>- All terminating EW top mat reinforcing shall be hooked</li><li>- Where the angle becomes such that the mat reinforcing cannot penetrate the inner wall reinforcing. The reinforcing may terminate immediately in front of the wall reinforcing inside the haunch. This is labeled Zone 1 in the sketch.</li><li>- In Zone 1, single haunch bars that interfere with penetration of mat reinforcing into the haunch shall be relocated to allow penetration. Relocation will be to the nearest adjacent placement opportunity without regard to the 8" spacing module. Clear spacing, however, between haunch bars shall be maintained.</li><li>- The total number of haunch bars will remain unchanged.</li><li>- In Zone 1, provide a curved band of reinforcing at the typical size and spacing of the mat within the wall.</li><li>- Where the angle becomes such that the mat reinforcing cannot penetrate the haunch without relocating more than one haunch bar, reinforcing may terminate at the toe of the haunch. This is labeled Zone 2 in the sketch.</li><li>- In Zone 2, provide a curved band of reinforcing at the typical size and spacing of the mat within the haunch.</li><li>- Zone 1 and Zone 2 bands will overlap typical reinforcing by the distance LTS.</li></ul>					
	<p>This eventually prevents the reinforcing from penetrating the haunch and wall reinforcing curtains to reach the edge of the mat.</p> <p>Per field coordination, please confirm it is acceptable to terminate EW top mat reinforcing in a hook prior to reaching the edge of the mat slab provided the following provisions are as followed:</p> <ul style="list-style-type: none"><li>- All terminating EW top mat reinforcing shall be hooked</li><li>- Where the angle becomes such that the mat reinforcing cannot penetrate the inner wall reinforcing. The reinforcing may terminate immediately in front of the wall reinforcing inside the haunch. This is labeled Zone 1 in the sketch.</li><li>- In Zone 1, single haunch bars that interfere with penetration of mat reinforcing into the haunch shall be relocated to allow penetration. Relocation will be to the nearest adjacent placement opportunity without regard to the 8" spacing module. Clear spacing, however, between haunch bars shall be maintained.</li><li>- The total number of haunch bars will remain unchanged.</li><li>- In Zone 1, provide a curved band of reinforcing at the typical size and spacing of the mat within the wall.</li><li>- Where the angle becomes such that the mat reinforcing cannot penetrate the haunch without relocating more than one haunch bar, reinforcing may terminate at the toe of the haunch. This is labeled Zone 2 in the sketch.</li><li>- In Zone 2, provide a curved band of reinforcing at the typical size and spacing of the mat within the haunch.</li><li>- Zone 1 and Zone 2 bands will overlap typical reinforcing by the distance LTS.</li></ul>					
<b>T-0789</b>	<b>ASI 106 - Forced Air Thermal Cooling addition to LCC Nodes</b>	<b>Closed</b>	<b>01</b>	<b>10/07/2013</b>	<b>10/17/2013</b>	<b>10/21/2013</b>
	<b>From:</b> Webcor Construction LP      Robert Kjome					
	<b>REQUEST:</b> Reference: Attached Bradken letter  ASI 106 specification section 05 15 21 2.1.B.2.a.1.f calls					
	<b>ANSWER:</b> Reference: Attached Bradken letter  ASI 106 specification section 05 15 21 2.1.B.2.a.1.f					







<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-0792</b>	<b>SSS - Anchor Bolt Detail Clarification</b>	<b>Closed</b>	<b>01</b>	<b>10/07/2013</b>	<b>10/17/2013</b>	<b>10/21/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Drawing: S1-5051  1) The plate washer will clear the fillet weld by 3/16". This is not sufficient to accommodate the maximum anchor bolt as-built tolerance based on the maximum oversize holes per A.I.S.C. Please advise.  2) The plate washer will clear the fillet weld by 1/4". This is not sufficient to accommodate the maximum anchor bolt as-built tolerance based on the maximum oversize holes per A.I.S.C. Confirm it is acceptable to locate the anchor bolts 5 1/2" from the center of the column.		<b>ANSWER:</b> Reference Drawing: S1-5051  1) The plate washer will clear the fillet weld by 3/16". This is not sufficient to accommodate the maximum anchor bolt as-built tolerance based on the maximum oversize holes per A.I.S.C. Please advise.  2) The plate washer will clear the fillet weld by 1/4". This is not sufficient to accommodate the maximum anchor bolt as-built tolerance based on the maximum oversize holes per A.I.S.C. Confirm it is acceptable to locate the anchor bolts 5 1/2" from the center of the column.				
<b>T-0793</b>	<b>SSS - Connection Plates at Type 2 Drag Connections</b>	<b>Closed</b>	<b>01</b>	<b>10/07/2013</b>	<b>10/17/2013</b>	<b>10/22/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> On S1-5017 for the Type 2 Drag connections there are finger type connections where the carrying plates on the beams slide between the framing plates. In order for the beams to side down between these shop attached plates during erection please confirm a 1/8" clearance is acceptable.		<b>ANSWER:</b> On S1-5017 for the Type 2 Drag connections there are finger type connections where the carrying plates on the beams slide between the framing plates. In order for the beams to side down between these shop attached plates during erection please confirm a 1/8" clearance is acceptable.				
<b>T-0795</b>	<b>SSS - Transfer Girder Stiffener Configuration</b>	<b>Closed</b>	<b>01</b>	<b>10/07/2013</b>	<b>10/17/2013</b>	<b>10/11/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference Drawings: S1-4302 & S1-5052  Stiffeners required on TR9 transfer girder (A/ S1-4302) at line F are fouling. Stiffeners were detailed as per 2/ S1-5052 and 4/ S1-5052. See attached sketch CD RFI 040 SK1 for clarification. We propose to trim the stiffeners by ½" to avoid fouling.  Please advise if this proposal is acceptable.		<b>ANSWER:</b> Reference Drawings: S1-4302 & S1-5052  Stiffeners required on TR9 transfer girder (A/ S1-4302) at line F are fouling. Stiffeners were detailed as per 2/ S1-5052 and 4/ S1-5052. See attached sketch CD RFI 040 SK1 for clarification. We propose to trim the stiffeners by ½" to avoid fouling.  Please advise if this proposal is acceptable.				



<i>Number</i>	<i>Subject</i>	<i>Status</i>	<i>Choice Required</i>	<i>Date Created</i>	<i>Date Required</i>	<i>Date Answered</i>
<b>T-0797</b>	<b>BGP - Mat Slab Construction Joint Conflicts in Area 8</b>	<b>Closed</b>	<b>CR</b>	<b>10/08/2013</b>	<b>10/18/2013</b>	<b>10/16/2013</b>
<div> <div> <b>From:</b> Webcor Construction LP           Jackson Tukuafu         </div> <div> <b>REQUEST:</b>            Please refer to attached photos, excerpt drawing CJ-05 from submittal package TG0600-030.3 and SCCI sketch SK-0341.             The east side of the mat slab construction joint of Area 8 (S108) has several constructability issues with the mat keyway and other project structure elements. The following are identified conflicts and SCCI proposed remediation:             1. The current east construction joint layout in Area 8 falls within the row of micropiles as shown in attached Photo-1 and Photo-2. SCCI intends to jog the joint an addition 12" +/- to the East of GL 16.6 to clear the micropile conflict            2. The east construction joint of area 8 currently jogs thru the thickened slab section at GL 16.6/G.3. SCCI intends to shift the joint Eastward to capture the thickened section within the Area 8 pour.             Please confirm the revised construction joint layout shown the attached SCCI sketch SK-341 is acceptable.         </div> <div> <b>ANSWER:</b>             Please refer to attached photos, excerpt drawing CJ-05 from submittal package TG0600-030.3 and SCCI sketch SK-0341.             The east side of the mat slab construction joint of Area 8 (S108) has several constructability issues with the mat keyway and other project structure elements. The following are identified conflicts and SCCI proposed remediation:             1. The current east construction joint layout in Area 8 falls within the row of micropiles as shown in attached Photo-1 and Photo-2. SCCI intends to jog the joint an addition 12" +/- to the East of GL 16.6 to clear the micropile conflict            2. The east construction joint of area 8 currently jogs thru the thickened slab section at GL 16.6/G.3. SCCI intends to shift the joint Eastward to capture the thickened section within the Area 8 pour.             Please confirm the revised construction joint layout shown the attached SCCI sketch SK-341 is acceptable.         </div> </div>						
<b>T-0798</b>	<b>BGP - Mat Slab Construction Joint (east side) Conflicts in Area 09</b>	<b>Closed</b>	<b>CR</b>	<b>10/08/2013</b>	<b>10/18/2013</b>	<b>10/16/2013</b>
<div> <div> <b>From:</b> Webcor Construction LP           Jackson Tukuafu         </div> <div> <b>REQUEST:</b>            Please refer to attached SCCI sketch SK-345 and drawing (CJ-05) excerpt from submittal package TG0600-30.2.             The east side of the mat slab construction joint of Area 09 (S109) has several constructability issues wih the mat keyway and other project structure elements. SCCI proposes to install the CJ between area 09 and 10 as shown on the attached sketch.             Please confirm the revised construction joint layout as shown in the attached SCCI sketch SK-342 is acceptable.         </div> <div> <b>ANSWER:</b>             Please refer to attached SCCI sketch SK-345 and drawing (CJ-05) excerpt from submittal package TG0600-30.2.             The east side of the mat slab construction joint of Area 09 (S109) has several constructability issues with the mat keyway and other project structure elements. SCCI proposes to install the CJ between area 09 and 10 as shown on the attached sketch.             Please confirm the revised construction joint layout as shown in the attached SCCI sketch SK-342 is acceptable.         </div> </div>						



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<b>T-0799</b>	<b>BGP - Partition Wall Pilaster and Plumbing Conflict at GL C.5/4.8</b>	<b>Closed</b>	<b>CR</b>	<b>10/08/2013</b>	<b>10/18/2013</b>	<b>10/10/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to drawing S1-2052 and S1-9050.  The reinforcement for the partition wall pilaster at approximately GL C.5/4.8 is in conflict with the drainage pipe below. Per note 3 on detail 9/S1-9050 the ties will be installed if possible.  Two vertical bars in the pilaster will have to be bent in order to clear the pipe and two others will have to be slightly displaced to clear the pipe. See the attached Gerdau sketch SK-93 for details.  Please confirm the revised reinforcement detail for the partition wall pilaster as detailed in sketch SK-93 is acceptable.						<b>ANSWER:</b>  Please refer to drawing S1-2052 and S1-9050.  The reinforcement for the partition wall pilaster at approximately GL C.5/4.8 is in conflict with the drainage pipe below. Per note 3 on detail 9/S1-9050 the ties will be installed if possible.  Two vertical bars in the pilaster will have to be bent in order to clear the pipe and two others will have to be slightly displaced to clear the pipe. See the attached Gerdau sketch SK-93 for details.  Please confirm the revised reinforcement detail for the partition wall pilaster as detailed in sketch SK-93 is acceptable.
<b>T-0800</b>	<b>SSS - Top of Base Plate Elevation Clarification</b>	<b>Closed</b>	<b>01</b>	<b>10/08/2013</b>	<b>10/18/2013</b>	<b>10/09/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  Reference Drawing: S1-3621, S1-5051  The top of base plate elevation at Grids 21.0/D.4 & 21.0/E.6 is shown as -4'-4 1/2 in 2/S1-5051 but when working with detail 5/S1-3621, the top of base plate elevation is -4' -6 1/2. Please refer to attached CD RFI # 041 SK1 to SK3 and provide the top of base plate elevation to be used at the noted Grids.						<b>ANSWER:</b>  Reference Drawing: S1-3621, S1-5051  The top of base plate elevation at Grids 21.0/D.4 & 21.0/E.6 is shown as -4'-4 1/2 in 2/S1-5051 but when working with detail 5/S1-3621, the top of base plate elevation is -4' -6 1/2. Please refer to attached CD RFI # 041 SK1 to SK3 and provide the top of base plate elevation to be used at the noted Grids.
<b>T-0801</b>	<b>SSS - Revit Model Dimension Verification</b>	<b>Closed</b>	<b>01</b>	<b>10/08/2013</b>	<b>10/18/2013</b>	<b>10/09/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  On S1-2302, S1-2303 & S1-2304 there are some beam & HSS member locations that are not located on the design drawings therefore we have used the Revit model to locate these members. On sketch CD RFI 047 SK1 to SK3 please verify all clouded dimensions that were taken from the latest Revit model received 9/12/13 to locate the steel in question.						<b>ANSWER:</b>  On S1-2302, S1-2303 & S1-2304 there are some beam & HSS member locations that are not located on the design drawings therefore we have used the Revit model to locate these members. On sketch CD RFI 047 SK1 to SK3 please verify all clouded dimensions that were taken from the latest Revit model received 9/12/13 to locate the steel in question.



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<b>T-0802</b>	<b>BGP - Mat Slab Construction Joint (east side) Conflicts in Area 10</b>	<b>Closed</b>	<b>CR</b>	<b>10/08/2013</b>	<b>10/18/2013</b>	<b>10/16/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached SCCI sketch SK-345 and drawing (CJ-05) excerpt from submittal package TG0600-30.2.  The east side of the mat slab construction joint of Area 10 (S110) has several constructability issues with the mat keyway and other project structure elements. SCCI proposed to install the CJ between area 10 and 11 as shown on the attached sketches.  Please confirm the revised construction joint layout as shown in the attached SCCI sketch SK-345 is acceptable.						<b>ANSWER:</b>  Please refer to attached SCCI sketch SK-345 and drawing (CJ-05) excerpt from submittal package TG0600-30.2.  The east side of the mat slab construction joint of Area 10 (S110) has several constructability issues with the mat keyway and other project structure elements. SCCI proposed to install the CJ between area 10 and 11 as shown on the attached sketches.  Please confirm the revised construction joint layout as shown in the attached SCCI sketch SK-345 is acceptable.
<b>T-0803</b>	<b>SSS - 2nd Level Revit Model Dimension Verification</b>	<b>Closed</b>	<b>01</b>	<b>10/08/2013</b>	<b>10/18/2013</b>	<b>10/09/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  On S1-2402, S1-2403, S1-2404, S1-2406 & S1-2407 there are some beam & HSS member locations that are not located on the design drawings therefore we have used the Revit model to locate these members. On sketches CD RFI 048 SK1 to SK5 please verify all clouded dimensions that were taken from the latest Revit model received 9/12/13 to locate the steel in question						<b>ANSWER:</b>  On S1-2402, S1-2403, S1-2404, S1-2406 & S1-2407 there are some beam & HSS member locations that are not located on the design drawings therefore we have used the Revit model to locate these members. On sketches CD RFI 048 SK1 to SK5 please verify all clouded dimensions that were taken from the latest Revit model received 9/12/13 to locate the steel in question
<b>T-0803.1</b>	<b>SSS - 2nd Level Revit Model Dimension Verification</b>	<b>Closed</b>	<b>CR</b>	<b>11/22/2013</b>	<b>12/02/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  On the response to Webcor RFI # T-0769 (SK RFI # SK 050) & T-0803 SK RFI # 067) we have reviewed and located most of the beam locations in question using the nearest gridlines, architectural dwg's, partial plans, equal spacing, etc per the noted guidelines in the response. However on drawings S1-2402, S1-2403, S1-2404, S1-2406 & S1-2407 there are still some beam locations that cannot be located and require verification therefore on sketches CD RFI 048.1 SK1 to SK5 please verify all clouded dimensions in RED as noted to close this RFI.						<b>ANSWER:</b>  On the response to Webcor RFI # T-0769 (SK RFI # SK 050) & T-0803 SK RFI # 067) we have reviewed and located most of the beam locations in question using the nearest gridlines, architectural dwg's, partial plans, equal spacing, etc per the noted guidelines in the response. However on drawings S1-2402, S1-2403, S1-2404, S1-2406 & S1-2407 there are still some beam locations that cannot be located and require verification therefore on sketches CD RFI 048.1 SK1 to SK5 please verify all clouded

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T-0804	SSS - W21 Beam Substitution	Closed	01	10/08/2013	10/18/2013	10/11/2013
<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>With reference to the W21x44 and W21x50 beams shown on Dwgs S1-2302 to S1-2307 (Ground Level), S1-2402 to S1-2407 (Second Level), Dwgs S1-2502 to S1-2507 (Bus Level) and Dwgs S1-2602 to S1-2607 (Roof Park Level), these beams have relatively narrow flanges. These beams sizes are problematic with regard to stability during erection for spans over 30 feet in length. The substitution of the W21x48 for the W21x44 and W21x55 for the W21x50 would resolve the stability issue. Please advise if these substitutions are acceptable.</p>			<p>With reference to the W21x44 and W21x50 beams shown on Dwgs S1-2302 to S1-2307 (Ground Level), S1-2402 to S1-2407 (Second Level), Dwgs S1-2502 to S1-2507 (Bus Level) and Dwgs S1-2602 to S1-2607 (Roof Park Level), these beams have relatively narrow flanges. These beams sizes are problematic with regard to stability during erection for spans over 30 feet in length. The substitution of the W21x48 for the W21x44 and W21x55 for the W21x50 would resolve the stability issue. Please advise if these substitutions are acceptable.</p>			







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	<p>48"diameter bridge piers support the TG03 BSE (Balfour installed) temporary bridges on 1st street, Fremont street and Beale street.</p> <p>Provide specification for positional couplers to be used, and confirm that rebar has appropriate concrete cover with positional coupler use.</p> <p>This detail will be part of the TG07.2 scope of works.</p>					
	<p>the 48"diameter bridge piers support the TG03 BSE (Balfour installed) temporary bridges on 1st street, Fremont street and Beale street.</p> <p>Provide specification for positional couplers to be used, and confirm that rebar has appropriate concrete cover with positional coupler use.</p> <p>This detail will be part of the TG07.2 scope of works.</p>					
T-0808	<p><b>SSS - Material Grade Certification</b></p> <p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Please refer to attached CD RFI 046 SK1 to SK5 sketches and confirm all connection material shown on drawing S1-5051 is ASTM A36 material per the material note for plates in SS-2 on drawing S-0007 unless specifically noted on the drawing.</p>	Closed	01	10/10/2013	10/20/2013	10/18/2013
	<p><b>ANSWER:</b></p> <p>Please refer to attached CD RFI 046 SK1 to SK5 sketches and confirm all connection material shown on drawing S1-5051 is ASTM A36 material per the material note for plates in SS-2 on drawing S-0007 unless specifically noted on the drawing.</p>					
T-0809	<p><b>SSS - Shear Plate Connections</b></p> <p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>For the typical shear plate connections per detail 1/S1-5011 see sketches CD RFI 060 SK1 &amp; SK2 for items 1, 2 &amp; 3 noted below.</p> <p>1. Confirm it is acceptable to locate the bolts 2 3/4" from face of beam web as shown for duplication of shear plate marks.</p> <p>2. Confirm it is acceptable to cope the beam to match the "k" distance of the supported beam (W24) while maintaining a 1/2" minimum clearance to avoid cutting inside the "k" in lieu of the 1/2" max. shown in detail 1/S1-5011.</p> <p>3. Confirm the shear plate thickness and weld size at a W16x31 to W24x68 connection as per Note 3 in 1/S1-5011 is 3/8" shear plate and 1/4" weld.</p>	Closed	01	10/10/2013	10/20/2013	10/22/2013
	<p><b>ANSWER:</b></p> <p>For the typical shear plate connections per detail 1/S1-5011 see sketches CD RFI 060 SK1 &amp; SK2 for items 1, 2 &amp; 3 noted below.</p> <p>1. Confirm it is acceptable to locate the bolts 2 3/4" from face of beam web as shown for duplication of shear plate marks.</p> <p>2. Confirm it is acceptable to cope the beam to match the "k" distance of the supported beam (W24) while maintaining a 1/2" minimum clearance to avoid cutting inside the "k" in lieu of the 1/2" max. shown in detail 1/S1-5011.</p> <p>3. Confirm the shear plate thickness and weld size at a W16x31 to W24x68 connection as per Note 3 in 1/S1-5011 is 3/8" shear plate and 1/4" weld.</p>					





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	of Cast Nodes.				node face. Please verify a corresponding scribe line will be added to the face of Cast Nodes.	
<b>T-0813</b>	<b>SSS - Kick Angle Requirements</b>	<b>Closed</b>	<b>01</b>	<b>10/10/2013</b>	<b>10/20/2013</b>	<b>10/21/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Please refer to sketch CD RFI #070 SK1. The BU members on Grid 1 are not noted as MF, TR or TPG and it is not clear which kicker brace detail on S1-5015 applies. Please advise which kicker brace detail on S1-5015 is to be applied along Grid 1					<b>ANSWER:</b> Please refer to sketch CD RFI #070 SK1. The BU members on Grid 1 are not noted as MF, TR or TPG and it is not clear which kicker brace detail on S1-5015 applies. Please advise which kicker brace detail on S1-5015 is to be applied along Grid 1	
<b>T-0814</b>	<b>SSS - Missing BU Members in the Bottom Flange Brace Schedule</b>	<b>Closed</b>	<b>01</b>	<b>10/10/2013</b>	<b>10/20/2013</b>	<b>10/14/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Per detail 7/S1-5015 please refer to sketch CD RFI # 072 SK1 and supply the information for the missing BU 30x18x1x1.5 & BU 30x22x1.5x2 members in the schedule.					<b>ANSWER:</b> Per detail 7/S1-5015 please refer to sketch CD RFI # 072 SK1 and supply the information for the missing BU 30x18x1x1.5 & BU 30x22x1.5x2 members in the schedule.	
<b>T-0815</b>	<b>SSS -Missing Kicker Brace Details</b>	<b>Closed</b>	<b>01</b>	<b>10/10/2013</b>	<b>10/20/2013</b>	<b>10/21/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> At the Bus level near grid line 12 and at grids 18 & 26 please refer to sketches CD RFI # 073 SK1 to SK3 and supply the appropriate kicker brace detail on S1-5015 to be used for the noted beams as these beams are not MF beams, Transfer Girders or Tapered Girders.					<b>ANSWER:</b> At the Bus level near grid line 12 and at grids 18 & 26 please refer to sketches CD RFI # 073 SK1 to SK3 and supply the appropriate kicker brace detail on S1-5015 to be used for the noted beams as these beams are not MF beams, Transfer Girders or Tapered Girders.	
<b>T-0816</b>	<b>BGP - Revised Placement Tolerance at Top Mat Reinforcement</b>	<b>Closed</b>	<b>CR</b>	<b>10/10/2013</b>	<b>10/20/2013</b>	<b>10/22/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>					<b>ANSWER:</b>	



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	<p>Please refer to drawing S1-2052 and ACI 117.</p> <p>Please confirm it is acceptable to increase the top mat slab reinforcement placement tolerance from +/-1/2" to +1/2" and -1" as discussed and coordinated with TT field representative. This would also change the concrete cover tolerance from -1/2" to +/-1/2".</p>					
T-0817	BGP -Compressible material between concrete structure & CDSM wall	Closed	01	10/11/2013	10/21/2013	10/23/2013
<p><b>From:</b> Webcor Construction LP                      Michael Spillane</p> <p><b>REQUEST:</b></p> <p>The contractor has raised a concern see letter in exhibit A attached.</p> <p>Does the design team envisage any possible issues with the CDSM wall if the waterproofing substrate becomes compressed between the permanent structure and the CDSM wall once the level D bracing is removed? The same question applies when the re-bracing is installed against the permanent foundation walls.</p>						
<p><b>ANSWER:</b></p> <p>The contractor has raised a concern see letter in exhibit A attached.</p> <p>Does the design team envisage any possible issues with the CDSM wall if the waterproofing substrate becomes compressed between the permanent structure and the CDSM wall once the level D bracing is removed? The same question applies when the re-bracing is installed against the permanent foundation walls.</p>						
T-0818	SSS- Kicker Brace Connection to Underside of Beam Flange	Closed	01	10/10/2013	10/20/2013	10/17/2013
<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>On S1-2505 at grid lines 20.1/E please refer to sketches CD RFI # 074 SK1 &amp; SK2 and supply a connection detail for the kicker brace to the underside of the beam flange as shown.</p>						
<p><b>ANSWER:</b></p> <p>On S1-2505 at grid lines 20.1/E please refer to sketches CD RFI # 074 SK1 &amp; SK2 and supply a connection detail for the kicker brace to the underside of the beam flange as shown.</p>						



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<b>T-0819</b>	<b>SSS -Gusset Plates at Kicker Angle Connections</b>	<b>Closed</b>	<b>01</b>	<b>10/10/2013</b>	<b>10/20/2013</b>	<b>10/14/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
On S1-5015 for the bottom flange connection and the kicker angle connection clarification please refer to sketches CD RFI # 077 SK1 & SK2 for items 1 & 2:			On S1-5015 for the bottom flange connection and the kicker angle connection clarification please refer to sketches CD RFI # 077 SK1 & SK2 for items 1 & 2:			
1) Confirm it is acceptable to cut the gusset plate as shown to avoid a pointed corner as the weld will not be effective in the shaded triangle area.			1) Confirm it is acceptable to cut the gusset plate as shown to avoid a pointed corner as the weld will not be effective in the shaded triangle area.			
2) Confirm it is acceptable to cut the gusset plate as shown to avoid a pointed corner as the weld will not be effective in the shaded triangle area.			2) Confirm it is acceptable to cut the gusset plate as shown to avoid a pointed corner as the weld will not be effective in the shaded triangle area.			
<b>T-0820</b>	<b>SSS - Missing Beam Connection Details</b>	<b>Closed</b>	<b>01</b>	<b>10/10/2013</b>	<b>10/10/2013</b>	<b>10/22/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At the ground level north of grid line G at grids 2, 3 & 4 please refer to sketches CD RFI # 078 SK1 to SK4 for items 1 to 4 below and supply connection details as noted.			At the ground level north of grid line G at grids 2, 3 & 4 please refer to sketches CD RFI # 078 SK1 to SK4 for items 1 to 4 below and supply connection details as noted.			
1) Supply a connection detail.			1) Supply a connection detail.			
2) Confirm connection is per 12/S1?J5010.			2) Confirm connection is per 12/S1?J5010.			
3) Supply a connection detail.			3) Supply a connection detail.			
4) Supply a connection detail.			4) Supply a connection detail.			



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<b>T-0821</b>	<b>BGP - Plumbing Line in Area 4 Stairway</b>	<b>Closed</b>	<b>CR</b>	<b>10/10/2013</b>	<b>10/20/2013</b>	<b>10/31/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> Reference Drawing P1-2022 between Line C/4-5  Per drawing P1-2022, a 6" sanitary line and vent connection is shown inside the Area 4 stairway. WOJV recognizes the need to flush the sprinkler system and/or needed drain. However, per CBC Code 2007 section 1020.1.2, plumbing line or drains are not listed under Penetrations.  Please confirm the plumbing line detailed inside the Area 4 stairway will comply with the referenced code section.						<b>ANSWER:</b> Reference Drawing P1-2022 between Line C/4-5  Per drawing P1-2022, a 6" sanitary line and vent connection is shown inside the Area 4 stairway. WOJV recognizes the need to flush the sprinkler system and/or needed drain. However, per CBC Code 2007 section 1020.1.2, plumbing line or drains are not listed under Penetrations.  Please confirm the plumbing line detailed inside the Area 4 stairway will comply with the referenced code section.
<b>T-0822</b>	<b>SSS - Angle Connection Details at GL 23</b>	<b>Closed</b>	<b>01</b>	<b>10/11/2013</b>	<b>10/21/2013</b>	<b>10/14/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b> On S1-2305 around the light column @ grid 23 see attached CD RFI 062 SK1 and confirm details 6 & 7/S1-5015 may be applied at the noted (16) locations. If not, supply a detail reference.						<b>ANSWER:</b> On S1-2305 around the light column @ grid 23 see attached CD RFI 062 SK1 and confirm details 6 & 7/S1-5015 may be applied at the noted (16) locations. If not, supply a detail reference.
<b>T-0822.1</b>	<b>SSS - Angle Connection Details at GL 23</b>	<b>Closed</b>	<b>CR</b>	<b>12/03/2013</b>	<b>12/13/2013</b>	<b>12/13/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> RFI T-0822 (attached for reference) confirmed the use of details 6 and 7/S1-5015 at the 16 highlighted areas. Please refer to CD RFI 062.1 SK1 and confirm that the weld dimension "A" indicated on 7/S1-5015 applies to skewed angle connections as indicated on the sketch attached. Otherwise, please provide the required welding information.						<b>ANSWER:</b> RFI T-0822 (attached for reference) confirmed the use of details 6 and 7/S1-5015 at the 16 highlighted areas. Please refer to CD RFI 062.1 SK1 and confirm that the weld dimension "A" indicated on 7/S1-5015 applies to skewed angle connections as indicated on the sketch attached. Otherwise, please provide the required welding information.
<b>T-0823</b>	<b>SSS - Bolted Beam Connections</b>	<b>Closed</b>	<b>01</b>	<b>10/11/2013</b>	<b>10/21/2013</b>	<b>10/14/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	<p>Per S1-5012 for the typical bolt beam connections please refer to sketches CD RFI # 079 SK1 to SK3 for items 1 to 7:</p> <ol style="list-style-type: none"><li>1) Confirm the noted dimension may be 1 3/4" in details 1 &amp; 2/S1-5012 to match 3/S1-5012.</li><li>2) Confirm the noted dimensions are acceptable for details 1, 2 &amp; 3/S1-5012.</li><li>3) Supply plate thickness.</li><li>4) Supply welding for shear plate to column.</li><li>5) Confirm dimensions are acceptable.</li><li>6) Confirm dimensions are acceptable.</li><li>7) Supply plate thickness.</li></ol>					<p>Per S1-5012 for the typical bolt beam connections please refer to sketches CD RFI # 079 SK1 to SK3 for items 1 to 7:</p> <ol style="list-style-type: none"><li>1) Confirm the noted dimension may be 1 3/4" in details 1 &amp; 2/S1-5012 to match 3/S1-5012.</li><li>2) Confirm the noted dimensions are acceptable for details 1, 2 &amp; 3/S1-5012.</li><li>3) Supply plate thickness.</li><li>4) Supply welding for shear plate to column.</li><li>5) Confirm dimensions are acceptable.</li><li>6) Confirm dimensions are acceptable.</li><li>7) Supply plate thickness.</li></ol>
<b>T-0824</b>	<b>SSS - Bottom Flange Connection Plate</b>	<b>Closed</b>	<b>01</b>	<b>10/11/2013</b>	<b>10/21/2013</b>	<b>10/22/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						<b>ANSWER:</b>
<p>Per detail 6/S1-5015 for the bottom flange connection plate please refer to sketches CD RFI # 069 SK1, SK2 &amp; SK3.</p> <p>1) Access for field welding the web extension plate per 6/S1-5015 is a problem at the noted location as well as other similar locations.</p> <p>Confirm the web extension plate may be typically omitted when the dimension shown as 1 3/4" is 3" or less.</p>						<p>Per detail 6/S1-5015 for the bottom flange connection plate please refer to sketches CD RFI # 069 SK1, SK2 &amp; SK3.</p> <p>1) Access for field welding the web extension plate per 6/S1-5015 is a problem at the noted location as well as other similar locations.</p> <p>Confirm the web extension plate may be typically omitted when the dimension shown as 1 3/4" is 3" or less.</p>



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<b>T-0825</b>	<b>SSS - W30 Beam to Girder where bf exceeds 22</b>	<b>Closed</b>	<b>01</b>	<b>10/11/2013</b>	<b>10/21/2013</b>	<b>10/17/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Robert Kjome</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
On S1-2505 along grid line 20.1/E.6 where the W30x108 beam frames into the MF girder please refer to sketches CD RFI # 076 SK1 & SK2 for items 1 & 2 noted below.			On S1-2505 along grid line 20.1/E.6 where the W30x108 beam frames into the MF girder please refer to sketches CD RFI # 076 SK1 & SK2 for items 1 & 2 noted below.			
1) The noted "MF" beam is a BU-44x24x1.25x2.75. Detail 1/S1-5011 does not apply as "bf" exceeds 22". Please supply a typical connection for a round circle on plans when the "bf" exceeds 22 (work with item 2 on SK2)			1) The noted "MF" beam is a BU-44x24x1.25x2.75. Detail 1/S1-5011 does not apply as "bf" exceeds 22". Please supply a typical connection for a round circle on plans when the "bf" exceeds 22 (work with item 2 on SK2)			
2) Please note that if a full depth shear plate is used it will foul the beam extension plate per 6/S1-5015. Please clarify.			2) Please note that if a full depth shear plate is used it will foul the beam extension plate per 6/S1-5015. Please clarify.			
<b>T-0826</b>	<b>SSS - Oversized Hole Size in Web Stiffeners</b>	<b>Closed</b>	<b>01</b>	<b>10/14/2013</b>	<b>10/24/2013</b>	<b>10/22/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Robert Kjome</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please confirm it is acceptable to oversize the bolt holes in the web stiffeners to the bolt diameter + 3/16". Reference Detail 1 on S1-5019 and CD RFI 055 SK1 for additional information.			Please confirm it is acceptable to oversize the bolt holes in the web stiffeners to the bolt diameter + 3/16". Reference Detail 1 on S1-5019 and CD RFI 055 SK1 for additional information.			
<b>T-0826.1</b>	<b>SSS - Clarification of Oversized Holes in Web Stiffeners</b>	<b>Closed</b>	<b>CR</b>	<b>11/11/2013</b>	<b>11/21/2013</b>	<b>11/15/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference the response to W/O RFI # T-0826 (SK 114 & CD 055D), attached for reference.			Reference the response to W/O RFI # T-0826 (SK 114 & CD 055D), attached for reference.			
W/O RFI T-0826 response states that "Use of oversize bolt holes in this drag connection is not acceptable." Confirm that the response above applies only to conditions when the web stiffener plate is the outside plate in a connection and that the 3/16" oversize holes for the web stiffener plates are acceptable in details 1 & 2/S1-5016 when the web stiffener plate is not the outside plate in the connection.			W/O RFI T-0826 response states that "Use of oversize bolt holes in this drag connection is not acceptable." Confirm that the response above applies only to conditions when the web stiffener plate is the outside plate in a connection and that the 3/16" oversize holes for the web stiffener plates are acceptable in details 1 & 2/S1-5016 when the web stiffener plate is not the outside plate in the connection.			





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<b>T-0827</b>	<b>BGP - Clarification to Galvanized Steel Plate at Seismic Joint in Area 16</b>	<b>Closed</b>	<b>CR</b>	<b>10/14/2013</b>	<b>10/24/2013</b>	<b>10/28/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached drawing detail 7/A1-8881 and 4/S1-3010.  Detail 7/A1-8881 (and other details on A1-8881) call for a 5/8" x 6' galvanized steel plate secured to the mud slab and soldier piles. This plate does not appear on the structural details for the seismic joint in drawing S1-3010.  1. Please explain the functionality and purpose of the galvanized steel plate shown in 7/A1-8881 and 1/A1-8881 in relation to the seismic joint assembly.  2. Please provide revised structural drawings showing all welding and design criteria required to attach and secure the "5/8" THK x 6' wide galv steel plate" to the mud slab and soldier beam, respectively.						<b>ANSWER:</b>  Please refer to attached drawing detail 7/A1-8881 and 4/S1-3010.  Detail 7/A1-8881 (and other details on A1-8881) call for a 5/8" x 6' galvanized steel plate secured to the mud slab and soldier piles. This plate does not appear on the structural details for the seismic joint in drawing S1-3010.  1. Please explain the functionality and purpose of the galvanized steel plate shown in 7/A1-8881 and 1/A1-8881 in relation to the seismic joint assembly.  2. Please provide revised structural drawings showing all welding and design criteria required to attach and secure the "5/8" THK x 6' wide galv steel plate" to the mud slab and soldier beam, respectively.
<b>T-0828</b>	<b>SSS - Locations for Scratch Plate for BRBs</b>	<b>Closed</b>	<b>01</b>	<b>10/14/2013</b>	<b>10/14/2013</b>	<b>10/17/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  Please reference the sketch attached and verify the proposed scratch plate end locations and surface locations are acceptable.						<b>ANSWER:</b>  Please reference the sketch attached and verify the proposed scratch plate end locations and surface locations are acceptable.
<b>T-0829</b>	<b>BSE - Voids Across Top of CDSM Wall on the West side of Zone 1</b>	<b>Closed</b>	<b>01</b>	<b>10/15/2013</b>	<b>10/25/2013</b>	<b>10/21/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  Reference Photos: attached  There are a number of voids that run across the top of the CDSM wall on the West side of zone 1 (see attached photos). During prior conversations between W/O and Arup there has been discussion of filling these voids with material. Please provide the material and application desired by the design team to fill these voids.						<b>ANSWER:</b>  Reference Photos: attached  There are a number of voids that run across the top of the CDSM wall on the West side of zone 1 (see attached photos). During prior conversations between W/O and Arup there has been discussion of filling these voids with material. Please provide the material and application desired by the design team to fill these voids.



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T-0830	SSS - Type T, TT, and TTT Base Plate Anchor Rod Location Confirmation	Closed	01	10/15/2013	10/25/2013	10/21/2013
<div><div><div>From: Webcor Construction LP</div><div>Robert Kjome</div></div><div><div>REQUEST:</div><div>Detail 7 on S1-5051 provides locations where type TT and type TTT base plate anchor rods will be installed. There are other details throughout the plans that contradict the columns base plate anchor rod locations provided in 7/S1-5051. For example: 7/S1-5051 shows a column at gridline 10.1/G.3 as having a type TTT base plate anchor rod detail; however, 1/S1-3610 shows the column at 10.1 and G.3 as having a type T base plate anchor rod detail.</div><div>Please confirm that detail 7/S1-5051 provides the correct base plate anchor rod detail for each of the columns.</div><div>Please provide a type T base plate anchor rod detail.</div></div><div><div>ANSWER:</div><div>Detail 7 on S1-5051 provides locations where type TT and type TTT base plate anchor rods will be installed. There are other details throughout the plans that contradict the columns base plate anchor rod locations provided in 7/S1-5051. For example: 7/S1-5051 shows a column at gridline 10.1/G.3 as having a type TTT base plate anchor rod detail; however, 1/S1-3610 shows the column at 10.1 and G.3 as having a type T base plate anchor rod detail.</div><div>Please confirm that detail 7/S1-5051 provides the correct base plate anchor rod detail for each of the columns.</div><div>Please provide a type T base plate anchor rod detail.</div></div></div>						
T-0831	BGP - Area 11 Clear Cover to the Vertical Reinforcement on the Foundation Wall	Closed	01	10/22/2013	11/12/2013	10/29/2013
<div><div><div>From: Webcor Construction LP</div><div>Michael Spillane</div></div><div><div>REQUEST:</div><div>Further to response to RFI T-609 this RFI shows the areas of foundation wall/embedded column in pour Area 11, on the north &amp; south wall elevations which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A</div><div>Exhibit - B &amp; C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement in this case only pile number 225 on the north elevation has this issue.</div><div>RFI T - 783 shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 11.</div><div>Please confirm that the clear cover between the waterproofing system and the vertical reinforcement as outlined at these locations is acceptable.</div></div><div><div>ANSWER:</div><div>Further to response to RFI T-609 this RFI shows the areas of foundation wall/embedded column in pour Area 11, on the north &amp; south wall elevations which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A</div><div>Exhibit - B &amp; C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement in this case only pile number 225 on the north elevation has this issue.</div><div>RFI T - 783 shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 11.</div><div>Please confirm that the clear cover between the waterproofing system and the vertical reinforcement as outlined at these locations is acceptable.</div></div></div>						
T-0832	BGP - Area 12 Clear Cover to the Vertical Reinforcement on the Foundation Wall	Closed	01	10/24/2013	11/05/2013	10/29/2013



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<b>From:</b> Webcor Construction LP	Michael Spillane					
<b>REQUEST:</b>	<b>ANSWER:</b>					
Further to response to RFI T-609 this RFI shows the areas of foundation wall/embedded column in pour Area 12, on the north & south wall elevations which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A		Further to response to RFI T-609 this RFI shows the areas of foundation wall/embedded column in pour Area 12, on the north & south wall elevations which will have greater than 6" of clear cover to the vertical reinforcement for location plan see exhibit - A				
Exhibit - B & C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement in this case only two pile numbers 237 & 238 on the north elevation has this issue.		Exhibit - B & C depict the amount and location of the foundation walls which the will have greater than 6" of clear cover to the vertical reinforcement in this case only two pile numbers 237 & 238 on the north elevation has this issue.				
RFI T - 784 shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 12.		RFI T - 784 shows the thinning of the wall with the revised reinforcement spacing due to CDSM pile encroachment in Area 12.				
Please confirm that the clear cover between the waterproofing system and the vertical reinforcement as outlined at these locations is acceptable.		Please confirm that the clear cover between the waterproofing system and the vertical reinforcement as outlined at these locations is acceptable.				
<b>T-0833</b>	<b>BGP - Embed Clarification at Elevator Rail Support</b>	<b>Closed</b>	<b>CR</b>	<b>10/16/2013</b>	<b>10/26/2013</b>	<b>10/30/2013</b>
<b>From:</b> Webcor Construction LP	Jackson Tukuafu					
<b>REQUEST:</b>	<b>ANSWER:</b>					
Please confirm the length of the elevator rail support embed dimension is 2'-7", as shown in the attached detail drawing 4/S1-7630..		Please confirm the length of the elevator rail support embed dimension is 2'-7", as shown in the attached detail drawing 4/S1-7630..				



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<b>T-0834</b>	<b>BGP - Structural Steel Embeds in Concourse Slab/Columns</b>	<b>Closed</b>	<b>CR</b>	<b>10/17/2013</b>	<b>10/27/2013</b>	<b>10/24/2013</b>
<div><div><b>From:</b> Webcor Construction LP      Jackson Tukuafu</div><div><b>REQUEST:</b><p>Attached is a rebar congestion model of the concourse slab and column C2 at C/24.9. As is apparent, the structural steel shear lug portion of the plate embed is in conflict with the reinforcing steel and will not fit with required rebar spacing. The rebar conflicts with the shear lug and blockout that are present, include but are not limited to:</p><ul style="list-style-type: none"><li>- Typical MFB Beam at C/24.9 (blue colored bars in model)</li><li>- B-68 Beam (yellow colored bars in model)</li><li>- Main concourse slab (pink colored bars in model)</li><li>- Column C-2 vertical T-Heads (purple colored bars in model)</li></ul><p>Please provide a solution that will provide a constructible blockout and embedment of the structural steel plate.</p></div><div><b>ANSWER:</b><p>Attached is a rebar congestion model of the concourse slab and column C2 at C/24.9. As is apparent, the structural steel shear lug portion of the plate embed is in conflict with the reinforcing steel and will not fit with required rebar spacing. The rebar conflicts with the shear lug and blockout that are present, include but are not limited to:</p><ul style="list-style-type: none"><li>- Typical MFB Beam at C/24.9 (blue colored bars in model)</li><li>- B-68 Beam (yellow colored bars in model)</li><li>- Main concourse slab (pink colored bars in model)</li><li>- Column C-2 vertical T-Heads (purple colored bars in model)</li></ul><p>Please provide a solution that will provide a constructible blockout and embedment of the structural steel plate.</p></div></div>						
<b>T-0835</b>	<b>BGP - Vehicle Ramp Beam and Wall Support Embed Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>10/17/2013</b>	<b>10/27/2013</b>	<b>10/29/2013</b>
<div><div><b>From:</b> Webcor Construction LP      Jackson Tukuafu</div><div><b>REQUEST:</b><p>Please reference attached drawings S1-2251, A1-7401, S1-3411, S1-3203 and S1-3204.</p><ol style="list-style-type: none"><li>1. Please confirm the beam support angle/plate as shown on D1 of S1-3411 are located where shown on drawing S1-2251 (notation in red). There will be a total of three total embeds.</li><li>2. Please confirm the wall support angle/plate (two total embeds) shown on detail D6/S1-3203 and D10/S1-3204 are located where shown on the notated drawing S1-2251 (notation in green).</li><li>3. Please provide a drawing that shows the acute and obtuse angles for embeds highlighted on A1-7401.</li></ol><p>Please advise.</p></div><div><b>ANSWER:</b><p>Please reference attached drawings S1-2251, A1-7401, S1-3411, S1-3203 and S1-3204.</p><ol style="list-style-type: none"><li>1. Please confirm the beam support angle/plate as shown on D1 of S1-3411 are located where shown on drawing S1-2251 (notation in red). There will be a total of three total embeds.</li><li>2. Please confirm the wall support angle/plate (two total embeds) shown on detail D6/S1-3203 and D10/S1-3204 are located where shown on the notated drawing S1-2251 (notation in green).</li><li>3. Please provide a drawing that shows the acute and obtuse angles for embeds highlighted on A1-7401.</li></ol><p>Please advise.</p></div></div>						
<b>T-0835.1</b>	<b>BGP - Vehicle Ramp Beam Support Embeds</b>	<b>Closed</b>	<b>CR</b>	<b>11/05/2013</b>	<b>11/15/2013</b>	<b>11/19/2013</b>



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<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI T-0835, RFI T-0453.1 and attached SKA-2863.			Please reference RFI T-0835, RFI T-0453.1 and attached SKA-2863.			
RFI Response T-0453.1, stated that in lieu of bending the L8x8x1-1/8" member, it was acceptable to weld two 1-1/8" thick plates together in order to achieve desired obtuse and acute angles.			RFI Response T-0453.1, stated that in lieu of bending the L8x8x1-1/8" member, it was acceptable to weld two 1-1/8" thick plates together in order to achieve desired obtuse and acute angles.			
1. Please confirm that additional embeds per detail 1 S1-3411, not reference in RFI T-0453.1, can be welded to create the specified angles per RFI response T-0853 (this will be an additional 2 angles). Please reference attached SKA-2863 for specified angles and locations of embeds in question.			1. Please confirm that additional embeds per detail 1 S1-3411, not reference in RFI T-0453.1, can be welded to create the specified angles per RFI response T-0853 (this will be an additional 2 angles). Please reference attached SKA-2863 for specified angles and locations of embeds in question.			
<b>T-0836</b>	<b>BGP - Sump Pit Rebar Tail and Trestle Pile @ GL 18.5/E - Area 9</b>	<b>Closed</b>	<b>CR</b>	<b>10/17/2013</b>	<b>10/27/2013</b>	<b>10/23/2013</b>
<hr/>						
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: RFI T-0644			Reference: RFI T-0644			
Three of the sump pit lower mat #11 tails near grid line 18.5/E are in conflict with the nearby trestle pile. The bars have been trimmed to clear the trestle pile and provide an LTE of 34" instead of 60" as required per plans.			Three of the sump pit lower mat #11 tails near grid line 18.5/E are in conflict with the nearby trestle pile. The bars have been trimmed to clear the trestle pile and provide an LTE of 34" instead of 60" as required per plans.			
Typically, a bent bar would be spliced to the interrupted bar as required in SKS-0281 in the response to RFI T-066; however, the trimmed bars have a 70" length which would not meet the 78" LTS requirement. Gerdau propose to leave the 3 ea trimmed bars as-is and not incorporate an additional spliced bent bar. Please confirm if this is acceptable.			Typically, a bent bar would be spliced to the interrupted bar as required in SKS-0281 in the response to RFI T-066; however, the trimmed bars have a 70" length which would not meet the 78" LTS requirement. Gerdau propose to leave the 3 ea trimmed bars as-is and not incorporate an additional spliced bent bar. Please confirm if this is acceptable.			



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<b>T-0837</b>	<b>BGP - Structural Details for Elevator Door Sill Plate Angles on Concourse Level</b>	<b>Closed</b>	<b>CR</b>	<b>10/17/2013</b>	<b>10/26/2013</b>	<b>11/07/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached drawing A1-2824 through A1-2847.  The architectural drawing note at the elevator door sill plates refer to the structural drawings for details. However, the current structural drawing set do not provide the applicable misc metal angle detail.  Please provide structural detail drawings showing the typical misc metal elevator door sill support angle. Please include mounting detail to concourse slab or topping slab detail, misc. metal details, and all pertinent information to accurately detail the elevator door sill plate angle.						<b>ANSWER:</b>  Please refer to attached drawing A1-2824 through A1-2847.  The architectural drawing note at the elevator door sill plates refer to the structural drawings for details. However, the current structural drawing set do not provide the applicable misc metal angle detail.  Please provide structural detail drawings showing the typical misc metal elevator door sill support angle. Please include mounting detail to concourse slab or topping slab detail, misc. metal details, and all pertinent information to accurately detail the elevator door sill plate angle.
<b>T-0837.1</b>	<b>BGP - Elevator Sill Conflict with Elevator Rail Embed Plate</b>	<b>Closed</b>	<b>CR</b>	<b>11/19/2013</b>	<b>11/29/2013</b>	<b>12/04/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  The architectural elevator sill angle/emebed (continous) detail shown in 4/A1-7576 is in conflict with the structural elevator rail support embed drawing in 4/S1-7130.  Please advise.						<b>ANSWER:</b>  The architectural elevator sill angle/emebed (continous) detail shown in 4/A1-7576 is in conflict with the structural elevator rail support embed drawing in 4/S1-7130.  Please advise.
<b>T-0838</b>	<b>BGP - Concourse Slab Opening Dimension Clarification at GL C/13</b>	<b>Closed</b>	<b>CR</b>	<b>10/17/2013</b>	<b>10/26/2013</b>	<b>10/25/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached drawing A1-2844 and S1-2204.  The slab opening east of GL 13 and north of GL C shown on drawing A1-2844 appears to be in conflict with the slab opening shown on drawing S1-2204.  Please confirm the aforementioned slab opening is 26'-3" x 8'-8 3/4".						<b>ANSWER:</b>  Please refer to attached drawing A1-2844 and S1-2204.  The slab opening east of GL 13 and north of GL C shown on drawing A1-2844 appears to be in conflict with the slab opening shown on drawing S1-2204.  Please confirm the aforementioned slab opening is 26'-3" x 8'-8 3/4".



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Time: 07:01 AM  
Job: 30100

<i>Number</i>	<i>Subject</i>	<i>Status</i>	<i>Choice Required</i>	<i>Date Created</i>	<i>Date Required</i>	<i>Date Answered</i>
<b>T-0838.1</b>	<b>BGP - Concourse Slab Opening Dimension Clarification at GL C/13</b>	<b>Closed</b>	<b>CR</b>	<b>10/29/2013</b>	<b>11/08/2013</b>	<b>11/20/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to the attached drawing A1-2844, S1-2204 and RFI T-0838.  WOJV is in receipt of AAI's response to RFI T-0838, in which the slab opening dimension is referenced in a drawing that has yet to be issued for construction (A1-2844, ASI 107).  Please provide the dimensions for the slab opening east of GL 13 and north of GL C as located on the current contract drawing A1-2844 dated 08/31/2012.						<b>ANSWER:</b>  Please refer to the attached drawing A1-2844, S1-2204 and RFI T-0838.  WOJV is in receipt of AAI's response to RFI T-0838, in which the slab opening dimension is referenced in a drawing that has yet to be issued for construction (A1-2844, ASI 107).  Please provide the dimensions for the slab opening east of GL 13 and north of GL C as located on the current contract drawing A1-2844 dated 08/31/2012.
<b>T-0839</b>	<b>SSS - Bolt Specifications</b>	<b>Closed</b>	<b>01</b>	<b>10/18/2013</b>	<b>10/28/2013</b>	<b>10/30/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  As per the Contract Drawings and Specifications all high strength bolts shall be A325, A490 & A354 BD. All TC bolts shall additionally conform to ASTM F1852 & F2280.  1. We propose to use TC bolts for all connections (shop & field) 1-1/8" diameter or less (unless galvanized). All galvanized bolts to be standard A325.  2. All bolts 1-1/4" to 1-1/2" diameter to be standard A490.  3. All bolts larger than 1-1/2" diameter to be standard A354 BD  Please confirm this is acceptable.						<b>ANSWER:</b>  As per the Contract Drawings and Specifications all high strength bolts shall be A325, A490 & A354 BD. All TC bolts shall additionally conform to ASTM F1852 & F2280.  1. We propose to use TC bolts for all connections (shop & field) 1-1/8" diameter or less (unless galvanized). All galvanized bolts to be standard A325.  2. All bolts 1-1/4" to 1-1/2" diameter to be standard A490.  3. All bolts larger than 1-1/2" diameter to be standard A354 BD  Please confirm this is acceptable.
<b>T-0840</b>	<b>SSS - Means &amp; Methods - Erection Devices</b>	<b>Closed</b>	<b>01</b>	<b>10/21/2013</b>	<b>10/31/2013</b>	<b>10/23/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  As per OSHA Standards Sub Part R Steel Erection 1926.756 (C)(1) When two structural members are sharing common connection holes, at least one bolt shall remain connected to the first member unless a shop or field						<b>ANSWER:</b>  As per OSHA Standards Sub Part R Steel Erection 1926.756 (C)(1) When two structural members are sharing common connection holes, at least one bolt shall remain connected to the first member unless a





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	<p>attached seat or equivalent connection device is supplied with the member to prevent displacement. We propose the Means &amp; Methods depicted in the attached sketch SK-1A to meet these OSHA requirements. Please advise if this is acceptable.</p>					<p>shop or field attached seat or equivalent connection device is supplied with the member to prevent displacement. We propose the Means &amp; Methods depicted in the attached sketch SK-1A to meet these OSHA requirements. Please advise if this is acceptable.</p>
<b>T-0841</b>	<b>SSS - Transfer Girder Splice Conflict with Clip Angle Connection</b>	<b>Closed</b>	<b>01</b>	<b>10/18/2013</b>	<b>10/28/2013</b>	<b>10/24/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> <p>As noted on sketches CD RFI 075 SK1 through SK8, there are several instances where the clip angle at the beam to transfer girder connection is in conflict with the transfer girder web splice. This condition occurs at TR7, TR8, TR11, TR19.1, TR19.9, TR21, and TR24.</p> <p>Please reference the sketches attached and confirm the modified transfer girder web splice locations are acceptable to avoid conflict with the beam clip angle connection.</p>						<b>ANSWER:</b> <p>As noted on sketches CD RFI 075 SK1 through SK8, there are several instances where the clip angle at the beam to transfer girder connection is in conflict with the transfer girder web splice. This condition occurs at TR7, TR8, TR11, TR19.1, TR19.9, TR21, and TR24.</p> <p>Please reference the sketches attached and confirm the modified transfer girder web splice locations are acceptable to avoid conflict with the beam clip angle connection.</p>
<b>T-0842</b>	<b>SSS - Full Height Columns</b>	<b>Closed</b>	<b>01</b>	<b>10/18/2013</b>	<b>10/28/2013</b>	<b>10/24/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> <p>Please refer to the attached drawing, S1-4104. The columns shown in the transverse frame elevation that extend from the ground level to the roof level typically have a field splice located 4' above the bus deck slab. Please confirm that this field splice may be eliminated and that it is acceptable to provide full height columns.</p> <p>The detail is shown at GL 7 &amp; GL 8. Other locations are similar.</p> <p>Note that a shop splice may be required due to limitations in mill rolling lengths.</p>						<b>ANSWER:</b> <p>Please refer to the attached drawing, S1-4104. The columns shown in the transverse frame elevation that extend from the ground level to the roof level typically have a field splice located 4' above the bus deck slab. Please confirm that this field splice may be eliminated and that it is acceptable to provide full height columns.</p> <p>The detail is shown at GL 7 &amp; GL 8. Other locations are similar.</p> <p>Note that a shop splice may be required due to limitations in mill rolling lengths.</p>





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0842.1</b>	<b>SSS - Moment Frame Column Splice</b>	<b>Closed</b>	<b>CR</b>	<b>01/27/2014</b>	<b>02/06/2014</b>	<b>02/04/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>						
In reference to W/O RFI T-0842, in which permission was given to eliminate field splices in the built-up columns, please see the following:						
At certain column locations (see S1-4102 at GL4, for example) the thickness of the flange is constant throughout the height of the column. The fabricator will seek to provide single piece flanges when material availability permits. In instances where the availability of certain plate sizes does not permit the fabricator to provide a single piece / full height flange plate, a shop splice will need to be introduced.						
Please identify any locations or areas along the height of the column flange that a shop splice is not permissible so that these limitations may be considered while finalizing our shop details and plate purchases.						
<b>ANSWER:</b>						
In reference to W/O RFI T-0842, in which permission was given to eliminate field splices in the built-up columns, please see the following:						
At certain column locations (see S1-4102 at GL4, for example) the thickness of the flange is constant throughout the height of the column. The fabricator will seek to provide single piece flanges when material availability permits. In instances where the availability of certain plate sizes does not permit the fabricator to provide a single piece / full height flange plate, a shop splice will need to be introduced.						
Please identify any locations or areas along the height of the column flange that a shop splice is not permissible so that these limitations may be considered while finalizing our shop details and plate purchases.						
<b>T-0843</b>	<b>SSS - PJP Welds at Roof Node to Brace Beam</b>	<b>Closed</b>	<b>01</b>	<b>10/18/2013</b>	<b>10/28/2013</b>	<b>10/25/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>						
Reference is made to sheet S1-4205, Detail 2 "Brace Detail" which specifies a 1 ¾" effective weld from roof node to brace beam. Sheet S1-5131, Detail 1, Side View F specifies a bevel of 2 3/8" x 45 degrees for the weld joint area in question.						
Sheet S-0007, General Note SC-4 states that weld sizes shown are considered effective weld sizes. Prequalified weld joint BTC-P4-GF (attached for reference) states that the effective weld size shall equal the bevel size for flat and horizontal weld positions.						
These welds are intended to be performed in the horizontal or flat position. Please confirm that a bevel size of 1 ¾" to equal the specified weld size of 1 ¾" is acceptable and conforms to the requirements of note SC-4 and AWS 2010 D1.1 Detail BTC-P4-GF attached.						
<b>ANSWER:</b>						
Reference is made to sheet S1-4205, Detail 2 "Brace Detail" which specifies a 1 ¾" effective weld from roof node to brace beam. Sheet S1-5131, Detail 1, Side View F specifies a bevel of 2 3/8" x 45 degrees for the weld joint area in question.						
Sheet S-0007, General Note SC-4 states that weld sizes shown are considered effective weld sizes. Prequalified weld joint BTC-P4-GF (attached for reference) states that the effective weld size shall equal the bevel size for flat and horizontal weld positions.						
These welds are intended to be performed in the horizontal or flat position. Please confirm that a bevel size of 1 ¾" to equal the specified weld size of 1 ¾" is acceptable and conforms to the requirements of note SC-4 and AWS 2010 D1.1 Detail BTC-P4-GF attached.						

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
<b>T-0844</b>	<b>SSS - PJP Weld at Roof Node to EBF Link Beam</b>	<b>Closed</b>	<b>01</b>	<b>10/18/2013</b>	<b>10/28/2013</b>	<b>10/24/2013</b>
<div> <div> <b>From:</b> Webcor Construction LP           Robert Kjome         </div> <div> <b>REQUEST:</b> <p>Reference is made to sheet S1-4205, Detail 1 "EBF Link Beam Detail" which specifies a 2 ¼" effective weld from roof node to EBF Link beam. Sheet S1-5131, Detail 2, Side View F specifies a bevel of 2 3/8" x 45 degrees for the weld joint area in question.</p> <p>Sheet S-0007, General Note SC-4 states that weld sizes shown are considered effective weld sizes. Prequalified weld joint BTC-P4-GF (attached for reference) states that the effective weld size shall equal the bevel size for flat and horizontal weld positions.</p> <p>These welds are intended to be performed in the horizontal or flat position. Please confirm that a bevel size of 2 ¼" to equal the specified weld size of 2 ¼" is acceptable and conforms to the requirements of note SC-4 and AWS 2010 D1.1 Detail BTC-P4-GF attached.</p> </div> <div> <b>ANSWER:</b> <p>Reference is made to sheet S1-4205, Detail 1 "EBF Link Beam Detail" which specifies a 2 ¼" effective weld from roof node to EBF Link beam. Sheet S1-5131, Detail 2, Side View F specifies a bevel of 2 3/8" x 45 degrees for the weld joint area in question.</p> <p>Sheet S-0007, General Note SC-4 states that weld sizes shown are considered effective weld sizes. Prequalified weld joint BTC-P4-GF (attached for reference) states that the effective weld size shall equal the bevel size for flat and horizontal weld positions.</p> <p>These welds are intended to be performed in the horizontal or flat position. Please confirm that a bevel size of 2 ¼" to equal the specified weld size of 2 ¼" is acceptable and conforms to the requirements of note SC-4 and AWS 2010 D1.1 Detail BTC-P4-GF attached.</p> </div> </div>						
<b>T-0845</b>	<b>SSS - Welding Type 61 Roof Nodes to Roof Beams</b>	<b>Closed</b>	<b>01</b>	<b>10/21/2013</b>	<b>10/31/2013</b>	<b>11/05/2013</b>
<div> <div> <b>From:</b> Webcor Construction LP           Robert Kjome         </div> <div> <b>REQUEST:</b> <p>Reference Drawings: S1-4205, S1-5132, S-0007</p> <p>Reference is made to sheet S1-5132, Detail 1, Side View D which specifies a bevel of 1" x 45 degrees for the weld joint for Type 61 roof nodes to the roof beam.</p> <p>Sheet S-0007, General Note SC-4 states that weld sizes shown are considered effective weld sizes. Prequalified weld joint BTC-P4-GF (attached for reference) states that the effective weld size shall equal the bevel size for flat and horizontal weld positions.</p> <p>These welds are intended to be performed in the horizontal or flat position. Based on the information provided above, please provide the required effective weld size at the area in question and confirm the bevel size is to match the specified weld size.</p> </div> <div> <b>ANSWER:</b> <p>Reference Drawings: S1-4205, S1-5132, S-0007</p> <p>Reference is made to sheet S1-5132, Detail 1, Side View D which specifies a bevel of 1" x 45 degrees for the weld joint for Type 61 roof nodes to the roof beam.</p> <p>Sheet S-0007, General Note SC-4 states that weld sizes shown are considered effective weld sizes. Prequalified weld joint BTC-P4-GF (attached for reference) states that the effective weld size shall equal the bevel size for flat and horizontal weld positions.</p> <p>These welds are intended to be performed in the horizontal or flat position. Based on the information provided above, please provide the required effective weld size at the area in question and confirm the bevel size is to match the specified weld size.</p> </div> </div>						



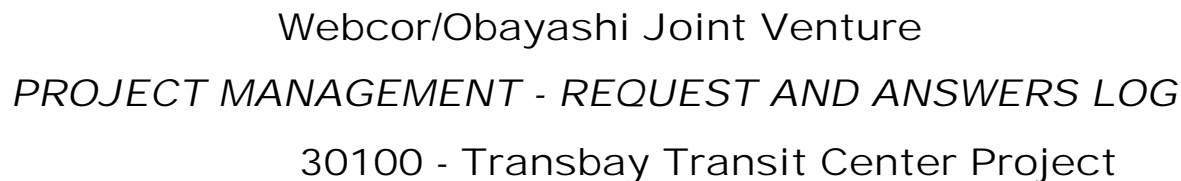
Webcor/Obayashi Joint Venture

PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

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Date: 02/11/2015  
Time: 07:01 AM  
Job: 30100

Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-0846	SSS - Grade 60 A615 Threaded Anchor Rod	Closed	01	10/21/2013	10/31/2013	10/23/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
With reference to the Grade 60 A615 Type T threaded anchor rod specified on detail 7/S1-5051 (attached), we request to substitute this material for the higher Grade 75 A615 anchor rod at no additional cost.		With reference to the Grade 60 A615 Type T threaded anchor rod specified on detail 7/S1-5051 (attached), we request to substitute this material for the higher Grade 75 A615 anchor rod at no additional cost.				
Please confirm this is acceptable.		Please confirm this is acceptable.				
T-0847	SSS - Weld Process for Roof Nodes at Roof Beams	Closed	01	10/21/2013	10/31/2013	10/28/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Please reference sheet S1-5131 Detail 1 Section F, Detail 2 Section F, and sheet S1-5132 Section D. OIW is proposing to perform the CJP welds from P3 to P4 using a "Narrow Gap Improved Electroslag Weld (NGI ESW)" process. AWS D1.8 Section 6.2.1 allows the use of alternate weld processes contingent upon approval by the Engineer.		Please reference sheet S1-5131 Detail 1 Section F, Detail 2 Section F, and sheet S1-5132 Section D. OIW is proposing to perform the CJP welds from P3 to P4 using a "Narrow Gap Improved Electroslag Weld (NGI ESW)" process. AWS D1.8 Section 6.2.1 allows the use of alternate weld processes contingent upon approval by the Engineer.				
Attached is a detailed narrative and supporting data for this welding process including the following: -Process Details, General Parameters, and Practices from ARCMATIC (OIW welding consultant) -Sample Welding Procedure Data Sheets (WPS) including MTRs and destructive testing		Attached is a detailed narrative and supporting data for this welding process including the following: -Process Details, General Parameters, and Practices from ARCMATIC (OIW welding consultant) -Sample Welding Procedure Data Sheets (WPS) including MTRs and destructive testing				
Upon conceptual approval of this process, applicable and job specific PQR/WPS data will be provided for Engineer review.		Upon conceptual approval of this process, applicable and job specific PQR/WPS data will be provided for Engineer review.				
Please confirm that NGI ESW welding process is acceptable in this application.		Please confirm that NGI ESW welding process is acceptable in this application.				
T-0847.1	SSS - Weld Process for Roof Nodes at Roof Beams	Closed	CR	11/25/2013	12/05/2013	11/26/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST:		ANSWER:				
The response to RFI T-0847 states that "WPS shall be prepared in accordance with AWS D1.5," while the specifications require that welds be prepped in accordance with AWS D1.1 and D1.8. Please verify that the reference		The response to RFI T-0847 states that "WPS shall be prepared in accordance with AWS D1.5," while the specifications require that welds be prepped in accordance with AWS D1.1 and D1.8. Please verify				



# 30100 - Transbay Transit Center Project



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0849</b>	<b>BGP - Mat Slab Layer 3 Lap Splice Relocation in Area 11 thru 16</b>	<b>Closed</b>	<b>CR</b>	<b>10/21/2013</b>	<b>10/31/2013</b>	<b>10/23/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to drawing S1-2052.  Due to limited access between the waterproofing and access trestle, Gerdau proposes to shorten the mat slab typical layer three (North-South) 67'-0" bars at Areas 11 through 16. This requires the lap splice location to be moved from the center of column line, as specified on Note 1 of the Mat Top Bar Notes in S1-2052, to the location shown in the attached Gerdau sketch SK-99.  Please confirm the revised lap splice detail shown in Gerdau sketch SK-99 is acceptable.		<b>ANSWER:</b>  Please refer to drawing S1-2052.  Due to limited access between the waterproofing and access trestle, Gerdau proposes to shorten the mat slab typical layer three (North-South) 67'-0" bars at Areas 11 through 16. This requires the lap splice location to be moved from the center of column line, as specified on Note 1 of the Mat Top Bar Notes in S1-2052, to the location shown in the attached Gerdau sketch SK-99.  Please confirm the revised lap splice detail shown in Gerdau sketch SK-99 is acceptable.				
<b>T-0850</b>	<b>BGP - Request for 14 day Concrete Compressive Strength test on future mat slab p</b>	<b>Closed</b>	<b>CR</b>	<b>10/22/2013</b>	<b>11/01/2013</b>	<b>10/25/2013</b>
<b>From:</b> Webcor Construction LP Michael Spillane						
<b>REQUEST:</b>  Per discussion with TT field Engineer and TJPA representatives, WOJV is asking for all future mat slab pours that one of the two concrete test cylinders allotted for the 28 day compressive strength test could be tested at 14 days instead, This information will be used to assess the concrete strength for the level D bracing removal.  Please confirm if this would be acceptable.		<b>ANSWER:</b>  Per discussion with TT field Engineer and TJPA representatives, WOJV is asking for all future mat slab pours that one of the two concrete test cylinders allotted for the 28 day compressive strength test could be tested at 14 days instead, This information will be used to assess the concrete strength for the level D bracing removal.  Please confirm if this would be acceptable.				
<b>T-0852</b>	<b>SSS - Weld Returns at EBF Link Beams</b>	<b>Closed</b>	<b>01</b>	<b>10/24/2013</b>	<b>11/03/2013</b>	<b>10/25/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  Detail 3 on sheet S1-4205 indicates the weld requirements from the underside of the EBF link beam (28" W) to the roof node (24" W). Detail 3 requires a 3 ½" reinforcing weld to be returned (boxing) 6" at each interior corner of the welded roof node. The distance from the roof node to the edge of the girder flange is only 2" on each side based on the dimensions noted above (reference drawings attached).		<b>ANSWER:</b>  Detail 3 on sheet S1-4205 indicates the weld requirements from the underside of the EBF link beam (28" W) to the roof node (24" W). Detail 3 requires a 3 ½" reinforcing weld to be returned (boxing) 6" at each interior corner of the welded roof node. The distance from the roof node to the edge of the girder flange is only 2" on each side based on the dimensions noted above (reference drawings attached).				



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>Please confirm it is acceptable for the returns running longitudinal to the direction of the EBF Beam to be made as 1 ½" reinforcing fillets, while the weld running transverse to the girder flange remain at 3 ½" as specified. Reference the attached detail showing this condition.</p>					<p>Please confirm it is acceptable for the returns running longitudinal to the direction of the EBF Beam to be made as 1 ½" reinforcing fillets, while the weld running transverse to the girder flange remain at 3 ½" as specified. Reference the attached detail showing this condition.</p>
T-0853	<b>SSS - Transfer Girder Field Splice</b>	Closed	01	10/24/2013	11/03/2013	11/04/2013
	<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>In order to facilitate self-supporting erection of the transfer girders during temporary conditions prior to the completion of the field welded splice joints, please confirm it is acceptable to utilize a temporary connection plate that will bolt the two transfer girders together while the weld takes place, as shown on the attached sketches GS-1.0 and GS-2.0. The temporary connection plate will be removed and open holes will be permanently filled with A325 bolts.</p>					<p><b>ANSWER:</b></p> <p>In order to facilitate self-supporting erection of the transfer girders during temporary conditions prior to the completion of the field welded splice joints, please confirm it is acceptable to utilize a temporary connection plate that will bolt the two transfer girders together while the weld takes place, as shown on the attached sketches GS-1.0 and GS-2.0. The temporary connection plate will be removed and open holes will be permanently filled with A325 bolts.</p>
T-0854	<b>SSS - Type 4 Drag Connection (Y)</b>	Closed	01	10/25/2013	11/04/2013	10/29/2013
	<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>For Type 4 Drag connection (Y) per detail 1/S1-5019 please refer to sketches CD RFI # 082 SK1 to SK3 for items 1 &amp; 2 noted below. Note sample location is on S1-2402 near grids 2/C.3 shown on SK2.</p> <p>1) See SK2 &amp; SK3 and confirm this 18" applies at all locations noted as "Y" on plans as this will place the bolts exceedingly outside the supporting beam profile. 2) Please clarify which plan drawings this note applies to.</p>					<p><b>ANSWER:</b></p> <p>For Type 4 Drag connection (Y) per detail 1/S1-5019 please refer to sketches CD RFI # 082 SK1 to SK3 for items 1 &amp; 2 noted below. Note sample location is on S1-2402 near grids 2/C.3 shown on SK2.</p> <p>1) See SK2 &amp; SK3 and confirm this 18" applies at all locations noted as "Y" on plans as this will place the bolts exceedingly outside the supporting beam profile. 2) Please clarify which plan drawings this note applies to.</p>
T-0855	<b>SSS - Double Angled Connection</b>	Closed	01	10/25/2013	11/04/2013	10/29/2013
	<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p>					











<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>REQUEST:</b> On details 2, 3 & 4/S1-7108 and section A/S1-7136 per ASI 0106 please refer to sketches CD RFI 106 SK1 & SK2 and clarify the discrepancy in framing that is shown on the referenced drawings. Note the elevator vertical was removed on ASI 0106 but a similar vertical is shown on section C/S1-7136.						
<b>ANSWER:</b> On details 2, 3 & 4/S1-7108 and section A/S1-7136 per ASI 0106 please refer to sketches CD RFI 106 SK1 & SK2 and clarify the discrepancy in framing that is shown on the referenced drawings. Note the elevator vertical was removed on ASI 0106 but a similar vertical is shown on section C/S1-7136.						
<b>T-0859.1</b>	<b>SSS - Elevator Framing</b>	<b>Closed</b>	<b>CR</b>	<b>12/19/2013</b>	<b>12/29/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> In attached drawings S1-7108, S1-7136 and S1-7137, we have highlighted structural members we consider are part of TG07.1R scope around elevators PE502 and PE503. Please confirm.						
<b>ANSWER:</b> In attached drawings S1-7108, S1-7136 and S1-7137, we have highlighted structural members we consider are part of TG07.1R scope around elevators PE502 and PE503. Please confirm.						
<b>T-0860</b>	<b>BGP - Area 3 Drill and Epoxy Walls</b>	<b>Closed</b>	<b>01</b>	<b>10/25/2013</b>	<b>11/04/2013</b>	<b>11/07/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference: Contract Dwg. A1-2122 to A1-2123, and attached sketch  Rebar dowels were installed for future partition walls at gridlines A-F/1-6 during Area 3 mat slab pour on September 7, 2013. Due to conflicts with equipment access for the removal of Level C and D shoring struts and walers, selected rebar areas as shown on the attached drawing will need to be cut at and removed. Any additional walls that are found to be blocking access once operations have begun will be analyzed on an as needed basis. Please confirm it is acceptable to cut rebar dowels in the partition walls as shown on the attached sketch and on an as needed basis, with exception to columns and wall pilasters, then return to drill and bond after bracing procedures are complete. Scanning will be included.						
<b>ANSWER:</b> Reference: Contract Dwg. A1-2122 to A1-2123, and attached sketch  Rebar dowels were installed for future partition walls at gridlines A-F/1-6 during Area 3 mat slab pour on September 7, 2013. Due to conflicts with equipment access for the removal of Level C and D shoring struts and walers, selected rebar areas as shown on the attached drawing will need to be cut at and removed. Any additional walls that are found to be blocking access once operations have begun will be analyzed on an as needed basis. Please confirm it is acceptable to cut rebar dowels in the partition walls as shown on the attached sketch and on an as needed basis, with exception to columns and wall pilasters, then return to drill and bond after bracing procedures are complete. Scanning will be included.						
<b>T-0861</b>	<b>BGP - Interior Wall Thickness Change Clarification in Area 8 &amp; 11</b>	<b>Closed</b>	<b>CR</b>	<b>10/28/2013</b>	<b>11/07/2013</b>	<b>11/06/2013</b>





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0862</b>	<b>SSS -Full Height Stiffener Detail Clarifications</b>	<b>Closed</b>	<b>01</b>	<b>10/28/2013</b>	<b>10/28/2013</b>	<b>11/05/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Please reference detail 1/S1-5013 regarding the full height fitted stiffener detail and confirm the following:  1) Confirm it is acceptable to provide a 2" end distance typically at beams with 7/8" dia. bolts in lieu of the 1 3/4" end distance noted by the "2db" dimension.  2) Confirm the stiffener width is to equal the beam "a" dimension, defined as [bf - tw]/2, thus the noted dimension should read "2db min."		<b>ANSWER:</b>  Please reference detail 1/S1-5013 regarding the full height fitted stiffener detail and confirm the following:  1) Confirm it is acceptable to provide a 2" end distance typically at beams with 7/8" dia. bolts in lieu of the 1 3/4" end distance noted by the "2db" dimension.  2) Confirm the stiffener width is to equal the beam "a" dimension, defined as [bf - tw]/2, thus the noted dimension should read "2db min."				
<b>T-0863</b>	<b>SSS - Double Angled Connections at TPG1 &amp; TPG3</b>	<b>Closed</b>	<b>01</b>	<b>10/28/2013</b>	<b>11/07/2013</b>	<b>11/07/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  For the double angle beam connections per detail 1/S1-5010 into the TPG1 & TPG3 roof girders on detail 1/S1-4200 are problematic due to the thick flanges. See sketches CD RFI 091 SK1 & SK2 for items 1 & 2 below for proposed modified connection.  1) Confirm it is acceptable to reduce the end distance on the connection angles to 1 1/4" per A.I.S.C.13th Edition Table J3.4 in order to fit the connection angles inside the beams at the TPG1 & TPG3 girders.  2) Confirm it is acceptable to cut the beam flanges flush as shown when the connection angles encroach into the beam "k" area beyond A.I.S.C. allowable limits.		<b>ANSWER:</b>  For the double angle beam connections per detail 1/S1-5010 into the TPG1 & TPG3 roof girders on detail 1/S1-4200 are problematic due to the thick flanges. See sketches CD RFI 091 SK1 & SK2 for items 1 & 2 below for proposed modified connection.  1) Confirm it is acceptable to reduce the end distance on the connection angles to 1 1/4" per A.I.S.C.13th Edition Table J3.4 in order to fit the connection angles inside the beams at the TPG1 & TPG3 girders.  2) Confirm it is acceptable to cut the beam flanges flush as shown when the connection angles encroach into the beam "k" area beyond A.I.S.C. allowable limits.				
<b>T-0864</b>	<b>SSS - Beam to Column Connection at Roof Level</b>	<b>Closed</b>	<b>01</b>	<b>10/28/2013</b>	<b>11/07/2013</b>	<b>11/04/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>  Reference sheet S1-2606 for the BU beam to column connection at grids 31/D.4 and 31/E.6. Please confirm it is acceptable to reduce the "Lev" dimension indicated on 3/S1-5011 from 5" to 2 3/4" in order to clear the BU flange to web weld as indicated in CD RFI 092 SK1 & SK2.		<b>ANSWER:</b>  Reference sheet S1-2606 for the BU beam to column connection at grids 31/D.4 and 31/E.6. Please confirm it is acceptable to reduce the "Lev" dimension indicated on 3/S1-5011 from 5" to 2 3/4" in order to clear the BU flange to web weld as indicated in CD				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<p>If this is not acceptable, please provide an alternate detail for this condition.</p>						
<p>RFI 092 SK1 &amp; SK2.</p>						
<p>If this is not acceptable, please provide an alternate detail for this condition.</p>						
<b>T-0865</b>	<b>SSS - Clarifications for Kicker Brace at Ground Level</b>	<b>Closed</b>	<b>01</b>	<b>10/28/2013</b>	<b>11/07/2013</b>	<b>11/07/2013</b>
<p><b>From:</b> Webcor Construction LP      Robert Kjome</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please refer to detail 6/S1-5022 and verify the kicker brace requirements at ground level as noted on CD RFI 093 SK1 to SK3 and in the items below:			Please refer to detail 6/S1-5022 and verify the kicker brace requirements at ground level as noted on CD RFI 093 SK1 to SK3 and in the items below:			
1) Confirm the alternate bracing connection proposed in CD RFI 093 SK1 is acceptable.			1) Confirm the alternate bracing connection proposed in CD RFI 093 SK1 is acceptable.			
2) Supply the weld size and length for brace angles to $\frac{1}{2}$ inch plate.			2) Supply the weld size and length for brace angles to $\frac{1}{2}$ inch plate.			
3) Confirm the work point location indicated is acceptable (intended to match S1-5015 details).			3) Confirm the work point location indicated is acceptable (intended to match S1-5015 details).			
4) Confirm the reference to S1-2304 should be added to the referenced detail and the reference to S1-2307 should be deleted.			4) Confirm the reference to S1-2304 should be added to the referenced detail and the reference to S1-2307 should be deleted.			
5) Confirm detail 6/S1-5022 applies only to grid lines 16.9, 19.1, 24.9 & 27.1 on the Ground Level as referenced on plans.			5) Confirm detail 6/S1-5022 applies only to grid lines 16.9, 19.1, 24.9 & 27.1 on the Ground Level as referenced on plans.			
6) Confirm detail 6/S1-5022 is typical for all braces along grid line 16.9, similar to grid line 19.1.			6) Confirm detail 6/S1-5022 is typical for all braces along grid line 16.9, similar to grid line 19.1.			
7) Confirm detail 6/S1-5022 is typical for all braces along grid line 27.1, similar to grid line 19.1.			7) Confirm detail 6/S1-5022 is typical for all braces along grid line 27.1, similar to grid line 19.1.			
<b>T-0866</b>	<b>SSS - Bending Radius at Skewed Beam Connections</b>	<b>Closed</b>	<b>01</b>	<b>10/28/2013</b>	<b>11/07/2013</b>	<b>11/07/2013</b>
<p><b>From:</b> Webcor Construction LP      Robert Kjome</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Refer to details 7 & 8/S1-5010 regarding bending radius requirements for skewed beam connections. The radius indicated in CD RFI 095 SK2 is per A.I.S.C. (2.5t for A572 GR50 material). Please confirm it is acceptable to proceed per this criteria.			Refer to details 7 & 8/S1-5010 regarding bending radius requirements for skewed beam connections. The radius indicated in CD RFI 095 SK2 is per A.I.S.C. (2.5t for A572 GR50 material). Please confirm it is acceptable to proceed per this criteria.			



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-0867	SSS - W24 Skewed Beam Connections at Grid 6.C.3	Closed	01	10/28/2013	11/07/2013	11/07/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Refer to drawing S1-2303 (CD RFI 096 SK1) indicating the portion of the W24x68 running between GL C.3 and GL 6. CD RFI 096 SK2 shows the tight design requirements for this beam run connecting to TR6. Please advise if this portion of the W24x68 beam can be eliminated due to the tight design requirements. If eliminating this portion of the beam is not acceptable, please provide an alternate connection detail to TR6, as detail 8/S1-5010 will not work at this location.		Refer to drawing S1-2303 (CD RFI 096 SK1) indicating the portion of the W24x68 running between GL C.3 and GL 6. CD RFI 096 SK2 shows the tight design requirements for this beam run connecting to TR6. Please advise if this portion of the W24x68 beam can be eliminated due to the tight design requirements. If eliminating this portion of the beam is not acceptable, please provide an alternate connection detail to TR6, as detail 8/S1-5010 will not work at this location.				
T-0868	SSS - Framing Clarification for W21 Beams at Ground Level	Closed	01	10/28/2013	11/07/2013	11/07/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Refer to the areas indicated on S1-2303 between grids 10.1 & 11 and D & F (CD RFI 097 SK1). Please confirm the noted W21x50 beams are at top of steel elevation 19'-1 5/8" and the BU-WT's are not required.		Refer to the areas indicated on S1-2303 between grids 10.1 & 11 and D & F (CD RFI 097 SK1). Please confirm the noted W21x50 beams are at top of steel elevation 19'-1 5/8" and the BU-WT's are not required.				
T-0868.1	SSS - Framing Clarification for W21 Beams at Ground Level	Closed	CR	11/25/2013	12/05/2013	12/20/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST:		ANSWER:				
Per the response to W/O RFI T-0868 (SK RFI 135), the TOS for the W21 should be at 16'-11" and the BU-WTs are required to support the slab at 19'-9 1/8". Based on this response, please confirm the following:		Per the response to W/O RFI T-0868 (SK RFI 135), the TOS for the W21 should be at 16'-11" and the BU-WTs are required to support the slab at 19'-9 1/8". Based on this response, please confirm the following:				
1) The difference between the TOS elevations per the response to SK RFI 135 requires a BU-WT with a total height of 2'-2 5/8", exceeding the maximum height dimension indicated on 5/S1-5002. Please confirm it is acceptable to proceed with detail 5/S1-5002 and the required BU-WT height of 2'-2 5/8" at this location.		1) The difference between the TOS elevations per the response to SK RFI 135 requires a BU-WT with a total height of 2'-2 5/8", exceeding the maximum height dimension indicated on 5/S1-5002. Please confirm it is acceptable to proceed with detail 5/S1-5002 and the required BU-WT height of 2'-2 5/8" at this location.				
2) Please confirm it is acceptable to stop the BU-WTs 1" clear from the edge of the transfer girder flange to allow for erection clearance or advise if the BU-WTs are required to extend to the face of the transfer girder web for deck support. (Reference CD RFI 097.1 SK1)		2) Please confirm it is acceptable to stop the BU-WTs 1" clear from the edge of the transfer girder flange to allow for erection clearance or advise if the BU-WTs are required to extend to the face of the transfer girder web for deck support. (Reference CD RFI 097.1 SK1)				



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>3) The W21 connection to the transfer girder at grid line 11 fouls the bottom flange of the girder and cap plate of the train box columns as indicated on CD RFI 097.1 SK1. Please provide an alternate connection detail at this location.</p> <p>4) As indicated on CD RFI 097.1 SK2, there is no support down for the escalator slab perpendicular to the W21 near the edge of the knock-out slab and the W21 supporting the S4 escalator slab. Please advise if deck support is required at this location and, if so, please provide details as required.</p>					
T-0868.2	SSS - Escalator Pit Framing Details GL10.1	Closed	CR	03/24/2014	04/03/2014	04/09/2014
<p><b>From:</b> Webcor Construction LP Gregory Kemerer</p>						
<p><b>REQUEST:</b></p> <p>This is a follow-up RFI to RFI T-0868.1 (SK 135.1, CD 097.1) See attached CD RFI # 097.2 SK1 to SK3 for items 1 to 3:</p> <p>1) The section looking west on SK1 as modeled on SK2 is not what is shown in detail 4/A1-7550 (SK3) as referenced in T-0868.1 #4 response. Confirm the structural drawing SK1 &amp; SK2 as shown are correct.</p> <p>2) As per Thornton Tomasetti's revised email response to RFI T-0868.1 (see SK4), the bent plate should be added at knock-out slab edge as shown on 4/A1-7550 (SK3). Please confirm.</p> <p>3) On 4/A1-7550 (SK3) there is gauge or bent plate shown at the pit slab edge. Please verify if bent plate should be provided or will this be gauge plate?</p>						
<p><b>ANSWER:</b></p> <p>This is a follow-up RFI to RFI T-0868.1 (SK 135.1, CD 097.1) See attached CD RFI # 097.2 SK1 to SK3 for items 1 to 3:</p> <p>1) The section looking west on SK1 as modeled on SK2 is not what is shown in detail 4/A1-7550 (SK3) as referenced in T-0868.1 #4 response. Confirm the structural drawing SK1 &amp; SK2 as shown are correct.</p> <p>2) As per Thornton Tomasetti's revised email response to RFI T-0868.1 (see SK4), the bent plate should be added at knock-out slab edge as shown on 4/A1-7550 (SK3). Please confirm.</p> <p>3) On 4/A1-7550 (SK3) there is gauge or bent plate shown at the pit slab edge. Please verify if bent plate should be provided or will this be gauge plate?</p>						
T-0869	SSS - Coping Brace Beam Bottom Flange	Closed	01	10/29/2013	11/08/2013	11/11/2013







<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-0872	SSS - Drag Connection Clarification for Kicker Brace	Closed	01	10/30/2013	11/09/2013	11/07/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:			ANSWER:			
Reference drawing S1-2303 and CD RFI 116 SK1 indicating the W40 beam connection to TR11 at Grid F.11. This detail requires a full height shear plate per 1/S1-5019 and bracing per 5/S1-5015. (Reference CD RFI 116 SK2). This same condition occurs on S1-2303 along grid D.11.			Reference drawing S1-2303 and CD RFI 116 SK1 indicating the W40 beam connection to TR11 at Grid F.11. This detail requires a full height shear plate per 1/S1-5019 and bracing per 5/S1-5015. (Reference CD RFI 116 SK2). This same condition occurs on S1-2303 along grid D.11.			
Please confirm it is acceptable to connect the required kicker brace to the 1 ½" full depth shear plate and increase the gusset plate below the beam to 1 ½" thick. Otherwise, please provide an acceptable detail for this condition.			Please confirm it is acceptable to connect the required kicker brace to the 1 ½" full depth shear plate and increase the gusset plate below the beam to 1 ½" thick. Otherwise, please provide an acceptable detail for this condition.			
T-0873	BGP - Spandrel Beam Modifications in Area 8	Closed	CR	10/30/2013	10/30/2013	11/07/2013
From: Webcor Construction LP Michael Spillane						
REQUEST:			ANSWER:			
Further to response to RFI T-637 please find attached proposed changes to the spandrel beams in pour Area 8 for location plan see exhibit - A Exhibit - B shows the plan view of the modification necessary to the spandrel beam on the north and south elevations due to the revised reinforcement width of the foundation wall due to encroachment of the CDSM beams as well as typical cross sections of the revised spandrel beams. RFI T - 724 shows the extent of the modification to the foundation wall on the north and south elevations of Area 8.			Further to response to RFI T-637 please find attached proposed changes to the spandrel beams in pour Area 8 for location plan see exhibit - A Exhibit - B shows the plan view of the modification necessary to the spandrel beam on the north and south elevations due to the revised reinforcement width of the foundation wall due to encroachment of the CDSM beams as well as typical cross sections of the revised spandrel beams. RFI T - 724 shows the extent of the modification to the foundation wall on the north and south elevations of Area 8.			
Please confirm that this modification as outlined at these locations is acceptable.			Please confirm that this modification as outlined at these locations is acceptable.			
T-0874	BGP - Spandrel Beam Modifications in Area 9	Closed	01	10/31/2013	11/10/2013	11/12/2013
From: Webcor Construction LP Michael Spillane						
REQUEST:			ANSWER:			
Further to response to RFI T-637 please find attached proposed changes to the spandrel beams in pour Area 8 for location plan see exhibit - A			Further to response to RFI T-637 please find attached proposed changes to the spandrel beams in pour Area 8 for location plan see exhibit - A			





<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
	<p>Exhibit - B shows the plan view of the modification necessary to the spandrel beam on the north and south elevations due to the revised reinforcement width of the foundation wall due to encroachment of the CDSM beams as well as typical cross sections of the revised spandrel beams.</p> <p>RFI T - 742 shows the extent of the modification to the foundation wall on the north and south elevations of Area 9.</p> <p>Please confirm that this modification as outlined at these locations is acceptable.</p>					
<b>T-0875</b>	<b>BGP- Trestle piles No 20 &amp; 21 in conflict with beams at Lower Concourse level</b>	<b>Closed</b>	<b>CR</b>	<b>11/01/2013</b>	<b>11/11/2013</b>	<b>11/07/2013</b>
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>						
<p>Following a review and discussion with Thornton Tomasetti on the trestle pile locations, it has been noted that trestle pile numbers 20 and 21 (see sketches attached) are in conflict with beams (B4A) at the escalator pits on the lower concourse slab elevation between gridline 11-12, D-F. The contractor is proposing to blockout a section of slab as shown on the sketch, this blockout section would then be infilled once the trestle pile has been removed.</p> <p>The contractor is to insure that the appropriate reinforcement lap splices are present between these concrete pours.</p> <p>Please confirm if this option would be acceptable</p>						
<b>ANSWER:</b>						
<p>Following a review and discussion with Thornton Tomasetti on the trestle pile locations, it has been noted that trestle pile numbers 20 and 21 (see sketches attached) are in conflict with beams (B4A) at the escalator pits on the lower concourse slab elevation between gridline 11-12, D-F. The contractor is proposing to blockout a section of slab as shown on the sketch, this blockout section would then be infilled once the trestle pile has been removed.</p> <p>The contractor is to insure that the appropriate reinforcement lap splices are present between these concrete pours.</p> <p>Please confirm if this option would be acceptable</p>						





<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>drawings conflict with many of the dimensions provided on the architectural slab edge plans (AI-2842 through AI-2847).</p> <p>Please see attached for observed conflicts (highlighted). Please confirm that the dimensions shown on the architectural plans at the slab openings are correct.</p>					
	<p>from these drawings conflict with many of the dimensions provided on the architectural slab edge plans (AI-2842 through AI-2847).</p> <p>Please see attached for observed conflicts (highlighted). Please confirm that the dimensions shown on the architectural plans at the slab openings are correct.</p>					
T-0879	<b>BGP - Elevator Opening Embed Conflicts with Future Walls</b>	Closed	CR	11/04/2013	11/14/2013	11/19/2013
	<p><b>From:</b> Webcor Construction LP Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Please refer to attached Detail4 on SI-7630, attached AI-2202 thru AI-2205 and AI-2207. The following drawings are for reference SI-2202 thru SI-2205 and SI-2207, SI-7130, SI-7132, SI-7134, SI-7136 and SI-7139.</p> <p>Please confirm no conflict exists between embed Detail 4 on S1-7630 and future walls highlighted on attached architectural drawings.</p>					
	<p><b>ANSWER:</b></p> <p>Please refer to attached Detail4 on SI-7630, attached AI-2202 thru AI-2205 and AI-2207. The following drawings are for reference SI-2202 thru SI-2205 and SI-2207, SI-7130, SI-7132, SI-7134, SI-7136 and SI-7139.</p> <p>Please confirm no conflict exists between embed Detail 4 on S1-7630 and future walls highlighted on attached architectural drawings.</p>					
T-0879.1	<b>BGP - Conflict of Elevator Opening Embed and Future Walls</b>	Closed	CR	11/25/2013	12/05/2013	12/09/2013
	<p><b>From:</b> Webcor Construction LP Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>SCCI is in receipt of RFI response T-0879. TT's response does not fully address the conflict brought up in the original RFI. TG06.0 contract drawings do not show a curb at the edge of the elevator openings at the lower concourse level.</p> <p>Please address and provide details regarding the embed in question in RFI T-0879.</p>					
	<p><b>ANSWER:</b></p> <p>SCCI is in receipt of RFI response T-0879. TT's response does not fully address the conflict brought up in the original RFI. TG06.0 contract drawings do not show a curb at the edge of the elevator openings at the lower concourse level.</p> <p>Please address and provide details regarding the embed in question in RFI T-0879.</p>					
T-0880	<b>BGP - Receptacle Requirements at Elevator Pits Near GL 19/E and 20/G</b>	Closed	CR	11/04/2013	11/14/2013	11/13/2013
	<p><b>From:</b> Webcor Construction LP Jackson Tukuafu</p>					



# 30100 - Transbay Transit Center Project



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0882</b>	<b>BGP - Column Tie Change from T9 to T12</b>	<b>Closed</b>	<b>CR</b>	<b>11/05/2013</b>	<b>11/15/2013</b>	<b>11/13/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to drawing S1-3304 to S1-3306.  Please confirm if it is acceptable to replace the typical T9 column ties (90° or 135° bend on either end) with T12 ties (135° bends on both ends). See the attached SCCI sketch SK-101 for further details.						<b>ANSWER:</b>  Please refer to drawing S1-3304 to S1-3306.  Please confirm if it is acceptable to replace the typical T9 column ties (90° or 135° bend on either end) with T12 ties (135° bends on both ends). See the attached SCCI sketch SK-101 for further details.
<b>T-0883</b>	<b>SSS - Brace Beam Connection Details</b>	<b>Closed</b>	<b>01</b>	<b>11/05/2013</b>	<b>11/15/2013</b>	<b>11/18/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  Please review sketch CD RFI 059 SK1 and details 1/S1-5016 and 1/S1-5018 for type 1 - drag connection details on brace beams at the Bus Deck Level framing plan.  1). Please verify the bottom flange of brace beams noted in detail 1/S1-5016 can be cut flush to the beam web on both sides of web allowing beam to be erected between the shop welded connection plates on the cast node.  2). Verify the diagonal bracing beam web connection plate noted in detail 1/S1-5018 can be shifted to the acute angle side of the connection as indicated in the attached sketch and bottom flange cut flush to the web to allow beam to be dropped into location in the field.  3). Please provide welding details for the relocated web connection plate to the supporting grid beam as connection plates will overlap at these locations.  4). Please verify if additional bolts are required connecting the flange plate where the dimension to the plate edge and the last row of connection bolts exceeds limitations noted in the 13th Edition (AISC) manual section 16.1-J3, Item 5a.						<b>ANSWER:</b>  Please review sketch CD RFI 059 SK1 and details 1/S1-5016 and 1/S1-5018 for type 1 - drag connection details on brace beams at the Bus Deck Level framing plan.  1). Please verify the bottom flange of brace beams noted in detail 1/S1-5016 can be cut flush to the beam web on both sides of web allowing beam to be erected between the shop welded connection plates on the cast node.  2). Verify the diagonal bracing beam web connection plate noted in detail 1/S1-5018 can be shifted to the acute angle side of the connection as indicated in the attached sketch and bottom flange cut flush to the web to allow beam to be dropped into location in the field.  3). Please provide welding details for the relocated web connection plate to the supporting grid beam as connection plates will overlap at these locations.  4). Please verify if additional bolts are required connecting the flange plate where the dimension to the plate edge and the last row of connection bolts exceeds limitations noted in the 13th Edition (AISC) manual section 16.1-J3, Item 5a.
<b>T-0883.1</b>	<b>SSS - Brace Beam Connection Details</b>	<b>Closed</b>	<b>CR</b>	<b>12/11/2013</b>	<b>12/21/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						

Webcor/Obayashi Joint Venture  
*PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG*  
 30100 - Transbay Transit Center Project



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Time: 07:01 AM

Job: 30100

Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-0886	BGP - Round Column Tie-Hook Modification	Closed	CR	11/07/2013	11/17/2013	11/15/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST: Please refer to drawing S1-3304.  In the round columns (type A1, A2, A3, B1, B2 and B3), Gerdau proposes to change the 90° hooks to 135° hooks in order to allow for more room to install the vertical bars and their couplers. Please refer to attached SCCI sketch SK-RFI-373 for reference of proposed detail.  Please confirm if this is acceptable.		ANSWER: Please refer to drawing S1-3304.  In the round columns (type A1, A2, A3, B1, B2 and B3), Gerdau proposes to change the 90° hooks to 135° hooks in order to allow for more room to install the vertical bars and their couplers. Please refer to attached SCCI sketch SK-RFI-373 for reference of proposed detail.  Please confirm if this is acceptable.				
T-0887	SSS - Moment Beam to Column Web Connection Clarifications	Closed	01	11/07/2013	11/17/2013	11/19/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST: Please refer to the moment beam to column web connection details on 5/S1-5012, 10/S1-5013, and 2/S1-5019 in regards to the following: 1) Please confirm the dimensions and weld prep noted are acceptable. [Reference CD RFI 080 SK1] 2) Confirm the increased thickness and placement of the continuity plate are acceptable to allow for beam over roll. [Reference CD RFI 080 SK1] 3) Please confirm the continuity plate dimensions noted on CD RFI 080 SK2 are acceptable. Note that the "a" dimension shown is defined as ½(bf-tw). 4) Please confirm the dimensions and weld prep indicated for the Type 4 Drag connection are acceptable. [Reference CD RFI 080 SK3]		ANSWER: Please refer to the moment beam to column web connection details on 5/S1-5012, 10/S1-5013, and 2/S1-5019 in regards to the following: 1) Please confirm the dimensions and weld prep noted are acceptable. [Reference CD RFI 080 SK1] 2) Confirm the increased thickness and placement of the continuity plate are acceptable to allow for beam over roll. [Reference CD RFI 080 SK1] 3) Please confirm the continuity plate dimensions noted on CD RFI 080 SK2 are acceptable. Note that the "a" dimension shown is defined as ½(bf-tw). 4) Please confirm the dimensions and weld prep indicated for the Type 4 Drag connection are acceptable. [Reference CD RFI 080 SK3]				
T-0887.1	SSS - Moment Beam to Column Web Connection Clarifications	Closed	CR	12/11/2013	12/21/2013	12/16/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST: After reviewing the response to item 2 on SK RFI 104 we believe a thickness increase should be allowed for the bottom continuity plate to allow for mill tolerance of rolled sections as per AISC Table 1-22(attached).  1) Due to mill tolerances the actual depth of a beam can over run in depth from -1/8" to +1/8" at the beam		ANSWER: After reviewing the response to item 2 on SK RFI 104 we believe a thickness increase should be allowed for the bottom continuity plate to allow for mill tolerance of rolled sections as per AISC Table 1-22(attached).  1) Due to mill tolerances the actual depth of a beam can over run in depth from -1/8" to +1/8" at the beam				





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	<p>centerline.</p> <p>2) Due to mill tolerances the axis of the flanges in relation to the beam web can have an out of square effect of as much as 5/16" from toe to toe of the beam flange.</p> <p>Increasing the continuity thickness provides a reasonable land for back up material for the fill penetration weld required in the field (see attached sketch SK1 for clarification)</p> <p>Please confirm it is acceptable to increase the bottom continuity plates by 1/4".</p>					
T-0888	SSS - Rebar Holes and Headed Stud Details	Closed	CR	11/07/2013	11/17/2013	11/14/2013
	From: Webcor Construction LP  Gregory Kemerer					
	REQUEST:					
	<p>Please reference detail 9/S1-3701 and the noted grid lines G.9.9 and G.10.1 on S1-2303 and provide clarification on the following items. Refer to CD RFI 105 SK1 through SK3 for additional information.</p> <p>1) Confirm the headed studs in the transfer girder per 11/S1-3701 may be located as shown.</p> <p>2) Confirm the slope of MFB 6 is 1.097° as indicated in CD RFI 105 SK2 or advise otherwise.</p> <p>3) Provide the vertical dimension indicated on CD RFI 105 SK2 to located PL 1 ½" x 14" x 2'-6" (added in ASI 106).</p> <p>4) Confirm it is acceptable to locate the first row of holes 6" above the underside of the transfer girder as indicated in CD RFI 105 SK2.</p> <p>5) Provide the vertical dimension required to locate the row of 3" dia. holes indicated in CD RFI 105 SK2.</p> <p>6) The hole indicated fouls the stiffener as shown in CD RFI 105 SK3. Confirm the spacing may be reduced to 5" at this location to clear the stiffeners and weld for the stiffeners to the beam web.</p> <p>7) The two holes indicated on CD RFI 105 SK3 are located directly adjacent to the stiffeners with no clearance. Please advise if this condition is acceptable or if the holes are to be shifted to avoid the stiffeners.</p> <p>8) Provide the vertical dimension required to locate the 3" dia. holes as indicated in CD RFI 105 SK2. Please verify the other holes in this row are to be located per the angle and spacing in items 2 &amp; 7.</p>					
	ANSWER:					
	<p>Please reference detail 9/S1-3701 and the noted grid lines G.9.9 and G.10.1 on S1-2303 and provide clarification on the following items. Refer to CD RFI 105 SK1 through SK3 for additional information.</p> <p>1) Confirm the headed studs in the transfer girder per 11/S1-3701 may be located as shown.</p> <p>2) Confirm the slope of MFB 6 is 1.097° as indicated in CD RFI 105 SK2 or advise otherwise.</p> <p>3) Provide the vertical dimension indicated on CD RFI 105 SK2 to located PL 1 ½" x 14" x 2'-6" (added in ASI 106).</p> <p>4) Confirm it is acceptable to locate the first row of holes 6" above the underside of the transfer girder as indicated in CD RFI 105 SK2.</p> <p>5) Provide the vertical dimension required to locate the row of 3" dia. holes indicated in CD RFI 105 SK2.</p> <p>6) The hole indicated fouls the stiffener as shown in CD RFI 105 SK3. Confirm the spacing may be reduced to 5" at this location to clear the stiffeners and weld for the stiffeners to the beam web.</p> <p>7) The two holes indicated on CD RFI 105 SK3 are located directly adjacent to the stiffeners with no clearance. Please advise if this condition is acceptable or if the holes are to be shifted to avoid the stiffeners.</p> <p>8) Provide the vertical dimension required to locate the 3" dia. holes as indicated in CD RFI 105 SK2. Please verify the other holes in this row are to be located per</p>					





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the angle and spacing in items 2 & 7.						
<b>T-0889</b>	<b>SSS - Rebar Hole Clarifications For TR11</b>	<b>Closed</b>	<b>CR</b>	<b>11/08/2013</b>	<b>11/18/2013</b>	<b>11/15/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference grid G.11 on S1-2303 and provide clarification on the following items per detail 8/S1-3702.			Please reference grid G.11 on S1-2303 and provide clarification on the following items per detail 8/S1-3702.			
1) Provide the vertical dimension required to locate PL 1 ½" x 14" x 2'-6" in alignment with the lenton couplers as indicated in CD RFI 107 SK 2.			1) Provide the vertical dimension required to locate PL 1 ½" x 14" x 2'-6" in alignment with the lenton couplers as indicated in CD RFI 107 SK 2.			
2) Provide the vertical dimension required to locate the hole indicated in CD RFI 107 SK2, which is shown to be 3" from the end of TR11. Please confirm the other holes in this row are to be located per the spacing shown and the angle confirmed in item 3.			2) Provide the vertical dimension required to locate the hole indicated in CD RFI 107 SK2, which is shown to be 3" from the end of TR11. Please confirm the other holes in this row are to be located per the spacing shown and the angle confirmed in item 3.			
3) Confirm the slope of MFB 5 is 1.057°as indicated in CD RFI 107 SK 2 or advise otherwise.			3) Confirm the slope of MFB 5 is 1.057°as indicated in CD RFI 107 SK 2 or advise otherwise.			
<b>T-0890</b>	<b>SSS - Rebar Hole Clarifications for Transfer Girders</b>	<b>Closed</b>	<b>CR</b>	<b>11/08/2013</b>	<b>11/18/2013</b>	<b>11/14/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference grid C.9 & C.11 at the ends of the transfer girders shown on S1-2303 and provide clarification on the following:			Please reference grid C.9 & C.11 at the ends of the transfer girders shown on S1-2303 and provide clarification on the following:			
1) Confirm the noted angle (1.23°) is the correct slope of MFB1 & MFB12 (per Revit Model). If not, provide the correct angle.			1) Confirm the noted angle (1.23°) is the correct slope of MFB1 & MFB12 (per Revit Model). If not, provide the correct angle.			
2) Provide the vertical dimension indicated on CD RFI 109 SK2 required to located the first hole and confirm the remaining holes are to be located per the angle noted in item 1 and the spacing indicated on detail 6/S1-3702.			2) Provide the vertical dimension indicated on CD RFI 109 SK2 required to located the first hole and confirm the remaining holes are to be located per the angle noted in item 1 and the spacing indicated on detail 6/S1-3702.			
3) Confirm the 3" dimension shown to locate the first hole is acceptable or provide an alternate dimension.			3) Confirm the 3" dimension shown to locate the first hole is acceptable or provide an alternate dimension.			





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<b>T-0892</b>	<b>level B bracing - Concourse Slab elevation conflicts gridline 1- 9</b>	<b>Closed</b>	<b>01</b>	<b>11/08/2013</b>	<b>11/18/2013</b>	
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Michael Spillane</div></div><div><b>REQUEST:</b><p>Further to email from the design team (Lee Ishida of Thornton Tomasetti) dated 09/03/13) "the design team wants to pursue with option on SK-2, provided the layout of the pin-pile columns has been coordinated with the moment frame beams so that the block-outs indicated in the sketch do not interfere with the moment frame beams" this option on SK-2 will be used where the strut support beams of the trestle and the internal bracing system are in conflict with the concourse slab, on the other conflicts around the perimeter CDSM wall where the lookout supporting the walers are in conflict with the waterproofing lap length requirements, the lookouts will be relocated above the walers to achieve the necessary lap requirements.</p><p>Please confirm if this is acceptable.</p></div><div><b>ANSWER:</b><p>Further to email from the design team (Lee Ishida of Thornton Tomasetti) dated 09/03/13) "the design team wants to pursue with option on SK-2, provided the layout of the pin-pile columns has been coordinated with the moment frame beams so that the block-outs indicated in the sketch do not interfere with the moment frame beams" this option on SK-2 will be used where the strut support beams of the trestle and the internal bracing system are in conflict with the concourse slab, on the other conflicts around the perimeter CDSM wall where the lookout supporting the walers are in conflict with the waterproofing lap length requirements, the lookouts will be relocated above the walers to achieve the necessary lap requirements.</p><p>Please confirm if this is acceptable.</p></div></div>						
<b>T-0892.1</b>	<b>BGP Level B bracing - Concourse Slab elevation conflict gridline 1-9</b>	<b>Closed</b>	<b>CR</b>	<b>11/13/2013</b>	<b>11/23/2013</b>	<b>12/04/2013</b>
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Michael Spillane</div></div><div><b>REQUEST:</b><p>The answer to RFI T-0892 does not answer the intended question, it was not a question on waterproofing requirements, the intended question was to confirm that the design team wish to proceed with the preferred option on sketch SK-2 i.e. to moving the conflicting Level B internal bracing elements to the revised location above the struts or walers whichever is applicable, if this is an acceptable solution, WOJV will proceed and engage the contractor and the Engineer of record for the bracing system to elaborate on this design and install these fixes in the field.</p><p>WOJV have already established the waterproofing lap length requirements in coming up with these fixes.</p><p>Please confirm that this is the preferred solution.</p></div><div><b>ANSWER:</b><p>The answer to RFI T-0892 does not answer the intended question, it was not a question on waterproofing requirements, the intended question was to confirm that the design team wish to proceed with the preferred option on sketch SK-2 i.e. to moving the conflicting Level B internal bracing elements to the revised location above the struts or walers whichever is applicable, if this is an acceptable solution, WOJV will proceed and engage the contractor and the Engineer of record for the bracing system to elaborate on this design and install these fixes in the field.</p><p>WOJV have already established the waterproofing lap length requirements in coming up with these fixes.</p><p>Please confirm that this is the preferred solution.</p></div></div>						
<b>T-0893</b>	<b>BGP - F15 Fixtures on Dimmeable or Non-Dimmeable Lighting Circuits</b>	<b>Closed</b>	<b>CR</b>	<b>11/11/2013</b>	<b>11/21/2013</b>	<b>11/13/2013</b>



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T-0894	From: Webcor Construction LP Jackson Tukuafu	Closed	CR	11/11/2013	11/21/2013	11/22/2013
	SSS - Double Angle Connection Conflicts Along GL					
T-0894	From: Webcor Construction LP Gregory Kemerer	Closed	CR	11/11/2013	11/21/2013	11/22/2013
	SSS - Double Angle Connection Conflicts Along GL					

REQUEST:

Please refer to drawing E-0006.

General Note N on DWG E-0006 states in part: "Allocate a maximum of three dimmable lighting branch circuits (multiwire) per conduit home run. Allocate a maximum of six non-dimmed lighting circuits per conduit home run." The type F15 fixtures used throughout the job on the train platform level are fed from, Panels designated "EDMH," which are dimming panels.

Are the circuits feeding these lights considered dimmable lighting branch circuits? Please advise.

ANSWER:

Please refer to drawing E-0006.

General Note N on DWG E-0006 states in part: "Allocate a maximum of three dimmable lighting branch circuits (multiwire) per conduit home run. Allocate a maximum of six non-dimmed lighting circuits per conduit home run." The type F15 fixtures used throughout the job on the train platform level are fed from, Panels designated "EDMH," which are dimming panels.

Are the circuits feeding these lights considered dimmable lighting branch circuits? Please advise.

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T-0897	<b>SSS - NE Coordinate Accuracy</b>  From: Webcor Construction LP  Gregory Kemerer	Closed	CR	11/12/2013	11/22/2013	11/19/2013
<b>REQUEST:</b>  Reference is made to drawing C-0100, "TTC Grid and Alignment Control." The northing and easting coordinates are provided with only two decimal places, producing a considerable amount of calculated error between the coordinates and the gridline dimensions. Please provide the N/E coordinates with at least four decimal places to reduce the calculated error from the gridline dimensions.  Additionally, the N/E coordinates provided at Grid 2/W appear to intersect with Grid 2/G. Please advise if these grid lines intersect and if the N/E coordinates provided also apply to 2/G.		<b>ANSWER:</b>  Reference is made to drawing C-0100, "TTC Grid and Alignment Control." The northing and easting coordinates are provided with only two decimal places, producing a considerable amount of calculated error between the coordinates and the gridline dimensions. Please provide the N/E coordinates with at least four decimal places to reduce the calculated error from the gridline dimensions.  Additionally, the N/E coordinates provided at Grid 2/W appear to intersect with Grid 2/G. Please advise if these grid lines intersect and if the N/E coordinates provided also apply to 2/G.				



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<b>T-0898</b>	<b>SSS - Weld Access Hole and Weld Tab Sizes at CJP</b>	<b>Closed</b>	<b>CR</b>	<b>11/12/2013</b>	<b>11/22/2013</b>	<b>11/22/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference detail 4/S1-4205 indicating the EBF Link Beam cross section, also detailed in OIW sketch 2770-SKTH01 attached.			Please reference detail 4/S1-4205 indicating the EBF Link Beam cross section, also detailed in OIW sketch 2770-SKTH01 attached.			
1) The specified 1" x 5" weld access hole does not allow for weld runoff tabs to be added as specified in AWS D1.8 paragraph 6.11.1.			1) The specified 1" x 5" weld access hole does not allow for weld runoff tabs to be added as specified in AWS D1.8 paragraph 6.11.1.			
a. Please confirm that the 1.5" x 5' weld access holes detailed in OIW SK 2770-SK-TH01 are acceptable to accommodate the 1" weld tabs.			a. Please confirm that the 1.5" x 5' weld access holes detailed in OIW SK 2770-SK-TH01 are acceptable to accommodate the 1" weld tabs.			
b. Please confirm that the weld tabs are to remain after welding as allowed by AWS D1.8 paragraph 6.11.			b. Please confirm that the weld tabs are to remain after welding as allowed by AWS D1.8 paragraph 6.11.			
2) The specified CJP weld using a backing fillet and welded substantially from one side increases weld distortion compared to a balanced weld. Please confirm that the proposed double bevel CJP weld is acceptable.			2) The specified CJP weld using a backing fillet and welded substantially from one side increases weld distortion compared to a balanced weld. Please confirm that the proposed double bevel CJP weld is acceptable.			
<b>T-0898.1</b>	<b>SSS - Weld Access Hole and Weld Tab Sizes at CJP</b>	<b>Closed</b>	<b>CR</b>	<b>12/06/2013</b>	<b>12/16/2013</b>	<b>12/20/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per the response to Webcor/Obayashi RFI T-0898 (SK 146), weld tabs are not required for stiffeners at EBF girders and it is acceptable for weld ends to be cascaded down as down in Figure C-6.3 of AWS D1.8.			Per the response to Webcor/Obayashi RFI T-0898 (SK 146), weld tabs are not required for stiffeners at EBF girders and it is acceptable for weld ends to be cascaded down as down in Figure C-6.3 of AWS D1.8.			
In accordance with this response, please reference SK-TH01 attached and confirm that the "extent of CJP weld/UT testing" and "cascaded weld area" detailed are acceptable.			In accordance with this response, please reference SK-TH01 attached and confirm that the "extent of CJP weld/UT testing" and "cascaded weld area" detailed are acceptable.			
<b>T-0899</b>	<b>BGP - Electrical Room Dimensions in RFI 778.1,780.1,781.1 &amp; 782.1</b>	<b>Closed</b>	<b>CR</b>	<b>11/12/2013</b>	<b>11/19/2013</b>	<b>11/15/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Jackson Tukuafu</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The AAI mark ups included in the responses to RFI 778.1, RFI 780.1, RFI 781.1 and RFI 782.1 do not reflect dimensions in the latest ASI 107 documents or submittal			The AAI mark ups included in the responses to RFI 778.1, RFI 780.1, RFI 781.1 and RFI 782.1 do not reflect dimensions in the latest ASI 107 documents or			



Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
	<p>review comments in the Comprehensive Mat Slab Drawings in submittal drawings package TG0600-0103.</p> <p>For example, the face of wall of Electrical Room B2460 per Response to RFI 780.1 is shown as 4'-0" from GL 15, however the latest drawing issued in ASI 107 A1-2124 shows the face of wall to this room as 3'-7 5/8" from GL 15. Shimmick has poured this area(Area 8) per ASI 104 which shows this dimension to be 3'-8". The next area to be impacted by these discrepancies will be placed on 11/24/2013.</p> <p>This discrepancy is present in all of the dimensions issued in the mark-ups included in the RFI responses (attached) and the rooms shown in RFI 781.1 and 782.1 are scheduled to pour on 11/24/13.</p> <p>Please provide a conformed drawing that shows the current layout for the following Electrical Rooms: B2640, B2461, B2441, B2560.</p>					<p>submittal review comments in the Comprehensive Mat Slab Drawings in submittal drawings package TG0600-0103.</p> <p>For example, the face of wall of Electrical Room B2460 per Response to RFI 780.1 is shown as 4'-0" from GL 15, however the latest drawing issued in ASI 107 A1-2124 shows the face of wall to this room as 3'-7 5/8" from GL 15. Shimmick has poured this area(Area 8) per ASI 104 which shows this dimension to be 3'-8". The next area to be impacted by these discrepancies will be placed on 11/24/2013.</p> <p>This discrepancy is present in all of the dimensions issued in the mark-ups included in the RFI responses (attached) and the rooms shown in RFI 781.1 and 782.1 are scheduled to pour on 11/24/13.</p> <p>Please provide a conformed drawing that shows the current layout for the following Electrical Rooms: B2640, B2461, B2441, B2560.</p>
T-0900	<p><b>SSS - Weld Test Requirements for Castings</b></p> <p><b>From:</b> Webcor Construction LP Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>The cast node material is not a prequalified base material, thus a PQR test for all welds to the cast material is required. The cast node manufacturer, Bradken, has indicated that all test materials will be supplied in flat plate form only. While AWS D1.1 Table 4.1, Note b qualifies that pipe diameters greater than or equal to 24" may be tested on flat plate, AWS D1.1 Table 4.1 requires that all pipes under 24" must be tested in tubular form. Please confirm it is acceptable to perform all PQR testing for castings less than 24" in diameter, including the 16" diameter castings at the Light Column, on flat cast plate material.</p>	Closed	CR	11/13/2013	11/23/2013	11/21/2013
						<p><b>ANSWER:</b></p> <p>The cast node material is not a prequalified base material, thus a PQR test for all welds to the cast material is required. The cast node manufacturer, Bradken, has indicated that all test materials will be supplied in flat plate form only. While AWS D1.1 Table 4.1, Note b qualifies that pipe diameters greater than or equal to 24" may be tested on flat plate, AWS D1.1 Table 4.1 requires that all pipes under 24" must be tested in tubular form. Please confirm it is acceptable to perform all PQR testing for castings less than 24" in diameter, including the 16" diameter castings at the Light Column, on flat cast plate material.</p>
T-0901	<p><b>SSS - Edge of Slab Support Clarifications</b></p> <p><b>From:</b> Webcor Construction LP Gregory Kemerer</p>	Closed	CR	11/13/2013	11/23/2013	11/26/2013







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	<p><b>REQUEST:</b></p> <p>During level D bracing removal at area 7,a column rebar dowel was bent, as shown in the attached photograph.</p> <p>Please see below repair options:</p> <ol style="list-style-type: none"> <li>1. Abandon the bent rebar leave it in its current position projecting 5' above the mat slab, place and additional equal size bar alongside the damaged bar as a replacement with possibly a 90 degrees hook at the base</li> <li>2. Leave the dowel as is, couple the upper section of the bar onto it and bring it back in line as the bar continues vertically</li> <li>3. Drill and epoxy in a new same sized bar beside the damaged one, the slab would have to be scanned for rebar location and new location pick to avoid damaging the existing reinforcement.</li> <li>4. Concrete around the rebar to be removed the bar would then be cut and a new bar welded to it.</li> <li>5. Concrete around the rebar to be removed .The bar would be cut and a barlock would be used to couple the rebar this could be difficult to achieve due to congestion with the top mat reinforcement and the depth required for the bar lock to be fitted</li> </ol> <p>Please advise on which option is acceptable.</p>					
	<p><b>ANSWER:</b></p> <p>During level D bracing removal at area 7,a column rebar dowel was bent, as shown in the attached photograph.</p> <p>Please see below repair options:</p> <ol style="list-style-type: none"> <li>1. Abandon the bent rebar leave it in its current position projecting 5' above the mat slab, place and additional equal size bar alongside the damaged bar as a replacement with possibly a 90 degrees hook at the base</li> <li>2. Leave the dowel as is, couple the upper section of the bar onto it and bring it back in line as the bar continues vertically</li> <li>3. Drill and epoxy in a new same sized bar beside the damaged one, the slab would have to be scanned for rebar location and new location pick to avoid damaging the existing reinforcement.</li> <li>4. Concrete around the rebar to be removed the bar would then be cut and a new bar welded to it.</li> <li>5. Concrete around the rebar to be removed .The bar would be cut and a barlock would be used to couple the rebar this could be difficult to achieve due to congestion with the top mat reinforcement and the depth required for the bar lock to be fitted</li> </ol> <p>Please advise on which option is acceptable.</p>					
T-0903	SSS - Location of Roof Beams for W-1 Glazing	Closed	CR	11/14/2013	11/24/2013	11/19/2013
	From: Webcor Construction LP Gregory Kemerer					
	<p><b>REQUEST:</b></p> <p>1) The W-1 glazing system wireframe transmitted by Webcor/Obayashi locates the beams that back up the W-1 glazing system supports. At the roof level on S1-2602 to S1-2607, refer to sketches CD RFI 133 SK1 to SK6 and verify the clouded dimensions in red which locate the beams in question based on the structural wireframe model transmitted.</p>					
	<p><b>ANSWER:</b></p> <p>1) The W-1 glazing system wireframe transmitted by Webcor/Obayashi locates the beams that back up the W-1 glazing system supports. At the roof level on S1-2602 to S1-2607, refer to sketches CD RFI 133 SK1 to SK6 and verify the clouded dimensions in red which locate the beams in question based on the structural wireframe model transmitted.</p>					



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Job: 30100

<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
T-0903.1	<b>SSS - Location of Roof Beams for W-1 Glazing</b>  <b>From:</b> Webcor Construction LP  <b>REQUEST:</b>  Per the discussion at the Structural RFI Meeting 12/5/13, please provide a revised response to the following RFI:  1) The W-1 glazing system wireframe transmitted by Webcor/Obayashi locates the beams that back up the W-1 glazing system supports. At the roof level on S1-2602 to S1-2607, refer to sketches CD RFI 133 SK1 to SK6 and verify the clouded dimensions in red which locate the beams in question based on the structural wireframe model transmitted.  2) Once the beam locations in question are confirmed, it is requested that any revisions that impact the location of any beam be addressed in a written or marked up formation in lieu of a revised wireframe model. Please confirm this is acceptable.	Closed	CR	12/06/2013	12/16/2013	12/09/2013
	<b>ANSWER:</b>  Per the discussion at the Structural RFI Meeting 12/5/13, please provide a revised response to the following RFI:  1) The W-1 glazing system wireframe transmitted by Webcor/Obayashi locates the beams that back up the W-1 glazing system supports. At the roof level on S1-2602 to S1-2607, refer to sketches CD RFI 133 SK1 to SK6 and verify the clouded dimensions in red which locate the beams in question based on the structural wireframe model transmitted.  2) Once the beam locations in question are confirmed, it is requested that any revisions that impact the location of any beam be addressed in a written or marked up formation in lieu of a revised wireframe model. Please confirm this is acceptable.					



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<b>T-0904</b>	<b>SSS - W-1 Glazing Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>11/14/2013</b>	<b>11/24/2013</b>	<b>12/04/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference is made to the W-1 glazing support connection details indicated on 1 &4/S1-8001 and CD RFI 136 SK1 to SK3 in regards to the following: 1) Confirm the hole locations for the W-1 glazing "CP1" locations are acceptable as shown or supply alternate locations. 2) Confirm the holes for W-1 glazing connections are to be 1 9/16" dia. 3) Confirm the hole locations for W-1 glazing "CP2" locations are acceptable as shown or supply alternate locations. 4) Confirm the holes for the W-1 glazing connections are to be 1 9/16" dia. as indicated. 5) The 1" plate located between the beam web and the 2 ½" plate has been detailed to terminate 5" below and above the beam flanges as indicated in 7/S1-8001. This places the edge of the plate near the center of the W1 "CP2" connection bolts as shown on CD RFI 136 SK3. Please confirm this is the intent for the 1" plate at this location.			Reference is made to the W-1 glazing support connection details indicated on 1 &4/S1-8001 and CD RFI 136 SK1 to SK3 in regards to the following: 1) Confirm the hole locations for the W-1 glazing "CP1" locations are acceptable as shown or supply alternate locations. 2) Confirm the holes for W-1 glazing connections are to be 1 9/16" dia. 3) Confirm the hole locations for W-1 glazing "CP2" locations are acceptable as shown or supply alternate locations. 4) Confirm the holes for the W-1 glazing connections are to be 1 9/16" dia. as indicated. 5) The 1" plate located between the beam web and the 2 ½" plate has been detailed to terminate 5" below and above the beam flanges as indicated in 7/S1-8001. This places the edge of the plate near the center of the W1 "CP2" connection bolts as shown on CD RFI 136 SK3. Please confirm this is the intent for the 1" plate at this location.			



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<b>T-0904.1</b>	<b>SSS - W-1 Glazing Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>12/12/2013</b>	<b>12/22/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  As a follow-up to Webcor/Obayashi RFI T-0904 (SK RFI 182), please see attached CD RFI 136.1 SK1 and SK2 in reference to the following:  1) T-0904 Item 5: The response references the stiffener in detail 1A/S1-8001, while the question is regarding the 1" plate wedged between the BU-Beam web and the 2 ½" thick plate per detail 7/S1-8001. The top and bottom edges of the 1" plate are close to the bolts as shown on SK2. If this is the intent, confirm items 1a and 1b on SK2: a. Confirm the 1" edge distance is sufficient. b. Confirm it is acceptable to notch the 1" plate with partial 1 9/16 dia. holes at 4 locations to accommodate the bolts.  2) T-0904 Item 2: The response does not clarify the requested hole diameters. Please confirm the holes are 1 9/16" diameter for "CP2" connections.						
						<b>ANSWER:</b>  As a follow-up to Webcor/Obayashi RFI T-0904 (SK RFI 182), please see attached CD RFI 136.1 SK1 and SK2 in reference to the following:  1) T-0904 Item 5: The response references the stiffener in detail 1A/S1-8001, while the question is regarding the 1" plate wedged between the BU-Beam web and the 2 ½" thick plate per detail 7/S1-8001. The top and bottom edges of the 1" plate are close to the bolts as shown on SK2. If this is the intent, confirm items 1a and 1b on SK2: a. Confirm the 1" edge distance is sufficient. b. Confirm it is acceptable to notch the 1" plate with partial 1 9/16 dia. holes at 4 locations to accommodate the bolts.  2) T-0904 Item 2: The response does not clarify the requested hole diameters. Please confirm the holes are 1 9/16" diameter for "CP2" connections.
<b>T-0905</b>	<b>BGP - Light Column Anchor Bolts Conflict with Rebar</b>	<b>Closed</b>	<b>CR</b>	<b>11/15/2013</b>	<b>11/25/2013</b>	<b>11/27/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached drawing S1-3009, S1-6008.  SCCI encountered potential conflict between anchor bolts of light column (layout depicted in 2/S1-6008) with light column rebar as shown in 1/S 1-3009. Please find attached model depicting conflict between bundles of 2 ea rebar #11@16" OC with the layout of the anchor bolt.  Please advise and provide parameters with which the rebar may be moved to clear the anchor bolts.						
						<b>ANSWER:</b>  Please refer to attached drawing S1-3009, S1-6008.  SCCI encountered potential conflict between anchor bolts of light column (layout depicted in 2/S1-6008) with light column rebar as shown in 1/S 1-3009. Please find attached model depicting conflict between bundles of 2 ea rebar #11@16" OC with the layout of the anchor bolt.  Please advise and provide parameters with which the rebar may be moved to clear the anchor bolts.
<b>T-0906</b>	<b>BGP - Omitting the Grout Port at all Applicable Column Base Plates</b>	<b>Closed</b>	<b>CR</b>	<b>11/15/2013</b>	<b>11/25/2013</b>	<b>12/12/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please reference column base plate details on Sheet S1-						
						<b>ANSWER:</b>  Please reference column base plate details on Sheet





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<b>T-0908</b>	<b>BGP - Column Base Plate Shear Key Block-out Dimension</b>	<b>Closed</b>	<b>CR</b>	<b>11/15/2013</b>	<b>11/25/2013</b>	<b>11/20/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please refe to attached detail A on sheet S1-5051.		Please refe to attached detail A on sheet S1-5051.				
Please confirm it is acceptable to reduce the overall 14" shear key block-out dimension to 10"; therefore, allowing for 2-inches of clearance all around the shear key as discused and coordinated during the 11/12/2013 mock-up review. See attached detail A/S1-5051 for mark-ups.		Please confirm it is acceptable to reduce the overall 14" shear key block-out dimension to 10"; therefore, allowing for 2-inches of clearance all around the shear key as discused and coordinated during the 11/12/2013 mock-up review. See attached detail A/S1-5051 for mark-ups.				
Please note the revised column base plate block-out is typical for Type I and II.		Please note the revised column base plate block-out is typical for Type I and II.				
<b>T-0908.1</b>	<b>BGP - Concrete Beam Top Bar Spacing and Layering</b>	<b>Closed</b>	<b>CR</b>	<b>11/22/2013</b>	<b>12/02/2013</b>	<b>12/04/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please refer to drawing S1-3400 and RFI T-0908.		Please refer to drawing S1-3400 and RFI T-0908.				
In order to clear the 10" shear key block-out as approved in RFI T-908, please confirm it is acceptable to place the additional short bars in a typical concrete beam in a second layer. Also, please confirm it is acceptable to increase the space between the top and short bars near the center of the beam to 10".		In order to clear the 10" shear key block-out as approved in RFI T-908, please confirm it is acceptable to place the additional short bars in a typical concrete beam in a second layer. Also, please confirm it is acceptable to increase the space between the top and short bars near the center of the beam to 10".				
Please reference the attached photo for more details.		Please reference the attached photo for more details.				
<b>T-0909</b>	<b>BGP - Cast-In Place Plumbing Fixtures on Concourse Level</b>	<b>Closed</b>	<b>CR</b>	<b>11/15/2013</b>	<b>11/25/2013</b>	<b>11/25/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
As discussed in the 10/28/2013 ASI 104 Concourse Plumbing design meeting, this RFI is requesting confirmation that it is acceptable for the Early Below Grade Package (TG06) contractor to block out the concourse slab where plumbing fixtures are shown to be embedded in concrete.		As discussed in the 10/28/2013 ASI 104 Concourse Plumbing design meeting, this RFI is requesting confirmation that it is acceptable for the Early Below Grade Package (TG06) contractor to block out the concourse slab where plumbing fixtures are shown to be embedded in concrete.				
General notes in TG06 drawing P-0005 call for sleeves only in elevated slabs in the EBGp. However, for the future main package plumber to be able to install the cast		General notes in TG06 drawing P-0005 call for sleeves only in elevated slabs in the EBGp. However, for the future main package plumber to be able to				





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	<p>in place floor sink and floor drain fixtures, larger openings and structural details are needed. The contractor is proposing to install square blockouts sized larger than these fixtures so that they can be installed and grouted in a later date by the main package plumber. The desired benefit of this proposed logic is that concourse plumbing will be installed by one trade contractor who will provide a single source warranty for the work. Also, the later installation allows for more precise coordination of fixture rim elevations.</p> <p>If this proposed sequence is not acceptable, CIP plumbing fixtures will need to be supplied and installed by the BPG (TG06) contractor. If this proposal is acceptable, please provide blockout size, rebar trim details and rebar doweling details for floor sinks and also floor drains. Sample product data for the fixtures are attached for reference and for sizing of openings.</p>					<p>install the cast in place floor sink and floor drain fixtures, larger openings and structural details are needed. The contractor is proposing to install square blockouts sized larger than these fixtures so that they can be installed and grouted in a later date by the main package plumber. The desired benefit of this proposed logic is that concourse plumbing will be installed by one trade contractor who will provide a single source warranty for the work. Also, the later installation allows for more precise coordination of fixture rim elevations.</p> <p>If this proposed sequence is not acceptable, CIP plumbing fixtures will need to be supplied and installed by the BPG (TG06) contractor. If this proposal is acceptable, please provide blockout size, rebar trim details and rebar doweling details for floor sinks and also floor drains. Sample product data for the fixtures are attached for reference and for sizing of openings.</p>
<b>T-0909.1</b>	<b>BGP - Cast-In Place Plumbing Fixtures on Concourse Level</b>	<b>Closed</b>	<b>CR</b>	<b>12/11/2013</b>	<b>12/21/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference response to RFI 909.						Reference response to RFI 909.
For floor sinks (FSK) shown cast into Lower Concourse structural slab CM/GC proposes to block out 18"x18" square centered on center of fixture. Propose using detail 1/S-3501 for trimming rebars through this blockout (TG06 contractor). Fixture to be placed and grouted back in as part of main package (TG-10.2) plumbing scope installation. Doweling and pourback details to be designed by TG-10.2 plumbing trade contractor.						For floor sinks (FSK) shown cast into Lower Concourse structural slab CM/GC proposes to block out 18"x18" square centered on center of fixture. Propose using detail 1/S-3501 for trimming rebars through this blockout (TG06 contractor). Fixture to be placed and grouted back in as part of main package (TG-10.2) plumbing scope installation. Doweling and pourback details to be designed by TG-10.2 plumbing trade contractor.
For floor drains (FD) shown cast into Lower Concourse structural slab CM/GC proposes to block out 12"x12" square centered on center of fixture. Propose using detail 1/S-3501 for trimming rebars should they encroach into the blockout. Fixture to be placed and grouted back in as part of main package (TG-10.2) plumbing scope installation. Doweling and pourback details to be designed by TG-10.2 plumbing trade contractor.						For floor drains (FD) shown cast into Lower Concourse structural slab CM/GC proposes to block out 12"x12" square centered on center of fixture. Propose using detail 1/S-3501 for trimming rebars should they encroach into the blockout. Fixture to be placed and grouted back in as part of main package (TG-10.2) plumbing scope installation. Doweling and





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	<p>Please confirm the above proposed scope is structurally acceptable.</p>				<p>pourback details to be designed by TG-10.2 plumbing trade contractor.</p>	
					<p>Please confirm the above proposed scope is structurally acceptable.</p>	
T-0910	BGP - Mechanical Couplers at Top of Partition Walls	Closed	CR	11/18/2013	11/28/2013	12/02/2013
	<p><b>From:</b> Webcor Construction LP                      Jackson Tukuafu</p>					
	<p><b>REQUEST:</b></p> <p>Please refer to attached drawing excerpts from sheet S1-2052 and 4/S1-3205.</p> <p>The typical wall section shown on S1/-2025 for the tank walls directs the reader to section 4 on S1-3205. When reviewing this section the design calls for mechanical couplers at the tops of the walls per detail 6/S1-3001. The formsaver coupler depicted within this detail is a threaded product that will not support a hooked or bent bar because the specific orientation of the hook is not possible.</p> <p>Please provide direction on how to proceed.</p>				<p><b>ANSWER:</b></p> <p>Please refer to attached drawing excerpts from sheet S1-2052 and 4/S1-3205.</p> <p>The typical wall section shown on S1/-2025 for the tank walls directs the reader to section 4 on S1-3205. When reviewing this section the design calls for mechanical couplers at the tops of the walls per detail 6/S1-3001. The formsaver coupler depicted within this detail is a threaded product that will not support a hooked or bent bar because the specific orientation of the hook is not possible.</p> <p>Please provide direction on how to proceed.</p>	



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<b>T-0911</b>	<b>BGP - Seismic Joint Specification Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>11/18/2013</b>	<b>11/24/2013</b>	<b>11/25/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please reference Specification Section 07 09 16 - 2.6.A.1.		Please reference Specification Section 07 09 16 - 2.6.A.1.				
The aforementioned section states, "Provide joint assemblies in single lengths between changes in direction with vulcanized, mitered comers where joint changes directions or abuts other materials."		The aforementioned section states, "Provide joint assemblies in single lengths between changes in direction with vulcanized, mitered comers where joint changes directions or abuts other materials."				
1. Please confirm that this is in reference to the Omega Seal gasket, and not the clamping system and embedded steel.		1. Please confirm that this is in reference to the Omega Seal gasket, and not the clamping system and embedded steel.				
2. Please confirm that it is acceptable to use clamping components with 4'-0" maximum lengths with butt joints not to exceed 1/8".		2. Please confirm that it is acceptable to use clamping components with 4'-0" maximum lengths with butt joints not to exceed 1/8".				
3. Please confirm that it is acceptable to use 14' max lengths on steel embed with butt joints not to exceed 1/8".		3. Please confirm that it is acceptable to use 14' max lengths on steel embed with butt joints not to exceed 1/8".				
<b>T-0912</b>	<b>SSS - GFRC Drawings</b>	<b>Closed</b>	<b>CR</b>	<b>11/18/2013</b>	<b>11/28/2013</b>	<b>12/04/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>		<b>ANSWER:</b>				
On the Type 2 (M) Drag connection per detail 1/S1-5017 refer to sketch CD RFI 117 SK1 for the GFRC question below. Note 4 references GFRC drawings. The connections in the clouded areas cannot be completed until the GFRC information is issued. Please supply the necessary information.		On the Type 2 (M) Drag connection per detail 1/S1-5017 refer to sketch CD RFI 117 SK1 for the GFRC question below. Note 4 references GFRC drawings. The connections in the clouded areas cannot be completed until the GFRC information is issued. Please supply the necessary information.				
<b>T-0913</b>	<b>BGP - Seismic Joint Detail Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>11/18/2013</b>	<b>11/24/2013</b>	<b>11/25/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please reference details 7/A1-8881 (ASI #107) and 4/S1-3010 (ASI #100).		Please reference details 7/A1-8881 (ASI #107) and 4/S1-3010 (ASI #100).				
1. Detail 7/A1-8881 calls for a "neoprene gasket compressed by bar and bolt typ." Please provide sizes for tabs and bolts. Also, provide welding instructions (if necessary).		1. Detail 7/A1-8881 calls for a "neoprene gasket compressed by bar and bolt typ." Please provide sizes for tabs and bolts. Also, provide welding instructions (if necessary).				
2. The same detail shows pipe penetrations through the		2. The same detail shows pipe penetrations through				





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additional information on the indicated members with horizontal and vertical leg dimensions.						
T-0915	<b>SSS - Connection Clarifications For Beam Cope</b>	Closed	CR	11/18/2013	11/28/2013	12/04/2013
From: Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At sample locations on S1-2303 along line 9 between grids D & F and refer to sketches CD RFI 118 SK1 to SK3 for items 1, 2 & 3 for beam cope clearance.			At sample locations on S1-2303 along line 9 between grids D & F and refer to sketches CD RFI 118 SK1 to SK3 for items 1, 2 & 3 for beam cope clearance.			
1) The 1/2" max clearance per 1/S1-5010 is not sufficient to clear the k of the W40x183. Confirm it is acceptable to increase the clearance to 1 11/16" to avoid coping the beam inside the k.			1) The 1/2" max clearance per 1/S1-5010 is not sufficient to clear the k of the W40x183. Confirm it is acceptable to increase the clearance to 1 11/16" to avoid coping the beam inside the k.			
2) The 1/2" max clearance per 12/S1-5010 is not sufficient to clear the k of the W24x68. Confirm it is acceptable to increase the clearance to 15/16" to avoid coping the beam inside the k.			2) The 1/2" max clearance per 12/S1-5010 is not sufficient to clear the k of the W24x68. Confirm it is acceptable to increase the clearance to 15/16" to avoid coping the beam inside the k.			
3) Confirm it is typically acceptable to increase the 1/2" max clearance at other similar connections on this project to avoid cutting the beams inside the k.			3) Confirm it is typically acceptable to increase the 1/2" max clearance at other similar connections on this project to avoid cutting the beams inside the k.			





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T-0919	<b>SSS - Beam Bottom Flange Bracing Connection</b>  From: Webcor Construction LP Gregory Kemerer	Closed	CR	11/18/2013	11/28/2013	12/04/2013
<b>REQUEST:</b> Refer to the beam bottom flange bracing connection detailed on 8/S1-5015 and CD RFI 127 SK1 & SK2 for the following items: 1) In order to support erection requirements, please confirm it is acceptable to: a. Typically locate the bolts shown 3" from the underside of the top flange and 3" from the face of the beam web as indicated in CD RFI 127 SK2. b. Typically locate the bolt 3" from the top of the flange indicated. c. Typically locate the bolt outside the beam profile as shown to make the brace erectable. 2) Confirm the stitch plates should be ½" thick to match the ½" thick gusset plates at each end. 3) Please confirm that it is acceptable to provide slotted holes in the brace at the end connections.		<b>ANSWER:</b> Refer to the beam bottom flange bracing connection detailed on 8/S1-5015 and CD RFI 127 SK1 & SK2 for the following items: 1) In order to support erection requirements, please confirm it is acceptable to: a. Typically locate the bolts shown 3" from the underside of the top flange and 3" from the face of the beam web as indicated in CD RFI 127 SK2. b. Typically locate the bolt 3" from the top of the flange indicated. c. Typically locate the bolt outside the beam profile as shown to make the brace erectable. 2) Confirm the stitch plates should be ½" thick to match the ½" thick gusset plates at each end. 3) Please confirm that it is acceptable to provide slotted holes in the brace at the end connections.				



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<b>T-0919.1</b>	<b>SSS - Detail Clarification at Angle Brace</b>	<b>Closed</b>	<b>CR</b>	<b>12/31/2013</b>	<b>01/10/2014</b>	<b>01/13/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  The braces per detail 8/S1-5015 have been added in the model for the area between grids 1.4 to 19.9 as shown on attached SK2. Please see attached CD RFI 127.1 SK1 & SK2 for items 1 & 2:  1.) Work with SK1 & SK2 and confirm the request in the response to RFI T-0919 (SK 173 & CD 127) item 1 to have the work points for the braces located at the intersection of the top/bottom of beams on center of beams.  2.) If the response to item 1 above is yes, please supply the size of the gusset plates as the dimensioning proposed in CD RFI 127 will not work with the revised work point locations.		<b>ANSWER:</b>  The braces per detail 8/S1-5015 have been added in the model for the area between grids 1.4 to 19.9 as shown on attached SK2. Please see attached CD RFI 127.1 SK1 & SK2 for items 1 & 2:  1.) Work with SK1 & SK2 and confirm the request in the response to RFI T-0919 (SK 173 & CD 127) item 1 to have the work points for the braces located at the intersection of the top/bottom of beams on center of beams.  2.) If the response to item 1 above is yes, please supply the size of the gusset plates as the dimensioning proposed in CD RFI 127 will not work with the revised work point locations.				
<b>T-0919.2</b>	<b>SSS - Detail Clarification at Angle Brace</b>	<b>Closed</b>	<b>CR</b>	<b>02/06/2014</b>	<b>02/16/2014</b>	<b>02/14/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  This is a follow-up to Webcor RFI # T-0919.1 (SK RFI # 173.1 & CD RFI # 127.1)  Per the conference call discussion on 1/16/14 with Webcor, Skanska & Thornton Tomasetti, Candraft was to layout the bracing work points to the underside of the top beam flange superseding the response to locate the work at the top of the beam. Please review the attached two sketches CD RFI 127.2 SK1 & SK2 showing the revised work point locations and confirm this is the intent of the design parameters. Note that the bracing work points are not indicated on 8/S1-5015 and we feel that the original sketch CD RFI 127 SK2 (Relabeled CD RFI 127.2 SK3) conforms to the design with the bolt closer to the inside profile of the beam members and would like to use as modeled.		<b>ANSWER:</b>  This is a follow-up to Webcor RFI # T-0919.1 (SK RFI # 173.1 & CD RFI # 127.1)  Per the conference call discussion on 1/16/14 with Webcor, Skanska & Thornton Tomasetti, Candraft was to layout the bracing work points to the underside of the top beam flange superseding the response to locate the work at the top of the beam. Please review the attached two sketches CD RFI 127.2 SK1 & SK2 showing the revised work point locations and confirm this is the intent of the design parameters. Note that the bracing work points are not indicated on 8/S1-5015 and we feel that the original sketch CD RFI 127 SK2 (Relabeled CD RFI 127.2 SK3) conforms to the design with the bolt closer to the inside profile of the beam members and would like to use as modeled.				
<b>T-0920</b>	<b>SSS - Kicker Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>11/18/2013</b>	<b>11/28/2013</b>	<b>11/22/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>		<b>ANSWER:</b>				



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	Refer to sketches CD RFI 126 SK1 to SK3 for items 1 to 4:  1) Confirm it is acceptable to locate the brace at the bevel shown to fit the steel framing in lieu of the 2:1 bevel per 5/S1-5015. 2) Confirm it is acceptable to locate the brace at the bevel shown to fit the steel framing in lieu of the 2:1 bevel per 5/S1-5015. 3) Confirm it is acceptable to increase the thickness of the full depth shear plate to 1" per 5/S1-5015 and connect the kicker brace to the full depth shear plate as shown. 4) Confirm that it is acceptable to typically apply item 3 at other similar conditions.					Refer to sketches CD RFI 126 SK1 to SK3 for items 1 to 4:  1) Confirm it is acceptable to locate the brace at the bevel shown to fit the steel framing in lieu of the 2:1 bevel per 5/S1-5015. 2) Confirm it is acceptable to locate the brace at the bevel shown to fit the steel framing in lieu of the 2:1 bevel per 5/S1-5015. 3) Confirm it is acceptable to increase the thickness of the full depth shear plate to 1" per 5/S1-5015 and connect the kicker brace to the full depth shear plate as shown. 4) Confirm that it is acceptable to typically apply item 3 at other similar conditions.
<b>T-0920.1</b>	<b>SSS - Kicker Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>12/11/2013</b>	<b>12/21/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Per the response to Webcor RFI T-0920 (SK RFI # 172) it was confirmed acceptable to increase the full depth shear plate from 3/8" to 1". Upon further review of this location there would be an issue where a 1" thick shear plate would foul the bolts connecting the top flange of the Transfer Girder to the Cruciform column base plate.  1). Please confirm it is acceptable to center the 3/8" shear between the bolts and move the beam 5/8" south from its original location.  2). Please confirm the 1" gusset plate welded to the bottom flange of the beam can be moved off the center line of the beam to line up with the 3/8" full depth shear plate. The gusset would move 7/16" from the centerline at this location.  3). If it is acceptable to line up the gusset with the full depth shear plate, there will be a 5/8" discrepancy between the two plates. Please confirm if stitch plates with varying thicknesses can be used to make up the difference.  4). Confirm that it is acceptable to typically apply the						<b>ANSWER:</b>  Per the response to Webcor RFI T-0920 (SK RFI # 172) it was confirmed acceptable to increase the full depth shear plate from 3/8" to 1". Upon further review of this location there would be an issue where a 1" thick shear plate would foul the bolts connecting the top flange of the Transfer Girder to the Cruciform column base plate.  1). Please confirm it is acceptable to center the 3/8" shear between the bolts and move the beam 5/8" south from its original location.  2). Please confirm the 1" gusset plate welded to the bottom flange of the beam can be moved off the center line of the beam to line up with the 3/8" full depth shear plate. The gusset would move 7/16" from the centerline at this location.  3). If it is acceptable to line up the gusset with the full depth shear plate, there will be a 5/8" discrepancy between the two plates. Please confirm if stitch plates with varying thicknesses can be used to make up the difference.





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	above items at other similar conditions.					
	5). If any of the above suggestions will not work, please provide an alternate detail for these conditions.					
<b>T-0920.2</b>	<b>SSS - Kicker Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>12/16/2013</b>	<b>12/26/2013</b>	<b>01/02/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>						
Per the response to Webcor RFI T-0920 (SK RFI # 172) clarification is required regarding the last statement where "Coping the bottom flange of the shallow beam is not allowed" and also per the response to Webcor RFI T-0934 (SK RFI # 187) clarification is required regarding the last statement where coping the bottom flange of the shallow beam shall not exceed 1" from the end of the beam.						
1). Per detail 1/S1-5010 where there is a double sided beam connection, the bottom flange is required to be coped in the shallow beam in order to allow for installation of the bolts from the shallow beam side. Note the bolts cannot be torqued if erected from the other side and would also foul the shallow bottom beam flange in question. On the attached sketches CD RFI # 126.1 SK1 to SK4 show some typical sample conditions illustrating the clearance required. The beam flange cope lengths required will range from 4" long in most cases to 4 1/2" at larger web thicknesses. Please verify the shallow bottom beam flanges can be coped for bolt clearance and erection of the beams as noted on SK2 to SK4.						
2). Per detail 9/S1-5010 where the WT on the top of the beam flange is required to extend to the end of the beam please verify the beam flange can be coped back to the "k1" of the beam in order to get full bearing and weld for the WT and to clear bolts as noted on SK2.						
<b>ANSWER:</b>						
Per the response to Webcor RFI T-0920 (SK RFI # 172) clarification is required regarding the last statement where "Coping the bottom flange of the shallow beam is not allowed" and also per the response to Webcor RFI T-0934 (SK RFI # 187) clarification is required regarding the last statement where coping the bottom flange of the shallow beam shall not exceed 1" from the end of the beam.						
1). Per detail 1/S1-5010 where there is a double sided beam connection, the bottom flange is required to be coped in the shallow beam in order to allow for installation of the bolts from the shallow beam side. Note the bolts cannot be torqued if erected from the other side and would also foul the shallow bottom beam flange in question. On the attached sketches CD RFI # 126.1 SK1 to SK4 show some typical sample conditions illustrating the clearance required. The beam flange cope lengths required will range from 4" long in most cases to 4 1/2" at larger web thicknesses. Please verify the shallow bottom beam flanges can be coped for bolt clearance and erection of the beams as noted on SK2 to SK4.						
2). Per detail 9/S1-5010 where the WT on the top of the beam flange is required to extend to the end of the beam please verify the beam flange can be coped back to the "k1" of the beam in order to get full bearing and weld for the WT and to clear bolts as noted on SK2.						
<b>T-0921</b>	<b>SSS - Detail Clarifications For Edge of Slab Supports</b>	<b>Closed</b>	<b>CR</b>	<b>11/18/2013</b>	<b>11/28/2013</b>	<b>11/25/2013</b>



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	<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>Per details 8 &amp; 10/S1-5001, refer to sketches CD RFI 121 SK1 for items 1 &amp; 2:</p> <p>1) Confirm the noted area indicates that the concrete slab is not required and the edge plates may be terminated as shown.</p> <p>2) Confirm the noted area indicates that the concrete slab is not required and the edge plates may be terminated as shown.</p>					<p><b>ANSWER:</b></p> <p>Per details 8 &amp; 10/S1-5001, refer to sketches CD RFI 121 SK1 for items 1 &amp; 2:</p> <p>1) Confirm the noted area indicates that the concrete slab is not required and the edge plates may be terminated as shown.</p> <p>2) Confirm the noted area indicates that the concrete slab is not required and the edge plates may be terminated as shown.</p>
T-0922	SSS - W-1 Support Connection Clarifications at Bus Deck	Closed	CR	11/18/2013	11/28/2013	12/12/2013
	<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>Refer to CK RFI 125 SK1, SK2A, SK2B, SK3, and SK4 requesting clarification at the Bus Deck level on the following:</p> <p>1) Confirm the noted connection should be a moment connection.</p> <p>2) At the noted location, two supports for CP5 connections are required adjacent to Grid 9. Based on the CP5 detail requirements, a 1 ¼" horizontal stiffener should span from shear plate to shear plate per 1B/S1-8003; however, because these two connection points span the same beam, the horizontal stiffener would foul the incoming beam to shear plate connection, as there is a horizontal stiffener welded on both sides of the shear plate. Please provide a solution for this condition.</p> <p>3) Confirm the vertical spacing of the 1 ¼" horizontal stiffeners is acceptable to accommodate the connection bolts on the incoming beams.</p> <p>4) In the beam connection shown in detail 1/S1-8003, the required shear plate will foul the 2" web reinforcement plate required per 1/S1-5017. Please confirm the shear plate is to be welded to the 2" web reinforcement plate with a ½"double fillet weld per 1/S1-8003 or provide an alternate connection detail.</p> <p>5) A CJP weld is required at the flange connections shown on CD RFI 125 SK2A and SK2B; however, the indicated flanges are out of alignment per the dimensions shown. Please advise on the welding or connection requirements at this condition.</p> <p>6) Confirm the noted 1" stiffener plate per 1/S1-8003 may be welded to the 2" web reinforcement plate as shown in</p>				<p><b>ANSWER:</b></p> <p>Refer to CK RFI 125 SK1, SK2A, SK2B, SK3, and SK4 requesting clarification at the Bus Deck level on the following:</p> <p>1) Confirm the noted connection should be a moment connection.</p> <p>2) At the noted location, two supports for CP5 connections are required adjacent to Grid 9. Based on the CP5 detail requirements, a 1 ¼" horizontal stiffener should span from shear plate to shear plate per 1B/S1-8003; however, because these two connection points span the same beam, the horizontal stiffener would foul the incoming beam to shear plate connection, as there is a horizontal stiffener welded on both sides of the shear plate. Please provide a solution for this condition.</p> <p>3) Confirm the vertical spacing of the 1 ¼" horizontal stiffeners is acceptable to accommodate the connection bolts on the incoming beams.</p> <p>4) In the beam connection shown in detail 1/S1-8003, the required shear plate will foul the 2" web reinforcement plate required per 1/S1-5017. Please confirm the shear plate is to be welded to the 2" web reinforcement plate with a ½"double fillet weld per 1/S1-8003 or provide an alternate connection detail.</p> <p>5) A CJP weld is required at the flange connections shown on CD RFI 125 SK2A and SK2B; however, the indicated flanges are out of alignment per the dimensions shown. Please advise on the welding or connection requirements at this condition.</p> <p>6) Confirm the noted 1" stiffener plate per 1/S1-8003</p>	



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	<p>SK2A and SK3.</p> <p>7) Due to the placement of the 1/14" horizontal stiffener plates and required 1/2" fillet weld, the bolts for the beam connections will not be erectable. Please confirm it is acceptable to clip the horizontal stiffener plates as shown to accommodate the erection bolts or supply an alternate solution.</p> <p>8) Confirm it is acceptable to cut the 1 1/4" horizontal stiffener plates as shown to avoid fouling the 2" web reinforcement plate or supply an alternate solution.</p> <p>9) Confirm the hole locations for the W-1 glazing system per 1/S1-8003 are acceptable as shown or supply alternate locations.</p> <p>10) Confirm 1 9/16" dia. holes are acceptable or provide alternate hole size.</p>					<p>may be welded to the 2" web reinforcement plate as shown in SK2A and SK3.</p> <p>7) Due to the placement of the 1/14" horizontal stiffener plates and required 1/2" fillet weld, the bolts for the beam connections will not be erectable. Please confirm it is acceptable to clip the horizontal stiffener plates as shown to accommodate the erection bolts or supply an alternate solution.</p> <p>8) Confirm it is acceptable to cut the 1 1/4" horizontal stiffener plates as shown to avoid fouling the 2" web reinforcement plate or supply an alternate solution.</p> <p>9) Confirm the hole locations for the W-1 glazing system per 1/S1-8003 are acceptable as shown or supply alternate locations.</p> <p>10) Confirm 1 9/16" dia. holes are acceptable or provide alternate hole size.</p>
<b>T-0922.1</b>	<b>SSS - W-1 Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>01/17/2014</b>	<b>01/27/2014</b>	<b>02/06/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p> <p><b>REQUEST:</b></p> <p>This is a follow-up RFI to RFI T-0922 (SK 171 CD 125) See attached CD RFI # 125.1 SK1 to SK4 for items 1 to 3:</p> <p>1.) The 2" stiffener per 1/S1-8003 (ASI 109) and the beam connection shear plate foul each other as shown. Confirm it is acceptable to offset the 2" stiffener as required and use it as the shear plate for the beam connection. If not, supply an alternate solution.</p> <p>2.) Please supply missing dimensions.</p> <p>3.) The noted dimensions per 1/S1-8003 in ASI 109 (SK4) will result in the bottom holes fouling the beam flange. Please issue revised hole locations to suit the beam sizes.</p> <p>See attached CD RFI # 125.2 SK1 to SK3 for item #4:</p> <p>4.) The RFI T-0922 item 5 instruction to supply a PJP weld with a 3/4" fillet weld on top as shown is not possible as there is only 9/16" of material remaining on top as shown. A PJP weld requires a 0" gap which is not possible as there</p>					<p><b>ANSWER:</b></p> <p>This is a follow-up RFI to RFI T-0922 (SK 171 CD 125) See attached CD RFI # 125.1 SK1 to SK4 for items 1 to 3:</p> <p>1.) The 2" stiffener per 1/S1-8003 (ASI 109) and the beam connection shear plate foul each other as shown. Confirm it is acceptable to offset the 2" stiffener as required and use it as the shear plate for the beam connection. If not, supply an alternate solution.</p> <p>2.) Please supply missing dimensions.</p> <p>3.) The noted dimensions per 1/S1-8003 in ASI 109 (SK4) will result in the bottom holes fouling the beam flange. Please issue revised hole locations to suit the beam sizes.</p> <p>See attached CD RFI # 125.2 SK1 to SK3 for item #4:</p> <p>4.) The RFI T-0922 item 5 instruction to supply a PJP weld with a 3/4" fillet weld on top as shown is not possible as there</p>



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	is no erection clearance. Please supply an alternate weld.					is only 9/16" of material remaining on top as shown. A PJP weld requires a 0" gap which is not possible as there is no erection clearance. Please supply an alternate weld.
<b>T-0922.2</b>	<b>SSS - W-1 Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>01/17/2014</b>	<b>01/27/2014</b>	<b>01/21/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  This is a follow-up RFI to RFI T-0922 item 5 (SK 171 CD 125)  See attached CD RFI # 125.2 SK1 to SK3:  The RFI T-0922 item 5 instruction to supply a PJP weld with a 3/4" fillet weld on top as shown is not possible as there is only 9/16" of material remaining on top as shown.  Also, a PJP weld requires a 0" gap and this is not possible as there is no erection clearance. Please supply an alternate weld.						<b>ANSWER:</b>  This is a follow-up RFI to RFI T-0922 item 5 (SK 171 CD 125)  See attached CD RFI # 125.2 SK1 to SK3:  The RFI T-0922 item 5 instruction to supply a PJP weld with a 3/4" fillet weld on top as shown is not possible as there is only 9/16" of material remaining on top as shown. Also, a PJP weld requires a 0" gap and this is not possible as there is no erection clearance. Please supply an alternate weld.
<b>T-0923</b>	<b>SSS - W-1 Glazing System CP6 Connections</b>	<b>Closed</b>	<b>CR</b>	<b>11/19/2013</b>	<b>11/29/2013</b>	<b>12/11/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  Refer to CD RFI 128 SK1 through SK4 in response to the following regarding the W-1 glazing system connection "CP6" at the bus deck level: 1) The indicated CP6 connections foul the beam connections as indicated in SK3A and SK3B. Please provide a solution to this condition. 2) Confirm the holes for the "CP6" connections may be typically located as shown in SK4 along Grids B & H. 3) Confirm the connection holes for "CP6" are 1 9/16" diameter or provide the required hole diameter.						<b>ANSWER:</b>  Refer to CD RFI 128 SK1 through SK4 in response to the following regarding the W-1 glazing system connection "CP6" at the bus deck level: 1) The indicated CP6 connections foul the beam connections as indicated in SK3A and SK3B. Please provide a solution to this condition. 2) Confirm the holes for the "CP6" connections may be typically located as shown in SK4 along Grids B & H. 3) Confirm the connection holes for "CP6" are 1 9/16" diameter or provide the required hole diameter.



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T-0923.1	SSS - Dimension Clarification for W-1 Glazing	Closed	CR	01/06/2014	01/16/2014	01/14/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:		ANSWER:				
See attached CD RFI # 128.1 SK1 & SK2:		See attached CD RFI # 128.1 SK1 & SK2:				
RFI T-0923 SK4 was submitted with the center of "CP6" down 2'-0 1/4 from the top of steel. The 2'-0 1/4 dimension was taken from the Rhino model. The response to RFI T-0923 item 2 has changed the 2'-0 1/4 dimension to 2'-0 1/8.		RFI T-0923 SK4 was submitted with the center of "CP6" down 2'-0 1/4 from the top of steel. The 2'-0 1/4 dimension was taken from the Rhino model. The response to RFI T-0923 item 2 has changed the 2'-0 1/4 dimension to 2'-0 1/8.				
1) Confirm the 2'-0 1/4 dimension from the Rhino model is correct.		1) Confirm the 2'-0 1/4 dimension from the Rhino model is correct.				
2) Confirm the locations for all connections for the W-1 glazing system on the Bus Level may be taken from the Rhino model.		2) Confirm the locations for all connections for the W-1 glazing system on the Bus Level may be taken from the Rhino model.				
T-0923.2	SSS - W-1 Glazing System CP6 Connections	Closed	CR	05/13/2014	05/23/2014	05/27/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:		ANSWER:				
See attached CD RFI # 128.2 SK1 & SK2 for items 1 & 2:		See attached CD RFI # 128.2 SK1 & SK2 for items 1 & 2:				
1) The 1 1/2" thick plate per 4/S1-8003 with the 1/2" fillet welds will interfere with the "CP6" bolts if the beam is placed on the center of the CP6 connection per RFI T-0923 (SK1). Confirm it is acceptable to place the 1 1/2" thick plate on the center of the "CP6" connection and off-set the beam accordingly.		1) The 1 1/2" thick plate per 4/S1-8003 with the 1/2" fillet welds will interfere with the "CP6" bolts if the beam is placed on the center of the CP6 connection per RFI T-0923 (SK1). Confirm it is acceptable to place the 1 1/2" thick plate on the center of the "CP6" connection and off-set the beam accordingly.				
2) Detail 4/S1-8003 calls for the PL 1 1/2" to be 9" wide. Confirm it is acceptable to make the PL 5 1/2" wide as shown to avoid interference with the bolts for the safety handrail post.		2) Detail 4/S1-8003 calls for the PL 1 1/2" to be 9" wide. Confirm it is acceptable to make the PL 5 1/2" wide as shown to avoid interference with the bolts for the safety handrail post.				
T-0924	BGP - Column Stirrups and Ties at Top of Concourse (Mock-Up Review)	Closed	CR	11/19/2013	11/19/2013	11/22/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:		ANSWER:				
Please refer to drawing S1-3304, 3305 and 5051.		Please refer to drawing S1-3304, 3305 and 5051.				
Please confirm that it is acceptable to install the top		Please confirm that it is acceptable to install the top				



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	column stirrups and tie at 12.5" from the top of concrete at concourse level for the concrete columns with anchor base plates.					
T-0925	<b>BGP - Moment Frame Beam Top Tie 180-degree Hook (Mock-Up Review)</b>  <b>From:</b> Webcor Construction LP                      Jackson Tukuafu  <b>REQUEST:</b>  Please refer to attached drawing 2/S1-3600.  In order to clear the additional top bars in the top layer of moment frame beam, Gerdau proposes change one end the moment frame beam top tie hook from 135° to 180°. The opposite end of the tie will remain as a 90° hook.  Please confirm if this is acceptable.	Closed	CR	11/19/2013	11/29/2013	11/22/2013
		</				



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<b>T-0927</b>	<b>BGP - Injection Hose Testing Criteria</b>	<b>Closed</b>	<b>CR</b>	<b>11/21/2013</b>	<b>12/01/2013</b>	<b>12/04/2013</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Jackson Tukuafu</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference attached Grace/DeNeef Technical Letter, Submittal TG0600-0025, and Spec Section 03 15 00, 3.4, A.			Please reference attached Grace/DeNeef Technical Letter, Submittal TG0600-0025, and Spec Section 03 15 00, 3.4, A.			
Spec Section 03 15 00, 3.4, A states, "After concrete has cured for a minimum of 30 days, test the integrity of the entire hose system by compressed air. Ensure that a positive pressure can be maintained for at least 5 minutes."			Spec Section 03 15 00, 3.4, A states, "After concrete has cured for a minimum of 30 days, test the integrity of the entire hose system by compressed air. Ensure that a positive pressure can be maintained for at least 5 minutes."			
Page 14 of the "Applicator Manual" included in Submittal TG0600-0025 states that "each section of INJECTO should be pressure tested with water to a minimum pressure of 1 00 psi, to insure migration of water through the entire joint. If excessive water leakage out of joint is observed, this may indicate the presence of honeycombs or voids and should be noted on job report..."			Page 14 of the "Applicator Manual" included in Submittal TG0600-0025 states that "each section of INJECTO should be pressure tested with water to a minimum pressure of 1 00 psi, to insure migration of water through the entire joint. If excessive water leakage out of joint is observed, this may indicate the presence of honeycombs or voids and should be noted on job report..."			
In addition, the attached Grace/DeNeef Technical Letter also notes that the INJECTO should be tested with water (not air).			In addition, the attached Grace/DeNeef Technical Letter also notes that the INJECTO should be tested with water (not air).			
Please confirm that it is acceptable to test the integrity of the INJECTO hoses with water as required by the manufacturer.			Please confirm that it is acceptable to test the integrity of the INJECTO hoses with water as required by the manufacturer.			





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<b>T-0927.1</b>	<b>BGP - Injection Hose Testing Criteria</b>	<b>Closed</b>	<b>CR</b>	<b>01/06/2014</b>	<b>01/16/2014</b>	<b>01/21/2014</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please reference attached Grace/Deneef Technical Letter and RFI #T-0927 response.  RFI T-0927 response states that "contractor shall use air to test hoses as specified," but the specifications call out several different types of injection hoses. Although, air testing may be suitable for other products specified, Grace/Deneef requires water testing for the INJECTO Tube system. The attached technical letter from Grace/Deneef states that "INJECTO is an open system, and any air pumped in will begin to flow immediately through the 35 micron filter and polypropylene mesh out into the concrete."  Please review and confirm that water testing is acceptable for the Deneef INJECTO Tube system on the TTC project.						
						<b>ANSWER:</b>  Please reference attached Grace/Deneef Technical Letter and RFI #T-0927 response.  RFI T-0927 response states that "contractor shall use air to test hoses as specified," but the specifications call out several different types of injection hoses. Although, air testing may be suitable for other products specified, Grace/Deneef requires water testing for the INJECTO Tube system. The attached technical letter from Grace/Deneef states that "INJECTO is an open system, and any air pumped in will begin to flow immediately through the 35 micron filter and polypropylene mesh out into the concrete."  Please review and confirm that water testing is acceptable for the Deneef INJECTO Tube system on the TTC project.
<b>T-0927.2</b>	<b>BGP - Injection Hose Testing Criteria</b>	<b>Closed</b>	<b>01</b>	<b>02/18/2014</b>	<b>02/28/2014</b>	<b>02/19/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>  Per conference call with design team, please confirm that it is acceptable to test the waterstop injection hoses with water as recommended by manufacturer.						
						<b>ANSWER:</b>  Per conference call with design team, please confirm that it is acceptable to test the waterstop injection hoses with water as recommended by manufacturer.
<b>T-0928</b>	<b>RFI T-0928 SSS - Detail Clarification at Cast Node Connections</b>	<b>Closed</b>	<b>CR</b>	<b>11/22/2013</b>	<b>12/02/2013</b>	<b>12/04/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Please reference the cast node connection details 2/S1-4354 and 2/S1-4355 shown on CD RFI 131 SK1 and verify the following. 1) Confirm the indicated 4'-0" radius is acceptable or provide alternate dimension. 2) Confirm the indicated 4'-0" radius is acceptable or provide alternate dimension.						
						<b>ANSWER:</b>  Please reference the cast node connection details 2/S1-4354 and 2/S1-4355 shown on CD RFI 131 SK1 and verify the following. 1) Confirm the indicated 4'-0" radius is acceptable or provide alternate dimension. 2) Confirm the indicated 4'-0" radius is acceptable or provide alternate dimension.





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<b>T-0929</b>	<b>SSS - Connection Clarification at Edge of Slab GL 11</b>	<b>Closed</b>	<b>CR</b>	<b>11/22/2013</b>	<b>12/02/2013</b>	<b>12/06/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Refer to S1-2403 for locations near grids D.11 and F.11 indicated on CD RFI 135 SK1. As detailed in CD RFI 135 SK2, the L5x5 connection angles required per detail 1/S1-5010 will extend beyond the edge of slab by 3/16". Please confirm this is acceptable or provide an alternate detail for this condition.						<b>ANSWER:</b> Refer to S1-2403 for locations near grids D.11 and F.11 indicated on CD RFI 135 SK1. As detailed in CD RFI 135 SK2, the L5x5 connection angles required per detail 1/S1-5010 will extend beyond the edge of slab by 3/16". Please confirm this is acceptable or provide an alternate detail for this condition.
<b>T-0930</b>	<b>SSS - Scope Confirmation at Stairs</b>	<b>Closed</b>	<b>CR</b>	<b>11/22/2013</b>	<b>12/02/2013</b>	<b>11/25/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Per the response to TG07.1R-0041, "the scope of work for stair posts, landing framing, stringers, and checkered plate tread and riser will be included in a future bid package." In accordance with TG07.1R-0041, please confirm this response applies to the entirety of the TG07.1R scope including, but not limited to, drawings S1-7001 through S1-7016.						<b>ANSWER:</b> Per the response to TG07.1R-0041, "the scope of work for stair posts, landing framing, stringers, and checkered plate tread and riser will be included in a future bid package." In accordance with TG07.1R-0041, please confirm this response applies to the entirety of the TG07.1R scope including, but not limited to, drawings S1-7001 through S1-7016.
<b>T-0931</b>	<b>SSS - Connection Clarifications at Isolation Bearings</b>	<b>Closed</b>	<b>CR</b>	<b>11/22/2013</b>	<b>12/02/2013</b>	<b>11/26/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Please refer to the isolation bearing details on S1-5021 and CD RFI 138 SK1 & SK2 attached for the following items: 1) Please provide dimensions required to located bolts. 2) Confirm the cap plate may be welded as indicated in the attached sketch. 3) Please provide dimensions required to located bolts. 4) Please provide dimensions required to located bolts. 5) Please provide dimensions required to located bolts. 6) Please provide dimensions required to located bolts.						<b>ANSWER:</b> Please refer to the isolation bearing details on S1-5021 and CD RFI 138 SK1 & SK2 attached for the following items: 1) Please provide dimensions required to located bolts. 2) Confirm the cap plate may be welded as indicated in the attached sketch. 3) Please provide dimensions required to located bolts. 4) Please provide dimensions required to located bolts. 5) Please provide dimensions required to located bolts. 6) Please provide dimensions required to located bolts.



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T-0931.1	<b>SSS -Connection Clarifications at Isolation Bearings</b>  From: Webcor Construction LP                      Stephanie Azzolino  <b>REQUEST:</b>  Please refer to the isolation bearing details on SK1 & SK2 attached for the following items: 1) Please provide the bolt pattern & size connecting the isolation bearings to the W12x65 & W8x31. 2) Please provide the bolt pattern & size connecting the isolation bearings to the 3" steel plate & C15x40.	Closed	CR	01/22/2014	02/01/2014	02/03/2014
	<b>ANSWER:</b>  Please refer to the isolation bearing details on SK1 & SK2 attached for the following items: 1) Please provide the bolt pattern & size connecting the isolation bearings to the W12x65 & W8x31. 2) Please provide the bolt pattern & size connecting the isolation bearings to the 3" steel plate & C15x40.					
T-0932	<b>SSS - Detail Clarification at Hanger Support</b>  From: Webcor Construction LP                      Gregory Kemerer  <b>REQUEST:</b>  Refer to S1-2503 near grid 9.9/C and CD RFI 139 SK1 & SK2 which indicate that the W12x65 hanger support beam fouls the skewed W40x327. This same condition occurs at Grid 9.9/G.  Please confirm it is acceptable to trim the bottom flange of the W12x65 beam to maintain a ½" gap between the beam flanges.	Closed	CR	11/22/2013	12/02/2013	11/26/2013
	<b>ANSWER:</b>  Refer to S1-2503 near grid 9.9/C and CD RFI 139 SK1 & SK2 which indicate that the W12x65 hanger support beam fouls the skewed W40x327. This same condition occurs at Grid 9.9/G.  Please confirm it is acceptable to trim the bottom flange of the W12x65 beam to maintain a ½" gap between the beam flanges.					
T-0933	<b>SSS - Slab Opening Discrepancy at F.5</b>  From: Webcor Construction LP                      Gregory Kemerer  <b>REQUEST:</b>  The slab opening near grid F.5 indicated on drawings S1-2302 and 2/S1-7101 (SK1 & SK2) does not match the location indicated on drawing A1-2862 (SK3). Please clarify the correct slab opening location and provide dimensions to locate the slab opening and perimeter steel.	Closed	CR	11/22/2013	12/02/2013	12/09/2013
	<b>ANSWER:</b>  The slab opening near grid F.5 indicated on drawings S1-2302 and 2/S1-7101 (SK1 & SK2) does not match the location indicated on drawing A1-2862 (SK3). Please clarify the correct slab opening location and provide dimensions to locate the slab opening and perimeter steel.					
T-0934	<b>SSS - Beam Connection Clarifications</b>  From: Webcor Construction LP                      Gregory Kemerer  <b>REQUEST:</b>  Please refer sketches CD RFI 141 SK1 to SK7 for beam to beam connection clarifications required per items 1 to 4 below:	Closed	CR	11/22/2013	12/02/2013	12/06/2013
	<b>ANSWER:</b>  Please refer sketches CD RFI 141 SK1 to SK7 for beam to beam connection clarifications required per items 1 to 4 below:					



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	<div>1) On S1-2505 between grids 1.4 &amp; 2, the required (9) bolts per 1/S1-5010 will not fit in the W33 due to the size of the supporting BU beam. Confirm (8) bolts as shown are acceptable or supply an alternate solution. See SK1 &amp; SK2.</div> <div>2) On S1-2505 near grids 24.9/E, the required (9) bolts per 1/S1-5010 will not fit in the W33 due to the size of the supporting BU beam. Confirm (8) bolts as shown are acceptable or supply an alternate solution. See SK3 &amp; SK4.</div> <div>3) On S1-2507 near grids 33.2/E, the required (10) bolts per 1/S1-5010 will not fit in the W36 due to the size of the supporting BU beam. Confirm (8) bolts as shown are acceptable or supply an alternate solution. See SK5 &amp; SK6.</div> <div>4) On S1-2403 at grids 8/D.8, the required (8) bolts per 1/S1-5010 will not fit in the W30 due to the size of the supporting BU beam. Confirm (7) bolts as shown are acceptable or supply an alternate solution. See SK7.</div>					<div>1) On S1-2505 between grids 1.4 &amp; 2, the required (9) bolts per 1/S1-5010 will not fit in the W33 due to the size of the supporting BU beam. Confirm (8) bolts as shown are acceptable or supply an alternate solution. See SK1 &amp; SK2.</div> <div>2) On S1-2505 near grids 24.9/E, the required (9) bolts per 1/S1-5010 will not fit in the W33 due to the size of the supporting BU beam. Confirm (8) bolts as shown are acceptable or supply an alternate solution. See SK3 &amp; SK4.</div> <div>3) On S1-2507 near grids 33.2/E, the required (10) bolts per 1/S1-5010 will not fit in the W36 due to the size of the supporting BU beam. Confirm (8) bolts as shown are acceptable or supply an alternate solution. See SK5 &amp; SK6.</div> <div>4) On S1-2403 at grids 8/D.8, the required (8) bolts per 1/S1-5010 will not fit in the W30 due to the size of the supporting BU beam. Confirm (7) bolts as shown are acceptable or supply an alternate solution. See SK7.</div>
T-0935	<div>BGP - Lower Concourse Typical Moment Frame Beam Dimensions</div> <div>From: Webcor Construction LP Jackson Tukuafu</div> <div>REQUEST:</div> <div>Please refer to drawing S1-2204 and S1-2205.</div> <div>Plan sheets S1-2204 and S1-2205 show 8 Moment Frame Beams (MFB) from GL 14 to GL 20.1 designated as typical. There are no section views of these beams which show the dimensions, as the other MFB have.</div> <div>Please provide both the Width and Depth of the typical MFB in the lower concourse level.</div>	Closed	CR	11/22/2013	12/02/2013	11/25/2013
	<div>ANSWER:</div> <div>Please refer to drawing S1-2204 and S1-2205.</div> <div>Plan sheets S1-2204 and S1-2205 show 8 Moment Frame Beams (MFB) from GL 14 to GL 20.1 designated as typical. There are no section views of these beams which show the dimensions, as the other MFB have.</div> <div>Please provide both the Width and Depth of the typical MFB in the lower concourse level.</div>					



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<b>T-0936</b>	<b>SSS - HSS Hanger Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>11/22/2013</b>	<b>12/02/2013</b>	<b>12/04/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Refer to S1-2403 (CD SK1) which indicates the edge detail between grids 8 & 9.9 is to be constructed with an L5x5x3/8 angle per 9/S1-5000. Please confirm it is acceptable to extend the W24x68 beam to the edge of slab, eliminate the L5x5x3/8, and connect the HSS 5" hanger to the W24x68 similar to the detail shown on 1/S1-5020 or provide an alternate detail for this connection.						<b>ANSWER:</b> Refer to S1-2403 (CD SK1) which indicates the edge detail between grids 8 & 9.9 is to be constructed with an L5x5x3/8 angle per 9/S1-5000. Please confirm it is acceptable to extend the W24x68 beam to the edge of slab, eliminate the L5x5x3/8, and connect the HSS 5" hanger to the W24x68 similar to the detail shown on 1/S1-5020 or provide an alternate detail for this connection.
<b>T-0937</b>	<b>SSS - SMRF Flared End Connection</b>	<b>Closed</b>	<b>CR</b>	<b>11/22/2013</b>	<b>12/02/2013</b>	<b>12/02/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Refer to the SMRF flared end connections detailed in CD RFI 144 SK1 to SK5 and clarify the following:  Detail 9/S1-4202 indicates that the flared beam flange is to be the same width as the column flange while detail 5/S14202 does not match this detail and indicates a narrower flared beam flange. Please confirm that the beam flange width is as noted on the elevation drawings and the flange width shall increase at the flared ends to match the column width per detail 9/S1-4202.						<b>ANSWER:</b> Refer to the SMRF flared end connections detailed in CD RFI 144 SK1 to SK5 and clarify the following:  Detail 9/S1-4202 indicates that the flared beam flange is to be the same width as the column flange while detail 5/S14202 does not match this detail and indicates a narrower flared beam flange. Please confirm that the beam flange width is as noted on the elevation drawings and the flange width shall increase at the flared ends to match the column width per detail 9/S1-4202.
<b>T-0938</b>	<b>BGP - One-Way Slab Shrinkage and Temperature (S&amp;T) Bars at Columns</b>	<b>Closed</b>	<b>CR</b>	<b>11/22/2013</b>	<b>12/02/2013</b>	<b>11/25/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> Please refer to drawing S1-3500  In order to alleviate congestion in a condition where columns cross lower concourse support beams, please confirm that it is acceptable to eliminate the top and bottom shrinkage and temperature bars for the one-way slabs up to 12" from the face of support column.						<b>ANSWER:</b> Please refer to drawing S1-3500  In order to alleviate congestion in a condition where columns cross lower concourse support beams, please confirm that it is acceptable to eliminate the top and bottom shrinkage and temperature bars for the one-way slabs up to 12" from the face of support column.

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<b>T-0939</b>	<b>SSS - Connection Clarifications at Moment Beams</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>12/06/2013</b>
<div> <div> <b>From:</b> Webcor Construction LP Gregory Kemerer </div> </div>						
<b>REQUEST:</b>  Refer to CD RFI 132 SK1 to SK4 requiring clarification on the moment beam to beam connections per the following.  1) At the location indicated on CD RFI 132 SK1, the continuity plate will foul the bolts if (8) are provided per 1/S15010. Please confirm it is acceptable to provide (6) bolts in the W30x99 as shown on CD RFI 132 SK2. 2) Confirm the continuity plate detailed on CD RFI 132 SK2 is correct as shown with tf and bf per W24x68. 3) Please confirm it is acceptable to provide one continuity plate with a slot 1/8" larger than the beam web and the 3 ½" beam cope as indicated on CD RFI 132 SK2 to allow for a continuous CJP weld of the continuity plate. 4) At the location indicated on CD RFI 132 SK1, the continuity plate will foul the bolts if (11) are provided per 1/S1-5010. Please confirm it is acceptable to provide (8) bolts in the W40x277 as shown on CD RFI 132 SK3. 5) Confirm the continuity plate detailed on CD RFI 132 SK3 is correct as shown with tf and bf per W30x99. 6) Please confirm it is acceptable to provide one continuity plate with a slot 1/8" larger than the beam web and the 3 ½" beam cope as indicated on CD RFI 132 SK3 to allow for a continuous CJP weld of the continuity plate. 7) At the location indicated on CD RFI 132 SK1, the continuity plate will foul the bolts if (9) are provided per 1/S15010. Please confirm it is acceptable to provide (8) bolts in the W33x118 as shown on CD RFI 132 SK4. 8) Please confirm it is acceptable to provide one continuity plate with a slot 1/8" larger than the beam web and the 3 ½" beam cope as indicated on CD RFI 132 SK4 to allow for a continuous CJP weld of the continuity plate. 9) Confirm the continuity plate detailed on CD RFI 132 SK4 is correct as shown with tf and bf per W30x99. 10) Confirm the response to items 1 to 9 may be typically applied at other similar conditions/locations or provide a typical solution for the condition where the required continuity plate extends into the double angle connection of a deeper beam.			<b>ANSWER:</b>  Refer to CD RFI 132 SK1 to SK4 requiring clarification on the moment beam to beam connections per the following.  1) At the location indicated on CD RFI 132 SK1, the continuity plate will foul the bolts if (8) are provided per 1/S15010. Please confirm it is acceptable to provide (6) bolts in the W30x99 as shown on CD RFI 132 SK2.  2) Confirm the continuity plate detailed on CD RFI 132 SK2 is correct as shown with tf and bf per W24x68. 3) Please confirm it is acceptable to provide one continuity plate with a slot 1/8" larger than the beam web and the 3 ½" beam cope as indicated on CD RFI 132 SK2 to allow for a continuous CJP weld of the continuity plate. 4) At the location indicated on CD RFI 132 SK1, the continuity plate will foul the bolts if (11) are provided per 1/S1-5010. Please confirm it is acceptable to provide (8) bolts in the W40x277 as shown on CD RFI 132 SK3. 5) Confirm the continuity plate detailed on CD RFI 132 SK3 is correct as shown with tf and bf per W30x99. 6) Please confirm it is acceptable to provide one continuity plate with a slot 1/8" larger than the beam web and the 3 ½" beam cope as indicated on CD RFI 132 SK3 to allow for a continuous CJP weld of the continuity plate. 7) At the location indicated on CD RFI 132 SK1, the continuity plate will foul the bolts if (9) are provided per 1/S15010. Please confirm it is acceptable to provide (8) bolts in the W33x118 as shown on CD RFI 132 SK4. 8) Please confirm it is acceptable to provide one continuity plate with a slot 1/8" larger than the beam web and the 3 ½" beam cope as indicated on CD RFI 132 SK4 to allow for a continuous CJP weld of the continuity plate. 9) Confirm the continuity plate detailed on CD RFI 132 SK4 is correct as shown with tf and bf per W30x99. 10) Confirm the response to items 1 to 9 may be typically applied at other similar conditions/locations or provide a typical solution for the condition where the required continuity plate extends into the double angle connection of a deeper beam.			



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<b>T-0939.1</b>	<b>SSS - Connection Clarifications at Moment Beams</b>	<b>Closed</b>	<b>CR</b>	<b>12/19/2013</b>	<b>12/29/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> At beam to beam moment connections as noted in RFI # T-0939 and other similar locations please confirm if the continuity plate is required when the nominal depth of the beam shown as dimension "X" is 3" or less as per CD RFI 132.1 SK1.					<b>ANSWER:</b> At beam to beam moment connections as noted in RFI # T-0939 and other similar locations please confirm if the continuity plate is required when the nominal depth of the beam shown as dimension "X" is 3" or less as per CD RFI 132.1 SK1.	
<b>T-0940</b>	<b>SSS - Shear Plate Dimension</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>11/26/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> Please confirm that the dimension indicated on CD RFI 130 SK1 at the Type 1 Drag Connection per detail 1/S1-5016 is to be taken from the Thornton Tomasetti Tekla Model.					<b>ANSWER:</b> Please confirm that the dimension indicated on CD RFI 130 SK1 at the Type 1 Drag Connection per detail 1/S1-5016 is to be taken from the Thornton Tomasetti Tekla Model.	
<b>T-0941</b>	<b>SSS - Beam Connection Details</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>11/25/2013</b>	<b>12/04/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> On S1-2604 near grids 16/E (near grid 25 sim.) at the Penthouse column base connections refer to sketches CD RFI 146A SK1 & SK2 for items 1a & 1b noted below.  1a) The noted beams connect to the supporting beam with double angles per S1-5010 but they will not be erectable due to the stiffeners per 11/S1-7630 (SK2). Confirm it is acceptable to use a pulled-out full depth shear plate per 4/S1-5013 at each end.  1b) Similar conditions occur on S1-2606 about grid 25. Confirm the solution in item 1a may be applied at other similar conditions.					<b>ANSWER:</b> On S1-2604 near grids 16/E (near grid 25 sim.) at the Penthouse use column base connections refer to sketches CD RFI 146A SK1 & SK2 for items 1a & 1b noted below.  1a) The noted beams connect to the supporting beam with double angles per S1-5010 but they will not be erectable due to the stiffeners per 11/S1-7630 (SK2). Confirm it is acceptable to use a pulled-out full depth shear plate per 4/S1-5013 at each end.  1b) Similar conditions occur on S1-2606 about grid 25. Confirm the solution in item 1a may be applied at other similar conditions.	
<b>T-0941.1</b>	<b>SSS - PE403 &amp; 404 Framing at Roof Level</b>	<b>Closed</b>	<b>CR</b>	<b>03/24/2014</b>	<b>04/03/2014</b>	<b>04/04/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>					<b>ANSWER:</b>	



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T-0942	<b>SSS - Shaw Alley Bridge Connections</b>  From: Webcor Construction LP  Gregory Kemerer	Closed	CR	11/25/2013	12/05/2013	12/19/2013
<b>REQUEST:</b> On S1- 2403 at the Shaw Alley Bridge refer to sketches CD RFI 1 04 SK1 to SK4 for items 1 to 6: 1) Confirm the horizontal long slots in detail 5/S1- 5013 apply only at this connection. 2) Confirm the closure plate may be welded as shown in li eu of the requested butt weld. 3) Confirm the closure plate may be welded as shown in li eu of the requested butt weld. 4) Confirm the weld is a PJP weld. 5) Confirm this CJP weld may be welded as shown. 6) Confirm the detail on SK2B is acceptable for 5/S1- 2403 Plan A.		<b>ANSWER:</b> On S1- 2403 at the Shaw Alley Bridge refer to sketches CD R FI 104 SK1 to SK4 for items 1 to 6: 1) Confirm the horizontal long slots in detail 5/S1- 5013 apply only at this connection. 2) Confirm the closure plate may be welded as shown in lieu of the requested butt weld. 3) Confirm the closure plate may be welded as shown in lieu of the requested butt weld. 4) Confirm the weld is a PJP weld. 5) Confirm this CJP weld may be welded as shown. 6) Confirm the detail on SK2B is acceptable for 5/S1- 2403 Plan A.				





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<b>T-0943</b>	<b>SSS - Light Column Base Details</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>12/11/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1. Please supply the material manufacture(s) for the "SEAL RING" a catalog cut or other information. Additionally, please supply the specifications for the material, size necessary to fit specified tube and other information necessary to install the seal rings.			1. Please supply the material manufacture(s) for the "SEAL RING" a catalog cut or other information. Additionally, please supply the specifications for the material, size necessary to fit specified tube and other information necessary to install the seal rings.			
2. Please provide weld size and weld process indicated on the attached sketch.			2. Please provide weld size and weld process indicated on the attached sketch.			
3. Please provide omitted dimensions for "CAVITY TUBE" requested on the attached sketch			3. Please provide omitted dimensions for "CAVITY TUBE" requested on the attached sketch			
4. Please confirm that welding the WELDED STEEL TUBE to the SOLID ROUND ANCHOR PLATE is acceptable and the alteration of the A722 plate by the welding process is acceptable.			4. Please confirm that welding the WELDED STEEL TUBE to the SOLID ROUND ANCHOR PLATE is acceptable and the alteration of the A722 plate by the welding process is acceptable.			
5. Please confirm missing dimension for anchor bolt projection. Note on 4/S1-6008 states "END OF ANCHOR BAR LEFT AFTER TRIMMING MUST BE LONG ENOUGH TO ALLOW RE-TENSIONING LATER". Skanska will provide projection of ½ coupler length to attached stressing rod at future date. Please confirm compatibility with TJPA's stressing system used later may be different than Contractor's stressing system.			5. Please confirm missing dimension for anchor bolt projection. Note on 4/S1-6008 states "END OF ANCHOR BAR LEFT AFTER TRIMMING MUST BE LONG ENOUGH TO ALLOW RE-TENSIONING LATER". Skanska will provide projection of ½ coupler length to attached stressing rod at future date. Please confirm compatibility with TJPA's stressing system used later may be different than Contractor's stressing system.			





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<b>T-0944</b>	<b>SSS - Beam Connection Clarification at Edge of Slab</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>12/04/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Refer to CD RFI 169 SK1 and SK2 showing beam connections into slab openings near grid 11/C on S1-2403.  1) The double angle connection required per S1-5010 will extend past the edge of slab as shown on CD RFI 169 SK2. Please confirm it is acceptable to replace these connections with shear plate connections per S1-5011 or provide an alternate solution.  2) Confirm it is typically acceptable to replace the double angle connections with shear plate connections when the double angles extend past the edge of slab.						<b>ANSWER:</b>  Refer to CD RFI 169 SK1 and SK2 showing beam connections into slab openings near grid 11/C on S1-2403.  1) The double angle connection required per S1-5010 will extend past the edge of slab as shown on CD RFI 169 SK2. Please confirm it is acceptable to replace these connections with shear plate connections per S1-5011 or provide an alternate solution.  2) Confirm it is typically acceptable to replace the double angle connections with shear plate connections when the double angles extend past the edge of slab.
<b>T-0945</b>	<b>SSS - Connection Clarification at Slab Edge</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>12/06/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Refer to S1-2403 near grid 9/E for slab edge support connections as indicated on CD RFI 170 SK1 and SK2. The backup kicker brace detailed on 9/S1-5000 will fit in condition # 1 as indicated, but it will not fit in conditions #2 and #3 due to the limited difference in beam depth.  Please confirm it is acceptable to omit the kicker braces at conditions #2 and #3 or provide an alternate detail for these conditions.						<b>ANSWER:</b>  Refer to S1-2403 near grid 9/E for slab edge support connections as indicated on CD RFI 170 SK1 and SK2. The backup kicker brace detailed on 9/S1-5000 will fit in condition # 1 as indicated, but it will not fit in conditions #2 and #3 due to the limited difference in beam depth.  Please confirm it is acceptable to omit the kicker braces at conditions #2 and #3 or provide an alternate detail for these conditions.
<b>T-0946</b>	<b>Dimension Clarification at Edge of Slab</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>12/04/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Refer to CD RFI 172 SK1 & SK2 regarding the following question along grid lines C & G on Level 2 at the edge of slab. The dimension indicated in CD RFI 172 SK1 is shown as 7" or 8" on S1-2402, S1-2403, and S1-2404 while detail 1/S15032 shows this as a 6" dimension.  Please confirm the 7" and 8" dimensions currently						<b>ANSWER:</b>  Refer to CD RFI 172 SK1 & SK2 regarding the following question along grid lines C & G on Level 2 at the edge of slab. The dimension indicated in CD RFI 172 SK1 is shown as 7" or 8" on S1-2402, S1-2403, and S1-2404 while detail 1/S15032 shows this as a 6" dimension.



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	modeled based on the plan drawings are to be used and the 6" dimension in detail 1/S1-5032 does not apply at specified locations.				Please confirm the 7" and 8" dimensions currently modeled based on the plan drawings are to be used and the 6" dimension in detail 1/S1-5032 does not apply at specified locations.	
<b>T-0947</b>	<b>SSS - Continuity Plate Foul at Column Web</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>12/04/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Refer to grids 11/D and 11/F on B/S1-4106 at Level 2 as indicated on CD RFI 173 SK 1 & SK2. The continuity plate required per 4/S1-5012 will foul the WT in the column web at the location indicated on CD RFI 173 SK2.  Please confirm it is acceptable to supply (2) continuity plates, one on each side of the stem of the WT, or provide an alternate detail for this condition.					<b>ANSWER:</b> Refer to grids 11/D and 11/F on B/S1-4106 at Level 2 as indicated on CD RFI 173 SK 1 & SK2. The continuity plate required per 4/S1-5012 will foul the WT in the column web at the location indicated on CD RFI 173 SK2.  Please confirm it is acceptable to supply (2) continuity plates, one on each side of the stem of the WT, or provide an alternate detail for this condition.	
<b>T-0948</b>	<b>SSS - Connection Clarifications at Beams to Transfer Girder</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>12/06/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> On S1-2303 at grids 11/C @ beams to transfer girder refer to sketch CD RFI 178 SK1: The bolts thru the column cap plate & Transfer Girder per 4/S1-5052 are fouling the 3" connection plate as shown. Please supply a solution.					<b>ANSWER:</b> On S1-2303 at grids 11/C @ beams to transfer girder refer to sketch CD RFI 178 SK1: The bolts thru the column cap plate & Transfer Girder per 4/S1-5052 are fouling the 3" connection plate as shown. Please supply a solution.	

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<b>T-0950</b>	<b>SSS - Stair &amp; Elevator Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>12/09/2013</b>
<div> <div> <b>From:</b> Webcor Construction LP Gregory Kemerer </div> <div> <b>REQUEST:</b> <p>For typical stair &amp; elevator connections refer to sketches CD RFI 180 SK1 to SK3 for items 1 to 5:</p> <p>1) Confirm this connection may be applied as shown on SK2B (item 2).</p> <p>2) Confirm connection as shown is acceptable. All not shown is per 2/S1-7600).</p> <p>3) Confirm it is acceptable to substitute the L3x3 angle with L5x3 angle. The connection with the L3x3x3/8 angle is not possible as the brace angles will foul the horizontal legs as shown.</p> <p>4) Confirm the same dimensions may be used when detail 2D/S1-7600 occurs.</p> <p>5) Confirm this is the correct interpretation of the weld for the angles to the HSS beam.</p> </div> <div> <b>ANSWER:</b> <p>For typical stair &amp; elevator connections refer to sketches CD RFI 180 SK1 to SK3 for items 1 to 5:</p> <p>1) Confirm this connection may be applied as shown on SK2B (item 2).</p> <p>2) Confirm connection as shown is acceptable. All not shown is per 2/S1-7600).</p> <p>3) Confirm it is acceptable to substitute the L3x3 angle with L5x3 angle. The connection with the L3x3x3/8 angle is not possible as the brace angles will foul the horizontal legs as shown.</p> <p>4) Confirm the same dimensions may be used when detail 2D/S1-7600 occurs.</p> <p>5) Confirm this is the correct interpretation of the weld for the angles to the HSS beam.</p> </div> </div>						



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<b>T-0951</b>	<b>SSS - Knock-Out Slab Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>11/25/2013</b>	<b>12/05/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>On S1-2303 there is a detail 7/S1-5004 shown near grids F/11 to supply bent plate to support the permanent slab. This is a general bent plate detail for the knock-out areas and does not provide enough detail at the stepped Transfer Girder. Please see the following questions below:</p> <p>1) Please verify if bent plate is required parallel to the Transfer Girder along grid 11 to support the permanent slab at the knockout areas? If yes, will new beams be needed to support the bent plate and slab? Please provide size and location if new beams are needed at these areas. See RFI 185 SK1 &amp; SK2.</p> <p>2) Please verify step in slab from grid D to F along grid line 11 will incase the Transfer Girder? Will headed studs be required at the transfer Girder web? If so, please provide size and spacing. See RFI 185 SK1 &amp; SK2.</p> <p>3) Please verify it is the designs intent to have the edge of the knock-out slab extend past the edge of the Transfer Girder flange at grid line 10.1? If yes, please provide details to support the edge of the permanent slab at these locations. See RFI 185 SK1.</p> <p>4) Please confirm only steel highlighted in yellow will require bent plate to support the permanent slab? See RFI 185 SK1.</p> <p>5). Please clarify if any slab support is required for the knock-out slab at the edge of the escalator pit as shown on detail 6/S17660, referenced from the escalator plan on 1/S1-7302? Should the knock-out slab be separated in some way from the curb/wall of the escalator pit? See RFI 185 SK1.</p>			<p>On S1-2303 there is a detail 7/S1-5004 shown near grids F/11 to supply bent plate to support the permanent slab. This is a general bent plate detail for the knock-out areas and does not provide enough detail at the stepped Transfer Girder. Please see the following questions below:</p> <p>1) Please verify if bent plate is required parallel to the Transfer Girder along grid 11 to support the permanent slab at the knockout areas? If yes, will new beams be needed to support the bent plate and slab? Please provide size and location if new beams are needed at these areas. See RFI 185 SK1 &amp; SK2.</p> <p>2) Please verify step in slab from grid D to F along grid line 11 will incase the Transfer Girder? Will headed studs be required at the transfer Girder web? If so, please provide size and spacing. See RFI 185 SK1 &amp; SK2.</p> <p>3) Please verify it is the designs intent to have the edge of the knock-out slab extend past the edge of the Transfer Girder flange at grid line 10.1? If yes, please provide details to support the edge of the permanent slab at these locations. See RFI 185 SK1.</p> <p>4) Please confirm only steel highlighted in yellow will require bent plate to support the permanent slab? See RFI 185 SK1.</p> <p>5). Please clarify if any slab support is required for the knock-out slab at the edge of the escalator pit as shown on detail 6/S17660, referenced from the escalator plan on 1/S1-7302? Should the knock-out slab be separated in some way from the curb/wall of the escalator pit? See RFI 185 SK1.</p>			



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<b>T-0951.1</b>	<b>SSS - Knock-Out Slab Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>03/10/2014</b>	<b>03/20/2014</b>	<b>03/20/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 325 SK1 for items 1 & 2: 1) Confirm the edge plate per T-0951 (SK 232, CD 185) SKS-0315 terminates 1'-0 from Grid 'D' or supply the missing dimension. 2) Confirm the edge plate per T-0951 (SK 232, CD 185) SKS-0315 terminates 8" from Grid 'F' or supply the missing dimension.						<b>ANSWER:</b> See attached CD RFI # 325 SK1 for items 1 & 2: 1) Confirm the edge plate per T-0951 (SK 232, CD 185) SKS-0315 terminates 1'-0 from Grid 'D' or supply the missing dimension. 2) Confirm the edge plate per T-0951 (SK 232, CD 185) SKS-0315 terminates 8" from Grid 'F' or supply the missing dimension.
<b>T-0952</b>	<b>BGP - Use of historical concrete strength test results</b>	<b>Closed</b>	<b>CR</b>	<b>11/27/2013</b>	<b>12/07/2013</b>	<b>12/05/2013</b>
<b>From:</b> Webcor Construction LP Michael Spillane						
<b>REQUEST:</b> Further to discussion with Thornton Tomasetti field personnel.  WOJV is asking for the remainder of the Mat slab pour, that the requirements per specification Section 31 55 00 1.4J may be deemed satisfied after 14 days to start removing the level D bracing based on historical data of the 284 concrete strength test results completed to date.  Please confirm if this would be acceptable						<b>ANSWER:</b> Further to discussion with Thornton Tomasetti field personnel.  WOJV is asking for the remainder of the Mat slab pour, that the requirements per specification Section 31 55 00 1.4J may be deemed satisfied after 14 days to start removing the level D bracing based on historical data of the 284 concrete strength test results completed to date.  Please confirm if this would be acceptable
<b>T-0953</b>	<b>SSS - Pin &amp; Pipe Connections at Bus Deck Level</b>	<b>Closed</b>	<b>CR</b>	<b>12/02/2013</b>	<b>12/12/2013</b>	<b>12/20/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> On S1-2505 at grids 21 & 22 refer to sketches CD RFI 149 SK1 to SK5 for items 1 to 3:  1) The plates for the pin connection per 5/S1-5017 foul the beam connections. See SK5 and confirm it is acceptable to modify the pin location as shown to avoid fouling the beam connections. 2) The pipe with connections per 5/S1-5017 at both ends will not be erectable without cutting the flanges on the beam stubs. Confirm it is acceptable to cut the bottom flanges as shown on SK5 or supply an alternate solution.						<b>ANSWER:</b> On S1-2505 at grids 21 & 22 refer to sketches CD RFI 149 S K1 to SK5 for items 1 to 3:  1) The plates for the pin connection per 5/S1-5017 foul the beam connections. See SK5 and confirm it is acceptable to modify the pin location as shown to avoid fouling the beam connections. 2) The pipe with connections per 5/S1-5017 at both ends will not be erectable without cutting the flanges on the beam stubs. Confirm it is acceptable to cut the bottom flanges as shown on SK5 or supply an









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<b>T-0955.3</b>	<b>SSS - HSS Stair Framing ST304</b>	<b>Closed</b>	<b>CR</b>	<b>03/20/2014</b>	<b>03/30/2014</b>	<b>04/07/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 175.3 SK1 to SK3 for items 1 to 4:  1) The noted dimension is shown as 12'-6 1/4 in RFI T-0955.1 (SK 224.1, CD 175.1) & A1-2863. Which is correct? 2) Supply a connection detail for the posts below. 3) Work with SK3 and confirm the noted dimensions are correct (to match the slab opening dimensions on the Ground Level). 4) Per S1-2403 the W12x14 beam is shown centered on the HSS12x6 stair posts. With the 5" offset dimension to the edge of slab, the south end of the slab opening will be 12'-4 7/8 from Grid C.3. Work with SK3 and confirm this is acceptable.						<b>ANSWER:</b> See attached CD RFI # 175.3 SK1 to SK3 for items 1 to 4: 1) The noted dimension is shown as 12'-6 1/4 in RFI T-0955.1 (SK 224.1, CD 175.1) & A1-2863. Which is correct? 2) Supply a connection detail for the posts below. 3) Work with SK3 and confirm the noted dimensions are correct (to match the slab opening dimensions on the Ground Level). 4) Per S1-2403 the W12x14 beam is shown centered on the HSS12x6 stair posts. With the 5" offset dimension to the edge of slab, the south end of the slab opening will be 12'-4 7/8 from Grid C.3. Work with SK3 and confirm this is acceptable.
<b>T-0956</b>	<b>SSS - Connections at Escalator Areas</b>	<b>Closed</b>	<b>CR</b>	<b>12/02/2013</b>	<b>12/12/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> On S1-7303 at the escalator areas refer to sketches CD RFI 177 SK1 & SK8 for items 1 to 6: 1) The elevation of the low beams cannot be determined as the information for the low slab is not shown on A1-2893 (SK2). Please supply the elevation for the low beam as shown on SK3, SK4, SK5 & SK7 2) Confirm the WT on top of the low W18x35 is required at (4) locations as shown. 3) Supply dimension. 4) Supply dimension. 5) Confirm the edge plate is to extend up to the top of low slab. 6) Supply a connection detail as 1/S1-7604 (SK7) does not represent the actual condition.						<b>ANSWER:</b> On S1-7303 at the escalator areas refer to sketches CD RFI 177 SK1 & SK8 for items 1 to 6: 1) The elevation of the low beams cannot be determined as the information for the low slab is not shown on A1-2893 (SK2). Please supply the elevation for the low beam as shown on SK3, SK4, SK5 & SK7 2) Confirm the WT on top of the low W18x35 is required at (4) locations as shown. 3) Supply dimension. 4) Supply dimension. 5) Confirm the edge plate is to extend up to the top of low slab. 6) Supply a connection detail as 1/S1-7604 (SK7) does not represent the actual condition.
<b>T-0957</b>	<b>SSS - Column Flange Plate Thickness Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>12/03/2013</b>	<b>12/13/2013</b>	<b>12/09/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Reference the sample location indicated on A/S1-4102 (CD RFI 186 SK1) and confirm the thicker flange plates of						<b>ANSWER:</b> Reference the sample location indicated on A/S1-4102 (CD RFI 186 SK1) and confirm the thicker flange





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<b>T-0960</b>	<b>SSS - Cast Node Weight and Center of Gravity</b>	<b>Closed</b>	<b>CR</b>	<b>12/03/2013</b>	<b>12/03/2013</b>	<b>12/04/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  As per drawing S-0007 Note SS-8 Skanska is preparing our erection procedures. In order to accurately incorporate the cast nodes into our calculations please provide the latest weight and center of gravity (in x, y, z) for each of the cast nodes.		<b>ANSWER:</b>  As per drawing S-0007 Note SS-8 Skanska is preparing our erection procedures. In order to accurately incorporate the cast nodes into our calculations please provide the latest weight and center of gravity (in x, y, z) for each of the cast nodes.				
<b>T-0961</b>	<b>SSS - Slab Opening Locations at Roof Park Level</b>	<b>Closed</b>	<b>CR</b>	<b>12/04/2013</b>	<b>12/14/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Reference A1-2902 and A1-2903 and provide the slab opening locations for the following items:  1) Provide the missing dimension for the slab opening size as indicated on SK1. 2) Confirm the dimensions noted on SK2 located the west side of the two slab openings. 3) Supply the dimension to locate the north edge of slab opening from grid D.8 as indicated in SK2. 4) Supply the dimension to locate the south edge of slab opening from grid E.6 as indicated in SK2.		<b>ANSWER:</b>  Reference A1-2902 and A1-2903 and provide the slab opening locations for the following items:  1) Provide the missing dimension for the slab opening size as indicated on SK1. 2) Confirm the dimensions noted on SK2 located the west side of the two slab openings. 3) Supply the dimension to locate the north edge of slab opening from grid D.8 as indicated in SK2. 4) Supply the dimension to locate the south edge of slab opening from grid E.6 as indicated in SK2.				
<b>T-0962</b>	<b>SSS - Slab Opening Locations at Ground Level</b>	<b>Closed</b>	<b>CR</b>	<b>12/04/2013</b>	<b>12/14/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Refer to A1-2862 and CD RFI 195 SK3 which indicate a slab opening which is not shown on S1-2302 and 3/S1-7004.  Please review SK1 through SK3 attached and clarify the slab opening requirement at the location indicated.		<b>ANSWER:</b>  Refer to A1-2862 and CD RFI 195 SK3 which indicate a slab opening which is not shown on S1-2302 and 3/S1-7004.  Please review SK1 through SK3 attached and clarify the slab opening requirement at the location indicated.				
<b>T-0963</b>	<b>SSS - Edge of Slab Clarifications at Second Level</b>	<b>Closed</b>	<b>CR</b>	<b>12/04/2013</b>	<b>12/14/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Reference CD RFI 196 SK1 to SK3 for edge of slab clarifications required at the second level near grid 11.E as		<b>ANSWER:</b>  Reference CD RFI 196 SK1 to SK3 for edge of slab clarifications required at the second level near grid				



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	<p>follows:</p> <p>1) The blue dimensions indicated on SK1 are per A1-2883. Please confirm these dimensions are to be used to locate the steel and edge plates on S1-2403.</p> <p>2) The blue dimensions indicated on SK2 are per A1-2883. Please confirm these dimensions are to be used to locate the steel and edge plates on 2/S1-7302.</p> <p>3) Please clarify the dimension discrepancy between A1-2883 and S1-7302 as indicated on SK2.</p> <p>4) Please clarify the dimension discrepancy between A1-2883 and S1-7302 as indicated on SK2.</p> <p>5) Confirm the built-up walls are 9" thick as indicated on CD RFI 196 SK2.</p> <p>6) Confirm the green lines indicated on SK3 represent the edge of slab on S1-2403.</p> <p>7) Confirm the purple lines indicated on SK3 represent the edge of slab on 2/S1-7302.</p> <p>8) The adjustment indicated on SK3 and A1-2883 is not shown on S1-2403. Please confirm the dimensions indicated on A1-2883 are correct.</p>					<p>11.E as follows:</p> <p>1) The blue dimensions indicated on SK1 are per A1-2883. Please confirm these dimensions are to be used to locate the steel and edge plates on S1-2403.</p> <p>2) The blue dimensions indicated on SK2 are per A1-2883. Please confirm these dimensions are to be used to locate the steel and edge plates on 2/S1-7302.</p> <p>3) Please clarify the dimension discrepancy between A1-2883 and S1-7302 as indicated on SK2.</p> <p>4) Please clarify the dimension discrepancy between A1-2883 and S1-7302 as indicated on SK2.</p> <p>5) Confirm the built-up walls are 9" thick as indicated on CD RFI 196 SK2.</p> <p>6) Confirm the green lines indicated on SK3 represent the edge of slab on S1-2403.</p> <p>7) Confirm the purple lines indicated on SK3 represent the edge of slab on 2/S1-7302.</p> <p>8) The adjustment indicated on SK3 and A1-2883 is not shown on S1-2403. Please confirm the dimensions indicated on A1-2883 are correct.</p>
T-0964	SSS - Elevator PE202 Dimension Clarifications	Closed	CR	12/04/2013	12/14/2013	12/19/2013
From: Webcor Construction LP		Gregory Kemerer				
REQUEST:		ANSWER:				
For elevator PE202, refer to sketches CD RFI 198 SK1 to SK3 for the following items:		For elevator PE202, refer to sketches CD RFI 198 SK1 to SK3 for the following items:				
1) Confirm the noted dimension should read 4'-8 ½" to match A1-2862 as indicated on SK1 in order to have the elevator posts align with the edge of slab.		1) Confirm the noted dimension should read 4'-8 ½" to match A1-2862 as indicated on SK1 in order to have the elevator posts align with the edge of slab.				
2) Confirm the noted dimension should read 8'- 2 ½" to match A1-2862.		2) Confirm the noted dimension should read 8'- 2 ½" to match A1-2862.				
3) Confirm the noted dimension should read 3'-7" to match A1-2862 to have the elevator posts align with the edge of slab.		3) Confirm the noted dimension should read 3'-7" to match A1-2862 to have the elevator posts align with the edge of slab.				
4) Confirm the slab opening is per A1-2882 and the elevator posts align with the edge of slab.		4) Confirm the slab opening is per A1-2882 and the elevator posts align with the edge of slab.				
5) Confirm the noted dimension should read 8'-6 ½" to mat A1-2892.		5) Confirm the noted dimension should read 8'-6 ½" to mat A1-2892.				
6) Confirm the noted dimension should read 4'-8 ½" to match A1-2892 to have the elevator posts align with edge of slab.		6) Confirm the noted dimension should read 4'-8 ½" to match A1-2892 to have the elevator posts align with edge of slab.				



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0965</b>	<b>SSS - Elevator SE401 Dimension Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>12/04/2013</b>	<b>12/14/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference CD RFI 200 SK1 which indicates that the slab opening dimensions required to locate elevator SE401 on 1/S1-7113 do not agree with A1-2864. Please confirm the dimensions shown on A1-2864 are correct and the SE401 elevator posts align with the edge of slab.			Reference CD RFI 200 SK1 which indicates that the slab opening dimensions required to locate elevator SE401 on 1/S1-7113 do not agree with A1-2864. Please confirm the dimensions shown on A1-2864 are correct and the SE401 elevator posts align with the edge of slab.			
<b>T-0966</b>	<b>SSS - Cruciform Column Splice</b>	<b>Closed</b>	<b>CR</b>	<b>12/04/2013</b>	<b>12/14/2013</b>	<b>12/11/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At multiple cruciform column locations (S1-4301 thru S1-4308 at Grids C & G), detail 2/S1-4350 has a 24" wide column flange flaring out to 36" flange at the connection to the cast node & transfer girder. It is the preference of the fabricator to utilize a CJP spliced flange plate where the flare (radius) starts. Please confirm this is acceptable.			At multiple cruciform column locations (S1-4301 thru S1-4308 at Grids C & G), detail 2/S1-4350 has a 24" wide column flange flaring out to 36" flange at the connection to the cast node & transfer girder. It is the preference of the fabricator to utilize a CJP spliced flange plate where the flare (radius) starts. Please confirm this is acceptable.			
<b>T-0967</b>	<b>Procedure for the removal of the level D bracing</b>	<b>Closed</b>	<b>01</b>	<b>12/05/2013</b>	<b>12/15/2013</b>	<b>12/09/2013</b>
<b>From:</b> Webcor Construction LP                      Michael Spillane						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Procedure for the removal of the level D bracing:			Procedure for the removal of the level D bracing:			
Webcor-Obayashi will review the 14 day compressive strength reports issued by the independent test lab (ISI) for the applicable pour area. In the review WOJV is to ensure that the "lower-bound" concrete strength exceeds 3000psi at 14 days. "Lower-bound" will be understood as the mean minus one standard deviation. If the calculated lower-bound strength <3000 psi, the bracing removal would not continue until results are received satisfying the lower bound criteria. Further, If any single compressive strength test is < 2500 psi, the bracing removal would not continue until results are received satisfying the minimum strength criteria.			Webcor-Obayashi will review the 14 day compressive strength reports issued by the independent test lab (ISI) for the applicable pour area. In the review WOJV is to ensure that the "lower-bound" concrete strength exceeds 3000psi at 14 days. "Lower-bound" will be understood as the mean minus one standard deviation. If the calculated lower-bound strength <3000 psi, the bracing removal would not continue until results are received satisfying the lower bound criteria. Further, If any single compressive strength test is < 2500 psi, the bracing removal would not continue until results are received satisfying the minimum strength criteria.			
Please confirm if this would be acceptable			Please confirm if this would be acceptable			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
T-0968	SSS - Light Column Cast Node Weld Prep	Closed	CR	12/06/2013	12/16/2013	12/16/2013
<div><div>From: Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Following discussions on the light column cast node weld prep, please confirm approval for use of joint B-U4a-GF in the flat position as a shop weld and that this joint is not prohibited under clause 2.18 - Prohibited Joints and Welds.			Following discussions on the light column cast node weld prep, please confirm approval for use of joint B-U4a-GF in the flat position as a shop weld and that this joint is not prohibited under clause 2.18 - Prohibited Joints and Welds.			





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0969</b>	<b>SSS - Filler Metal Usage on Group IV Grade HPS70W Material</b>	<b>Closed</b>	<b>CR</b>	<b>12/06/2013</b>	<b>12/16/2013</b>	<b>12/20/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Observation: Job specifications and Code AWS D1.1, Table 3.1, matching strength filler metal combinations for Group IV material, specifies for use an E91XTX for FCAW and F9XX for SAW process(s).</p> <p>Concern: ASTM A709GR 70W material hardening during welding (alloying up) as each weld layer is deposited (in 2" to 4" material thickness). An increased hardness value is expected and the actual concern is that, in this instance, the E91XX specified will create an overmatching filler metal condition during the welding process.</p> <p>Review: The AWS D1.1 2008 edition in table 3.1 for ASTM A709 Grade HPS70W specified a minimum of 70 ksi Yield Point and 90-110 ksi Tensile Range. In comparison, the AWS D1.1 2010 edition, a revision was made on this same material and the Tensile Range was dropped to 85 ksi minimum and maximum to remain at 110 (85-110).</p> <p>Research: Currently for seismic application, the filler metal companies have seismic testing certificates for E81XX and F8XX electrodes. The Tensile test range for AWS D1.8 requirements is 80ksi minimum, but the manufacturers' test results consistently come in at 88 to 95 ksi, which would meet the 85-110 ksi range for the material. The two manufacturers contacted, ESAB and Lincoln, are willing to do seismic testing (test data) for the purpose of supplying AWS D1.8 seismic certificates to meet the E91XX requirements. However, when reviewing the current test data from the manufacturer, the test tensile range is 90-110 ksi, but the results are 97-110 ksi.</p> <p>Conclusion: TMF and their welding consultants believe that starting with a near Minimum Tension ksi (under match in classification/specification) that allows the use of E81TXX or F8XX electrodes with current seismic certificates would be best for the welding of the A709 HPS70W material due to the 2-4" thickness in this application.</p> <p>Please confirm this proposal is acceptable.</p>			<p>Observation: Job specifications and Code AWS D1.1, Table 3.1, matching strength filler metal combinations for Group IV material, specifies for use an E91XTX for FCAW and F9XX for SAW process(s).</p> <p>Concern: ASTM A709GR 70W material hardening during welding (alloying up) as each weld layer is deposited (in 2" to 4" material thickness). An increased hardness value is expected and the actual concern is that, in this instance, the E91XX specified will create an overmatching filler metal condition during the welding process.</p> <p>Review: The AWS D1.1 2008 edition in table 3.1 for ASTM A709 Grade HPS70W specified a minimum of 70 ksi Yield Point and 90-110 ksi Tensile Range. In comparison, the AWS D1.1 2010 edition, a revision was made on this same material and the Tensile Range was dropped to 85 ksi minimum and maximum to remain at 110 (85-110).</p> <p>Research: Currently for seismic application, the filler metal companies have seismic testing certificates for E81XX and F8XX electrodes. The Tensile test range for AWS D1.8 requirements is 80ksi minimum, but the manufacturers' test results consistently come in at 88 to 95 ksi, which would meet the 85-110 ksi range for the material. The two manufacturers contacted, ESAB and Lincoln, are willing to do seismic testing (test data) for the purpose of supplying AWS D1.8 seismic certificates to meet the E91XX requirements. However, when reviewing the current test data from the manufacturer, the test tensile range is 90-110 ksi, but the results are 97-110 ksi.</p> <p>Conclusion: TMF and their welding consultants believe that starting with a near Minimum Tension ksi (under match in classification/specification) that allows the use of E81TXX or F8XX electrodes with current seismic certificates would be best for the welding of the A709 HPS70W material due to the 2-4" thickness in this application.</p> <p>Please confirm this proposal is acceptable.</p>			







<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0970.1</b>	<b>SSS - Pretensioned Rods at Moment Columns</b>	<b>Closed</b>	<b>CR</b>	<b>01/16/2014</b>	<b>01/26/2014</b>	<b>01/28/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  Following the response to RFI T-0970 and after further review of #3 & 10, we agree the rods can be pretensioned from the bottom. Candraft have run a sanity check at several locations to confirm there is adequate clearance for the tensioning device and will continue to do so as other locations are detailed. Any interference will be addressed in future RFIs.  Please confirm it is acceptable to use a standard flat washer in lieu of the plate washer as the holes are not oversized and this will also allow for easier workability during the pretensioning operation from the bottom (see Dyson catalog cut attached). If this is acceptable the 24" dimension indicated from bottom continuity plate to top of built up T & TT (on 3/S1-5050) and the 6x6x2" plate washer under the top nut will not require to be changed (as per #3, 10 & 14 RFI T-0970).  Please confirm this proposal is acceptable.						
						<b>ANSWER:</b>  Following the response to RFI T-0970 and after further review of #3 & 10, we agree the rods can be pretensioned from the bottom. Candraft have run a sanity check at several locations to confirm there is adequate clearance for the tensioning device and will continue to do so as other locations are detailed. Any interference will be addressed in future RFIs.  Please confirm it is acceptable to use a standard flat washer in lieu of the plate washer as the holes are not oversized and this will also allow for easier workability during the pretensioning operation from the bottom (see Dyson catalog cut attached). If this is acceptable the 24" dimension indicated from bottom continuity plate to top of built up T & TT (on 3/S1-5050) and the 6x6x2" plate washer under the top nut will not require to be changed (as per #3, 10 & 14 RFI T-0970). Please confirm this proposal is acceptable.
<b>T-0971</b>	<b>SSS - Column Side Plates Dimension Increase</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2013</b>	<b>12/09/2013</b>	<b>12/11/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  Please refer to drawing S1-2203 and S1-5050.  On S1-2203 at grids 9/C refer to sketches CD RFI 161 SK1 & SK2 regarding anchor bolts at column side plates. For the column side plates per detail 4/S1-5050 and anchor bolts it is not possible to insert the nuts & plate washers for the anchor bolts with the 3'-0" side plate dimension.  Confirm it is acceptable to increase the noted dimension to 3'-5" or supply an alternate solution.						
						<b>ANSWER:</b>  Please refer to drawing S1-2203 and S1-5050.  On S1-2203 at grids 9/C refer to sketches CD RFI 161 SK1 & SK2 regarding anchor bolts at column side plates. For the column side plates per detail 4/S1-5050 and anchor bolts it is not possible to insert the nuts & plate washers for the anchor bolts with the 3'-0" side plate dimension.  Confirm it is acceptable to increase the noted dimension to 3'-5" or supply an alternate solution.
<b>T-0972</b>	<b>SSS - Stair Post Base Detail at GL 11/D</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2013</b>	<b>12/09/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  On S1-2303 near grids 11/D at the Stair post base refer to						
						<b>ANSWER:</b>  On S1-2303 near grids 11/D at the Stair post base



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T-0973	<div>SSS - Transfer Girder Kicker Brace Connection</div> <div>From: Webcor Construction LPGregory Kemerer</div> <div>REQUEST: For the angle brace connection per detail 5/S1-5015 see sketches CD RFI 061 SK1 &amp; SK2 for items noted below.  1) For the Transfer Girder bottom flange bracing connection, confirm if bracing is required when the dimension from the bottom flange of the framing member to the top of the transfer girder bottom flange is less than 1'-3". See SK2 grid line 3/D.4 as an example.  If bracing is required please provide typical details.</div>	Closed	CR	12/09/2013	12/19/2013	12/20/2013
	<div>sketches CD RFI 174 SK1 &amp; SK2 for items 1 &amp; 2:  1) Skanska (Candraft) have reviewed the Architectural &amp; Structural drawings and have been unable to verify the offset dimension of the built up WT from the center of the W27x84. The built up WT is shown on SK2 as per the revit model, please confirm this is correct or provide the required dimensions.  2) If the location is correct confirm it is acceptable to shop weld the BU WT to the supported beam and field weld the remaining piece to the supported beam.</div>					<div>refer to sketches CD RFI 174 SK1 &amp; SK2 for items 1 &amp; 2:  1) Skanska (Candraft) have reviewed the Architectural &amp; Structural drawings and have been unable to verify the offset dimension of the built up WT from the center of the W27x84. The built up WT is shown on SK2 as per the revit model, please confirm this is correct or provide the required dimensions.  2) If the location is correct confirm it is acceptable to shop weld the BU WT to the supported beam and field weld the remaining piece to the supported beam.</div> <div>ANSWER: For the angle brace connection per detail 5/S1-5015 see sketches CD RFI 061 SK1 &amp; SK2 for items noted below.  1) For the Transfer Girder bottom flange bracing connection, confirm if bracing is required when the dimension from the bottom flange of the framing member to the top of the transfer girder bottom flange is less than 1'-3". See SK2 grid line 3/D.4 as an example.  If bracing is required please provide typical details.</div>

<i>Number</i>	<i>Subject</i>	<i>Status</i>	<i>Choice Required</i>	<i>Date Created</i>	<i>Date Required</i>	<i>Date Answered</i>
<b>T-0974</b>	<b>SSS - Pin Details in Drawing 1/S1-5017</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>12/11/2013</b>
<div> <div> <b>From:</b> Webcor Construction LP Gregory Kemerer </div> <div> <b>REQUEST:</b>  For Drag connections per detail 1/S1-5017 refer to sketches CD RFI 123 SK1 &amp; SK2 for the following items noted:   1) Confirm the size of the hole required thru the beam web, web stiffeners and shear plates is the diameter of pin +1/32". Confirm if any additional tolerance is allowed for hot dipped galvanized pins.  3) Confirm flanges can be cut flush to the beam web. Note that the flanges need to be cut flush only to the web stiffeners for erection access purposes.  4) Confirm if a radius is required when cutting the flange flush to the beam. If required confirm a radius of 1-1/2" is acceptable.  5) On RFI T-0737 Skanska requested to provide a cotter pin to further secure the nuts from backing off. Please confirm it is acceptable to provide one nut with the cotter pin as detailed on SK2.  6) Confirm the material grade for the pins and nuts is A668 Class M.  7) Confirm all pins and nuts are to be hot dipped galvanized. </div> <div> <b>ANSWER:</b>  For Drag connections per detail 1/S1-5017 refer to sketches CD RFI 123 SK1 &amp; SK2 for the following items noted:   1) Confirm the size of the hole required thru the beam web, web stiffeners and shear plates is the diameter of pin +1/32". Confirm if any additional tolerance is allowed for hot dipped galvanized pins.  3) Confirm flanges can be cut flush to the beam web. Note that the flanges need to be cut flush only to the web stiffeners for erection access purposes.  4) Confirm if a radius is required when cutting the flange flush to the beam. If required confirm a radius of 1-1/2" is acceptable.  5) On RFI T-0737 Skanska requested to provide a cotter pin to further secure the nuts from backing off. Please confirm it is acceptable to provide one nut with the cotter pin as detailed on SK2.  6) Confirm the material grade for the pins and nuts is A668 Class M.  7) Confirm all pins and nuts are to be hot dipped galvanized. </div> </div>						
<b>T-0974.1</b>	<b>SSS - Nut Material Grade</b>	<b>Closed</b>	<b>CR</b>	<b>03/05/2014</b>	<b>03/15/2014</b>	<b>03/06/2014</b>
<div> <div> <b>From:</b> Webcor Construction LP Stephanie Azzolino </div> <div> <b>REQUEST:</b>  The contract documents do not contain a specific material grade for the nuts for large diameter pin connections (6, 7 and 8" diameter pins). In SK RFI 169A (T-0974), our detailer asked for clarification on the material grade for these nuts. It was confirmed that A668 Cl. M would be acceptable. Given the function of these nuts are to hold the pin in place and not apply a clamping force to the assembly, we do not consider that these nuts need to be a high strength forged material. Please refer to AISC Table 15-8 which shows a thin cap plates for pins overs 10" in diameter which reinforces this position.   Please confirm it is acceptable to use A572 Gr.50 plate to fabricate these nuts. </div> <div> <b>ANSWER:</b>  The contract documents do not contain a specific material grade for the nuts for large diameter pin connections (6, 7 and 8" diameter pins). In SK RFI 169A (T-0974), our detailer asked for clarification on the material grade for these nuts. It was confirmed that A668 Cl. M would be acceptable. Given the function of these nuts are to hold the pin in place and not apply a clamping force to the assembly, we do not consider that these nuts need to be a high strength forged material. Please refer to AISC Table 15-8 which shows a thin cap plates for pins overs 10" in diameter which reinforces this position.   Please confirm it is acceptable to use A572 Gr.50 plate to fabricate these nuts. </div> </div>						





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-0976</b>	<b>SSS - Transfer Girder Kicker Connection Conflicts</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Please refer to drawing 5/S1-5015.  For the Transfer Girder angle connections see sketches CD RFI 063 SK1 to SK4 for items 1, 2, 3 & 4 noted below.  1) The kicker angle fouls the vertical stiffener, this is typical at similar locations. We propose notching the leg of the fouling angle and using a two bolt connection in lieu of welding or provide a typical solution. 2) Due to welding access issues we propose to use a two bolt connection, typical at similar locations. Confirm this is acceptable. 3) The kicker fouls the stiffener and the kicker gusset is too close to the stiffener for welding access. Similar conditions occur at other locations on the Ground Level. Please provide a typical solution. 4) The kicker gusset fouls the stiffener plate. Similar conditions occur at other locations on the Ground Level. Confirm it is acceptable to use the stiffener as the kicker gusset and increase the gusset thickness at the other end to match or provide an alternative detail.						
						<b>ANSWER:</b> Please refer to drawing 5/S1-5015.  For the Transfer Girder angle connections see sketches CD RFI 063 SK1 to SK4 for items 1, 2, 3 & 4 noted below.  1) The kicker angle fouls the vertical stiffener, this is typical at similar locations. We propose notching the leg of the fouling angle and using a two bolt connection in lieu of welding or provide a typical solution. 2) Due to welding access issues we propose to use a two bolt connection, typical at similar locations. Confirm this is acceptable. 3) The kicker fouls the stiffener and the kicker gusset is too close to the stiffener for welding access. Similar conditions occur at other locations on the Ground Level. Please provide a typical solution. 4) The kicker gusset fouls the stiffener plate. Similar conditions occur at other locations on the Ground Level. Confirm it is acceptable to use the stiffener as the kicker gusset and increase the gusset thickness at the other end to match or provide an alternative detail.
<b>T-0977</b>	<b>SSS - Handling Holes at Basket Column Pins</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> The clevis pins for basket columns detailed on S1-5133 do not provide means to safely handle the material during manufacturing, coating, and field assembly. To aid in these processes, please advise if it is acceptable to drill and tap 1-8 x 2" deep in the center of the pins at both ends.						
						<b>ANSWER:</b> The clevis pins for basket columns detailed on S1-5133 do not provide means to safely handle the material during manufacturing, coating, and field assembly. To aid in these processes, please advise if it is acceptable to drill and tap 1-8 x 2" deep in the center of the pins at both ends.
<b>T-0977.1</b>	<b>SSS - Handling Holes at Basket Column Pins</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/02/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> The clevis pins for basket columns detailed on S1-5133 do						
						<b>ANSWER:</b> The clevis pins for basket columns detailed on S1-



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T-0978	<div>SSS - Clevis Pin Material at Roof and Bus Deck</div> <div>From: Webcor Construction LP      Robert Kjome</div> <div>REQUEST: Reference drawing S-0007, General Note SS-2, which requires that all clevis pins meet ASTM A668 Class M. Oregon Iron Works is requesting approval to supply these pins from round bar AISI 4340 NQ&amp;T (normalized, quenched, and tempered), produced to ASTM A434 grade BD. Please confirm if this is an acceptable material for clevis pins at the following locations: 1. Roof Level pins for type 71 and 72 castings shown on sheets S1-5131, S1-5132, S1-5133. 2. Bus Deck pins detailed on S1-5017 Detail 1 for Type 2M connections.</div>	Closed	01	12/09/2013	12/09/2013	04/01/2014
T-0979	<div>SSS - Curved Connection Detail at Light Column</div> <div>From: Webcor Construction LP      Gregory Kemerer</div> <div>REQUEST: On S1-2305 at grids 23/E, refer to sketch CD RFI 103 SK1 and supply a detail showing how to splice the curved W27x84 beams.</div>	Closed	CR	12/09/2013	12/09/2013	12/11/2013



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0979.1</b>	<b>SSS - Curved Connection Detail at Light Column</b>	<b>Closed</b>	<b>CR</b>	<b>01/17/2014</b>	<b>01/27/2014</b>	<b>02/03/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The response to WOJV RFI T-0979 indicated that the curved W27x84 beams shown on S1-2305 at grid 23/E are to be connected together with a single shear plate connection per 1/S1-5011.			The response to WOJV RFI T-0979 indicated that the curved W27x84 beams shown on S1-2305 at grid 23/E are to be connected together with a single shear plate connection per 1/S1-5011.			
The referenced detail shows a beam to beam "T" connection, rather than two rolled shapes butting up to each other. Please clarify how the connection shown on 1/S1-5011 is to be applied to curved beam connections.			The referenced detail shows a beam to beam "T" connection, rather than two rolled shapes butting up to each other. Please clarify how the connection shown on 1/S1-5011 is to be applied to curved beam connections.			





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0980</b>	<b>SSS - BU Girders Connection Clarifications at Ground Level</b>	<b>Closed</b>	<b>01</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>The W40x503 beams along grids C &amp; G on the Ground Level have been substituted with BU beams per RFI # T-0704.1. This changes the flange copes in details 3 &amp; 7/S1-4350. Please refer to attached CD RFI 162 SK1 &amp; SK2 for the following items:</p> <p>1) Please confirm it is acceptable to extend the web plate above the BU beam and cut the top flange plate flush to the web plate as shown. Confirm the CJP weld indicated is acceptable to weld the top flange to the web plate. The web to flange fillet welds per RFI T-0704.1 will be applied beyond the shown CJP welds.</p> <p>2) Confirm it is acceptable to stop the bottom flange plate of the BU WT short as shown, extend the web plate of the BU WT to the web plate of the BU beam and weld as shown. The web to flange fillet welds per RFI # T- 0704.1 will be applied beyond the shown CJP welds.</p> <p>3) Confirm it is acceptable to have a continuous 4" vertical bolt spacing in lieu of the pattern interruption as shown in detail 3/S1-4350 to avoid cutting the bottom flange of the BU beam. This may mean that the holes for the 1 1/2" dia. bolts near the WT to BU beam web weld will have to be drilled after the weld is made.</p> <p>4) Confirm it is acceptable to have a continuous 4" vertical bolt spacing in lieu of the pattern interruption as shown in detail 7/S1-4350 to avoid the bolts fouling the web to flange fillet welds. This may mean that the holes for the 1 1/2" dia. bolts near the WT to BU beam web weld will have to be drilled after the weld is made.</p>			<p>The W40x503 beams along grids C &amp; G on the Ground Level have been substituted with BU beams per RFI # T-0704.1. This changes the flange copes in details 3 &amp; 7/S1-4350. Please refer to attached CD RFI 162 SK1 &amp; SK2 for the following items:</p> <p>1) Please confirm it is acceptable to extend the web plate above the BU beam and cut the top flange plate flush to the web plate as shown. Confirm the CJP weld indicated is acceptable to weld the top flange to the web plate. The web to flange fillet welds per RFI T-0704.1 will be applied beyond the shown CJP welds.</p> <p>2) Confirm it is acceptable to stop the bottom flange plate of the BU WT short as shown, extend the web plate of the BU WT to the web plate of the BU beam and weld as shown. The web to flange fillet welds per RFI # T- 0704.1 will be applied beyond the shown CJP welds.</p> <p>3) Confirm it is acceptable to have a continuous 4" vertical bolt spacing in lieu of the pattern interruption as shown in detail 3/S1-4350 to avoid cutting the bottom flange of the BU beam. This may mean that the holes for the 1 1/2" dia. bolts near the WT to BU beam web weld will have to be drilled after the weld is made.</p> <p>4) Confirm it is acceptable to have a continuous 4" vertical bolt spacing in lieu of the pattern interruption as shown in detail 7/S1-4350 to avoid the bolts fouling the web to flange fillet welds. This may mean that the holes for the 1 1/2" dia. bolts near the WT to BU beam web weld will have to be drilled after the weld is made.</p>			





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Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-0982	SSS - Elevator Rail Support Connection Clarifications	Closed	01	12/09/2013	12/19/2013	01/10/2014
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Please refer to Elevator Rail Support drawings S1-7130 through S1-7139 and provide clarification on the following: 1) At locations where the HSS members span two equally sized support beams, please confirm connection detail 1/S1-7630 typically applies and the HSS member is to be located direction under the beams. Refer to SK1, SK2, SK5, SK6, and SK7 for reference. 2) At locations where the lower HSS member spans two unequally sized beams, it is assumed that the HSS member will connect to the shallower beam per detail 1/S1-7630. Please confirm and provide a typical connection detail for the HSS member to the deeper beam. Reference SK1, SK2, SK5, SK6 for reference. 3) Confirm the HSS beams indicated on SK1, SK2, and SK7 are located flush with the top of slab per 1/S1-7630. 4) Confirm the plates indicated on SK3 & SK4 may be cut as shown on details 1&4/S1-7630 to achieve an effective weld along the full length. 5) Provide a connection detail for the HSS 12x6 to the W21, W24, and W36 beams at the locations indicated on SK7. 6) Provide the elevation of the lower HSS 12x6 indicated on SK RFI 239 SK2 and the connection details required at each end. 7) Confirm the elevation of the W21s indicated on SK5. 8) Provide a connection detail for upper and lower HSS12x6 to HSS12x6 at locations with no floor slab on SK5. 9) Provide a connection detail for upper and lower HSS12x6 to W16 at locations with no floor slab on SK5. 10) Provide a connection detail for HSS12x6 at the W21 indicated on SK5 where there is no edge plate as shown on detail 1//S1-7130.					Please refer to Elevator Rail Support drawings S1-7130 through S1-7139 and provide clarification on the following: 1) At locations where the HSS members span two equally sized support beams, please confirm connection detail 1/S1-7630 typically applies and the HSS member is to be located direction under the beams. Refer to SK1, SK2, SK5, SK6, and SK7 for reference. 2) At locations where the lower HSS member spans two unequally sized beams, it is assumed that the HSS member will connect to the shallower beam per detail 1/S1-7630. Please confirm and provide a typical connection detail for the HSS member to the deeper beam. Reference SK1, SK2, SK5, SK6 for reference. 3) Confirm the HSS beams indicated on SK1, SK2, and SK7 are located flush with the top of slab per 1/S1-7630. 4) Confirm the plates indicated on SK3 & SK4 may be cut as shown on details 1&4/S1-7630 to achieve an effective weld along the full length. 5) Provide a connection detail for the HSS 12x6 to the W21, W24, and W36 beams at the locations indicated on SK7. 6) Provide the elevation of the lower HSS 12x6 indicated on SK RFI 239 SK2 and the connection details required at each end. 7) Confirm the elevation of the W21s indicated on SK5. 8) Provide a connection detail for upper and lower HSS12x6 to HSS12x6 at locations with no floor slab on SK5. 9) Provide a connection detail for upper and lower HSS12x6 to W16 at locations with no floor slab on SK5. 10) Provide a connection detail for HSS12x6 at the W21 indicated on SK5 where there is no edge plate as shown on detail 1//S1-7130.	
T-0983	SSS - ST201 and PE201 Anchor Bolt Clarifications	Closed	CR	12/09/2013	12/19/2013	12/27/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST:		ANSWER:				



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
	<p>Refer to CD RFI 203 SK1 to SK3 requesting clarification on Stair ST201 and Elevator PE201 per the following:</p> <p>1) Refer to detail 4/S1-7605 and CD RFI 203 SK1 indicating the ½" dimension between the washers and the HSS column. When considering the 5/16" fillet weld at this location, there is only 3/16" clear between the plate washers and the HSS column, which is not sufficient to allow for anchor bolt as-built variations to suit the 13/16" dia. oversize holes. Please confirm it is acceptable to increase the 2" typ. dimension indicated to 2 ½".</p> <p>2) It is not clear what is meant by "SIM." Please confirm detail 4/S1-7605 may be applied at all HSS columns at Stair 201/Elevator PE201.</p> <p>3) Please provide an anchor bolt detail for the noted two WF columns.</p> <p>4) Confirm the underside grout elevation is 23.42'.</p> <p>5) Confirm the underside grout elevation is 24.08' (3 locations).</p> <p>6) Confirm the underside grout elevation is 22.42' (4 locations).</p> <p>7) Confirm the underside grout elevation is 22.92'.</p> <p>8) Confirm the underside grout elevation is 22.42'.</p> <p>9) Confirm the underside grout elevation is 22.92'.</p> <p>10) Provide the underside grout elevation at the location indicated.</p>					<p>Refer to CD RFI 203 SK1 to SK3 requesting clarification on Stair ST201 and Elevator PE201 per the following:</p> <p>1) Refer to detail 4/S1-7605 and CD RFI 203 SK1 indicating the ½" dimension between the washers and the HSS column. When considering the 5/16" fillet weld at this location, there is only 3/16" clear between the plate washers and the HSS column, which is not sufficient to allow for anchor bolt as-built variations to suit the 13/16" dia. oversize holes. Please confirm it is acceptable to increase the 2" typ. dimension indicated to 2 ½".</p> <p>2) It is not clear what is meant by "SIM." Please confirm detail 4/S1-7605 may be applied at all HSS columns at Stair 201/Elevator PE201.</p> <p>3) Please provide an anchor bolt detail for the noted two WF columns.</p> <p>4) Confirm the underside grout elevation is 23.42'.</p> <p>5) Confirm the underside grout elevation is 24.08' (3 locations).</p> <p>6) Confirm the underside grout elevation is 22.42' (4 locations).</p> <p>7) Confirm the underside grout elevation is 22.92'.</p> <p>8) Confirm the underside grout elevation is 22.42'.</p> <p>9) Confirm the underside grout elevation is 22.92'.</p> <p>10) Provide the underside grout elevation at the location indicated.</p>
<b>T-0984</b>	<b>SSS - W33 Connection at GL 11</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>12/20/2013</b>
<b>From:</b> Webcor Construction LP		Gregory Kemerer				
<b>REQUEST:</b>		<b>ANSWER:</b>				
<p>On S1-2303 there are two W33x118 beams between grids D.8/E.2 that connect to the stepped Transfer Girder along grid line 11. These connections should be typical double angle shear connections, but due to the location of the stiffeners for the Moment frame column cap/base plate there is a fouling issue. Please see the following questions</p> <p>below:</p> <p>1) Please verify a partial full depth shear plate connection similar to detail 2/S1-5011 can be provided at these locations in lieu of the double angle shear connections.</p>		<p>On S1-2303 there are two W33x118 beams between grids D.8/E.2 that connect to the stepped Transfer Girder along grid line 11. These connections should be typical double angle shear connections, but due to the location of the stiffeners for the Moment frame column cap/base plate there is a fouling issue. Please see the following questions</p> <p>below:</p> <p>1) Please verify a partial full depth shear plate connection similar to detail 2/S1-5011 can be provided at these locations in lieu of the double angle shear</p>				



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
T-0985	<p>The shear plate cannot be full depth as it will foul the bolts connecting the Transfer Girder bottom flange to the column cap/base plate. See CD RFI 204 SK1 to SK3.</p> <p>2) If a shear plate connection is acceptable at these locations, please verify plate thickness &amp; welding per 2/S15011. See CD RFI 204 SK1 to SK3.</p> <p>3) The numbers of bolts in a single row per the schedule on 2/S1-5011 cannot be provided if bolt spacing and edge distance are to be maintained due to the difference in elevation between the Transfer girder and W33 beam. Please verify if it is acceptable to provide a double row with a total of 12 - 1" A325N bolts. See CD RFI 204 SK1 to SK3.</p> <p>4) On S1-2303 there is bracing shown at the end of the W33 beams to the Transfer Girder. These brace members cannot be provided as the bottom of the W33 beam and the bottom of the Transfer Girder flange nearly line up, there will be nothing to connect the braces to. Please verify that the braces shown per plan are not required at these locations. See CD RFI 204 SK1 &amp; SK2.</p> <p><b>SSS - Elevator Connection Clarifications</b></p> <p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Refer to detail 6/S1-7630 and advise how the vertical posts are intended to attach to the double horizontal HSS10x10 as no bolts or welds are indicated.</p>	Closed	01	12/09/2013	12/19/2013	12/26/2013
						<p><b>ANSWER:</b></p> <p>Refer to detail 6/S1-7630 and advise how the vertical posts are intended to attach to the double horizontal HSS10x10 as no bolts or welds are indicated.</p>



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-0986</b>	<b>SSS - Connection Clarifications at Bus Deck Level</b>	<b>Closed</b>	<b>01</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>12/20/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At a sample location on S1-2503 near grid 10.1/C, refer to CD RFI 197 SK1 & SK2 requesting clarification on the following:			At a sample location on S1-2503 near grid 10.1/C, refer to CD RFI 197 SK1 & SK2 requesting clarification on the following:			
1) The double angle connection per S1-5010 for the W12x40's fouls the connection from the W30x99 to the column. Confirm it is acceptable to connect the W12x40's to the W30x99 with shear plate connections per S1- 5011.			1) The double angle connection per S1-5010 for the W12x40's fouls the connection from the W30x99 to the column. Confirm it is acceptable to connect the W12x40's to the W30x99 with shear plate connections per S1- 5011.			
2) This condition occurs at grids C/9.9, G9.9, G/10.1, C/19.9, C/20.1, G/19.9, and G/20.1. Please confirm the solution for item 1 may be applied at these locations.			2) This condition occurs at grids C/9.9, G9.9, G/10.1, C/19.9, C/20.1, G/19.9, and G/20.1. Please confirm the solution for item 1 may be applied at these locations.			
<b>T-0987</b>	<b>SSS - Elevator PE202 Dimension and Connection Clarifications</b>	<b>Closed</b>	<b>01</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>01/09/2014</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Refer to CD RFI 199 SK1 requesting clarifications for dimensions and connections at Elevator PE202 as follows:			Refer to CD RFI 199 SK1 requesting clarifications for dimensions and connections at Elevator PE202 as follows:			
1) Detail 8/S1-5004 shows the edge of slab is to be 1'-0" from the toe of the WF beam, but based on the dimensions shown on S1-2502, the 1'-0" requirement is met only on the west side of the elevator opening. The north, south, and east sides do not meet the 1'-0" requirement. Confirm the dimensions to locate the elevator opening WF perimeter beams are correct as indicated on S1-2502.			1) Detail 8/S1-5004 shows the edge of slab is to be 1'-0" from the toe of the WF beam, but based on the dimensions shown on S1-2502, the 1'-0" requirement is met only on the west side of the elevator opening. The north, south, and east sides do not meet the 1'-0" requirement. Confirm the dimensions to locate the elevator opening WF perimeter beams are correct as indicated on S1-2502.			
2) Please supply the missing dimensions to locate the HSS 12x6x1/2 on four (4) sides of the elevator opening.			2) Please supply the missing dimensions to locate the HSS 12x6x1/2 on four (4) sides of the elevator opening.			
3) Please clarify how the HSS12x6x1/2 perimeter members are supported and connected to each other at the corners.			3) Please clarify how the HSS12x6x1/2 perimeter members are supported and connected to each other at the corners.			
4) Confirm edge plate per 8/S1-5000 is required on 4 sides of the elevator opening as none are indicated on detail 8/S1-5004.			4) Confirm edge plate per 8/S1-5000 is required on 4 sides of the elevator opening as none are indicated on detail 8/S1-5004.			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0987.1</b>	<b>SSS - Elevator PE202 Dimension and Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/31/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 199.1 SK1 & SK2 for items 1 to 4: 1) The noted dimension per S1-2502 places the beam inside the slab opening. Please provide the location of the beam to suit the slab opening per A1-2892 (SK1). 2) Confirm dimensions per T-0987 are still correct with revised opening location/size on A1-2892 or supply new dimensions. 3) Confirm dimensions per T-0987 are still correct with revised opening location/size on A1-2892 or supply new dimensions. 4) Confirm the connection details as shown are correct at the noted locations.			See attached CD RFI # 199.1 SK1 & SK2 for items 1 to 4: 1) The noted dimension per S1-2502 places the beam inside the slab opening. Please provide the location of the beam to suit the slab opening per A1-2892 (SK1). 2) Confirm dimensions per T-0987 are still correct with revised opening location/size on A1-2892 or supply new dimensions. 3) Confirm dimensions per T-0987 are still correct with revised opening location/size on A1-2892 or supply new dimensions. 4) Confirm the connection details as shown are correct at the noted locations.			
<b>T-0988</b>	<b>SSS - W21 Full Depth Connection at Transfer Girder</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
On S1-2303 there is a W21x50 beam just south of grid D that connects to the Transfer Girder along grid line 10.1. There is a similar W21x50 along 10.1 north of grid F that is shown with a full depth shear connection to the Transfer Girder.  Please advise if the W21x50 near grid D should also be a full depth shear plate connection. See CD RFI 207 SK1.			On S1-2303 there is a W21x50 beam just south of grid D that connects to the Transfer Girder along grid line 10.1. There is a similar W21x50 along 10.1 north of grid F that is shown with a full depth shear connection to the Transfer Girder.  Please advise if the W21x50 near grid D should also be a full depth shear plate connection. See CD RFI 207 SK1.			
<b>T-0988.1</b>	<b>SSS - W21 to Transfer Girder Connection</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/13/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per the response to Webcor RFI # T-0988 (SK RFI # 258), on S1-2303 the double angle beam connection near grids 10.1/D will foul stiffener plates at the Transfer girders, same as near grid 10.1/F. On sketch CD RFI 207.1 SK1 shows the stiffener plates at this location on line 10.1 north of grid line D. Please verify a shear plate can be used as request ed or provide an alternate connection.			Per the response to Webcor RFI # T-0988 (SK RFI # 258), on S1-2303 the double angle beam connection near grids 10.1/D will foul stiffener plates at the Transfer girders, same as near grid 10.1/F. On sketch CD RFI 207.1 SK1 shows the stiffener plates at this location on line 10.1 north of grid line D. Please verify a shear plate can be used as request ed or provide an alternate connection.			





<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-0989</b>	<b>SSS - Beam to Column Connection at Bus Deck</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>12/20/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> 1) On S1-2503 at grids 11/C "H < D" indicating that detail 5/S1-5011 would apply. Based on the "H" and "D" dimension indicated on SK2, please verify detail 4/S1-5011 can be used at this condition as noted on sketches CD RFI 208 SK1 & SK2. 2) Based on a review of the project conditions, please verify that detail 5/S1-5011 will only be applied at grids 20.1/C, 20.1/G, 21/C, 21/G, 22/C & 22/G at the Bus deck level per note # 3 on 4/S1-5011.						<b>ANSWER:</b> 1) On S1-2503 at grids 11/C "H < D" indicating that detail 5/S1-5011 would apply. Based on the "H" and "D" dimension indicated on SK2, please verify detail 4/S1-5011 can be used at this condition as noted on sketches CD RFI 208 SK1 & SK2. 2) Based on a review of the project conditions, please verify that detail 5/S1-5011 will only be applied at grids 20.1/C, 20.1/G, 21/C, 21/G, 22/C & 22/G at the Bus deck level per note # 3 on 4/S1-5011.
<b>T-0990</b>	<b>SSS - Skewed Beam to Beam Connection</b>	<b>Closed</b>	<b>01</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b> Reference sketches CD RFI 193 SK1 & SK2 indicating one specific location where the bolt spacing provided in detail 8/S1-5010 will not work as the bolts will foul each other. In the specific case shown on SK2, the "H1" dimension will need to be increased to 7 1/2" to avoid the fouling issue. Please confirm it is typically acceptable to increase the "H" or "H1" dimensions as required to allow sufficient clearance between the bolts for installation and tightening. If not, supply an alternate solution.  NOTE: RFI #T-0976 item 4 requested permission to typically move the shear plate to the opposite side of the skewed beam from what is shown in 8/S1-5010 to allow erection access for the skewed beams.						<b>ANSWER:</b> Reference sketches CD RFI 193 SK1 & SK2 indicating one specific location where the bolt spacing provided in detail 8/S1-5010 will not work as the bolts will foul each other. In the specific case shown on SK2, the "H1" dimension will need to be increased to 7 1/2" to avoid the fouling issue. Please confirm it is typically acceptable to increase the "H" or "H1" dimensions as required to allow sufficient clearance between the bolts for installation and tightening. If not, supply an alternate solution.  NOTE: RFI #T-0976 item 4 requested permission to typically move the shear plate to the opposite side of the skewed beam from what is shown in 8/S1-5010 to allow erection access for the skewed beams.
<b>T-0991</b>	<b>SSS - Tapered Girder Flange Plate Connection</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2013</b>	<b>12/19/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> On S1-2603 at grids 9.9/B, 10.1/B, 9.9/H & 10.1/H shown on sketches CD RFI # 211 SK1 & SK2, the spacing for the Tapered girder flange plates per detail 7/S1-5032 will foul the W24x68 beam web.  Please verify the bolt spacing can be adjusted to 5 1/4" to clear the incoming W24 beam webs as indicated on CD						<b>ANSWER:</b> On S1-2603 at grids 9.9/B, 10.1/B, 9.9/H & 10.1/H shown on sketches CD RFI # 211 SK1 & SK2, the spacing for the Tapered girder flange plates per detail 7/S1-5032 will foul the W24x68 beam web.  Please verify the bolt spacing can be adjusted to 5 1/4" to clear the incoming W24 beam webs as





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	RFI 211 SK2.					indicated on CD RFI 211 SK2.
<b>T-0992</b>	<b>BGP - Column at GL 16.9/G Coupler Stagger</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/17/2013</b>	<b>12/12/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> Please refer to drawing S1-3304 and S1-3301.  Detail 2/S1-3301 requires the couplers of adjacent column vertical bars to be staggered with a vertical distance of 24" or more; however, at gridlines 16.9/G there is a column dowel that should have been a shorter bar (L) but was installed as a longer bar (H) and casted in the mat foundation concrete. This does not allow for the stagger pattern as required. See the attached sketch SK-RFI-114 for more details. Gerdau proposes to leave the bar as-is.  Please confirm if this is acceptable.						<b>ANSWER:</b> Please refer to drawing S1-3304 and S1-3301.  Detail 2/S1-3301 requires the couplers of adjacent column vertical bars to be staggered with a vertical distance of 24" or more; however, at gridlines 16.9/G there is a column dowel that should have been a shorter bar (L) but was installed as a longer bar (H) and casted in the mat foundation concrete. This does not allow for the stagger pattern as required. See the attached sketch SK-RFI-114 for more details. Gerdau proposes to leave the bar as-is.  Please confirm if this is acceptable.
<b>T-0993</b>	<b>SSS - Deck Support at Columns</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Detail 9/S1-5000 provides a typical detail for slab edge supports. However, no detail is provided for slab edge support at columns. On S1-2403 @ sample grid locations 10.1/C & 10.1/D for slab edge supports, refer to sketches CD RFI 219 SK1 to SK4 for items 1 & 2:  1) Confirm the connections for the angles to the column flange are acceptable as shown or supply a new detail. Note all not shown is per 9/S1-5000 & RFI T-0901.  2) Confirm the connections for the angles to the column web are acceptable as shown or supply a new detail. Note all not shown is per 9/S1-5000 & RFI T-0901.						<b>ANSWER:</b> Detail 9/S1-5000 provides a typical detail for slab edge supports. However, no detail is provided for slab edge support at columns. On S1-2403 @ sample grid locations 10.1/C & 10.1/D for slab edge supports, refer to sketches CD RFI 219 SK1 to SK4 for items 1 & 2:  1) Confirm the connections for the angles to the column flange are acceptable as shown or supply a new detail. Note all not shown is per 9/S1-5000 & RFI T-0901.  2) Confirm the connections for the angles to the column web are acceptable as shown or supply a new detail. Note all not shown is per 9/S1-5000 & RFI T-0901.



# Webcor/Obayashi Joint Venture

## PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

### 30100 - Transbay Transit Center Project

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<b>T-0994</b>	<b>SSS - Lateral Bracing Clarifications at Ground Level</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  At the Lateral brace detail 3/S1-3503 refer to sketch CD RFI 213 SK1 for items 1 to 3:  1) Since detail 3/S1-3503 does not occur along grid 'C' and does at grid 'G', confirm the correct detail reference should read 1/S1-5022. 2) Referenced detail 8/S1-5015 does not show a full depth stiffener at the brace to beam connection. Confirm it is acceptable to proceed with the connection as shown in 8/S1-5015 & RFI T-0919. If not, supply the thickness and width of the full depth stiffener including welding for the stiffener. 3) Confirm the gusset dimensions as shown are acceptable.						<b>ANSWER:</b>  At the Lateral brace detail 3/S1-3503 refer to sketch CD RFI 213 SK1 for items 1 to 3:  1) Since detail 3/S1-3503 does not occur along grid 'C' and does at grid 'G', confirm the correct detail reference should read 1/S1-5022. 2) Referenced detail 8/S1-5015 does not show a full depth stiffener at the brace to beam connection. Confirm it is acceptable to proceed with the connection as shown in 8/S1-5015 & RFI T-0919. If not, supply the thickness and width of the full depth stiffener including welding for the stiffener. 3) Confirm the gusset dimensions as shown are acceptable.
<b>T-0995</b>	<b>SSS - Concrete Beam to Drag Beam Detail</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  1) Per details 1 & 4/S1-5022 shown on sketch CD RFI 214 SK1, please confirm the noted 1" stiffeners on detail 4/S1-5022 are also required in detail 1/S1-5022 along grid 'G'. 2) Please provide the weld for the 1" stiffeners indicated on detail 4/S1-5022.						<b>ANSWER:</b>  1) Per details 1 & 4/S1-5022 shown on sketch CD RFI 214 SK1, please confirm the noted 1" stiffeners on detail 4/S1-5022 are also required in detail 1/S1-5022 along grid 'G'. 2) Please provide the weld for the 1" stiffeners indicated on detail 4/S1-5022.
<b>T-0996</b>	<b>SSS - Beam to Beam Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  On S1-2403 at grids 11/C refer to sketches CD RFI 156 SK1 & SK2. After applying the double angle connection per S1-5010 for the W12x14 to the W30x99, there is insufficient room to connect the W30x99 to the column flange per 8/S1-5012. Confirm it is acceptable to connect the W12x14 to the W30x99 using a shear plate per S1-5011 or supply an alternate solution at this location.						<b>ANSWER:</b>  On S1-2403 at grids 11/C refer to sketches CD RFI 156 SK1 & SK2. After applying the double angle connection per S1-5010 for the W12x14 to the W30x99, there is insufficient room to connect the W30x99 to the column flange per 8/S1-5012. Confirm it is acceptable to connect the W12x14 to the W30x99 using a shear plate per S1-5011 or supply an alternate solution at this location.



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-0997</b>	<b>SSS - Steel Framing Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> On S1-2604 near grids 16/D refer to sketches CD RFI 158 SK1 & SK2 and confirm the (3) W16x26 beams are not required and may be deleted as the edge of slab is located only 1'-3" east of grid 16 per A1-2904 as shown on SK2.						<b>ANSWER:</b> On S1-2604 near grids 16/D refer to sketches CD RFI 158 SK1 & SK2 and confirm the (3) W16x26 beams are not required and may be deleted as the edge of slab is located only 1'-3" east of grid 16 per A1-2904 as shown on SK2.
<b>T-0998</b>	<b>SSS - Thread Diamter at Pretensioned Rod Detail</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Please refer to drawing S1-5052.  On 3/S1-5052 @ the Pretensioned Rod detail refer to sketches CD RFI 229 SK1 & SK2.  The actual major thread diameters of the pre-tensioned rods in detail 3/S1-5052 do not equal the nominal diameters shown. See the actual diameters on SK2 and confirm the holes in all elements that the anchor rods pass thru as shown in details 2 & 6/S1-5052 will be 1/16" over the major thread diameter.  Please confirm this is acceptable.						<b>ANSWER:</b> Please refer to drawing S1-5052.  On 3/S1-5052 @ the Pretensioned Rod detail refer to sketches CD RFI 229 SK1 & SK2.  The actual major thread diameters of the pre-tensioned rods in detail 3/S1-5052 do not equal the nominal diameters shown. See the actual diameters on SK2 and confirm the holes in all elements that the anchor rods pass thru as shown in details 2 & 6/S1-5052 will be 1/16" over the major thread diameter.  Please confirm this is acceptable.
<b>T-0999</b>	<b>SSS - Stair Detail Reference Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/10/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> On detail 3/S1-7008 refer to sketch CD RFI 164 SK1 and review the noted detail reference does not appear to be the correct detail at the noted location. Should this read 6/S1-7601 and not 3/S1-7601? Please clarify.						<b>ANSWER:</b> On detail 3/S1-7008 refer to sketch CD RFI 164 SK1 and review the noted detail reference does not appear to be the correct detail at the noted location. Should this read 6/S1-7601 and not 3/S1-7601? Please clarify.
<b>T-1000</b>	<b>SSS - Machine Lower Nozzles Perpendicular to Pipe</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/10/2013</b>	<b>01/13/2014</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> Please refer to drawing S1-5111 thru S1-5133.						<b>ANSWER:</b> Please refer to drawing S1-5111 thru S1-5133.



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>In recent meetings, Webcor/Obayashi has made it clear that the same Bus Deck Cast Node geometry will be used at multiple locations even though the angle of the lower Basket Columns changes at each Node. This adds a level of complexity and cost to the joint between the Cast Node and Basket Column Pipe due to the kink imposed on that joint as a result of the following:</p> <ul style="list-style-type: none"><li>- The Lower Pipe Columns will be required to be "miter cut" instead of a traditional square cut end. (Please note Spec Section OS 10 00, paragraph 3.2.M.1 states "Bearing ends of columns shall be milled or sawn square perpendicular to axis of the column.")</li><li>- Miter cut Pipe will have an ellipse cross section and will not match the circular Casting Node.</li><li>- Backing bars used to full pen weld the Pipe Column to the Cast Node would need to be custom machined to match the ellipse Pipe and circular Node to eliminate weld gaps. This significantly increases the complexity and risk for successfully welding the joint, and reduces the adjustability for fit up of these joints in the shop and the field.</li></ul> <p>This kink can be accommodated either by machining the nozzle of the Cast Node to be perpendicular to the pipe, or by machining the pipe end at a mitered angle to match the Cast Node.</p> <p>Since this joint on the Cast Node is already being machined, Skanska/OIW believes that the more desirable and less expensive option is to machine the nozzle of the Cast Node perpendicular to the axis of the Basket Column Pipe . As the nozzles will each be custom machined regardless, machining them to match the pipe axis should be a relatively low cost change .</p> <p>Skanska/OIW requests that the lower nozzle of each Bus Deck Cast Nodes to be machined perpendicular to the axis of the adjoining lower Basket Column Pipe. A negative response will result in a cost increase and a time increase.</p>					<p>In recent meetings, Webcor/Obayashi has made it clear that the same Bus Deck Cast Node geometry will be used at multiple locations even though the angle of the lower Basket Columns changes at each Node. This adds a level of complexity and cost to the joint between the Cast Node and Basket Column Pipe due to the kink imposed on that joint as a result of the following:</p> <ul style="list-style-type: none"><li>- The Lower Pipe Columns will be required to be "miter cut" instead of a traditional square cut end. (Please note Spec Section OS 10 00, paragraph 3.2.M.1 states "Bearing ends of columns shall be milled or sawn square perpendicular to axis of the column.")</li><li>- Miter cut Pipe will have an ellipse cross section and will not match the circular Casting Node.</li><li>- Backing bars used to full pen weld the Pipe Column to the Cast Node would need to be custom machined to match the ellipse Pipe and circular Node to eliminate weld gaps. This significantly increases the complexity and risk for successfully welding the joint, and reduces the adjustability for fit up of these joints in the shop and the field.</li></ul> <p>This kink can be accommodated either by machining the nozzle of the Cast Node to be perpendicular to the pipe, or by machining the pipe end at a mitered angle to match the Cast Node.</p> <p>Since this joint on the Cast Node is already being machined, Skanska/OIW believes that the more desirable and less expensive option is to machine the nozzle of the Cast Node perpendicular to the axis of the Basket Column Pipe . As the nozzles will each be custom machined regardless, machining them to match the pipe axis should be a relatively low cost change .</p> <p>Skanska/OIW requests that the lower nozzle of each Bus Deck Cast Nodes to be machined perpendicular to the axis of the adjoining lower Basket Column Pipe. A negative response will result in a cost increase and a time increase.</p>



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1001</b>	<b>SSS - Shear Plate Weld Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
On S1-2603 at grids 11/D for the W40 beam connections into the column web call for details 3/S1-5011 with 3 & 4/S15013 to be used. On sketches CD RFI 218 SK1 & SK2 please verify the large 2 3/8" single sided PJP weld for the 2 1/2" thick shear plate required at this location.			On S1-2603 at grids 11/D for the W40 beam connections into the column web call for details 3/S1-5011 with 3 & 4/S15013 to be used. On sketches CD RFI 218 SK1 & SK2 please verify the large 2 3/8" single sided PJP weld for the 2 1/2" thick shear plate required at this location.			
<b>T-1002</b>	<b>SSS - Web to Flange Welds at EBF Girders</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please refer to drawing S1-4205.			Please refer to drawing S1-4205.			
Please see Plan Sheet S1-4205 Detail 1 for typical details at EBF Link Beams. The typical arrangement specifies a transition from CJP weld to fillet welds and incorporates a weld access hole to separate the 2 welds. In an effort to reduce the number of weld access holes and the inherent issues that can arise with them, Oregon Iron Works is proposing to extend the CJP welds to the end of the girders thus removing the weld access holes at the weld transition point.			Please see Plan Sheet S1-4205 Detail 1 for typical details at EBF Link Beams. The typical arrangement specifies a transition from CJP weld to fillet welds and incorporates a weld access hole to separate the 2 welds. In an effort to reduce the number of weld access holes and the inherent issues that can arise with them, Oregon Iron Works is proposing to extend the CJP welds to the end of the girders thus removing the weld access holes at the weld transition point.			
Please see attached OIW sketch 2770-SK-TH02 representing a typical EBF Blank Beam Fabrication. It is Skanska/OIW's intent to extend the UT testing 1'-0" beyond the specified CJP weld zone. The balance of the weld will be MT/VT tested as required by Contract Documents.			Please see attached OIW sketch 2770-SK-TH02 representing a typical EBF Blank Beam Fabrication. It is Skanska/OIW's intent to extend the UT testing 1'-0" beyond the specified CJP weld zone. The balance of the weld will be MT/VT tested as required by Contract Documents.			
Please confirm that the proposed welding and NDE is acceptable for all EBF link Beams at the roof perimeter. There is no cost or time impact with this change.			Please confirm that the proposed welding and NDE is acceptable for all EBF link Beams at the roof perimeter. There is no cost or time impact with this change.			
<b>T-1003</b>	<b>SSS - Connection Clarification at Sloping Moment Beams</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/20/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
	<p>At a sample location on S1-2503 at grids 9/F refer to sketches CD RFI 150 SK1 &amp; SK2 as noted below.</p> <p>As the sloping BU beam rises 1/2" above the opposite BU beam, the thickness of the top continuity plate will be increased to 3-1/4". Confirm this is the design intent and should be applied typically at similar conditions.</p>					<p>At a sample location on S1-2503 at grids 9/F refer to sketches CD RFI 150 SK1 &amp; SK2 as noted below.</p> <p>As the sloping BU beam rises 1/2" above the opposite BU beam, the thickness of the top continuity plate will be increased to 3-1/4". Confirm this is the design intent and should be applied typically at similar conditions.</p>
<b>T-1004</b>	<b>SSS - Pins at Roof Clevises and Perimeter Bus Deck</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/12/2013</b>
<b>From:</b> Webcor Construction LP		Gregory Kemerer				
<b>REQUEST:</b>		<b>ANSWER:</b>				
<p>Please refer to the the following: S1-5017, S1-5131, S1-5132, S1-5133, 05 10 00 - 2.3.J &amp; 3.2.B.2.</p> <p>Paragraph 3.2.B.2 specifies the holes for the pins shall be no more than 1/32" over the diameter of the pin.</p> <p>Paragraph 2.3.J specifies the pins to be Hot Dip Galvanized (HDG). This combination will lead to interference at assembly due to the following factors:</p> <ol style="list-style-type: none"><li>1) Tolerance in bored hole diameter of 0.010 (+/- .010)</li><li>2) Tolerance of Pin diameter of 0.010 (+/- .005)</li><li>3) Tolerance of galvanize thickness at pin of 0.012 (+/- .006/side x 2)</li><li>4) Tolerance in thickness of primer at pin holes of 0.002 (+/- .001/side x 2)</li></ol> <p>The stack-up of tolerances is 0.034" which is greater than the specified 1/32" maximum clearance.</p> <p>Skanska/Oregon Iron Works is requesting approval to supply the pins and bored holes to the following nominal values and within the tolerance identified above. These values are measured after machining and prior to coating.</p> <ol style="list-style-type: none"><li>1. 7" diameter pins:<ol style="list-style-type: none"><li>a. Pin diameter = 6.906" (bored holes -1/8")</li><li>b. Bored holes = 7.032"</li></ol></li><li>2. 8" diameter pins:<ol style="list-style-type: none"><li>a. Pin diameter = 7.906 (bored holes -1/8")</li><li>b. Bored holes = 8.032"</li></ol></li></ol> <p>Note that zinc coating is not a hardened material, and the coating on the pins will be prone to galling while attempting to install in a horizontal position. Skanska/OIW suggests investigating alternate pin coatings; for example,</p>		<p>Please refer to the the following: S1-5017, S1-5131, S1-5132, S1-5133, 05 10 00 - 2.3.J &amp; 3.2.B.2.</p> <p>Paragraph 3.2.B.2 specifies the holes for the pins shall be no more than 1/32" over the diameter of the pin. Paragraph 2.3.J specifies the pins to be Hot Dip Galvanized (HDG). This combination will lead to interference at assembly due to the following factors:</p> <ol style="list-style-type: none"><li>1) Tolerance in bored hole diameter of 0.010 (+/- .010)</li><li>2) Tolerance of Pin diameter of 0.010 (+/- .005)</li><li>3) Tolerance of galvanize thickness at pin of 0.012 (+/- .006/side x 2)</li><li>4) Tolerance in thickness of primer at pin holes of 0.002 (+/- .001/side x 2)</li></ol> <p>The stack-up of tolerances is 0.034" which is greater than the specified 1/32" maximum clearance.</p> <p>Skanska/Oregon Iron Works is requesting approval to supply the pins and bored holes to the following nominal values and within the tolerance identified above. These values are measured after machining and prior to coating.</p> <ol style="list-style-type: none"><li>1. 7" diameter pins:<ol style="list-style-type: none"><li>a. Pin diameter = 6.906" (bored holes -1/8")</li><li>b. Bored holes = 7.032"</li></ol></li><li>2. 8" diameter pins:<ol style="list-style-type: none"><li>a. Pin diameter = 7.906 (bored holes -1/8")</li><li>b. Bored holes = 8.032"</li></ol></li></ol> <p>Note that zinc coating is not a hardened material, and the coating on the pins will be prone to galling while</p>				









<i>Number</i>	<i>Subject</i>	<i>Status</i>	<i>Choice Required</i>	<i>Date Created</i>	<i>Date Required</i>	<i>Date Answered</i>
	<p><b>REQUEST:</b></p> <p>Please refer to drawing S1-2603.</p> <p>On S1-2603 near grids 9/F refer to sketch CD RFI 153 SK1 and confirm it is acceptable to align the noted W30x108 with the W30x90 on the south side of PE302. This will give us an off-set of 6 3/4" on the east end between the W30x108 &amp; W24x76, which will allow a double angle connection per S1-5010. If not, supply an alternate solution as a double angle connection cannot be applied with the current beam locations because the bolts will foul the beam web on the opposite side.</p>					
	<p><b>ANSWER:</b></p> <p>Please refer to drawing S1-2603.</p> <p>On S1-2603 near grids 9/F refer to sketch CD RFI 153 SK1 and confirm it is acceptable to align the noted W30x108 with the W30x90 on the south side of PE302. This will give us an off-set of 6 3/4" on the east end between the W30x108 &amp; W24x76, which will allow a double angle connection per S1-5010. If not, supply an alternate solution as a double angle connection cannot be applied with the current beam locations because the bolts will foul the beam web on the opposite side.</p>					
<b>T-1007</b>	<b>SSS - Framing &amp; Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/30/2013</b>
	<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>On S1-2303 near grids 12/C refer to sketches CD RFI 221 SK1 &amp; SK2 for items 1 to 4:</p> <ol style="list-style-type: none"> <li>1) It appears the plan shows diagonal braces similar to 12/S1-3703 but details 3/S1-3705 &amp; 5/S1-3705 do not show the bracing. Are braces required?</li> <li>If braces are required, please see items 2, 3 &amp; 4.</li> <li>2) Supply the location of the braces from grid 'C' considering the dimensions on TR12 shown on SK2 and the connection to the girder per 8/S1-5005.</li> <li>3) Supply the underside of slab elevation at the brace located per dimension supplied in item 2.</li> <li>4) Supply the underside of slab elevation at the brace located per dimension supplied in item 2.</li> </ol>					
	<p><b>ANSWER:</b></p> <p>On S1-2303 near grids 12/C refer to sketches CD RFI 221 SK1 &amp; SK2 for items 1 to 4:</p> <ol style="list-style-type: none"> <li>1) It appears the plan shows diagonal braces similar to 12/S1-3703 but details 3/S1-3705 &amp; 5/S1-3705 do not show the bracing. Are braces required?</li> <li>If braces are required, please see items 2, 3 &amp; 4.</li> <li>2) Supply the location of the braces from grid 'C' considering the dimensions on TR12 shown on SK2 and the connection to the girder per 8/S1-5005.</li> <li>3) Supply the underside of slab elevation at the brace located per dimension supplied in item 2.</li> <li>4) Supply the underside of slab elevation at the brace located per dimension supplied in item 2.</li> </ol>					



Webcor/Obayashi Joint Venture

PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

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T-1007.1	SSS - Framing & Connection Clarifications	Closed	CR	04/17/2014	04/27/2014	04/30/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:  This is a follow-up RFI to RFI T-1007: Per the response to item #1 a dimension of 40' 4-3/4" was provided that differed to the dimension indicated on the Revit model. Candraft & Skanska have reviewed the Structural and Architectural drawings provided and cannot come up with this dimension. This is typical at every location where we are required to calculate the slope of the slab to locate the kicker angles connected to the underside of the slab. Please provide the dimensions indicated on SK1 thru SK4 or clearly direct Skanska as to where to find this information.		ANSWER:  This is a follow-up RFI to RFI T-1007: Per the response to item #1 a dimension of 40' 4-3/4" was provided that differed to the dimension indicated on the Revit model. Candraft & Skanska have reviewed the Structural and Architectural drawings provided and cannot come up with this dimension. This is typical at every location where we are required to calculate the slope of the slab to locate the kicker angles connected to the underside of the slab. Please provide the dimensions indicated on SK1 thru SK4 or clearly direct Skanska as to where to find this information.				
T-1008	SSS - Framing & Connection Clarifications	Closed	CR	12/10/2013	12/20/2013	01/24/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:  On S1-2303 near grids 12/G refer to sketches CD RFI 222 SK1 to SK3 for items 1 to 5: 1) Confirm this dimension is correct (from Revit model) to be used to determine the slope of MFB4. If not, supply the dimension. 2) Work with SK2 and confirm the location of the braces to avoid fouling connection on TR12. 3) Supply dimension (to be used to locate PL 2 1/2 x 9 x 2'-6). 4) Supply the underside of slab dimension at the location of the brace per item 2. 5) Supply the underside of slab dimension at the location of the brace per item 2.		ANSWER:  On S1-2303 near grids 12/G refer to sketches CD RFI 222 SK1 to SK3 for items 1 to 5: 1) Confirm this dimension is correct (from Revit model) to be used to determine the slope of MFB4. If not, supply the dimension. 2) Work with SK2 and confirm the location of the braces to avoid fouling connection on TR12. 3) Supply dimension (to be used to locate PL 2 1/2 x 9 x 2'-6). 4) Supply the underside of slab dimension at the location of the brace per item 2. 5) Supply the underside of slab dimension at the location of the brace per item 2.				
T-1009	SSS - Shear Plate Connection at Weak Axis Column Web	Closed	01	12/10/2013	12/20/2013	12/19/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST:  On S1-2603 near grids 9/D the grid locations for the note indicating to use detail 3/S1-5011 at the weak axis at MF columns is unclear. On sketch CD RFI 227 SK1 please		ANSWER:  On S1-2603 near grids 9/D the grid locations for the note indicating to use detail 3/S1-5011 at the weak axis at MF columns is unclear. On sketch CD RFI 227				



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	verify detail 3/S1-5011 only applies to grid lines 9/D & 9/F.					SK1 please verify detail 3/S1-5011 only applies to grid lines 9/D & 9/F.
<b>T-1010</b>	<b>SSS - Detail Clarification &amp; Locations for Concrete Beams &amp; Plate Connections</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/30/2013</b>
	<b>From:</b> Webcor Construction LP  <b>REQUEST:</b> At the ground level for the concrete beam locations and 1" plate requirements refer to sketches CD RFI 230 SK1 to SK7 for items 1 to 5: Note the structural & architectural drawings do not locate these members in question. 1) Confirm all dimensions for the spacing of the concrete are correct as shown. 2) Supply all clouded concrete beam location dimensions. 3) Confirm the intended location for the 1" stiffener plates is correct as shown. 4) Confirm the welding for the 1" stiffeners is acceptable as shown. 5) Confirm the noted stiffeners are also required in detail 4B.					<b>ANSWER:</b> At the ground level for the concrete beam locations and 1" plate requirements refer to sketches CD RFI 230 SK1 to SK7 for items 1 to 5: Note the structural & architectural drawings do not locate these members in question. 1) Confirm all dimensions for the spacing of the concrete are correct as shown. 2) Supply all clouded concrete beam location dimensions. 3) Confirm the intended location for the 1" stiffener plates is correct as shown. 4) Confirm the welding for the 1" stiffeners is acceptable as shown. 5) Confirm the noted stiffeners are also required in detail 4B.

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
<b>T-1010.1</b>	<b>SSS - Concrete Beam Location for Slab Support</b>	<b>Closed</b>	<b>CR</b>	<b>01/24/2014</b>	<b>02/03/2014</b>	<b>02/13/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
<p>This is a follow-up RFI to RFI T-1010 (SK 287 CD 230)  See attached CD RFI # 230.1 SK1 to SK6 for SK1 to SK6 for items 1 to 9:</p> <p>1) Shown are concrete beam locations to suit slab opening dimensions on A1-2862. Confirm the dimensions are correct.  2) Shown are concrete beam locations to suit retractable bollard locations on A1-2862 &amp; A1-2863. Confirm the dimensions are correct.  3) Shown are concrete beam locations to suit slab opening dimensions on A1-2863. Confirm the dimensions are correct.  4) Shown are concrete beam locations to suit retractable bollard locations on A1-2864. Confirm the dimensions are correct.  5) There is no information on A1-2864 to assist in locating the noted MFB1. Please supply dimension.  6) There is no information on A1-2864 to assist in locating the noted MFB1. Please supply dimension.  7) There is no information on A1-2865 to assist in locating the noted MFB1. Please supply dimension.  8) Shown are concrete beam locations to suit slab opening dimensions on A1-2865. Confirm the dimensions are correct.  9)There is no information on A1-2867 to assist in locating the noted MFB1's.Please supply dimensions</p>		<p>This is a follow-up RFI to RFI T-1010 (SK 287 CD 230)  See attached CD RFI # 230.1 SK1 to SK6 for SK1 to SK6 for items 1 to 9:</p> <p>1) Shown are concrete beam locations to suit slab opening dimensions on A1-2862. Confirm the dimensions are correct.  2) Shown are concrete beam locations to suit retractable bollard locations on A1-2862 &amp; A1-2863. Confirm the dimensions are correct.  3) Shown are concrete beam locations to suit slab opening dimensions on A1-2863. Confirm the dimensions are correct.  4) Shown are concrete beam locations to suit retractable bollard locations on A1-2864. Confirm the dimensions are correct.  5) There is no information on A1-2864 to assist in locating the noted MFB1. Please supply dimension.  6) There is no information on A1-2864 to assist in locating the noted MFB1. Please supply dimension.  7) There is no information on A1-2865 to assist in locating the noted MFB1. Please supply dimension.  8) Shown are concrete beam locations to suit slab opening dimensions on A1-2865. Confirm the dimensions are correct.  9)There is no information on A1-2867 to assist in locating the noted MFB1's.Please supply dimensions</p>				
<b>T-1011</b>	<b>SSS - Slab Dimension at Seismic Joints</b>	<b>Closed</b>	<b>CR</b>	<b>12/11/2013</b>	<b>12/21/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>		<b>ANSWER:</b>				
<p>Plan drawings S1-2503 and S1-2505 indicated a Type S8 floor type at the seismic joints at the Bus Deck level. Based on the Type S8 floor type detailed on detail 4/S1-5003, please confirm the dimensions indicated on CD RFI 202 SK1 for the structural slab and architectural topping thicknesses are accurate.</p>		<p>Plan drawings S1-2503 and S1-2505 indicated a Type S8 floor type at the seismic joints at the Bus Deck level. Based on the Type S8 floor type detailed on detail 4/S1-5003, please confirm the dimensions indicated on CD RFI 202 SK1 for the structural slab and architectural topping thicknesses are accurate.</p>				
<b>T-1012</b>	<b>SSS - Connection for BU Girder into W40 Beam</b>	<b>Closed</b>	<b>CR</b>	<b>12/11/2013</b>	<b>12/21/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						





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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

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T-1014	BGP - Moment Frame Beam Tie Configuration	Closed	CR	12/11/2013	12/21/2013	12/12/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST: Please refer to detail 2/S1-3600.  Due to the possibility of limited access during the installation of the individual moment frame hairpins as detailed in 2/S1-3600, SCCI/Gerdau proposes to modify the typical moment frame beam tie configuration to what is shown in the attached SCCI sketch SK-RFI-399.  Please confirm if this is acceptable.						ANSWER: Please refer to detail 2/S1-3600.  Due to the possibility of limited access during the installation of the individual moment frame hairpins as detailed in 2/S1-3600, SCCI/Gerdau proposes to modify the typical moment frame beam tie configuration to what is shown in the attached SCCI sketch SK-RFI-399.  Please confirm if this is acceptable.
T-1015	BGP - Moment Frame Cap Ties at shear Key Blockout	Closed	CR	12/11/2013	12/21/2013	12/12/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST: In order to avoid the shear key blockout and anchor bolts in the MF joint, SCCI/Gerdau proposes to eliminate up to two cap ties where the spacing is 4" and one cap tie where the spacing is 6". Cap ties will resume at regular spacing no further than 1" from the beyond the anchor bolts or blockout.  Reference the attached photo for more details. Is this acceptable?						ANSWER: In order to avoid the shear key blockout and anchor bolts in the MF joint, SCCI/Gerdau proposes to eliminate up to two cap ties where the spacing is 4" and one cap tie where the spacing is 6". Cap ties will resume at regular spacing no further than 1" from the beyond the anchor bolts or blockout.  Reference the attached photo for more details. Is this acceptable?
T-1016	BGP - Concourse Slab Elevation at NW Corner of Area 3/Zone 1	Closed	CR	12/11/2013	12/21/2013	12/12/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST: Please refer to attached drawing S1-2202.  Please clarify the concourse slab thickness in gridline area 1-2 and A-C. It is unclear if the area is marked as RCS8 or RCS1.						ANSWER: Please refer to attached drawing S1-2202.  Please clarify the concourse slab thickness in gridline area 1-2 and A-C. It is unclear if the area is marked as RCS8 or RCS1.
T-1017	SSS - Location Clarification for Lateral Bracing	Closed	CR	12/11/2013	12/21/2013	12/16/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST:						ANSWER:







<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1019</b>	<b>SSS - Transfer Girder CJP Web-Flange Welds</b>	<b>Closed</b>	<b>CR</b>	<b>12/11/2013</b>	<b>12/21/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> In orThe web to flange T-joint CJP welds for the transfer girders shown on drawings S1-4300 to S1-4308 do not indicate that a reinforcing fillet weld is required. Note 1 on 4/S1-4202 calls for reinforcing fillets to T-joint groove welds of SLRS members. Please confirm that a reinforcing fillet weld in not required for the transfer girder web to flange T-joint CJP welds.						<b>ANSWER:</b> In orThe web to flange T-joint CJP welds for the transfer girders shown on drawings S1-4300 to S1-4308 do not indicate that a reinforcing fillet weld is required. Note 1 on 4/S1-4202 calls for reinforcing fillets to T-joint groove welds of SLRS members. Please confirm that a reinforcing fillet weld in not required for the transfer girder web to flange T-joint CJP welds.
<b>T-1020</b>	<b>SSS - Type 2 Drag Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>12/11/2013</b>	<b>12/21/2013</b>	<b>12/16/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> For Type 2 Drag Connection Clarifications refer to sketches CD RFI 148 SK1 to SK2 for items 1 to 5:  1) Supply erection gap between web reinforcement plate and shear plate on column. 2) Supply erection gap between 2" plate and shear plate on column. 3) Supply erection gap between 1 1/2" web doubler plate and 2" thick plate. 4) Supply erection gap between beam web and shear plate on column. 5) Supply erection gap between 2.5" plate and shear plate on column.						<b>ANSWER:</b> For Type 2 Drag Connection Clarifications refer to sketches CD RFI 148 SK1 to SK2 for items 1 to 5:  1) Supply erection gap between web reinforcement plate and shear plate on column. 2) Supply erection gap between 2" plate and shear plate on column. 3) Supply erection gap between 1 1/2" web doubler plate and 2" thick plate. 4) Supply erection gap between beam web and shear plate on column. 5) Supply erection gap between 2.5" plate and shear plate on column.
<b>T-1021</b>	<b>SSS - Rebar Holes and Headed Stud Details at Ground Level</b>	<b>Closed</b>	<b>01</b>	<b>12/12/2013</b>	<b>12/22/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP Robert Kjome						
<b>REQUEST:</b> Refer to sketch CD RFI 105.1 SK1. The 2 1/2 x 14 x 2'-6" plate has been set per the elevation given in RFI # T-0888 item 3 and the 3" dia. rebar holes have been set at 1 3/4" above the underside of MFB 6 per RFI # T0888 item 8. This results in the plate fouling the rebar holes as shown.  Please advise.  Note: the same occurs at grid 10.1.						<b>ANSWER:</b> Refer to sketch CD RFI 105.1 SK1. The 2 1/2 x 14 x 2'-6" plate has been set per the elevation given in RFI # T-0888 item 3 and the 3" dia. rebar holes have been set at 1 3/4" above the underside of MFB 6 per RFI # T0888 item 8. This results in the plate fouling the rebar holes as shown.  Please advise.  Note: the same occurs at grid 10.1.





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<b>T-1022</b>	<b>SSS - Headed Stud and Hole Clarifications at Transfer Girders</b>	<b>Closed</b>	<b>CR</b>	<b>12/12/2013</b>	<b>12/22/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
This is a follow-up RFI to Webcor RFI #T-0890 (SK RFI # 150 & CD RFI # 109) Refer to sketches CD RFI 109.1 SK1 to SK3. The response in Webcor RFI # T-0890 has been applied at grid 11 as shown on SK2 but the response to T-0890 with the information shown in details 6/S1-3702 & 2/S1-3705 cannot be applied at grid 9 as shown on SK3. There insufficient space to fit the (50) headed studs as requested.			This is a follow-up RFI to Webcor RFI #T-0890 (SK RFI # 150 & CD RFI # 109) Refer to sketches CD RFI 109.1 SK1 to SK3. The response in Webcor RFI # T-0890 has been applied at grid 11 as shown on SK2 but the response to T-0890 with the information shown in details 6/S1-3702 & 2/S1-3705 cannot be applied at grid 9 as shown on SK3. There insufficient space to fit the (50) headed studs as requested.			
Please supply a new detail for the TR9 location.			Please supply a new detail for the TR9 location.			
<b>T-1023</b>	<b>SSS - Deck Support Angle Spacing</b>	<b>Closed</b>	<b>01</b>	<b>12/12/2013</b>	<b>12/22/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At a sample location on S1-2403 between grids D & F west of line 10 refer to sketches CD RFI 155 SK1 & SK2 for angle spacing question below.			At a sample location on S1-2403 between grids D & F west of line 10 refer to sketches CD RFI 155 SK1 & SK2 for angle spacing question below.			
Detail 9/S1-5000 (see SK2) states that the maximum spacing for the deck support angles and bracing is 8'-0. As shown, the spacing of the steel framing on S1-2403 (SK1) exceeds 8'-0. Confirm the framing as shown on S1-2403 is acceptable and no further action is required or supply a revised partial plan to show the revised framing to meet the criteria in detail 9/S1-5000.			Detail 9/S1-5000 (see SK2) states that the maximum spacing for the deck support angles and bracing is 8'-0. As shown, the spacing of the steel framing on S1-2403 (SK1) exceeds 8'-0. Confirm the framing as shown on S1-2403 is acceptable and no further action is required or supply a revised partial plan to show the revised framing to meet the criteria in detail 9/S1-5000.			
<b>T-1024</b>	<b>SSS - Transfer Girder Studs and Rebar Holes</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At TR8 near grid line G refer to sketches CD RFI 220 SK1 to SK3 for items 1 to 3: 1) Confirm the headed studs as shown are correct (work with item 2). 2) Detail 2/S1-5023 is referenced with a "SIM' designation and it is not clear what is required on grid 8 for the additional headed studs shown in detail 2/S1-5023.			At TR8 near grid line G refer to sketches CD RFI 220 SK1 to SK3 for items 1 to 3: 1) Confirm the headed studs as shown are correct (work with item 2). 2) Detail 2/S1-5023 is referenced with a "SIM' designation and it is not clear what is required on grid 8 for the additional headed studs shown in detail 2/S1-			



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<b>T-1027</b>	<b>SSS - Deck Support at Transfer Girders</b>	<b>Open</b>	<b>01</b>	<b>12/12/2013</b>	<b>12/22/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
<p>At sample locations on S1-2303 @ grid lines 9.9 &amp; 10.1 refer to sketch CD RFI 226 SK1 for items 1 &amp; 2 below regarding deck support requirements.</p> <p>1) T/Steel at beams = 19'-1 5/8 and the T/Steel for TR9.9 &amp; TR10.1 = 19'-1 7/16 (19.12'). This leaves a difference of 3/16" as shown on SK1. Confirm deck support angles are not required along grids 9.9 &amp; 10.1 between grids D-G.</p> <p>2) If deck support angles are required, supply welding for the angles as the 1/4" fillet weld per details 8/S1-3705 and 10 &amp; 11/S1-5002 cannot be achieved with the 3/16" elevation difference.</p>		<p>At sample locations on S1-2303 @ grid lines 9.9 &amp; 10.1 refer to sketch CD RFI 226 SK1 for items 1 &amp; 2 below regarding deck support requirements.</p> <p>1) T/Steel at beams = 19'-1 5/8 and the T/Steel for TR9.9 &amp; TR10.1 = 19'-1 7/16 (19.12'). This leaves a difference of 3/16" as shown on SK1. Confirm deck support angles are not required along grids 9.9 &amp; 10.1 between grids D-G.</p> <p>2) If deck support angles are required, supply welding for the angles as the 1/4" fillet weld per details 8/S1-3705 and 10 &amp; 11/S1-5002 cannot be achieved with the 3/16" elevation difference.</p>				
<b>T-1028</b>	<b>SSS - Shaw Alley Bridge End Plates</b>	<b>Closed</b>	<b>CR</b>	<b>12/12/2013</b>	<b>12/22/2013</b>	<b>12/12/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>		<b>ANSWER:</b>				
<p>Please confirm the 14 1/2" long end plates shown in 5/S1-5004 are not in TG07.1R scope as the plates are welded to reinforcing steel supplied by others and so could only be installed by future concrete trade subcontractor. See attached referenced drawing S1-5004.</p>		<p>Please confirm the 14 1/2" long end plates shown in 5/S1-5004 are not in TG07.1R scope as the plates are welded to reinforcing steel supplied by others and so could only be installed by future concrete trade subcontractor. See attached referenced drawing S1-5004.</p>				
<b>T-1029</b>	<b>SSS - Pretensioned Rod at Cruciform Columns</b>	<b>Closed</b>	<b>01</b>	<b>12/12/2013</b>	<b>12/22/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>		<b>ANSWER:</b>				
<p>On 2 &amp; 6/S1-5052 @ the Pretensioned Rod details refer to sketches CD RFI 228 SK1 &amp; SK2 for items 1 &amp; 2:</p> <p>1) The WT surface below will be milled to bear against the 2 1/2" thick plate. Work with item 2 below as shown on SK2 and confirm welding at this joint is not required as none is shown.</p> <p>2) Similar to detail 2/S1-5052 as shown on SK1 (item 1 above), the contact surface will be milled for bearing as requested. Please confirm the noted 1/2" fillet welds for the built-up TT section to the 4" thick plate are to be applied as shown.</p>		<p>On 2 &amp; 6/S1-5052 @ the Pretensioned Rod details refer to sketches CD RFI 228 SK1 &amp; SK2 for items 1 &amp; 2:</p> <p>1) The WT surface below will be milled to bear against the 2 1/2" thick plate. Work with item 2 below as shown on SK2 and confirm welding at this joint is not required as none is shown.</p> <p>2) Similar to detail 2/S1-5052 as shown on SK1 (item 1 above), the contact surface will be milled for bearing as requested. Please confirm the noted 1/2" fillet welds for the built-up TT section to the 4" thick plate are to be applied as shown.</p>				



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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

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T-1031	SSS - Typical Deck Support Details at Columns	Closed	CR	12/12/2013	12/22/2013	12/19/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST: 1/S1-5001 refer to sketch CD RFI 119 SK1 & SK2. Confirm the specified L3x3x12GA deck support angles are adequate given the approximate length of these angles will be 3' 9-1/2". The contractor proposes to use A36 L3x3x5/16" at these typical details. Please confirm this is acceptable.		ANSWER: 1/S1-5001 refer to sketch CD RFI 119 SK1 & SK2. Confirm the specified L3x3x12GA deck support angles are adequate given the approximate length of these angles will be 3' 9-1/2". The contractor proposes to use A36 L3x3x5/16" at these typical details. Please confirm this is acceptable.				
T-1032	SSS - Detail Clarification at Bent Plate to Sloping Beams	Closed	CR	12/12/2013	12/22/2013	12/30/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST: Refer to sketch CD RFI 231 SK1 and confirm it is acceptable to fabricate the double bent deck support plate as shown when the beam is sloping and the underside of slab is horizontal, resulting in a variable height along the deck support plate. It is not possible to model a double bent plate with a variable height in Tekla.		ANSWER: Refer to sketch CD RFI 231 SK1 and confirm it is acceptable to fabricate the double bent deck support plate as shown when the beam is sloping and the underside of slab is horizontal, resulting in a variable height along the deck support plate. It is not possible to model a double bent plate with a variable height in Tekla.				
T-1033	SSS - Weld Clarifications at Light Columns	Closed	CR	12/12/2013	12/22/2013	12/20/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST: Per detail At the light column bases refer to sketch CD RFI 167 SK1 for items 1 & 2: 1) Confirm the CJP weld designation applies to the 1" thick web and the 2" thick flanges to the column base plates. 2) Supply the weld requirements for the 1" shear key web to the 2" shear key flanges.		ANSWER: Per detail At the light column bases refer to sketch CD RFI 167 SK1 for items 1 & 2: 1) Confirm the CJP weld designation applies to the 1" thick web and the 2" thick flanges to the column base plates. 2) Supply the weld requirements for the 1" shear key web to the 2" shear key flanges.				
T-1034	SSS - Material Grade and CVN Requirements	Closed	CR	12/12/2013	12/22/2013	12/12/2013
From: Webcor Construction LP Gregory Kemerer						
REQUEST: 1. At 1/S1-4205 EBF LINK BEAM DETAIL, there is a section 4/S1-4205 that cuts an EBF LINK BEAM CROSS SECTION. The same section 4/S1-4205 is cut on 2/S1-		ANSWER: 1. At 1/S1-4205 EBF LINK BEAM DETAIL, there is a section 4/S1-4205 that cuts an EBF LINK BEAM CROSS SECTION. The same section 4/S1-4205 is				





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<b>T-1037</b>	<b>SSS - Typical Kicker Brace Detail</b>	<b>Closed</b>	<b>CR</b>	<b>12/12/2013</b>	<b>12/22/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference the bracing connection details provided on S1-5015. At conditions where a full depth shear plate and bracing are required, there is consistently a conflict between the bottom gusset plate and shear plate. Based on the weld requirements for the kicker brace connection, the following is proposed to avoid this conflict: 1) Connect the kicker directly to the shear plate 2) Eliminate the bottom gusset 3) Offset the top gusset (below the beam) by the width of the beam web to align with the shear plate 4) Match the thickness of the gusset and stitch plates to the shear plate thickness 5) Shape the bottom of the shear plate, where necessary, to achieve the required angle brace weld  Please confirm this is an acceptable typical solution for the conditions shown in the sketches attached and at other typical locations where bracing and a full depth shear plate are required.			Reference the bracing connection details provided on S1-5015. At conditions where a full depth shear plate and bracing are required, there is consistently a conflict between the bottom gusset plate and shear plate. Based on the weld requirements for the kicker brace connection, the following is proposed to avoid this conflict: 1) Connect the kicker directly to the shear plate 2) Eliminate the bottom gusset 3) Offset the top gusset (below the beam) by the width of the beam web to align with the shear plate 4) Match the thickness of the gusset and stitch plates to the shear plate thickness 5) Shape the bottom of the shear plate, where necessary, to achieve the required angle brace weld  Please confirm this is an acceptable typical solution for the conditions shown in the sketches attached and at other typical locations where bracing and a full depth shear plate are required.			
<b>T-1038</b>	<b>Spandrel Beam Reinforcement clarification Area 1-9</b>	<b>Closed</b>	<b>01</b>	<b>12/13/2013</b>	<b>12/23/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Further to discussion with Thornton Tomasetti design Engineer Kerem Gulec on the responses to the RFI received to date on the spandrel Beams modifications for area's 1-9 which include: RFI's T-0707, 708, 713, 717, 718, 719, 873 & 874 the response to these RFI's specified that a "lap splices shall be provided where the beam rebar is transitioned from the spacing in the construction drawings to the modified spacing" However following discussion this now will change to "Horizontal Rebar Bar spacing between modified spacing and construction drawings spacing will transitions over a distance of 6' on either side of the modified cross-section and thus removing the need to provide the additional lap splices. See a typical example attached.  Please confirm this is acceptable.			Further to discussion with Thornton Tomasetti design Engineer Kerem Gulec on the responses to the RFI received to date on the spandrel Beams modifications for area's 1-9 which include: RFI's T-0707, 708, 713, 717, 718, 719, 873 & 874 the response to these RFI's specified that a "lap splices shall be provided where the beam rebar is transitioned from the spacing in the construction drawings to the modified spacing" However following discussion this now will change to "Horizontal Rebar Bar spacing between modified spacing and construction drawings spacing will transitions over a distance of 6' on either side of the modified cross-section and thus removing the need to provide the additional lap splices. See a typical example attached.  Please confirm this is acceptable.			



Webcor/Obayashi Joint Venture

PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

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Date: 02/11/2015  
Time: 07:01 AM  
Job: 30100

<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1039</b>	<b>SSS - Stitch Bolts on Kicker Braces</b>	<b>Closed</b>	<b>CR</b>	<b>12/16/2013</b>	<b>12/26/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Details 4, 5 & 7/S1-5015 do not show a stitch bolt requirement for the kicker braces. At a sample location and detail shown on sketches CD RFI 066 SK1 & SK2 please confirm none are required or supply the necessary information.					<b>ANSWER:</b>  Details 4, 5 & 7/S1-5015 do not show a stitch bolt requirement for the kicker braces. At a sample location and detail shown on sketches CD RFI 066 SK1 & SK2 please confirm none are required or supply the necessary information.	
<b>T-1039.1</b>	<b>SSS - Stitch Bolts on Kicker Braces</b>	<b>Closed</b>	<b>CR</b>	<b>02/03/2014</b>	<b>02/13/2014</b>	<b>02/10/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  The response to T-1039 references details 3/S1-3703 & 6/S1-5022 for stitch plate information although these sections are only cut at Ground Level at Fremont & First Street. Please confirm the response is intended to indicate stitch plates are required at all kicker brace locations including details 4, 5 & 7/S1-5015 as per details 3/S1-3703 & 6/S1-5022. If so please confirm detail 8/S1-5015 can also be used for the stitch plate detail.					<b>ANSWER:</b>  The response to T-1039 references details 3/S1-3703 & 6/S1-5022 for stitch plate information although these sections are only cut at Ground Level at Fremont & First Street. Please confirm the response is intended to indicate stitch plates are required at all kicker brace locations including details 4, 5 & 7/S1-5015 as per details 3/S1-3703 & 6/S1-5022. If so please confirm detail 8/S1-5015 can also be used for the stitch plate detail.	
<b>T-1040</b>	<b>BGP - Width and Depth of Intermediate Beam in Lower Concourse at GL E.6/7</b>	<b>Closed</b>	<b>CR</b>	<b>12/17/2013</b>	<b>12/27/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP                      Jackson Tukuafu						
<b>REQUEST:</b>  Please reference contract drawing S1-2203.  Plan sheet S1-2203 shows an intermediate beam at gridline E.6 from gridline 6 to gridline 8 (see highlighted area attached). The Section 2/S1-3400 does not give the specific dimensions for a beam with change in slab elevation. Please provide both width and depth of the beam at this location in the lower concourse.					<b>ANSWER:</b>  Please reference contract drawing S1-2203.  Plan sheet S1-2203 shows an intermediate beam at gridline E.6 from gridline 6 to gridline 8 (see highlighted area attached). The Section 2/S1-3400 does not give the specific dimensions for a beam with change in slab elevation. Please provide both width and depth of the beam at this location in the lower concourse.	
<b>T-1041</b>	<b>SSS - CJP Weld Prep between Ground Level Cast Node and Transfer Girder</b>	<b>Closed</b>	<b>CR</b>	<b>12/17/2013</b>	<b>12/27/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>					<b>ANSWER:</b>	





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>Our fabricator Thompson Metal Fab has requested a 2" 45° bevel be incorporated into the ground level cast node machining drawings. This weld is detailed on 6/S1-4350. See attached sketch for bevel detail. Please confirm approval for weld prep detailed in attached sketches.</p>					<p>Our fabricator Thompson Metal Fab has requested a 2" 45° bevel be incorporated into the ground level cast node machining drawings. This weld is detailed on 6/S1-4350. See attached sketch for bevel detail. Please confirm approval for weld prep detailed in attached sketches.</p>
<b>T-1042</b>	<b>BGP - Geothermal Manifold Locations for Fields 3, 4, 5, 6, 7, 8, 9, and 10</b>	<b>Closed</b>	<b>CR</b>	<b>12/17/2013</b>	<b>12/27/2013</b>	<b>01/07/2014</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> <p>Per the drawings, the manifold is to be located at an elevation no greater than 14' below finish grade (street) elevation. Per conversations in the preparatory DFOW meeting and other coordination meetings, the Engineer planned to have the manifold in a specific location. Attached are elevation drawings for Field 3, 4, 5, 6, 7, 8, 9 and 10 Manifolds. Please confirm that the attached elevation details work with the desgner's intent for the manifold locations for said Fields.</p> <p>Note that Riser 10 has been relocated approximately 4' East between piles 231 and 232 to allow for the required 10' minimum spacing for future column installation.</p>						<b>ANSWER:</b> <p>Per the drawings, the manifold is to be located at an elevation no greater than 14' below finish grade (street) elevation. Per conversations in the preparatory DFOW meeting and other coordination meetings, the Engineer planned to have the manifold in a specific location. Attached are elevation drawings for Field 3, 4, 5, 6, 7, 8, 9 and 10 Manifolds. Please confirm that the attached elevation details work with the desgner's intent for the manifold locations for said Fields.</p> <p>Note that Riser 10 has been relocated approximately 4' East between piles 231 and 232 to allow for the required 10' minimum spacing for future column installation.</p>
<b>T-1043</b>	<b>BGP - Elevator Sill Support Angle Dimensions</b>	<b>Closed</b>	<b>CR</b>	<b>12/17/2013</b>	<b>12/27/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> <p>Please refer attached SKA-2916 through SKA-2921, and A1-7576.</p> <p>1. Please confirm Elevator Sill Support Angle at GL 2/E.2 is 4'-4" in length</p> <p>2. Please confirm all other elevator sill support angles highlighted on the attached drawings extend the entire length of slab opening/pit, except where the angle terminates as prescribes in RFI response T-0837.1</p>						<b>ANSWER:</b> <p>Please refer attached SKA-2916 through SKA-2921, and A1-7576.</p> <p>1. Please confirm Elevator Sill Support Angle at GL 2/E.2 is 4'-4" in length</p> <p>2. Please confirm all other elevator sill support angles highlighted on the attached drawings extend the entire length of slab opening/pit, except where the angle terminates as prescribes in RFI response T-0837.1</p>





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<b>T-1044</b>	<b>SSS - Personnel and Material Hoist Layout</b>	<b>Closed</b>	<b>CR</b>	<b>12/17/2013</b>	<b>12/17/2013</b>	<b>12/20/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Information Requested: For reference, please use drawings A101 - A110 from Exhibit A of the Subcontractor Bid Package Manual and Forms - Contract #30100071W, #30100071C and #30100071E.			Information Requested: For reference, please use drawings A101 - A110 from Exhibit A of the Subcontractor Bid Package Manual and Forms - Contract #30100071W, #30100071C and #30100071E.			
Skanska would like to confirm that the personnel and material hoist layout will be installed at the locations as shown on drawings A101 - A110. In addition, please provide the dimensions of the hoist openings.			Skanska would like to confirm that the personnel and material hoist layout will be installed at the locations as shown on drawings A101 - A110. In addition, please provide the dimensions of the hoist openings.			
In order for the hoists to be installed, steel framing will have to be left out until the hoists are removed. Please provide back-up engineering that allows for this to take place & provide any weldaments or bracing required.			In order for the hoists to be installed, steel framing will have to be left out until the hoists are removed. Please provide back-up engineering that allows for this to take place & provide any weldaments or bracing required.			
<b>T-1045</b>	<b>BSE - Micropile Relocations -Zone 3</b>	<b>Closed</b>	<b>01</b>	<b>12/17/2013</b>	<b>12/27/2013</b>	<b>01/07/2014</b>
<b>From:</b> Webcor Construction LP                      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Fourteen (14) micropiles located under Span 3.7 and 3.8 in Zone 3 have to be relocated in the field due to their proximity to the Trestle Deck. Micropiles E343/E354/E363/E375/E390/ E401/E411 have to be relocated 5' to the North and micropiles E340/E353/E362/E371/E386/ E400/E410 5' to the South. See attached sketch of micropiles in question.			Fourteen (14) micropiles located under Span 3.7 and 3.8 in Zone 3 have to be relocated in the field due to their proximity to the Trestle Deck. Micropiles E343/E354/E363/E375/E390/ E401/E411 have to be relocated 5' to the North and micropiles E340/E353/E362/E371/E386/ E400/E410 5' to the South. See attached sketch of micropiles in question.			
Please confirm these relocations are acceptable.			Please confirm these relocations are acceptable.			
<b>T-1045.1</b>	<b>Micropile Relocations -Zone 3 &amp; 4</b>	<b>Closed</b>	<b>01</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/14/2014</b>
<b>From:</b> Webcor Construction LP                      Michael Spillane						
<b>REQUEST:</b>			<b>ANSWER:</b>			
BBil is proposing to move 49 micropiles located within Zone 3 & 4 due to their close proximity to the Trestle/bridge Deck. Micropiles E375/E390/ E401/E411/E654/E670/E704/E738/E769/E800/E815/E826 would all be relocated 5' to the North, micropiles E492/E512/E537/E564/E587/E610/E630 would also relocated 3' north. The micropiles			BBil is proposing to move 49 micropiles located within Zone 3 & 4 due to their close proximity to the Trestle/bridge Deck. Micropiles E375/E390/ E401/E411/E654/E670/E704/E738/E769/E800/E815/E826 would all be relocated 5' to the North, micropiles E492/E512/E537/E564/E587/E610/E630 would also relocated 3' north. The micropiles			



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
	<p>E371/E386/E400/E410/E488/E526/E534/E559/E578/E605 /E622/E650/E666/E700/E734 /E765/E796/E811/E825 would also be relocated 5' to the South. Micropiles E416/E417/E418/E419/E420/E426/E427/E428/E429/E430 /E431 would also be relocated 3' to the West. See attached sketch of micro piles in question.</p> <p>Please confirm this is acceptable</p>					<p>E371/E386/E400/E410/E488/E526/E534/E559/E578/E 605/E622/E650/E666/E700/E734 /E765/E796/E811/E825 would also be relocated 5' to the South. Micropiles E416/E417/E418/E419/E420/E426/E427/E428/E429/E 430/E431 would also be relocated 3' to the West. See attached sketch of micro piles in question.</p> <p>Please confirm this is acceptable</p>
<b>T-1046</b>	<b>SSS - Transfer Girder Weld Access Holes</b>	<b>Closed</b>	<b>CR</b>	<b>12/17/2013</b>	<b>12/27/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>Please confirm the weld access holes detailed on SK1 are acceptable for all Transfer Girder field splice connections.</p>			<b>ANSWER:</b> <p>Please confirm the weld access holes detailed on SK1 are acceptable for all Transfer Girder field splice conn ections.</p>			
<b>T-1047</b>	<b>SSS - Field Splice Locations</b>	<b>Closed</b>	<b>CR</b>	<b>12/17/2013</b>	<b>12/27/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>Skanska has evaluated adding, relocating or removing sev eral field splices on a number of the transfer girders in ord er to reduce segment weights for critical picks, avoid interf erences with longitudinal framing members, increase stabil ity of the girder segments during erection and to optimize our erection sequencing.</p> <p>Please confirm the field splice locations indicated on the at tached sketches (SK1 thru SK34) are acceptable.</p>			<b>ANSWER:</b> <p>Skanska has evaluated adding, relocating or removing several field splices on a number of the transfer girder s in order to reduce segment weights for critical picks, avoid interferences with longitudinal framing members, increase stability of the girder segments during erecti on and to optimize our erection sequencing.</p> <p>Please confirm the field splice locations indicated on t he attached sketches (SK1 thru SK34) are acceptable.</p>			



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
<b>T-1048</b>	<b>SSS - Elevator Rail Support Embedded Plate</b>	<b>Closed</b>	<b>CR</b>	<b>12/17/2013</b>	<b>12/27/2013</b>	<b>12/17/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Elevator rail support detail 4/S1&#8208;7630 indicates a shop assembled support with embedded plates. As the package delineation line shows the ½" thick embedded plate is not in Skanska's scope of work. The embedded plates will be supplied and installed by Shimmick and Skanska will field weld the HSS with end plates to the embedded plate as indicated on SK3. Please confirm this is acceptable.			Elevator rail support detail 4/S1&#8208;7630 indicates a shop assembled support with embedded plates. As the package delineation line shows the ½" thick embedded plate is not in Skanska's scope of work. The embedded plates will be supplied and installed by Shimmick and Skanska will field weld the HSS with end plates to the embedded plate as indicated on SK3. Please confirm this is acceptable.			
<b>T-1048.1</b>	<b>SSS - Elevator Rail Supports Erection Aids</b>	<b>Closed</b>	<b>CR</b>	<b>01/08/2014</b>	<b>01/18/2014</b>	<b>01/14/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 183.1 SK1A, SK1B, SK2A & SK2B for items 1 & 2:			See attached CD RFI # 183.1 SK1A, SK1B, SK2A & SK2B for items 1 & 2:			
1.) Confirm the elevator rail support connection with erection aids is acceptable as shown.			1.) Confirm the elevator rail support connection with erection aids is acceptable as shown.			
2.) Confirm the elevator rail support connection with erection aids is acceptable as shown.			2.) Confirm the elevator rail support connection with erection aids is acceptable as shown.			
<b>T-1049</b>	<b>BGP - Column Base Plate Clearance Lower Concourse Slab</b>	<b>Closed</b>	<b>01</b>	<b>01/14/2014</b>		<b>01/27/2014</b>
<b>From:</b> Webcor Construction LP                      Adib Sassine						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref: 1 and 3/S1-5051, S1-3600, S1-2205			Ref: 1 and 3/S1-5051, S1-3600, S1-2205			
To erect and plumb Lower Concourse Column with base plates Types I as shown on schedule 1/S1-5051 and II at 7/F.8 shown on detail 5A/S1-5051, erection aids will be required at the base plate. However, due to the depression, rebar running thru the depression and based on our experience with the grouting at column base plate mock-up, allowable clearances to set these base plates may not be adequate. As an example, column at GL C/24.9, the bottom of type I C base plate is within 1" from			To erect and plumb Lower Concourse Column with base plates Types I as shown on schedule 1/S1-5051 and II at 7/F.8 shown on detail 5A/S1-5051, erection aids will be required at the base plate. However, due to the depression, rebar running thru the depression and based on our experience with the grouting at column base plate mock-up, allowable clearances to set these base plates may not be adequate. As an example, column at GL C/24.9, the bottom of type I C			



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	<p>the top of rebar and does not have adequate area for shim packs.</p> <p>Question # 1:</p> <p>To provide adequate erection aids, please review the following options and advise as to which one is acceptable:</p> <p>Option 1: Lower rebar around the base plate area by 1" to allow for 2" clear between rebar and bottom of base plate. Install 4 shim packs for erection purposes under each corner of the base plate on top of level concrete surface.</p> <p>Option 2: Stop or adjust reinforcing steel under the base plate and use shim packs for erection on top of level concrete surface.</p> <p>Option 3: Do not modify rebar, raise base plate elevation by 1" to provide minimum of 2" clearance under the base plate. Locate two shim packs next to key plates and install two additional erection aid threaded bolts with leveling nut drilled in concrete by Skanska as shown on the attached sketch SK-2.</p> <p>Question # 2:</p> <p>There is a 3" dimension between edge of steel plate and edge of depressed slab. Pls confirm if 6" dimension is acceptable in lieu of 3" around the base plates Type I C , I B and Type II at 7/F.8.</p>					
	<p>base plate is within 1" from the top of rebar and does not have adequate area for shim packs.</p> <p>Question # 1:</p> <p>To provide adequate erection aids, please review the following options and advise as to which one is acceptable:</p> <p>Option 1: Lower rebar around the base plate area by 1" to allow for 2" clear between rebar and bottom of base plate. Install 4 shim packs for erection purposes under each corner of the base plate on top of level concrete surface.</p> <p>Option 2: Stop or adjust reinforcing steel under the base plate and use shim packs for erection on top of level concrete surface.</p> <p>Option 3: Do not modify rebar, raise base plate elevation by 1" to provide minimum of 2" clearance under the base plate. Locate two shim packs next to key plates and install two additional erection aid threaded bolts with leveling nut drilled in concrete by Skanska as shown on the attached sketch SK-2.</p> <p>Question # 2:</p> <p>There is a 3" dimension between edge of steel plate and edge of depressed slab. Pls confirm if 6" dimension is acceptable in lieu of 3" around the base plates Type I C , I B and Type II at 7/F.8.</p>					
<b>T-1050</b>	<b>SSS - Field Splice Framing Interference</b>	<b>Closed</b>	<b>CR</b>	<b>12/19/2013</b>	<b>12/29/2013</b>	<b>12/30/2013</b>
<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At two locations, TR5 & TR33.2 the framing beam end connections foul the Transfer girder field splices. Please verify beam framing adjustments shown on CD RFI # 163 SK3 & SK10 are acceptable.			At two locations, TR5 & TR33.2 the framing beam end connections foul the Transfer girder field splices. Please verify beam framing adjustments shown on CD RFI # 163 SK3 & SK10 are acceptable.			



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-1051</b>	<b>SSS - BRB Gusset Plate Connections</b>	<b>Closed</b>	<b>CR</b>	<b>12/19/2013</b>	<b>12/29/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The details on S1-4206 & S1-4207 do not provide the information required to finalize the shape of the BRB gusset plates. Please see questions below and noted on sketches CD RFI 236 SK1 & SK2. 1) Please provide a typ. minimum dimension to maintain from the edge of the Clevis plate to the corners of the gusset. See SK1 & SK2. 2) Please verify the typ. length for the gussets on 1 & 5/S1-4206, 1/S1-4207, see SK1. 3) Please verify the typ. length for the gusset on 2/S1-4206, see SK2. 4) Please verify if a typ. minimum width for the gusset on 2/S1-4206 is to be maintained or the shape of the gusset can be based from the offset of the edge of the Clevis plate to the corners of the gusset? See SK2. 5) Please verify if the 1/2" stiffener should maintain a minimum width or should the stiffener extend to the edge of the beam flange? also please verify if the corners of the stiffeners should be shaped? if so, please provide details. See SK2.			The details on S1-4206 & S1-4207 do not provide the information required to finalize the shape of the BRB gusset plates. Please see questions below and noted on sketches CD RFI 236 SK1 & SK2. 1) Please provide a typ. minimum dimension to maintain from the edge of the Clevis plate to the corners of the gusset. See SK1 & SK2. 2) Please verify the typ. length for the gussets on 1 & 5/S1-4206, 1/S1-4207, see SK1. 3) Please verify the typ. length for the gusset on 2/S1-4206, see SK2. 4) Please verify if a typ. minimum width for the gusset on 2/S1-4206 is to be maintained or the shape of the gusset can be based from the offset of the edge of the Clevis plate to the corners of the gusset? See SK2. 5) Please verify if the 1/2" stiffener should maintain a minimum width or should the stiffener extend to the edge of the beam flange? also please verify if the corners of the stiffeners should be shaped? if so, please provide details. See SK2.			
<b>T-1052</b>	<b>SSS - W10 Detail Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>12/19/2013</b>	<b>12/29/2013</b>	<b>01/02/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1. Confirm the dimensions as shown are correct and match the W-10 system. 2. The noted elevation shown on 87'-4" in details 1,4,9/S1-8008 conflicts with A1-2903. Please verify correct elevation. 3. Supply the offset from top of curb to determine the location of the 3/8" x 6" x 6" stiffener plates. 4. The 5/16" fillet weld all around is only possible on one side of the post due to the 10 1/8" flange width on the W27x114 and the limited remaining distance on the end of the beam as shown. Confirm it is acceptable to supply a 5/16" PJP weld on 3 sides.			1. Confirm the dimensions as shown are correct and match the W-10 system. 2. The noted elevation shown on 87'-4" in details 1,4,9/S1-8008 conflicts with A1-2903. Please verify correct elevation. 3. Supply the offset from top of curb to determine the location of the 3/8" x 6" x 6" stiffener plates. 4. The 5/16" fillet weld all around is only possible on one side of the post due to the 10 1/8" flange width on the W27x114 and the limited remaining distance on the end of the beam as shown. Confirm it is acceptable to supply a 5/16" PJP weld on 3 sides.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1052.1</b>	<b>SSS - W10 Detail Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>01/16/2014</b>	<b>01/26/2014</b>	<b>01/28/2014</b>
<b>From:</b> Skanska USA Civil West California DisRyan Clayton						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The response to RFI T-1052 (SK RFI 309.1), states that "The 5/16" fillet weld is on 2 sides only (not all around as stated in this RFI). The width of the stiffeners can be reduced to match the W27 beam flange width. Don't see a problem in performing the double fillet weld, however, a CJP weld to replace the double fillet weld is acceptable." The original question asked permission to use a PJP weld in lieu of the double fillet weld, not a CJP weld. Please clarify the following:			The response to RFI T-1052 (SK RFI 309.1), states that "The 5/16" fillet weld is on 2 sides only (not all around as stated in this RFI). The width of the stiffeners can be reduced to match the W27 beam flange width. Don't see a problem in performing the double fillet weld, however, a CJP weld to replace the double fillet weld is acceptable." The original question asked permission to use a PJP weld in lieu of the double fillet weld, not a CJP weld. Please clarify the following:			
1) Skanska disagrees with the note that the referenced weld is shown as being required on 2 sides only. Please review the attached SK2 and confirm the welding locations as shown are acceptable.			1) Skanska disagrees with the note that the referenced weld is shown as being required on 2 sides only. Please review the attached SK2 and confirm the welding locations as shown are acceptable.			
2.) As there is insufficient landing to perform the 5/16" fillet as originally detailed, please confirm the welding as per CD RFI 240.1 SK2 is acceptable.			2.) As there is insufficient landing to perform the 5/16" fillet as originally detailed, please confirm the welding as per CD RFI 240.1 SK2 is acceptable.			
<b>T-1053</b>	<b>SSS - Roof Park Level W40 to BU Girder Connections</b>	<b>Closed</b>	<b>CR</b>	<b>12/19/2013</b>	<b>12/29/2013</b>	<b>12/31/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
On S1-2602 to S1-2607 along lines B & H the bottom flanges of the sloping W40x264 moment beams are deeper than the BU 40 girders by 5/16" of an inch as noted on sketch CD RFI # 217 SK1.			On S1-2602 to S1-2607 along lines B & H the bottom flanges of the sloping W40x264 moment beams are deeper than the BU 40 girders by 5/16" of an inch as noted on sketch CD RFI # 217 SK1.			
1). To accommodate for the depth discrepancy verify a 1/2" plate can be added to the bottom of the BU 40 girders and the welds as noted on sketch SK1.			1). To accommodate for the depth discrepancy verify a 1/2" plate can be added to the bottom of the BU 40 girders and the welds as noted on sketch SK1.			
2). Also for the top & bottom flange welds for the W40x264 sloping beams verify the CJP weld noted on the sketch SK1.			2). Also for the top & bottom flange welds for the W40x264 sloping beams verify the CJP weld noted on the sketch SK1.			
3). Option # 2 is to move the work points of the W40x264 beams up 5/16" thus flanges would then be flush for both W40 & BU 40 members.			3). Option # 2 is to move the work points of the W40x264 beams up 5/16" thus flanges would then be flush for both W40 & BU 40 members.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1053.1</b>	<b>SSS - Roof Park Level W40 to BU Girder Connections</b>	<b>Closed</b>	<b>CR</b>	<b>01/21/2014</b>	<b>01/31/2014</b>	<b>01/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Reference the response to RFI T-1053. Per the conversation during the 1/21/14 Structural Issues Meeting, please address the following:  1) Please confirm that a 1/2" plate is acceptable as described in RFI T-1053, item #1  2) Please confirm that a CJP weld will be acceptable in lieu of a PJP weld, as described in RFI T-1053, item #2						<b>ANSWER:</b>  Reference the response to RFI T-1053. Per the conversation during the 1/21/14 Structural Issues Meeting, please address the following:  1) Please confirm that a 1/2" plate is acceptable as described in RFI T-1053, item #1  2) Please confirm that a CJP weld will be acceptable in lieu of a PJP weld, as described in RFI T-1053, item #2
<b>T-1054</b>	<b>SSS - Light Column Reference Detail Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>12/19/2013</b>	<b>12/29/2013</b>	<b>12/20/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  ASI 0106 changed the majority of the detail and section references on drawing S1-6005 that result in incomplete or incorrect traceability. These changes were not clouded. Two possible issues exist as a result of these changes:  1) Some of the revised detail/section references were revised in error, and/or 2) Some of the revised detail/section references are correct and the referenced drawing requires either a revision to match the sourced reference or the addition of a new detail/section.  Please advise.						<b>ANSWER:</b>  ASI 0106 changed the majority of the detail and section references on drawing S1-6005 that result in incomplete or incorrect traceability. These changes were not clouded. Two possible issues exist as a result of these changes:  1) Some of the revised detail/section references were revised in error, and/or 2) Some of the revised detail/section references are correct and the referenced drawing requires either a revision to match the sourced reference or the addition of a new detail/section.  Please advise.
<b>T-1055</b>	<b>SSS - Tapered Girder Connections</b>	<b>Closed</b>	<b>CR</b>	<b>12/19/2013</b>	<b>12/29/2013</b>	<b>12/30/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  At the roof Tapered girders refer to sketches CD RFI 238 SK1 to SK3 for items 1 & 2. The proposed erection method for the Tapered Girders on the roof is to shop attach both connection angles at the roof perimeter (See SK3), tip the girder into the connected position on the perimeter BU-Girders and then lower the left end between the double shear plates on the columns (see SK2). In						<b>ANSWER:</b>  At the roof Tapered girders refer to sketches CD RFI 238 SK1 to SK3 for items 1 & 2. The proposed erection method for the Tapered Girders on the roof is to shop attach both connection angles at the roof perimeter (See SK3), tip the girder into the connected position on the perimeter BU-Girders and then lower the left end between the double shear plates on the





<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
	<p>order to erect these girders confirm the dimensions may be increased as noted:</p> <p>1) With the connection angles tight against the BU-Girder on the right end, the 2" clear dimension per detail 2/S1-5016 will need to increase to 3 1/2" assuming the back-up bar will not exceed 3/4" thick. Confirm it is acceptable to increase the dimension as noted for all Tapered Girders.</p> <p>2) This 1" dimension will need to increase to 2 1/2" to be able to erect the girder as described on SK1. Confirm this is acceptable.</p>				<p>columns (see SK2). In order to erect these girders confirm the dimensions may be increased as noted:</p> <p>1) With the connection angles tight against the BU-Girder on the right end, the 2" clear dimension per detail 2/S1-5016 will need to increase to 3 1/2" assuming the back-up bar will not exceed 3/4" thick. Confirm it is acceptable to increase the dimension as noted for all Tapered Girders.</p> <p>2) This 1" dimension will need to increase to 2 1/2" to be able to erect the girder as described on SK1. Confirm this is acceptable.</p>	
<b>T-1056</b>	<b>SSS - Edge Plate Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>12/20/2013</b>	<b>12/20/2013</b>	<b>12/31/2013</b>
<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>On S1-2604 &amp; 2605 between grids 17 to 24 &amp; D to F refer to sketches CD RFI 159 SK1 to SK3 for items 1 &amp; 2 for edge plate clarification. Detail 1/S1-8000 and details 1, 3 &amp; 4/S1-8016 show edge plate on the beam. Please confirm/clarify the following items:</p> <p>1) Confirm the edge plates on the noted details is per 8/S1-5000.</p> <p>2) The vertical leg of the edge plate appears to extend above the slab but does not extend up to the construction joint. Confirm the vertical leg terminates at the top of roof slab or clarify the vertical height.</p>			<p>On S1-2604 &amp; 2605 between grids 17 to 24 &amp; D to F refer to sketches CD RFI 159 SK1 to SK3 for items 1 &amp; 2 for edge plate clarification. Detail 1/S1-8000 and details 1, 3 &amp; 4/S1-8016 show edge plate on the beam. Please confirm/clarify the following items:</p> <p>1) Confirm the edge plates on the noted details is per 8/S1-5000.</p> <p>2) The vertical leg of the edge plate appears to extend above the slab but does not extend up to the construction joint. Confirm the vertical leg terminates at the top of roof slab or clarify the vertical height.</p>			







<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1057</b>	<b>SSS - Bus Deck Level Edge of Slab Plate Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>12/20/2013</b>	<b>12/30/2013</b>	<b>01/09/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
For edge for slab framing @ slab notch refer to sketches CD RFI 234 SK1 to SK3 for items 1 to 3:			For edge for slab framing @ slab notch refer to sketches CD RFI 234 SK1 to SK3 for items 1 to 3:			
1) Work with SK2 & SK3 and supply the location, angle orientation and connection detail for the L6x6x3/8 in light of the beam flange cut-back as shown.			1) Work with SK2 & SK3 and supply the location, angle orientation and connection detail for the L6x6x3/8 in light of the beam flange cut-back as shown.			
2) Supply the location, angle orientation and a connection detail for the L6x6x3/8 in detail 2 & 4/S1-2550.			2) Supply the location, angle orientation and a connection detail for the L6x6x3/8 in detail 2 & 4/S1-2550.			
3) Supply the location, angle orientation and a connection detail for the (2) L6x6x3/8 in detail 6/S1-2550.			3) Supply the location, angle orientation and a connection detail for the (2) L6x6x3/8 in detail 6/S1-2550.			
<b>T-1057.1</b>	<b>SSS - Bus Deck Level Edge of Slab Plate Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/28/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
This is a follow-up RFI to RFI T-1057 (SK 300, CD 234) See attached CD RFI # 234.1 SK1 & SK2 for items 1 to 3:			This is a follow-up RFI to RFI T-1057 (SK 300, CD 234) See attached CD RFI # 234.1 SK1 & SK2 for items 1 to 3:			
1) Confirm the noted detail reference should read 6/S1-2550 Sim.			1) Confirm the noted detail reference should read 6/S1-2550 Sim.			
2) Confirm the angles may typically be located 1/2" from end of top beam flange as shown.			2) Confirm the angles may typically be located 1/2" from end of top beam flange as shown.			
3) Supply a connection detail for the end of the angle.			3) Supply a connection detail for the end of the angle.			
<b>T-1058</b>	<b>SSS - Brace Detail Clarifications at Spandrel Beams</b>	<b>Closed</b>	<b>CR</b>	<b>12/20/2013</b>	<b>12/30/2013</b>	<b>01/02/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At 2nd level & Bus deck level Spandrel beams refer to sketches CD RFI 235 SK1 & SK2 for items 1 to 8:			At 2nd level & Bus deck level Spandrel beams refer to sketches CD RFI 235 SK1 & SK2 for items 1 to 8:			
1) Detail 1/S1-8020 is not referenced on the structural Bus Deck Level plans. Please clarify where this detail applies.			1) Detail 1/S1-8020 is not referenced on the structural Bus Deck Level plans. Please clarify where this detail applies.			
2) Detail 4/S1-8020 is not referenced on the structural Second Level plans. Please clarify where this detail applies.			2) Detail 4/S1-8020 is not referenced on the structural Second Level plans. Please clarify where this detail applies.			
3) Supply the information showing the W-2 mullion locations to help locate the angle braces in details 1 &			3) Supply the information showing the W-2 mullion			



Webcor/Obayashi Joint Venture  
PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG  
30100 - Transbay Transit Center Project

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Date: 02/11/2015  
Time: 07:01 AM  
Job: 30100

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>4/S1-8020. 4) Confirm the work point for the brace is on beam center at top of bottom flange in details 1 &amp; 4/S1-8020. 5) Supply the work point location for the brace from top of beam in details 1 &amp; 4/S1-8020. 6) Confirm the noted plate size in details 1 &amp; 4/S1-8020 is a minimum size and may be increased to facilitate the connection. 7) Supply stitch plate requirements in details 1 &amp; 4/S1-8020. 8) Confirm the brace in detail 1/S1-8020 may be connected beyond the beam flange as shown in 4/S1-8020 (SK2) to facilitate the erection of the brace.</p>					
	<p>locations to help locate the angle braces in details 1 &amp; 4/S1-8020. 4) Confirm the work point for the brace is on beam center at top of bottom flange in details 1 &amp; 4/S1-8020. 5) Supply the work point location for the brace from top of beam in details 1 &amp; 4/S1-8020. 6) Confirm the noted plate size in details 1 &amp; 4/S1-8020 is a minimum size and may be increased to facilitate the connection. 7) Supply stitch plate requirements in details 1 &amp; 4/S1-8020. 8) Confirm the brace in detail 1/S1-8020 may be connected beyond the beam flange as shown in 4/S1-8020 (SK2) to facilitate the erection of the brace.</p>					
<b>T-1058.1</b>	<b>SSS - Brace Detail Clarifications at Spandrel Beams</b>	<b>Closed</b>	<b>CR</b>	<b>01/29/2014</b>	<b>02/08/2014</b>	<b>02/03/2014</b>
<p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>This is a follow-up RFI for RFI T-1058 (SK 301 CD 235) Please confirm the following for RFI T-1058 responses: Item 1: Confirm the correct architectural drawings showing the W-2 system on the Bus Level are A1-2502 thru A12504 and not as noted in the response.. Item 2: Confirm the correct architectural drawings showing the W-2 system on the Second Level are A1-2402 thru A1-2404 and not as noted in the response.</p>			<p>This is a follow-up RFI for RFI T-1058 (SK 301 CD 235) Please confirm the following for RFI T-1058 responses: Item 1: Confirm the correct architectural drawings showing the W-2 system on the Bus Level are A1-2502 thru A12504 and not as noted in the response.. Item 2: Confirm the correct architectural drawings showing the W-2 system on the Second Level are A1-2402 thru A1-2404 and not as noted in the response.</p>			
<b>T-1058.2</b>	<b>SSS - Brace Detail Clarifications at Spandrel Beams</b>	<b>Closed</b>	<b>CR</b>	<b>02/06/2014</b>	<b>02/16/2014</b>	<b>02/12/2014</b>
<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Per item (3) on RFI T-1058, please supply the information showing the W-2 mullion locations to help locate the angle braces in details 1 &amp; 4/S1-8020.</p>			<p>Per item (3) on RFI T-1058, please supply the information showing the W-2 mullion locations to help locate the angle braces in details 1 &amp; 4/S1-8020.</p>			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1058.3</b>	<b>SSS - W-2 Bracing Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>04/01/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
As a follow up to SK RFI 301, 301.1, and ASI 111, please review the following requesting further clarification for the W2 kicker brace locations: 1) (SK1 & SK2) Detail 1/A1-8151 indicates mullions on the W-2 curtain wall from grade level to the bus deck level. However, the section cut 2/A1-8151 only shows bracing and stiffeners at the Bus Deck level framing, not the Second Level framing. Please confirm no kicker braces or stiffeners are required at W-2 mullions at the second level.  2) (SK1) Please advise if Bus Deck Level mullion spacing shown on A1-8155 is to be pulled from grid line 10 or 10.1. It appears that the spacing is typically for a 42'-6" bay. 3) (SK3) Per A1-8151, kicker braces per 4/S1-8020 are required as shown. Please confirm. 4) (SK5) Per A1-8152, kicker braces per 4/S1-8020 are required as shown. Please confirm. 5) The mullion locations on A1-8157 cannot be located with the information provided. Please supply the dimensions off grid lines to locate the bracing and mullions. 6) It appears that W-2 mullions occur along the entire wall on grid line 1.4, however A1-8157 only shows mullions between grid lines C and D.4. If mullions occur south of GL D.4, please provide the dimensions required to locate the mullions, braces, and stiffeners. 7) A1-8158 is shown on grid line 2.5. Please advise if A1-8158 is to be mirrored on GL 3.5 or provide the required mullion spacing. 8) A1-8158 does not provide the locations of the mullions. Please supply the required dimensions off grid lines. 9) A1-8159 is shown on grid 8.5. Please advise if this elevation also applies on grid 9.5 or provide the required mullion spacing. 10) The locations of the mullions cannot be located with the information provided on A1-8159. Please provide a reference dimension off a grid line to locate the mullions. 11) A1-8152 is reference on A1-8150 between grid lines 2 and 3, with grid lines 2 and 3 shown on the elevation. Please confirm this detail also applies at the corners on grid lines 4, 8, 10, 12, and 14. 12) Please confirm details 1 and 4 on S1-8020 do not apply to drawings A1-8156, A1-8165, A1-8166. 13) A1-5167 is reference on the south portion of grid 16.9. Please advise if this detail also applies on the north portion of grid 16.9.			As a follow up to SK RFI 301, 301.1, and ASI 111, please review the following requesting further clarification for the W2 kicker brace locations: 1) (SK1 & SK2) Detail 1/A1-8151 indicates mullions on the W-2 curtain wall from grade level to the bus deck level. However, the section cut 2/A1-8151 only shows bracing and stiffeners at the Bus Deck level framing, not the Second Level framing. Please confirm no kicker braces or stiffeners are required at W-2 mullions at the second level. 2) (SK1) Please advise if Bus Deck Level mullion spacing shown on A1-8155 is to be pulled from grid line 10 or 10.1. It appears that the spacing is typically for a 42'-6" bay. 3) (SK3) Per A1-8151, kicker braces per 4/S1-8020 are required as shown. Please confirm. 4) (SK5) Per A1-8152, kicker braces per 4/S1-8020 are required as shown. Please confirm. 5) The mullion locations on A1-8157 cannot be located with the information provided. Please supply the dimensions off grid lines to locate the bracing and mullions. 6) It appears that W-2 mullions occur along the entire wall on grid line 1.4, however A1-8157 only shows mullions between grid lines C and D.4. If mullions occur south of GL D.4, please provide the dimensions required to locate the mullions, braces, and stiffeners. 7) A1-8158 is shown on grid line 2.5. Please advise if A1-8158 is to be mirrored on GL 3.5 or provide the required mullion spacing. 8) A1-8158 does not provide the locations of the mullions. Please supply the required dimensions off grid lines. 9) A1-8159 is shown on grid 8.5. Please advise if this elevation also applies on grid 9.5 or provide the required mullion spacing. 10) The locations of the mullions cannot be located with the information provided on A1-8159. Please provide a reference dimension off a grid line to locate the mullions. 11) A1-8152 is reference on A1-8150 between grid lines 2 and 3, with grid lines 2 and 3 shown on the elevation. Please confirm this detail also applies at the corners on grid lines 4, 8, 10, 12, and 14. 12) Please confirm details 1 and 4 on S1-8020 do not			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
	<p>14) It appears that partial dimensions are provided (between grid lines E.6-F.7) for the mullion locations on A1-8161 along grid line 27 on the Second Level. Please provide the remaining dimensions required to locate the mullions along the remainder of grid line 27.</p> <p>15) No detail reference is provided on the north half of grid line 27 and grid line 32.5. Please provide the mullion spacing requirements at these locations.</p> <p>16) A1-8156 is referenced on grid line F.7 between grid lines 27 and 32.5 on the Second Level, but this detail does not appear to be correct. Please confirm the reference should be A1-8160 or provide the corrected reference.</p>					<p>apply to drawings A1-8156, A1-8165, A1-8166.</p> <p>13) A1-5167 is reference on the south portion of grid 16.9. Please advise if this detail also applies on the north portion of grid 16.9.</p> <p>14) It appears that partial dimensions are provided (between grid lines E.6-F.7) for the mullion locations on A1-8161 along grid line 27 on the Second Level. Please provide the remaining dimensions required to locate the mullions along the remainder of grid line 27.</p> <p>15) No detail reference is provided on the north half of grid line 27 and grid line 32.5. Please provide the mullion spacing requirements at these locations.</p> <p>16) A1-8156 is referenced on grid line F.7 between grid lines 27 and 32.5 on the Second Level, but this detail does not appear to be correct. Please confirm the reference should be A1-8160 or provide the corrected reference.</p>
<b>T-1059</b>	<b>SSS - EOS Closure Details at Columns</b>	<b>Closed</b>	<b>CR</b>	<b>12/20/2013</b>	<b>12/30/2013</b>	<b>01/10/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>At the 2nd level at sample locations on S1-2403 @ grids 9.9/C &amp; G please verify the edge of slab closure detail at the column grid lines per detail 1/S1-5004 shown on sketch CD RFI 246 SK1 is acceptable.</p>						<b>ANSWER:</b> <p>At the 2nd level at sample locations on S1-2403 @ grids 9.9/C &amp; G please verify the edge of slab closure detail at the column grid lines per detail 1/S1-5004 shown on sketch CD RFI 246 SK1 is acceptable.</p>
<b>T-1060</b>	<b>SSS - Shop Primer Coat Exclusion Areas</b>	<b>Closed</b>	<b>CR</b>	<b>12/20/2013</b>	<b>12/30/2013</b>	<b>12/26/2013</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>Specification section 05 10 003.2 P.3b specifically excludes shop paint from areas to be enclosed in concrete and cementitious fireproofing. Drawing A-8662 matrix shows 3 different types of fireproofing, SFRM, IFRM-1 and IFRM-2. Please confirm which of these are cement based so we can determine shop painting limits.</p>						<b>ANSWER:</b> <p>Specification section 05 10 003.2 P.3b specifically excludes shop paint from areas to be enclosed in concrete and cementitious fireproofing. Drawing A-8662 matrix shows 3 different types of fireproofing, SFRM, IFRM-1 and IFRM-2. Please confirm which of these are cement based so we can determine shop painting limits.</p>







<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1060.2</b>	<b>SSS - Shop Primer Coat Exclusion Areas</b>	<b>Closed</b>	<b>CR</b>	<b>01/27/2014</b>	<b>02/06/2014</b>	<b>02/12/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Details 5, 6, and 7/A1-8662 indicate "12 inches of fireproofing required on stiffener fins, typical."			Details 5, 6, and 7/A1-8662 indicate "12 inches of fireproofing required on stiffener fins, typical."			
1) Please provide a UL assembly # and details for conditions where cruciform columns are enclosed with exterior wall cladding or interior furred-out walls.			1) Please provide a UL assembly # and details for conditions where cruciform columns are enclosed with exterior wall cladding or interior furred-out walls.			
2) Structural drawings reference cruciform columns while A1-8662 references "stiffener fins." Please advise if the A1-8662 drawings are intended to show the cruciform columns, and provide revised drawings as necessary. Please provide the applicable UL assembly for cruciform columns with any revised details.			2) Structural drawings reference cruciform columns while A1-8662 references "stiffener fins." Please advise if the A1-8662 drawings are intended to show the cruciform columns, and provide revised drawings as necessary. Please provide the applicable UL assembly for cruciform columns with any revised details.			
3) Please advise if the cruciform columns are to be fireproofed SFRM-1 per spec section 07 81 00			3) Please advise if the cruciform columns are to be fireproofed SFRM-1 per spec section 07 81 00			
<b>T-1060.3</b>	<b>SSS - Finish Requirements at Isolation Bearings</b>	<b>Closed</b>	<b>CR</b>	<b>02/13/2014</b>	<b>02/23/2014</b>	<b>02/20/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1) As noted in the sketch below, exposed surfaces of the isolation bearing pads are to be hot dip galvanized per ASTM A123. Please confirm this is acceptable in accordance with specification section 13 48 63-2.3.A.2.			1) As noted in the sketch below, exposed surfaces of the isolation bearing pads are to be hot dip galvanized per ASTM A123. Please confirm this is acceptable in accordance with specification section 13 48 63-2.3.A.2.			
2) In accordance with A1-8662 and the response to WOJV T-1060.1, all other steel associated with isolation bearings, such as those members shown on S1-5021, will be bare steel. This is in anticipation of receiving SFRM by others. Please confirm.			2) In accordance with A1-8662 and the response to WOJV T-1060.1, all other steel associated with isolation bearings, such as those members shown on S1-5021, will be bare steel. This is in anticipation of receiving SFRM by others. Please confirm.			
<b>T-1060.4</b>	<b>SSS - Ancillary Steel Fireproofing Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>02/26/2014</b>	<b>03/08/2014</b>	<b>03/06/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference A1-8662 which indicates that beams and columns from the Lower Concourse level to the Roof Park Level, as well as beams at the Roof Level, are to receive			Reference A1-8662 which indicates that beams and columns from the Lower Concourse level to the Roof Park Level, as well as beams at the Roof Level, are to			



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	<p>SFRM. As noted in the response to WOJV RFI T-1060.2, it is typically recommended by the fire protection manufacturer that attachments to primary structural members are fire proofed to the same level as the protected structural elements to mitigate thermal bridging.</p> <p>Please confirm that all ancillary components for beams and columns scheduled to receive SFRM are also to be prepped to receive SFRM. This includes, but is not limited to, kicker braces, hangers, stiffeners, connection plates, gusset plates, outriggers, and connection angles.</p>					
T-1061	<p><b>SSS - Weld Access Hole Details at Column Webs to Base &amp; Cap Plates</b></p> <p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>On details 4/S1-5052 &amp; 1/S1-5052 (sim.) refer to sketch CD RFI 134B.1 SK1 and verify the weld access hole size and radius in the column webs is acceptable.</p>	Closed	CR	12/20/2013	12/30/2013	12/30/2013







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	<p>per Detail A. If future devices are to be installed on that column at a different height, then an extension box can be installed, and conduit can be run from the extension box on the surface of the column.</p> <p>Please confirm this is acceptable.</p>					<p>column at a height of 13'-9" to center per Detail A. If future devices are to be installed on that column at a different height, then an extension box can be installed, and conduit can be run from the extension box on the surface of the column.</p> <p>Please confirm this is acceptable.</p>
<b>T-1065</b>	<b>BGP - Elevation Discrepancy at Escalator Pit near GL 21/E.2</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/02/2014</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> <p>Please refer to attached drawing S1-2205, S1-7660 and SKA-2919.</p> <p>The depth from the concourse TOC to the TOC in the pit conflicts in Details 10 and 11 of drawing sheet S1-7660. Detail 11 shows a distance of 4-feet from the concourse finished floor to the TOC in the pit. As drawn, this indicates a concourse TOC to pit TOC depth of 3'-9". Detail 10 shows the dimensions between the concourse TOC and the bottom of the slab in the pit as 4-feet, and this indicates a concourse TOC to pit TOC depth of 3'-7". Elevations provided in SK2919 also indicate the concourse TOC to pit TOC distance is 3'-7".</p> <p>Please clarify the correct depth dimensions for the pits represented in Details 10 and 11 of S1-7660.</p>						<b>ANSWER:</b> <p>Please refer to attached drawing S1-2205, S1-7660 and SKA-2919.</p> <p>The depth from the concourse TOC to the TOC in the pit conflicts in Details 10 and 11 of drawing sheet S1-7660. Detail 11 shows a distance of 4-feet from the concourse finished floor to the TOC in the pit. As drawn, this indicates a concourse TOC to pit TOC depth of 3'-9". Detail 10 shows the dimensions between the concourse TOC and the bottom of the slab in the pit as 4-feet, and this indicates a concourse TOC to pit TOC depth of 3'-7". Elevations provided in SK2919 also indicate the concourse TOC to pit TOC distance is 3'-7".</p> <p>Please clarify the correct depth dimensions for the pits represented in Details 10 and 11 of S1-7660.</p>
<b>T-1066</b>	<b>BGP - Moment Frame Beam and Column Conflict GL 21</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/09/2014</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> <p>Please refer to attached drawing S1-2025, S1-3304 ans S1-3621.</p> <p>Please confirm that the moment frame beam at GL 21 is 66-inches. Columns at GL C21 and G21 are 68-inches which make them 2-inches wider than the moment frame beam.</p>						<b>ANSWER:</b> <p>Please refer to attached drawing S1-2025, S1-3304 ans S1-3621.</p> <p>Please confirm that the moment frame beam at GL 21 is 66-inches. Columns at GL C21 and G21 are 68-inches which make them 2-inches wider than the moment frame beam.</p>



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<b>T-1067</b>	<b>SSS - Stair and Elevator Connections</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>12/30/2013</b>	<b>01/13/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>		<b>ANSWER:</b>				
For typical stair & elevator connections refer to sketches CD RFI 181 SK1 to SK3 for items 1 to 11: 1. Please consider attached detail (CD RFI 181 SK4) as an alternative for elevator post bases shown in 1/S1-7600 and 10/S1-7600. 2. Confirm a 1/16" gap between post and angle on each side is acceptable (CD RFI 181 SK1). 3. Plate washers are not shown for the slotted holes for 1" dia. A307 bolts. Are they required? (CD RFI 181 SK1) 4. Supply dimensions for kicker brace connections to composite deck requested on (CD RFI 181 SK2) 5. Confirm gusset and hole dimensions at top of kicker brace (Detail E) are same as shown for bottom of brace connection shown in CD RFI 181 SK2B.		For typical stair & elevator connections refer to sketches CD RFI 181 SK1 to SK3 for items 1 to 11: 1. Please consider attached detail (CD RFI 181 SK4) as an alternative for elevator post bases shown in 1/S1-7600 and 10/S1-7600. 2. Confirm a 1/16" gap between post and angle on each side is acceptable (CD RFI 181 SK1). 3. Plate washers are not shown for the slotted holes for 1" dia. A307 bolts. Are they required? (CD RFI 181 SK1) 4. Supply dimensions for kicker brace connections to composite deck requested on (CD RFI 181 SK2) 5. Confirm gusset and hole dimensions at top of kicker brace (Detail E) are same as shown for bottom of brace connection shown in CD RFI 181 SK2B.				
<b>T-1067.1</b>	<b>SSS - Stair and Elevator Connections</b>	<b>Closed</b>	<b>CR</b>	<b>03/18/2014</b>	<b>03/28/2014</b>	<b>03/31/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached SK1 & 2 for items 1 & 2: 1) Details B, C & D are not practical as it is not known where the deck bottom flutes will be located at the time of modeling this project. As designed it is not possible to position the brace so the anchor bolts will be located on the centers of bottom flutes. Please confirm it is acceptable to model an oversized plate with additional staggered holes to account for the unknown position of the bottom flutes or supply an alternate detail.  2) Confirm a vertical short slot can be provided in the angle of the top bolted connection to allow for fabrication tolerances of the WF beam.		See attached SK1 & 2 for items 1 & 2: 1) Details B, C & D are not practical as it is not known where the deck bottom flutes will be located at the time of modeling this project. As designed it is not possible to position the brace so the anchor bolts will be located on the centers of bottom flutes. Please confirm it is acceptable to model an oversized plate with additional staggered holes to account for the unknown position of the bottom flutes or supply an alternate detail.  2) Confirm a vertical short slot can be provided in the angle of the top bolted connection to allow for fabrication tolerances of the WF beam.				
<b>T-1067.2</b>	<b>SSS - Stair and Elevator Connections</b>	<b>Closed</b>	<b>CR</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/28/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>		<b>ANSWER:</b>				



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T-1068	<p>As per the response to T-1067.1 Skanska confirms these details will require field adjustment and proposes the following as per SK1:</p> <p>1) Confirm it is acceptable to (a) field weld the bottom connection after aligning the top connection to center of low flutes when deck flutes are parallel to kicker and (b) field drill the top connection after aligning the L6 to center of low flutes when deck flutes are perpendicular to the kicker.</p> <p>2) Confirm the diameter of expansion bolt required.</p> <p>3) Confirm the field drilled top connection is acceptable at deck flutes perpendicular to the kicker as this will vary from the direction in T-1067 #5.</p> <p><b>SSS - Perimeter Connections at GL C&amp;G</b></p> <p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>Details 7 &amp; 8/S1-3703 are shown on sheet S1-2305 as typical sections for beams connecting perpendicular to the perimeter BU &amp; WF beams at grid lines C &amp; G. These sections reflect the varying elevation differences between the two members. In most conditions, the remaining depth of the beam framing into the perimeter BU or WF will only allow for a two bolt connection as shown in details 7 &amp; 8/S1-3703.</p> <p>1) Please confirm it is acceptable to use a two bolt shear plate connection for any beam size where the remaining depth of the connecting beam will only allow for two bolts. The shear plate thickness and welding will be per the schedule on 1/S1-5011.</p> <p>2) Please confirm at some locations it is acceptable to cut the flange flush on one side of the beam to maintain edge distance.</p> <p>3) Please confirm edge distance can be reduced where needed to complete connection.</p> <p>4) Please confirm a double angle connection should be used when the varying elevations will allow for more than a two bolt connection. The angle size &amp; thickness will be per the schedule on 1/S1-5010.</p> <p>5) Please confirm the maximum amount of bolts that will be used would be based on the remaining depth of the connecting beam.</p>	Closed	CR	12/30/2013	01/09/2014	01/13/2014
	<p><b>ANSWER:</b></p> <p>As per the response to T-1067.1 Skanska confirms these details will require field adjustment and proposes the following as per SK1:</p> <p>1) Confirm it is acceptable to (a) field weld the bottom connection after aligning the top connection to center of low flutes when deck flutes are parallel to kicker and (b) field drill the top connection after aligning the L6 to center of low flutes when deck flutes are perpendicular to the kicker.</p> <p>2) Confirm the diameter of expansion bolt required.</p> <p>3) Confirm the field drilled top connection is acceptable at deck flutes perpendicular to the kicker as this will vary from the direction in T-1067 #5.</p> <p>Details 7 &amp; 8/S1-3703 are shown on sheet S1-2305 as typical sections for beams connecting perpendicular to the perimeter BU &amp; WF beams at grid lines C &amp; G. These sections reflect the varying elevation differences between the two members. In most conditions, the remaining depth of the beam framing into the perimeter BU or WF will only allow for a two bolt connection as shown in details 7 &amp; 8/S1-3703.</p> <p>1) Please confirm it is acceptable to use a two bolt shear plate connection for any beam size where the remaining depth of the connecting beam will only allow for two bolts. The shear plate thickness and welding will be per the schedule on 1/S1-5011.</p> <p>2) Please confirm at some locations it is acceptable to cut the flange flush on one side of the beam to maintain edge distance.</p> <p>3) Please confirm edge distance can be reduced where needed to complete connection.</p> <p>4) Please confirm a double angle connection should be used when the varying elevations will allow for more than a two bolt connection. The angle size &amp; thickness will be per the schedule on 1/S1-5010.</p> <p>5) Please confirm the maximum amount of bolts that will be used would be based on the remaining depth of the connecting beam.</p>					



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<b>T-1069</b>	<b>SSS - Connection at Crash Rail Supports</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/09/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At the Bus deck level at the Crash Rail supports beams, verify when larger beams are framing into smaller beams that details 1/S1-5031 will be used with detail 1/S1-5011 for the number of bolts required. At sample locations on S1-2502 & S1-25 03, refer to sketches CD RFI 248 SK1 to SK3 and verify the 3 Types indicated.			At the Bus deck level at the Crash Rail supports beams, verify when larger beams are framing into smaller beams that details 1/S1-5031 will be used with detail 1/S1-5011 for the number of bolts required. At sample locations on S1-2502 & S1-25 03, refer to sketches CD RFI 248 SK1 to SK3 and verify the 3 Types indicated.			
Note: The other ends of the beams in question are connected per the typical detail 1/S1-8000 at the grid lines unless indicated with a moment connection.			Note: The other ends of the beams in question are connected per the typical detail 1/S1-8000 at the grid lines unless indicated with a moment connection.			
<b>T-1070</b>	<b>SSS - Connection Clarification at Escalator Areas</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/16/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1). On 1/S1-7303 at Escalator E309 & E310 at detail 5/S1-7661 verify 4 - 7/8" A325N (non TC) bolts can be used in lieu of the 5/16" field weld that would be required, see sketches CD RFI 243 SK1 & SK2 for reference.			1). On 1/S1-7303 at Escalator E309 & E310 at detail 5/S1-7661 verify 4 - 7/8" A325N (non TC) bolts can be used in lieu of the 5/16" field weld that would be required, see sketches CD RFI 243 SK1 & SK2 for reference.			
2). Per detail 5/S1-7661 verify the stiffener plates are 2/3/4" wide to match the beam flange with as noted on sketch CD RFI 243 SK2.			2). Per detail 5/S1-7661 verify the stiffener plates are 2/3/4" wide to match the beam flange with as noted on sketch CD RFI 243 SK2.			
<b>T-1071</b>	<b>SSS - Edge of Slab Support at Protected Zones</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/13/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At the Bus deck level (S1-2503) @ grids D/9.9 & 10.1 and F/9.9 & 10.1 the 3/8" edge of slab bent plate protrudes 3'-4 & 2'-3 from grid lines 9.9 & 10.1 respectively as shown on sketch CD RFI 244 SK1. Due to the 5'-0 protected zone at these lo cations the angle supports per detail 9/S1-5000 cannot be attached due to no welding is allowed in this area. Please advise on this non supported area and other similar type areas where no welding is allowed in the protected zones.			At the Bus deck level (S1-2503) @ grids D/9.9 & 10.1 and F/9.9 & 10.1 the 3/8" edge of slab bent plate protrudes 3'-4 & 2'-3 from grid lines 9.9 & 10.1 respectively as shown on sketch CD RFI 244 SK1. Due to the 5'-0 protected zone at these lo cations the angle supports per detail 9/S1-5000 cannot be attached due to no welding is allowed in this area. Please advise on this non supported area and other similar type areas where no welding is allowed in the protected zones.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1071.1</b>	<b>SSS - Edge of slab support @ protected zones</b>	<b>Closed</b>	<b>CR</b>	<b>02/11/2014</b>	<b>02/21/2014</b>	<b>02/24/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> 1). Per the response to Webcor RFI # T-1071 (SK RFI # 313) please verify the bolted end connections for the new L5x5x3/8 angle now required as shown on sketch CD RFI # 244.1 SK1. 2). To eliminate any overhead field welding confirm the bent plate required will be shop attached to the angle.						<b>ANSWER:</b> 1). Per the response to Webcor RFI # T-1071 (SK RFI # 313) please verify the bolted end connections for the new L5x5x3/8 angle now required as shown on sketch CD RFI # 244.1 SK1. 2). To eliminate any overhead field welding confirm the bent plate required will be shop attached to the angle.
<b>T-1072</b>	<b>SSS - Clarify Beam Connections at Protected Zones</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/07/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> At the Bus deck level near grids 10.1/F & 10.1/D please verify the double angle connections for the W16 beams can partially connect into the protected zones for the BU moment girders as shown on sketch CD RFI 247 SK1. If not please supply an alternate connection.						<b>ANSWER:</b> At the Bus deck level near grids 10.1/F & 10.1/D please verify the double angle connections for the W16 beams can partially connect into the protected zones for the BU moment girders as shown on sketch CD RFI 247 SK1. If not please supply an alternate connection.
<b>T-1073</b>	<b>SSS - North Exit Mezzanine Support</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/24/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> Please refer to detail 1/S1-2252 in regards to the following clarifications for the North Exit Mezzanine:  1.) Please provide connection details for MC4x13.8 channels framing into the W12x40 beam, CMU wall, and adjacent MC4x13.8 members. 2.) Please confirm how MC4x13.8 channel, east of GL 24, is supported at east end. 3.) Please provide the required dimension to locate the east end of the W12x40 member. 4.) Bracing for the W12x40 appears to be located slightly west of CL 23. Please provide the indicated dimension to locate bracing. Verify that this is the only location to receive bracing along the length of the W12x40. 5.) a. Please confirm the splice locations indicated on SK1 for the W12x40 beam are acceptable. Note that the splice just west of CL 23 may need to be shifted slightly depending on the response to item #3.						<b>ANSWER:</b> Please refer to detail 1/S1-2252 in regards to the following clarifications for the North Exit Mezzanine:  1.) Please provide connection details for MC4x13.8 channels framing into the W12x40 beam, CMU wall, and adjacent MC4x13.8 members. 2.) Please confirm how MC4x13.8 channel, east of GL 24, is supported at east end. 3.) Please provide the required dimension to locate the east end of the W12x40 member. 4.) Bracing for the W12x40 appears to be located slightly west of CL 23. Please provide the indicated dimension to locate bracing. Verify that this is the only location to receive bracing along the length of the W12x40. 5.) a. Please confirm the splice locations indicated on SK1 for the W12x40 beam are acceptable. Note that the splice just west of CL 23 may need to be shifted



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
<p>b. Please provide a splice detail for the W12x40. Note that bolted splice connections are preferred.</p>						
<p>slightly depending on the response to item #3. b. Please provide a splice detail for the W12x40. Note that bolted splice connections are preferred.</p>						
<b>T-1073.1</b>	<b>SSS - North Exit Mezzanine Support</b>	<b>Closed</b>	<b>CR</b>	<b>12/08/2014</b>	<b>12/18/2014</b>	<b>12/19/2014</b>
<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: 1/S1-2252 Location: Zone 3, Mezzanine Grid Line: B & 23 Add'l Doc Ref's: CD RFI # 206.1 SK1, RFI T-1073 Response			Contract Doc Ref: 1/S1-2252 Location: Zone 3, Mezzanine Grid Line: B & 23 Add'l Doc Ref's: CD RFI # 206.1 SK1, RFI T-1073 Response			
RFI T-1073 requested connection details for the MC4x13.8 channels froming into the W12x40 beam, CMU wall, and adjacent MC4x13.8 members.			RFI T-1073 requested connection details for the MC4x13.8 channels froming into the W12x40 beam, CMU wall, and adjacent MC4x13.8 members.			
The response to for the channel to channel connection is inadequate. Fillet welding the connection all around is not workable, as the top and bottom surfaces are flush.			The response to for the channel to channel connection is inadequate. Fillet welding the connection all around is not workable, as the top and bottom surfaces are flush.			
Confirm the connection as shown in SK1 is acceptable or supply another workable connection.			Confirm the connection as shown in SK1 is acceptable or supply another workable connection.			





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<b>T-1074</b>	<b>SSS - Crash Rail at Bus Deck</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2013</b>	<b>01/09/2014</b>	<b>01/13/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
On the Bus deck level, at the Crash Rail detail 1/S1-8000, refer to sketches CD RFI 242 SK1 to SK3 for items 1 to 3:			On the Bus deck level, at the Crash Rail detail 1/S1-8000, refer to sketches CD RFI 242 SK1 to SK3 for items 1 to 3:			
1.) Confirm the noted weld is acceptable.			1.) Confirm the noted weld is acceptable.			
2.) Confirm the noted weld is acceptable.			2.) Confirm the noted weld is acceptable.			
3.) Please confirm it is acceptable to provide a 7/8" plate in lieu of a 13/16" plate, as a 13/16" plate to match the flange thickness is not available. Note this creates a 1/16" gap between the top of the stiffener and underside of the beam flange as indicated in SK3. Please confirm this is acceptable or provide an alternate solution.			3.) Please confirm it is acceptable to provide a 7/8" plate in lieu of a 13/16" plate, as a 13/16" plate to match the flange thickness is not available. Note this creates a 1/16" gap between the top of the stiffener and underside of the beam flange as indicated in SK3. Please confirm this is acceptable or provide an alternate solution.			
<b>T-1075</b>	<b>SSS - Girder Weld Details</b>	<b>Closed</b>	<b>CR</b>	<b>12/31/2013</b>	<b>01/10/2014</b>	<b>01/08/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
For girder weld details, refer to detail 7/S1-4202 & CD RFI 241 SK1 for the following:			For girder weld details, refer to detail 7/S1-4202 & CD RFI 241 SK1 for the following:			
1). Please verify that holes are not required in the built up members as shown on CD RFI SK1.			1). Please verify that holes are not required in the built up members as shown on CD RFI SK1.			
2). Please verify the noted welds as shown on CD RFI SK1.			2). Please verify the noted welds as shown on CD RFI SK1.			
3). Please verify the weld transition as shown on CD RFI SK1.			3). Please verify the weld transition as shown on CD RFI SK1.			
<b>T-1076</b>	<b>SSS - Transfer Girder Stiffener &amp; Shear Plates</b>	<b>Closed</b>	<b>CR</b>	<b>12/31/2013</b>	<b>01/10/2014</b>	<b>01/07/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1). At the Transfer girder stiffener & shear plates noted on S1-5052 & 2/S1-4350 verify the plates corner access hole size with a 1/2" radius when the stiffener & shear plates are welded with a CJP prep as noted on sketch CD RFI # 166.1 SK1 is acceptable.			1). At the Transfer girder stiffener & shear plates noted on S1-5052 & 2/S1-4350 verify the plates corner access hole size with a 1/2" radius when the stiffener & shear plates are welded with a CJP prep as noted on sketch CD RFI # 166.1 SK1 is acceptable.			





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T-1077	<b>Bracing removal/re-bracing sequence on the west end of Zone 1</b>  From: Webcor Construction LP      Michael Spillane	Closed	01	01/02/2014	01/12/2014	01/13/2014
<b>REQUEST:</b>  Bracing removal/re-bracing sequence on the west end of Zone 1 WOJV is proposing the following sequence for the re-bracing/ Bracing removal for the west side of Zone 1 See sketches SK-1, 2 & 3 attached.  For level C strut removal see sequence on attached sketch SK1. WOJV is proposing to remove level C bracing in three defined areas. 1. Remove level C Cross lot struts and walers from east to west direction once the walls and RB re-bracing is installed and stressed. 2. Remove level C struts and walers from south west corner once the walls and RB re-bracing rakers beneath are installed. 3. Remove level C struts and walers from north west corner once the walls and RB rakers beneath have been installed.  For level B strut removal see sequence and defined areas on attached sketch SK2 1. Remove level B struts and walers from east to west direction once the lower concourse slab beneath has been place, cured and reached the required design strength. 2. Remove level B struts and walers from south west and north west corner once the lower concourse slab beneath has been place, cured and reached the required design strength.  For level A strut removal see sequence on attached sketch SK3 1. Remove level A cross lot struts and walers from east to west direction once the RA re-bracing is installed and		<b>ANSWER:</b>  Bracing removal/re-bracing sequence on the west end of Zone 1 WOJV is proposing the following sequence for the re-bracing/ Bracing removal for the west side of Zone 1 See sketches SK-1, 2 & 3 attached.  For level C strut removal see sequence on attached sketch SK1. WOJV is proposing to remove level C bracing in three defined areas. 1. Remove level C Cross lot struts and walers from east to west direction once the walls and RB re-bracing is installed and stressed. 2. Remove level C struts and walers from south west corner once the walls and RB re-bracing rakers beneath are installed. 3. Remove level C struts and walers from north west corner once the walls and RB rakers beneath have been installed.  For level B strut removal see sequence and defined areas on attached sketch SK2 1. Remove level B struts and walers from east to west direction once the lower concourse slab beneath has been place, cured and reached the required design strength. 2. Remove level B struts and walers from south west and north west corner once the lower concourse slab beneath has been place, cured and reached the required design strength.  For level A strut removal see sequence on attached sketch SK3 1. Remove level A cross lot struts and walers from				



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	<p>stressed.</p> <p>2. Remove level A struts and walers from south west corner once all the RA re-bracing rakers and +7.00' diaphragm slab beneath have been installed.</p> <p>3. Remove level A struts and walers from north west corner once the RA re-bracing rakers beneath have been installed.</p> <p>Please confirm if this sequence would be acceptable</p>					<p>east to west direction once the RA re-bracing is installed and stressed.</p> <p>2. Remove level A struts and walers from south west corner once all the RA re-bracing rakers and +7.00' diaphragm slab beneath have been installed.</p> <p>3. Remove level A struts and walers from north west corner once the RA re-bracing rakers beneath have been installed.</p> <p>Please confirm if this sequence would be acceptable</p>
T-1078	<p><b>SSS - Machine Type 1 Drag Connection Pads</b></p> <p>From: Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>The Type 1 Drag Connection shear plates are shown on drawing S1-5016 to be oriented perpendicular to the connection face of the cast node and further they are shown to be centered with respect to the width of the connection face. OIW has discovered that this is in error; the shear plates are neither centered on the face nor do they project perpendicular from the face. These conditions significantly increase the complexity of this welded joint.</p> <p>OIW would like to use a CNC milling machine to prepare the surface of the Type 1 Drag Connection pads on the cast nodes in order to provide a perpendicular surface for the shear plates to attach to. Please see attached sketch showing proposed machining.</p> <p>1. Please indicate if it is acceptable to machine these surfaces.</p> <p>2. Please indicate if there is adequate stock to allow machining of these surfaces or if additional stock must be added.</p>	Closed	CR	01/02/2014	01/12/2014	01/15/2014
						<p><b>ANSWER:</b></p> <p>The Type 1 Drag Connection shear plates are shown on drawing S1-5016 to be oriented perpendicular to the connection face of the cast node and further they are shown to be centered with respect to the width of the connection face. OIW has discovered that this is in error; the shear plates are neither centered on the face nor do they project perpendicular from the face. These conditions significantly increase the complexity of this welded joint.</p> <p>OIW would like to use a CNC milling machine to prepare the surface of the Type 1 Drag Connection pads on the cast nodes in order to provide a perpendicular surface for the shear plates to attach to. Please see attached sketch showing proposed machining.</p> <p>1. Please indicate if it is acceptable to machine these surfaces.</p> <p>2. Please indicate if there is adequate stock to allow machining of these surfaces or if additional stock must be added.</p>
T-1078.1	<p><b>SSS - Machine Type 1 Drag Connection Pads</b></p> <p>From: Webcor Construction LP                      Gregory Kemerer</p>	Closed	CR	02/25/2014	03/07/2014	03/04/2014



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T-1079	<b>Bracing removal-rebracing sequence on the East end of Zone 4</b>	Closed	01	01/02/2014	01/12/2014	01/13/2014
	<b>From:</b> Webcor Construction LP      Michael Spillane					
T-1079	<b>REQUEST:</b>  The response to Skanska RFI 294 (W/O T-1078) states that machining the internal drag pads is Contractors means and methods. In order to simplify modelling, detailing, fabrication and welding of the drag pads, Skanska's fabricator OIW intends to machine the internal drag pads.  In so doing, there is potential that the "minimum 1-inch extrusion" called out for the drag pads on the bus deck nodes on the structural drawings may be violated in some cases (refer to 1/S1-5121 attached).  Assuming that this minimum dimension was provided to accommodate fabrication, and given that the responsibility for fabrication and erection is with Skanska, please confirm that it would be acceptable for Skanska to violate the 1inch minimum extrusion with machining of the pad.  With the design team's approval, Skanska hereby proposes to move forward with machining of the pads without the submission of RFIs for every bus deck node. RFIs will only be submitted for those cases where the planned machining of the pad may slightly undercut an adjacent pad or where the planned machining may slightly bite into the main body of the node.					
	<b>ANSWER:</b>  The response to Skanska RFI 294 (W/O T-1078) states that machining the internal drag pads is Contractors means and methods. In order to simplify modelling, detailing, fabrication and welding of the drag pads, Skanska's fabricator OIW intends to machine the internal drag pads.  In so doing, there is potential that the "minimum 1-inch extrusion" called out for the drag pads on the bus deck nodes on the structural drawings may be violated in some cases (refer to 1/S1-5121 attached).  Assuming that this minimum dimension was provided to accommodate fabrication, and given that the responsibility for fabrication and erection is with Skanska, please confirm that it would be acceptable for Skanska to violate the 1inch minimum extrusion with machining of the pad.  With the design team's approval, Skanska hereby proposes to move forward with machining of the pads without the submission of RFIs for every bus deck node. RFIs will only be submitted for those cases where the planned machining of the pad may slightly undercut an adjacent pad or where the planned machining may slightly bite into the main body of the node.					
T-1079	<b>REQUEST:</b>  Bracing removal/re-bracing sequence on the East end of Zone 4 WOJV is proposing the following sequence for the re-bracing/ Bracing removal for the East side of Zone 4 See sketches SK1, 2, 3 & 4 attached.  For level D strut removal see sequence on attached sketch SK1. WOJV is proposing to remove level D bracing in two defined areas. 1. Remove level D Cross lot struts and walers from west to east direction once the mat slab beneath has been place, cured and reached adequate strength.					
	<b>ANSWER:</b>  Bracing removal/re-bracing sequence on the East end of Zone 4 WOJV is proposing the following sequence for the re-bracing/ Bracing removal for the East side of Zone 4 See sketches SK1, 2, 3 & 4 attached.  For level D strut removal see sequence on attached sketch SK1. WOJV is proposing to remove level D bracing in two defined areas. 1. Remove level D Cross lot struts and walers from west to east direction once the mat slab beneath has been place, cured and reached adequate strength.					

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	W33x130.  If not, supply an alternate detail.				connection for the W33x130.  If not, supply an alternate detail.	
<b>T-1081</b>	<b>BGP -Shear Wall Horizontal Hooks Near GL 1.4-K</b>	<b>Closed</b>	<b>01</b>	<b>01/03/2014</b>	<b>01/13/2014</b>	<b>01/07/2014</b>
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>  At the shear wall (first lift) near grids 1.4/K, the tails to the horizontal hooks which terminate at the columns were erroneously cut in the field and no longer provide the proper development length. See the attached sketch for specific portions of the shear wall affected. Per field discussions with the TT engineer on site, Gerdau proposes to leave the hooks that have been cut "as-is" and to add a standard 180° #9 hook to allow for proper development of the horizontal bar. Please confirm if this is acceptable						
					<b>ANSWER:</b>  At the shear wall (first lift) near grids 1.4/K, the tails to the horizontal hooks which terminate at the columns were erroneously cut in the field and no longer provide the proper development length. See the attached sketch for specific portions of the shear wall affected. Per field discussions with the TT engineer on site, Gerdau proposes to leave the hooks that have been cut "as-is" and to add a standard 180° #9 hook to allow for proper development of the horizontal bar. Please confirm if this is acceptable	
<b>T-1083</b>	<b>BGP - Geothermal Riser Pressure Gauge Location</b>	<b>Closed</b>	<b>CR</b>	<b>01/06/2014</b>	<b>12/30/2013</b>	<b>01/17/2014</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Previous geothermal fields and risers had a "cat walk" behind the risers at grade. Additional pipe and 90s were added to bring the gauges up to grade to allow for pressure monitoring from this "catwalk." At fields 09-15 no cat walk exists, thus no location to access these gauges from.  Please provide the location for the geothermal riser gauges for inspection from Field 09 through Field 15.						
					<b>ANSWER:</b>  Previous geothermal fields and risers had a "cat walk" behind the risers at grade. Additional pipe and 90s were added to bring the gauges up to grade to allow for pressure monitoring from this "catwalk." At fields 09-15 no cat walk exists, thus no location to access these gauges from.  Please provide the location for the geothermal riser gauges for inspection from Field 09 through Field 15.	



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<b>T-1084</b>	<b>SSS - Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>01/06/2014</b>	<b>01/16/2014</b>	<b>01/17/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 250 SK1 & SK2 and supply the welding for the noted connection as S+t per 8/S1-5012 will result in 1 5/8" fillet welds.						<b>ANSWER:</b>  See attached CD RFI # 250 SK1 & SK2 and supply the welding for the noted connection as S+t per 8/S1-5012 will result in 1 5/8" fillet welds.
<b>T-1085</b>	<b>SSS - Framing Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>01/06/2014</b>	<b>01/16/2014</b>	<b>01/17/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 252 SK1 & SK2 for items 1 & 2:  1.) Confirm the 3" drag plate typically extends from the column to the W24x55 as shown. 2.) Confirm detail 8/S1-5020 may be modified as shown to suit the actual condition. If not, supply a new detail showing the drag plate in its sloped position.						<b>ANSWER:</b>  See attached CD RFI # 252 SK1 & SK2 for items 1 & 2:  1.) Confirm the 3" drag plate typically extends from the column to the W24x55 as shown. 2.) Confirm detail 8/S1-5020 may be modified as shown to suit the actual condition. If not, supply a new detail showing the drag plate in its sloped position.
<b>T-1086</b>	<b>SSS - Missing Brace Locations</b>	<b>Closed</b>	<b>CR</b>	<b>01/06/2014</b>	<b>01/16/2014</b>	<b>01/17/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 253 SK1 & SK2 and please confirm the kicker brace locations as shown are acceptable. If not, supply the location from a grid line.						<b>ANSWER:</b>  See attached CD RFI # 253 SK1 & SK2 and please confirm the kicker brace locations as shown are acceptable. If not, supply the location from a grid line.
<b>T-1087</b>	<b>SSS - Connection Clarifications for Skewed Beams</b>	<b>Closed</b>	<b>CR</b>	<b>01/07/2014</b>	<b>01/17/2014</b>	<b>01/17/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Reference details 7 & 8/S1-5010 and CD RFI 094 SK1 to SK7 for clarifications required on skewed beam connections as noted below.  1.) Refer to S1-2303 and CD RFI SK1 indicating an example location where a skewed beam and standard beam connection occur at the same location on the support beam. As noted on CD RFI 094 SK4, the connections foul at this typical condition. Please confirm it is typically acceptable to replace one of the connections						<b>ANSWER:</b>  Reference details 7 & 8/S1-5010 and CD RFI 094 SK1 to SK7 for clarifications required on skewed beam connections as noted below.  1.) Refer to S1-2303 and CD RFI SK1 indicating an example location where a skewed beam and standard beam connection occur at the same location on the support beam. As noted on CD RFI 094 SK4, the connections foul at this typical condition. Please confirm it is typically acceptable to replace one of the



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	<p>with a shear plate connection per detail 1/S1-5011 or supply an alternate typical solution.</p> <p>2.) Refer to S1-2303 and CD RFI SK1 indicating an example location for two-sided skewed beam connections. As noted on CD RFI 094 SK5, the non -symmetrical bolt locations in detail 7/S1-5050 will not work at two-sided connections. Please confirm it is typically acceptable to locate the bolts as shown at two-sided connections or provide an alternate detail for this condition.</p> <p>3.) Refer to S1-2305 and CD RFI SK2 indicating an example location of a two-sided skewed beam connection. As noted on CD RFI 094 SK 6, the non-symmetrical bolt location in detail 8/S1-5010 will not work at two-sided connections. Please supply a typical alternate detail for these conditions.</p> <p>4.) Detail 8/S1-5010 shows the shear plate on the obtuse side. Confirm it is acceptable to locate the shear plate on the acute side for beam erection access purposes as noted on CD RFI 094 SK 6.</p> <p>5.) Refer to S1-2303 and CD RFI 094 SK3 indicating an example location where details 7 &amp; 8/S1-5010 occur at the same location based on the angles of the skewed beams. Please confirm that one of the connections may be typically replaced with a skewed shear plate per 1/S1-5011 to avoid the conflict shown on CD RFI 094 SK7, or supply a new typical alternate detail.</p>				<p>connections with a shear plate connection per detail 1/S1-5011 or supply an alternate typical solution.</p> <p>2.) Refer to S1-2303 and CD RFI SK1 indicating an example location for two-sided skewed beam connections. As noted on CD RFI 094 SK5, the non -symmetrical bolt locations in detail 7/S1-5050 will not work at two-sided connections. Please confirm it is typically acceptable to locate the bolts as shown at two-sided connections or provide an alternate detail for this condition.</p> <p>3.) Refer to S1-2305 and CD RFI SK2 indicating an example location of a two-sided skewed beam connection. As noted on CD RFI 094 SK 6, the non-symmetrical bolt location in detail 8/S1-5010 will not work at two-sided connections. Please supply a typical alternate detail for these conditions.</p> <p>4.) Detail 8/S1-5010 shows the shear plate on the obtuse side. Confirm it is acceptable to locate the shear plate on the acute side for beam erection access purposes as noted on CD RFI 094 SK 6.</p> <p>5.) Refer to S1-2303 and CD RFI 094 SK3 indicating an example location where details 7 &amp; 8/S1-5010 occur at the same location based on the angles of the skewed beams. Please confirm that one of the connections may be typically replaced with a skewed shear plate per 1/S1-5011 to avoid the conflict shown on CD RFI 094 SK7, or supply a new typical alternate detail.</p>	
T-1087.1	SSS - Connection Clarifications for Skewed Beams	Closed	CR	09/10/2014	09/20/2014	09/19/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:			ANSWER:			
See attached CD RFI # 094.1 SK1: The response in RFI T-1087 (SK 122, CD 094) item 3 does not work as the bolts/shear plate foul each other as shown. Supply an alternate typical detail for skewed 2 sided beam to beam connections.			See attached CD RFI # 094.1 SK1: The response in RFI T-1087 (SK 122, CD 094) item 3 does not work as the bolts/shear plate foul each other as shown. Supply an alternate typical detail for skewed 2 sided beam to beam connections.			









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T-1091	SSS - Transfer Girder Rebar Hole Spacing	Closed	CR	01/08/2014	01/18/2014	01/24/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:			ANSWER:			
See attached CD RFI # 258 SK1 to SK3 for items 1 to 3:			See attached CD RFI # 258 SK1 to SK3 for items 1 to 3:			
1.) The spacing for the #4 stirrups is given as 5 1/2" & 8". Confirm 5 1/2" is correct.			1.) The spacing for the #4 stirrups is given as 5 1/2" & 8". Confirm 5 1/2" is correct.			
2.) Confirm the first holes for the #4 stirrup may be located 5 3/4" from the end of girder (centered between the headed studs).			2.) Confirm the first holes for the #4 stirrup may be located 5 3/4" from the end of girder (centered between the headed studs).			
3.) The 2" dia. holes for the #4 stirrups foul the stiffeners at (4) locations. Confirm it is acceptable to move the holes as shown.			3.) The 2" dia. holes for the #4 stirrups foul the stiffeners at (4) locations. Confirm it is acceptable to move the holes as shown.			





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<b>T-1093</b>	<b>BGP - Foundation Wall Mix Placed in Shear Wall</b>	<b>Closed</b>	<b>CR</b>	<b>01/09/2014</b>	<b>01/19/2014</b>	<b>01/14/2014</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Please reference TG06.0 technical specs section 033020.2.1 and cast-in-place mix designs submittal numbers: TG0600-203 (Foundation Walls) and TG0600-204 (Slabs, Beams and Shear Walls).  Foundation Wall cast-in-place mix satisfies all requirements prescribed in table 2-1 "Concrete Properties" (033020.2.1) for the Shear Wall cast-in-place mix design. In order to limit site congestion (1 concrete pump vs. 2 concrete pumps) and to aid in logistic coordination between trade subcontractors (BBII Steel offhaul and/or bracing/rebracing work and SCCI concrete placing activities). SCCI is proposing to utilize the Foundation Wall mix when placing the shear walls. Per the project schedule there will be instances in which a foundation wall and shear wall that are in close proximity, are to be poured on the same day. If the same mix is approved to be used for both types of walls, one pump can be utilized vs. two.  Is this proposed mix design variance acceptable?						
<b>ANSWER:</b>  Please reference TG06.0 technical specs section 033020.2.1 and cast-in-place mix designs submittal numbers: TG0600-203 (Foundation Walls) and TG0600-204 (Slabs, Beams and Shear Walls).  Foundation Wall cast-in-place mix satisfies all requirements prescribed in table 2-1 "Concrete Properties" (033020.2.1) for the Shear Wall cast-in-place mix design. In order to limit site congestion (1 concrete pump vs. 2 concrete pumps) and to aid in logistic coordination between trade subcontractors (BBII Steel offhaul and/or bracing/rebracing work and SCCI concrete placing activities). SCCI is proposing to utilize the Foundation Wall mix when placing the shear walls. Per the project schedule there will be instances in which a foundation wall and shear wall that are in close proximity, are to be poured on the same day. If the same mix is approved to be used for both types of walls, one pump can be utilized vs. two.  Is this proposed mix design variance acceptable?						
<b>T-1094</b>	<b>SSS - End Transfer Girder Details at GL16G</b>	<b>Closed</b>	<b>CR</b>	<b>01/09/2014</b>	<b>01/19/2014</b>	<b>01/16/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI # 263 SK1 to SK3 for items 1 to 6:  1.) Supply the slope angle for MFB1. 2.) Confirm the noted information is the correct information to determine the top end of MFB1. 3.) Supply the noted dimension (to be used to locate PL 2 1/2 x 9 x 2'-6). 4.) Confirm the braces shown on S1-2304 (SK1) may be located as shown to avoid fouling the stiffeners in Girder TR16. 5.) Supply the underside of slab dimension at the location of the brace per item 4. 6.) Supply the underside of slab dimension at the location of the brace per item 4.						
<b>ANSWER:</b>  See attached CD RFI # 263 SK1 to SK3 for items 1 to 6:  1.) Supply the slope angle for MFB1. 2.) Confirm the noted information is the correct information to determine the top end of MFB1. 3.) Supply the noted dimension (to be used to locate PL 2 1/2 x 9 x 2'-6). 4.) Confirm the braces shown on S1-2304 (SK1) may be located as shown to avoid fouling the stiffeners in Girder TR16. 5.) Supply the underside of slab dimension at the location of the brace per item 4. 6.) Supply the underside of slab dimension at the location of the brace per item 4.						

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T-1095	SSS - End Transfer Girder Details at GL14G	Closed	CR	01/09/2014	01/19/2014	01/16/2014
<p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p>						
<p><b>REQUEST:</b></p> <p>See attached CD RFI # 262 SK1 to SK3 for items 1 to 6:</p> <ol style="list-style-type: none"> <li>1.) Supply the slope angle for MFB4.</li> <li>2.) Confirm the noted information is the correct information to determine the top end of MFB4.</li> <li>3.) Supply the noted dimension (to be used to locate PL 2 1/2 x 9 x 2'-6).</li> <li>4.) Confirm the braces shown on S1-2304 (SK1) may be located as shown to avoid fouling the stiffeners in Girder TR14.</li> <li>5.) Supply the underside of slab dimension at the location of the brace per item 4.</li> <li>6.) Supply the underside of slab dimension at the location of the brace per item 4.</li> </ol>			<p><b>ANSWER:</b></p> <p>See attached CD RFI # 262 SK1 to SK3 for items 1 to 6:</p> <ol style="list-style-type: none"> <li>1.) Supply the slope angle for MFB4.</li> <li>2.) Confirm the noted information is the correct information to determine the top end of MFB4.</li> <li>3.) Supply the noted dimension (to be used to locate PL 2 1/2 x 9 x 2'-6).</li> <li>4.) Confirm the braces shown on S1-2304 (SK1) may be located as shown to avoid fouling the stiffeners in Girder TR14.</li> <li>5.) Supply the underside of slab dimension at the location of the brace per item 4.</li> <li>6.) Supply the underside of slab dimension at the location of the brace per item 4.</li> </ol>			



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<b>T-1097</b>	<b>SSS - End Transfer Girder Details at GL19.9 &amp; 20.1G</b>	<b>Closed</b>	<b>CR</b>	<b>01/09/2014</b>	<b>01/19/2014</b>	<b>01/16/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 264 SK1 to SK4 for items 1 to 8:			See attached CD RFI # 264 SK1 to SK4 for items 1 to 8:			
1.) Supply the slope angle for MFB1.			1.) Supply the slope angle for MFB1.			
2.) Confirm the noted information is the correct information to determine the top end of MFB1.			2.) Confirm the noted information is the correct information to determine the top end of MFB1.			
3.) Supply the noted dimension (to be used to locate PL 2 1/2 x 9 x 2'-6).			3.) Supply the noted dimension (to be used to locate PL 2 1/2 x 9 x 2'-6).			
4.) The braces per 5/S1-5015 as shown on plan (SK1) will cross each other between Grids 19.9 & 20.1 as shown on SK3 & SK4. There is insufficient room on Girders TR19.9 & TR20.1 to accommodate these brace connections without the braces fouling each other. Please work with SK3 & SK4 and provide a solution.			4.) The braces per 5/S1-5015 as shown on plan (SK1) will cross each other between Grids 19.9 & 20.1 as shown on SK3 & SK4. There is insufficient room on Girders TR19.9 & TR20.1 to accommodate these brace connections without the braces fouling each other. Please work with SK3 & SK4 and provide a solution.			
5.) Supply the underside of slab dimension at the location of the brace per item 4.			5.) Supply the underside of slab dimension at the location of the brace per item 4.			
6.) Supply the underside of slab dimension at the location of the brace per item 4.			6.) Supply the underside of slab dimension at the location of the brace per item 4.			
7.) Supply the underside of slab dimension at the location of the brace per item 4.			7.) Supply the underside of slab dimension at the location of the brace per item 4.			
8.) Supply the underside of slab dimension at the location of the brace per item 4.			8.) Supply the underside of slab dimension at the location of the brace per item 4.			





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<b>T-1098</b>	<b>SSS - End Transfer Girder Details at GL16C</b>	<b>Closed</b>	<b>CR</b>	<b>01/09/2014</b>	<b>01/19/2014</b>	<b>01/16/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 266 SK1 & SK2 for items 1 to 3:			See attached CD RFI # 266 SK1 & SK2 for items 1 to 3:			
1.) Supply the location of the braces from grid 'C' considering the dimensions on TR16 shown on SK2 and the connection for the braces to the girder per 8/S1-5015.			1.) Supply the location of the braces from grid 'C' considering the dimensions on TR16 shown on SK2 and the connection for the braces to the girder per 8/S1-5015.			
2.) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.			2.) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.			
3.) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.			3.) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.			
<b>T-1099</b>	<b>SSS - End Transfer Girder Details at GL14C</b>	<b>Closed</b>	<b>CR</b>	<b>01/09/2014</b>	<b>01/19/2014</b>	<b>01/28/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 265 SK1 & SK2 for items 1 to 3:			See attached CD RFI # 265 SK1 & SK2 for items 1 to 3:			
1.) Supply the location of the braces from grid 'C' considering the dimensions on TR14 shown on SK2 and the connection for the braces to the girder per 8/S1-5015.			1.) Supply the location of the braces from grid 'C' considering the dimensions on TR14 shown on SK2 and the connection for the braces to the girder per 8/S1-5015.			
2.) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.			2.) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.			
3.) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.			3.) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.			
<b>T-1100</b>	<b>SSS - End Transfer Girder Details at GL19.9 &amp; 20.1C</b>	<b>Closed</b>	<b>CR</b>	<b>01/10/2014</b>	<b>01/20/2014</b>	<b>01/28/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 267 SK1 & SK2 for items 1 & 2:			See attached CD RFI # 267 SK1 & SK2 for items 1 & 2:			
1.) Supply the location of the braces from Grid C considering the dimensions on TR19.9 & TR20.1 per 3/S1-3705 as			1.) Supply the location of the braces from Grid C consider			



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T-1100.1	<div>SSS - End Transfer Girder Details at GL19.9 &amp; 20.1C</div> <div>From: Webcor Construction LP</div> <div>Stephanie Azzolino</div>	Closed	CR	03/12/2014	03/22/2014	03/31/2014
REQUEST:		ANSWER:				
<p>shown on SK2 and the connections for the braces to the Girders per 8/S1-5015.</p> <p>2.) Supply the underside of slab elevations at each brace located per dimensions supplied in item 1.</p>		<p>ing the dimensions on TR19.9 &amp; TR20.1 per 3/S1-3705 as shown on SK2 and the connections for the braces to the Girders per 8/S1-5015.</p> <p>2.) Supply the underside of slab elevations at each brace located per dimensions supplied in item 1.</p>				
<p>See attached CD RFI # 267.1 SK1 for items 1 to 3:</p> <p>At GL 22/C detail 3/S1-3705 is noted as typical at Grids 19.9 &amp; 20.1 on S1-2305. However, TR19.9 &amp; TR20.1 do not match the information as shown in detail 3/S1-3705. See SK1 for the outline of the MFB1 and please provide direction for the following items at Grids 19.9 &amp; 20.1.</p> <p>1) The 112 headed studs requested in detail 3/S1-3705 will not fit inside the MFB1 as is shown. Confirm it is acceptable to eliminate the top row of headed studs and supply a total of 98 headed studs (49 per side) or supply an alternate solution.</p> <p>2) The 2" diameter holes for the #4 stirrups will not fit inside TR19.9 &amp; TR20.1. Give direction on the bottom stirrup holes and confirm the top stirrup holes are still to be supplied as shown.</p> <p>3) Confirm the 2 1/2" diameter holes in the bottom stiffeners are no longer required as they will foul the bottom flange of TR19.9 &amp; TR20.1.</p>		<p>See attached CD RFI # 267.1 SK1 for items 1 to 3:</p> <p>At GL 22/C detail 3/S1-3705 is noted as typical at Grids 19.9 &amp; 20.1 on S1-2305. However, TR19.9 &amp; TR20.1 do not match the information as shown in detail 3/S1-3705. See SK1 for the outline of the MFB1 and please provide direction for the following items at Grids 19.9 &amp; 20.1.</p> <p>1) The 112 headed studs requested in detail 3/S1-3705 will not fit inside the MFB1 as is shown. Confirm it is acceptable to eliminate the top row of headed studs and supply a total of 98 headed studs (49 per side) or supply an alternate solution.</p> <p>2) The 2" diameter holes for the #4 stirrups will not fit inside TR19.9 &amp; TR20.1. Give direction on the bottom stirrup holes and confirm the top stirrup holes are still to be supplied as shown.</p> <p>3) Confirm the 2 1/2" diameter holes in the bottom stiffeners are no longer required as they will foul the bottom flange of TR19.9 &amp; TR20.1.</p>				





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T-1101	<b>SSS - Connections for Rigging Schemes</b>  From: Webcor Construction LP Gregory Kemerer	Closed	CR	01/10/2014	01/20/2014	02/05/2014
<b>REQUEST:</b>  Skanska is reviewing the rigging schemes required to erect the Transfer Girders, Built-up Columns and Tapered Roof Girders. Please confirm drilling holes for the bolted connection in the following members is acceptable so Candraft can incorporate them into the model as per: 1) Transfer Girders sketches R-1A & R-1B. 2) Built-up Columns sketches R-2A, R-2B & R-2C. 3) Tapered Roof Girders sketches R-5A & R-5B.		<b>ANSWER:</b>  Skanska is reviewing the rigging schemes required to erect the Transfer Girders, Built-up Columns and Tapered Roof Girders. Please confirm drilling holes for the bolted connection in the following members is acceptable so Candraft can incorporate them into the model as per: 1) Transfer Girders sketches R-1A & R-1B. 2) Built-up Columns sketches R-2A, R-2B & R-2C. 3) Tapered Roof Girders sketches R-5A & R-5B.				
T-1102	<b>SSS - Type III Column Base Embedded Plate</b>  From: Webcor Construction LP Gregory Kemerer	Closed	CR	01/09/2014	01/20/2014	01/10/2014
<b>REQUEST:</b>  Type III column base detail 8/S1-5051 indicates an embedded plate, as the package delineation line shows the ½" thick embedded plate is not in Skanska's scope of work. TG06 trade subcontractor will be required to coordinate locating the shear studs to clear the congested rebar at these locations. The embedded plates will be supplied and installed by others and Skanska will field weld the L4x3 to the embedded plate as indicated on SK1. Please confirm this is acceptable.		<b>ANSWER:</b>  Type III column base detail 8/S1-5051 indicates an embedded plate, as the package delineation line shows the ½" thick embedded plate is not in Skanska's scope of work. TG06 trade subcontractor will be required to coordinate locating the shear studs to clear the congested rebar at these locations. The embedded plates will be supplied and installed by others and Skanska will field weld the L4x3 to the embedded plate as indicated on SK1. Please confirm this is acceptable.				
T-1103	<b>BGP - Increased Slump Specification Limit for Mixes with High-Range Water Redu</b> Closed		CR	01/13/2014	01/23/2014	01/15/2014
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please reference attached letter Authored by Robert Foley, CEMEX QC Manager, dated 1/2/2014 and TG06.0 technical spec section 033020.2.3.F.1.b.  SCCI and CEMEX are proposing the following guidelines regarding slump of cast-in-place mix designs that contain 30% or higher fly-ash (CM) and HRWR:  1. Maximum 8-inch slump will continue to be the target slump for delivery of concrete mixes with HRWR. 2. 9-inch and higher slump will be considered an action limit. Whenever slump of consecutive loads exceeds 9		<b>ANSWER:</b>  Please reference attached letter Authored by Robert Foley, CEMEX QC Manager, dated 1/2/2014 and TG06.0 technical spec section 033020.2.3.F.1.b.  SCCI and CEMEX are proposing the following guidelines regarding slump of cast-in-place mix designs that contain 30% or higher fly-ash (CM) and HRWR:  1. Maximum 8-inch slump will continue to be the target slump for delivery of concrete mixes with HRWR. 2. 9-inch and higher slump will be considered an				



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	<p>inches, actions will be taken to reduce subsequent slump measurements.</p> <p>3. Batches with slump as high as 10.5 inches will be accepted provided the batch weights are evaluated to verify the batch did not include water content that exceeds mix design w/c ratio; and the concrete is not visibly segregating.</p> <p>Are these revised guidelines acceptable?</p>					<p>action limit. Whenever slump of consecutive loads exceeds 9 inches, actions will be taken to reduce subsequent slump measurements.</p> <p>3. Batches with slump as high as 10.5 inches will be accepted provided the batch weights are evaluated to verify the batch did not include water content that exceeds mix design w/c ratio; and the concrete is not visibly segregating.</p> <p>Are these revised guidelines acceptable?</p>
<b>T-1104</b>	<b>BGP - Increase Concourse Slab Maximum construction Joint Spacing</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2014</b>	<b>01/23/2014</b>	<b>01/28/2014</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>						<b>ANSWER:</b>
<p>Please reference TG06.0 contract specs section 033020.3.2.A.4, submittal TG0600-030.2 and attached drawing showing proposed CJ layout per variance below. SCCI is proposing to increase the allowable maximum construction joint spacing in the lower concourse slab:</p> <p>With the use of currently approved Concourse Slab cast-in-place mix design, SCCI is proposing to eliminate every other construction joint. See attached pages for reference example. Maximum construction joint spacing would be 96-feet. Joint location will always land on wall joint location below per 033020.3 .2.A.4.</p> <p>Construction joint layout submittal TG0600-030 will be revised and resubmitted to reflect any change made to currently approved layout.</p> <p>Is this acceptable?</p>						<p>Please reference TG06.0 contract specs section 033020.3.2.A.4, submittal TG0600-030.2 and attached drawing showing proposed CJ layout per variance below. SCCI is proposing to increase the allowable maximum construction joint spacing in the lower concourse slab:</p> <p>With the use of currently approved Concourse Slab cast-in-place mix design, SCCI is proposing to eliminate every other construction joint. See attached pages for reference example. Maximum construction joint spacing would be 96-feet. Joint location will always land on wall joint location below per 033020.3 .2.A.4.</p> <p>Construction joint layout submittal TG0600-030 will be revised and resubmitted to reflect any change made to currently approved layout.</p> <p>Is this acceptable?</p>



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T-1104.1	BGP - Increase Concourse Construction Joint Spacing	Closed	01	12/15/2014	12/25/2014	12/18/2014
<div><div>From: Webcor Construction LP</div><div>Claude Titcher</div></div>						
REQUEST:			ANSWER:			
<div>Contract Doc Ref: 03 30 20 3.2A.4</div> <div>Location: Lower Concourse</div> <div>Grid Line: 30-35</div> <div>Add'l Doc Ref's: RFI T-1104</div>			<div>Contract Doc Ref: 03 30 20 3.2A.4</div> <div>Location: Lower Concourse</div> <div>Grid Line: 30-35</div> <div>Add'l Doc Ref's: RFI T-1104</div>			
<div>Please reference TG06.0 contract spec section 033020.3.2.A.4, RFI T-1104 RESPONSE BGP - Increase Concourse Slab Maximum Construction Joint Spacing, and attached drawing showing proposed CJ layout.</div>			<div>Please reference TG06.0 contract spec section 033020.3.2.A.4, RFI T-1104 RESPONSE BGP - Increase Concourse Slab Maximum Construction Joint Spacing, and attached drawing showing proposed CJ layout.</div>			
<div>SCCI is proposing to increase the allowable maximum construction joint spacing in the lower concourse slab from 96' in Area 15 &amp; 16. Maximum construction joint spacing would be 100' for Area 15 and 99' for Area 16. Joint location will always land on wall joint location below per 033002.3.2.A.4.</div>			<div>SCCI is proposing to increase the allowable maximum construction joint spacing in the lower concourse slab from 96' in Area 15 &amp; 16. Maximum construction joint spacing would be 100' for Area 15 and 99' for Area 16. Joint location will always land on wall joint location below per 033002.3.2.A.4.</div>			
<div>Construction joint layout submittal TG0600-030 will be revised and resubmitted to reflect any change made to currently approved layout.</div>			<div>Construction joint layout submittal TG0600-030 will be revised and resubmitted to reflect any change made to currently approved layout.</div>			
<div>Is this acceptable?</div>			<div>Is this acceptable?</div>			
T-1105	SSS - Elevator Rail Supports Erection Aids	Closed	CR	01/14/2014	01/24/2014	01/27/2014
<div>From: Skanska USA Civil West California Dis</div> <div>Ryan Clayton</div>						
REQUEST:			ANSWER:			
<div>See attached CD RFI # 183.1 SK1A, SK1B, SK2A &amp; SK2B for items 1 &amp; 2:</div> <div>1.) Confirm the elevator rail support connection with erection aids is acceptable as shown.</div> <div>2.) Confirm the elevator rail support connection with erection aids is acceptable as shown.</div>			<div>See attached CD RFI # 183.1 SK1A, SK1B, SK2A &amp; SK2B for items 1 &amp; 2:</div> <div>1.) Confirm the elevator rail support connection with erection aids is acceptable as shown.</div> <div>2.) Confirm the elevator rail support connection with erection aids is acceptable as shown.</div>			
T-1105.1	SSS - Elevator Rail Support Details	Closed	CR	02/12/2014	02/22/2014	
<div>From: Webcor Construction LP</div> <div>Stephanie Azzolino</div>						
REQUEST:			ANSWER:			
<div>RFI # changed to T-1105.2</div>			<div>RFI # changed to T-1105.2</div>			



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<b>T-1105.2</b>	<b>SSS - Elevator Rail Support Details</b>	<b>Closed</b>	<b>CR</b>	<b>02/12/2014</b>	<b>02/22/2014</b>	<b>02/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The response to RFI T-1105 directs the contractor to issued for bid drawings. Please advise if the new details 7/S17630 & 8/S1-7630 are intended to be for construction, if so please indicate which ASI package these revised drawings will be formally issued with.			The response to RFI T-1105 directs the contractor to issued for bid drawings. Please advise if the new details 7/S17630 & 8/S1-7630 are intended to be for construction, if so please indicate which ASI package these revised drawings will be formally issued with.			
<b>T-1106</b>	<b>SSS - Pretensioned Rod Bearing Plate Hole Dia</b>	<b>Closed</b>	<b>CR</b>	<b>01/14/2014</b>	<b>01/24/2014</b>	<b>01/24/2014</b>
<b>From:</b> Skanska USA Civil West California DisRyan Clayton						
<b>REQUEST:</b>			<b>ANSWER:</b>			
With reference to detail 6/S1-5052 (attached) please review the following:			With reference to detail 6/S1-5052 (attached) please review the following:			
Due to the limited access at the top of the built-up TT please confirm it is acceptable to increase the diameter of the hole in the 4" bearing plate to the maximum allowable size of 3-3/4" as per ASIC table 14-2 (attached) to allow for additional tolerance and workability when installing the 2-1/2" diameter 18' rod. The oversized side hole will only be required at the 17 built-up TT locations and the 6x6x2" plate washer hole will remain the major diameter of the rod + 1/16".			Due to the limited access at the top of the built-up TT please confirm it is acceptable to increase the diameter of the hole in the 4" bearing plate to the maximum allowable size of 3-3/4" as per ASIC table 14-2 (attached) to allow for additional tolerance and workability when installing the 2-1/2" diameter 18' rod. The oversized side hole will only be required at the 17 built-up TT locations and the 6x6x2" plate washer hole will remain the major diameter of the rod + 1/16".			
Please confirm this proposal is acceptable.			Please confirm this proposal is acceptable.			
<b>T-1107</b>	<b>SSS - Connection Clarification at Roof Level GL 11</b>	<b>Closed</b>	<b>CR</b>	<b>01/14/2014</b>	<b>01/24/2014</b>	<b>01/27/2014</b>
<b>From:</b> Skanska USA Civil West California DisRyan Clayton						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 256 SK1 & SK2.			See attached CD RFI # 256 SK1 & SK2.			
Due to the thick flanges of the W40x593, it is not possible to provide the required 10 bolts in the W36x160 per S1-5010.			Due to the thick flanges of the W40x593, it is not possible to provide the required 10 bolts in the W36x160 per S1-5010.			
Please confirm it is acceptable to provide 9 bolts as shown or supply a new detail.			Please confirm it is acceptable to provide 9 bolts as shown or supply a new detail.			



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<b>T-1108</b>	<b>SSS - Edge of Slab Location Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>01/14/2014</b>	<b>01/24/2014</b>	<b>01/29/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 261 SK1 to SK4:			See attached CD RFI # 261 SK1 to SK4:			
S1-2503 (SK1) shows the beam as 9" from the edge of slab. 1/S1-7303 (SK2) shows the edge of slab as 31'-11 1/2 from Grid 11 but A1-2893 (SK3) shows the edge of slab as 31'-11 from Grid 11.			S1-2503 (SK1) shows the beam as 9" from the edge of slab. 1/S1-7303 (SK2) shows the edge of slab as 31'-11 1/2 from Grid 11 but A1-2893 (SK3) shows the edge of slab as 31'-11 from Grid 11.			
SK4 shows what is currently in the model.			SK4 shows what is currently in the model.			
Please advise of any correction that needs to be made in the model due to the discrepancy for the edge of slab location.			Please advise of any correction that needs to be made in the model due to the discrepancy for the edge of slab location.			
<b>T-1109</b>	<b>SSS - Pretension Rod Finish Requirement</b>	<b>Closed</b>	<b>CR</b>	<b>01/14/2014</b>	<b>01/24/2014</b>	<b>01/17/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
With reference to pretensioned rods required as per schedule 3/S1-5052 please review the following:			With reference to pretensioned rods required as per schedule 3/S1-5052 please review the following:			
As the pretensioned rods on the cruciform columns are to receive a fireproof coating please confirm the rods are to be supplied plain (no finish required) as per General Note SS-10 - All steel members and embedded steel angles and plates not painted, coated with fireproofing, nor protected by concrete cover, shall be hot-dipped galvanized.			As the pretensioned rods on the cruciform columns are to receive a fireproof coating please confirm the rods are to be supplied plain (no finish required) as per General Note SS-10 - All steel members and embedded steel angles and plates not painted, coated with fireproofing, nor protected by concrete cover, shall be hot-dipped galvanized.			
Please confirm this is acceptable.			Please confirm this is acceptable.			
<b>T-1109.1</b>	<b>SSS - Pretension Rod Finish Requirement</b>	<b>Closed</b>	<b>CR</b>	<b>01/27/2014</b>	<b>02/06/2014</b>	<b>02/12/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The response to RFI T-1109 (attached) indicates that the pretensioned rods are to receive fireproofing.			The response to RFI T-1109 (attached) indicates that the pretensioned rods are to receive fireproofing.			
1) Please provide the UL assembly for the rod fireproofing. 2) Please confirm that the rods will be Sprayed Fire Resistive Materials SFRM-1 to match the column			1) Please provide the UL assembly for the rod fireproofing. 2) Please confirm that the rods will be Sprayed Fire			



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	fireproofing system.				Resistive Materials SFRM-1 to match the column fireproofing system.	
T-1110	SSS - Welded Reinforcement at Light Column Tendons	Closed	CR	01/14/2014	01/24/2014	02/03/2014
From: Webcor Construction LP                      Stephanie Azzolino						
REQUEST:			ANSWER:			
Reference details 1 and 5 on S1-6008 which indicate that welded reinforcement bars are "to be determined by post-tensioning system supplier." Per detail 4/S1-6008, the PT anchor bolt supplier is Dywidag.			Reference details 1 and 5 on S1-6008 which indicate that welded reinforcement bars are "to be determined by post-tensioning system supplier." Per detail 4/S1-6008, the PT anchor bolt supplier is Dywidag.			
Per the email attached, Dywidag's representative states that additional reinforcing bars are not required provided the concrete strength is sufficient and that the anchorages are not located particularly close to an exterior concrete face.			Per the email attached, Dywidag's representative states that additional reinforcing bars are not required provided the concrete strength is sufficient and that the anchorages are not located particularly close to an exterior concrete face.			
Based on the maximum permissible jacking load and associated maximum bearing stress of 3.8ksi, please confirm the concrete strength is sufficient and that the reinforcing bars can be eliminated at the Light Column tendons.			Based on the maximum permissible jacking load and associated maximum bearing stress of 3.8ksi, please confirm the concrete strength is sufficient and that the reinforcing bars can be eliminated at the Light Column tendons.			



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
T-1111	<b>SSS - Framing &amp; Connection Clarifications</b>	Closed	CR	01/14/2014	01/24/2014	01/28/2014
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 254 SK1 to SK4 for items 1 to 7: 1.) It appears the noted section references do not apply on the noted level of steel but the detail should be applied on S1-2403. Work with SK1 & SK4 and confirm or clarify how the detail is to be applied at this level. 2.) If detail 1/S1-7661 is to be applied on the noted level, please respond to the following: a.) Confirm 1/S1-7661 applies within the 10'-11 area. b.) Supply information for how to apply 1/S1-7661 at the 2 1/4" slab transition per A1-2883 as documented in RFI T-0963 (SK 247 & CD 196) 3.) Confirm noted dimensions are correct. 4.) Confirm the L8x8x3/4 does not need to be welded to the plate and/or to the L8x4x1/2. If yes, supply the welding requirement. 5.) The noted information is not clear. Please supply information for the plate and welding. 6.) Confirm the horizontal leg of the L8x4x1/2 does not need to be welded to the beam flange. If yes, supply the welding requirement. 7.) Confirm a slab closure plate per 8/S1-5000 is required on center of beam or clarify the edge of slab along this beam.			See attached CD RFI # 254 SK1 to SK4 for items 1 to 7: 1.) It appears the noted section references do not apply on the noted level of steel but the detail should be applied on S1-2403. Work with SK1 & SK4 and confirm or clarify how the detail is to be applied at this level. 2.) If detail 1/S1-7661 is to be applied on the noted level, please respond to the following: a.) Confirm 1/S1-7661 applies within the 10'-11 area. b.) Supply information for how to apply 1/S1-7661 at the 2 1/4" slab transition per A1-2883 as documented in RFI T-0963 (SK 247 & CD 196) 3.) Confirm noted dimensions are correct. 4.) Confirm the L8x8x3/4 does not need to be welded to the plate and/or to the L8x4x1/2. If yes, supply the welding requirement. 5.) The noted information is not clear. Please supply information for the plate and welding. 6.) Confirm the horizontal leg of the L8x4x1/2 does not need to be welded to the beam flange. If yes, supply the welding requirement. 7.) Confirm a slab closure plate per 8/S1-5000 is required on center of beam or clarify the edge of slab along this beam.			





<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-1112	SSS - Detail Clarifications	Closed	CR	01/14/2014	01/24/2014	01/28/2014
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See att ached CD RFI # 259 SK1 & SK2 for items 1 to 3: 1.) The 1" MAX is not achievable with the hole locations shown. The actual gap will be 1 13/16 as shown on SK2. Confirm this is acceptable. 2.)The stitch plate will foul the web of the WT20x105.5 above if it is located at mid-span. Confirm it is acceptable to locate the stitch plate 7/16" clear of the WT as shown on SK2. 3.)It is not clear what is meant by the noted size of the shim plates. Confirm it is acceptable to locate the corner of the MC10 2 7/16" below the top of the WT20x105.5 to clear the "k" and to have the shim plates match the profile of the MC10 as shown on SK2. The shim plate size is 10" x 2'-0 1/2.			See att ached CD RFI # 259 SK1 & SK2 for items 1 to 3: 1.) The 1" MAX is not achievable with the hole locations shown. The actual gap will be 1 13/16 as shown on SK2. Confirm this is acceptable. 2.)The stitch plate will foul the web of the WT20x105.5 above if it is located at mid-span. Confirm it is acceptable to locate the stitch plate 7/16" clear of the WT as shown on SK2. 3.)It is not clear what is meant by the noted size of the shim plates. Confirm it is acceptable to locate the corner of the MC10 2 7/16" below the top of the WT20x105.5 to clear the "k" and to have the shim plates match the profile of the MC10 as shown on SK2. The shim plate size is 10" x 2'-0 1/2.			
T-1113	SSS - Light Column Template Air Gap	Closed	CR	01/14/2014	01/14/2014	01/17/2014
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Installation of the template at the base of the light column DYWIDAG anchor rod will result in a 1/16" air gap (see SK1). Please confirm it will be acceptable to fill this air gap with either Teflon tape or caulking.			Installation of the template at the base of the light column DYWIDAG anchor rod will result in a 1/16" air gap (see SK1). Please confirm it will be acceptable to fill this air gap with either Teflon tape or caulking.			
T-1114	BGP - Concrete Samples for Columns	Closed	01	01/15/2014	01/25/2014	01/21/2014
<div><div><b>From:</b> Webcor Construction LP</div><div>Robert Kjome</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Spec: 03 30 20-3.a.			Reference Spec: 03 30 20-3.a.			
"The TJPA Representative shall conduct tests of concrete as follows:			"The TJPA Representative shall conduct tests of concrete as follows:			
a. Testing frequency: Sample sets for all tests listed below of each concrete design mix placed each day shall be taken not less than once a day, nor less than once for each 100 cubic yards of concrete, nor less than once for each 5000 square feet of surface area for the mat, cast-in-place formed concrete			a. Testing frequency: Sample sets for all tests listed below of each concrete design mix placed each day shall be taken not less than once a day, nor less than once for each 100 cubic yards of concrete, nor less than once for each 5000 square feet of surface area for the mat, cast-in-place formed concrete			





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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

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	<p>slabs or walls. Additional tests shall be performed if deemed necessary by the TJPA Representative. Sample each column, regardless of other frequencies listed above."</p> <p>We request that the last sentence "Sample each column, regardless of other frequencies listed above", be deleted. The current testing of columns would fall under the statement to test "...not less than once a day, nor less than once for each 100 cubic yards". As the current schedule shows two columns to be poured per day, this will produce one set per day for testing.</p>					
	<p>slabs or walls. Additional tests shall be performed if deemed necessary by the TJPA Representative. Sample each column, regardless of other frequencies listed above."</p> <p>We request that the last sentence "Sample each column, regardless of other frequencies listed above", be deleted. The current testing of columns would fall under the statement to test "...not less than once a day, nor less than once for each 100 cubic yards". As the current schedule shows two columns to be poured per day, this will produce one set per day for testing.</p>					
T-1115	<b>BSE -Alternate Micropile Method in Buttress Area</b>	Closed	01	01/16/2014	01/26/2014	01/31/2014
	<p><b>From:</b> Balfour Beatty Infrastructure, Inc. Kelly Phariss</p> <p><b>REQUEST:</b></p> <p>DTDS is concerned about delays and extra costs resulting from drilling Micropiles adjacent to buttress piles from Gridlines 26.5 to 30. As stated in our Contract Change Order request (CCO #04) regarding "Final Micropile Layout - Additional Micropiles" (attached for reference), drilling for the micropiles may encounter overbreak pile concrete and grout placed during buttress pile remediation. The current drilling system cannot be used to drill through the pile overbreak and/or remediation grout. The reduced pile spacing from 10 feet on center to 5 feet and less may also cause problems such as communication between piles.</p> <p>Significant additional costs and schedule delays will result should DTDS have to change our procedure and/or equipment to drill through buttress pile concrete and/or remediation grout. Delays will also be realized should DTDS have to change our drilling sequence to mitigate problems that may arise from the reduced pile spacing.</p> <p>Should detrimental issues arise, DTDS proposes to drill, install, and grout micropile dowels in the center of the existing buttress piles as an alternative to drilling adjacent to buttress piles. A micropile dowel could take the place of</p>					
	<p><b>ANSWER:</b></p> <p>DTDS is concerned about delays and extra costs resulting from drilling Micropiles adjacent to buttress piles from Gridlines 26.5 to 30. As stated in our Contract Change Order request (CCO #04) regarding "Final Micropile Layout - Additional Micropiles" (attached for reference), drilling for the micropiles may encounter overbreak pile concrete and grout placed during buttress pile remediation. The current drilling system cannot be used to drill through the pile overbreak and/or remediation grout. The reduced pile spacing from 10 feet on center to 5 feet and less may also cause problems such as communication between piles.</p> <p>Significant additional costs and schedule delays will result should DTDS have to change our procedure and/or equipment to drill through buttress pile concrete and/or remediation grout. Delays will also be realized should DTDS have to change our drilling sequence to mitigate problems that may arise from the reduced pile spacing.</p> <p>Should detrimental issues arise, DTDS proposes to drill, install, and grout micropile dowels in the center of</p>					



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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

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	<p>a micropile as necessary. A dowel would consist of the same #20 Gr. 80 reinforcing bar used for the micropiles. A six- inch diameter, 20 foot long hole would be drilled in the center of the buttress pile. An additional drill rig will be required to perform the drilling. A 25' bar would be set with centralizers and tremie grouted with the same grout used for the micropiles. Based on an assumed minimum Buttress pile concrete and grout strength of 3,000 psi, the developmental length (ld) of a #20 bar is 182.5 inches (15.2 feet). 20 feet embedded would develop the yield strength of the #20 bar (393 kip) and exceed the design micropile load of 308 kips.</p> <p><math>ld = (80,000 \text{ psi} / (20 * \sqrt{3000 \text{ psi}})) * 2.5 \text{ in} = 182.5 \text{ in}.</math></p> <p>Accepting this alternative would mitigate delays and extra costs that will result should buttress pile concrete and/or grout be encountered while drilling adjacent to these piles.</p> <p>Please confirm that this alternative micropile procedure is acceptable.</p>					
T-1116	<p><b>BSE - Micropile Removal and Relocation in Buttress Area</b></p> <p>From: Webcor Construction LP      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>WOJV recieved FO T-00008 9/07/2012 which added micropiles within the footprint of the buttress shafts. RFI T-0323.1 returned 10/24/12 directed BBII to install buttress shaft E4, which is in direct conflict with Micropile E520.</p> <p>BBII proposes to:</p> <p>Option 1. Remove Micropile E520 Option 2. Drill Micropile E520 into the center of the buttress shaft as proposed in RFI T-1115 Option 3. Relocate Micropile E520 to a location provided by the design team.</p> <p>Also, BBII is requesting that they be permitted to relocate</p>	Closed	01	01/16/2014	01/26/2014	01/31/2014
	<p>the existing buttress piles as an alternative to drilling adjacent to buttress piles. A micropile dowel could take the place of a micropile as necessary. A dowel would consist of the same #20 Gr. 80 reinforcing bar used for the micropiles. A six- inch diameter, 20 foot long hole would be drilled in the center of the buttress pile. An additional drill rig will be required to perform the drilling. A 25' bar would be set with centralizers and tremie grouted with the same grout used for the micropiles. Based on an assumed minimum Buttress pile concrete and grout strength of 3,000 psi, the developmental length (ld) of a #20 bar is 182.5 inches (15.2 feet). 20 feet embedded would develop the yield strength of the #20 bar (393 kip) and exceed the design micropile load of 308 kips.</p> <p><math>ld = (80,000 \text{ psi} / (20 * \sqrt{3000 \text{ psi}})) * 2.5 \text{ in} = 182.5 \text{ in}.</math></p> <p>Accepting this alternative would mitigate delays and extra costs that will result should buttress pile concrete and/or grout be encountered while drilling adjacent to these piles.</p> <p>Please confirm that this alternative micropile procedure is acceptable.</p>					

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	Micropile E519, 1' to the South, to allow further clearance form Buttress Shaft E4.					Also, BBII is requesting that they be permitted to relocate Micropile E519, 1' to the South, to allow further clearance form Buttress Shaft E4.
T-1117	BGP - Geothermal Trench Backfill and Compaction Requirements in Zones 3 & 4	Closed	CR	01/16/2014	01/26/2014	01/24/2014
	From: Webcor Construction LP Jackson Tukuafu					
	REQUEST:  There are areas in Zone 3 (and perhaps in Zone 4) that Geothermal trenches will be trenched through that Arup identified as unsuitable soils (high in bay mud) , which is of such nature as to be incapable of being compacted to specific density using ordinary methods of optimum moisture content. Additionally, there are areas in Zone 3 (and perhaps in Zone 4) that Geothermal trenches will be trenched through that Arup identified as in-situ suitable, which are incapable of being compacted.  - Spec. 23-57-34 Ground Loop Heat Exchanger states "placing and compacting soils the loop installation, the trenches shall be back filled per IGSHPA with loose soil minimizing air gaps or voids and then marked with warning tape. After bedding around the loop and header piping, the backfill shall be watered to settle the loose soil to ensure there are no air gaps along the length of the pipe."  - Spec. 31-23-34 Trenching and Backfill states "All backfill will be placed in horizontal layers not more than (8) inches thick before compaction, and each layer shall be satisfactorily compacted by mechanical means. Flooding or jetting will not be allowed. Compact soil to not less than 95 percent maximum dry density according to ASTM D1557.  Is the following procedure acceptable for placing and compacting soils in the Geothermal Piping trenches in the areas with unsuitable soils (high amounts of bay mud), and suitable in-situ non-compactable as identified by Arup? 1. After the Geothermal piping is installed and tested, these trenches will be filled with available approved suitable materials from onsite excavations or 300 psi					ANSWER:  There are areas in Zone 3 (and perhaps in Zone 4) that Geothermal trenches will be trenched through that Arup identified as unsuitable soils (high in bay mud) , which is of such nature as to be incapable of being compacted to specific density using ordinary methods of optimum moisture content. Additionally, there are areas in Zone 3 (and perhaps in Zone 4) that Geothermal trenches will be trenched through that Arup identified as in-situ suitable, which are incapable of being compacted.  - Spec. 23-57-34 Ground Loop Heat Exchanger states "placing and compacting soils the loop installation, the trenches shall be back filled per IGSHPA with loose soil minimizing air gaps or voids and then marked with warning tape. After bedding around the loop and header piping, the backfill shall be watered to settle the loose soil to ensure there are no air gaps along the length of the pipe."  - Spec. 31-23-34 Trenching and Backfill states "All backfill will be placed in horizontal layers not more than (8) inches thick before compaction, and each layer shall be satisfactorily compacted by mechanical means. Flooding or jetting will not be allowed. Compact soil to not less than 95 percent maximum dry density according to ASTM D1557.  Is the following procedure acceptable for placing and compacting soils in the Geothermal Piping trenches in the areas with unsuitable soils (high amounts of bay mud), and suitable in-situ non-compactable as identified by Arup? 1. After the Geothermal piping is installed and tested,

<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
	<p>CLSM as approved by the TJPA Rep.</p> <p>2. Geothermal piping trenches soils will be placed per Geothermal Spec. 23-57-34 Ground Loop Heat Exchanger which states "the trenches shall be back filled per IGSHPA with loose soil minimizing air gaps or voids and then marked with warning tape."</p> <p>3. Soil bedding and backfill around the loop and header piping, shall be placed to ensure there are no air gaps along the length of the pipe (water will not drain well, so will be used sparingly and only if necessary).</p> <p>4. All backfill will be placed in horizontal layers not more than (8) inches thick before compaction, and each layer shall be satisfactorily compacted by mechanical means (e.g. pogo stick/power puff tools) .</p> <p>5. Flooding or jetting will not be allowed.</p> <p>6. Soils will be compacted using steps above and best construction practices.</p> <p>7. Trench fill and adjacent areas will not be tested to verify the "not less than 95 percent maximum dry density" according to ASTM D1557. The TJPA Reps will not perform density and moisture content tests specified in the Trenching and Backfill Spec. 31-23-34. In lieu of testing, the TJPA Geotechnical Inspection and Testing Agency will perform full time inspection of the fill and compaction process to verify procedure steps are followed, the suitability of the fill and that soils compaction is achieved.</p>					
	<p>these trenches will be filled with available approved suitable materials from onsite excavations or 300 psi CLSM as approved by the TJPA Rep.</p> <p>2. Geothermal piping trenches soils will be placed per Geothermal Spec. 23-57-34 Ground Loop Heat Exchanger which states "the trenches shall be back filled per IGSHPA with loose soil minimizing air gaps or voids and then marked with warning tape."</p> <p>3. Soil bedding and backfill around the loop and header piping, shall be placed to ensure there are no air gaps along the length of the pipe (water will not drain well, so will be used sparingly and only if necessary).</p> <p>4. All backfill will be placed in horizontal layers not more than (8) inches thick before compaction, and each layer shall be satisfactorily compacted by mechanical means (e.g. pogo stick/power puff tools) .</p> <p>5. Flooding or jetting will not be allowed.</p> <p>6. Soils will be compacted using steps above and best construction practices.</p> <p>7. Trench fill and adjacent areas will not be tested to verify the "not less than 95 percent maximum dry density" according to ASTM D1557. The TJPA Reps will not perform density and moisture content tests specified in the Trenching and Backfill Spec. 31-23-34. In lieu of testing, the TJPA Geotechnical Inspection and Testing Agency will perform full time inspection of the fill and compaction process to verify procedure steps are followed, the suitability of the fill and that soils compaction is achieved.</p>					
<b>T-1118</b>	<b>BGP - Knockout Wall Neoprene Pad Width Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>01/17/2014</b>	<b>01/27/2014</b>	<b>01/21/2014</b>
	<p><b>From:</b> Webcor Construction LP Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Please refer to attached drawing S1-3204.</p> <p>Details 1, 2, and 4 on S1-3204 call out a 1/4-inch x 8-inch continuous neoprene pad to be placed between the shear wall pilaster and the knockout wall. The bearing surface of the pilaster is 12-inches, so the 8-inch pad will no adequately cover the bearing surface.</p>					
	<p><b>ANSWER:</b></p> <p>Please refer to attached drawing S1-3204.</p> <p>Details 1, 2, and 4 on S1-3204 call out a 1/4-inch x 8-inch continuous neoprene pad to be placed between the shear wall pilaster and the knockout wall. The bearing surface of the pilaster is 12-inches, so the 8-inch pad will no adequately cover the bearing surface.</p>					





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1120</b>	<b>BGP - Horizontal Hooks in Shear Walls 2nd Lift and Above</b>	<b>Closed</b>	<b>CR</b>	<b>01/17/2014</b>	<b>01/27/2014</b>	<b>01/20/2014</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached drawing 4/S1-3261.  Please confirm if it is acceptable to replace the shear wall horizontal bars which have a 90-degree hook with a 180-degree hook. See attached drawing for more details. This change will only be applied to the second lift of shear walls and above (approx. EL. -20.56 and above).						<b>ANSWER:</b>  Please refer to attached drawing 4/S1-3261.  Please confirm if it is acceptable to replace the shear wall horizontal bars which have a 90-degree hook with a 180-degree hook. See attached drawing for more details. This change will only be applied to the second lift of shear walls and above (approx. EL. -20.56 and above).
<b>T-1121</b>	<b>SSS - Bus Deck Level Edge of Slab Plate Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>01/17/2014</b>	<b>01/27/2014</b>	<b>01/31/2014</b>
<b>From:</b> Webcor Construction LP Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI 233 SK1 to SK3 for reference.  After reviewing the structural steel documents for TG 07.1R, a detail is not provided for deck support around the bus deck level cast nodes. Please confirm that the intent is to utilize, 12 gauge sheet metal, in these areas as shown on 1/S1-5001. The sheet metal will follow the contour of the cast node, providing a 1" gap per A1-2893 (SK2).						<b>ANSWER:</b>  See attached CD RFI 233 SK1 to SK3 for reference.  After reviewing the structural steel documents for TG 07.1R, a detail is not provided for deck support around the bus deck level cast nodes. Please confirm that the intent is to utilize, 12 gauge sheet metal, in these areas as shown on 1/S1-5001. The sheet metal will follow the contour of the cast node, providing a 1" gap per A1-2893 (SK2).
<b>T-1122</b>	<b>SSS - Edge Plate Detail at Steel Drag Beam</b>	<b>Closed</b>	<b>CR</b>	<b>01/17/2014</b>	<b>01/27/2014</b>	<b>02/03/2014</b>
<b>From:</b> Webcor Construction LP Stephanie Azzolino						
<b>REQUEST:</b>  Reference detail 4 on S1-5022 which indicates that where "S" is less than or equal to 3", a double bent plate is to be used. Based on the information provided in SK RFI 266.1 SK2, this cannot be achieved, as the minimum overall height for a Z angle is 4" and the minimum thickness for a Z angle is ½".  1.) Please advise if it is acceptable to use the detail approved in RFI T-1032 (for sloping conditions), shown on SK RFI 266.1 SK1, at all locations where "S" is less than 4". 2.) Please advise if it is acceptable to use a Z angle at all locations where the "S" dimension is greater than or equal to 4" and the required thickness is ½", in lieu of the bolted						<b>ANSWER:</b>  Reference detail 4 on S1-5022 which indicates that where "S" is less than or equal to 3", a double bent plate is to be used. Based on the information provided in SK RFI 266.1 SK2, this cannot be achieved, as the minimum overall height for a Z angle is 4" and the minimum thickness for a Z angle is ½".  1.) Please advise if it is acceptable to use the detail approved in RFI T-1032 (for sloping conditions), shown on SK RFI 266.1 SK1, at all locations where "S" is less than 4". 2.) Please advise if it is acceptable to use a Z angle at all locations where the "S" dimension is greater than or equal to 4" and the required thickness is ½", in lieu



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
	<p>connection.</p> <p>3.) Please advise if it is acceptable to use the detail approved in RFI T-1032 (for sloping conditions), shown on SK RFI 266.1 SK1, at locations where "S" is greater than or equal to 4" and the required thickness is 3/8", in lieu of the bolted connection.</p> <p>4.) Note that based on the bending radius and edge distances shown in 4/S1-5022, the minimum height for bolted connections is 4 ¾" as indicated on CD RFI 215.1 SK1 attached. If the bolted connection is required, please verify the maximum height "S" for the bent plate detail in 4A/S1-5022 may be increased to 4 ¾".</p>					
	<p>of the bolted connection.</p> <p>3.) Please advise if it is acceptable to use the detail approved in RFI T-1032 (for sloping conditions), shown on SK RFI 266.1 SK1, at locations where "S" is greater than or equal to 4" and the required thickness is 3/8", in lieu of the bolted connection.</p> <p>4.) Note that based on the bending radius and edge distances shown in 4/S1-5022, the minimum height for bolted connections is 4 ¾" as indicated on CD RFI 215.1 SK1 attached. If the bolted connection is required, please verify the maximum height "S" for the bent plate detail in 4A/S1-5022 may be increased to 4 ¾".</p>					
<b>T-1122.1</b>	<b>SSS - Edge Plate Detail at Steel Drag Beam</b>	<b>Closed</b>	<b>CR</b>	<b>02/05/2014</b>	<b>02/15/2014</b>	<b>02/14/2014</b>
<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Detail 4 on S1-5022 calls for a double bent plate for slab support where "S" is less than or equal to 3". However, the minimum overall height for a double bent angle of the required thicknesses is 4" and the minimum overall height for the bolted connections is 4 ¾". Further, it has been determined that the minimum overall height for the detail proposed for sloping conditions is 2".</p> <p>For instances where the difference between the underside of the slab and top of beam is less than 2", please confirm it is acceptable to use an angle as depicted in CD RFI 279 SK2. Otherwise, please provide an alternate detail for this condition.</p> <p>A sample location of this condition has been included in SK1 for reference, where an angle is proposed for the 1" vertical height difference.</p>			<p>Detail 4 on S1-5022 calls for a double bent plate for slab support where "S" is less than or equal to 3". However, the minimum overall height for a double bent angle of the required thicknesses is 4" and the minimum overall height for the bolted connections is 4 ¾". Further, it has been determined that the minimum overall height for the detail proposed for sloping conditions is 2".</p> <p>For instances where the difference between the underside of the slab and top of beam is less than 2", please confirm it is acceptable to use an angle as depicted in CD RFI 279 SK2. Otherwise, please provide an alternate detail for this condition.</p> <p>A sample location of this condition has been included in SK1 for reference, where an angle is proposed for the 1" vertical height difference.</p>			





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1123</b>	<b>SSS - End Transfer Girder Details at GL 7C</b>	<b>Closed</b>	<b>CR</b>	<b>01/17/2014</b>	<b>01/27/2014</b>	<b>02/03/2014</b>
<b>From:</b> Skanska USA Civil West California DisRyan Clayton						
<b>REQUEST:</b> See attached CD RFI # 268 SK1 to SK3 for items 1 to 4: 1) Confirm the noted are acceptable location dimensions f or the headed studs per 7/S1-3702 SIM. 2) Confirm the noted are acceptable location dimensions f or the 2" dia. holes per 7/S1-3702 SIM. 3) Supply the location of the 2" dia. holes from top of girde r as shown. 4) Confirm the headed studs and 2" dia. holes may be mo ved as necessary to avoid fouling the stiffeners.			<b>ANSWER:</b> See attached CD RFI # 268 SK1 to SK3 for items 1 to 4: 1) Confirm the noted are acceptable location dimensio ns for the headed studs per 7/S1-3702 SIM. 2) Confirm the noted are acceptable location dimensio ns for the 2" dia. holes per 7/S1-3702 SIM. 3) Supply the location of the 2" dia. holes from top of g irder as shown. 4) Confirm the headed studs and 2" dia. holes may be moved as necessary to avoid fouling the stiffeners.			
<b>T-1124.1</b>	<b>SSS - Plate Grade Substitution</b>	<b>Closed</b>	<b>CR</b>	<b>02/21/2014</b>	<b>03/03/2014</b>	<b>03/03/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b> In response to RFI T-1124 & the Structural Coordination Meeting on February 6, 2014:  The moment frame columns and light column base plates are identified on the structural drawings for TG07.1R as material grade ASTM A572 Grade 50. Please confirm that it is acceptable to use ASTM A572 Grade 42 modified to achieve a minimum specified yield strength of 50 ksi for all plate exceeding 4½ in thickness at these locations.  Please note that the mill certification will read ASTM A572-Gr 42 but the reports will indicate a yield strength of 50 ksi.			<b>ANSWER:</b> In response to RFI T-1124 & the Structural Coordination Meeting on February 6, 2014:  The moment frame columns and light column base plates are identified on the structural drawings for TG07.1R as material grade ASTM A572 Grade 50. Please confirm that it is acceptable to use ASTM A572 Grade 42 modified to achieve a minimum specified yield strength of 50 ksi for all plate exceeding 4½ in thickness at these locations.  Please note that the mill certification will read ASTM A572-Gr 42 but the reports will indicate a yield strength of 50 ksi.			
<b>T-1125</b>	<b>BGP - Glass Guard Rail Embed A529 Grade 55 Steel in Lieu of A36</b>	<b>Closed</b>	<b>CR</b>	<b>01/21/2014</b>	<b>01/31/2014</b>	<b>01/30/2014</b>
<b>From:</b> Webcor Construction LP                      Jackson Tukuafu						
<b>REQUEST:</b> Please confirm it is acceptable to use A529 Grade 55 steel in lieu of A36 steel for the 3/8 x 7 flat bar portion of the glass guard rail embeds as shown on detail 7 of S1-3410.			<b>ANSWER:</b> Please confirm it is acceptable to use A529 Grade 55 steel in lieu of A36 steel for the 3/8 x 7 flat bar portion of the glass guard rail embeds as shown on detail 7 of S1-3410.			







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	<p>and the connection for the brace to the Girder per 8/S1-5015.</p> <p>2) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.</p> <p>3) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.</p> <p>4) Provide the noted dimensions to locate the 2" dia. holes.</p> <p>5) Provide locations for the 2 1/2" dia. holes from center of TR6 and from top of TR6.</p> <p>6) Provide dimension to locate the 2" dia. holes.</p> <p>7a) Confirm it is acceptable to move the headed studs or rebar holes as necessary to avoid fouling the stiffeners.</p> <p>7b) Provide the minimum clearance between the stiffener and the headed studs.</p>					<p>considering the end dimensions of TR4 shown on SK2 and the connection for the brace to the Girder per 8/S1-5015.</p> <p>2) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.</p> <p>3) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.</p> <p>4) Provide the noted dimensions to locate the 2" dia. holes.</p> <p>5) Provide locations for the 2 1/2" dia. holes from center of TR6 and from top of TR6.</p> <p>6) Provide dimension to locate the 2" dia. holes.</p> <p>7a) Confirm it is acceptable to move the headed studs or rebar holes as necessary to avoid fouling the stiffeners.</p> <p>7b) Provide the minimum clearance between the stiffener and the headed studs.</p>
T-1128	SSS - End Transfer Girder Details at GL 2C	Closed	CR	01/21/2014	01/31/2014	02/06/2014
From: Webcor Construction LP Stephanie Azzolino						
REQUEST:		ANSWER:				
<p>See attached CD RFI # 271 SK1 to SK3 for items 1 to 7:</p> <p>1) Supply the location of the braces from Grid C considering the end dimensions of TR2 shown on SK2 and the connection for the brace to the Girder per 8/S1-5015.</p> <p>2) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.</p> <p>3) Supply the underside of slab elevation at the brace located per dimension supplied in item 1 .</p> <p>4) Provide the noted dimensions to locate the 2" dia. holes.</p> <p>5) Provide locations for the 2 1/2" dia. holes from center of TR6 and from top of TR6.</p> <p>6) Provide dimension to locate the 2" dia. holes.</p> <p>7a) Confirm it is acceptable to move the headed studs or rebar holes as necessary to avoid fouling the stiffeners.</p> <p>7b) Provide the minimum clearance between the stiffener and the headed studs.</p>		<p>See attached CD RFI # 271 SK1 to SK3 for items 1 to 7:</p> <p>1) Supply the location of the braces from Grid C considering the end dimensions of TR2 shown on SK2 and the connection for the brace to the Girder per 8/S1-5015.</p> <p>2) Supply the underside of slab elevation at the brace located per dimension supplied in item 1.</p> <p>3) Supply the underside of slab elevation at the brace located per dimension supplied in item 1 .</p> <p>4) Provide the noted dimensions to locate the 2" dia. holes.</p> <p>5) Provide locations for the 2 1/2" dia. holes from center of TR6 and from top of TR6.</p> <p>6) Provide dimension to locate the 2" dia. holes.</p> <p>7a) Confirm it is acceptable to move the headed studs or rebar holes as necessary to avoid fouling the stiffeners.</p> <p>7b) Provide the minimum clearance between the stiffener and the headed studs.</p>				



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T-1129	SSS - End Transfer Girder Details at GL 5C	Closed	CR	01/21/2014	01/31/2014	02/06/2014
From: Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached CD RFI # 273 SK1 to SK4 for items 1 to 3: 1) Confirm the noted dimensions for locating the headed studs are acceptable or supply alternate dimensions. 2) Confirm the noted dimensions for locating the 2" dia. holes are acceptable or supply alternate dimensions. 3a) Confirm it is acceptable to move the headed studs or rebar holes as necessary to avoid fouling the stiffeners. 3b) Provide the minimum clearance between the stiffener and the headed studs.		See attached CD RFI # 273 SK1 to SK4 for items 1 to 3: 1) Confirm the noted dimensions for locating the headed studs are acceptable or supply alternate dimensions. 2) Confirm the noted dimensions for locating the 2" dia. holes are acceptable or supply alternate dimensions. 3a) Confirm it is acceptable to move the headed studs or rebar holes as necessary to avoid fouling the stiffeners. 3b) Provide the minimum clearance between the stiffener and the headed studs.				
T-1130	SSS - End Transfer Girder Details at GL 3C	Closed	CR	01/21/2014	01/31/2014	02/04/2014
From: Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached CD RFI # 274 SK1 to SK4 for items 1 to 4: 1) Supply the location for the holes in the stiffeners (information not shown on S1-3600): a) Dimensions from center of TR3 b) Dimension from top of bottom flange of TR3 2) 4/S1-3707 shows 5 1/2" and 6/S1-3702 shows 6" spacing for the headed studs. Confirm 5 1/2" in acceptable. 3) It is not clear where the 2" dia. holes are to be located. 4/S1-3707 shows the concrete extending to the bottom of TR3 and 6/S13702 shows the concrete stopping above the top of the boittom flange of TR3. Please confirm the location of the 2" dia. holes as shown on SK3 are acceptable or supply the location dimensions. 4a) Confirm it is acceptable to move the headed studs or rebar holes as necessary to avoid fouling the stiffeners. 4b) Provide the minimum clearance between the stiffener and the headed studs.		See attached CD RFI # 274 SK1 to SK4 for items 1 to 4: 1) Supply the location for the holes in the stiffeners (information not shown on S1-3600): a) Dimensions from center of TR3 b) Dimension from top of bottom flange of TR3 2) 4/S1-3707 shows 5 1/2" and 6/S1-3702 shows 6" spacing for the headed studs. Confirm 5 1/2" in acceptable. 3) It is not clear where the 2" dia. holes are to be located. 4/S1-3707 shows the concrete extending to the bottom of TR3 and 6/S13702 shows the concrete stopping above the top of the boittom flange of TR3. Please confirm the location of the 2" dia. holes as shown on SK3 are acceptable or supply the location dimensions. 4a) Confirm it is acceptable to move the headed studs or rebar holes as necessary to avoid fouling the stiffeners. 4b) Provide the minimum clearance between the stiffener and the headed studs.				
T-1131	SSS - Transfer Girder Shear Details at GL 1.4	Closed	CR	01/22/2014	02/01/2014	02/04/2014





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<b>T-1134</b>	<b>SSS - Transfer Girder Web Plate Detail at GL 9.9 &amp; 10.1</b>	<b>Closed</b>	<b>CR</b>	<b>01/22/2014</b>	<b>02/01/2014</b>	<b>02/04/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 283 SK1: The plates are shown as 2'-6 long on each side but the width of the concrete is only 3'-6 wide. This will result in the plates extending outside the concrete beam. Please confirm this is the intent or supply a revised plate length.			See attached CD RFI # 283 SK1: The plates are shown as 2'-6 long on each side but the width of the concrete is only 3'-6 wide. This will result in the plates extending outside the concrete beam. Please confirm this is the intent or supply a revised plate length.			
<b>T-1135</b>	<b>SSS - Transfer Girder Web Plate Details</b>	<b>Closed</b>	<b>CR</b>	<b>01/22/2014</b>	<b>02/01/2014</b>	<b>02/04/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 284 SK1 for items 1 to 4: 1) Confirm the plates are required on each side. 2) The plate is shown as 2'-6 long but the width of the concrete is only 3'-6 wide. This will result in the plates extending outside the concrete beam. Please confirm this is the intent or supply a revised plate length. 3) Confirm the correct reference is 9/S1-3701. 4) Confirm the edge of the plate should be aligned with the end of the Girder.			See attached CD RFI # 284 SK1 for items 1 to 4: 1) Confirm the plates are required on each side. 2) The plate is shown as 2'-6 long but the width of the concrete is only 3'-6 wide. This will result in the plates extending outside the concrete beam. Please confirm this is the intent or supply a revised plate length. 3) Confirm the correct reference is 9/S1-3701. 4) Confirm the edge of the plate should be aligned with the end of the Girder.			
<b>T-1136</b>	<b>SSS - Double Angle Connection</b>	<b>Closed</b>	<b>CR</b>	<b>01/23/2014</b>	<b>02/02/2014</b>	<b>02/04/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 272 SK1 to SK3 for items 1 & 2: 1) There is insufficient room to provide a double angel connection per 1/S1-5010 for the W12x14 & the W16x26. Confirm it is acceptable to supply a shear plate connection per 1/S1-5011 for the W16x26 to the W12x14 as shown or supply an alternate solution. 2) Confirm the W16x26 may be connected to the W16x26 using a shear plate similar to SK2 & SK3.			See attached CD RFI # 272 SK1 to SK3 for items 1 & 2: 1) There is insufficient room to provide a double angel connection per 1/S1-5010 for the W12x14 & the W16x26. Confirm it is acceptable to supply a shear plate connection per 1/S1-5011 for the W16x26 to the W12x14 as shown or supply an alternate solution. 2) Confirm the W16x26 may be connected to the W16x26 using a shear plate similar to SK2 & SK3.			



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T-1137	SSS - Drag Plate Splice Detail	Closed	CR	01/23/2014	02/02/2014	02/06/2014
From: Webcor Construction LP Stephanie Azzolino						
REQUEST:						ANSWER:
Please refer to attached sketches SK1 & SK2 for the following:						Please refer to attached sketches SK1 & SK2 for the following:
Due to lifting capacity while unloading material, THC will need to order the 3" plate at a maximum of 40'0" length Please confirm a shop splice using CPBG during fabrication to achieve the final lengths of 53'0" and 62'6".						Due to lifting capacity while unloading material, THC will need to order the 3" plate at a maximum of 40'0" length Please confirm a shop splice using CPBG during fabrication to achieve the final lengths of 53'0" and 62'6".
T-1138	SSS - Double Angle Connection	Closed	CR	01/24/2014	02/03/2014	02/06/2014
From: Webcor Construction LP Stephanie Azzolino						
REQUEST:						ANSWER:
See attached CD RFI # 281 SK1 & SK2: There is insufficient room to connect the W16x26 to the W27x84 using the double angle connection per 1/S1-5010.						See attached CD RFI # 281 SK1 & SK2: There is insufficient room to connect the W16x26 to the W27x84 using the double angle connection per 1/S1-5010.
Confirm it is acceptable to connect the W16x26 to the W27x84 using a shear plate per 1/S1-5011 as shown or supply an alternate solution.						Confirm it is acceptable to connect the W16x26 to the W27x84 using a shear plate per 1/S1-5011 as shown or supply an alternate solution.
T-1139	SSS - WT Deck Support Requirements	Closed	CR	01/24/2014	02/03/2014	02/07/2014
From: Webcor Construction LP Stephanie Azzolino						
REQUEST:						ANSWER:
See attached CD RFI # 282 SK1 to SK4 for items 1 to 5: 1) The noted WT is shown as stopping short of the concrete curb on S1-2304 (SK1). a) Confirm the WT is to extend as shown. b) It appears the WT will interfere with the rebars below the curb per 6/S1-5002 (SK2). Confirm the WT is located as shown and the rebars will be modified. 2) It is not clear from S1-2304 (SK1) what the deck support requirements are above the noted beam are. Please supply a detail. 3) It is not clear from S1-2304 (SK1) what the deck support requirements are above the noted beam are. Please supply a detail. 4) It appears the WT will interfere with the rebars below						See attached CD RFI # 282 SK1 to SK4 for items 1 to 5: 1) The noted WT is shown as stopping short of the concrete curb on S1-2304 (SK1). a) Confirm the WT is to extend as shown. b) It appears the WT will interfere with the rebars below the curb per 6/S1-5002 (SK2). Confirm the WT is located as shown and the rebars will be modified. 2) It is not clear from S1-2304 (SK1) what the deck support requirements are above the noted beam are. Please supply a detail. 3) It is not clear from S1-2304 (SK1) what the deck support requirements are above the noted beam are. Please supply a detail.



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	<p>the curb per 6/S1-5002 (SK2). Confirm the WT is located as shown and the rebars will be modified.</p> <p>5) The items below have been identified at specific locations on S1-2304 but similar conditions appear repeatedly on the Ground Level. Confirm the responses to items 1 to 4 may be applied typically on the Ground Floor at similar conditions.</p>					
T-1140	<p><b>SSS - Bus Deck Level Perimeter Weld Prep</b></p> <p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p> <p><b>REQUEST:</b></p> <p>See attached CD RFI # 285 SK1:</p> <p>The flange of the sloping W40x277 is 1/8" below the flange of the flange of the W40x297 at the top and 1/16" below at the bottom, making the requested CJP weld not achievable.</p> <p>Confirm the welds with the flange alignments as shown are acceptable.</p> <p>Note: moving the sloping W40x277 up to align with the top edge of the W40x297 is not an option as this will move the sloping W40x277 out of the normal sloping plane.</p> <p>Please note that this condition repeats at all the same conditions along Grids B &amp; H.</p>	Open	CR	01/24/2014	02/03/2014	02/06/2014
				</		





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<b>T-1141</b>	<b>SSS - AESS at Grand Hall and Shaw Alley Bridge</b>	<b>Closed</b>	<b>CR</b>	<b>01/27/2014</b>	<b>02/06/2014</b>	<b>02/12/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  1. Reference drawing A1-8661, issued for construction, which appears to have information missing for the Grand Hall AESS requirements. Please reference details E, F, and G on A1-8661 attached and clarify the AESS requirements at the noted locations. 2. Detail C on drawing A1-8661 indicates that the HSS 16x16x5/8" member supporting the Shaw Alley Bridge is AESS. However, details C and D on A1-8662 indicate that the HSS 16x16x5/8" member, BU girder, and HSS 5x1/2" posts at the Shaw Alley Bridge are to receive IFRM-1. Please clarify the coating requirements at the Shaw Alley Bridge.						<b>ANSWER:</b>  1. Reference drawing A1-8661, issued for construction, which appears to have information missing for the Grand Hall AESS requirements. Please reference details E, F, and G on A1-8661 attached and clarify the AESS requirements at the noted locations. 2. Detail C on drawing A1-8661 indicates that the HSS 16x16x5/8" member supporting the Shaw Alley Bridge is AESS. However, details C and D on A1-8662 indicate that the HSS 16x16x5/8" member, BU girder, and HSS 5x1/2" posts at the Shaw Alley Bridge are to receive IFRM-1. Please clarify the coating requirements at the Shaw Alley Bridge.
<b>T-1142</b>	<b>BGP - Grounding Rod at Buttress Pile in Zone 4</b>	<b>Closed</b>	<b>CR</b>	<b>01/27/2014</b>	<b>02/06/2014</b>	<b>02/03/2014</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to drawing E1-2026.  In Zone 4, an overlapping series of concrete buttress piles were poured along the North Wall of the excavation, extending towards the south wall.  In this area, the final grade of the excavation will be the concrete buttress piles. The attached photo shows the buttress pile layout with the grounding ring/ground rods overlayed on it. The ground rods need to be driven 10' deep. Please confirm that the rods which conflict with the buttress piles could be moved away from the north CDSM wall and to the void area of the buttress piles as shown in the attached SCCI sketch SK-SCCI_RFI421.						<b>ANSWER:</b>  Please refer to drawing E1-2026.  In Zone 4, an overlapping series of concrete buttress piles were poured along the North Wall of the excavation, extending towards the south wall.  In this area, the final grade of the excavation will be the concrete buttress piles. The attached photo shows the buttress pile layout with the grounding ring/ground rods overlayed on it. The ground rods need to be driven 10' deep. Please confirm that the rods which conflict with the buttress piles could be moved away from the north CDSM wall and to the void area of the buttress piles as shown in the attached SCCI sketch SK-SCCI_RFI421.
<b>T-1143</b>	<b>BSE - Reduced Micropile Testing Requirement in Unsuitable Material Areas</b>	<b>Closed</b>	<b>CR</b>	<b>01/27/2014</b>	<b>02/06/2014</b>	<b>01/28/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  Balfour Beatty Infrastructure Inc. (BBII) has experienced						<b>ANSWER:</b>  Balfour Beatty Infrastructure Inc. (BBII) has







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	<p>EDMH-B1-C-EMG in Electric Room B1541. Plan sheet E1-3204 Detail 6 referenced does not seem to be included in the contract drawings.</p> <p>DWG E1-4107 indicates the F15 fixtures in zone 7 are to be fed from EDMH-B2-D-EMG in Electric Room in Electric Room B1644 in Lower Concourse. Per the "Equipment Naming" char on DWG E1-0010, the B2 in the panel name indicates that it is on the Train Platform Level. However, the room number indicates that it is indeed on the Lower Concourse level.</p> <p>1. Please provide an enlarged room plan showing the location of each of the following panels: EDMH-B1-A-EMG, EDMH-B1-B-EMG and EDMH-B1-C-EMG</p> <p>2. Please confirm that panel EDMH-B1-D-EMG is in ROom B1644 per detail E1-3203 (dated 8/30/12) on that the panel was incorrectly labeled EDMH-B2-D-EMG on E1-4107</p>					<p>DWG E1-4105 and DWG E1-4106 indicate the F15 fixtures in Zone 5 and Zone 6 are to be fed from Panel EDMH-B1-C-EMG in Electric Room B1541. Plan sheet E1-3204 Detail 6 referenced does not seem to be included in the contract drawings.</p> <p>DWG E1-4107 indicates the F15 fixtures in zone 7 are to be fed from EDMH-B2-D-EMG in Electric Room in Electric Room B1644 in Lower Concourse. Per the "Equipment Naming" char on DWG E1-0010, the B2 in the panel name indicates that it is on the Train Platform Level. However, the room number indicates that it is indeed on the Lower Concourse level.</p> <p>1. Please provide an enlarged room plan showing the location of each of the following panels: EDMH-B1-A-EMG, EDMH-B1-B-EMG and EDMH-B1-C-EMG</p> <p>2. Please confirm that panel EDMH-B1-D-EMG is in ROom B1644 per detail E1-3203 (dated 8/30/12) on that the panel was incorrectly labeled EDMH-B2-D-EMG on E1-4107</p>
T-1144.1	BGP - Electrical Rooms B1222, B1223, B1560 & B1561 Enlarged Plan Sheet Discrepancies	Closed				
	From: Webcor Construction LP Claude Titcher					
	REQUEST: <p>Ref: E1-2202, E1-3201</p> <p>Electric Rooms B1222 and B1223 on plan sheet EI-2202 (Issued in ASI 104) and rooms B1560 and B1561 on plan sheet EI-2205 (Issued in ASI 104) do not match the enlarged room plans shown on EI-3201 (IFC) and EI-3202 (IFC) respectively.</p> <p>Please issue revised electrical drawings including, but not limited to EI-3201 and EI-3202.</p>					ANSWER: <p>Ref: E1-2202, E1-3201</p> <p>Electric Rooms B1222 and B1223 on plan sheet EI-2202 (Issued in ASI 104) and rooms B1560 and B1561 on plan sheet EI-2205 (Issued in ASI 104) do not match the enlarged room plans shown on EI-3201 (IFC) and EI-3202 (IFC) respectively.</p> <p>Please issue revised electrical drawings including, but not limited to EI-3201 and EI-3202.</p>
T-1145	BGP - Plumbing and Floor Drawing Detail Discrepancies	Closed	CR	01/27/2014	02/06/2014	02/10/2014
	From: Webcor Construction LP Jackson Tukuafu					
	REQUEST:					ANSWER:



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	<p>Please refer to attached drawing A1-2224, A1-2844, A1-2225, A1-2845, A1-2846, A1-2226 and excerpt from spec section 22 05 30, 3.2.</p> <p>Details for plumbing and floor drains in the drawings for the Lower Concourse have the following discrepancies:</p> <p>1. Drawing A1-2224 left of gridline (GL) 13 between GL B - GL C shows two plumbing details and in drawing A1-2844 these plumbing details are not shown</p> <p>2. Drawings A1-2845 and A1-2225 between GL 22 - GL 23 and GL G - GL H where A1-2845 shows a plumbing (PLBG) detail and A1-2225 shows a floor drain (FD) detail</p> <p>3. Drawings A1-2846 and A1-2226 between GL 29 - GL 30 and GL G - GL H where A1-2846 shows a plumbing (PLBG) detail and A1-2226 shows a floor drain (FD) detail</p> <p>4. A plumbing detail shown in drawing A-2225 on GL G between GL 24 - GL 24.9 is not shown in drawing A1-2845.</p> <p>5. Furthermore, the PLBG callouts in all the Architectural and Structural drawings do not include the size for each pipe or sleeve. Plumbing sleeve details in spec section 22 05 30 - 3, do not state the required clearance spacing needed.</p> <p>Please verify the conflicting plumbing and floor drain details, the diameter size of each pipe or sleeve detail, and clarify the clearance space for sleeves required for plumbing.</p>			<p>Please refer to attached drawing A1-2224, A1-2844, A1-2225, A1-2845, A1-2846, A1-2226 and excerpt from spec section 22 05 30, 3.2.</p> <p>Details for plumbing and floor drains in the drawings for the Lower Concourse have the following discrepancies:</p> <p>1. Drawing A1-2224 left of gridline (GL) 13 between GL B - GL C shows two plumbing details and in drawing A1-2844 these plumbing details are not shown</p> <p>2. Drawings A1-2845 and A1-2225 between GL 22 - GL 23 and GL G - GL H where A1-2845 shows a plumbing (PLBG) detail and A1-2225 shows a floor drain (FD) detail</p> <p>3. Drawings A1-2846 and A1-2226 between GL 29 - GL 30 and GL G - GL H where A1-2846 shows a plumbing (PLBG) detail and A1-2226 shows a floor drain (FD) detail</p> <p>4. A plumbing detail shown in drawing A-2225 on GL G between GL 24 - GL 24.9 is not shown in drawing A1-2845.</p> <p>5. Furthermore, the PLBG callouts in all the Architectural and Structural drawings do not include the size for each pipe or sleeve. Plumbing sleeve details in spec section 22 05 30 - 3, do not state the required clearance spacing needed.</p> <p>Please verify the conflicting plumbing and floor drain details, the diameter size of each pipe or sleeve detail, and clarify the clearance space for sleeves required for plumbing.</p>			
T-1145.1	BGP - Plumbing and Floor Drain Drawing Details	Closed	01	02/12/2014	02/22/2014	03/05/2014	
From: Webcor Construction LP		Claude Titcher					
REQUEST:		ANSWER:					
Reference RFI T-1145 Response BGP - Plumbing and Floor Drawing Detail Discrepancies		Reference RFI T-1145 Response BGP - Plumbing and Floor Drawing Detail Discrepancies					
The diameter size of PLBG callouts were not provided in the response (only sleeve schedule was provided).		The diameter size of PLBG callouts were not provided in the response (only sleeve schedule was provided).					



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	<p>TG06.0 Plumbing drawings are for reference only and not for construction and have not been revised through the current ASI's and SK.A's.</p> <p>Please provide diameter size of plumbing penetrations or revised plumbing drawings coordinated with updated Architectural and Structural drawings.</p>					
T-1145.2	<b>BGP - Plumbing and Floor Drain Drawing Details</b>	Closed	01	03/11/2014	03/21/2014	03/24/2014
	<p><b>From:</b> Webcor Construction LP                      Claude Titche</p> <p><b>REQUEST:</b></p> <p>Reference RFI T-1145.1 BGP - Response - Plumbing and Floor Drain Drawing Discrepancies</p> <p>The diameter size of PLBG callouts were not provided in ASI-112 and ASI-113 referred to by the response.</p> <p>Please provide diameter size of PLBG callouts.</p>					



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<b>T-1145.3</b>	<b>BGP - Plumbing and Floor Drain Drawing Details</b>	<b>Closed</b>	<b>01</b>	<b>03/24/2014</b>	<b>04/03/2014</b>	<b>03/27/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
As discussed in the 3.24.2014 submittal TG0600-121 comment review meeting, please issue current design drawings P1-2202 through P1-2211 so that the size of plumbing pipes that pass through the concourse slab can be ascertained for the purposes of sizing the plumbing penetration sleeves per PSK-0051 (issued in RFI 1145rev0). The -8.30.2012 Issued for Construction - Below Grade Package- P1-2202 through P1-2211 pipe plans do not match up to the current sleeve layout as shown in sheets A1-2842, A1-2843, A1-2844, A1-2845, A1-2846, A1-2850 & A1-2851.			As discussed in the 3.24.2014 submittal TG0600-121 comment review meeting, please issue current design drawings P1-2202 through P1-2211 so that the size of plumbing pipes that pass through the concourse slab can be ascertained for the purposes of sizing the plumbing penetration sleeves per PSK-0051 (issued in RFI 1145rev0). The -8.30.2012 Issued for Construction - Below Grade Package- P1-2202 through P1-2211 pipe plans do not match up to the current sleeve layout as shown in sheets A1-2842, A1-2843, A1-2844, A1-2845, A1-2846, A1-2850 & A1-2851.			
The size of the Concourse level slab sleeves required in the above mentioned A1-2800 series drawings cannot be determined without the current coordinated plumbing design.			The size of the Concourse level slab sleeves required in the above mentioned A1-2800 series drawings cannot be determined without the current coordinated plumbing design.			
<b>T-1146</b>	<b>SSS - W16 connection fouls W33 connection at grid 14F</b>	<b>Closed</b>	<b>CR</b>	<b>01/27/2014</b>	<b>02/06/2014</b>	<b>02/14/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
On S1-2304 there are W16's centered under CMU wall connecting into a W40 along grid F near grid 14. The drag connection for the W33 connecting into the Transfer Girder and the W16 connections will foul each other. Please verify the following: 1. Per the schedule on 1/S1-5010 the W16 connections should be three bolts. Please verify it is acceptable to reduce the bolts form three to two bolts? 2. Please verify it is acceptable to move the drag connection down to clear the two bolt W16 connection? 3. Please verify coping the bottom flange of the W16's to clear the W40 drag connection is acceptable?			On S1-2304 there are W16's centered under CMU wall connecting into a W40 along grid F near grid 14. The drag connection for the W33 connecting into the Transfer Girder and the W16 connections will foul each other. Please verify the following: 1. Per the schedule on 1/S1-5010 the W16 connections should be three bolts. Please verify it is acceptable to reduce the bolts form three to two bolts? 2. Please verify it is acceptable to move the drag connection down to clear the two bolt W16 connection? 3. Please verify coping the bottom flange of the W16's to clear the W40 drag connection is acceptable?			



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T-1147	SSS - Double Angle Shear Connection	Closed	CR	02/03/2014	02/13/2014	02/13/2014
From: Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached CD RFI # 277 SK1 to SK3 for items 1 to 3: In order to maintain interior TOS (Top of Steel) elevations shown on the contract documents, the connection detail as shown on 1/S1-5010 needs to be modified along the ground level perimeter beams at certain locations. Please confirm the following: 1) Confirm the proposed connection is acceptable as shown or supply an alternate detail. 2) Confirm the proposed connection is acceptable as shown or supply an alternate detail. 3) Confirm the connections in items 1 & 2 may be applied typically at other similar conditions.		See attached CD RFI # 277 SK1 to SK3 for items 1 to 3: In order to maintain interior TOS (Top of Steel) elevations shown on the contract documents, the connection detail as shown on 1/S1-5010 needs to be modified along the ground level perimeter beams at certain locations. Please confirm the following: 1) Confirm the proposed connection is acceptable as shown or supply an alternate detail. 2) Confirm the proposed connection is acceptable as shown or supply an alternate detail. 3) Confirm the connections in items 1 & 2 may be applied typically at other similar conditions.				
T-1147.1	SSS - Ground Level Perimeter Framing Clarification at GL14	Closed	CR	03/17/2014	03/27/2014	03/31/2014
From: Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached CD RFI # 331 SK1:  The connection per RFI T-1147 (SK 356, CD 277) item 2 on SK3 will not work here as the connection angles will foul the Girder connection to the column as shown.  Confirm it is acceptable to connect the W16 to the W40 per 1/S1-5028 with S < 12" or supply an alternate connection detail.		See attached CD RFI # 331 SK1:  The connection per RFI T-1147 (SK 356, CD 277) item 2 on SK3 will not work here as the connection angles will foul the Girder connection to the column as shown.  Confirm it is acceptable to connect the W16 to the W40 per 1/S1-5028 with S < 12" or supply an alternate connection detail.				
T-1148	SSS - Steel Chemical Composition	Closed	CR	02/03/2014	02/13/2014	02/12/2014
From: Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Basket column pipes of wall thickness less than or equal to 1" will be produced from rolled and seam welded plate per API-5L (X65 for ground level to bus deck level pipes and X52 for bus deck level to roof level). For wall thicknesses greater than 1", the pipes will be produced using the centrifugal cast process as defined in the project		Basket column pipes of wall thickness less than or equal to 1" will be produced from rolled and seam welded plate per API-5L (X65 for ground level to bus deck level pipes and X52 for bus deck level to roof level). For wall thicknesses greater than 1", the pipes will be produced using the centrifugal cast process as				



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T-1149	<b>SSS - Erection Aids for Studded Plate</b>  <b>From:</b> Webcor Construction LP                      Stephanie Azzolino  <b>REQUEST:</b>  Please confirm the concept of the temporary erection angles required to field weld the studded plates detailed in SK 1 are acceptable and can be incorporated into the model.	Closed	CR	02/03/2014	02/13/2014	02/10/2014
						<b>ANSWER:</b>  Please confirm the concept of the temporary erection angles required to field weld the studded plates detailed in SK1 are acceptable and can be incorporated into the model.
T-1150	<b>BGP - Geothermal Pressure Gauge Monitoring</b>  <b>From:</b> Webcor Construction LP                      Jackson Tukuafu  <b>REQUEST:</b>  Spec section 23 57 34, Part 3.2.J the geothermal loops are to be pressurized to 60 psi and monitored during construction to detect possible damage. The geothermal loops will be re-pressurized to 60 psi and the gauges orientated to be viewed from the trestle. Please confirm these gauges can be monitored and documented weekly until the completion of Airco's contract scope.	Closed	CR	02/05/2014	02/15/2014	02/14/2014
						<b>ANSWER:</b>  Spec section 23 57 34, Part 3.2.J the geothermal loops are to be pressurized to 60 psi and monitored during construction to detect possible damage. The geothermal loops will be re-pressurized to 60 psi and the gauges orientated to be viewed from the trestle. Please confirm these gauges can be monitored and documented weekly until the completion of Airco's contract scope.
T-1152	<b>SSS - Added Steel Members at Stair 501</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer  <b>REQUEST:</b>  Drawing S1-2205 on ASI 106 appears to add steel members at Stair 501 (see SK1) on the lower concourse level. The associated detail, 6/S1-7016, was not updated to reflect this change. Please advise if these members were added in error. If this is an intended change, please provide the appropriate details.	Closed	CR	02/05/2014	02/15/2014	02/26/2014
						<b>ANSWER:</b>  Drawing S1-2205 on ASI 106 appears to add steel members at Stair 501 (see SK1) on the lower concourse level. The associated detail, 6/S1-7016, was not updated to reflect this change. Please advise if these members were added in error. If this is an intended change, please provide the appropriate details.





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T-1153	<b>BGP - Geothermal Riser 11 Location</b>  From: Webcor Construction LP Jackson Tukuafu	Closed	CR	02/05/2014	02/15/2014	02/14/2014
<b>REQUEST:</b>  Please confirm the Geothermal Riser 11 can be installed between soldier pile 274 and 275. This location is within 1' of contract drawings. Note that due to schedule constraints, the chipping has already begun at this location.		<b>ANSWER:</b>  Please confirm the Geothermal Riser 11 can be installed between soldier pile 274 and 275. This location is within 1' of contract drawings. Note that due to schedule constraints, the chipping has already begun at this location.				
T-1154	<b>BGP - Consolidation of Geothermal Fields 13 14 15</b>  From: Webcor Construction LP Claude Titché	Closed	01	02/07/2014	02/17/2014	02/14/2014
<b>REQUEST:</b>  Please confirm Airco is to consolidate Fields 13(4 loops), FI4(10 loops) and FI5(6 loops) totaling 20 loops into ONLY Fields 13 and 14 each with 10 loops(20 loops total) per discussion with EOR James Bradshaw on 2/4/14.  Please provide direction for the new Riser Locations for Fields 13 and 14: Option A) Field 13- 10ft East of GL 33, Field 14- 10ft West of GL 33 Option B) Field 13- East of GL 31, Field 14- 10' East of GL 33		<b>ANSWER:</b>  Please confirm Airco is to consolidate Fields 13(4 loops), FI4(10 loops) and FI5(6 loops) totaling 20 loops into ONLY Fields 13 and 14 each with 10 loops(20 loops total) per discussion with EOR James Bradshaw on 2/4/14.  Please provide direction for the new Riser Locations for Fields 13 and 14: Option A) Field 13- 10ft East of GL 33, Field 14- 10ft West of GL 33 Option B) Field 13- East of GL 31, Field 14- 10' East of GL 33				
T-1155	<b>BGP - SFPUC Grounding Company Room B1441</b>  From: Webcor Construction LP Claude Titché	Closed	01	02/07/2014	02/17/2014	02/19/2014
<b>REQUEST:</b>  There are 5 grounding PUC risers terminating in the SFPUC Grounding EAST room B1441, however Detail 4 on E 1-6006 indicates seven(7) ground rod risers which are to be welded to the mesh ground grid. Please confirm the number of grounding risers in the East room B1441.		<b>ANSWER:</b>  There are 5 grounding PUC risers terminating in the SFPUC Grounding EAST room B1441, however Detail 4 on E 1-6006 indicates seven(7) ground rod risers which are to be welded to the mesh ground grid. Please confirm the number of grounding risers in the East room B1441.				
T-1156	<b>BGP - Lighting and Telecom Layout Drawing Discrepancies (A vs E)</b>  From: Webcor Construction LP Claude Titché	Closed	01	02/07/2014	02/17/2014	02/19/2014
<b>REQUEST:</b>  There are some discrepancies between the Train Platform		<b>ANSWER:</b>  There are some discrepancies between the Train				





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	<p>Level Lighting Plans(E Drawings) and the Lower Concourse Level Slab Edge Plans(A Drawings). Please provide direction and revised drawings for the following instances.</p> <p>1) The Lighting Plans show two exit signs in close proximity, yet on the Slab Edge Plan only one EJB layout is shown: AI-2844/EI-4104 at approximate Grid Lines: -14.5/F -16.6/A.6 -16.7/F AI-2850/EI-4110 at approximate Grid Lines: -2.8/V.8</p> <p>2) The Slab Edge Plan shows EJB layout, yet there are no fixtures on the Lighting Plan: AI-2842/EI-4102 at approximate Grid Lines: -1.6/C.5 -2.7-3.5/C.3 (Three EJBs in a row) -4.5/C.3 (Two EJBs) AI-2846/EI-4106 at approximate Grid Lines: -25.7/F.I -26.6/C.5 -26.7/A.6</p> <p>3) The Lighting Plan shows fixtures, yet there is no layout on the Slab Edge Plan: AI-2845/EI-4105 at approximate Grid Lines: -19.2/A.6 -21.2/C.6 -21.2/F</p> <p>4) There was no Train Platform Level Zone 11 Lighting Plan (EI-4111) included with the contract documents, but there is an EJB layout on Lower Concourse Level Zone 11 Slab Edge Plan(AI-2851 ).</p> <p>5) Telecommunications Drawing Lower Concourse Level Zone 2 Floor Plan(TEI-2202) has six(6) 4" conduit sleeves on Grid Line I between GL E and F. the Slab Edge Plan(AI-2842) does not have a layout for these sleeves.</p>					
	<p>Platform Level Lighting Plans(E Drawings) and the Lower Concourse Level Slab Edge Plans(A Drawings). Please provide direction and revised drawings for the following instances.</p> <p>1) The Lighting Plans show two exit signs in close proximity, yet on the Slab Edge Plan only one EJB layout is shown: AI-2844/EI-4104 at approximate Grid Lines: -14.5/F -16.6/A.6 -16.7/F AI-2850/EI-4110 at approximate Grid Lines: -2.8/V.8</p> <p>2) The Slab Edge Plan shows EJB layout, yet there are no fixtures on the Lighting Plan: AI-2842/EI-4102 at approximate Grid Lines: -1.6/C.5 -2.7-3.5/C.3 (Three EJBs in a row) -4.5/C.3 (Two EJBs) AI-2846/EI-4106 at approximate Grid Lines: -25.7/F.I -26.6/C.5 -26.7/A.6</p> <p>3) The Lighting Plan shows fixtures, yet there is no layout on the Slab Edge Plan: AI-2845/EI-4105 at approximate Grid Lines: -19.2/A.6 -21.2/C.6 -21.2/F</p> <p>4) There was no Train Platform Level Zone 11 Lighting Plan (EI-4111) included with the contract documents, but there is an EJB layout on Lower Concourse Level Zone 11 Slab Edge Plan(AI-2851 ).</p> <p>5) Telecommunications Drawing Lower Concourse Level Zone 2 Floor Plan(TEI-2202) has six(6) 4" conduit sleeves on Grid Line I between GL E and F. the Slab Edge Plan(AI-2842) does not have a layout for these sleeves.</p>					



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<b>T-1156.1</b>	<b>BGP - Telecom Drawing Discrepancies</b>	<b>Closed</b>	<b>01</b>	<b>02/25/2014</b>	<b>03/07/2014</b>	<b>03/13/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						
The response to RFI T-1156 provided a number of drawings which were not previously provided. Only TE1 drawings TE1-0000, TE1-2202, TE12203 and TE12207 were issued with the TG06 scope of work.						
The telecom sleeve locations on SKA-3060, SKA-3063 and SKA-3064 do not match the TE1 contract drawings.						
The discrepancies between the telecom drawings and the slab edge drawings are:						
I) GL 5/A - Telecom sleeves indicated on TEI-2202, no layout dimensions on SKA-3059						
2) GL 10/J -Telecomsleeves on TEI-2203 are not shown in the same location as on SKA-3060						
3) GL 29.5/A - No TEI drawing provided, however SKA-3063 has layout for three(3) sleeves						
4) GL 33/A - Telecom sleeves indicated on TEI-2207, but no layout dimensions on SKA-3064						
Please provide an up to date set of ALL TE I drawings which show telecom sleeves and are consistent with the Lower Concourse Level Slab Edge Drawings.						
<b>ANSWER:</b>						
The response to RFI T-1156 provided a number of drawings which were not previously provided. Only TE1 drawings TE1-0000, TE1-2202, TE12203 and TE12207 were issued with the TG06 scope of work.						
The telecom sleeve locations on SKA-3060, SKA-3063 and SKA-3064 do not match the TE1 contract drawings.						
The discrepancies between the telecom drawings and the slab edge drawings are:						
I) GL 5/A - Telecom sleeves indicated on TEI-2202, no layout dimensions on SKA-3059						
2) GL 10/J -Telecomsleeves on TEI-2203 are not shown in the same location as on SKA-3060						
3) GL 29.5/A - No TEI drawing provided, however SKA-3063 has layout for three(3) sleeves						
4) GL 33/A - Telecom sleeves indicated on TEI-2207, but no layout dimensions on SKA-3064						
Please provide an up to date set of ALL TE I drawings which show telecom sleeves and are consistent with the Lower Concourse Level Slab Edge Drawings.						
<b>T-1157</b>	<b>SSS - Fireproofing Clarification At Light Column</b>	<b>Closed</b>	<b>CR</b>	<b>02/07/2014</b>	<b>02/17/2014</b>	<b>02/20/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>						
See attached CD RFI # 293 SK1 for items 1 to 3:						
1) Supply the elevation to determine the termination of IFRM-2.						
2) Confirm the IFRM-2 finish applies to the sides and top of the base plates but not the bottom surface.						
3) Supply the finish requirements for the plate washers and the protection caps for the anchor bolts above the base plates.						
<b>ANSWER:</b>						
See attached CD RFI # 293 SK1 for items 1 to 3:						
1) Supply the elevation to determine the termination of IFRM-2.						
2) Confirm the IFRM-2 finish applies to the sides and top of the base plates but not the bottom surface.						
3) Supply the finish requirements for the plate washers and the protection caps for the anchor bolts above the base plates.						



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<b>T-1158</b>	<b>BGP - Geothermal Field 12 Layout</b>	<b>Closed</b>	<b>CR</b>	<b>02/07/2014</b>	<b>02/17/2014</b>	<b>02/14/2014</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1Please confirm it is acceptable to decrease the minimum 4-inch center to center dimension required by the specifications to 2-feet center to center in order to install the required 10 loops in Geo Field 12 in the new East to West orientation. The 83-inch diameter will be maintained at the end of the loops. Please note that the 2-foot center to center dimension will require significant hand digging to maintain trench separation and stability.			1Please confirm it is acceptable to decrease the minimum 4-inch center to center dimension required by the specifications to 2-feet center to center in order to install the required 10 loops in Geo Field 12 in the new East to West orientation. The 83-inch diameter will be maintained at the end of the loops. Please note that the 2-foot center to center dimension will require significant hand digging to maintain trench separation and stability.			
2. Please confirm it is acceptable to reduce the geothermal 6" clearance around micropiles to 4".			2. Please confirm it is acceptable to reduce the geothermal 6" clearance around micropiles to 4".			
<b>T-1159</b>	<b>SSS- Bracing Requirements at W-1 Connections</b>	<b>Closed</b>	<b>CR</b>	<b>02/07/2014</b>	<b>02/17/2014</b>	<b>02/20/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See at tached CD RFI # 286 SK1 to SK4: The attached sketches show (2) conditions where the back-up angle braces at the "CP6" (W-1 facade connection) connect ions are very close to the floor beams. Please review and confirm that the braces are required at all noted locations no matter what their proximity to the beams is. If not, supply a maximum/minimum off-set dimension criteria for omitting the braces.			See at tached CD RFI # 286 SK1 to SK4: The attached sketches show (2) conditions where the back-up angle braces at the "CP6" (W-1 facade connection) connect ions are very close to the floor beams. Please review and confirm that the braces are required at all noted locations no matter what their proximity to the beams is. If not, supply a maximum/minimum off-set dimension criteria for omitting the braces.			
<b>T-1160</b>	<b>BGP - Spandrel Beam Modifications in Area 10</b>	<b>Closed</b>	<b>01</b>	<b>02/12/2014</b>	<b>02/22/2014</b>	<b>02/13/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Documents: Exhibits A - B			Reference Documents: Exhibits A - B			
Further to response to RFI T-637 please find attached proposed changes to the spandrel beams in pour Area 10 for location plan see exhibit - A			Further to response to RFI T-637 please find attached proposed changes to the spandrel beams in pour Area 10 for location plan see exhibit - A			
Exhibit - B shows the plan view of the modification necessary to the spandrel beam on the north and south elevations due to the revised reinforcement width of the foundation wall due to encroachment of the CDSM beams			Exhibit - B shows the plan view of the modification necessary to the spandrel beam on the north and south elevations due to the revised reinforcement width of the foundation wall due to encroachment of			



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	as well as typical cross sections of the revised spandrel beams.  RFI T - 0743 shows the extent of the modification to the foundation wall on the north and south elevations of Area 10.  Please confirm that these modifications as outlined at these locations are acceptable.					
	the CDSM beams as well as typical cross sections of the revised spandrel beams.  RFI T - 0743 shows the extent of the modification to the foundation wall on the north and south elevations of Area 10.  Please confirm that these modifications as outlined at these locations are acceptable.					
<b>T-1161</b>	<b>BSE- Replacement and Removal of Waler Lookouts GL 9.5 West</b>	<b>Closed</b>	<b>01</b>	<b>02/11/2014</b>	<b>02/21/2014</b>	<b>02/18/2014</b>
<b>From:</b> Webcor Construction LP      Robert Kjome						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Due to a revision in the concourse slab elevation, waler lookouts from gridline 9.5 west must be removed to allow for construction of the slab. BBII's EOR for the internal bracing has approved the use of additional 6x6x3/8" angle braces to replace the lookouts in conflict. Reference the attached RFI response, supplemental calculations, and details from PB&A.  Please confirm it is acceptable to proceed with removal of the lookouts per PB&A's RFI response.			Due to a revision in the concourse slab elevation, waler lookouts from gridline 9.5 west must be removed to allow for construction of the slab. BBII's EOR for the internal bracing has approved the use of additional 6x6x3/8" angle braces to replace the lookouts in conflict. Reference the attached RFI response, supplemental calculations, and details from PB&A.  Please confirm it is acceptable to proceed with removal of the lookouts per PB&A's RFI response.			





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	<p>indicated.</p> <p>PB &amp; A response :The replacement WF beams or angles shall be installed on each side of the pipe strut. If an existing angle brace has been installed on the soldier beam on the side of the pipe strut, the replacement WF beam or the angle shall be installed on the adjacent soldier beam away from the pipe strut. The angle brace is move from the waler beam flange for accessibility per sketch SK-1 attached.</p> <p>3. Either an identical WF beam or an angle brace as proposed for a replacement element of the existing lookouts is to be attached to WF soldier pile beams above a waler according to the RFI. However, a number of soldier piles already connect angle braces, and those soldier piles with an existing brace may have a difficulty to accommodate an additional WF beam or angle. Lookouts were placed at least 4 per waler and one on each side of a strut per the drawings and calculations, and the same number and locations of replacement elements are required.</p> <p>Please verify the existing angle braces are to be used as a part of the lookout replacements or the same number of new angle braces as the existing lookout beams are to be added in addition to the existing angle braces. Please verify the proposed replacement elements can be installed without a conflict of existing angle braces.</p> <p>PB &amp; A response: At least the same number of replacement WF beams as that of the existing lookouts shall be installed. For the replacement angle option, minimum 4 additional angle braces excluding the existing ones are required.</p> <p>Please confirm it is acceptable to proceed with removal of the lookouts per PB&amp;A's RFI response. Note that the RFI response provided by PB&amp;A has been amended to address comments from urs in response to RFU T-1161.</p>					<p>capacities. If this is not achieved, the welding provided cannot be relied upon for transmission of loads. If an angle brace needs to move away from waler beam flange for accessibility, the distance shall be indicated.</p> <p>PB &amp; A response :The replacement WF beams or angles shall be installed on each side of the pipe strut. If an existing angle brace has been installed on the soldier beam on the side of the pipe strut, the replacement WF beam or the angle shall be installed on the adjacent soldier beam away from the pipe strut. The angle brace is move from the waler beam flange for accessibility per sketch SK-1 attached.</p> <p>3. Either an identical WF beam or an angle brace as proposed for a replacement element of the existing lookouts is to be attached to WF soldier pile beams above a waler according to the RFI. However, a number of soldier piles already connect angle braces, and those soldier piles with an existing brace may have a difficulty to accommodate an additional WF beam or angle. Lookouts were placed at least 4 per waler and one on each side of a strut per the drawings and calculations, and the same number and locations of replacement elements are required.</p> <p>Please verify the existing angle braces are to be used as a part of the lookout replacements or the same number of new angle braces as the existing lookout beams are to be added in addition to the existing angle braces. Please verify the proposed replacement elements can be installed without a conflict of existing angle braces.</p> <p>PB &amp; A response: At least the same number of replacement WF beams as that of the existing lookouts shall be installed. For the replacement angle option, minimum 4 additional angle braces excluding the existing ones are required.</p> <p>Please confirm it is acceptable to proceed with</p>



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<b>T-1162.1</b>	<b>SSS - AESS Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>03/10/2014</b>	<b>03/20/2014</b>	<b>03/17/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 289 SK1 for items 1 to 3:  1) Please confirm that the AESS begins immediately above the IFRM-2 areas noted on details A & E/A1-8662  2) Supply the noted dimension to determine the AESS boundary  3) Supply the noted angle to determine the AESS boundary					<b>ANSWER:</b> See attached CD RFI # 289 SK1 for items 1 to 3:  1) Please confirm that the AESS begins immediately above the IFRM-2 areas noted on details A & E/A1-8662  2) Supply the noted dimension to determine the AESS boundary  3) Supply the noted angle to determine the AESS boundary	
<b>T-1163</b>	<b>SSS - Train Box Column Cap Plate Machining</b>	<b>Closed</b>	<b>CR</b>	<b>02/11/2014</b>	<b>02/21/2014</b>	<b>02/14/2014</b>
<b>From:</b> Webcor Construction LP Stephanie Azzolino						
<b>REQUEST:</b> Detail 4/S1-5052 indicates "Grind Surface to Extra Smooth" for the surface finish required for the machined out section of the Train Box Column Cap plate. This is not an industry standard reference. Our fabricator proposes to machine this surface to 125 RA. See attached backup information.  Please confirm this is acceptable.					<b>ANSWER:</b> Detail 4/S1-5052 indicates "Grind Surface to Extra Smooth" for the surface finish required for the machined out section of the Train Box Column Cap plate. This is not an industry standard reference. Our fabricator proposes to machine this surface to 125 RA. See attached backup information.  Please confirm this is acceptable.	
<b>T-1164</b>	<b>SSS - Connection Details at Corner Roof Girders</b>	<b>Closed</b>	<b>CR</b>	<b>02/12/2014</b>	<b>02/22/2014</b>	<b>02/25/2014</b>
<b>From:</b> Webcor Construction LP Stephanie Azzolino						
<b>REQUEST:</b> Please see attached Roof Girder Blank drawing submittal returned by WOJV on 1/27/14 "Approved as Noted". The Engineer's directive to change the configuration of the "stub beam" is inconsistent with the current Plan Sheets. Upon further discussions held in the Structural Coordination meeting on 2/6/14, OIOW has been directed to proceed with the 2.25" T&B flange at the stub beam, as detailed on 10/S1-8001, and modify the web of the stub beam to align with grid lines 1 and 33.5.					<b>ANSWER:</b> Please see attached Roof Girder Blank drawing submittal returned by WOJV on 1/27/14 "Approved as Noted". The Engineer's directive to change the configuration of the "stub beam" is inconsistent with the current Plan Sheets. Upon further discussions held in the Structural Coordination meeting on 2/6/14, OIOW has been directed to proceed with the 2.25" T&B flange at the stub beam, as detailed on 10/S1-8001, and modify the web of the stub beam to align with grid lines 1 and 33.5.	





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T-1165	<p>Please see attached "preliminary" shop drawings representing the requested changes to the stub beam detail.</p> <p>Please confirm the changes to this drawing are consistent with the Engineer's intent. Upon confirmation, OIW will make the appropriate changes to drawings GB107 through GB110 and resubmit for record.</p> <p>Please also note that the weld prep in detail 5 on GB107-2 has been modified per the 2/6/14 coordination meeting.</p>	Closed	01	02/12/2014	02/22/2014	02/21/2014
	<p><b>BSE - Relocate Micropiles E872 and E874</b></p> <p><b>From:</b> Webcor Construction LP                      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>BBII discovered that Micropiles E872 and E874 were in conflict with struts STD-71 and STD-70 respectively. BBII proposes the following:</p> <p>- Relocate Micropile W872 3' South West - Relocate Micropile W874 8' South West</p> <p>Please confirm these changes are acceptable.</p>					
	<p>Please see attached "preliminary" shop drawings representing the requested changes to the stub beam detail.</p> <p>Please confirm the changes to this drawing are consistent with the Engineer's intent. Upon confirmation, OIW will make the appropriate changes to drawings GB107 through GB110 and resubmit for record.</p> <p>Please also note that the weld prep in detail 5 on GB107-2 has been modified per the 2/6/14 coordination meeting.</p>					
	<p><b>ANSWER:</b></p> <p>BBII discovered that Micropiles E872 and E874 were in conflict with struts STD-71 and STD-70 respectively. BBII proposes the following:</p> <p>- Relocate Micropile W872 3' South West - Relocate Micropile W874 8' South West</p> <p>Please confirm these changes are acceptable.</p>					
T-1166	<p><b>SSS - Dimension Clarification</b></p> <p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p> <p><b>REQUEST:</b></p> <p>Please see attached red lines and confirm correct north elevation dimensions for the basket column work points.</p>	Closed	CR	02/12/2014	02/22/2014	02/25/2014
	<p><b>ANSWER:</b></p> <p>Please see attached red lines and confirm correct north elevation dimensions for the basket column work points.</p>					



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<b>T-1167</b>	<b>BGP - Geothermal Manifold Location for Risers 3 and 4</b>	<b>Closed</b>	<b>01</b>	<b>02/12/2014</b>	<b>02/22/2014</b>	<b>02/21/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Per discussions with Geothermal EOR (WSP), Airco was directed to route the geothermal risers below the bottom of the Air Duct and above the top of the conduit rack against the foundation wall.  Please confirm the attached sleeve detail for Geothermal Riser 3 & 4 is acceptable.		<b>ANSWER:</b>  Per discussions with Geothermal EOR (WSP), Airco was directed to route the geothermal risers below the bottom of the Air Duct and above the top of the conduit rack against the foundation wall.  Please confirm the attached sleeve detail for Geothermal Riser 3 & 4 is acceptable.				
<b>T-1168</b>	<b>SSS - HSS Splice Detail</b>	<b>Closed</b>	<b>CR</b>	<b>02/12/2014</b>	<b>02/22/2014</b>	<b>02/24/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Please reference attached sketch SK1:  The detail on 3/S1-7630 calls for a flare bevel complete penetration weld. Skanska believes that this is not a flare bevel condition and proposes the use of a CP weld with a ¼" root and 30 degree weld prep with full backing plate.  Please confirm this is acceptable.		<b>ANSWER:</b>  Please reference attached sketch SK1:  The detail on 3/S1-7630 calls for a flare bevel complete penetration weld. Skanska believes that this is not a flare bevel condition and proposes the use of a CP weld with a ¼" root and 30 degree weld prep with full backing plate.  Please confirm this is acceptable.				
<b>T-1169</b>	<b>BGP - Geothermal Manifold Locations for Risers 5 through 10</b>	<b>Closed</b>	<b>01</b>	<b>02/13/2014</b>	<b>02/23/2014</b>	<b>02/21/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Per discussions with Geothermal EOR (WSP), SCCI was directed to route the geothermal risers below the bottom of the Air Duct and above the top of the conduit rack against the foundation wall.  Please confirm the attached sleeve detail for geothermal riser 5 through 10 is acceptable.		<b>ANSWER:</b>  Per discussions with Geothermal EOR (WSP), SCCI was directed to route the geothermal risers below the bottom of the Air Duct and above the top of the conduit rack against the foundation wall.  Please confirm the attached sleeve detail for geothermal riser 5 through 10 is acceptable.				
<b>T-1170</b>	<b>SSS - Light Column Base Plate and Corrosion Protection</b>	<b>Closed</b>	<b>CR</b>	<b>02/13/2014</b>	<b>02/23/2014</b>	<b>02/25/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  As a follow up to review comments provided in Submittal		<b>ANSWER:</b>  As a follow up to review comments provided in				



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	<p>Package TG0701-023.1 SSS - Light Column Anchor Bolts (#1.5E), please review and respond to the following items:</p> <p>1) Base plate hole oversize and subsequent sizing of the top plate washer</p> <p>As a follow up to the conference call held 1/29/14 for the Light Column anchor bolts, Dywidag provided the following information via the email attached: "If we spec a 50ksi plate and use the full width of the spherical washer, the plate works as-is. Leaving the plate design alone (with the assumed actual contact width of the washer) the plate still works at a 6" hole diameter. This is designing for 95% of the ultimate capacity of the bar."</p> <p>The maximum outside diameter of the HSS tube inside the base plate is 3.5". Within a 6" hole, this allows for 1.25" of clearance all around the tube, which is more than adequate clearance based on the AISC allowable tolerances for installations of anchor-rod groups. Reference AISC Steel Construction Manual, Section 16.7.5.1 attached, which does not allow for any more than ¼" variation between anchor rod groups nor between anchor rod group and column lines. Please confirm it is acceptable to proceed with a 6" base plate hole size to avoid modifications to the top anchor plate grade and geometry.</p> <p>2) Adequate corrosion protection of bar with clearance provided between 2.79" bar and 3" galvanized tube</p> <p>See the email attached from Dywidag stating that the provided clearance allows for adequate corrosion protection of the anchor bar. Please confirm the galvanized duct is acceptable as detailed based on this information.</p>					
	<p>Submittal Package TG0701-023.1 SSS - Light Column Anchor Bolts (#1.5E), please review and respond to the following items:</p> <p>1) Base plate hole oversize and subsequent sizing of the top plate washer</p> <p>As a follow up to the conference call held 1/29/14 for the Light Column anchor bolts, Dywidag provided the following information via the email attached: "If we spec a 50ksi plate and use the full width of the spherical washer, the plate works as-is. Leaving the plate design alone (with the assumed actual contact width of the washer) the plate still works at a 6" hole diameter. This is designing for 95% of the ultimate capacity of the bar."</p> <p>The maximum outside diameter of the HSS tube inside the base plate is 3.5". Within a 6" hole, this allows for 1.25" of clearance all around the tube, which is more than adequate clearance based on the AISC allowable tolerances for installations of anchor-rod groups. Reference AISC Steel Construction Manual, Section 16.7.5.1 attached, which does not allow for any more than ¼" variation between anchor rod groups nor between anchor rod group and column lines. Please confirm it is acceptable to proceed with a 6" base plate hole size to avoid modifications to the top anchor plate grade and geometry.</p> <p>2) Adequate corrosion protection of bar with clearance provided between 2.79" bar and 3" galvanized tube</p> <p>See the email attached from Dywidag stating that the provided clearance allows for adequate corrosion protection of the anchor bar. Please confirm the galvanized duct is acceptable as detailed based on this information.</p>					
T-1171	SSS - Galvanizing Steel Composite Deck	Closed	CR	02/13/2014	02/23/2014	02/24/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:			ANSWER:			







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<b>T-1176</b>	<b>SSS - Finish Requirements at BRBs</b>	<b>Closed</b>	<b>CR</b>	<b>02/19/2014</b>	<b>03/01/2014</b>	<b>02/27/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Detail B/A1-8662 indicates that BRBs at Ground Level between GL D.4/28 and D.4/31 are to receive IFRM-2 fire protection. All other BRBs are to receive SFRM fire protection. Please refer to SK RFI 397 SK1 attached and clarify the following finish requirements for the BRBs:</p> <p>1) Details 1, 3, and 5 on S1-4206 indicate that bottom gusset plates are to be galvanized. Please confirm that only the bottom gusset plates are to be galvanized.</p> <p>2) For BRBs to receive SFRM fire protection:</p> <p>a. Please confirm the extent of the SFRM at the braces as indicated by the red outline in details 2, 3, and 6. Note that all materials indicated to receive SFRM will be bare steel. Steel will be prepped in accordance with 07 81 00-3.2.B.</p> <p>b. Please confirm the pin, bolts, plates, and top gussets shown in details 2 and 6 are to be bare steel in anticipation of receiving SFRM by others.</p> <p>3) For BRBs to receive IFRM fire protection:</p> <p>a. Please confirm the extent of the IFRM at the braces as indicated by the blue outline in details 2, 3, and 6. Materials indicated to receive IFRM will be prepped and primed in accordance with 07 81 23.</p> <p>b. Detail B/A1-8662 graphically indicates that only the braces and not the gussets are to receive IFRM. Please confirm the final finish of the bottom gusset plates is to be galvanized and provide the finish requirements for the top gussets.</p> <p>c. Please provide the finish requirements for the pins and 1" thick plates indicated on 6/S1-4206.</p> <p>d. Provide the finish requirements for the bolts indicated in detail 6/S1-4206.</p>			<p>Detail B/A1-8662 indicates that BRBs at Ground Level between GL D.4/28 and D.4/31 are to receive IFRM-2 fire protection. All other BRBs are to receive SFRM fire protection. Please refer to SK RFI 397 SK1 attached and clarify the following finish requirements for the BRBs:</p> <p>1) Details 1, 3, and 5 on S1-4206 indicate that bottom gusset plates are to be galvanized. Please confirm that only the bottom gusset plates are to be galvanized.</p> <p>2) For BRBs to receive SFRM fire protection:</p> <p>a. Please confirm the extent of the SFRM at the braces as indicated by the red outline in details 2, 3, and 6. Note that all materials indicated to receive SFRM will be bare steel. Steel will be prepped in accordance with 07 81 00-3.2.B.</p> <p>b. Please confirm the pin, bolts, plates, and top gussets shown in details 2 and 6 are to be bare steel in anticipation of receiving SFRM by others.</p> <p>3) For BRBs to receive IFRM fire protection:</p> <p>a. Please confirm the extent of the IFRM at the braces as indicated by the blue outline in details 2, 3, and 6. Materials indicated to receive IFRM will be prepped and primed in accordance with 07 81 23.</p> <p>b. Detail B/A1-8662 graphically indicates that only the braces and not the gussets are to receive IFRM. Please confirm the final finish of the bottom gusset plates is to be galvanized and provide the finish requirements for the top gussets.</p> <p>c. Please provide the finish requirements for the pins and 1" thick plates indicated on 6/S1-4206.</p> <p>d. Provide the finish requirements for the bolts indicated in detail 6/S1-4206.</p>			



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<b>T-1176.1</b>	<b>SSS - Finish Requirements at BRBs</b>	<b>Closed</b>	<b>CR</b>	<b>03/18/2014</b>	<b>03/28/2014</b>	<b>03/18/2014</b>
<div><div><p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p><p><b>REQUEST:</b></p><p>Skanska is proceeding as follows for BRBs based on the response to WOJV T-1176:</p><p>1) Bottom gusset plates shall be galvanized.</p><p>2) For BRBs scheduled to receive SFRM fire protection:</p><p>    a. Materials in red indicated to receive SFRM will be bare steel. Steel will be prepped in accordance with 07 81 00-3.2.B.</p><p>    b. The pins, bolts, plates, and top gussets shown in details 2 and 6 shall be bare steel to receive SFRM.</p><p>3) For BRBs to receive IFRM fire protection:</p><p>    a. Prime coat for IFRM will be provided to the extent indicated by the blue outline in details 2, 3, and 6.</p><p>        Materials indicated to receive IFRM will be prepped and primed in accordance with 07 81 23.</p><p>    b. The response to WOJV T-1176 indicates finishes for top gusset plates shall be per the documents.</p><p>        Note that the contract documents and specifications do not indicate whether top gusset plates are to be galvanized, bare steel to receive SFRM, or IFRM. Based on the interface between gussets, bracing, and pinned components, it appears that the top gusset plate shall be prepped and primed to receive IFRM. Please confirm.</p><p>    c. The response to WOJV T-1176 indicates finishes for the pins, bolts, and 1" thick plates shall be per the documents. Note that the contract documents and specifications do not indicate whether these components are to be galvanized, bare steel to receive SFRM, or IFRM. Based on the interface between gussets, bracing, and pinned components, it appears that the pins, bolts, and 1" thick plates shall be prepped and primed to receive IFRM. Please confirm.</p><p>Please advise if exception is taken to any of the noted finishes.</p></div><div><p><b>ANSWER:</b></p><p>Skanska is proceeding as follows for BRBs based on the response to WOJV T-1176:</p><p>1) Bottom gusset plates shall be galvanized.</p><p>2) For BRBs scheduled to receive SFRM fire protection:</p><p>    a. Materials in red indicated to receive SFRM will be bare steel. Steel will be prepped in accordance with 07 81 00-3.2.B.</p><p>    b. The pins, bolts, plates, and top gussets shown in details 2 and 6 shall be bare steel to receive SFRM.</p><p>3) For BRBs to receive IFRM fire protection:</p><p>    a. Prime coat for IFRM will be provided to the extent indicated by the blue outline in details 2, 3, and 6.</p><p>        Materials indicated to receive IFRM will be prepped and primed in accordance with 07 81 23.</p><p>    b. The response to WOJV T-1176 indicates finishes for top gusset plates shall be per the documents.</p><p>        Note that the contract documents and specifications do not indicate whether top gusset plates are to be galvanized, bare steel to receive SFRM, or IFRM. Based on the interface between gussets, bracing, and pinned components, it appears that the top gusset plate shall be prepped and primed to receive IFRM. Please confirm.</p><p>    c. The response to WOJV T-1176 indicates finishes for the pins, bolts, and 1" thick plates shall be per the documents. Note that the contract documents and specifications do not indicate whether these components are to be galvanized, bare steel to receive SFRM, or IFRM. Based on the interface between gussets, bracing, and pinned components, it appears that the pins, bolts, and 1" thick plates shall be prepped and primed to receive IFRM. Please confirm.</p><p>Please advise if exception is taken to any of the noted finishes.</p></div></div>						
<b>T-1177</b>	<b>SSS - Erection Aids</b>	<b>Closed</b>	<b>CR</b>	<b>02/19/2014</b>	<b>03/01/2014</b>	<b>03/03/2014</b>









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<b>T-1180</b>	<b>SSS - Roof Level Support Framing to Drum Cafe</b>	<b>Closed</b>	<b>CR</b>	<b>02/19/2014</b>	<b>03/01/2014</b>	<b>02/28/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Framing plan S1-2605 in ASI 105 shows support framing at roof level for the W20 Drum Café. A note (see attached sketch) indicates that steel beams are to be "aligned with each drum café column See S1-6100." Please provide the referenced drawing.			Framing plan S1-2605 in ASI 105 shows support framing at roof level for the W20 Drum Café. A note (see attached sketch) indicates that steel beams are to be "aligned with each drum café column See S1-6100." Please provide the referenced drawing.			
<b>T-1181</b>	<b>BGP - Proposed Revised Location of the Reinforcement Lap Splices at the Lower ( Closed</b>		<b>01</b>	<b>02/21/2014</b>	<b>03/03/2014</b>	<b>03/04/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titché</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Further to discussion with Thornton Tomasetti design Engineer Kerem Gulec, WOJV is requesting that the horizontal reinforcement lap splices of the spandrel beams and the top horizontal reinforcement of the lower concourse slabs can both be located anywhere within the middle 1/3th span between the moment frame beams.			Further to discussion with Thornton Tomasetti design Engineer Kerem Gulec, WOJV is requesting that the horizontal reinforcement lap splices of the spandrel beams and the top horizontal reinforcement of the lower concourse slabs can both be located anywhere within the middle 1/3th span between the moment frame beams.			
Please confirm it would be acceptable			Please confirm it would be acceptable			
<b>T-1182</b>	<b>BGP - Mat Slab - Top Rebar Splice Location at Light Tower Anchor Bolt</b>	<b>Closed</b>	<b>01</b>	<b>02/21/2014</b>	<b>03/03/2014</b>	<b>03/28/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titché</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached sketches.			See attached sketches.			
To accomodate the installation of Light Column Anchor Bolt assemblies, SCCI proposes to locally move the splice location of the top mat slab rebar towards the West.			To accomodate the installation of Light Column Anchor Bolt assemblies, SCCI proposes to locally move the splice location of the top mat slab rebar towards the West.			
Please confirm that this is acceptable.			Please confirm that this is acceptable.			
<b>T-1183</b>	<b>SSS - Interference at GL15</b>	<b>Closed</b>	<b>CR</b>	<b>02/21/2014</b>	<b>03/03/2014</b>	<b>03/04/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
On the attached sketches CD RFI # 298 SK1 & SK2 the			On the attached sketches CD RFI # 298 SK1 & SK2			



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>double angle connections per S1-5010 are not possible at the noted location as the W16 &amp; W24 are off-set 2 7/16" from each other.</p> <p>Confirm it is acceptable to supply full depth shear plates as shown with plate thickness, welding and bolts per S1-5011</p>					
T-1184	SSS - Steel Connection Interference GL 15D	Closed	CR	02/21/2014	03/03/2014	03/04/2014
<p>From: Webcor Construction LP                      Stephanie Azzolino</p>						
REQUEST:			ANSWER:			
<p>At grid line D/15, the W40 to column connection will conflict with the W16 connection to the W40 (SK2). Please confirm it is acceptable to connect the W40 (SK1 &amp; SK2) to the indicated column as shown, using 3 rows of 4 bolts in lieu of the 2 rows of 6 bolts per 5/S1-4206.</p>			<p>At grid line D/15, the W40 to column connection will conflict with the W16 connection to the W40 (SK2). Please confirm it is acceptable to connect the W40 (SK1 &amp; SK2) to the indicated column as shown, using 3 rows of 4 bolts in lieu of the 2 rows of 6 bolts per 5/S1-4206.</p>			
T-1185	SSS - Shear Connection Bolt Layout at GL 19.1	Closed	CR	02/21/2014	03/03/2014	03/04/2014
<p>From: Webcor Construction LP                      Stephanie Azzolino</p>						
REQUEST:			ANSWER:			
<p>On the attached sketches CD RFI # 300 SK1 &amp; SK2 the 11 bolts per 2/S1-5011 will not fit in a single row in the beams.</p> <p>Confirm it is acceptable to locate the bolts as shown at 13 locations along Grid line 19.1.</p>			<p>On the attached sketches CD RFI # 300 SK1 &amp; SK2 the 11 bolts per 2/S1-5011 will not fit in a single row in the beams.</p> <p>Confirm it is acceptable to locate the bolts as shown at 13 locations along Grid line 19.1.</p>			





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1188</b>	<b>SSS - Finish Requirements at Basket Columns</b>	<b>Closed</b>	<b>CR</b>	<b>02/21/2014</b>	<b>03/03/2014</b>	<b>03/03/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>A1-8660 conceptually indicates the boundaries of Category 2 and Category 3 AESS requirements for the basket columns. Based on this information and the documents provided for the TG07.1R trade package, SK1 has been provided to depict Skanska's understanding of the basket column finish requirements. Please review the following and confirm the various finish boundaries associated with the basket columns:</p> <p>1) Confirm the Category 2 AESS boundary is from the Ground Level cast node to the end of the pipe column from the Ground Level to the Bus Deck Level as indicated in SK1.</p> <p>2) Confirm the Category 3 AESS begins at the Bus Deck level cast node and extends through the pipe column to the Roof Level cast node as indicated in SK1.</p> <p>3) Confirm the AESS boundary ends at the Roof Level cast nodes and that the Roof Level connection plates are to be delivered on site in the bare steel condition to receive SFRM.</p> <p>4) Confirm the AESS boundary at the Bus Deck cast nodes extends to the cast node pad, with all shear plates, reinforcement plates, and perimeter beams to receive SFRM as indicated in SK1.</p> <p>5) Confirm these boundaries can be typically applied to all perimeter basket columns at North, South, East, and West exterior elevations.</p>			<p>A1-8660 conceptually indicates the boundaries of Category 2 and Category 3 AESS requirements for the basket columns. Based on this information and the documents provided for the TG07.1R trade package, SK1 has been provided to depict Skanska's understanding of the basket column finish requirements. Please review the following and confirm the various finish boundaries associated with the basket columns:</p> <p>1) Confirm the Category 2 AESS boundary is from the Ground Level cast node to the end of the pipe column from the Ground Level to the Bus Deck Level as indicated in SK1.</p> <p>2) Confirm the Category 3 AESS begins at the Bus Deck level cast node and extends through the pipe column to the Roof Level cast node as indicated in SK1.</p> <p>3) Confirm the AESS boundary ends at the Roof Level cast nodes and that the Roof Level connection plates are to be delivered on site in the bare steel condition to receive SFRM.</p> <p>4) Confirm the AESS boundary at the Bus Deck cast nodes extends to the cast node pad, with all shear plates, reinforcement plates, and perimeter beams to receive SFRM as indicated in SK1.</p> <p>5) Confirm these boundaries can be typically applied to all perimeter basket columns at North, South, East, and West exterior elevations.</p>			
<b>T-1189</b>	<b>SSS - Missing Dimensions and Connection Details</b>	<b>Closed</b>	<b>CR</b>	<b>02/24/2014</b>	<b>03/06/2014</b>	<b>03/06/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>1) Work with SK2 and provide the missing dimension.</p> <p>2) Detail 10/S1-7600 will not work as the noted posts are off-set from the beam. Provide a connection detail.</p> <p>3) Supply the missing dimensions.</p>			<p>1) Work with SK2 and provide the missing dimension.</p> <p>2) Detail 10/S1-7600 will not work as the noted posts are off-set from the beam. Provide a connection detail.</p> <p>3) Supply the missing dimensions.</p>			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1190</b>	<b>16" Slab Negative Moments at North-South Walls</b>	<b>Closed</b>	<b>01</b>	<b>02/24/2014</b>	<b>03/06/2014</b>	<b>02/26/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>  Please reference attached Memo from seers consulting Engineer.  There is a certain concern that once the LL= 150 PSF is imposed on the LCL deck there will be excess negative moments generated at the interface between weak axis of the supported slab and wall-spandrel beam. This may require additional top reinforcement in the weak axis of the one-way slab.  Please confirm that there are no additional bars needed at the LCL spandrel/wall interface, and that loading Table from S-1002 applies.						<b>ANSWER:</b>  Please reference attached Memo from seers consulting Engineer.  There is a certain concern that once the LL= 150 PSF is imposed on the LCL deck there will be excess negative moments generated at the interface between weak axis of the supported slab and wall-spandrel beam. This may require additional top reinforcement in the weak axis of the one-way slab.  Please confirm that there are no additional bars needed at the LCL spandrel/wall interface, and that loading Table from S-1002 applies.
<b>T-1191</b>	<b>SSS - Elevator Pit Framing Steel</b>	<b>Closed</b>	<b>CR</b>	<b>02/24/2014</b>	<b>03/06/2014</b>	<b>03/04/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Please confirm it is acceptable to drill (4) holes in the W18 x50 to hold it in position for field welding. See attached sketches CD RFI # 311 SK1 & SK2 for clarification.						<b>ANSWER:</b>  Please confirm it is acceptable to drill (4) holes in the W18x50 to hold it in position for field welding. See attached sketches CD RFI # 311 SK1 & SK2 for clarification.
<b>T-1192</b>	<b>BSE - Steel Plate at CDSM Piles 450-451</b>	<b>Closed</b>	<b>01</b>	<b>02/24/2014</b>	<b>03/06/2014</b>	<b>03/04/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>  During level 4 and 5 excavation, a high volume leak occurred in zone 4 between CDSM pile 450 and 451. In an effort to stabilize the CDSM panel and repair the leak, BBII installed a steel road plate between soldier piles 167-168 and injected grout behind it.  BBII is concerned that removing the plate will likely cause the panel to become destabilized and could reopen the flow of water. BBII survey of the plate indicates that the plate is behind the theoretical face of CDSM wall and does not encroach into the permanent structure - reference the attached drawing.  Please confirm it is acceptable to leave this plate in place.						<b>ANSWER:</b>  During level 4 and 5 excavation, a high volume leak occurred in zone 4 between CDSM pile 450 and 451. In an effort to stabilize the CDSM panel and repair the leak, BBII installed a steel road plate between soldier piles 167-168 and injected grout behind it.  BBII is concerned that removing the plate will likely cause the panel to become destabilized and could reopen the flow of water. BBII survey of the plate indicates that the plate is behind the theoretical face of CDSM wall and does not encroach into the permanent structure - reference the attached drawing.  Please confirm it is acceptable to leave this plate in place.



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>The edges of the plate may be grinded to provide a smooth transition to the CDSM wall for waterproofing.</p>					<p>place. The edges of the plate may be grinded to provide a smooth transition to the CDSM wall for waterproofing.</p>
T-1193	<b>BGP - Gridline Offset Discrepancies</b>	Closed	01	02/24/2014	03/06/2014	03/03/2014
	<p><b>From:</b> Webcor Construction LP                      Claude Titche</p> <p><b>REQUEST:</b></p> <p>Reference called out dimensions on S1-2203 and S1-2204 for GL 12-13. SCCI believes that "42'-0" TO GRID 12" on S1-2204 is a typo and that dimensions shown on S1-2203 is accurate, which is consistent with the typical grid lines C-C.</p> <p>Please confirm.</p>					<p><b>ANSWER:</b></p> <p>Reference called out dimensions on S1-2203 and S1-2204 for GL 12-13. SCCI believes that "42'-0" TO GRID 12" on S1-2204 is a typo and that dimensions shown on S1-2203 is accurate, which is consistent with the typical grid lines C-C.</p> <p>Please confirm.</p>
T-1194	<b>BGP - Unmarked Members on S1-2203</b>	Closed	01	02/24/2014	03/06/2014	02/25/2014
	<p><b>From:</b> Webcor Construction LP                      Claude Titche</p> <p><b>REQUEST:</b></p> <p>Reference attached CD S1-2203</p> <p>It is unclear what the clouded member along the South foundation depicts. Is this member a wall of concourse beam?</p> <p>Please clarify, and provide dimensions, offsets, and type.</p>					<p><b>ANSWER:</b></p> <p>Reference attached CD S1-2203</p> <p>It is unclear what the clouded member along the South foundation depicts. Is this member a wall of concourse beam?</p> <p>Please clarify, and provide dimensions, offsets, and type.</p>



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1194.1</b>	<b>BGP - Unmarked Member on S1-2203</b>	<b>Closed</b>	<b>01</b>	<b>02/27/2014</b>	<b>03/09/2014</b>	<b>03/07/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  The response to RFI T-1194 conflicts with Submittal TG0600-364 response.  RFI T-1194 response noted that the curved member shown is future CMU wall. However, response to TG0600-354 submittal noted that the member is Curved Concrete Beams- CB24, CB55 and CB15.  Please clarify and provide latest drawings to reflect noted change shown in Submittal TG0600-354 if the curved member is a beam.						<b>ANSWER:</b>  The response to RFI T-1194 conflicts with Submittal TG0600-364 response.  RFI T-1194 response noted that the curved member shown is future CMU wall. However, response to TG0600-354 submittal noted that the member is Curved Concrete Beams- CB24, CB55 and CB15.  Please clarify and provide latest drawings to reflect noted change shown in Submittal TG0600-354 if the curved member is a beam.
<b>T-1194.2</b>	<b>BGP - Unmarked Member on S1-2203</b>	<b>Closed</b>	<b>01</b>	<b>03/10/2014</b>	<b>03/20/2014</b>	<b>03/14/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Refer to RFI's T-1194, T-1194.1, and ASI 106 S1-2203  Based on previous RFI T-1194-series responses SCCI still doesn't have contract drawing that depicts details of the curved members, deck beams along B65 (South spandrel beam). The most current version of S1-2203, that is available to SCCI does not specify beam type, nor radius of these curved members.  Please provide details for the curved beams along the South B65, i.e. issue most current version of S 1- 2200-series drawings with beam callouts and offsets (distances and radiuses).						<b>ANSWER:</b>  Refer to RFI's T-1194, T-1194.1, and ASI 106 S1-2203  Based on previous RFI T-1194-series responses SCCI still doesn't have contract drawing that depicts details of the curved members, deck beams along B65 (South spandrel beam). The most current version of S1-2203, that is available to SCCI does not specify beam type, nor radius of these curved members.  Please provide details for the curved beams along the South B65, i.e. issue most current version of S 1- 2200-series drawings with beam callouts and offsets (distances and radiuses).
<b>T-1195</b>	<b>BGP - Geothermal Header Pipe Size at Fields 13 and 14</b>	<b>Closed</b>	<b>01</b>	<b>02/25/2014</b>	<b>03/07/2014</b>	<b>03/07/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Reference attached drawing  Geothermal Fields 13, 14, and 15 have been condensed into two(2) fields (Geothermal Fields 13 and 14).						<b>ANSWER:</b>  Reference attached drawing  Geothermal Fields 13, 14, and 15 have been condensed into two(2) fields (Geothermal Fields 13 and 14).



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>Please provide the pipe sizes for the header piping in these two fields.</p>					<p>Please provide the pipe sizes for the header piping in these two fields.</p>
T-1197	SSS - Weld Access Hole for TG Stiffener Plate	Closed	CR	02/25/2014	03/07/2014	03/10/2014
	<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>A weld access hole is required where the direction of the weld changes from horizontal to vertical on detail 3/S1-4350. Please confirm the weld access hole detailed on SK2 is acceptable.</p>					<p><b>ANSWER:</b></p> <p>A weld access hole is required where the direction of the weld changes from horizontal to vertical on detail 3/S1-4350. Please confirm the weld access hole detailed on SK2 is acceptable.</p>
T-1197.1	SSS - Weld Access Hole for TG Shear Plate	Closed	CR	03/24/2014	04/03/2014	04/04/2014
	<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>On detail 3/S1-4350 due to the weld prep required for a 3" shear plate as indicated on SK1, the weld access hole previously requested in RFI T-1197 will not be adequate. Please confirm Option 1 or Option 2 on SK2 are acceptable or provide an alternative detail.</p> <p>Also please confirm the weld access hole dimensions may be adjusted proportionally for a 4" shear plate as detailed on 3/S1-4351 in accordance AWS weld access hole requirements.</p>					<p><b>ANSWER:</b></p> <p>On detail 3/S1-4350 due to the weld prep required for a 3" shear plate as indicated on SK1, the weld access hole previously requested in RFI T-1197 will not be adequate. Please confirm Option 1 or Option 2 on SK2 are acceptable or provide an alternative detail.</p> <p>Also please confirm the weld access hole dimensions may be adjusted proportionally for a 4" shear plate as detailed on 3/S1-4351 in accordance AWS weld access hole requirements.</p>
T-1198	SSS - Dimension Clarification for W-1 Fitted Stiffeners	Closed	CR	02/25/2014	03/07/2014	03/04/2014
	<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>Please provide the indicated dimension required to determine the depth of the 1 1/2 fitted stiffeners.</p>					<p><b>ANSWER:</b></p> <p>Please provide the indicated dimension required to determine the depth of the 1 1/2 fitted stiffeners.</p>





Webcor/Obayashi Joint Venture

PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

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Date: 02/11/2015  
Time: 07:01 AM  
Job: 30100

<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-1199</b>	<b>SSS - Bi-Fold Door Support Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>02/25/2014</b>	<b>03/07/2014</b>	<b>03/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  See attached sketches CD RFI # 306 SK1 to SK6 for items 1 to 5: 1) The noted dimension does not match the information in details 9 & 10/S1-5020 (SK2 & SK3), which show 1'-11 from Grid G. Please clarify which dimension is correct. 2) Confirm the L8x8 extends from W27 to W27 as shown in detail 10/S1-5050 (SK3). 3) Supply missing dimension. 4) Confirm the HSS8x8 with PL1x8 at Grid 14 terminates as shown or supply more information. 5) Confirm the HSS8x8 with PL1x8 at Grid 15 terminates as shown or supply more information.						<b>ANSWER:</b>  See attached sketches CD RFI # 306 SK1 to SK6 for items 1 to 5: 1) The noted dimension does not match the information in details 9 & 10/S1-5020 (SK2 & SK3), which show 1'-11 from Grid G. Please clarify which dimension is correct. 2) Confirm the L8x8 extends from W27 to W27 as shown in detail 10/S1-5050 (SK3). 3) Supply missing dimension. 4) Confirm the HSS8x8 with PL1x8 at Grid 14 terminates as shown or supply more information. 5) Confirm the HSS8x8 with PL1x8 at Grid 15 terminates as shown or supply more information.
<b>T-1200</b>	<b>SSS - Doubler Plate Detail Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>02/25/2014</b>	<b>03/07/2014</b>	<b>03/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  See attached sketches CD RFI # 307 SK1 & SK2 for items 1 to 4: 1) Confirm it is acceptable to cut the doubler plate as shown to clear the double angle connection. 2) Confirm it is acceptable to terminate the doubler plate 1/2" from the beam flange as shown as the beam flange is moment welded. 3) The shear plate connection for the W16 to the W33x221 fouls the bolts as shown. Confirm it is acceptable to increase the 1'-7 dimension to 1'-10 1/2. If not, supply an alternate detail. 4) Confirm the shear plate may be partially welded to the doubler plate and partially welded to the web of the W33x221.						<b>ANSWER:</b>  See attached sketches CD RFI # 307 SK1 & SK2 for items 1 to 4: 1) Confirm it is acceptable to cut the doubler plate as shown to clear the double angle connection. 2) Confirm it is acceptable to terminate the doubler plate 1/2" from the beam flange as shown as the beam flange is moment welded. 3) The shear plate connection for the W16 to the W33x221 fouls the bolts as shown. Confirm it is acceptable to increase the 1'-7 dimension to 1'-10 1/2. If not, supply an alternate detail. 4) Confirm the shear plate may be partially welded to the doubler plate and partially welded to the web of the W33x221.  W33x221.
<b>T-1200.1</b>	<b>SSS - Doubler Plate Detail Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>07/02/2014</b>	<b>07/12/2014</b>	<b>07/14/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 307.1 SK1: The noted W16x26 is not erectable unless the connection plate on the north end is pulled out as shown in SECTION						<b>ANSWER:</b>  See attached CD RFI # 307.1 SK1: The noted W16x26 is not erectable unless the connection plate on the north end is pulled out as





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1203</b>	<b>SSS - Stair Escalator Framing Detail</b>	<b>Closed</b>	<b>CR</b>	<b>02/26/2014</b>	<b>03/08/2014</b>	<b>03/11/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  On attached sketches CD RFI # 309 SK1 & SK2 confirm the connection for the W21x50 to the top of the W30x90 is acceptable as shown or supply a new detail.						<b>ANSWER:</b>  On attached sketches CD RFI # 309 SK1 & SK2 confirm the connection for the W21x50 to the top of the W30x90 is acceptable as shown or supply a new detail.
<b>T-1204</b>	<b>SSS - Support Steel at Large Slab Openings</b>	<b>Closed</b>	<b>CR</b>	<b>02/26/2014</b>	<b>03/08/2014</b>	<b>03/03/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  The perimeter support steel at large slab openings as detailed on 12/S1-5003 will not work at GL 16.9/D as indicated on SK1 due to the 12" top of slab elevation changes around these openings. Please provide details for the support steel required at this location including all connection details.						<b>ANSWER:</b>  The perimeter support steel at large slab openings as detailed on 12/S1-5003 will not work at GL 16.9/D as indicated on SK1 due to the 12" top of slab elevation changes around these openings. Please provide details for the support steel required at this location including all connection details.
<b>T-1205</b>	<b>BGP - Lower Concourse Blockouts to Pour Train Level Partition Walls</b>	<b>Closed</b>	<b>01</b>	<b>02/26/2014</b>	<b>03/08/2014</b>	<b>03/04/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  SCCI is planning on pouring as much of the Train Level Partition Walls in Area 3 and 4 prior to the Lower Concourse Deck being poured (see attached Structural Drawings for reference). However, some of the partition walls conflict with the Rakers used for bracing. Additionally, there are some partition walls in Areas 6-16 that are added in ASI #110 that is now included in TG06 scope.  Since the Rakers will not be removed until after the Lower Concourse Deck is poured, SCCI proposes installing blockouts in the Lower Concourse above the walls in order to complete the concrete pours for the partition walls in Areas 3 and 4. The same blockouts will be installed for the partition walls on the mat slab level in Areas 6-16, added as part of ASI 110. The blockouts will be 6"x 12" for RCS 1 reinforcement and 9"x 12" for RCS8 reinforcement, both at 4' O.C. The blockouts would be positioned in the space between bars as to not affect the reinforcement layout.						<b>ANSWER:</b>  SCCI is planning on pouring as much of the Train Level Partition Walls in Area 3 and 4 prior to the Lower Concourse Deck being poured (see attached Structural Drawings for reference). However, some of the partition walls conflict with the Rakers used for bracing. Additionally, there are some partition walls in Areas 6-16 that are added in ASI #110 that is now included in TG06 scope.  Since the Rakers will not be removed until after the Lower Concourse Deck is poured, SCCI proposes installing blockouts in the Lower Concourse above the walls in order to complete the concrete pours for the partition walls in Areas 3 and 4. The same blockouts will be installed for the partition walls on the mat slab level in Areas 6-16, added as part of ASI 110. The blockouts will be 6"x 12" for RCS 1 reinforcement and 9"x 12" for RCS8 reinforcement, both at 4' O.C. The blockouts would be positioned in the space between bars as to not affect the



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	Please confirm this is acceptable.				reinforcement layout.	
					Please confirm this is acceptable.	
T-1206	BGP - Geothermal Header Pipe Size at Field 12	Closed	01	02/27/2014	03/09/2014	03/07/2014
From: Webcor Construction LP Claude Titche						
REQUEST:					ANSWER:	
See attached drawing					See attached drawing	
The orientation and layout of Geothermal Field 12 has changed significantly. Please provide the pipe sizes for the header piping in this Field.					The orientation and layout of Geothermal Field 12 has changed significantly. Please provide the pipe sizes for the header piping in this Field.	



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1207</b>	<b>SSS - Missing Elevator Information</b>	<b>Closed</b>	<b>01</b>	<b>02/28/2014</b>	<b>02/28/2014</b>	<b>03/17/2014</b>
<div><div><b>From:</b> Webcor Construction LP  <b>REQUEST:</b> See attached CD RFI #314 SK1 to SK3 for items 1 to 8: 1) There appears to be a beam missing as there is no support for the slab edge plate per 8/S1-5000. Please advise. 2) The double angle connection per 1/S1-5010 will extend beyond the edge of slab located 5" from the center of beams. Confirm it is acceptable to connect the beams using the shear plates connection per 1/S1-5011 at the 7 locations noted. 3) Details 4-7/S1-5015 do not apply at the noted 4 locations. Please clarify which bracing detail is to be applied. 4) Details 4-7/S1-5015 do not apply at the noted 2 locations. Please clarify which bracing detail is to be applied. 5) Confirm the noted stiffener is a 3/8" plate per 1/A1-7600. 6) Confirm the 3/8 side plates are to extend the full length of the elevator slab opening. 7) Confirm the noted weld is acceptable. 8) The double angle connection per 1/S1-5010 will fold the shear plate to the column web. Confirm it is acceptable to connect the noted beam with a shear plate per 1/S1-5011</div><div><b>ANSWER:</b> See attached CD RFI #314 SK1 to SK3 for items 1 to 8: 1) There appears to be a beam missing as there is no support for the slab edge plate per 8/S1-5000. Please advise. 2) The double angle connection per 1/S1-5010 will extend beyond the edge of slab located 5" from the center of beams. Confirm it is acceptable to connect the beams using the shear plates connection per 1/S1-5011 at the 7 locations noted. 3) Details 4-7/S1-5015 do not apply at the noted 4 locations. Please clarify which bracing detail is to be applied. 4) Details 4-7/S1-5015 do not apply at the noted 2 locations. Please clarify which bracing detail is to be applied. 5) Confirm the noted stiffener is a 3/8" plate per 1/A1-7600. 6) Confirm the 3/8 side plates are to extend the full length of the elevator slab opening. 7) Confirm the noted weld is acceptable. 8) The double angle connection per 1/S1-5010 will fold the shear plate to the column web. Confirm it is acceptable to connect the noted beam with a shear plate per 1/S1-5011</div></div>						
<b>T-1208</b>	<b>SSS - Verify no Bent Plate Welds at Protected Zones</b>	<b>Closed</b>	<b>01</b>	<b>02/28/2014</b>	<b>03/10/2014</b>	<b>03/11/2014</b>
<div><div><b>From:</b> Webcor Construction LP  <b>REQUEST:</b> Per the approval comment on A2647 and RFI T-1071 occurs North of line D and South of line F along line 10.1 for loose bent plate conditions and does not apply at this location. This beam is between greids D and F. Please verify for the continuous shop attached bent plate condition no weld will occur within the noted protected zones. Note this is the same for A2659 and other similar conditions.</div><div><b>ANSWER:</b> Per the approval comment on A2647 and RFI T-1071 occurs North of line D and South of line F along line 10.1 for loose bent plate conditions and does not apply at this location. This beam is between greids D and F. Please verify for the continuous shop attached bent plate condition no weld will occur within the noted protected zones. Note this is the same for A2659 and other similar conditions.</div></div>						
<b>T-1209</b>	<b>BGP - Plumbing Sleeve Manufacturer</b>	<b>Closed</b>	<b>01</b>	<b>02/28/2014</b>	<b>03/10/2014</b>	<b>03/10/2014</b>



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<hr/>						
From: Webcor Construction LP                      Claude Titcher						
REQUEST:						ANSWER:
The response to SCCI Submittal, TG0600-044-BGP-Concourse Plumbing and Piping Sleeves-Product Data and Shop Drawings, states that sleeves and flanges shall be 18 gauge minimum per specs. From the three manufactures, per Spec Section 22 05 30 - 2.1 .B, RK Industries is the only manufacturer that provides plumbing sleeves but with flanges that are 26 gauge only.						The response to SCCI Submittal, TG0600-044-BGP-Concourse Plumbing and Piping Sleeves-Product Data and Shop Drawings, states that sleeves and flanges shall be 18 gauge minimum per specs. From the three manufactures, per Spec Section 22 05 30 - 2.1 .B, RK Industries is the only manufacturer that provides plumbing sleeves but with flanges that are 26 gauge only.
In order to proceed, please provide a manufacturer that fabricates sleeves according to TG06.0 specs or allow flanges that are 26 gauge to be used.						In order to proceed, please provide a manufacturer that fabricates sleeves according to TG06.0 specs or allow flanges that are 26 gauge to be used.
T-1211	BGP - Lower Concourse Blockout - Shifted Bars Near Piles	Closed	01	03/04/2014	03/14/2014	03/06/2014
<hr/>						
From: Webcor Construction LP                      Claude Titcher						
REQUEST:						ANSWER:
Where trestle piles protrude through the lower concourse deck, block-outs in the concrete slab will be installed and interrupted deck reinforcing will be spliced with formsavers. For bars near the extents of the trestle pile, please confirm if it is acceptable to shift the typical deck bars (#8 and #9) beyond the allowed placing tolerances to avoid interruption of the bar. If acceptable, please provide tolerances for shifting of the bar and minimum clear spacing between rebar similar to conditions listed in the response to RFI T-0631. See attached sketch for details.						Where trestle piles protrude through the lower concourse deck, block-outs in the concrete slab will be installed and interrupted deck reinforcing will be spliced with formsavers. For bars near the extents of the trestle pile, please confirm if it is acceptable to shift the typical deck bars (#8 and #9) beyond the allowed placing tolerances to avoid interruption of the bar. If acceptable, please provide tolerances for shifting of the bar and minimum clear spacing between rebar similar to conditions listed in the response to RFI T-0631. See attached sketch for details.
<hr/>						
T-1212	BGP - Goethermal Manifold Sleeve Supports	Closed	01	03/04/2014	03/14/2014	03/07/2014
<hr/>						
From: Webcor Construction LP                      Claude Titcher						
REQUEST:						ANSWER:
Per discussions with EOR(WSP), please confirm geothermal contractor is to install temporary supports (see attached sketch) to facilitate the installation of the geothermal pipe sleeves(36" long) at the face of finish concrete( which is not in the current concrete package-TG06).						Per discussions with EOR(WSP), please confirm geothermal contractor is to install temporary supports (see attached sketch) to facilitate the installation of the geothermal pipe sleeves(36" long) at the face of finish concrete( which is not in the current concrete package- TG06).







<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
	<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 13 as well as all levels of the encroachment into the foundation wall between CDSM piles 265 to 288 on the north elevation and 495 to 517 to on the south elevation for location Plan see exhibit - A</p> <p>Exhibit - B, &amp; C depict the location and degree in which the SP are encroaching</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) between CDSM pile 262-263 to 270, 272-273 to 276 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 266 to 270, 274 &amp; 275. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 280 to 281-282 &amp; 284 to 290, WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 281, 284, 285, 286, 288-290. This foundation wall area was originally a WR1 reinforcement area #11@8"oc EF vertically and would change to #11@6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on detail A/Sk.1 (Exhibit -D).</p> <p>Between CDSM piles 270 to 272-273 &amp; 281-282 to 283-284, WOJV is proposing to decrease the specified 36" wall thickness to 32" &amp; 33" respectively to clear the encroaching SP 271, 272 &amp; 282. This foundation wall area was originally a embedment column with reinforcement in this area was a double layer of #11@6"oc EF vertically and would change to double layer of #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.4 option 1 (Exhibit -F).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 503 to 506 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 504, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall</p>					
	<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 13 as well as all levels of the encroachment into the foundation wall between CDSM piles 265 to 288 on the north elevation and 495 to 517 to on the south elevation for location Plan see exhibit - A</p> <p>Exhibit - B, &amp; C depict the location and degree in which the SP are encroaching</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) between CDSM pile 262-263 to 270, 272-273 to 276 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 266 to 270, 274 &amp; 275. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 280 to 281-282 &amp; 284 to 290, WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 281, 284, 285, 286, 288-290. This foundation wall area was originally a WR1 reinforcement area #11@8"oc EF vertically and would change to #11@6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on detail A/Sk.1 (Exhibit -D).</p> <p>Between CDSM piles 270 to 272-273 &amp; 281-282 to 283-284, WOJV is proposing to decrease the specified 36" wall thickness to 32" &amp; 33" respectively to clear the encroaching SP 271, 272 &amp; 282. This foundation wall area was originally a embedment column with reinforcement in this area was a double layer of #11@6"oc EF vertically and would change to double layer of #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.4 option 1 (Exhibit -F).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 503 to 506 WOJV is</p>					





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	<p>thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings. See Exhibit - G &amp; H showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>					<p>proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 504, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings. See Exhibit - G &amp; H showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>
T-1216.1	<b>BGP - CDSM Soldier Pile Encroachment Area 13</b>	Closed	01	03/24/2014	03/31/2014	03/27/2014
<b>From:</b> Webcor Construction LP      Michael Spillane						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Documents: Exhibits A - H						Reference Documents: Exhibits A - H
<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 13 as well as all levels of the encroachment into the foundation wall between CDSM piles 265 to 288 on the north elevation and 495 to 517 to on the south elevation for location Plan see exhibit - A</p> <p>Exhibit - B, &amp; C depict the location and degree in which the SP are encroaching</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) between CDSM pile 262-263 to 270, 272-273 to 276 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 266 to270,274 &amp; 275. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall</p>						<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 13 as well as all levels of the encroachment into the foundation wall between CDSM piles 265 to 288 on the north elevation and 495 to 517 to on the south elevation for location Plan see exhibit - A</p> <p>Exhibit - B, &amp; C depict the location and degree in which the SP are encroaching</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) between CDSM pile 262-263 to 270, 272-273 to 276 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 266 to270,274 &amp; 275. Originally these were WR1 reinforcement areas #11@8"oc EF</p>



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	<p>thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 280 to 281-282 &amp; 284 to 290, WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 281,284,285,286,288-290. This foundation wall area was originally a WR1 reinforcement area #11@8"oc EF vertically and would change to #11@6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on detail A/Sk.1 (Exhibit -D).</p> <p>Between CDSM piles 270 to 272-273 &amp; 281-282 to 283-284, WOJV is proposing to decrease the specified 36" wall thickness to 34" &amp; 33" respectively to clear the encroaching SP 271,272 &amp; 282. This foundation wall area was originally a embedment column with reinforcement in this area was a double layer of #11@6"oc EF vertically and would change to double layer of #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.4 option 1 (Exhibit -F).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 503 to 506 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 504, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings. See Exhibit - G &amp; H showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>					<p>vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 280 to 281-282 &amp; 284 to 290, WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 281,284,285,286,288-290. This foundation wall area was originally a WR1 reinforcement area #11@8"oc EF vertically and would change to #11@6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on detail A/Sk.1 (Exhibit -D).</p> <p>Between CDSM piles 270 to 272-273 &amp; 281-282 to 283-284, WOJV is proposing to decrease the specified 36" wall thickness to 34" &amp; 33" respectively to clear the encroaching SP 271,272 &amp; 282. This foundation wall area was originally a embedment column with reinforcement in this area was a double layer of #11@6"oc EF vertically and would change to double layer of #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.4 option 1 (Exhibit -F).</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 503 to 506 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 504, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings. See Exhibit - G &amp; H showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p>



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Please confirm if these solutions would be acceptable.						
T-1217	BGP - CDSM Soldier Pile Encroachment Area 14	Closed	01	03/21/2014	03/31/2014	03/27/2014
From: Webcor Construction LP Michael Spillane						
REQUEST:			ANSWER:			
Reference Documents: Exhibits A - H This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north & south wall in mat slab pour Area 14 as well as all levels of the encroachment into the foundation wall between CDSM piles 288 to 318 on the north elevation and 465 to 495 to on the south elevation for location Plan see exhibit - A. Exhibit - B & C depict the location and degree in which the SP are encroaching.			Reference Documents: Exhibits A - H This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north & south wall in mat slab pour Area 14 as well as all levels of the encroachment into the foundation wall between CDSM piles 288 to 318 on the north elevation and 465 to 495 to on the south elevation for location Plan see exhibit - A. Exhibit - B & C depict the location and degree in which the SP are encroaching.			
WOJV proposal North elevation on gridline A: (See Exhibit - B) Between CDSM piles 284 to 290, WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 284,285,286,288-290. This foundation wall area was originally a WR1 reinforcement area #11@8"oc EF vertically and would change to #11@6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on detail A/Sk.1 (Exhibit -D).			WOJV proposal North elevation on gridline A: (See Exhibit - B) Between CDSM piles 284 to 290, WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 284,285,286,288-290. This foundation wall area was originally a WR1 reinforcement area #11@8"oc EF vertically and would change to #11@6"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on detail A/Sk.1 (Exhibit -D).			
Between CDSM pile 299 to 301,305 to 312 & 315-316 to 322-323 WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 299 & 301. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).			Between CDSM pile 299 to 301,305 to 312 & 315-316 to 322-323 WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 299 & 301. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).			
Between CDSM piles 290 to 294-295, WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 291,292 & 293, Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit			Between CDSM piles 290 to 294-295, WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 291,292 & 293, Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to			



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	<p>- E)</p> <p>Between CDSM piles 301 to 305, WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 301,302 &amp; 304. Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit - E)</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 473 to 475 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 474, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p> <p>See Exhibit - F G &amp; H showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>					<p>#11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit - E)</p> <p>Between CDSM piles 301 to 305, WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 301,302 &amp; 304. Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit - E)</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 473 to 475 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 474, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p> <p>See Exhibit - F G &amp; H showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>
<b>T-1218</b>	<b>BGP - CDSM Soldier Pile Encroachment Area 15</b>	<b>Closed</b>	<b>01</b>	<b>04/02/2014</b>	<b>04/12/2014</b>	<b>04/13/2014</b>
<b>From:</b> Webcor Construction LP Michael Spillane						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Documents: Exhibits A - G		Reference Documents: Exhibits A - G				
This RFI addresses the impact of the encroaching CDSM		This RFI addresses the impact of the encroaching				



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
	<p>soldier piles (SP) on the north &amp; south wall in mat slab pour Area 15 as well as all levels of the encroachment into the foundation wall between CDSM piles 318 to 343 on the north elevation and 440 to 465 on the south elevation for location Plan see exhibit - A. Exhibit - B &amp; C depict the location and degree in which the SP are encroaching.</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) Between CDSM pile 315-316 to 322-323, 326 to 330 &amp; 334 to 337-338. WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 316 to 322. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 322-323 to 326, 330 to 334 &amp; 337-338 to 341 WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 323 to 326 &amp; 330 to 335, 338 &amp; 339 Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit -E)</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 438 to 442 &amp; 445-446 to 448 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 441,446 &amp; 447, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p> <p>See Exhibit - F &amp; G showing details of transition between modified reinforcement to contract reinforcement. These solutions if approved would be incorporated into the TG06 shop drawings.</p>					<p>CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 15 as well as all levels of the encroachment into the foundation wall between CDSM piles 318 to 343 on the north elevation and 440 to 465 to on the south elevation for location Plan see exhibit - A. Exhibit - B &amp; C depict the location and degree in which the SP are encroaching.</p> <p>WOJV proposal North elevation on gridline A: (See Exhibit - B) Between CDSM pile 315-316 to 322-323, 326 to 330 &amp; 334 to 337-338. WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 316 to 322. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 322-323 to 326, 330 to 334 &amp; 337-338 to 341 WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 323 to 326 &amp; 330 to 335, 338 &amp; 339 Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit -E)</p> <p>WOJV proposal on the South elevation: (See Exhibit - B) Between CDSM piles 438 to 442 &amp; 445-446 to 448 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 441,446 &amp; 447, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p> <p>See Exhibit - F &amp; G showing details of transition between modified reinforcement to contract</p>



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<p>Please confirm if these solutions would be acceptable.</p> <p>reinforcement. These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>						
<b>T-1218.1</b>	<b>BGP - CDSM Soldier Pile Encroachment Area 15</b>	<b>Closed</b>	<b>01</b>	<b>05/02/2014</b>	<b>05/12/2014</b>	<b>05/08/2014</b>
<p><b>From:</b> Webcor Construction LP      Michael Spillane</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Documents: Exhibits A - G			Reference Documents: Exhibits A - G			
Due to revised surveying information received it become necessary to revise the area 15 wall encroachments fixes.			Due to revised surveying information received it become necessary to revise the area 15 wall encroachments fixes.			
This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north & south wall in mat slab pour Area 15 as well as all levels of the encroachment into the foundation wall between CDSM piles 318 to 343 on the north elevation and 440 to 465 to on the south elevation for location Plan see exhibit - A. Exhibit - B & C depict the location and degree in which the SP are encroaching.			This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north & south wall in mat slab pour Area 15 as well as all levels of the encroachment into the foundation wall between CDSM piles 318 to 343 on the north elevation and 440 to 465 to on the south elevation for location Plan see exhibit - A. Exhibit - B & C depict the location and degree in which the SP are encroaching.			
WOJV proposal North elevation on gridline A: (See Exhibit - B) Between CDSM pile 315-316 to 322-323, 326 to 330 & 334 to 337-338, 341 to 344. WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 316 to 322 & 343. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).			WOJV proposal North elevation on gridline A: (See Exhibit - B) Between CDSM pile 315-316 to 322-323, 326 to 330 & 334 to 337-338, 341 to 344. WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 316 to 322 & 343. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).			
Between CDSM piles 322-323 to 326, 330 to 334 & 337-338 to 341 WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 323 to 326 & 330 to 335, 338 & 339 Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit			Between CDSM piles 322-323 to 326, 330 to 334 & 337-338 to 341 WOJV is proposing to decrease the specified 36" wall thickness to 33.5" to clear the encroaching SP 323 to 326 & 330 to 335, 338 & 339 Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to			





Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
	<p>-E)</p> <p>WOJV proposal on the South elevation: (See Exhibit 4 B) Between CDSM piles 438 to 442 &amp; 445-446 to 448 WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 441,442,445,446 &amp; 447, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 441-442 to 445-446, WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 442 &amp; 445 Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit - E)</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p> <p>See Exhibit - F &amp; G showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>					<p>#11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit -E)</p> <p>WOJV proposal on the South elevation: (See Exhibit 4 B) Between CDSM piles 438 to 442 &amp; 445-446 to 448 WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 441,442,445,446 &amp; 447, originally this was a WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 441-442 to 445-446, WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 442 &amp; 445 Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit - E)</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p> <p>See Exhibit - F &amp; G showing details of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings.</p> <p>Please confirm if these solutions would be acceptable.</p>
T-1219	BGP - Spandrel Beam Modifications in Area 11	Closed	01	04/03/2014	04/13/2014	04/11/2014
From: Webcor Construction LP Michael Spillane						
REQUEST:		ANSWER:				
Further to response to RFI T-637 please find attached		Further to response to RFI T-637 please find attached				



Webcor/Obayashi Joint Venture

PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

<i>Number</i>	<i>Subject</i>	<i>Status</i>	<i>Choice Required</i>	<i>Date Created</i>	<i>Date Required</i>	<i>Date Answered</i>
	<p>proposed changes to the spandrel beams in pour Area 11 for location plan see exhibit - A Exhibit - B shows the plan view of the modification necessary to the spandrel beam on the north and south elevations due to the revised reinforcement width of the foundation wall due to encroachment of the CDSM beams as well as typical cross sections of the revised spandrel beams. RFI T - 0783 shows the extent of the modification to the foundation wall on the north and south elevations of Area 11.</p> <p>Please confirm that these modifications as outlined at these locations are acceptable.</p>					
T-1220.1	<p><b>BGP - SFPUC Grounding Details</b></p> <p><b>From:</b> Webcor Construction LP                      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>The response to RFI T-1220 stated that drawing E1-3212 had been issued in an ASI. This drawing has not been issued For Construction.</p> <p>Please provide sheet E1-3212</p>	Closed	01	03/24/2014	04/03/2014	03/24/2014
						<p><b>ANSWER:</b></p> <p>The response to RFI T-1220 stated that drawing E1-3212 had been issued in an ASI. This drawing has not been issued For Construction.</p> <p>Please provide sheet E1-3212</p>





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1221</b>	<b>BGP - 36" Pile Sleeve Joint</b>	<b>Closed</b>	<b>01</b>	<b>03/06/2014</b>	<b>03/16/2014</b>	<b>03/14/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div> <div><b>REQUEST:</b><p>Please reference attached sketch, photo, and Contract Drawings SI-3003 and SI-2025. SCCI welded the 36" pile sleeve (see S 1-2025) vertical CJP as depicted in the attached sketch due to a shop fabrication error (see attached photo). Per AWS DI .1, the pre-qualified joint designation B-U2a-GF for FCAW allows a 65 degree groove angle with detail and fit up tolerances. The welded joint is approximately 15 degrees out of tolerance and is on one side of the sleeve only (2 joints per sleeve). Due to the member considered as non-structural, SCCI requests this joint to be acceptable as welded at this location only. SCCI will adhere to Spec Section 05 50 10 - 2.5.C.2 - "Weld mat foundation sleeve components continuously and test their water tightness by filling with water. Monitor water level for 48 hours minimum. Dry and correct faulty welds and re-test until proven watertight." Is this acceptable?</p></div> <div><b>ANSWER:</b><p>Please reference attached sketch, photo, and Contract Drawings SI-3003 and SI-2025. SCCI welded the 36" pile sleeve (see S 1-2025) vertical CJP as depicted in the attached sketch due to a shop fabrication error (see attached photo). Per AWS DI .1, the pre-qualified joint designation B-U2a-GF for FCAW allows a 65 degree groove angle with detail and fit up tolerances. The welded joint is approximately 15 degrees out of tolerance and is on one side of the sleeve only (2 joints per sleeve). Due to the member considered as non-structural, SCCI requests this joint to be acceptable as welded at this location only. SCCI will adhere to Spec Section 05 50 10 - 2.5.C.2 - "Weld mat foundation sleeve components continuously and test their water tightness by filling with water. Monitor water level for 48 hours minimum. Dry and correct faulty welds and re-test until proven watertight." Is this acceptable?</p></div>						
<b>T-1222</b>	<b>SSS - Filler plate weld access at CP2 connection</b>	<b>Closed</b>	<b>CR</b>	<b>03/07/2014</b>	<b>03/17/2014</b>	<b>03/17/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div> <div><b>REQUEST:</b><p>Please reference Details 4 and 7 on sheet S1-8001.</p><p>Detail 7 specifies a 1" thick filler plate between the beam web and the 2 1/2" thick backing plate. Detail 4 Section E notes 1" typical from the edge of the fill plate to the adjoining stiffener or beam web. 1" opening does not allow for adequate access to perform the welding.</p><p>Oregon Iron Works(OIW) is requesting that all filler plates for CP2 connections have a minimum clearance of 1 1/2" from any adjoining edges. Additionally, OIW is requesting that this minimum clearance be applied to any similar areas that exhibit this limited access for welding.</p></div> <div><b>ANSWER:</b><p>Please reference Details 4 and 7 on sheet S1-8001.</p><p>Detail 7 specifies a 1" thick filler plate between the beam web and the 2 1/2" thick backing plate. Detail 4 Section E notes 1" typical from the edge of the fill plate to the adjoining stiffener or beam web. 1" opening does not allow for adequate access to perform the welding.</p><p>Oregon Iron Works(OIW) is requesting that all filler plates for CP2 connections have a minimum clearance of 1 1/2" from any adjoining edges. Additionally, OIW is requesting that this minimum clearance be applied to any similar areas that exhibit this limited access for welding.</p></div>						
<b>T-1223</b>	<b>SSS - Elevator Edge of Slab Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>03/07/2014</b>	<b>03/17/2014</b>	<b>03/21/2014</b>

Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
<p><b>From:</b> Webcor Construction LP</p> <p><b>REQUEST:</b></p> <p>At SE401 see attached CD RFI # 317 SK1 to SK5 for items 1 to 7:</p> <ol style="list-style-type: none"> <li>1) It is not clear what is meant by the noted slope designation.</li> <li>Confirm the roof slab slopes and the top of pad slab is EL 56'-11 per A1-2894 (SK3).</li> <li>2) Confirm the noted dimension is 1'-2 3/4 per 8/S1-5004 (SK4).</li> <li>3) This note is pointing to the edge of slab and giving a top of steel elevation. Work with SK5 and clarify.</li> <li>4) Confirm the noted dimensions should read 1'-9 &amp; 9'-2 per 1/S1-7113 &amp; RFI T-0965 (SK251, CD 200). If not, clarify.</li> <li>5) The noted HSS members are not shown on S1-2504 (SK1) or 3/S1-7113 (SK2) but this detail is referenced on both plans noted above.</li> <li>a) Please confirm the HSS members are required.</li> <li>b) If yes, supply the size.</li> <li>c) Supply the horizontal locations on plan</li> <li>d) Supply the elevation</li> <li>e) Supply connection details</li> <li>f) NOTE: the elevator post connections per 1/S1-7600 may foul the HSS members (work with SK5)</li> <li>6) Confirm the slab edge plate terminates below the 10" raised slab as shown.</li> <li>7) Confirm detail 1/S1-7600 with the full depth stiffener and kicker brace applies in the noted cases with no elevator post on top of the beam.</li> </ol>	<p>Stephanie Azzolino</p>					<p><b>ANSWER:</b></p> <p>At SE401 see attached CD RFI # 317 SK1 to SK5 for items 1 to 7:</p> <ol style="list-style-type: none"> <li>1) It is not clear what is meant by the noted slope designation.</li> <li>Confirm the roof slab slopes and the top of pad slab is EL 56'-11 per A1-2894 (SK3).</li> <li>2) Confirm the noted dimension is 1'-2 3/4 per 8/S1-5004 (SK4).</li> <li>3) This note is pointing to the edge of slab and giving a top of steel elevation. Work with SK5 and clarify.</li> <li>4) Confirm the noted dimensions should read 1'-9 &amp; 9'-2 per 1/S1-7113 &amp; RFI T-0965 (SK251, CD 200). If not, clarify.</li> <li>5) The noted HSS members are not shown on S1-2504 (SK1) or 3/S1-7113 (SK2) but this detail is referenced on both plans noted above.</li> <li>a) Please confirm the HSS members are required.</li> <li>b) If yes, supply the size.</li> <li>c) Supply the horizontal locations on plan</li> <li>d) Supply the elevation</li> <li>e) Supply connection details</li> <li>f) NOTE: the elevator post connections per 1/S1-7600 may foul the HSS members (work with SK5)</li> <li>6) Confirm the slab edge plate terminates below the 10" raised slab as shown.</li> <li>7) Confirm detail 1/S1-7600 with the full depth stiffener and kicker brace applies in the noted cases with no elevator post on top of the beam.</li> </ol>
<p><b>T-1223.1</b></p> <p><b>From:</b> Webcor Construction LP</p> <p><b>REQUEST:</b></p> <p>This is a follow-up RFI to RFI T-1223 (SK 426, CD 317) See attached CD RFI # 317.1 SK1 &amp; SK2 for items 1 &amp; 2:</p> <ol style="list-style-type: none"> <li>1) Confirm the beam locations from the edge of slab as shown are acceptable.</li> <li>2) The L5x5 angles will foul the shear plate for the W30 to the column connection if we use the double angle connection per 1/S1-5010 to connect the W16 to the W30. Confirm it is acceptable to connect the W16 to the W30 using a shear plate per 1/S1-5011.</li> </ol>	<p><b>SSS - SE401 Bus Deck Level Connection Clarification</b></p> <p>Stephanie Azzolino</p>	<p><b>Closed</b></p>	<p><b>CR</b></p>	<p><b>04/11/2014</b></p>	<p><b>04/21/2014</b></p>	<p><b>04/22/2014</b></p> <p><b>ANSWER:</b></p> <p>This is a follow-up RFI to RFI T-1223 (SK 426, CD 317) See attached CD RFI # 317.1 SK1 &amp; SK2 for items 1 &amp; 2:</p> <ol style="list-style-type: none"> <li>1) Confirm the beam locations from the edge of slab as shown are acceptable.</li> <li>2) The L5x5 angles will foul the shear plate for the W30 to the column connection if we use the double angle connection per 1/S1-5010 to connect the W16 to the W30. Confirm it is acceptable to connect the W16 to the W30 using a shear plate per 1/S1-5011.</li> </ol>



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-1224</b>	<b>BGP - Concourse Top Bars at Column Embedded Plates</b>	<b>Closed</b>	<b>01</b>	<b>03/10/2014</b>	<b>03/20/2014</b>	<b>03/18/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: SKS-0324 and SKS-0325 Per SKS-0324 and SKS-0325, where the top bars for the concourse slab intersect type 1 B and 1 C embedded base plates, the bars are to be trimmed and supplemented with a matching slab dowel in MF beam layer 1. The typical deck top reinforcing consists of a continuous run of #9 bars @ 12" and alternates with a #9 x 28'-0" long bars@ 12". See attached sketch for details. To avoid trimming a 28'-0" bar and splicing it with a 17'-0" bar, please confirm if it is acceptable to relocate the #9 x 28'-0" top bars@ 12" to the MF beam layer 1. The continuous #9 bars@ 12" will remain at 3/4" from the top of concrete and trimmed according to SKS-0324 and SKS-0325.			Reference: SKS-0324 and SKS-0325 Per SKS-0324 and SKS-0325, where the top bars for the concourse slab intersect type 1 B and 1 C embedded base plates, the bars are to be trimmed and supplemented with a matching slab dowel in MF beam layer 1. The typical deck top reinforcing consists of a continuous run of #9 bars @ 12" and alternates with a #9 x 28'-0" long bars@ 12". See attached sketch for details. To avoid trimming a 28'-0" bar and splicing it with a 17'-0" bar, please confirm if it is acceptable to relocate the #9 x 28'-0" top bars@ 12" to the MF beam layer 1. The continuous #9 bars@ 12" will remain at 3/4" from the top of concrete and trimmed according to SKS-0324 and SKS-0325.			







Webcor/Obayashi Joint Venture

PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

Page: 1363 of 2218  
Date: 02/11/2015  
Time: 07:01 AM  
Job: 30100

<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
T-1225.2	BSE - Level C Bracing Removal Sequence West Side of Zone 1	Closed	01	03/19/2014	03/29/2014	03/31/2014
From: Webcor Construction LP Michael Spillane						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See sketch SK -1 attached.		See sketch SK -1 attached.				
Sequence is as follows:		Sequence is as follows:				
1. install re-bracing struts RB-03,04 & 05 within this green clouded area (GL-06 to GL-7.5) area to East already completed.		1. install re-bracing struts RB-03,04 & 05 within this green clouded area (GL-06 to GL-7.5) area to East already completed.				
2. install re-bracing rackers RB-01 to 09 within this pink clouded area		2. install re-bracing rackers RB-01 to 09 within this pink clouded area				
3. Remove level C struts numbers 77-81 and walers from south west corner once the walls have reached required strength and RB re-bracing rackers are installed per Note 2		3. Remove level C struts numbers 77-81 and walers from south west corner once the walls have reached required strength and RB re-bracing rackers are installed per Note 2				
4. install re-bracing rackers RB-10 to 20 within this red clouded area		4. install re-bracing rackers RB-10 to 20 within this red clouded area				
5. Remove level C struts STC-01 to 12, 75 & 76 and corresponding walers from the walls once the RB re-bracing is completely installed within this area and the walls have reached the required design strength, the sequence for de-stressing the struts should be all diagonals completed prior to the de-stressing of the 3 cross lot struts (10,11 & 12)		5. Remove level C struts STC-01 to 12, 75 & 76 and corresponding walers from the walls once the RB re-bracing is completely installed within this area and the walls have reached the required design strength, the sequence for de-stressing the struts should be all diagonals completed prior to the de-stressing of the 3 cross lot struts (10,11 & 12)				
Please confirm if this sequence would be acceptable for Level C bracing removal.		Please confirm if this sequence would be acceptable for Level C bracing removal.				
BSE - Level C Bracing Removal Sequence West Side of Zone		BSE - Level C Bracing Removal Sequence West Side of Zone				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1226</b>	<b>SSS - Gap between bus deck perimeter beam and cast node</b>	<b>Closed</b>	<b>CR</b>	<b>03/10/2014</b>	<b>03/20/2014</b>	<b>03/11/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> See attached CD RFI # 334 SK1: Per detail 1A/S1-5017 confirm it is acceptable to maintain the existing cut on the beam considering the additional 1/4" machining at the cast nodes. The actual gap is now 1" plus 1/4" for machining.						<b>ANSWER:</b> See attached CD RFI # 334 SK1: Per detail 1A/S1-5017 confirm it is acceptable to maintain the existing cut on the beam considering the additional 1/4" machining at the cast nodes. The actual gap is now 1" plus 1/4" for machining.
<b>T-1227</b>	<b>BGP - SFPUC Transformer Pad Grounding</b>	<b>Closed</b>	<b>01</b>	<b>03/10/2014</b>	<b>03/20/2014</b>	<b>03/24/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b> Reference sheets E1-2202, E1-2203, E1-2205 and transformer rooms B1223, B1323, B1562.  Each room requires embedded steel plates for which the future transformers are to be welded to. Are all of the embedded plates tied to the same building ground system as shown for the vault room itself? If so, does SFPUC have a specific requirement for plate grounding?						<b>ANSWER:</b> Reference sheets E1-2202, E1-2203, E1-2205 and transformer rooms B1223, B1323, B1562.  Each room requires embedded steel plates for which the future transformers are to be welded to. Are all of the embedded plates tied to the same building ground system as shown for the vault room itself? If so, does SFPUC have a specific requirement for plate grounding?
<b>T-1227.1</b>	<b>BGP - SFPUC Plate Grounding</b>	<b>Closed</b>	<b>01</b>	<b>04/21/2014</b>	<b>05/01/2014</b>	<b>05/02/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b> See attached Sketch corresponding to sheet E1-2203  Does the attached sketch clarify the intent of requiring grounding for each SFPUC Transformer mounting plate? If so, is the intent to extend the same grounding conductor type/size from the already contracted embedded conductor? If not, please provide requirement assembly details.  Additionally, will SFPUC require any testing prior to conductor embedment at the concourse level?						<b>ANSWER:</b> See attached Sketch corresponding to sheet E1-2203  Does the attached sketch clarify the intent of requiring grounding for each SFPUC Transformer mounting plate? If so, is the intent to extend the same grounding conductor type/size from the already contracted embedded conductor? If not, please provide requirement assembly details.  Additionally, will SFPUC require any testing prior to conductor embedment at the concourse level?
<b>T-1228</b>	<b>BGP - Geothermal Riser Location for Field 12</b>	<b>Closed</b>	<b>01</b>	<b>03/12/2014</b>	<b>03/22/2014</b>	
<b>From:</b> Webcor Construction LP      Claude Titché						

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<b>REQUEST:</b> Please confirm the riser for geothermal field 12 is to be located between soldier piles 316 and 317 (approximately GL 30.4)					<b>ANSWER:</b> Please confirm the riser for geothermal field 12 is to be located between soldier piles 316 and 317 (approximately GL 30.4)
<b>T-1228.1</b>	<b>BGP - Geothermal Riser Location for Field 12</b>	<b>Closed</b>	<b>01</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/26/2014</b>
	<b>From:</b> Webcor Construction LP                      Claude Titché					
	<b>REQUEST:</b> Per field conversation with ARUP, SCCI was directed to locate the riser for Geothermal Field 12 between Soldier Piles 317 and 318 because of a leak in the CDSM wall between piles 316 and 317.  Please confirm this location is acceptable.					<b>ANSWER:</b> Per field conversation with ARUP, SCCI was directed to locate the riser for Geothermal Field 12 between Soldier Piles 317 and 318 because of a leak in the CDSM wall between piles 316 and 317.  Please confirm this location is acceptable.
<b>T-1229</b>	<b>BGP - As-Built Location of Sump Pit Near GL 14/G</b>	<b>Closed</b>	<b>01</b>	<b>03/12/2014</b>	<b>03/22/2014</b>	<b>03/14/2014</b>
	<b>From:</b> Webcor Construction LP                      Claude Titché					
	<b>REQUEST:</b> Please reference attached sketch that show as-built location of sump pit near GL 14/G.  Please confirm this as-built location is acceptable.					<b>ANSWER:</b> Please reference attached sketch that show as-built location of sump pit near GL 14/G.  Please confirm this as-built location is acceptable.
<b>T-1230</b>	<b>SSS - Machined Cap Plate Surface Finish</b>	<b>Closed</b>	<b>CR</b>	<b>03/12/2014</b>	<b>03/22/2014</b>	<b>03/14/2014</b>
	<b>From:</b> Webcor Construction LP                      Stephanie Azzolino					
	<b>REQUEST:</b> Please confirm it is acceptable to provide the machined surface of the train box column cap plates with a prime coat finish in lieu of the rust inhibiting coating. The surface prep will be SSPC-SP6 commercial blast cleaning with International Interzinc 315B primer (attached).					<b>ANSWER:</b> Please confirm it is acceptable to provide the machined surface of the train box column cap plates with a prime coat finish in lieu of the rust inhibiting coating. The surface prep will be SSPC-SP6 commercial blast cleaning with International Interzinc 315B primer (attached).



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-1230.1	SSS - Machined Cap Plate Surface Finish	Closed	CR	03/24/2014	04/03/2014	04/04/2014
<div> <div> <b>From:</b> Webcor Construction LP Gregory Kemerer </div> <div> <b>REQUEST:</b> <p>To clarify, the specifications section 05 10 00 - 3.2.P.2 requires finished bearing surfaces to be protected with a rust-inhibiting coating (which is typically a petroleum/grease or wax based product) which is to be removed immediately prior to erection. The Train Box Column Cap Plate is a finished bearing surface.</p> <p>The Interzinc 315 product referenced in the response to RFI T-1230 is specific to the IFRM Coating System. Skanska/TMF proposed an organic zinc rich primer (Interzinc 315B) as the rust inhibiting coating, which would not need to be removed prior to placement of the TG atop the cap plate. The proposed product meets or exceeds the primer specified in the 05 10 00 - 2.2.A spec section as noted in the attached manufacturer's letter.</p> <p>Please confirm this is acceptable.</p> </div> <div> <b>ANSWER:</b> <p>To clarify, the specifications section 05 10 00 - 3.2.P.2 requires finished bearing surfaces to be protected with a rust-inhibiting coating (which is typically a petroleum/grease or wax based product) which is to be removed immediately prior to erection. The Train Box Column Cap Plate is a finished bearing surface.</p> <p>The Interzinc 315 product referenced in the response to RFI T-1230 is specific to the IFRM Coating System. Skanska/TMF proposed an organic zinc rich primer (Interzinc 315B) as the rust inhibiting coating, which would not need to be removed prior to placement of the TG atop the cap plate. The proposed product meets or exceeds the primer specified in the 05 10 00 - 2.2.A spec section as noted in the attached manufacturer's letter.</p> <p>Please confirm this is acceptable.</p> </div> </div>						
T-1231	BGP - Zone 2 Lower Concourse Openings	Closed	01	03/13/2014	03/23/2014	03/21/2014
<div> <div> <b>From:</b> Webcor Construction LP Claude Titche </div> <div> <b>REQUEST:</b> <p>There are multiple discrepancies between Zone 2 Lower Concourse Drawings, recently issued in ASI #112 and ASI #113. See attached drawings. The discrepancies have been highlighted in yellow.</p> <p>AI-2842 (Lower Concourse Slab Edge Plan) generally does not show openings at these highlighted areas, while AI-2222 (Lower Concourse Wall Plan) do show openings. Please clarify whether the highlighted areas are openings or plumbing penetration. If they are plumbing penetrations, please provide sizes and sleeve sizes</p> </div> <div> <b>ANSWER:</b> <p>There are multiple discrepancies between Zone 2 Lower Concourse Drawings, recently issued in ASI #112 and ASI #113. See attached drawings. The discrepancies have been highlighted in yellow.</p> <p>AI-2842 (Lower Concourse Slab Edge Plan) generally does not show openings at these highlighted areas, while AI-2222 (Lower Concourse Wall Plan) do show openings. Please clarify whether the highlighted areas are openings or plumbing penetration. If they are plumbing penetrations, please provide sizes and sleeve sizes</p> </div> </div>						
T-1232	BGP - CDSM Wall Movement in Area 3 and Area 1 - West Wall	Closed	01	03/13/2014	03/23/2014	03/24/2014
<div> <div> <b>From:</b> Webcor Construction LP Claude Titche </div> </div>						



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<b>REQUEST:</b>  SCCI performed a survey of the CDSM Soldier Piles approximately March 2013 for the North of Area 3 and West of Area 3 and Area 1. SCCI re-surveyed the soldier piles again this year (March 2014) and have found that the Soldier Piles in the area have displaced into the structure by approximately 1.5" at both the North Wall and the West wall. Please find attached survey information and the changes noticed in the past year.  1. Please confirm that the additional encroachment and future potential movement of wall into the structure will not cause conflict with current and future rebar installation.  2. Please confirm and demonstrate that the future potential movement will not cause additional stress to the Wall Lift 1, which will be poured in the next week.  3. Please confirm that future potential CDSM movement in this area will not impact any of SCCI's future permanent work.					
	<b>ANSWER:</b>  SCCI performed a survey of the CDSM Soldier Piles approximately March 2013 for the North of Area 3 and West of Area 3 and Area 1. SCCI re-surveyed the soldier piles again this year (March 2014) and have found that the Soldier Piles in the area have displaced into the structure by approximately 1.5" at both the North Wall and the West wall. Please find attached survey information and the changes noticed in the past year.  1. Please confirm that the additional encroachment and future potential movement of wall into the structure will not cause conflict with current and future rebar installation.  2. Please confirm and demonstrate that the future potential movement will not cause additional stress to the Wall Lift 1, which will be poured in the next week.  3. Please confirm that future potential CDSM movement in this area will not impact any of SCCI's future permanent work.					
<b>T-1234</b>	<b>SSS - Continuity Plate Thickness</b>  <b>From:</b> Webcor Construction LP Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>03/14/2014</b>	<b>03/24/2014</b>	<b>03/31/2014</b>
	<b>REQUEST:</b>  1) Confirm it is acceptable to connect the W30x90 to the column with the bolts located as shown and the web extended as shown. 2) Confirm it is acceptable to connect the W12x40 to the W30x90 using a shear plate per 1/S1-5011. 3) At 19.9, gridlines C & G, the top flange of a BU-56x30x1.5x4 and sloping W40x327 weld to the continuity plate at the bus deck level. In order to provide a CJP between the aforementioned members and continuity plate, as well as keeping the moment frame members in line with the continuity plate, the continuity plate will need to be increased by 5/8". Please confirm this plate thickness increase to 4 5/8" is required or provide an alternate detail. 4) Please confirm that the solution provided for 3) can be used at other locations where the continuity plate thickness needs to be increased.					
	<b>ANSWER:</b>  1) Confirm it is acceptable to connect the W30x90 to the column with the bolts located as shown and the web extended as shown. 2) Confirm it is acceptable to connect the W12x40 to the W30x90 using a shear plate per 1/S1-5011. 3) At 19.9, gridlines C & G, the top flange of a BU-56x30x1.5x4 and sloping W40x327 weld to the continuity plate at the bus deck level. In order to provide a CJP between the aforementioned members and continuity plate, as well as keeping the moment frame members in line with the continuity plate, the continuity plate will need to be increased by 5/8". Please confirm this plate thickness increase to 4 5/8" is required or provide an alternate detail. 4) Please confirm that the solution provided for 3) can be used at other locations where the continuity plate thickness needs to be increased.					



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<b>T-1235</b>	<b>SSS - Thermally Cut Holes in Transfer Girders</b>	<b>Closed</b>	<b>CR</b>	<b>03/14/2014</b>	<b>03/24/2014</b>	<b>03/19/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> Please confirm it is acceptable to thermally cut the rebar holes and the column post-tensioned rod holes in the Transfer Girders using an automated (oxy-fuel) process. All other holes in the primary member will be drilled or punched.						<b>ANSWER:</b> Please confirm it is acceptable to thermally cut the rebar holes and the column post-tensioned rod holes in the Transfer Girders using an automated (oxy-fuel) process. All other holes in the primary member will be drilled or punched.
<b>T-1236</b>	<b>SSS - Slab Support Details at GL19E Opening</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/19/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> See attached CD RFI # 327 SK1:  The deck support detail per 4/S1-7660 & 10/S1-5002 will not work at the noted location.  Please supply a detail for this deck support.						<b>ANSWER:</b> See attached CD RFI # 327 SK1:  The deck support detail per 4/S1-7660 & 10/S1-5002 will not work at the noted location.  Please supply a detail for this deck support.
<b>T-1237</b>	<b>SSS - Base Plate Interference at 19.9G</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/21/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> See attached CD RFI # 328 SK1: The column base plate fouls girder TR19.9 at grid line G as shown. Please provide a solution.						<b>ANSWER:</b> See attached CD RFI # 328 SK1: The column base plate fouls girder TR19.9 at grid line G as shown. Please provide a solution.
<b>T-1238</b>	<b>SSS - Shear Plate Interference w Pretensioned Rod</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/21/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> See attached CD RFI # 335 SK1:  The pretensioned rod per detail 2/S1-5052 fouls the beam connection as shown. This occurs on S1-2305 at Grids 19.9/C,G; 20.1/C,G; 22/C,G & 24/C,G.  Please provide a solution.						<b>ANSWER:</b> See attached CD RFI # 335 SK1:  The pretensioned rod per detail 2/S1-5052 fouls the beam connection as shown. This occurs on S1-2305 at Grids 19.9/C,G; 20.1/C,G; 22/C,G & 24/C,G.  Please provide a solution.



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<b>T-1239</b>	<b>SSS -Deck Support Details at Protected Zones</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/31/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 339 SK1 for items 1 & 2: 1) Provide a typical detail showing how the decking for the lower slab is to be supported at the "Protected Zone". 2) Supply a typical detail for deck support across the column flange if required.			See attached CD RFI # 339 SK1 for items 1 & 2: 1) Provide a typical detail showing how the decking for the lower slab is to be supported at the "Protected Zone". 2) Supply a typical detail for deck support across the column flange if required.			
<b>T-1239.1</b>	<b>SSS - Deck Support Details at Protected Zones</b>	<b>Closed</b>	<b>CR</b>	<b>04/02/2014</b>	<b>04/12/2014</b>	<b>04/16/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
This is a follow-up to the response to RFI T-1239 (SK-454, CD 339): Confirm the Engineer is referring to drawing S1-5001 in his response to question 1 & 2 or provide clarification. 1) Detail 1/S1-5000 is Typical Headed Stud Spacing Detail not Typical Metal Deck Detail 2) Per the latest S1-5000 Rev5 (issued as ASI 114) there is no detail 3/S1-5000.			This is a follow-up to the response to RFI T-1239 (SK-454, CD 339): Confirm the Engineer is referring to drawing S1-5001 in his response to question 1 & 2 or provide clarification. 1) Detail 1/S1-5000 is Typical Headed Stud Spacing Detail not Typical Metal Deck Detail 2) Per the latest S1-5000 Rev5 (issued as ASI 114) there is no detail 3/S1-5000.			
<b>T-1239.2</b>	<b>SSS - Deck Support Details at Protected Zones</b>	<b>Closed</b>	<b>CR</b>	<b>04/21/2014</b>	<b>05/01/2014</b>	<b>05/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI T1239 response. The L3x3x12ga proposed to support the deck will have an approximate span of 8'. This is a concern for 12 gauge material to carry 7-1/2" of nominal weight concrete over an 8' span. 1) We propose to span the gauge angle from the L4x4 where it terminates on the girder web at the protected zone, which will shorten the span of the angle by approximately 3'. See SK1 for clarification. Please confirm this is acceptable. 2) A closure piece will need to be added and tack welded to the girder web. This was previously closed by the			Please reference RFI T1239 response. The L3x3x12ga proposed to support the deck will have an approximate span of 8'. This is a concern for 12 gauge material to carry 7-1/2" of nominal weight concrete over an 8' span. 1) We propose to span the gauge angle from the L4x4 where it terminates on the girder web at the protected zone, which will shorten the span of the angle by approximately 3'. See SK1 for clarification. Please confirm this is acceptable. 2) A closure piece will need to be added and tack welded to the girder web. This was previously closed by the			



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	removed L4x4. Please confirm this is acceptable. 3) Confirm 1 & 2 are acceptable at similar conditions.					removed L4x4. Please confirm this is acceptable. 3) Confirm 1 & 2 are acceptable at similar conditions.
<b>T-1240</b>	<b>SSS - Bus Deck Level Fouled Beam Connections at GL18</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/18/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI # 329 SK1 & SK2:  The connection for the W36 beams will foul the connection for the W21 to the W40.  Please provide a solution.						<b>ANSWER:</b>  See attached CD RFI # 329 SK1 & SK2:  The connection for the W36 beams will foul the connection for the W21 to the W40.  Please provide a solution.
<b>T-1241</b>	<b>SSS - Ground Level Perimeter Framing Clarification at GL 19.1</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/31/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI # 330 SK1:  The connection for the raised W14 beam per detail 1/S1-5028 with S<12" will not work at the noted locations and will foul the W40 beam connection into the Transfer girder.  Please provide an alternate connection detail.						<b>ANSWER:</b>  See attached CD RFI # 330 SK1:  The connection for the raised W14 beam per detail 1/S1-5028 with S<12" will not work at the noted locations and will foul the W40 beam connection into the Transfer girder.  Please provide an alternate connection detail.
<b>T-1242</b>	<b>BGP - Geothermal Manifold Location for Fields 12, 13, and 14</b>	<b>Closed</b>	<b>01</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/26/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Please provide exact required elevations for the geothermal riser manifold stub outs for Fields 12, 13 and 14. Please include the exact elevation for the temperature probe in Geo Field 14.  Note this information is HOT due to the recovery schedule in Fields 12, 13 and 14.						<b>ANSWER:</b>  Please provide exact required elevations for the geothermal riser manifold stub outs for Fields 12, 13 and 14. Please include the exact elevation for the temperature probe in Geo Field 14.  Note this information is HOT due to the recovery schedule in Fields 12, 13 and 14.



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<b>T-1243</b>	<b>SSS - Missing beam connection to TPG3 at GL 18, D &amp; F</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/19/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI # 332 SK1 & SK2:  Confirm it is acceptable to connect the W14x22 to the 2" stiffener plate/4" flange of the TPG3 as shown. If not, supply an alternate connection detail.		<b>ANSWER:</b>  See attached CD RFI # 332 SK1 & SK2:  Confirm it is acceptable to connect the W14x22 to the 2" stiffener plate/4" flange of the TPG3 as shown. If not, supply an alternate connection detail.				
<b>T-1244</b>	<b>SSS - Bus Deck Level Perimeter Framing Clarification at GL 16.9</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/18/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI # 340 SK1: Please confirm the beam flanges can remain as shown and a 1:2.5 tapered cut is not required.		<b>ANSWER:</b>  See attached CD RFI # 340 SK1: Please confirm the beam flanges can remain as shown and a 1:2.5 tapered cut is not required.				
<b>T-1245</b>	<b>SSS - Deviation - Cast Node Type 3 WC0003 - Dimensional Results</b>	<b>Void</b>	<b>01</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  See attached dimensional report and numbered print.  Bradken has found dimensional deviations due to tooling on Cast Node Type 3 PN WC0003.  Please confirm attached dimensions are acceptable. Please provide disposition on out-of-tolerance conditions.		<b>ANSWER:</b>  See attached dimensional report and numbered print.  Bradken has found dimensional deviations due to tooling on Cast Node Type 3 PN WC0003.  Please confirm attached dimensions are acceptable. Please provide disposition on out-of-tolerance conditions.				
<b>T-1246</b>	<b>SSS - Deviation - Cast Node Type LC 202 WC0055 - Dimensional Results</b>	<b>Void</b>	<b>01</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  See attached dimensional report and numbered print.  Bradken has found dimensional deviations due to tooling on Cast Node Type LC 202 PN WC0055.  Please confirm attached dimensions are acceptable. Please provide disposition on out-of-tolerance conditions.		<b>ANSWER:</b>  See attached dimensional report and numbered print.  Bradken has found dimensional deviations due to tooling on Cast Node Type LC 202 PN WC0055.  Please confirm attached dimensions are acceptable. Please provide disposition on out-of-tolerance conditions.				



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<b>T-1247</b>	<b>SSS - Deviation - Cast Node Type LC 203 WC0056 - Dimensional Results</b>	<b>Void</b>	<b>01</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  See attached dimensional report and numbered print.  Bradken has found dimensional deviations due to tooling on Cast Node Type LC 203 PN WC0056.  Please confirm attached dimensions are acceptable. Please provide disposition on out of tolerance conditions.						<b>ANSWER:</b>  See attached dimensional report and numbered print.  Bradken has found dimensional deviations due to tooling on Cast Node Type LC 203 PN WC0056.  Please confirm attached dimensions are acceptable. Please provide disposition on out of tolerance conditions.
<b>T-1248</b>	<b>SSS - Deviation Request Cast Node Type LC 204 WC0057 - Dimensional Results</b>	<b>Void</b>	<b>01</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  Please see attached dimensional report and numbered print.  Bradken has found dimensional deviations due to tooling on Cast Node Type LC 204 PN WC0057.  Please confirm attached dimensions are acceptable. Please provide disposition on out-of-tolerance conditions.						<b>ANSWER:</b>  Please see attached dimensional report and numbered print.  Bradken has found dimensional deviations due to tooling on Cast Node Type LC 204 PN WC0057.  Please confirm attached dimensions are acceptable. Please provide disposition on out-of-tolerance conditions.
<b>T-1249</b>	<b>SSS - Second Level HSS Connection Clarification at GL8</b>	<b>Closed</b>	<b>CR</b>	<b>03/17/2014</b>	<b>03/27/2014</b>	<b>03/31/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI # 341 SK1 & SK2: Confirm the closure plates and welding as shown is acceptable at the coped HSS elevator beam.						<b>ANSWER:</b>  See attached CD RFI # 341 SK1 & SK2: Confirm the closure plates and welding as shown is acceptable at the coped HSS elevator beam.
<b>T-1250</b>	<b>BGP - Lower Concourse Beam Discrepancies</b>	<b>Closed</b>	<b>01</b>	<b>03/18/2014</b>	<b>03/28/2014</b>	<b>03/28/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  Reference attached contract drawings.  Latest CDs S1-2203, S1-2210, and S1-2211 have added curved CB16 and CB&B7 downturned beams which						<b>ANSWER:</b>  Reference attached contract drawings.  Latest CDs S1-2203, S1-2210, and S1-2211 have added curved CB16 and CB&B7 downturned beams





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T-1252	<p>appear to be concerntric to South spandrel beam B65. However, no dimensions have been provided, i.e. radius, offset etc. Furthermore, CD s1-2211 shows tat CB16 and CB&amp;B7 do not follow the same radius. There appears to be an offset between the CB16 and CB/B7 at the MFB at GL 6, and the offset dimensions are not shown. If architectural drawings are used to find the missing radius (reference SKA-3017 and SKA-3026), one can find that the future partition walls at the South corridor have R=647" - 7 1/2" on both East and West side of MFB at GL6. This implies that the partition walls in this area would off center (non-concentric), and the South corridor would taper (or narrow) Down towards East. SCCI. believes that this was not designer's intent.</p> <p>Please provide accurate and consistent dimensions and offsets for the LCC beams and future partition walls.</p> <p><b>SSS - Deviation Due to Tooling - Cast Node Type LC 301 WC0058 - Dimensional Re Void</b></p> <p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p> <p><b>REQUEST:</b></p> <p>See attached Dimensional report and numbered print.</p> <p>Bradken has found dimensional deviations due to tooling on Cast Node Type LC 301 PN</p> <p>WC0058. Please confirm attached dimensions are acceptable.</p> <p>Please provide disposition on out of tolerance conditions</p>					
	<p>which appear to be concerntric to South spandrel beam B65. However, no dimensions have been provided, i.e. radius, offset etc. Furthermore, CD s1-2211 shows tat CB16 and CB&amp;B7 do not follow the same radius. There appears to be an offset between the CB16 and CB/B7 at the MFB at GL 6, and the offset dimensions are not shown. If architectural drawings are used to find the missing radius (reference SKA-3017 and SKA-3026), one can find that the future partition walls at the South corridor have R=647" - 7 1/2" on both East and West side of MFB at GL6. This implies that the partition walls in this area would off center (non-concentric), and the South corridor would taper (or narrow) Down towards East. SCCI. believes that this was not designer's intent.</p> <p>Please provide accurate and consistent dimensions and offsets for the LCC beams and future partition walls.</p>	CR		03/18/2014	03/28/2014	
	<p><b>ANSWER:</b></p> <p>See attached Dimensional report and numbered print.</p> <p>Bradken has found dimensional deviations due to tooling on Cast Node Type LC 301 PN</p> <p>WC0058. Please confirm attached dimensions are acceptable.</p> <p>Please provide disposition on out of tolerance conditions</p>					







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<b>T-1256</b>	<b>SSS - Deviation Due to Tooling - Cast Node Type LC 101 WC0050 - Dimensional R Void</b>			<b>03/18/2014</b>	<b>03/28/2014</b>	
	<b>From:</b> Webcor Construction LP      Stephanie Azzolino					
<b>T-1257</b>	<b>SSS - Deviation Due to Tooling - Cast Node Type LC 103 WC0052 - Dimensional R Void</b>			<b>03/18/2014</b>	<b>03/28/2014</b>	
	<b>From:</b> Webcor Construction LP      Stephanie Azzolino					

**REQUEST:**

See attached Dimensional report and numbered print.

Bradken has found dimensional deviations due to tooling on Cast Node Type 15 PN

WC0016. Please confirm attached dimensions are acceptable.

Please provide disposition on out of tolerance conditions.

**ANSWER:**

See attached Dimensional report and numbered print.

Bradken has found dimensional deviations due to tooling on Cast Node Type 15 PN

WC0016. Please confirm attached dimensions are acceptable.

Please provide disposition on out of tolerance conditions.

**REQUEST:**

See attached Dimensional report and numbered print.

Bradken has found dimensional deviations due to tooling on Cast Node Type LC 101 PN WC0050 (2 castings - LC 101-1 & LC 101-2).

Please confirm attached dimensions are acceptable.

**ANSWER:**

See attached Dimensional report and numbered print.

Bradken has found dimensional deviations due to tooling on Cast Node Type LC 101 PN WC0050 (2 castings - LC 101-1 & LC 101-2).

Please confirm attached dimensions are acceptable.

**REQUEST:**

See attached Dimensional report and numbered print.

Bradken has found dimensional deviations due to tooling on Cast Node Type LC 103 PN WC0052 (2 castings LC103-1 & LC103-2).

Please confirm attached dimensions are acceptable.

**ANSWER:**

See attached Dimensional report and numbered print.

Bradken has found dimensional deviations due to tooling on Cast Node Type LC 103 PN WC0052 (2 castings LC103-1 & LC103-2).

Please confirm attached dimensions are acceptable.





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<b>T-1261</b>	<b>SSS - Deviation Due to Tooling - Cast Node Type 5 WC0005 - Dimensional Results</b>	<b>Void</b>	<b>CR</b>	<b>03/18/2014</b>	<b>03/28/2014</b>	
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached Dimensional report and numbered print.			See attached Dimensional report and numbered print.			
Bradken has found dimensional deviations due to tooling on Cast Node Type 5 PN WC0005 (6 castings 5-1 thru 5-6).			Bradken has found dimensional deviations due to tooling on Cast Node Type 5 PN WC0005 (6 castings 5-1 thru 5-6).			
Please confirm attached dimensions are acceptable.			Please confirm attached dimensions are acceptable.			
<b>T-1262</b>	<b>SSS - Deviation Due to Tooling - Cast Node Type 71 WC0048 - Dimensional Results</b>	<b>Void</b>	<b>CR</b>	<b>03/18/2014</b>	<b>03/28/2014</b>	
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached Dimensional report and numbered print.			See attached Dimensional report and numbered print.			
Bradken has found dimensional deviations due to tooling on Cast Node Type 71 PN WC004 (67 Castings starting with 71-02).			Bradken has found dimensional deviations due to tooling on Cast Node Type 71 PN WC004 (67 Castings starting with 71-02).			
Please confirm attached dimensions are acceptable.			Please confirm attached dimensions are acceptable.			
<b>T-1263</b>	<b>SSS - Deviation Due to Tooling - Cast Node Type 72 WC0049 - Dimensional Results</b>	<b>Void</b>	<b>CR</b>	<b>03/18/2014</b>	<b>03/28/2014</b>	
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached Dimensional report and numbered print.			See attached Dimensional report and numbered print.			
Bradken has found dimensional deviations due to tooling on Cast Node Type 72 PN WC0049 (67 Castings starting with 72-02).			Bradken has found dimensional deviations due to tooling on Cast Node Type 72 PN WC0049 (67 Castings starting with 72-02).			
Please confirm attached dimensions are acceptable.			Please confirm attached dimensions are acceptable.			
<b>T-1264</b>	<b>SSS - Bus Deck Level Missing Post Sizes at GL 15 &amp; 16</b>	<b>Closed</b>	<b>CR</b>	<b>03/18/2014</b>	<b>03/28/2014</b>	<b>03/19/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 342 SK1: Confirm the (12) posts are the same sizes as the ones shown on the Second Level in detail 3/S1-7009.			See attached CD RFI # 342 SK1: Confirm the (12) posts are the same sizes as the ones shown on the Second Level in detail 3/S1-7009.			



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<b>T-1265</b>	<b>SSS - Approval Comment Clarification on CS2 Submitted Drawing</b>	<b>Closed</b>	<b>CR</b>	<b>03/19/2014</b>	<b>03/29/2014</b>	<b>03/31/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 343 SK1: The approval comment on A4714 (CS2) is contrary to the information issued in RFI T-1111, item #7 (SK 327, CD 254) Confirm the response to RFI T-1111, item #7 is valid and the approval mark-up may be ignored.						<b>ANSWER:</b> See attached CD RFI # 343 SK1: The approval comment on A4714 (CS2) is contrary to the information issued in RFI T-1111, item #7 (SK 327, CD 254) Confirm the response to RFI T-1111, item #7 is valid and the approval mark-up may be ignored.
<b>T-1266</b>	<b>SSS - ST304 Framing Details</b>	<b>Closed</b>	<b>CR</b>	<b>03/20/2014</b>	<b>03/30/2014</b>	<b>03/31/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 364 SK1 to SK3 for items 1 to 4:  1) Confirm the (4) W10x22 beams are located on the center of the supporting posts. If not, supply the location dimensions. 2) Supply the clouded dimensions (4 total). 3a) Confirm the stair stringers do not connect to the W12x14. 3b) If yes, supply a connection detail as 1 or 3/S1-7601 will not work with the 5" offset dimension. 3c) If yes, please supply east/west location dimensions for the stair stringers. 4) Confirm it is acceptable to prep the flanges of the WT5x15 for the 1/4" PJP weld in lieu of the beam flange as shown.						<b>ANSWER:</b> See attached CD RFI # 364 SK1 to SK3 for items 1 to 4:  1) Confirm the (4) W10x22 beams are located on the center of the supporting posts. If not, supply the location dimensions. 2) Supply the clouded dimensions (4 total). 3a) Confirm the stair stringers do not connect to the W12x14. 3b) If yes, supply a connection detail as 1 or 3/S1-7601 will not work with the 5" offset dimension. 3c) If yes, please supply east/west location dimensions for the stair stringers. 4) Confirm it is acceptable to prep the flanges of the WT5x15 for the 1/4" PJP weld in lieu of the beam flange as shown.
<b>T-1266.1</b>	<b>SSS - ST304 Framing Details</b>	<b>Closed</b>	<b>CR</b>	<b>04/04/2014</b>	<b>04/14/2014</b>	<b>04/16/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Reference RFI T-1266 (attached) and the associated SK1 & SK2 for the following:  1) The (4) W10x22 beams are in Skanska's scope. Only the stringers are design built and provided by the stair supplier. Please confirm these beams are located on the center of the supporting posts or supply the location dimensions. 2) The (6) W10x22 beams in question are also in						<b>ANSWER:</b> Reference RFI T-1266 (attached) and the associated SK1 & SK2 for the following:  1) The (4) W10x22 beams are in Skanska's scope. Only the stringers are design built and provided by the stair supplier. Please confirm these beams are located on the center of the supporting posts or supply the location dimensions. 2) The (6) W10x22 beams in question are also in



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T-1266.2	SSS - ST304 Framing Details	Closed	CR	04/30/2014	05/10/2014	05/06/2014
	From: Webcor Construction LP Stephanie Azzolino					
T-1267	SSS - Weld Detail at Escalator Support	Closed	CR	03/20/2014	03/30/2014	04/01/2014
	From: Webcor Construction LP Gregory Kemerer					
T-1268	SSS - Escalator framing details at gridline 11	Closed	CR	03/21/2014	03/31/2014	03/31/2014







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<b>T-1269.1</b>	<b>SSS - GL 15 Dimension Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>04/09/2014</b>	<b>04/19/2014</b>	<b>04/21/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> This is a follow-up RFI to RFI T-1269 (SK 461, CD 346) See attached CD RFI # 346.1 SK1 & SK2: It is not possible to supply 3 equally spaced bottom flange braces per 8/S1-5015 as requested in the above noted RFI item 3 due to the existing framing. Confirm it is acceptable to supply a brace per 4/S1-5015 at each W27x84 and 1 brace per 8/S1-5015 as shown.						<b>ANSWER:</b> This is a follow-up RFI to RFI T-1269 (SK 461, CD 346) See attached CD RFI # 346.1 SK1 & SK2: It is not possible to supply 3 equally spaced bottom flange braces per 8/S1-5015 as requested in the above noted RFI item 3 due to the existing framing. Confirm it is acceptable to supply a brace per 4/S1-5015 at each W27x84 and 1 brace per 8/S1-5015 as shown.
<b>T-1270</b>	<b>SSS - ST401 Geometry Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>03/21/2014</b>	<b>03/31/2014</b>	<b>04/01/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 347 SK1 to SK3 for items 1 & 2: 1) Shown are two versions of the slope geometry for ST401 with the variations noted. Please clarify which geometry is to be used. 2) Confirm a bent deck support plate per 9/S1-5012 will not be required on top of the W12x14 with the beams aligned as shown.						<b>ANSWER:</b> See attached CD RFI # 347 SK1 to SK3 for items 1 & 2: 1) Shown are two versions of the slope geometry for ST401 with the variations noted. Please clarify which geometry is to be used. 2) Confirm a bent deck support plate per 9/S1-5012 will not be required on top of the W12x14 with the beams aligned as shown.
<b>T-1271</b>	<b>SSS - ST401 Dimension and Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>03/21/2014</b>	<b>03/31/2014</b>	<b>04/08/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 348 SK1 to SK3 for items 1 to 8: 1) Supply location of sloping W16 beams from face of concrete wall. 2) Confirm location of W44x335's is acceptable as shown or supply the location. 3) Confirm HSS8x8x5/8 post locations are acceptable or supply the locations. 4) Clarify/supply edge plate requirements for the 'S4' slab at EL. 74'-10 as well as the sloping 'S4' slab. 5) Confirm dimension is acceptable or supply dimension. 6) Confirm dimensions per A1-2904. 7) The connection for the W40 to the HSS8x8 per 6/S1-7661 will not work at the corner. Supply an alternate connection.						<b>ANSWER:</b> See attached CD RFI # 348 SK1 to SK3 for items 1 to 8: 1) Supply location of sloping W16 beams from face of concrete wall. 2) Confirm location of W44x335's is acceptable as shown or supply the location. 3) Confirm HSS8x8x5/8 post locations are acceptable or supply the locations. 4) Clarify/supply edge plate requirements for the 'S4' slab at EL. 74'-10 as well as the sloping 'S4' slab. 5) Confirm dimension is acceptable or supply dimension. 6) Confirm dimensions per A1-2904. 7) The connection for the W40 to the HSS8x8 per





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	8) It appears this detail will not work with the elevations noted. Please supply an alternate detail.					6/S1-7661 will not work at the corner. Supply an alternate connection. 8) It appears this detail will not work with the elevations noted. Please supply an alternate detail.
T-1271.1	SSS - ST401 Dimension and Connection Clarifications	Closed	CR	05/13/2014	05/23/2014	05/27/2014
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 348.1 SK1 for items 1 & 2:			See attached CD RFI # 348.1 SK1 for items 1 & 2:			
1) The connection per 8/S1-5012 fouls the connection for the W27 as shown. Please supply a solution.			1) The connection per 8/S1-5012 fouls the connection for the W27 as shown. Please supply a solution.			
2) Verify shim extension and weld.			2) Verify shim extension and weld.			
T-1272	SSS - GL 7 Framing Clarification	Closed	CR	03/21/2014	03/31/2014	04/07/2014
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 356 SK1 & SK2: Due to the limited space between the top flange of the TR7 and the PL 4", it is not possible to supply the connection for the W40x183 per 1/S1-5011. Confirm it is acceptable to connect the W40 to the TR7 as shown. (All not shown is per 1/S1-5011).			See attached CD RFI # 356 SK1 & SK2: Due to the limited space between the top flange of the TR7 and the PL 4", it is not possible to supply the connection for the W40x183 per 1/S1-5011. Confirm it is acceptable to connect the W40 to the TR7 as shown. (All not shown is per 1/S1-5011).			



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<b>T-1273</b>	<b>SSS - Bus Deck Level Kicker Brace Material</b>	<b>Closed</b>	<b>CR</b>	<b>03/21/2014</b>	<b>03/31/2014</b>	<b>04/08/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 362 SK1 & SK2 for items 1 & 2: 1) Per S1-0007 (SK2), ASTM A709 applies only to connection material on the Bus Deck Level and braces are not considered connection material. Confirm ASTM A36 is acceptable.  2) Notes: a) The same approval comment occurs on drawings 9233, 9317, 9368 & 9378. b) The approval comment identified above was not included in Sequence CS1. Confirm the response to item 1 applies to all braces on the Bus Deck Level.						<b>ANSWER:</b> See attached CD RFI # 362 SK1 & SK2 for items 1 & 2: 1) Per S1-0007 (SK2), ASTM A709 applies only to connection material on the Bus Deck Level and braces are not considered connection material. Confirm ASTM A36 is acceptable.  2) Notes: a) The same approval comment occurs on drawings 9233, 9317, 9368 & 9378. b) The approval comment identified above was not included in Sequence CS1. Confirm the response to item 1 applies to all braces on the Bus Deck Level.
<b>T-1274</b>	<b>SSS - A4786 Edge Distance</b>	<b>Closed</b>	<b>CR</b>	<b>03/21/2014</b>	<b>03/31/2014</b>	<b>04/04/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 361 SK1: On A4786 (CS2) the connection angles have been located at the top to clear the 'k' of the W40X324 supporting beam. The depth of the bottom cope matches the 'k' of the W40x199. Therefore it is not possible to achieve the requested 1 3/4" edge distance. Confirm the 1 5/16" edge distance is acceptable as it exceeds the 1 1/8" minimum at a gas cut edge.						<b>ANSWER:</b> See attached CD RFI # 361 SK1: On A4786 (CS2) the connection angles have been located at the top to clear the 'k' of the W40X324 supporting beam. The depth of the bottom cope matches the 'k' of the W40x199. Therefore it is not possible to achieve the requested 1 3/4" edge distance. Confirm the 1 5/16" edge distance is acceptable as it exceeds the 1 1/8" minimum at a gas cut edge.
<b>T-1275</b>	<b>SSS - PE 403404 Dimension Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>03/21/2014</b>	<b>03/31/2014</b>	<b>04/01/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 358 SK1: The noted dimensions do not match. Please advise.						<b>ANSWER:</b> See attached CD RFI # 358 SK1: The noted dimensions do not match. Please advise.



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<b>T-1276</b>	<b>BGP - Lower Concourse Future Wall and Column Conflicts</b>	<b>Closed</b>	<b>01</b>	<b>03/21/2014</b>	<b>03/31/2014</b>	<b>04/01/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  The blockouts for the column steel and baseplates appear to be in conflict with concourse walls at a number of locations, see attached mark-up. Please confirm no formsavers are to be installed at the column locations. In addition, please provide details for the walls at these column locations.						<b>ANSWER:</b>  The blockouts for the column steel and baseplates appear to be in conflict with concourse walls at a number of locations, see attached mark-up. Please confirm no formsavers are to be installed at the column locations. In addition, please provide details for the walls at these column locations.
<b>T-1277</b>	<b>BGP - Lower Concourse Discrepancies</b>	<b>Closed</b>	<b>01</b>	<b>03/24/2014</b>	<b>04/03/2014</b>	<b>04/02/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  See attached CD's  There is a dimensional discrepancy between current structural and architectural drawings, at the concourse opening around GL C13.  Opening called out on A1-2844 (ASI 113) is larger than clear space between deck beams B9 and B10.  Please Clarify						<b>ANSWER:</b>  See attached CD's  There is a dimensional discrepancy between current structural and architectural drawings, at the concourse opening around GL C13.  Opening called out on A1-2844 (ASI 113) is larger than clear space between deck beams B9 and B10.  Please Clarify
<b>T-1278</b>	<b>SSS - W40 Moment Connection at Roof Park Level</b>	<b>Closed</b>	<b>CR</b>	<b>03/24/2014</b>	<b>04/03/2014</b>	<b>04/07/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 363 SK1: Detail 8/S1-5032 applies at the noted locations but a full depth shear plate cannot be supplied due to the moment connection on the W40x392's as shown. Confirm it is acceptable to stop the shear plate as shown or supply an alternate detail.						<b>ANSWER:</b>  See attached CD RFI # 363 SK1: Detail 8/S1-5032 applies at the noted locations but a full depth shear plate cannot be supplied due to the moment connection on the W40x392's as shown. Confirm it is acceptable to stop the shear plate as shown or supply an alternate detail.
<b>T-1279</b>	<b>SSS - PE403, PE404 Dimension Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>03/24/2014</b>	<b>04/03/2014</b>	<b>04/07/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 365 SK1 & SK2:						<b>ANSWER:</b>  See attached CD RFI # 365 SK1 & SK2:



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	Supply the edge of slab locations as shown.					
T-1280	<b>SSS - ST403 Missing Dimensions and Connections</b>	<b>Closed</b>	<b>CR</b>	<b>03/24/2014</b>	<b>04/03/2014</b>	<b>04/16/2014</b>
	<b>From:</b> Webcor Construction LP Gregory Kemerer					
	<b>REQUEST:</b> See attached CD RFI # 366 SK1 to SK5 for items 1 to 6:  1) Supply the missing clouded dimensions at (5) locations shown.  2) The connection for the W10x22 to the HSS post per 3/S1-5012 will interfere with the W10x22 to W10x22 beam connection as well as the WT5x15 per 6/S1-7601 at (6) locations noted. This is a typical condition on all landings in ST403. Please provide a solution.  3) Depending on the noted missing dimension, the beam to HSS6x6 post connection per 3/S1-5012 may not work. Please supply an alternate connection as necessary.  4) Supply the missing clouded dimensions at (5) locations shown.  5) Supply the missing clouded dimensions at (5) locations shown.  6) Supply the missing clouded dimensions at (5) locations shown.					
	<b>ANSWER:</b> See attached CD RFI # 366 SK1 to SK5 for items 1 to 6:  1) Supply the missing clouded dimensions at (5) locations shown.  2) The connection for the W10x22 to the HSS post per 3/S1-5012 will interfere with the W10x22 to W10x22 beam connection as well as the WT5x15 per 6/S1-7601 at (6) locations noted. This is a typical condition on all landings in ST403. Please provide a solution.  3) Depending on the noted missing dimension, the beam to HSS6x6 post connection per 3/S1-5012 may not work. Please supply an alternate connection as necessary.  4) Supply the missing clouded dimensions at (5) locations shown.  5) Supply the missing clouded dimensions at (5) locations shown.  6) Supply the missing clouded dimensions at (5) locations shown.					



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<b>T-1280.1</b>	<b>SSS - ST403 Missing Dimensions and Connections</b>	<b>Closed</b>	<b>CR</b>	<b>06/30/2014</b>	<b>07/10/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1) The WT5x15 interferes with the connection for the W10x22 to the HSS12x6x5/8 post. Please supply a solution.			1) The WT5x15 interferes with the connection for the W10x22 to the HSS12x6x5/8 post. Please supply a solution.			
2) Confirm the connection for the W10x22 to the HSS6x6 post is acceptable as shown or supply a new detail.			2) Confirm the connection for the W10x22 to the HSS6x6 post is acceptable as shown or supply a new detail.			
<b>T-1281</b>	<b>SSS - W8, W12 Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>03/24/2014</b>	<b>04/03/2014</b>	<b>04/16/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1) Confirm the spacing is correct as shown for the W-8 anchorage per 1/S1-8008 at Grids D.4 & E.6. If not, supply the spacing.			1) Confirm the spacing is correct as shown for the W-8 anchorage per 1/S1-8008 at Grids D.4 & E.6. If not, supply the spacing.			
2a) Detail 1/S1-8006 is cut thru grid 16.9 but no spacing is provided for the W-8 anchorage. Please clarify and provide the spacing.			2a) Detail 1/S1-8006 is cut thru grid 16.9 but no spacing is provided for the W-8 anchorage. Please clarify and provide the spacing.			
2b) Detail 1/S1-8006 is cut thru grid 16.9. Detail 1/S1-8006 shows the elevation at the top of the curb wall as EL. 86'-8 but A1-2904 shows the top of wall at EL. 85'-7. Please clarify.			2b) Detail 1/S1-8006 is cut thru grid 16.9. Detail 1/S1-8006 shows the elevation at the top of the curb wall as EL. 86'-8 but A1-2904 shows the top of wall at EL. 85'-7. Please clarify.			
3) Supply the dimensions to locate the HSS10x10x1/2 posts for the W-12 anchorage per details 1 & 3/S1-8016.			3) Supply the dimensions to locate the HSS10x10x1/2 posts for the W-12 anchorage per details 1 & 3/S1-8016.			
4) Confirm all the hi-lited locations summarize the W-12 anchorage steel per details 1, 3 & 4/S1-8016.			4) Confirm all the hi-lited locations summarize the W-12 anchorage steel per details 1, 3 & 4/S1-8016.			
5) Confirm the HSS10x10x1/2 posts per detail 1/S1-8016 are orientated radially on the center of the 18" thick wall.			5) Confirm the HSS10x10x1/2 posts per detail 1/S1-8016 are orientated radially on the center of the 18" thick wall.			
<b>T-1281.1</b>	<b>SSS - W8, W12 Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>06/20/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
This is a follow-up RFI to RFI T-1281 (SK 467, CD 350)			This is a follow-up RFI to RFI T-1281 (SK 467, CD 350)			
See attached CD RFI # 350.1 SK1 & SK2 for items 1 & 2:			See attached CD RFI # 350.1 SK1 & SK2 for items 1 & 2:			
1) It is not clear where the posts per 1/S1-8006 are to be located. Work with SK2 and clarify where the posts are						



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	<p>located on Grid 16.9 relative to the east-west beams west of Grid 16.9.</p> <p>2) Confirm it is the intent to have the noted dimensions as shown or should they be the same?</p>					
T-1281.2	<p><b>SSS - W8, W12 Connection Clarifications</b></p> <p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>See attached CD RFI # 350.2 SK1:</p> <p>1). The dimensions provided in 1/S1-6043 and 6/S1-6040 are not the same. Provide the clouded dimensions to the posts from the grids as shown.</p> <p>2). In conjunction with question # 1 and the response to RFI # T-1281.1, the drawings from TG08.10 DB IFB Addenda # 11 (5/16/14) have not been issued for construction. Please issue these drawings for construction or supply them as sketches in response to this RFI.</p>	Closed	CR	07/08/2014	07/18/2014	07/11/2014
	<p><b>ANSWER:</b></p> <p>See attached CD RFI # 350.2 SK1:</p> <p>1). The dimensions provided in 1/S1-6043 and 6/S1-6040 are not the same. Provide the clouded dimensions to the posts from the grids as shown.</p> <p>2). In conjunction with question # 1 and the response to RFI # T-1281.1, the drawings from TG08.10 DB IFB Addenda # 11 (5/16/14) have not been issued for construction. Please issue these drawings for construction or supply them as sketches in response to this RFI.</p>					
T-1281.3	<p><b>SSS - W8, W12 Connection Clarifications</b></p> <p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>See attached CD RFI # 350.3 SK1 &amp; SK2: S1-6030 does not show the noted dimensions as shown on SK2. Please supply the dimensions to locate the beam/post on center of the curb above.</p>	Closed	CR	10/02/2014	10/12/2014	11/03/2014
	<p><b>ANSWER:</b></p> <p>See attached CD RFI # 350.3 SK1 &amp; SK2: S1-6030 does not show the noted dimensions as shown on SK2. Please supply the dimensions to locate the beam/post on center of the curb above.</p>					



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T-1281.4	SSS - W8, W12 Connection Clarifications	Closed	CR	11/24/2014	12/04/2014	12/12/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:  Please refer to sketches SK1, SK2, SK3, and SKS-0422, and attached drawings provided in response to RFI T-1281.3  1) S1-6030 does not show the noted dimensions as shown on SK2. Please supply the dimensions to locate the beam/post on center of the curb above. Supply the requested dimensions or confirm that posts are not required between 22-23. 2) The model and the drawings match the clouded dimensions on drawings A1-2905 included with the response in RFI T-1281.3. Clarify the reason why this drawing was included in the response. 3) "Refer to sheet S1-6032 for W-8 shear key dimensions" on the cover sheet of RFI T-1281.3 is confusing. Confirm W8should read W-12 as S1-6032 shows only W-12 details. 4) Confirm the revisions to 4/S1-8016 on SK-0422 apply only to Grid 23. Note that 4/S1-8016 is not shown on the current S1-2605. 5) 4/S1-8016 does not reflect what is occurring on Grid 23. Please review and clarify the discrepancy. 6) 4/S1-8016 on SK-0422 shows insufficient information to provide shop drawings: a) How is the solid 12' x 12" bar welded to the base beam? b) Does the solid 12" x 12" bar extend the full height of the HSS14x14? c) Supply dimensions to locate the rebar holes in the HSS14x14. d) Supply the size of the rebar holes. 7) Currently the HSS10x10 posts per 1 & 3/S1-8016 have been located on the center of the 1'-6 thick curb per RFI T1281 (SK467, CD 350) item 5. Note that the revised dimensions were not marked on the CS10, ES1, or ES2 returned approval submittals. Confirm the posts locations must be revised as noted in RFI T-1281.3. 8) Supply the post locations in 1/S1-8008. Refer to SK3		ANSWER:  Please refer to sketches SK1, SK2, SK3, and SKS-0422, and attached drawings provided in response to RFI T-1281.3  1) S1-6030 does not show the noted dimensions as shown on SK2. Please supply the dimensions to locate the beam/post on center of the curb above. Supply the requested dimensions or confirm that posts are not required between 22-23. 2) The model and the drawings match the clouded dimensions on drawings A1-2905 included with the response in RFI T-1281.3. Clarify the reason why this drawing was included in the response. 3) "Refer to sheet S1-6032 for W-8 shear key dimensions" on the cover sheet of RFI T-1281.3 is confusing. Confirm W8should read W-12 as S1-6032 shows only W-12 details. 4) Confirm the revisions to 4/S1-8016 on SK-0422 apply only to Grid 23. Note that 4/S1-8016 is not shown on the current S1-2605. 5) 4/S1-8016 does not reflect what is occurring on Grid 23. Please review and clarify the discrepancy. 6) 4/S1-8016 on SK-0422 shows insufficient information to provide shop drawings: a) How is the solid 12' x 12" bar welded to the base beam? b) Does the solid 12" x 12" bar extend the full height of the HSS14x14? c) Supply dimensions to locate the rebar holes in the HSS14x14. d) Supply the size of the rebar holes. 7) Currently the HSS10x10 posts per 1 & 3/S1-8016 have been located on the center of the 1'-6 thick curb per RFI T1281 (SK467, CD 350) item 5. Note that the revised dimensions were not marked on the CS10, ES1, or ES2 returned approval submittals. Confirm the posts locations must be revised as noted in RFI T-1281.3. 8) Supply the post locations in 1/S1-8008. Refer to SK3				
T-1281.5A	SSS - Missing Connection Information at HSS Post GL 24	Closed	CR	12/18/2014	12/28/2014	01/06/2015
From: Webcor Construction LP Stephanie Azzolino						







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T-1281.6	SSS - W-12 Missing Information at Construction Joints GL 23-24	Closed	01	01/19/2015	01/29/2015	01/28/2015
<div><div>From: Webcor Construction LP</div><div>Andrew Kitchen</div></div>						
REQUEST:			ANSWER:			
Contract Doc Ref: T-1281.4, S1-2605 & 4/S1-8016			Contract Doc Ref: T-1281.4, S1-2605 & 4/S1-8016			
Location: GL 23-24			Location: GL 23-24			
Add'l Doc Ref: CD RFI 350.6 SK1			Add'l Doc Ref: CD RFI 350.6 SK1			
This is a follow-up RFI to RFI T-1281.4 (SK 467.5, CD 350.5). Please provide the following:			This is a follow-up RFI to RFI T-1281.4 (SK 467.5, CD 350.5). Please provide the following:			
1) Supply dimension to locate first rebar above construction joint.			1) Supply dimension to locate first rebar above construction joint.			
2) Supply location of construction joint.			2) Supply location of construction joint.			
3) Supply the diameter of the rebars.			3) Supply the diameter of the rebars.			





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<b>T-1283</b>	<b>SSS - Drag Beam Double Connection</b>	<b>Closed</b>	<b>CR</b>	<b>03/25/2014</b>	<b>04/04/2014</b>	<b>04/04/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 354 SK1 to SK3 for items 1 & 2:  1) Confirm the double angle connection per 5v/S1-5026 (SK2) occurs at all locations noted with an arrow on SK1 (16 total).  2) The double angle connection per 5v/S1-5026 & 6/S1-5026 does not allow access to install the threaded rod nuts (4-3/4" long x 4-1/2" wide) and washers in the field. Please confirm it is acceptable to provide two elongated hand holes (6" wide x 10" long) in each full height shear plate either side of the beam. Due to the restricted access, the post tensioning operation will need to be done from the top of the rod, contrary to the response to RFI T-0970.1. This will require the 6x6" oversized washer to be installed at the bottom and the standard flat washer to be installed at the top. Please confirm this is acceptable						<b>ANSWER:</b> See attached CD RFI # 354 SK1 to SK3 for items 1 & 2:  1) Confirm the double angle connection per 5v/S1-5026 (SK2) occurs at all locations noted with an arrow on SK1 (16 total).  2) The double angle connection per 5v/S1-5026 & 6/S1-5026 does not allow access to install the threaded rod nuts (4-3/4" long x 4-1/2" wide) and washers in the field. Please confirm it is acceptable to provide two elongated hand holes (6" wide x 10" long) in each full height shear plate either side of the beam. Due to the restricted access, the post tensioning operation will need to be done from the top of the rod, contrary to the response to RFI T-0970.1. This will require the 6x6" oversized washer to be installed at the bottom and the standard flat washer to be installed at the top. Please confirm this is acceptable
<b>T-1284</b>	<b>SSS - Erection of Drag Beam at Double Connection</b>	<b>Closed</b>	<b>CR</b>	<b>03/25/2014</b>	<b>04/04/2014</b>	<b>04/04/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 355 SK1 & SK2: The connection per 5v/S1-5026 creates erectability issues for the beams on Grids C & G. Please confirm it is acceptable to cope the bottom flange of the drag beam to clear the double angles.						<b>ANSWER:</b> See attached CD RFI # 355 SK1 & SK2: The connection per 5v/S1-5026 creates erectability issues for the beams on Grids C & G. Please confirm it is acceptable to cope the bottom flange of the drag beam to clear the double angles.



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<b>T-1285</b>	<b>BGP - Anchor Bolt Placement Tolerance</b>	<b>Closed</b>	<b>01</b>	<b>03/26/2014</b>	<b>04/05/2014</b>	<b>03/26/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Under the Below Grade Concrete TG-06 Contract, SCCI is bound by concrete standard for Anchor Bolt placement. Concrete industry standard, ASI 117-90, Section 2.3 allows a tolerance of vertical, lateral and level alignment of +/- 1".			Under the Below Grade Concrete TG-06 Contract, SCCI is bound by concrete standard for Anchor Bolt placement. Concrete industry standard, ASI 117-90, Section 2.3 allows a tolerance of vertical, lateral and level alignment of +/- 1".			
Per Anchor Bolt DFOW meeting on 11/29/2014, SCCI proposed to use ASCC (American Society of Concrete Contractors) "Anchor Bolt Tolerances" Position Statement #14 (attached) with the following tolerance for each bolt location: - 3/4 and 7/8 diameter bolts : +/- 1/4in - 1-, 1-1/4, and 1-1/2in diameter bolts: +/- 3/8in ; and - 1-3/4, 2-, and 2-1/2-in diameter bolts: +/- 1/2in			Per Anchor Bolt DFOW meeting on 11/29/2014, SCCI proposed to use ASCC (American Society of Concrete Contractors) "Anchor Bolt Tolerances" Position Statement #14 (attached) with the following tolerance for each bolt location: - 3/4 and 7/8 diameter bolts : +/- 1/4in - 1-, 1-1/4, and 1-1/2in diameter bolts: +/- 3/8in ; and - 1-3/4, 2-, and 2-1/2-in diameter bolts: +/- 1/2in			
Please confirm that the proposed anchor bolt placement tolerance as prescribed by ASCC "Anchor Bolt Tolerances" Position Statement #14 is acceptable. Please note that this tolerance is more stringent than ACI Concrete Standard ACI 17-90, Section 2.3.			Please confirm that the proposed anchor bolt placement tolerance as prescribed by ASCC "Anchor Bolt Tolerances" Position Statement #14 is acceptable. Please note that this tolerance is more stringent than ACI Concrete Standard ACI 17-90, Section 2.3.			
<b>T-1286</b>	<b>SSS - Bus Level Beam Splice at GL 18</b>	<b>Closed</b>	<b>CR</b>	<b>03/26/2014</b>	<b>04/05/2014</b>	<b>04/16/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At the Bus deck level see attached CD RFI # 368 SK1 & SK2: With the limitations of the B.U. Beam and the dimensions per 2/S1-5026 (SK1), we cannot fit 28 bolts in 4 rows per beam. Confirm 24 bolts per side are acceptable or supply an alternate solution.			At the Bus deck level see attached CD RFI # 368 SK1 & SK2: With the limitations of the B.U. Beam and the dimensions per 2/S1-5026 (SK1), we cannot fit 28 bolts in 4 rows per beam. Confirm 24 bolts per side are acceptable or supply an alternate solution.			
<b>T-1287</b>	<b>BGP - Mat Slab - Train Platform Future Wall Discrepancies</b>	<b>Closed</b>	<b>01</b>	<b>03/26/2014</b>	<b>04/05/2014</b>	<b>03/28/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CDs. It appears that all of the future platforms walls along GL E are encroaching into the columns along GL D.8 by 1 1/2".			See attached CDs. It appears that all of the future platforms walls along GL E are encroaching into the columns along GL D.8 by 1 1/2".			





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	due to the sensitivity for potential shrinkage cracking in the slab.					the 10 day cure time requirement. SCCI does not want to compromise the immediate moist cure cover installation due to the sensitivity for potential shrinkage cracking in the slab.
	Are either of these requests acceptable?					Are either of these requests acceptable?
<b>T-1289</b>	<b>SSS - ST401 Missing Dimensions and Connections</b>	<b>Closed</b>	<b>CR</b>	<b>03/26/2014</b>	<b>04/05/2014</b>	<b>04/16/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>						<b>ANSWER:</b>
See attached CD RFI # 367 SK1 to SK6 for items 1 to 13: 1) Review all the east/west post location dimensions on SK1 to SK6 and confirm they are correct as RFI T-1189 (SK419, CD 313) does not show the dimensions at all Levels. 2) The 18'-6 dimension to the edge of slab minus the 6" set-back dimension per S1-2304 locates the beam 18'-0 from Grid 15. This does not match the 18'-8 supplied in RFI T-1189 (SK 419, CD 313). Please clarify the discrepancy in beam and edge of slab location. 3) Confirm the noted section reference is correct. 4) The actual condition does not reflect what is requested in detail 3/S1-7601. Please clarify what is required. 5) Supply the missing clouded dimensions at (4) locations shown. 6) The detail shows the minimum offset dimension from the center of posts to the W10x22's in order to make the requested connections. Please review this with the actual dimensions and supply revised connection details as necessary. 7) Supply the missing clouded dimensions at (8) locations shown. 8) Supply the missing clouded dimensions at (8) locations shown. 9) Supply the missing clouded dimensions at (8) locations shown. 10) Connection section references are missing. Confirm they are correct as shown. 11) Supply the missing clouded dimensions at (8) locations shown. 12) Connection section references are missing. Confirm they are correct as shown. 13) Supply the missing clouded dimensions at (8)						See attached CD RFI # 367 SK1 to SK6 for items 1 to 13: 1) Review all the east/west post location dimensions on SK1 to SK6 and confirm they are correct as RFI T-1189 (SK419, CD 313) does not show the dimensions at all Levels. 2) The 18'-6 dimension to the edge of slab minus the 6" set-back dimension per S1-2304 locates the beam 18'-0 from Grid 15. This does not match the 18'-8 supplied in RFI T-1189 (SK 419, CD 313). Please clarify the discrepancy in beam and edge of slab location. 3) Confirm the noted section reference is correct. 4) The actual condition does not reflect what is requested in detail 3/S1-7601. Please clarify what is required. 5) Supply the missing clouded dimensions at (4) locations shown. 6) The detail shows the minimum offset dimension from the center of posts to the W10x22's in order to make the requested connections. Please review this with the actual dimensions and supply revised connection details as necessary. 7) Supply the missing clouded dimensions at (8) locations shown. 8) Supply the missing clouded dimensions at (8) locations shown. 9) Supply the missing clouded dimensions at (8) locations shown. 10) Connection section references are missing. Confirm they are correct as shown. 11) Supply the missing clouded dimensions at (8) locations shown.



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	locations shown.					12) Connection section references are missing. Confirm they are correct as shown. 13) Supply the missing clouded dimensions at (8) locations shown.
<b>T-1289.1</b>	<b>SSS - Stair ST401 dimension clarification</b>	<b>Closed</b>	<b>CR</b>	<b>05/23/2014</b>	<b>06/02/2014</b>	<b>05/27/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> This is a follow-up RFI to RFI T-1289 (SK 487, CD 367)  See attached CD RFI # 367.1 SK1 to SK3: The indicated dimensions between the stair stringers supplied in above noted RFI SSK-0341 & SSK-0342 (see SK1 & SK2) will not work. The width of the flange on the C12x20.7 = 3" and therefore the stringers will foul with the 2 3/4" dimensions supplied. See SK3 and clarify the dimensions between the stringers.						<b>ANSWER:</b>  This is a follow-up RFI to RFI T-1289 (SK 487, CD 367)  See attached CD RFI # 367.1 SK1 to SK3: The indicated dimensions between the stair stringers supplied in above noted RFI SSK-0341 & SSK-0342 (see SK1 & SK2) will not work. The width of the flange on the C12x20.7 = 3" and therefore the stringers will foul with the 2 3/4" dimensions supplied. See SK3 and clarify the dimensions between the stringers.
<b>T-1290</b>	<b>SSS - Mean Temperature in Service</b>	<b>Closed</b>	<b>CR</b>	<b>03/26/2014</b>	<b>04/05/2014</b>	<b>04/01/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> The contract specifications state that steel fabrication and erection are to: "Compensate for the difference between the temperature at the time of fabrication and the mean temperature in service." - Spec 05 10 00 - 3.2.B.1 "Compensate for the difference between the temperature at the time of erection and the mean temperature in service." - Spec. 05 10 00 - 3.3.A  Mean service temperature is referenced, but not defined. So that we are coordinated in our efforts, please identify the "Mean Service Temperature."						<b>ANSWER:</b>  The contract specifications state that steel fabrication and erection are to: "Compensate for the difference between the temperature at the time of fabrication and the mean temperature in service." - Spec 05 10 00 - 3.2.B.1 "Compensate for the difference between the temperature at the time of erection and the mean temperature in service." - Spec. 05 10 00 - 3.3.A  Mean service temperature is referenced, but not defined. So that we are coordinated in our efforts, please identify the "Mean Service Temperature."
<b>T-1291</b>	<b>SSS - Engineer's Comments on 643AC &amp; 645AC</b>	<b>Closed</b>	<b>CR</b>	<b>03/26/2014</b>	<b>04/05/2014</b>	<b>04/04/2014</b>



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<b>From:</b> Webcor Construction LP	Gregory Kemerer					
<b>REQUEST:</b>	<b>ANSWER:</b>					
See attached CD RFI # 372 SK1 & SK2 for items 1 & 2: Drawings 643AC and 645AC from TG0701-075 SSS - Structural Steel Shop DWG (CS3) GL 12-13 are attached for your reference	See attached CD RFI # 372 SK1 & SK2 for items 1 & 2: Drawings 643AC and 645AC from TG0701-075 SSS - Structural Steel Shop DWG (CS3) GL 12-13 are attached for your reference					
1) The noted approval comment does not help as the plate will foul the rebar holes when located 11" down. See SK2 and confirm the 8 7/8" dimension is acceptable or supply a workable solution.	1) The noted approval comment does not help as the plate will foul the rebar holes when located 11" down. See SK2 and confirm the 8 7/8" dimension is acceptable or supply a workable solution.					
2) The weld is not missing. It is shown in Detail 1 on drawing 643AC as WD1Q, which is correct per 6/S1-4350 as the weld is non-DCW. NOTE: The same applies to drawing 645AC.	2) The weld is not missing. It is shown in Detail 1 on drawing 643AC as WD1Q, which is correct per 6/S1- 4350 as the weld is non-DCW. NOTE: The same applies to drawing 645AC.					
<b>T-1292</b>	<b>BGP - Lower Concourse Electrical Room Layout</b>	<b>Closed</b>	<b>01</b>	<b>03/26/2014</b>	<b>04/05/2014</b>	<b>03/31/2014</b>
<b>From:</b> Webcor Construction LP	Claude Titche					
<b>REQUEST:</b>	<b>ANSWER:</b>					
Please see attached layouts for Lower Concourse Electrical Rooms B2280, B1563, B1644 and B1325.	Please see attached layouts for Lower Concourse Electrical Rooms B2280, B1563, B1644 and B1325.					
Please confirm the layouts are acceptable.	Please confirm the layouts are acceptable.					





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<b>T-1293</b>	<b>BGP - Lower Concourse Shear Wall Inconsistency</b>	<b>Closed</b>	<b>01</b>	<b>03/27/2014</b>	<b>04/06/2014</b>	<b>04/08/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>  See attached Contract Documents, A1-2202 and S1-2202.  An inconsistency was discovered between drawing A1-2202 and drawing S1-2202 regarding the West Throat Shear wall above the Lower Concourse. Sheet A1-2202 shows the shear wall stopping at the corridor, sheet S1-2202 shows the shear wall penetrating the corridor.  Please confirm which drawing, A1-2202 or S1-2202 governs.		<b>ANSWER:</b>  See attached Contract Documents, A1-2202 and S1-2202.  An inconsistency was discovered between drawing A1-2202 and drawing S1-2202 regarding the West Throat Shear wall above the Lower Concourse. Sheet A1-2202 shows the shear wall stopping at the corridor, sheet S1-2202 shows the shear wall penetrating the corridor.  Please confirm which drawing, A1-2202 or S1-2202 governs.				
<b>T-1293.1</b>	<b>BGP - Lower Concourse Shearwall Inconsistency</b>	<b>Closed</b>	<b>01</b>	<b>05/19/2014</b>	<b>05/29/2014</b>	<b>05/20/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>  RFI T-1293 response directed Webcor to modify the boundary of West Throat Shearwall W191F above the Lower Concourse per SKS-0339 and SKS-0340 to match drawing A1-2202. The response did not address the impact to Column C17 shown on 1/S1-2250. Please confirm that SKS-0348, SKS-0351, and SKS-0352 will be used in lieu of the RFI-1293 response and accompanying sketches.		<b>ANSWER:</b>  RFI T-1293 response directed Webcor to modify the boundary of West Throat Shearwall W191F above the Lower Concourse per SKS-0339 and SKS-0340 to match drawing A1-2202. The response did not address the impact to Column C17 shown on 1/S1-2250. Please confirm that SKS-0348, SKS-0351, and SKS-0352 will be used in lieu of the RFI-1293 response and accompanying sketches.				
<b>T-1294</b>	<b>SSS - Bus Deck Level Drag Beams Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>03/27/2014</b>	<b>04/06/2014</b>	<b>04/08/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  Per detail 1/S1-5018 see attached sketch CD RFI # 371 SK1  1). Due to the shear plate locations and erection clearance for the noted beams please verify one side of the bottom flange can be cut flush to the beam web in order to drop the beam straight down for erection. If not please supply an alternate connection.  2). From CS7 and on verify the bottom flange holes can		<b>ANSWER:</b>  Per detail 1/S1-5018 see attached sketch CD RFI # 371 SK1  1). Due to the shear plate locations and erection clearance for the noted beams please verify one side of the bottom flange can be cut flush to the beam web in order to drop the beam straight down for erection. If not please supply an alternate connection.  2). From CS7 and on verify the bottom flange holes				



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	start 4 1/2" from the required cope as shown.					can start 4 1/2" from the required cope as shown.
T-1295	<b>SSS - Extent of IFRM-1 Finishes</b>  From: Webcor Construction LP Gregory Kemerer	Closed	CR	03/27/2014	04/06/2014	04/09/2014
	<b>REQUEST:</b>  See attached CD RFI # 359 SK1 to SK5 for items 1 to 7: 1) Confirm that only steel visible inside the slab opening between the slab edges is IFRM-1 and the unexposed portion of these members is SFRM. If not, supply specific information for the extent of IFRM-1 including the beam end connection per 5/S1-5017. 2) Supply specific information for the extent of IFRM-1, including the beam end connection per 8/S1-5025. 3) Supply specific information for the extent of IFRM-1, including the beam end connection per 9/S1-5025. 4) Supply specific information for the extent of IFRM-1, including the beam end connection per 7/S1-5012. 5) Per S1-2606 & A/S1-4114, this beam is cantilevered over columns on each end. Supply specific information for the extent of IFRM-1 on each end of the cantilevered portion of the beam (Ref: A/S1-4114). 6) Confirm that only steel visible inside the slab opening between the slab edges is IFRM-1 and the unexposed portion of these members is SFRM. If not, supply specific information for the extent of IFRM-1 including the beam end connection per 4/S1-5012. 7) Detail 4/A1-8663 indicates that four MF Beams at the W-12 glass floor are to receive IFRM-1 coating. The MF beam along GL 19.1 is not indicated to receive IFRM coating, but spans a similar opening adjacent to the W-12 system. Please confirm that the beam indicated on SK4 along GL 19.1 is to receive SFRM and not IFRM-1.					<b>ANSWER:</b>  See attached CD RFI # 359 SK1 to SK5 for items 1 to 7: 1) Confirm that only steel visible inside the slab opening between the slab edges is IFRM-1 and the unexposed portion of these members is SFRM. If not, supply specific information for the extent of IFRM-1 including the beam end connection per 5/S1-5017. 2) Supply specific information for the extent of IFRM-1, including the beam end connection per 8/S1-5025. 3) Supply specific information for the extent of IFRM-1, including the beam end connection per 9/S1-5025. 4) Supply specific information for the extent of IFRM-1, including the beam end connection per 7/S1-5012. 5) Per S1-2606 & A/S1-4114, this beam is cantilevered over columns on each end. Supply specific information for the extent of IFRM-1 on each end of the cantilevered portion of the beam (Ref: A/S1-4114). 6) Confirm that only steel visible inside the slab opening between the slab edges is IFRM-1 and the unexposed portion of these members is SFRM. If not, supply specific information for the extent of IFRM-1 including the beam end connection per 4/S1-5012. 7) Detail 4/A1-8663 indicates that four MF Beams at the W-12 glass floor are to receive IFRM-1 coating. The MF beam along GL 19.1 is not indicated to receive IFRM coating, but spans a similar opening adjacent to the W-12 system. Please confirm that the beam indicated on SK4 along GL 19.1 is to receive SFRM and not IFRM-1.



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<b>T-1296</b>	<b>BGP - Concrete Plant Recertification Test Batch</b>	<b>Closed</b>	<b>01</b>	<b>03/31/2014</b>	<b>04/10/2014</b>	<b>04/02/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The 4th review comment to SUBM TG0600-095 (document enclosed as reference) states after plant recertification a "test batch" will be prepared and tested for accuracy. Attached is SCCI's concrete supplier (CEMEX) responses to the submittal review comments.			The 4th review comment to SUBM TG0600-095 (document enclosed as reference) states after plant recertification a "test batch" will be prepared and tested for accuracy. Attached is SCCI's concrete supplier (CEMEX) responses to the submittal review comments.			
Regarding the test batch, is it acceptable to either:			Regarding the test batch, is it acceptable to either:			
1) Have an Owner's representative perform a plant visit during production operations to observe batching tolerance during normal business hours?			1) Have an Owner's representative perform a plant visit during production operations to observe batching tolerance during normal business hours?			
or			or			
2) Neither perform the test batch nor plant visit?			2) Neither perform the test batch nor plant visit?			
<b>T-1297</b>	<b>SSS - EoS Bent Plate at Knock-out Slab GL11-12</b>	<b>Closed</b>	<b>CR</b>	<b>03/31/2014</b>	<b>04/10/2014</b>	<b>04/11/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 378 SK1 & SK2: The knock-out slab extends 1 1/4" onto the flange as shown. Confirm that is acceptable or provide alternate direction.			See attached CD RFI # 378 SK1 & SK2: The knock-out slab extends 1 1/4" onto the flange as shown. Confirm that is acceptable or provide alternate direction.			
<b>T-1298</b>	<b>SSS - Lift Eyes on Ground Cast Nodes</b>	<b>Closed</b>	<b>CR</b>	<b>03/31/2014</b>	<b>04/10/2014</b>	<b>04/09/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The contract drawings indicate that lift eyes will be provided for shop handling:			The contract drawings indicate that lift eyes will be provided for shop handling:			
"Provide picking eye(s) as required for the handling of the cast node in the foundry AND the shop of the steel fabricator. Picking eye(s) to be located within the interior of the casting's nozzle(s)"			"Provide picking eye(s) as required for the handling of the cast node in the foundry AND the shop of the steel fabricator. Picking eye(s) to be located within the interior of the casting's nozzle(s)"			
The lift eyes that were cast into the back side of the ground level cast nodes have now been machined off by Bradken, leaving only the lift eyes inside the nozzles. In			The lift eyes that were cast into the back side of the ground level cast nodes have now been machined off by			



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	<p>order to safely handle these castings, new lift features need to be added to the back side to replace those that were machined off.</p> <p>OIW notes that if the castings are picked using only the lift eyes inside the nozzle, the center of gravity causes the castings to rotate into an unworkable position. Some other manner of rigging must be employed in order to manipulate the castings into positions required for shop fabrication work. CastConnex noted exactly the same issue in their Release 1 Pre Shipping Inspection Summary, even noting that the improvised and awkward handling was causing damage to the castings:</p> <p>"The lifting lugs are located inside the nozzle-end on these parts making the machined nodes difficult to manipulate. Often the parts have to be lifted using straps thru the pin hole causing some broken edges on the radius of the pin hole."</p> <p>To facilitate handling the cast nodes in the shop in a way that is safe and that avoids damaging the castings, OIW proposes that drilled and tapped holes be added. The attached sketches (2773-SK-401 and 2773-SK-402) show a proposed arrangement of these holes. Note that the proposed locations are in areas that will not be visible in the final product, and also are in areas of minimal stress. OIW requests that the engineer of record review the attached drawings and determine if the proposed modifications to the cast node are acceptable.</p>					
	<p>Bradken, leaving only the lift eyes inside the nozzles. In order to safely handle these castings, new lift features need to be added to the back side to replace those that were machined off.</p> <p>OIW notes that if the castings are picked using only the lift eyes inside the nozzle, the center of gravity causes the castings to rotate into an unworkable position. Some other manner of rigging must be employed in order to manipulate the castings into positions required for shop fabrication work. CastConnex noted exactly the same issue in their Release 1 Pre Shipping Inspection Summary, even noting that the improvised and awkward handling was causing damage to the castings:</p> <p>"The lifting lugs are located inside the nozzle-end on these parts making the machined nodes difficult to manipulate. Often the parts have to be lifted using straps thru the pin hole causing some broken edges on the radius of the pin hole."</p> <p>To facilitate handling the cast nodes in the shop in a way that is safe and that avoids damaging the castings, OIW proposes that drilled and tapped holes be added. The attached sketches (2773-SK-401 and 2773-SK-402) show a proposed arrangement of these holes. Note that the proposed locations are in areas that will not be visible in the final product, and also are in areas of minimal stress. OIW requests that the engineer of record review the attached drawings and determine if the proposed modifications to the cast node are acceptable.</p>					
T-1299	SSS - Off-Set Beam Connection Modification at PE 403/404	Closed	CR	04/02/2014	04/12/2014	04/11/2014
From: Webcor Construction LP Stephanie Azzolino						
REQUEST:			ANSWER:			



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	See attached CD RFI # 377 SK1 & SK2 for items 1 & 2:  1) Confirm the connections as shown are acceptable at the noted off-set beam locations. If not, supply an alternate connection detail.  2) Confirm the details shown on SK2 may be applied typically at future similar cases. If not, supply the missing detail for off-set beam connections with double angles per 1/S1-5010.					See attached CD RFI # 377 SK1 & SK2 for items 1 & 2:  1) Confirm the connections as shown are acceptable at the noted off-set beam locations. If not, supply an alternate connection detail.  2) Confirm the details shown on SK2 may be applied typically at future similar cases. If not, supply the missing detail for off-set beam connections with double angles per 1/S1-5010.
T-1300	<b>SSS - Carboline AESS Primer</b>  <b>From:</b> Webcor Construction LP                      Stephanie Azzolino	Closed	CR	04/02/2014	04/12/2014	04/04/2014
	<b>REQUEST:</b>  Specification section 05 10 00-2.2.B.2.b indicates that Carboline Carbozinc 621 is to be used for non-galvanized steel to receive high performance coating. Reference is made to the letter provided by Carboline attached, which publishes that this (nearly obsolete) product has a recoat window of only 2-3 hours, which cannot be achieved on the Transbay project since subsequent overcoats will be installed in the field by others.  Carboline has provided information and validation for use of the Carbozinc 859 Organic Zinc Rich Epoxy as a replacement for Carbozinc 621, which provides protection and performance equal to or greater than Carbozinc 621. The Carbozinc 859 product provides an unlimited recoat window, allowing for intermediate and top coats to be applied by others in the field, in accordance with the project's schedule and contractual requirements.  Please confirm the Carbozinc 859 product is acceptable for use as the AESS primer.  Note: Specification section 09 97 16-2.2.A states that the listed manufacturers are acceptable subject to conformance					<b>ANSWER:</b>  Specification section 05 10 00-2.2.B.2.b indicates that Carboline Carbozinc 621 is to be used for non-galvanized steel to receive high performance coating. Reference is made to the letter provided by Carboline attached, which publishes that this (nearly obsolete) product has a recoat window of only 2-3 hours, which cannot be achieved on the Transbay project since subsequent overcoats will be installed in the field by others.  Carboline has provided information and validation for use of the Carbozinc 859 Organic Zinc Rich Epoxy as a replacement for Carbozinc 621, which provides protection and performance equal to or greater than Carbozinc 621. The Carbozinc 859 product provides an unlimited recoat window, allowing for intermediate and top coats to be applied by others in the field, in accordance with the project's schedule and contractual requirements.  Please confirm the Carbozinc 859 product is acceptable for use as the AESS primer.



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	<p>to requirements of Drawings, Schedules and Specifications. For this reason, Carbozinc 859 is proposed as the recommended product by the basis of design manufacturer, Carboline, rather than a product substitution.</p>					<p>Note: Specification section 09 97 16-2.2.A states that the listed manufacturers are acceptable subject to conformance to requirements of Drawings, Schedules and Specifications. For this reason, Carbozinc 859 is proposed as the recommended product by the basis of design manufacturer, Carboline, rather than a product substitution.</p>
T-1301	<p><b>SSS - Erection Aid at Roof Spandrel Beams</b></p> <p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p> <p><b>REQUEST:</b></p> <p>Reference SK-4.7 attached, which details the erection aid for the perimeter roof beams. The connection plate shown in A/SK-4.7 and B/SK-4.7 will also serve as the back-up bar for the CJP weld at the beam web.</p> <p>Our intent is to leave the connection plate/back-up bar in place after welding. Please confirm this is acceptable.</p>	Closed	CR	04/02/2014	04/12/2014	04/07/2014
						<p><b>ANSWER:</b></p> <p>Reference SK-4.7 attached, which details the erection aid for the perimeter roof beams. The connection plate shown in A/SK-4.7 and B/SK-4.7 will also serve as the back-up bar for the CJP weld at the beam web.</p> <p>Our intent is to leave the connection plate/back-up bar in place after welding. Please confirm this is acceptable.</p>
T-1302	<p><b>SSS - CP5 Connection Points</b></p> <p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p> <p><b>REQUEST:</b></p> <p>Detail 1/S1-8003 shows a corner to corner joint between the 2.5" mounting plate and the two supporting vertical stiffeners and the single horizontal stiffener on top.</p> <p>Please confirm the two vertical stiffeners can be repositioned 1/4" in towards the center of the connection as well as lowering the horizontal stiffener. These adjustments will provide a land for the specified fillet weld and minimize melt through and weld splatter at the corners of each plate. Backside stiffeners will be repositioned to match as required.</p>	Closed	CR	04/02/2014	04/12/2014	04/16/2014
						<p><b>ANSWER:</b></p> <p>Detail 1/S1-8003 shows a corner to corner joint between the 2.5" mounting plate and the two supporting vertical stiffeners and the single horizontal stiffener on top.</p> <p>Please confirm the two vertical stiffeners can be repositioned 1/4" in towards the center of the connection as well as lowering the horizontal stiffener. These adjustments will provide a land for the specified fillet weld and minimize melt through and weld splatter at the corners of each plate. Backside stiffeners will be repositioned to match as required.</p>



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T-1303	Wall Rebar in conflict with raker base plate 13 (GL1.5,D.8)	Closed	01	04/03/2014	04/03/2014	04/08/2014
<div><div><div>From: Webcor Construction LP</div><div>Michael Spillane</div></div><div><div>REQUEST:</div><div>Due to a conflict between the rebar dowels and the raker base plate 13. WOJV is proposing to cut the existing rebar dowels flush with the mat slab and drill and epoxy in a new same sized bar beside the existing one once the re-bracing raker is removed.</div><div>Please confirm if this would be acceptable</div></div><div><div>ANSWER:</div><div>Due to a conflict between the rebar dowels and the raker base plate 13. WOJV is proposing to cut the existing rebar dowels flush with the mat slab and drill and epoxy in a new same sized bar beside the existing one once the re-bracing raker is removed.</div><div>Please confirm if this would be acceptable</div></div></div>						





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<b>T-1304</b>	<b>SSS - Follow-up to CS3 Approval Comments</b>	<b>Closed</b>	<b>CR</b>	<b>04/04/2014</b>	<b>04/14/2014</b>	<b>04/21/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please review and confirm the items below regarding the TG0701-075 SSS - Structural Steel Shop DWG (CS3) GL 12-13 approval comments:			Please review and confirm the items below regarding the TG0701-075 SSS - Structural Steel Shop DWG (CS3) GL 12-13 approval comments:			
1) Drawing 2669 ~ the noted beam is not sloping per S1-4004. Confirm the drawing is correct as submitted and no action is required.			1) Drawing 2669 ~ the noted beam is not sloping per S1-4004. Confirm the drawing is correct as submitted and no action is required.			
2) Drawing 2707 ~ the noted beam is not sloping per S1-4015 (S1-4004 is not the correct drawing reference). Confirm the drawing is correct as submitted and no action is required.			2) Drawing 2707 ~ the noted beam is not sloping per S1-4015 (S1-4004 is not the correct drawing reference). Confirm the drawing is correct as submitted and no action is required.			
3) Drawings 2706, 2710, 2713AB ~ the 3 1/16" dimensions are necessary as the beam webs vary in thickness. The 3" dimension on the angles has been used at the thickest beam web. Confirm the drawings are correct as submitted and no action is required.			3) Drawings 2706, 2710, 2713AB ~ the 3 1/16" dimensions are necessary as the beam webs vary in thickness. The 3" dimension on the angles has been used at the thickest beam web. Confirm the drawings are correct as submitted and no action is required.			
4) Drawings 3792, 3793 & 3794, 4933, 4935, 4994, 4995, 4998 ~ The top cope on the right end is correct as shown. The Approver's reference to a 2" flange is incorrect as the TPG1 on Grid 13 has a 3 1/2" thick flange per S1-4200. We have limited the clearance at the top to 1/4" be able to fit the number of bolts per S1-5010. Confirm the drawings are correct as submitted and no action is required.			4) Drawings 3792, 3793 & 3794, 4933, 4935, 4994, 4995, 4998 ~ The top cope on the right end is correct as shown. The Approver's reference to a 2" flange is incorrect as the TPG1 on Grid 13 has a 3 1/2" thick flange per S1-4200. We have limited the clearance at the top to 1/4" be able to fit the number of bolts per S1-5010. Confirm the drawings are correct as submitted and no action is required.			
5) Drawings 4832 & 4924 ~ RFI T-0857 (SK 117, CD 089) asked for permission to cope the beam as shown on the submitted drawings on SK2 item 1 and the cope was approved in the response. Cutting one side of the flange is not sufficient to clear the brace. Confirm the drawings are correct as submitted and no action is required.			5) Drawings 4832 & 4924 ~ RFI T-0857 (SK 117, CD 089) asked for permission to cope the beam as shown on the submitted drawings on SK2 item 1 and the cope was approved in the response. Cutting one side of the flange is not sufficient to clear the brace. Confirm the drawings are correct as submitted and no action is required.			





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<b>T-1306</b>	<b>BGP - Geothermal Field 11 Mud Slab Rebar</b>	<b>Closed</b>	<b>01</b>	<b>04/07/2014</b>	<b>04/17/2014</b>	<b>04/13/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b> Please confirm it is acceptable to remove rebar in the mud slab of geothermal field 11 via contractors discretion. Please note rebar will be placed where micropiles are to be tested.						<b>ANSWER:</b> Please confirm it is acceptable to remove rebar in the mud slab of geothermal field 11 via contractors discretion. Please note rebar will be placed where micropiles are to be tested.
<b>T-1307</b>	<b>SSS - Ground Level Connection GL 8</b>	<b>Closed</b>	<b>CR</b>	<b>04/07/2014</b>	<b>04/17/2014</b>	<b>04/16/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 379 SK1 & SK2: Confirm it is acceptable to terminate the PL2x3/8 per 12/S1-5010 as shown to avoid fouling the connection angles. If not, supply a new detail.						<b>ANSWER:</b> See attached CD RFI # 379 SK1 & SK2: Confirm it is acceptable to terminate the PL2x3/8 per 12/S1-5010 as shown to avoid fouling the connection angles. If not, supply a new detail.
<b>T-1308</b>	<b>BGP - Geothermal Field 11 Disturbed Unsuitable Material</b>	<b>Closed</b>	<b>01</b>	<b>04/08/2014</b>	<b>04/18/2014</b>	<b>04/13/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b> During backfill activities in Geothermal Field 11 inclement weather occurred and as a result isolated areas of bay mud/clay are deemed unsuitable. Per field walk on 04-07-2014 with Arup these soft areas are found unacceptable to place mud lab. What are the procedures required to satisfy and meet an acceptable substrate for the mud slab.?						<b>ANSWER:</b> During backfill activities in Geothermal Field 11 inclement weather occurred and as a result isolated areas of bay mud/clay are deemed unsuitable. Per field walk on 04-07-2014 with Arup these soft areas are found unacceptable to place mud lab. What are the procedures required to satisfy and meet an acceptable substrate for the mud slab.?
<b>T-1309</b>	<b>SSS - Missing Stair ST202 information (GL 1.4-2)</b>	<b>Closed</b>	<b>CR</b>	<b>04/08/2014</b>	<b>04/18/2014</b>	<b>04/25/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 387 SK1 to SK6 for items 1 to 15: 1) Supply all clouded dimensions on SK1 thru SK6. 2) [Issue has been resolved, no further action necessary] 3) Supply the top of concrete elevation for the (4) posts. 4) This detail shows horizontal HSS beam but the beam is not shown here or on S1-2202. Please clarify.						<b>ANSWER:</b> See attached CD RFI # 387 SK1 to SK6 for items 1 to 15: 1) Supply all clouded dimensions on SK1 thru SK6. 2) [Issue has been resolved, no further action necessary] 3) Supply the top of concrete elevation for the (4) posts.



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	<p>5) This detail does not allow for any horizontal adjustment for concrete +/- location variances. Please review and advise.</p> <p>6) [This item is to be resolved by Skanska]</p> <p>7) Confirm these posts are continuous from 1/S1-7400 (SK1) to here.</p> <p>8) Clarify the noted landing steel and supply the member sizes, elevations &amp; dimensions.</p> <p>9) The minimum distance between the center of post and the W10x22 must be 1'-1 3/4 in order to connect per 3/S15012</p> <p>and 1/S1-5010. Supply alternate connection details if the dimension is less.</p> <p>This is a typical occurrence on all stairs.</p> <p>10) Confirm all stair landing beams are centered on the posts or supply offset dimensions.</p> <p>11) Supply a connection detail.</p> <p>12) The noted 2 braces per 1/S1-7600 will span across the slab opening. Confirm that is acceptable or give direction.</p> <p>13) Do the posts starting above the Lower Concourse Level in detail 2/S1-7400 (SK2) extend to the underside of the HSS beams? Please clarify.</p> <p>14) Clarify what supports the noted steel.</p> <p>15) Supply a corner connection detail.</p>					
	<p>4) This detail shows horizontal HSS beam but the beam is not shown here or on S1-2202. Please clarify.</p> <p>5) This detail does not allow for any horizontal adjustment for concrete +/- location variances. Please review and advise.</p> <p>6) [This item is to be resolved by Skanska]</p> <p>7) Confirm these posts are continuous from 1/S1-7400 (SK1) to here.</p> <p>8) Clarify the noted landing steel and supply the member sizes, elevations &amp; dimensions.</p> <p>9) The minimum distance between the center of post and the W10x22 must be 1'-1 3/4 in order to connect per 3/S15012</p> <p>and 1/S1-5010. Supply alternate connection details if the dimension is less.</p> <p>This is a typical occurrence on all stairs.</p> <p>10) Confirm all stair landing beams are centered on the posts or supply offset dimensions.</p> <p>11) Supply a connection detail.</p> <p>12) The noted 2 braces per 1/S1-7600 will span across the slab opening. Confirm that is acceptable or give direction.</p> <p>13) Do the posts starting above the Lower Concourse Level in detail 2/S1-7400 (SK2) extend to the underside of the HSS beams? Please clarify.</p> <p>14) Clarify what supports the noted steel.</p> <p>15) Supply a corner connection detail.</p>					
<b>T-1310</b>	<b>SSS - CS3 Review Clarifications for Spandrel Beams</b>	<b>Closed</b>	<b>CR</b>	<b>04/09/2014</b>	<b>04/19/2014</b>	<b>04/22/2014</b>
<b>From:</b> Webcor Construction LP		Gregory Kemerer				
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached CD RFI # 374 SK1 & SK2 for items 2 to 4, as a follow up to the review comments provided in Submittal Package TG0701-075 SSS - Structural Steel Shop DWG (CS3) GL 12-13:		See attached CD RFI # 374 SK1 & SK2 for items 2 to 4, as a follow up to the review comments provided in Submittal Package TG0701-075 SSS - Structural Steel Shop DWG (CS3) GL 12-13:				
1) [Item 1 has been responded to internally by Skanska. No further action is required]		1) [Item 1 has been responded to internally by Skanska. No further action is required]				
2a) The approver's reference to T-0923 has been superseded by follow-up RFI T-0923.1. This RFI confirmed 2'-0 1/4 to match the W-1 Rhino model. Please		2a) The approver's reference to T-0923 has been superseded by follow-up RFI T-0923.1. This RFI confirmed 2'-0 1/4 to match the W-1 Rhino model.				



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T-1311	<p>SSS - Rolled Pipe Seam Location at Basket Columns</p> <p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>Reference Submittal Package TG0701-075 SSS - Structural Steel Shop DWG (CS3) GL 12-13, sheet 1205 that indicates Skanska is to "confirm seam locations for rolled members." AESS specification section 05 12 14 only provides criteria to fully shape rolled members in shop to final curved shape (2.3.C.12) and to minimize distortion (3.7.C.1.a). No direction is provided in the specifications or contract drawings to orient the seam in any particular direction.</p> <p>Please advise if a specific orientation of rolled members is to be accommodated for upper basket column pipes (AESS Category 3) with wall thickness less than or equal to 1". Note that the seam location must not come in conflict with erection aids or other attachments. See drawing 1268 attached which indicates the orientation of erections aids.</p>	Closed	CR	04/09/2014	04/19/2014	04/16/2014
	<p>Please confirm the RFI response to T-0923.1 remains valid.</p> <p>2b) Not all CP6 connections are located 2'-0 1/4 below the top of steel on the Bus Deck Level. The dimensions shown on this drawing match the W-1 Rhino model. Please confirm that the dimensions provided in the Rhino model are to be followed and that no further action is required.</p> <p>3) Refer to RFI T-0738, which confirmed modifications to the plate thicknesses. Confirm no further action is required.</p> <p>4) The requested cope dimensions are shown. Confirm no further action is required.</p>					
	<p>Please confirm the RFI response to T-0923.1 remains valid.</p> <p>2b) Not all CP6 connections are located 2'-0 1/4 below the top of steel on the Bus Deck Level. The dimensions shown on this drawing match the W-1 Rhino model. Please confirm that the dimensions provided in the Rhino model are to be followed and that no further action is required.</p> <p>3) Refer to RFI T-0738, which confirmed modifications to the plate thicknesses. Confirm no further action is required.</p> <p>4) The requested cope dimensions are shown. Confirm no further action is required.</p>					
T-1312	<p>SSS - Hanger Above Connection Clarification</p>	Closed	CR	04/09/2014	04/19/2014	04/21/2014



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<b>From:</b> Webcor Construction LP		Gregory Kemerer				
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached CD RFI # 390 SK1 for items 1 & 2:		See attached CD RFI # 390 SK1 for items 1 & 2:				
1) Confirm a 2" gap is acceptable.		1) Confirm a 2" gap is acceptable.				
2) The bolts cannot be located as shown in detail 4/S1-5026. Confirm the bolt locations as shown are acceptable with the bottom flange of the beams coped for bolt access. If not, supply an alternate solution.		2) The bolts cannot be located as shown in detail 4/S1-5026. Confirm the bolt locations as shown are acceptable with the bottom flange of the beams coped for bolt access. If not, supply an alternate solution.				
<b>T-1313</b>	<b>SSS - Deck Support Detail at Column</b>	<b>Closed</b>	<b>CR</b>	<b>04/09/2014</b>	<b>04/19/2014</b>	<b>04/21/2014</b>
<b>From:</b> Webcor Construction LP		Gregory Kemerer				
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please reference detail 1/S1-5001. The L3x3x12ga deck support angle is shown coping into the web of the girder. At the majority of the conditions where this may occur the girders have thick and wide flanges. After the angle is coped as detailed we will have very little material to work with and is unlikely to have much structural integrity.		Please reference detail 1/S1-5001. The L3x3x12ga deck support angle is shown coping into the web of the girder. At the majority of the conditions where this may occur the girders have thick and wide flanges. After the angle is coped as detailed we will have very little material to work with and is unlikely to have much structural integrity.				
Please see attached alternative sketch. In this condition the angle is lapped on top of the girder and connected with fillet welds. The depth of the girder has no impact on the detail. This is a common detail in the metal decking industry.		Please see attached alternative sketch. In this condition the angle is lapped on top of the girder and connected with fillet welds. The depth of the girder has no impact on the detail. This is a common detail in the metal decking industry.				
Please confirm the proposed column angle support sketch is acceptable.		Please confirm the proposed column angle support sketch is acceptable.				







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5052.	Please advise.		5052.			
			Please advise.			
<b>T-1318</b>	<b>SSS - Second Level Protected Zone Connection Clarification GL 7</b>	<b>Closed</b>	<b>CR</b>	<b>04/11/2014</b>	<b>04/21/2014</b>	<b>04/22/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b> See attached CD RFI # 381 SK1 & SK2 for items 1 & 2: 1) The noted brace connection occurs within the "Protected Zone" per 1/S1-4201. Confirm that is acceptable or supply an alternate solution. 2) The gusset plate per 8/S1-5015 will foul the top bolts in the double angle connection per 1/S1-5010. Confirm it is acceptable to use a shear plate connection per 1/S1-5011 at this location or supply an alternate solution.			<b>ANSWER:</b> See attached CD RFI # 381 SK1 & SK2 for items 1 & 2: 1) The noted brace connection occurs within the "Protected Zone" per 1/S1-4201. Confirm that is acceptable or supply an alternate solution. 2) The gusset plate per 8/S1-5015 will foul the top bolts in the double angle connection per 1/S1-5010. Confirm it is acceptable to use a shear plate connection per 1/S1-5011 at this location or supply an alternate solution.			
<b>T-1318.1</b>	<b>SSS - Second Level Bent Shear Plate Around Protected Zone</b>	<b>Closed</b>	<b>CR</b>	<b>05/07/2014</b>	<b>05/17/2014</b>	<b>05/21/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 381.1 SK1 & SK2 for items 1 & 2: 1) The bent plate shown on TT SK-0345 cannot be bent with a sharp 90 degree bend as shown. Confirm the cold bending radius shown is acceptable or supply the bending radius to be used. 2) Confirm the welding as shown is acceptable.			<b>ANSWER:</b> See attached CD RFI # 381.1 SK1 & SK2 for items 1 & 2: 1) The bent plate shown on TT SK-0345 cannot be bent with a sharp 90 degree bend as shown. Confirm the cold bending radius shown is acceptable or supply the bending radius to be used. 2) Confirm the welding as shown is acceptable.			





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T-1319	<b>SSS - Missing beam sizes and connection clarification Ground Level GL 1.4</b>  From: Webcor Construction LP                      Stephanie Azzolino  <b>REQUEST:</b>  See attached CD RFI # 380 SK1 for items 1, 2 & 3: 1) Confirm the noted missing beam size is W12x14. 2) Confirm the noted missing beam size is W16x26. 3) Confirm the PL2x3/8 per 12/S1-5010 may terminate as shown to avoid fouling the connection for the W12x14 per 1/S1-5010. If not, supply a new detail.	Closed	CR	04/11/2014	04/21/2014	04/21/2014
	<b>ANSWER:</b>  See attached CD RFI # 380 SK1 for items 1, 2 & 3: 1) Confirm the noted missing beam size is W12x14. 2) Confirm the noted missing beam size is W16x26. 3) Confirm the PL2x3/8 per 12/S1-5010 may terminate as shown to avoid fouling the connection for the W12x14 per 1/S1-5010. If not, supply a new detail.					
T-1320	<b>BGP - Dewatering Sleeve at Mat Slab Depression</b>  From: Webcor Construction LP                      Claude Titché  <b>REQUEST:</b>  Grace requires that there be a minimum 8" clear horizontal plane to allow for the waterproofing membrane transition. Dewatering sleeve #38 is located on the edge of a sloped mat slab depression. Please confirm it is acceptable to lower the flat the mud slab to provide a sufficient horizontal plane to accommodate waterproofing. Sides of depressions will be sloped at 45deg.	Open	01	04/14/2014	04/24/2014	04/16/2014
	<b>ANSWER:</b>  Grace requires that there be a minimum 8" clear horizontal plane to allow for the waterproofing membrane transition. Dewatering sleeve #38 is located on the edge of a sloped mat slab depression. Please confirm it is acceptable to lower the flat the mud slab to provide a sufficient horizontal plane to accommodate waterproofing. Sides of depressions will be sloped at 45deg.					
T-1321	<b>BGP - Geothermal Riser Location Field 14</b>  From: Webcor Construction LP                      Claude Titché  <b>REQUEST:</b>  Please confirm the riser for Geothermal Field 14 is to be located between Soldier Piles 349 and 350. In addition, please confirm the temperature probe in Geothermal Field 14 is to be located between Soldier Piles 350 and 351.	Closed	01	04/14/2014	04/24/2014	04/22/2014
	<b>ANSWER:</b>  Please confirm the riser for Geothermal Field 14 is to be located between Soldier Piles 349 and 350. In addition, please confirm the temperature probe in Geothermal Field 14 is to be located between Soldier Piles 350 and 351.					
T-1322	<b>SSS - West Zone Bus Level and Roof Level Grade Clarifications</b>  From: Webcor Construction LP                      Stephanie Azzolino  <b>REQUEST:</b>  See attached CD RFI # 383 SK1 to SK3 for items 1 to 3: Drawings A1-2502 (SK1), A1-2892 (SK2), A1-2951 (SK4), and 1/A1-2951 (SK5) show a valley. A valley is not shown	Closed	CR	04/14/2014	04/24/2014	04/25/2014
	<b>ANSWER:</b>  See attached CD RFI # 383 SK1 to SK3 for items 1 to 3: Drawings A1-2502 (SK1), A1-2892 (SK2), A1-2951					





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	<p>on S1-2502 and S1-2602 and it is not clear what the intent is for the structural framing:</p> <p>1) In order to keep the diagonal brace framing along Grid H in one plane (shown in blue), it is necessary to introduce a valley as indicated by the red line. If not, twisting will be introduced into the framing and connection details 3/S15025 at Grid 1.4 and 1/S1-5018 at Grid 2. Per Note 6 on S1-2602 the beams are canted to match the slope of TPG1 on Grid 2 but the BU-40 on Grid 1 is horizontal. This will result in the beams having to twist. To avoid this a valley as shown on the architectural drawings is required. Please confirm a valley as indicated by the red line is required or clarify/supply the top of steel elevations between grids 1-2.</p> <p>2) If a valley is introduced, all beams crossing the red valley line will need to be broken-back/bent beams. Please supply a detail showing the splice at the bend lines.</p> <p>3) If the valley is to occur, please clarify the location of the valley as drawings A1-2502 (SK1) and A1-2892 (SK2) show conflicting information. The same condition shown here also occurs between Grids 1-2/B-D, 32.4-33.5/B-D &amp; 32.433.5/F-H. The same condition except opposite shown here occurs between Grids 1-2/F-H on the roof park level.</p> <p>4) It is not clear on S1-2606/S1-2607 where the east-west slopes start east of Grid 31.7. Please clarify the framing.</p>					<p>(SK4), and 1/A1-2951 (SK5) show a valley. A valley is not shown on S1-2502 and S1-2602 and it is not clear what the intent is for the structural framing:</p> <p>1) In order to keep the diagonal brace framing along Grid H in one plane (shown in blue), it is necessary to introduce a valley as indicated by the red line. If not, twisting will be introduced into the framing and connection details 3/S15025 at Grid 1.4 and 1/S1-5018 at Grid 2. Per Note 6 on S1-2602 the beams are canted to match the slope of TPG1 on Grid 2 but the BU-40 on Grid 1 is horizontal. This will result in the beams having to twist. To avoid this a valley as shown on the architectural drawings is required. Please confirm a valley as indicated by the red line is required or clarify/supply the top of steel elevations between grids 1-2.</p> <p>2) If a valley is introduced, all beams crossing the red valley line will need to be broken-back/bent beams. Please supply a detail showing the splice at the bend lines.</p> <p>3) If the valley is to occur, please clarify the location of the valley as drawings A1-2502 (SK1) and A1-2892 (SK2) show conflicting information. The same condition shown here also occurs between Grids 1-2/B-D, 32.4-33.5/B-D &amp; 32.433.5/F-H. The same condition except opposite shown here occurs between Grids 1-2/F-H on the roof park level.</p> <p>4) It is not clear on S1-2606/S1-2607 where the east-west slopes start east of Grid 31.7. Please clarify the framing.</p>
T-1322.1	SSS - West Zone Bus Level and Roof Level Grade Clarifications	Closed	CR	05/16/2014	05/26/2014	06/02/2014
From: Webcor Construction LP		Gregory Kemerer				
REQUEST:		ANSWER:				
See attached CD RFI # 383.1 SK1 to SK6 for items 1 to 10: The noted (2) skewed W40x277 form part of the bracing system and must remain in the same as the other braces.  This means that the noted W40x149 & W36x231 will not align with the top of the W40x297 on Grid 1. 1) Please advise how to make the moment weld for the for the W40x149 to the W40x297 with the flanges not aligned.		See attached CD RFI # 383.1 SK1 to SK6 for items 1 to 10: The noted (2) skewed W40x277 form part of the bracing system and must remain in the same as the other braces. This means that the noted W40x149 & W36x231 will not align with the top of the W40x297 on Grid 1. 1) Please advise how to make the moment weld for the for the W40x149 to the W40x297 with the flanges				



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	<p>2) Please supply a detail showing how the deck will be supported at the (2) lowered beams.</p> <p>3) Due to the beams not being aligned, confirm it is acceptable to connect these beams with shear plates per 1/S1-5011</p> <p>4) Confirm it is typically acceptable to lower the beams to align the corner of the flange with the top of the supporting beams as some connections are moment connections.</p> <p>5) Review all items on SK2 and confirm they meet the design intent.</p> <p>6) The noted (2) skewed W40x277 form part of the bracing system and must remain in the same as the other braces. This means that the (4) noted beams will not align with the top of the W40x297 on Grid 33.5. Please supply a detail showing how the deck will be supported at the (4) lowered beams.</p> <p>7) Confirm the T/Steel elevation at this location is EL. 55'-8 9/16 based on the north/south slope to keep the diagonal bracing in the same plane.</p> <p>8) Confirm the noted (2) beams will be canted to match the other beams based on the slope on TPG1 on Grid 2.</p> <p>9) Review all items on SK5 and confirm they meet the design intent.</p> <p>10) This is to confirm the response in RFI SK515 item 4: All beams within the blue boundaries on SK6 will slope as needed the have the top of beams flush with the top of the supporting beams per the noted T/Steel elevations. The east/west beams between Grids 32.4-33.2 will be canted to match the canted beams west of Grid 32.4.The east/west beams between Grids 33.2-33.5 will be canted to match the slope of the beams on Grid 33.2. Please confirm.</p>					<p>not aligned.</p> <p>2) Please supply a detail showing how the deck will be supported at the (2) lowered beams.</p> <p>3) Due to the beams not being aligned, confirm it is acceptable to connect these beams with shear plates per 1/S1-5011</p> <p>4) Confirm it is typically acceptable to lower the beams to align the corner of the flange with the top of the supporting beams as some connections are moment connections.</p> <p>5) Review all items on SK2 and confirm they meet the design intent.</p> <p>6) The noted (2) skewed W40x277 form part of the bracing system and must remain in the same as the other braces. This means that the (4) noted beams will not align with the top of the W40x297 on Grid 33.5. Please supply a detail showing how the deck will be supported at the (4) lowered beams.</p> <p>7) Confirm the T/Steel elevation at this location is EL. 55'-8 9/16 based on the north/south slope to keep the diagonal bracing in the same plane.</p> <p>8) Confirm the noted (2) beams will be canted to match the other beams based on the slope on TPG1 on Grid 2.</p> <p>9) Review all items on SK5 and confirm they meet the design intent.</p> <p>10) This is to confirm the response in RFI SK515 item 4: All beams within the blue boundaries on SK6 will slope as needed the have the top of beams flush with the top of the supporting beams per the noted T/Steel elevations. The east/west beams between Grids 32.4-33.2 will be canted to match the canted beams west of Grid 32.4.The east/west beams between Grids 33.2-33.5 will be canted to match the slope of the beams on Grid 33.2. Please confirm.</p>
T-1323	SSS - Bus Deck Level Slab Clarification	Closed	CR	04/14/2014	04/24/2014	04/22/2014
From: Webcor Construction LP		Stephanie Azzolino				
REQUEST:		ANSWER:				
Bus Deck Level drawings S1-2502 thru S1-2507 indicate an S8 type slab, as per metal deck schedule 2/S1-5000, S8 is a 10" structural slab with 4" second pour topping		Bus Deck Level drawings S1-2502 thru S1-2507 indicate an S8 type slab, as per metal deck schedule 2/S1-5000, S8 is a 10" structural slab with 4" second				



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	<p>slab. Sheet note #6 on S1-2502 indicates "shear studs are to extend a minimum of 2" into the second pour UON. See detail 9/S1-9004 for perimeter EoS conditions" (S1-9004 has not been provided).</p> <p>Drawings A1-2892 thru 2987 show a 1" topping slab typically between 10' north of GL D and 10' south of GL F. The slab outside of that region is typically shown as structural slab only.</p> <p>Confirm that decking drawings will be modeled as per the information provided on the structural contract drawings unless otherwise directed.</p>					<p>pour topping slab. Sheet note #6 on S1-2502 indicates "shear studs are to extend a minimum of 2" into the second pour UON. See detail 9/S1-9004 for perimeter EoS conditions" (S1-9004 has not been provided).</p> <p>Drawings A1-2892 thru 2987 show a 1" topping slab typically between 10' north of GL D and 10' south of GL F. The slab outside of that region is typically shown as structural slab only.</p> <p>Confirm that decking drawings will be modeled as per the information provided on the structural contract drawings unless otherwise directed.</p>
T-1324	SSS - BRB Clevis Plate Detail	Closed	CR	04/14/2014	04/24/2014	04/24/2014
From: Webcor Construction LP Stephanie Azzolino						
REQUEST:						ANSWER:
Skanska is the process of incorporating the AAN BRB Shop Drawings from Star Seismic into the Tekla model.						Skanska is the process of incorporating the AAN BRB Shop Drawings from Star Seismic into the Tekla model.
During the integration process we have identified a deviation from certain dimensions shown in the contract documents. Specifically, the 2" dimension shown from edge of clevis to top of concrete at the ground level connection and the 5" dimension shown from edge of clevis to the bottom flange of W section above.						During the integration process we have identified a deviation from certain dimensions shown in the contract documents. Specifically, the 2" dimension shown from edge of clevis to top of concrete at the ground level connection and the 5" dimension shown from edge of clevis to the bottom flange of W section above.
Below, is a table showing the "As Detailed" dimensions at each BRB.						Below, is a table showing the "As Detailed" dimensions at each BRB.
Please confirm that the 2" & 5" dimensions shown on S1-4206 are not controlling dimensions and the lengths of the BRB's from WP-WP as designed, detailed and approved, control.						Please confirm that the 2" & 5" dimensions shown on S1-4206 are not controlling dimensions and the lengths of the BRB's from WP-WP as designed, detailed and approved, control.
T-1324.1	SSS - BRB Clevis Plate Detail	Closed	CR	05/08/2014	05/18/2014	05/21/2014
From: Webcor Construction LP Gregory Kemerer						





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<i>Number</i>	<i>Subject</i>	<i>Status</i>	<i>Choice Required</i>	<i>Date Created</i>	<i>Date Required</i>	<i>Date Answered</i>
T-1326	BGP - Vehicle Ramp End Support Beams	Closed	01	04/16/2014	04/26/2014	04/22/2014
<div><div>From: Webcor Construction LP</div><div>Claude Titcher</div></div>						
REQUEST:			ANSWER:			
Please reference AI-7401 rev 4 and SI-2251 rev 5. Please also reference RFI T-0835			Please reference AI-7401 rev 4 and SI-2251 rev 5. Please also reference RFI T-0835			
1. S1-2251 shows additional beams added to vehicle ramp that intersect the South and West foundation walls. A 1-7401 does not appear to show the same quantity of beams intersecting the south perimeter wall. Please supply a revised architectural drawing or detail that shows any/all beams that are added from S1-2251, and that shows the angles at which those beams intersect walls.			1. S1-2251 shows additional beams added to vehicle ramp that intersect the South and West foundation walls. A 1-7401 does not appear to show the same quantity of beams intersecting the south perimeter wall. Please supply a revised architectural drawing or detail that shows any/all beams that are added from S1-2251, and that shows the angles at which those beams intersect walls.			
2. Please confirm AI-7401 revises the beam intersecting angles that were provided in RFI T-0835.			2. Please confirm AI-7401 revises the beam intersecting angles that were provided in RFI T-0835.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1327</b>	<b>BGP - Lower Concourse Beam Locations</b>	<b>Closed</b>	<b>01</b>	<b>04/16/2014</b>	<b>04/26/2014</b>	<b>04/19/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please confirm the bellow structural beam updates should be incorporated into the TG06 Scope of work.			Please confirm the bellow structural beam updates should be incorporated into the TG06 Scope of work.			
<p>1. CB24 is required between Grids 4 and 5. Beam centerline to be located 12'-3" south of Grid C.3.</p> <p>2. B1 shown west of Grid 5 at approximately Grid B.6 is not required and can be deleted from scope.</p> <p>3. The pair of B4's shown east of Grid 5 near Grid B are not required (opening has been removed). Beams can be deleted from scope.</p> <p>4. B24 is required between Grids 6 and 7. B23 is required between Grids 7 and 8. South face of both beams shall be located approximately 27" north of Grid E.6 and align with the northern most face of trestle pile blockouts in order that they not be interrupted by the blockouts. The coordinated location of the beam with blockouts will be submitted on the comprehensive layout drawings. RFI T-1040 response is superseded as is RFI T-876. Section 2/S1-3501 does not apply. SKS-0343(attached) will detail the section across drop.</p> <p>5. B25 is required between Grids 7 and 8 near Grid B.6. Beam will align with B29 to the west.</p> <p>6. CB15 is required between Grids 10.1 and 11 near Grid C.7. Beam centerline to be located 10'-4 north of Grid D.</p> <p>7. Beam between Grids 13 and 14 near Grid B.5 will frame escalator opening and be marked B12. Beam will be located at edge of opening shown on A1-2844. To minimize potential congestion at overlapping beams, beam between Grids 12 and 13 near Grid B.6 marked B9 will be located such that the north face of beam aligns with the south face of adjacent beam B12.</p> <p>8. B15 is required between Grids 14 and 15, 5'-10" south of Grid B.</p> <p>9. B53 is required between Grids 20.1 and 21 near Grid C.4 at the north face of the escalator pit. A B53 is also required at the south face of escalator pit. B4A is required at the west face of escalator pit between B53's. Companion B54's between Grids 21 and 22 will align with B53's. Beams will be located per pit dimensions given on A1-2845.</p> <p>10. CB8 is required between Grids 33.2 and 34. Beam centerline will be located 6'-4" south of Grid G.</p>			<p>1. CB24 is required between Grids 4 and 5. Beam centerline to be located 12'-3" south of Grid C.3.</p> <p>2. B1 shown west of Grid 5 at approximately Grid B.6 is not required and can be deleted from scope.</p> <p>3. The pair of B4's shown east of Grid 5 near Grid B are not required (opening has been removed). Beams can be deleted from scope.</p> <p>4. B24 is required between Grids 6 and 7. B23 is required between Grids 7 and 8. South face of both beams shall be located approximately 27" north of Grid E.6 and align with the northern most face of trestle pile blockouts in order that they not be interrupted by the blockouts. The coordinated location of the beam with blockouts will be submitted on the comprehensive layout drawings. RFI T-1040 response is superseded as is RFI T-876. Section 2/S1-3501 does not apply. SKS-0343(attached) will detail the section across drop.</p> <p>5. B25 is required between Grids 7 and 8 near Grid B.6. Beam will align with B29 to the west.</p> <p>6. CB15 is required between Grids 10.1 and 11 near Grid C.7. Beam centerline to be located 10'-4 north of Grid D.</p> <p>7. Beam between Grids 13 and 14 near Grid B.5 will frame escalator opening and be marked B12. Beam will be located at edge of opening shown on A1-2844. To minimize potential congestion at overlapping beams, beam between Grids 12 and 13 near Grid B.6 marked B9 will be located such that the north face of beam aligns with the south face of adjacent beam B12.</p> <p>8. B15 is required between Grids 14 and 15, 5'-10" south of Grid B.</p> <p>9. B53 is required between Grids 20.1 and 21 near Grid C.4 at the north face of the escalator pit. A B53 is also required at the south face of escalator pit. B4A is required at the west face of escalator pit between B53's. Companion B54's between Grids 21 and 22 will align with B53's. Beams will be located per pit dimensions given on A1-2845.</p> <p>10. CB8 is required between Grids 33.2 and 34. Beam centerline will be located 6'-4" south of Grid G.</p>			
Beam locations will be submitted for review with the comprehensive layouts.						



Webcor/Obayashi Joint Venture  
PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG  
30100 - Transbay Transit Center Project

Page: 1420 of 2218  
Date: 02/11/2015  
Time: 07:01 AM  
Job: 30100

<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
Beam locations will be submitted for review with the comprehensive layouts.						
<b>T-1328</b>	<b>BGP - Vehicle Ramp End Support Embeds</b>	<b>Closed</b>	<b>01</b>	<b>04/16/2014</b>	<b>04/26/2014</b>	<b>04/21/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Please reference SI-2251 rev 6, SI-3401 rev 7and S1-3411 rev 3.  1. Clouded area of attached SI-2251 shows beam types: B125, B132, B141 and B142. B125 is a 24" wide beam, B132 is a 22" wide beam, B141 is a 48" wide beam and B142 is a 30" wide beam. SI-3411 DI & DI0 depict the beam support embed. These two (2) details are for a 24" wide beam and a 48" wide beam only. Please provide details for 22" and 30" wide beams.  2. See attached SI-3411 DI & DI0. Embed details call out 1-1/4" threaded rod F1554 GR55 w/ 2-1/2" x 2-1/2" x 1/2" PL washer. Please clarify how the plate washer will be attached to the threaded rod and/or embed? Will the designer require any type of nut or weld that is not depicted?		<b>ANSWER:</b> Please reference SI-2251 rev 6, SI-3401 rev 7and S1-3411 rev 3.  1. Clouded area of attached SI-2251 shows beam types: B125, B132, B141 and B142. B125 is a 24" wide beam, B132 is a 22" wide beam, B141 is a 48" wide beam and B142 is a 30" wide beam. SI-3411 DI & DI0 depict the beam support embed. These two (2) details are for a 24" wide beam and a 48" wide beam only. Please provide details for 22" and 30" wide beams.  2. See attached SI-3411 DI & DI0. Embed details call out 1-1/4" threaded rod F1554 GR55 w/ 2-1/2" x 2-1/2" x 1/2" PL washer. Please clarify how the plate washer will be attached to the threaded rod and/or embed? Will the designer require any type of nut or weld that is not depicted?				
<b>T-1329</b>	<b>BGP - Lower Concourse Beam Locations, Added Beam</b>	<b>Closed</b>	<b>01</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/21/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Please confirm the below structural beam update should be incorporated into the TG06 Scope of work.  1. CB15 is required between Grids 14 and 15 near Grid C.3. Beam centerline to be located 7'-1 ¼" south of Grid C.3.  Beam locations will be submitted for review with the comprehensive layouts.		<b>ANSWER:</b> Please confirm the below structural beam update should be incorporated into the TG06 Scope of work.  1. CB15 is required between Grids 14 and 15 near Grid C.3. Beam centerline to be located 7'-1 ¼" south of Grid C.3.  Beam locations will be submitted for review with the comprehensive layouts.				











<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1331.1</b>	<b>SSS - Missing Stair ST301 information</b>	<b>Closed</b>	<b>CR</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>05/29/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  This is a follow up to Skanska RFI 521 which was sent on April 14, 2014. Please answer question #6 below which was not answered in the RFI response T-1331.  See attached CD RFI # 389 SK1 to SK7 for item 6:  6) The plan shows a moment connection but detail 3/S1-5012 does not. Please clarify the intent for the beam to post connections at (8) locations.						<b>ANSWER:</b>  This is a follow up to Skanska RFI 521 which was sent on April 14, 2014. Please answer question #6 below which was not answered in the RFI response T-1331.  See attached CD RFI # 389 SK1 to SK7 for item 6:  6) The plan shows a moment connection but detail 3/S1-5012 does not. Please clarify the intent for the beam to post connections at (8) locations.
<b>T-1332</b>	<b>SSS - Offset Connection Details at PE301-302</b>	<b>Closed</b>	<b>CR</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/28/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 392 SK1 & SK2 for items 1 to 7: 1) Confirm connection at offset beams as was typically requested in RFI T-1299. 2) Confirm connection at offset beams as was typically requested in RFI T-1299. 3) Confirm connection at offset beams. 4) Confirm connection at offset beams as was typically requested in RFI T-1299. 5) Confirm the short W16x26 beam on the west side of Grid 8 may be relocated to align with the W30x108. 6) Connections will foul. Supply an alternate detail. 7) Connections will foul. Supply an alternate detail.						<b>ANSWER:</b>  See attached CD RFI # 392 SK1 & SK2 for items 1 to 7: 1) Confirm connection at offset beams as was typically requested in RFI T-1299. 2) Confirm connection at offset beams as was typically requested in RFI T-1299. 3) Confirm connection at offset beams. 4) Confirm connection at offset beams as was typically requested in RFI T-1299. 5) Confirm the short W16x26 beam on the west side of Grid 8 may be relocated to align with the W30x108. 6) Connections will foul. Supply an alternate detail. 7) Connections will foul. Supply an alternate detail.
<b>T-1333</b>	<b>SSS - Bolt Edge Distance at Weld Access Hole</b>	<b>Closed</b>	<b>CR</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/28/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 395 SK1 & SK2: The weld access hole as shown on SK1 will create an insufficient edge distance for the 1 1/2" diameter bolt at this location. Please confirm this will be acceptable as modeled.						<b>ANSWER:</b>  See attached CD RFI # 395 SK1 & SK2: The weld access hole as shown on SK1 will create an insufficient edge distance for the 1 1/2" diameter bolt at this location. Please confirm this will be acceptable as modeled.



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1334</b>	<b>SSS - Curved Vertical E.O.S. Plate Connection at Light Column</b>	<b>Closed</b>	<b>CR</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/28/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached detail titled, "Built-Up Plate Assembly" (Skanska RFI SK1) for radius plate assembly. Confirm this is acceptable.						<b>ANSWER:</b> See attached detail titled, "Built-Up Plate Assembly" (Skanska RFI SK1) for radius plate assembly. Confirm this is acceptable.
<b>T-1335</b>	<b>SSS - Roof Deck HSS Bracing</b>	<b>Closed</b>	<b>CR</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/28/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 319.1 SK1 to SK3 for items 1 to 4:  1) Confirm this general layout locating the shop splices is acceptable. 2) Confirm dimensions. 3) Confirm dimension. 4) Confirm location points for 1/2x3 stiffeners.						<b>ANSWER:</b> See attached CD RFI # 319.1 SK1 to SK3 for items 1 to 4:  1) Confirm this general layout locating the shop splices is acceptable. 2) Confirm dimensions. 3) Confirm dimension. 4) Confirm location points for 1/2x3 stiffeners.
<b>T-1336</b>	<b>SSS - API 5L X65 Pipe to A216 WCC Casting (Field Weld ) PQR</b>	<b>Closed</b>	<b>CR</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/29/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Skanska is currently procuring weld test plates for qualifying field welds to API-5L Grade X65 pipe. For this, we propose to use API 2W plate. Whereas API 5L is strictly a pipe specification, API 2W is a plate specification but is commonly rolled and welded to create API 5L pipe. Below is a direct comparison between API 5L Gr. X65 allowable chemical and mechanical properties with our proposed API-2W plate material. Values for the plate are taken directly from the MTR for the plate we propose to use (attached and highlighted). All values for the API 2W plate we propose to use meet the requirements of the API 5L Grade X65 specification. Please confirm it is acceptable to use the API 2W plate for our PQRs.						<b>ANSWER:</b> Skanska is currently procuring weld test plates for qualifying field welds to API-5L Grade X65 pipe. For this, we propose to use API 2W plate. Whereas API 5L is strictly a pipe specification, API 2W is a plate specification but is commonly rolled and welded to create API 5L pipe. Below is a direct comparison between API 5L Gr. X65 allowable chemical and mechanical properties with our proposed API-2W plate material. Values for the plate are taken directly from the MTR for the plate we propose to use (attached and highlighted). All values for the API 2W plate we propose to use meet the requirements of the API 5L Grade X65 specification. Please confirm it is acceptable to use the API 2W plate for our PQRs.



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1337</b>	<b>SSS - Train Box Column Cap Plate at GL18</b>	<b>Closed</b>	<b>CR</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/29/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> The Train Box Column cap plates at GL 18 as shown on 5 A/ S1-5050 have been ordered and modeled as 3-1/2" thick plate instead of the 3-1/4" shown. The column flange and web length have been adjusted accordingly. Please confirm it is acceptable to proceed with the 3-1/2" thick cap plate.						<b>ANSWER:</b> The Train Box Column cap plates at GL 18 as shown on 5A/ S1-5050 have been ordered and modeled as 3-1/2" thick plate instead of the 3-1/4" shown. The column flange and web length have been adjusted accordingly. Please confirm it is acceptable to proceed with the 3-1/2" thick cap plate.
<b>T-1338</b>	<b>SSS - Gridline 18 - Gravity Moment Connections</b>	<b>Closed</b>	<b>CR</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/25/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> At the Roof Park level drawing S1-2604 at near grids F/18 & D/18 detail 8/S1-5032 is called out at the noted locations shown on sketches CD RFI 394 SK1 to SK3. Due to the large tapered girder and the requirement of a moment connection at these locations the doubler plate size has been modified to allow for the 11/16" all around fillet weld called for in detail 8/S1-5032. 1) Confirm that doubler plate modifications shown on CD RFI 394 SK2 are allowed. 2) Confirm that doubler plate modifications shown on CD RFI 394 SK3 are allowed.						<b>ANSWER:</b> At the Roof Park level drawing S1-2604 at near grids F/18 & D/18 detail 8/S1-5032 is called out at the noted locations shown on sketches CD RFI 394 SK1 to SK3. Due to the large tapered girder and the requirement of a moment connection at these locations the doubler plate size has been modified to allow for the 11/16" all around fillet weld called for in detail 8/S1-5032. 1) Confirm that doubler plate modifications shown on CD RFI 394 SK2 are allowed. 2) Confirm that doubler plate modifications shown on CD RFI 394 SK3 are allowed.
<b>T-1339</b>	<b>SSS - Deck Support Steel at Step in Slab GL 12-14</b>	<b>Closed</b>	<b>CR</b>	<b>04/17/2014</b>	<b>04/27/2014</b>	<b>04/30/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Refer to SK-1 thru SK-3 for 3 locations at step in slab between GL 12 - 14: 1) At section A-A on SK-2 an extended WT will expose the flange through the concrete. Confirm a bent plate as per SK-3 is acceptable. 2) At section B-B on SK-2 support is required at the extended upper deck over the W12x14 (A4987). Confirm a WT can be added per section B-B.						<b>ANSWER:</b> Refer to SK-1 thru SK-3 for 3 locations at step in slab between GL 12 - 14: 1) At section A-A on SK-2 an extended WT will expose the flange through the concrete. Confirm a bent plate as per SK-3 is acceptable. 2) At section B-B on SK-2 support is required at the extended upper deck over the W12x14 (A4987). Confirm a WT can be added per section B-B.

<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
<b>T-1339.1</b>	<b>SSS - Deck Support Steel at Step in Slab</b>	<b>Closed</b>	<b>CR</b>	<b>05/23/2014</b>	<b>06/02/2014</b>	<b>06/13/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  RFI SK534 (T-1339) addressed steps in the ground floor slab which occurred between grid lines 12-14. This condition also occurs between the following grids: 2-3, 3-4, 8-9 & 9-9.9 (16 locations).  Item #1 added a 3/8" bent plate to support the upper level deck and can be applied at two locations between grid 2-3 as indicated on SK1. At the other 14 locations the step is similar to item #2 of T-1339 which added an 18 gauge Z closure. At grid 12-14 the perpendicular support beam is located within 1'-9" of the step, at the 14 locations listed above the support beam varies from 2'-0" to 6'-0" and at two locations at grid 2-3 no support beam is provided. Refer to SK3 for clarification.  Please confirm the 18 gauge Z is sufficient to carry the load of the deck at the 14 locations or provide an alternative detail.			<b>ANSWER:</b>  RFI SK534 (T-1339) addressed steps in the ground floor slab which occurred between grid lines 12-14. This condition also occurs between the following grids: 2-3, 3-4, 8-9 & 9-9.9 (16 locations).  Item #1 added a 3/8" bent plate to support the upper level deck and can be applied at two locations between grid 2-3 as indicated on SK1. At the other 14 locations the step is similar to item #2 of T-1339 which added an 18 gauge Z closure. At grid 12-14 the perpendicular support beam is located within 1'-9" of the step, at the 14 locations listed above the support beam varies from 2'-0" to 6'-0" and at two locations at grid 2-3 no support beam is provided. Refer to SK3 for clarification.  Please confirm the 18 gauge Z is sufficient to carry the load of the deck at the 14 locations or provide an alternative detail.			

Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-1339.2	SSS - Deck Support Steel at Step in Slab	Closed	CR	06/18/2014	06/28/2014	07/09/2014
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>The response to RFI T-1399.1 indicated the zee closure was incorrectly shown on SK-3 and shall be supported on the lower beam, not on top of the deck. We agree the closure would be best supported on the lower beam, however the closure was placed on top of the deck so that the lower deck could bear 3" on the beam as is required for a type 3 deck as per 2/S1-5000.</p> <p>SSK-0355 revises the closure so that the bottom leg is underneath the lower deck and the top leg is above the upper deck. This creates several problems for installation. The most significant being that the closure now needs to be installed before the deck. Typically closures are installed after the deck so that the deck can be used as a working platform. SSK-0355 would require the installation of the closure on open iron, which while possible is not a safe working practice.</p> <p>Option 1: As a proposed solution to the issues indicated above, we propose to increase the beam size at the step from a W12x14 to W12x26 which has a 6-1/2" wide flange. We would also revise the zee shaped closure to a cee shaped closure with the top leg placed underneath the upper deck (see detail A SK-1). Also at locations where a WT will be added, a W12x14 will only be left with about 1-3/4" of exposed beam flange for bearing, instead of the required 3", increasing this to W14x26 will provide the required bearing (see detail B SK-1).</p> <p>Option 2: As an alternative, at locations where the top deck spans less than 3' the top deck load will be primarily supported by the upper WF beam. As detailed in SK-2 the zee plate will only act as a closure with minimal deck support and can be installed after the deck if allowed to bear on top of the lower deck. Note: this alternative does not provide a solution of the 1-3/4" bearing issue on detail B SK-1.</p> <p>Please confirm if option 1 or 2 is acceptable.</p>		<b>ANSWER:</b> <p>The response to RFI T-1399.1 indicated the zee closure was incorrectly shown on SK-3 and shall be supported on the lower beam, not on top of the deck. We agree the closure would be best supported on the lower beam, however the closure was placed on top of the deck so that the lower deck could bear 3" on the beam as is required for a type 3 deck as per 2/S1-5000.</p> <p>SSK-0355 revises the closure so that the bottom leg is underneath the lower deck and the top leg is above the upper deck. This creates several problems for installation. The most significant being that the closure now needs to be installed before the deck. Typically closures are installed after the deck so that the deck can be used as a working platform. SSK-0355 would require the installation of the closure on open iron, which while possible is not a safe working practice.</p> <p>Option 1: As a proposed solution to the issues indicated above, we propose to increase the beam size at the step from a W12x14 to W12x26 which has a 6-1/2" wide flange. We would also revise the zee shaped closure to a cee shaped closure with the top leg placed underneath the upper deck (see detail A SK-1). Also at locations where a WT will be added, a W12x14 will only be left with about 1-3/4" of exposed beam flange for bearing, instead of the required 3", increasing this to W14x26 will provide the required bearing (see detail B SK-1).</p> <p>Option 2: As an alternative, at locations where the top deck spans less than 3' the top deck load will be primarily supported by the upper WF beam. As detailed in SK-2 the zee plate will only act as a closure with minimal deck support and can be installed after the deck if allowed to bear on top of the lower deck. Note: this alternative does not provide a solution of the 1-3/4" bearing issue on detail B SK-1.</p> <p>Please confirm if option 1 or 2 is acceptable.</p>				
T-1340	BGP - Mechanical Room B2228 Pier/Wall Location	Closed	CR	04/18/2014	04/28/2014	04/30/2014
<b>From:</b> Webcor Construction LP Claude Titche						













<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1347</b>	<b>BGP - Column Jacket 1/2" thick x 4" wide Base Plate at Concourse</b>	<b>Closed</b>	<b>01</b>	<b>04/23/2014</b>	<b>05/03/2014</b>	<b>05/05/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference S 1-3503 rev 2, detail 6, AI-9208 rev 0 and AI-2842 rev 5.			Please reference S 1-3503 rev 2, detail 6, AI-9208 rev 0 and AI-2842 rev 5.			
1. Please see clouded areas of AI-2842. Please confirm 1/2" thick x 4" wide ring base plate is to be continuous around the entire column per details on AI-9208. Please confirm the designer does not want 1/2" ring to be coped where it intersects walls.			1. Please see clouded areas of AI-2842. Please confirm 1/2" thick x 4" wide ring base plate is to be continuous around the entire column per details on AI-9208. Please confirm the designer does not want 1/2" ring to be coped where it intersects walls.			
2. If coping is required, please provide details or direction on coping the 1/2" thick column jacket base plate.			2. If coping is required, please provide details or direction on coping the 1/2" thick column jacket base plate.			
<b>T-1348</b>	<b>BGP - SFPUC Electric Room Copper Mesh</b>	<b>Closed</b>	<b>01</b>	<b>04/23/2014</b>	<b>05/03/2014</b>	<b>04/29/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The response to RFI 1220.1 included E1-3212 which details the embedded grounding required for the SFPUC Electric Rooms. Note 5 indicates that a #6 copper mesh is to be installed.			The response to RFI 1220.1 included E1-3212 which details the embedded grounding required for the SFPUC Electric Rooms. Note 5 indicates that a #6 copper mesh is to be installed.			
Please provide the following information: 1) Type of mesh (Pure Copper or Copper Coated Steel) 2) Required spacing for grid of mesh			Please provide the following information: 1) Type of mesh (Pure Copper or Copper Coated Steel) 2) Required spacing for grid of mesh			
In addition, please provide a drawing with the dimensions to the location of the #2/0 copper pigtails which stub up into the future switchgear enclosures.			In addition, please provide a drawing with the dimensions to the location of the #2/0 copper pigtails which stub up into the future switchgear enclosures.			
<b>T-1348.1</b>	<b>BGP - SFPUC Electric Room Copper Mesh</b>	<b>Closed</b>	<b>01</b>	<b>05/29/2014</b>	<b>06/08/2014</b>	<b>06/04/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference plan sheet EI-3212.			Please reference plan sheet EI-3212.			
1) Note 4 on plan sheet EI-3212 states, "Cadweld main #4/0 grid to #6 mesh with #1 stranded Cu conductor (typical). Refer to Detail 1/EI-3212." Detail 1/EI-3212 does not show a #1 copper conductor connecting the grid to the			1) Note 4 on plan sheet EI-3212 states, "Cadweld main #4/0 grid to #6 mesh with #1 stranded Cu conductor (typical). Refer to Detail 1/EI-3212." Detail 1/EI-3212 does not show a #1 copper conductor			



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	<p>mesh. Please confirm the 4/0 grounding grid is to be connected directly to the grounding mesh.</p> <p>2) Please see the attached Detail 5/EI-3212. Please identify which part of the grounding grid system the highlighted portion belongs to and provide the proper wire size.</p> <p>3) Detail 5/EI/3212 designates the embedded conductors going to the battery rack as #2. Please confirm this means #2 AWG copper conductors. In addition, no designation is provided on Detail 4/EI-3212 at the pigtail which appears to be for the battery rack in B1289. Please confirm this is pigtail is to be #2 A WG conductor as well.</p>					<p>connecting the grid to the mesh. Please confirm the 4/0 grounding grid is to be connected directly to the grounding mesh.</p> <p>2) Please see the attached Detail 5/EI-3212. Please identify which part of the grounding grid system the highlighted portion belongs to and provide the proper wire size.</p> <p>3) Detail 5/EI/3212 designates the embedded conductors going to the battery rack as #2. Please confirm this means #2 AWG copper conductors. In addition, no designation is provided on Detail 4/EI-3212 at the pigtail which appears to be for the battery rack in B1289. Please confirm this is pigtail is to be #2 A WG conductor as well.</p>
T-1348.2	BGP - SFPUC Electric Room Copper Mesh	Closed	01	06/09/2014	06/09/2014	06/17/2014
<div><div><p>From: Webcor Construction LP</p><p>Claude Titcher</p><p>REQUEST:</p><p>The response to RFI T-1348.1 stated the #6 mesh shown on EI-3212/Detail 1 is to be interconnected to the grounding grid at "several locations" and that the #4/0 grid could be "directly connected" where it crosses or contacts the mesh. This response also stated that all other connections are to be #1bare CU connections.</p><p>1) Please provide the exact quantity of connections which are required from the #4/0 grid to the #6 mesh.</p><p>2) Please provide the interval distance between required direct connections from the #4/0 grid to the #6 mesh.</p><p>3) Plan sheet shows six (6) total #1 "remote connections" in each room. Please confirm this is the proper quantity required.</p></div><div><p>ANSWER:</p><p>The response to RFI T-1348.1 stated the #6 mesh shown on EI-3212/Detail 1 is to be interconnected to the grounding grid at "several locations" and that the #4/0 grid could be "directly connected" where it crosses or contacts the mesh. This response also stated that all other connections are to be #1bare CU connections.</p><p>1) Please provide the exact quantity of connections which are required from the #4/0 grid to the #6 mesh.</p><p>2) Please provide the interval distance between required direct connections from the #4/0 grid to the #6 mesh.</p><p>3) Plan sheet shows six (6) total #1 "remote connections" in each room. Please confirm this is the proper quantity required.</p></div></div>						
T-1349	BGP - Beam B52 Trestle Pile Conflict	Closed	01	04/23/2014	05/03/2014	04/23/2014





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<b>T-1351</b>	<b>BGP - Room B2230 Plumbing Opening Conflict</b>	<b>Closed</b>	<b>01</b>	<b>04/24/2014</b>	<b>05/04/2014</b>	<b>05/02/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  See attached drawings that show a B48 beam in direct conflict with the plumbing opening on the South wall of room B2230. Please confirm the location of the plumbing opening.					<b>ANSWER:</b>  See attached drawings that show a B48 beam in direct conflict with the plumbing opening on the South wall of room B2230. Please confirm the location of the plumbing opening.	
<b>T-1352</b>	<b>BGP - Manlift 2 Conflict with EJB and Cast in Strut</b>	<b>Closed</b>	<b>01</b>	<b>04/24/2014</b>	<b>05/04/2014</b>	<b>05/01/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  See attached sketch  The manlift 2 lower concourse blockout is in conflict with an electrical junction box and cast in strut.  Please confirm it is acceptable to move the electrical junction box 6 inches East to clear the blockout and the cast in strut 6 inches North to clear the blockout.					<b>ANSWER:</b>  See attached sketch  The manlift 2 lower concourse blockout is in conflict with an electrical junction box and cast in strut.  Please confirm it is acceptable to move the electrical junction box 6 inches East to clear the blockout and the cast in strut 6 inches North to clear the blockout.	
<b>T-1353</b>	<b>SSS - Specification Clarification - Rejectable Flaws</b>	<b>Void</b>	<b>CR</b>	<b>04/30/2014</b>	<b>05/10/2014</b>	
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  Section 05 10 00 - Structural Steel - 1.8 Quality Assurance by TJPA's Testing Agency - Paragraph D. - Sub. Para. 5 Welding: contains the following provisions:  c. Complete joint penetration welds: Test all complete joint penetration welds for soundness by means of either radiographic or ultrasonic testing in accordance with AWS D1.1 and ASTM E164 procedures. All flaws in plate or flange material revealed during such tests shall be repaired by the Contractor at the Contractor's expense.  d. Partial penetration welds: Test all partial penetration welds for soundness by means of visual and magnetic particle inspection, unless other methods are specified in the Contract Documents. All flaws in plate or flange material revealed					<b>ANSWER:</b>  Section 05 10 00 - Structural Steel - 1.8 Quality Assurance by TJPA's Testing Agency - Paragraph D. - Sub. Para. 5 Welding: contains the following provisions:  c. Complete joint penetration welds: Test all complete joint penetration welds for soundness by means of either radiographic or ultrasonic testing in accordance with AWS D1.1 and ASTM E164 procedures. All flaws in plate or flange material revealed during such tests shall be repaired by the Contractor at the Contractor's expense.  d. Partial penetration welds: Test all partial penetration welds for soundness by means of visual and magnetic particle inspection, unless other methods are specified in the Contract Documents. All flaws in plate or flange material revealed	



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	<p>led during such tests shall be repaired by the Contractor at the Contractor's expense.</p> <p>Please confirm the intent of the specification is to repair "All rejectable flaws in plate or flange material revealed during such tests" in accordance with D1.1 &amp; D1.8 acceptance criteria.</p>					<p>ial revealed during such tests shall be repaired by the Contractor at the Contractor's expense.</p> <p>Please confirm the intent of the specification is to repair "All rejectable flaws in plate or flange material revealed during such tests" in accordance with D1.1 &amp; D1.8 acceptance criteria.</p>
<b>T-1353.1</b>	<b>SSS - Specification Clarification - Rejectable Flaws</b>	<b>Closed</b>	<b>CR</b>	<b>04/30/2014</b>	<b>05/10/2014</b>	<b>05/16/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>Section 05 10 00 - Structural Steel - 1.8 Quality Assurance by TJPA's Testing Agency - Paragraph D. Sub. Para. 5 Welding: contains the following provisions: c. Complete joint penetration welds: Test all complete joint penetration welds for soundness by means of either radiographic or ultrasonic testing in accordance with AWS D1.1 and ASTM E164 procedures. All flaws in plate or flange material revealed during such tests shall be repaired by the Contractor at the Contractor's expense. d. Partial penetration welds: Test all partial penetration welds for soundness by means of visual and magnetic particle inspection, unless other methods are specified in the Contract Documents. All flaws in plate or flange material revealed during such tests shall be repaired by the Contractor at the Contractor's expense.</p> <p>Please confirm the intent of the specification is to repair "All rejectable flaws in plate or flange material revealed during such tests" in accordance with D1.1 &amp; D1.8 acceptance criteria.</p>						<b>ANSWER:</b> <p>Section 05 10 00 - Structural Steel - 1.8 Quality Assurance by TJPA's Testing Agency - Paragraph D. Sub. Para. 5 Welding: contains the following provisions: c. Complete joint penetration welds: Test all complete joint penetration welds for soundness by means of either radiographic or ultrasonic testing in accordance with AWS D1.1 and ASTM E164 procedures. All flaws in plate or flange material revealed during such tests shall be repaired by the Contractor at the Contractor's expense. d. Partial penetration welds: Test all partial penetration welds for soundness by means of visual and magnetic particle inspection, unless other methods are specified in the Contract Documents. All flaws in plate or flange material revealed during such tests shall be repaired by the Contractor at the Contractor's expense.</p> <p>Please confirm the intent of the specification is to repair "All rejectable flaws in plate or flange material revealed during such tests" in accordance with D1.1 &amp; D1.8 acceptance criteria.</p>
<b>T-1354</b>	<b>SSS - Response to CS5 Connection Clarification at GL 14</b>	<b>Closed</b>	<b>CR</b>	<b>04/30/2014</b>	<b>05/10/2014</b>	<b>05/09/2014</b>
<b>From:</b> Webcor Construction LP Stephanie Azzolino						





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T-1357	SSS - Protected Zone Marking	Closed	CR	04/30/2014	05/10/2014	05/07/2014
From: Webcor Construction LP                      Stephanie Azzolino						
REQUEST:		ANSWER:				
In reference to detail 10/S1-4202, please note the following:		In reference to detail 10/S1-4202, please note the following:				
To mark the plastic hinging zone, it is indicated that yellow striping is to be applied in addition to mounting a warning sign.		To mark the plastic hinging zone, it is indicated that yellow striping is to be applied in addition to mounting a warning sign.				
Please confirm that it is acceptable to use a Dixon Lumber Crayon to install the yellow striping. If this is not acceptable, please provide an alternate solution. See attached photo & catalog cut for example.		Please confirm that it is acceptable to use a Dixon Lumber Crayon to install the yellow striping. If this is not acceptable, please provide an alternate solution. See attached photo & catalog cut for example.				
T-1357.1	SSS - Protected Zone Marking	Closed	CR	07/08/2014	07/18/2014	07/14/2014
From: Webcor Construction LP                      Gregory Kemerer						
REQUEST:		ANSWER:				
Structural detail 10/S1-4202 shows a yellow paint marking to be applied to mask the protected zone. The attached products, J.P. Nissan Co & Brite Mark, are representative of what Skanska feels should be applied to satisfy this requirement.		Structural detail 10/S1-4202 shows a yellow paint marking to be applied to mask the protected zone. The attached products, J.P. Nissan Co & Brite Mark, are representative of what Skanska feels should be applied to satisfy this requirement.				
Skanska's experience with these paint coatings or marking systems is such that it retains its color and adherence to the structural steel well beyond the erection activity and is not detrimental to fireproofing systems. Please confirm that the aforementioned products are acceptable to be applied as the yellow paint marking.		Skanska's experience with these paint coatings or marking systems is such that it retains its color and adherence to the structural steel well beyond the erection activity and is not detrimental to fireproofing systems. Please confirm that the aforementioned products are acceptable to be applied as the yellow paint marking.				
*NOTE - the attached photo does not have the signage displayed; final product will include the signage as shown on 10/S1-4202. In addition, Skanska has spoken to technical representatives from both Carboline (Pyrocrete-40) & Isolatek (Cafco M-II) and they do not foresee any compatibility issues (See SK1 & SK2).		*NOTE - the attached photo does not have the signage displayed; final product will include the signage as shown on 10/S1-4202. In addition, Skanska has spoken to technical representatives from both Carboline (Pyrocrete-40) & Isolatek (Cafco M-II) and they do not foresee any compatibility issues (See SK1 & SK2).				





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<b>T-1358</b>	<b>SSS - Elevator Brace Cover Plates</b>	<b>Closed</b>	<b>CR</b>	<b>04/30/2014</b>	<b>05/10/2014</b>	<b>05/12/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  Please see attached RFI SK 525B for reference:  For the rounded cover plate as shown in the CD RFI 393 SK 1, please confirm that it is acceptable to use a cut section of Round HSS that fits over the outside of the brace within 3/16". The grade of steel for the HSS cover plate would be the same as the HSS brace.						<b>ANSWER:</b>  Please see attached RFI SK 525B for reference:  For the rounded cover plate as shown in the CD RFI 393 SK 1, please confirm that it is acceptable to use a cut section of Round HSS that fits over the outside of the brace within 3/16". The grade of steel for the HSS cover plate would be the same as the HSS brace.
<b>T-1359</b>	<b>SSS - Double Angle Connection Interference GL 8G</b>	<b>Closed</b>	<b>CR</b>	<b>04/30/2014</b>	<b>05/10/2014</b>	<b>05/09/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 403 SK1 & SK2: The double angle connection per 1/S1-5010 will not work at the noted location as it fouls the stiffener plate as shown. Confirm it is acceptable to use detail 2/S1-5011 with a one-sided 3/8" PJP weld due to the lack of welding access. If not, supply an alternate detail.						<b>ANSWER:</b>  See attached CD RFI # 403 SK1 & SK2: The double angle connection per 1/S1-5010 will not work at the noted location as it fouls the stiffener plate as shown. Confirm it is acceptable to use detail 2/S1-5011 with a one-sided 3/8" PJP weld due to the lack of welding access. If not, supply an alternate detail.
<b>T-1360</b>	<b>SSS - Shear Plate Connection Interference at GL 9F</b>	<b>Closed</b>	<b>CR</b>	<b>04/30/2014</b>	<b>05/10/2014</b>	<b>05/09/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  At grid location 9/F at the ground level (S1-2303) the kicker brace per detail 5/S1-5015 shares the full depth shear plate of the W40x211 above. Due to the position of the bolts at the bottom flange the shear plate cannot be increased to 1" thick, as required by 5/S1-5015, without clashing with the bolts. Reference attached sketch CD RFI # 402 SK1 & SK2. Please provide a connection for this location that meets all requirements.						<b>ANSWER:</b>  At grid location 9/F at the ground level (S1-2303) the kicker brace per detail 5/S1-5015 shares the full depth shear plate of the W40x211 above. Due to the position of the bolts at the bottom flange the shear plate cannot be increased to 1" thick, as required by 5/S1-5015, without clashing with the bolts. Reference attached sketch CD RFI # 402 SK1 & SK2. Please provide a connection for this location that meets all requirements.
<b>T-1361</b>	<b>SSS - Bearing Pads foul beam flange</b>	<b>Closed</b>	<b>CR</b>	<b>04/30/2014</b>	<b>05/10/2014</b>	<b>05/12/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI # 401 SK1 & SK2:						<b>ANSWER:</b>  See attached CD RFI # 401 SK1 & SK2:



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The Scougal Rubber Bearing Pads (Package Number TG0701-95.1) foul the flange of the W40x327 as shown.

Please provide a solution.

The Scougal Rubber Bearing Pads (Package Number TG0701-95.1) foul the flange of the W40x327 as shown.

Please provide a solution.





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<b>T-1362.1</b>	<b>BGP - Telecom Cast-In Elbow Radius - Lower Concourse</b>	<b>Closed</b>	<b>01</b>	<b>05/14/2014</b>	<b>05/24/2014</b>	<b>05/15/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached RFI T-1362 Response.		See attached RFI T-1362 Response.				
The response to RFI T-1362 states that 4"x90 elbows with factory 36" min - 42" radius are to be cast into concrete beam as shown on 6/TE1-8014. This contradicts with the 30" radius called out on sheet 6/TE1-8014 and 30" radius confirmed in the comprehensive layout drawings, TG0600-121.1. Please confirm if the 30" radius elbows currently installed per TG0600-121.1 will need to be swapped out for 4"x90 GRS ELBOW with Factory 36" min - 42", as stated in the response to RFI T-1362.		The response to RFI T-1362 states that 4"x90 elbows with factory 36" min - 42" radius are to be cast into concrete beam as shown on 6/TE1-8014. This contradicts with the 30" radius called out on sheet 6/TE1-8014 and 30" radius confirmed in the comprehensive layout drawings, TG0600-121.1. Please confirm if the 30" radius elbows currently installed per TG0600-121.1 will need to be swapped out for 4"x90 GRS ELBOW with Factory 36" min - 42", as stated in the response to RFI T-1362.				
<b>T-1363</b>	<b>BGP - Vehicle Ramp Beam Support Embeds - Threaded Rod, PL Washer and Nut</b>	<b>Closed</b>	<b>01</b>	<b>05/05/2014</b>	<b>05/15/2014</b>	<b>05/08/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please reference detail 1 and detail 10 on S1-3411. Please also reference RFI T-1326 and attached sketch BES-001.		Please reference detail 1 and detail 10 on S1-3411. Please also reference RFI T-1326 and attached sketch BES-001.				
RFI T-1326 provides angles at which vehicle support beams intersect the foundation walls. As shown on attached sketch BES-001 the built up 8x8 angle will be adjusted to match the angle at which the applicable beam intersects the wall. S1-3411 detail 1 and 10 call out for a 1-1/4" diameter threaded rod with plate washers and nuts running through slotted holes in the 8x8 angle. These rods, washers, and nuts will not be perpendicular to the 8x8 angle due to the angles at which the beams intersect the walls.		RFI T-1326 provides angles at which vehicle support beams intersect the foundation walls. As shown on attached sketch BES-001 the built up 8x8 angle will be adjusted to match the angle at which the applicable beam intersects the wall. S1-3411 detail 1 and 10 call out for a 1-1/4" diameter threaded rod with plate washers and nuts running through slotted holes in the 8x8 angle. These rods, washers, and nuts will not be perpendicular to the 8x8 angle due to the angles at which the beams intersect the walls.				
Please confirm no wedge, spacer, or shim will be required between the 8x8 built up angle and the plate washers to evenly distribute the load.		Please confirm no wedge, spacer, or shim will be required between the 8x8 built up angle and the plate washers to evenly distribute the load.				
<b>T-1364</b>	<b>BSE - Geothermal Field 12 Subgrade Acceptance</b>	<b>Closed</b>	<b>01</b>	<b>05/05/2014</b>	<b>05/15/2014</b>	<b>05/07/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				



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	<p>Please confirm Geothermal Field 12 buttress area subgrade was accpeted based on the methods TCCO, ARUP, WOJV and BBII discussed in the field, as follows:</p> <p>1. Aerate the area 2. Re-compact with compaction equipment 3. Inspect / Accept</p> <p>Please see attached email from ARUP (Stephen McLandrich) to TCCO (Jack Adams)</p>					
	<p>Please confirm Geothermal Field 12 buttress area subgrade was accpeted based on the methods TCCO, ARUP, WOJV and BBII discussed in the field, as follows:</p> <p>1. Aerate the area 2. Re-compact with compaction equipment 3. Inspect / Accept</p> <p>Please see attached email from ARUP (Stephen McLandrich) to TCCO (Jack Adams)</p>					
T-1365	SSS - Train Box Column Cap Plate Hole	Closed	CR	05/06/2014	05/16/2014	05/16/2014
	From: Webcor Construction LP Gregory Kemerer					
	REQUEST: <p>With reference to the Train Box Column cap plate, our machine shop has drilled (1) non-standard hole in (1) p395 cap plate due to a broken tool. See attached inspection report and photos.</p> <p>The hole is dimensionally in the correct position but has an internal spiral cut. The minimum diameter is 1-9/16" (1.563) and the max diameter is 1-49/64" (1.770).</p> <p>Please confirm if one of the following is acceptable: Option 1 - Proceed with the as-built condition, no further action required. Option 2 - Reem out the hole to 1-13/16 and use a 1-1/2" bolt. AISC Table J3.3 page 16.1-121 allows oversize hole in slip critical connections, a hardened washer will be installed over the oversized hole. (see attached) Option 3 - Drill the hole to 1-13/16 and use a 1-3/4" diameter bolt.</p>					
	ANSWER: <p>With reference to the Train Box Column cap plate, our machine shop has drilled (1) non-standard hole in (1) p395 cap plate due to a broken tool. See attached inspection report and photos.</p> <p>The hole is dimensionally in the correct position but has an internal spiral cut. The minimum diameter is 1-9/16" (1.563) and the max diameter is 1-49/64" (1.770).</p> <p>Please confirm if one of the following is acceptable: Option 1 - Proceed with the as-built condition, no further action required. Option 2 - Reem out the hole to 1-13/16 and use a 1-1/2" bolt. AISC Table J3.3 page 16.1-121 allows oversize hole in slip critical connections, a hardened washer will be installed over the oversized hole. (see attached) Option 3 - Drill the hole to 1-13/16 and use a 1-3/4" diameter bolt.</p>					



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<b>T-1366</b>	<b>SSS - Slab Opening Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>05/06/2014</b>	<b>05/16/2014</b>	<b>05/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 410 SK1 for items 1 & 2: 1) The Architectural Slab Edge Drawings indicate round slab openings greater than 8". Confirm structural perimeter steel is not required at all round openings or supply a detail. 2) It is not clear what is meant by large openings. Does detail 12/S1-5003 apply to all rectangular/square slab openings shown on the Architectural Slab Edge Drawings?  Please clarify.						<b>ANSWER:</b>  See attached CD RFI # 410 SK1 for items 1 & 2: 1) The Architectural Slab Edge Drawings indicate round slab openings greater than 8". Confirm structural perimeter steel is not required at all round openings or supply a detail. 2) It is not clear what is meant by large openings. Does detail 12/S1-5003 apply to all rectangular/square slab openings shown on the Architectural Slab Edge Drawings?  Please clarify.
<b>T-1366.1</b>	<b>SSS - Slab Opening Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>06/02/2014</b>	<b>06/12/2014</b>	<b>06/10/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Refer to attached CD RFI 410.1 SK1:  1) The response in RFI T-1366 (SK 551, CD 410) does not address rectangular slab openings where one side is less than 2'-0 and the other side is over 2'-0. See the example slab openings and clarify the perimeter steel requirement for all rectangular slab openings where one side is over 2'-0 and the other side is less than 2'-0. 2) The noted slab openings along with other slab openings shown on the Architectural Slab Edge Plans are NOT shown on S1-2302 and other structural plans. Confirm the slab openings on the Architectural Slab Edge Plans supersede the structural plans with updated structural drawings to follow.						<b>ANSWER:</b>  Refer to attached CD RFI 410.1 SK1:  1) The response in RFI T-1366 (SK 551, CD 410) does not address rectangular slab openings where one side is less than 2'-0 and the other side is over 2'-0. See the example slab openings and clarify the perimeter steel requirement for all rectangular slab openings where one side is over 2'-0 and the other side is less than 2'-0. 2) The noted slab openings along with other slab openings shown on the Architectural Slab Edge Plans are NOT shown on S1-2302 and other structural plans. Confirm the slab openings on the Architectural Slab Edge Plans supersede the structural plans with updated structural drawings to follow.
<b>T-1366.2</b>	<b>SSS - Slab Opening Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/14/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Following up on RFIs T-1366 and T-1366.1, please find attached table and exhibits of small slab openings with at least one side that is 2'-0" long.  For each of the minor slab openings included in the						<b>ANSWER:</b>  Following up on RFIs T-1366 and T-1366.1, please find attached table and exhibits of small slab openings with at least one side that is 2'-0" long.  For each of the minor slab openings included in the



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T-1367	<b>SSS - IFRM Prime Coat Requirement</b>  From: Webcor Construction LP                      Gregory Kemerer  <b>REQUEST:</b>  Specification section 07 81 23 (Intumescent Fire Resistive Materials), issued with Field Order 00027, lists three acceptable basis of design manufacturers for IFRM systems. Additionally, specification section 07 81 23, 1.7.C.2 states that the intumescent fire protection system is to be from a single source, indicating that the prime coat Skanska shop applies must be from the same manufacturer as the subsequent coats that are field applied.  Per the monthly schedule delivered to TJPA, the IFRM contractor is not to be selected until 10/16/14.  Skanska requires direction as to which of the three approved basis of design manufacturers will be used on subsequent coats in order to obtain a recommendation from that intumescent coating manufacturer as to the appropriate primer to be utilized.  Alternatively, priming of IFRM steel can be removed from Skanska's scope and added to the scope of the IFRM contractor. This will result in a cost increase.  Please provide direction.	Closed	CR	05/06/2014	05/16/2014	05/12/2014
						<b>ANSWER:</b>  Specification section 07 81 23 (Intumescent Fire Resistive Materials), issued with Field Order 00027, lists three acceptable basis of design manufacturers for IFRM systems. Additionally, specification section 07 81 23, 1.7.C.2 states that the intumescent fire protection system is to be from a single source, indicating that the prime coat Skanska shop applies must be from the same manufacturer as the subsequent coats that are field applied.  Per the monthly schedule delivered to TJPA, the IFRM contractor is not to be selected until 10/16/14.  Skanska requires direction as to which of the three approved basis of design manufacturers will be used on subsequent coats in order to obtain a recommendation from that intumescent coating manufacturer as to the appropriate primer to be utilized.  Alternatively, priming of IFRM steel can be removed from Skanska's scope and added to the scope of the IFRM contractor. This will result in a cost increase.  Please provide direction.
T-1368	<b>BGP - Moment Frame Beam Perimeter Stirrup at Column Supports</b>  From: Webcor Construction LP                      Claude Titcher  <b>REQUEST:</b>	Closed	01	05/06/2014	05/16/2014	05/07/2014
						<b>ANSWER:</b>





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T-1369	<div>SSS - Weld Electrode Subcomponents</div> <div>From: Webcor Construction LP</div> <div>Gregory Kemerer</div> <div>REQUEST: During a Quality Control review, performed by Thompson Metal Fab for compliance with the "Buy America" clause of the Contract, a question was noted regarding the proposed use of ESAB Spoolarc F9A4-ENi4 Submerged Arc Welding (SAW) Electrode for the welding of the HPS70W steel in the Train Box Columns. The attached letter from ESAB, dated April 22, 2014, describes the process that ESAB employs in the manufacture of the referenced product. It notes the fact that the tensile and yield properties of the specific filler metal alloy required to meet the Contract specifications of 90XX are not offered or produced by any mills within the United States. The alloy is provided as a "Greenrod" and is acquired outside of the States. The cold drawing, proprietary processing and packaging of the product is done, by ESAB, wholly within the United States and uses, other than the alloy itself, domestically produced and acquired materials. The Certificate of Conformance from ESAB states that the product is manufactured in the United States (attached).  An inquiry was made, by Skanska, to Lincoln Electric regarding a similar product classification that met the Contract requirement (F9A4-ENi5). Lincoln Electric advised Skanska that they also receive "Greenrod" and strip from sources outside of the United States for this product.  A subsequent Quality Control review by The Herrick Corporation revealed a similar condition for the use of the Lincoln Electric product Outershield XLH-70, AWS</div>	Closed	CR	05/07/2014	05/17/2014	05/21/2014
	<div>Due to the construction sequence, where the moment frame beams intersect a column support, the single-piece moment frame beam perimeter stirrups cannot be installed as shown in 2/S1-3600. Please confirm that it is acceptable to use an alternate 3-piece perimeter stirrup configuration where the moment frame beams intersect a column support. See the attached sketch for details</div> <div>ANSWER: During a Quality Control review, performed by Thompson Metal Fab for compliance with the "Buy America" clause of the Contract, a question was noted regarding the proposed use of ESAB Spoolarc F9A4-ENi4 Submerged Arc Welding (SAW) Electrode for the welding of the HPS70W steel in the Train Box Columns. The attached letter from ESAB, dated April 22, 2014, describes the process that ESAB employs in the manufacture of the referenced product. It notes the fact that the tensile and yield properties of the specific filler metal alloy required to meet the Contract specifications of 90XX are not offered or produced by any mills within the United States. The alloy is provided as a "Greenrod" and is acquired outside of the States. The cold drawing, proprietary processing and packaging of the product is done, by ESAB, wholly within the United States and uses, other than the alloy itself, domestically produced and acquired materials. The Certificate of Conformance from ESAB states that the product is manufactured in the United States (attached).  An inquiry was made, by Skanska, to Lincoln Electric regarding a similar product classification that met the Contract requirement (F9A4-ENi5). Lincoln Electric advised Skanska that they also receive "Greenrod" and strip from sources outside of the United States for this product.  A subsequent Quality Control review by The Herrick</div>					





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>classification E70T-1C-H8, FCAW process weld electrode. Once again, Lincoln has indicated that "Greenrod" for weld electrode products may not be domestically available and may be sourced from throughout the world. However, the manufacturing process, like ESAB, is done wholly within the United States. The Certificate of Conformance from Lincoln also states that the product is manufactured in the United States (see attached certificate and letter dated 04/28/2014).</p> <p>The CFR Part 661.5 of the Clause gives the following definitions:</p> <p>c) The steel and iron requirements apply to all construction materials made primarily of steel or iron and used in infrastructure projects such as transit or maintenance facilities, rail lines and bridges. These items include, but are not limited to, structural steel or iron beams and columns, running rail and contact rail. These requirements do not apply to steel or iron used as components or subcomponents of other manufactured products or rolling stock, or to bimetallic power rail incorporating steel or iron components.</p> <p>d) For a manufactured product to be considered produced in the United States:</p> <p>(1) All of the manufacturing process for the product must take place in the United States; and</p> <p>(2) All of the components of the product must be of U.S. origin. A component is considered of U.S. origin if it is manufactured in the United States, regardless of the origin of its subcomponents.</p> <p>Skanska has interpreted this clause to indicate that both ESAB and Lincoln supply welding electrodes for this Project that are wholly manufactured within the United States and that they satisfy the requirement of the "Buy America" clause.</p> <p>Please confirm that the use of manufactured weld electrodes on the Project for which the manufacturer is providing a Certificate of Conformance indicating that the entire manufacturing process is performed in the United States, but may contain alloy material produced elsewhere, meet the intent of the Contract "Buy America" clause.</p>					<p>Corporation revealed a similar condition for the use of the Lincoln Electric product Outershield XLH-70, AWS classification E70T-1C-H8, FCAW process weld electrode. Once again, Lincoln has indicated that "Greenrod" for weld electrode products may not be domestically available and may be sourced from throughout the world. However, the manufacturing process, like ESAB, is done wholly within the United States. The Certificate of Conformance from Lincoln also states that the product is manufactured in the United States (see attached certificate and letter dated 04/28/2014).</p> <p>The CFR Part 661.5 of the Clause gives the following definitions:</p> <p>c) The steel and iron requirements apply to all construction materials made primarily of steel or iron and used in infrastructure projects such as transit or maintenance facilities, rail lines and bridges. These items include, but are not limited to, structural steel or iron beams and columns, running rail and contact rail. These requirements do not apply to steel or iron used as components or subcomponents of other manufactured products or rolling stock, or to bimetallic power rail incorporating steel or iron components.</p> <p>d) For a manufactured product to be considered produced in the United States:</p> <p>(1) All of the manufacturing process for the product must take place in the United States; and</p> <p>(2) All of the components of the product must be of U.S. origin. A component is considered of U.S. origin if it is manufactured in the United States, regardless of the origin of its subcomponents.</p> <p>Skanska has interpreted this clause to indicate that both ESAB and Lincoln supply welding electrodes for this Project that are wholly manufactured within the United States and that they satisfy the requirement of the "Buy America" clause.</p> <p>Please confirm that the use of manufactured weld electrodes on the Project for which the manufacturer is providing a Certificate of Conformance indicating that the entire manufacturing process is performed in the United States, but may contain alloy material produced elsewhere, meet the intent of the Contract</p>



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"Buy America" clause.						
<b>T-1370</b>	<b>SSS - Roof Level Stiffener Plate GL 16</b>	<b>Closed</b>	<b>CR</b>	<b>05/07/2014</b>	<b>05/17/2014</b>	<b>05/13/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 404 SK1 & SK2: The 3/4" thick stiffener plates per 1/S1-7604 will foul the beam connections above. Please provide a solution.			See attached CD RFI # 404 SK1 & SK2: The 3/4" thick stiffener plates per 1/S1-7604 will foul the beam connections above. Please provide a solution.			
<b>T-1371</b>	<b>SSS - CP1 Connection Support Stiffeners</b>	<b>Closed</b>	<b>CR</b>	<b>05/07/2014</b>	<b>05/17/2014</b>	<b>05/21/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Following our discussion on the CP01 support stiffener welds at Thursday 05/01/14 structural issues coordination meeting, Skanska proposes to remove the "all-around" designation on the 3/4" PJP weld shown in section C 1/S1- 8001 and replace with 'four sides' written in the weld tail. The welds will terminate at the start of the clips at the internal corners. Please confirm this is acceptable. (See sketch below)			Following our discussion on the CP01 support stiffener welds at Thursday 05/01/14 structural issues coordination meeting, Skanska proposes to remove the "all-around" designation on the 3/4" PJP weld shown in section C 1/S1-8001 and replace with 'four sides' written in the weld tail. The welds will terminate at the start of the clips at the internal corners. Please confirm this is acceptable. (See sketch below)			



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<b>T-1372</b>	<b>BGP - CDSM Soldier Pile Encroachment Area 16</b>	<b>Closed</b>	<b>01</b>	<b>05/08/2014</b>	<b>05/18/2014</b>	<b>05/17/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Documents: Exhibits A - G			Reference Documents: Exhibits A - G			
<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 16 as well as all levels of the encroachment into the foundation wall between CDSM piles 343 to 440 on the north east and south elevations Plan see exhibit - A.</p> <p>Exhibit - B &amp; C depict the location and degree in which the SP are encroaching.</p> <p>WOJV proposal North elevation on gridline A (343 -369): (See Exhibit - B) Between CDSM pile 351 to 353. WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 352. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>WOJV proposal East elevation on gridline 35 (369 -414): (See Exhibit - B) Between CDSM pile 373 to 375,379 to 381, 387 to 399 &amp; 404 to 406. WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 374,380,388,390,391,395,398 &amp; 403. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>WOJV proposal on the South elevation on gridline A (415 - 440): (See Exhibit - B) Between CDSM piles 415 to 417 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 419, This foundation wall area was originally a embedment column (C -023) with reinforcement in this area was a double layer of #11@6"oc EF vertically and would change to double layer of #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail B/Sk.4 option 2 (Exhibit -F).</p> <p>Between CDSM piles 417 to 420 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear</p>			<p>This RFI addresses the impact of the encroaching CDSM soldier piles (SP) on the north &amp; south wall in mat slab pour Area 16 as well as all levels of the encroachment into the foundation wall between CDSM piles 343 to 440 on the north east and south elevations Plan see exhibit - A.</p> <p>Exhibit - B &amp; C depict the location and degree in which the SP are encroaching.</p> <p>WOJV proposal North elevation on gridline A (343 - 369): (See Exhibit - B) Between CDSM pile 351 to 353. WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 352. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>WOJV proposal East elevation on gridline 35 (369 - 414): (See Exhibit - B) Between CDSM pile 373 to 375,379 to 381, 387 to 399 &amp; 404 to 406. WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 374,380,388,390,391,395,398 &amp; 403. Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>WOJV proposal on the South elevation on gridline A (415 -440): (See Exhibit - B) Between CDSM piles 415 to 417 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 419, This foundation wall area was originally a embedment column (C -023) with reinforcement in this area was a double layer of #11@6"oc EF vertically and would change to double layer of #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail B/Sk.4 option 2</p>			



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	<p>the encroaching SP 416 Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 423 to 425-426, 428 to 434 &amp; 438 to 441-442 WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 424,428 to 432 &amp; 441 Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 425-426 to 428 WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 426 &amp; 427 This foundation wall area was originally a embedment column with reinforcement in this area was a double layer of #11@6"oc EF vertically and would change to double layer of #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.4 option 1 (Exhibit -F).</p> <p>Between CDSM piles 434 to 438 WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 436 &amp; 437 Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit -E)</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p> <p>See Exhibit - G shows a typical detail of transition between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings. Please confirm if these solutions would be acceptable.</p>					<p>(Exhibit -F).</p> <p>Between CDSM piles 417 to 420 WOJV is proposing to decrease the specified 36" wall thickness to 34" to clear the encroaching SP 416 Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 423 to 425-426, 428 to 434 &amp; 438 to 441-442 WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 424,428 to 432 &amp; 441 Originally these were WR1 reinforcement areas #11@8"oc EF vertically and would change to #11@6"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.1 (Exhibit - D).</p> <p>Between CDSM piles 425-426 to 428 WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 426 &amp; 427 This foundation wall area was originally a embedment column with reinforcement in this area was a double layer of #11@6"oc EF vertically and would change to double layer of #11@5"OC this reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.4 option 1 (Exhibit -F).</p> <p>Between CDSM piles 434 to 438 WOJV is proposing to decrease the specified 36" wall thickness to 33" to clear the encroaching SP 436 &amp; 437 Originally this was a WR2 reinforcement areas #11@6"oc EF vertically and would change to #11@5"OC, the reduction in foundation wall thickness would be compensated by reducing the rebar spacing predicated on Detail A/Sk.3 option 2 (Exhibit -E)</p> <p>In all other areas without CDSM pile encroachment issues the reinforcement will remain unchanged as per the Contract drawings.</p> <p>See Exhibit - G shows a typical detail of transition</p>



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<p>between modified reinforcement to contract reinforcement.</p> <p>These solutions if approved would be incorporated into the TG06 shop drawings. Please confirm if these solutions would be acceptable.</p>						
<b>T-1372.1</b>	<b>BGP - W140- C23 Embedded Columns Rebar</b>	<b>Closed</b>	<b>CR</b>	<b>12/30/2014</b>	<b>01/09/2015</b>	<b>01/04/2015</b>
<p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: S1-3300, S1-3306 Locatoion: Zone 4, Train Box Gridline: J/34.8 Add'l Doc Ref's: Gerdau's RFI#147, RFI T-1372			Contract Doc Ref: S1-3300, S1-3306 Locatoion: Zone 4, Train Box Gridline: J/34.8 Add'l Doc Ref's: Gerdau's RFI#147, RFI T-1372			
The Area 16 East End Seismic Joint on the South Wall (W140) does not allow for the placement of all 56ea #11 vertical bars @5" OC as called out in RFI T-1372.			The Area 16 East End Seismic Joint on the South Wall (W140) does not allow for the placement of all 56ea #11 vertical bars @5" OC as called out in RFI T-1372.			
Instead, only 48ea #11 vertical bars can be installed at 5" OC before the vertical bars interfere with the seismic joint.			Instead, only 48ea #11 vertical bars can be installed at 5" OC before the vertical bars interfere with the seismic joint.			
Note that C-23 embedded column only requires 48ea #11 vertical bars.			Note that C-23 embedded column only requires 48ea #11 vertical bars.			
Please confirm that it is acceptable to eliminate 2ea vertical bars on each face ( 4 faces for a total of 8 vertical bars) out at the eastern most end of Wall W140 at East Seismic Joint.			Please confirm that it is acceptable to eliminate 2ea vertical bars on each face ( 4 faces for a total of 8 vertical bars) out at the eastern most end of Wall W140 at East Seismic Joint.			



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<b>T-1373</b>	<b>BGP - Modifying Slump Limits for Mix 1557205</b>	<b>Closed</b>	<b>01</b>	<b>05/07/2014</b>	<b>05/17/2014</b>	<b>05/13/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Please reference attached letter dated 5/2/2014, authored by Robert Foley, CEMEX QC Manager.  On March 20, 2014 the column mix was modified to increase average compressive strength due to low test results for laboratory cured cylinders. The changes that were made to mix #1557205 to increase strength are having an effect on the slump variability. SCCI and CEMEX propose the design slump range be increased to 6 to 9 inches. Is this proposed change acceptable?						<b>ANSWER:</b>  Please reference attached letter dated 5/2/2014, authored by Robert Foley, CEMEX QC Manager.  On March 20, 2014 the column mix was modified to increase average compressive strength due to low test results for laboratory cured cylinders. The changes that were made to mix #1557205 to increase strength are having an effect on the slump variability. SCCI and CEMEX propose the design slump range be increased to 6 to 9 inches. Is this proposed change acceptable?
<b>T-1374</b>	<b>SSS - Stair Post Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>05/08/2014</b>	<b>05/18/2014</b>	<b>05/21/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 413 SK1 to SK3 for items 1 & 2: 1) Confirm the connection is acceptable as shown or supply an alternate detail. 2) Confirm the connection is acceptable as shown or supply an alternate detail.						<b>ANSWER:</b>  See attached CD RFI # 413 SK1 to SK3 for items 1 & 2: 1) Confirm the connection is acceptable as shown or supply an alternate detail. 2) Confirm the connection is acceptable as shown or supply an alternate detail.
<b>T-1375</b>	<b>SSS - Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>05/08/2014</b>	<b>05/18/2014</b>	<b>05/16/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 416 SK1 for items 1 & 2: 1) Confirm the missing beam size is a W12x14. 2) The connections will foul each other. Confirm it is acceptable to connect the WF beam with a shear plate per 1/S15011.						<b>ANSWER:</b>  See attached CD RFI # 416 SK1 for items 1 & 2: 1) Confirm the missing beam size is a W12x14. 2) The connections will foul each other. Confirm it is acceptable to connect the WF beam with a shear plate per 1/S15011.
<b>T-1376</b>	<b>SSS - Welding Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>05/08/2014</b>	<b>05/18/2014</b>	<b>05/21/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 417 SK1 & SK2: Supply welding information for thick flange/plate into thin						<b>ANSWER:</b>  See attached CD RFI # 417 SK1 & SK2: Supply welding information for thick flange/plate into



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	flange @ moment connection.					thin flange @ moment connection.
<b>T-1378</b>	<b>BGP - SFPUC Electrical Room Grounding</b>	<b>Closed</b>	<b>01</b>	<b>05/09/2014</b>	<b>05/19/2014</b>	<b>05/09/2014</b>
<b>From:</b> Shimmick Construction Company, Inc. Sylvia Hartanto						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI T-1227.1 - SFPUC Plate Grounding.			Please reference RFI T-1227.1 - SFPUC Plate Grounding.			
Please confirm the SFPUC Transformer mounting plate is to be installed by a future contractor and SCCI is only to stub the grounding conductor into each corner of the Electrical Room( see Note 1 on Plate Sheet EI-2203).			Please confirm the SFPUC Transformer mounting plate is to be installed by a future contractor and SCCI is only to stub the grounding conductor into each corner of the Electrical Room( see Note 1 on Plate Sheet EI-2203).			
If SCCI is to install this SFPUC Transformer mounting plate, provide the specifications, including but not limited to material type, size and thickness.			If SCCI is to install this SFPUC Transformer mounting plate, provide the specifications, including but not limited to material type, size and thickness.			
<b>T-1378.1</b>	<b>BGP - SFPUC Electrical Room Grounding</b>	<b>Closed</b>	<b>01</b>	<b>05/30/2014</b>	<b>06/09/2014</b>	<b>06/09/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please provide a drawing which shows the dimensioned locations of the SFPUC Transformer plates so the grounding tails can be stubbed out of the slab at the correct locations.			Please provide a drawing which shows the dimensioned locations of the SFPUC Transformer plates so the grounding tails can be stubbed out of the slab at the correct locations.			





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<b>T-1379</b>	<b>BGP - Pit and Pile Discrepancy at GL 27/E</b>	<b>Closed</b>	<b>01</b>	<b>05/12/2014</b>	<b>05/22/2014</b>	<b>05/14/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Please reference attached contract drawings sheet S1-2026.  The mat slab pit located at Gridline 27 and E has a width of 2'-6". The pile located within this specific pit (see marked up S1-2026 is a 36" pile as detailed. Please provide new dimensions for the pit incorporating the 36" pile.						<b>ANSWER:</b>  Please reference attached contract drawings sheet S1-2026.  The mat slab pit located at Gridline 27 and E has a width of 2'-6". The pile located within this specific pit (see marked up S1-2026 is a 36" pile as detailed. Please provide new dimensions for the pit incorporating the 36" pile.
<b>T-1379.1</b>	<b>BGP - Pit and Pile Discrepancy at GL 27-E</b>	<b>Void</b>	<b>01</b>	<b>05/21/2014</b>	<b>05/31/2014</b>	
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  SCCI is in receipt of RFI T-1379. SCCI disagrees with the response provided in the aforementioned RFI.  S1-2026 illustrates a 30" wide pile in a 30" wide pit at gridline 27/E. However, Submittal #TG0300- 284.4, which was reviewed and approved by the design team, shows the previously mentioned trestle pile as a 36" wide pile. It has also been confirmed in the field this pile is 36" wide pile.  Please provide applicable details (similar to D4 on SI-3009 & D2 on SI-3007) for permanent elements in the mat slab construction, including but not limited to galvanized sleeves and structural steel reinforcing, for the pit at GL 27/E which contains a 36" wide pile, not 30" as shown on the design drawing S 1-2026.						<b>ANSWER:</b>  SCCI is in receipt of RFI T-1379. SCCI disagrees with the response provided in the aforementioned RFI.  S1-2026 illustrates a 30" wide pile in a 30" wide pit at gridline 27/E. However, Submittal #TG0300- 284.4, which was reviewed and approved by the design team, shows the previously mentioned trestle pile as a 36" wide pile. It has also been confirmed in the field this pile is 36" wide pile.  Please provide applicable details (similar to D4 on SI-3009 & D2 on SI-3007) for permanent elements in the mat slab construction, including but not limited to galvanized sleeves and structural steel reinforcing, for the pit at GL 27/E which contains a 36" wide pile, not 30" as shown on the design drawing S 1-2026.
<b>T-1379.2</b>	<b>BGP - Pit and Pile Discrepancy at GL 27/E</b>	<b>Closed</b>	<b>01</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>06/12/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Please reference attached SI-2026, SI-3007, SI-3009, SCCI Mat Slab Comprehensive Drawing S113.0, Gerdau Mat Slab Rebar Shop Drawing, and Sketch 113-001.  For constructability reasons, SCCI proposes to enlarge the pit at GL 27/E in order to facilitate the installation of the						<b>ANSWER:</b>  Please reference attached SI-2026, SI-3007, SI-3009, SCCI Mat Slab Comprehensive Drawing S113.0, Gerdau Mat Slab Rebar Shop Drawing, and Sketch 113-001.  For constructability reasons, SCCI proposes to





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	<p>trestle pile sleeve and cap. Please see sketch 113-001 for details and dimensions. Rebar would be altered to accommodate enlarged pit dimensions. See sketch 113-001 for rebar details for pour-back portion of pit once pile is removed.</p> <p>Please confirm this is acceptable.</p>					<p>enlarge the pit at GL 27/E in order to facilitate the installation of the trestle pile sleeve and cap. Please see sketch 113-001 for details and dimensions. Rebar would be altered to accommodate enlarged pit dimensions. See sketch 113-001 for rebar details for pour-back portion of pit once pile is removed.</p> <p>Please confirm this is acceptable.</p>
T-1380	SSS - Field Weld Access at Deck Support Angles	Closed	CR	05/12/2014	05/22/2014	05/27/2014
From: Webcor Construction LP		Gregory Kemerer				
REQUEST:		ANSWER:				
<p>Refer to 1/S1-3701 for deck support angle welded to Transfer Girder web detail.</p> <p>Due to restricted access the decker cannot make the required field welds when the top of deck to underside of Transfer Girder flange is less than 12". See attached SK1 for clarification.</p> <p>1) Confirm a 3/8" bent plate can be welded to the toe of the top flange.</p> <p>2) Confirm the weld indicated on SK1 is acceptable.</p> <p>3) Confirm the deformed anchor rods can be welded to the bent plate.</p> <p>Where deck to underside of girder exceeds 12" the deck support angle will be detailed as per 1/S1-3701.</p>		<p>Refer to 1/S1-3701 for deck support angle welded to Transfer Girder web detail.</p> <p>Due to restricted access the decker cannot make the required field welds when the top of deck to underside of Transfer Girder flange is less than 12". See attached SK1 for clarification.</p> <p>1) Confirm a 3/8" bent plate can be welded to the toe of the top flange.</p> <p>2) Confirm the weld indicated on SK1 is acceptable.</p> <p>3) Confirm the deformed anchor rods can be welded to the bent plate.</p> <p>Where deck to underside of girder exceeds 12" the deck support angle will be detailed as per 1/S1-3701.</p>				









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<b>T-1384</b>	<b>BGP - Spandrel Beam Modifications in Area 14</b>	<b>Closed</b>	<b>01</b>	<b>05/14/2014</b>	<b>05/24/2014</b>	<b>05/22/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Reference Documents: Exhibits A - B  Further to response to RFI T-637 please find attached proposed changes to the spandrel beams in pour Area 14 for location plan see exhibit - A  Exhibit - B shows the plan view of the modification necessary to the spandrel beam on the north and south elevations due to the revised reinforcement width of the foundation wall due to encroachment of the CDSM beams as well as typical cross sections of the revised spandrel beams.  RFI T - 1384 shows the extent of the modification to the foundation wall on the north and south elevations of Area 14.  Please confirm that these modifications as outlined at these locations are acceptable.			<b>ANSWER:</b>  Reference Documents: Exhibits A - B  Further to response to RFI T-637 please find attached proposed changes to the spandrel beams in pour Area 14 for location plan see exhibit - A  Exhibit - B shows the plan view of the modification necessary to the spandrel beam on the north and south elevations due to the revised reinforcement width of the foundation wall due to encroachment of the CDSM beams as well as typical cross sections of the revised spandrel beams.  RFI T - 1384 shows the extent of the modification to the foundation wall on the north and south elevations of Area 14.  Please confirm that these modifications as outlined at these locations are acceptable.			

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**ANSWER:**





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<b>T-1387</b>	<b>SSS - Erection Plan Temporary Lugs</b>	<b>Closed</b>	<b>CR</b>	<b>05/12/2014</b>	<b>05/22/2014</b>	<b>05/23/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
During a recent structural issues coordination meeting, Skanska raised a concern regarding a submittal response received with package TG0701-29. Specifically, Skanska flagged a comment that indicated our temporary erection aids at the exterior moment frame columns needed to be located 36" below the moment frame beam to column connection at the bus deck level and also 36" away from the weld joint between the column and cast node/transfer girder at ground level. See attached drawings for ease of reference.			During a recent structural issues coordination meeting, Skanska raised a concern regarding a submittal response received with package TG0701-29. Specifically, Skanska flagged a comment that indicated our temporary erection aids at the exterior moment frame columns needed to be located 36" below the moment frame beam to column connection at the bus deck level and also 36" away from the weld joint between the column and cast node/transfer girder at ground level. See attached drawings for ease of reference.			
As discussed during the meeting, please confirm that this is not mandatory; and, if Skanska opts to leave the lugs as shown on the attached documents, it is acceptable.			As discussed during the meeting, please confirm that this is not mandatory; and, if Skanska opts to leave the lugs as shown on the attached documents, it is acceptable.			
<b>T-1388</b>	<b>SSS - Framing Connection Interference at GL7G</b>	<b>Closed</b>	<b>CR</b>	<b>05/13/2014</b>	<b>05/23/2014</b>	<b>05/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 414 SK1 & SK2: The connections for the W16x26 beams foul the connection for the W40x149 per 1/S1-5019. Confirm it is acceptable to reduce the 1'-11 dimension to 1'-6 and supply shear plate connections for the W16x26 beams per 1/S1-5011 as shown. If not, supply a new detail.			See attached CD RFI # 414 SK1 & SK2: The connections for the W16x26 beams foul the connection for the W40x149 per 1/S1-5019. Confirm it is acceptable to reduce the 1'-11 dimension to 1'-6 and supply shear plate connections for the W16x26 beams per 1/S1-5011 as shown. If not, supply a new detail.			
<b>T-1389</b>	<b>SSS - Stud Comment Clarification on Decking Drawings</b>	<b>Closed</b>	<b>CR</b>	<b>05/13/2014</b>	<b>05/23/2014</b>	<b>05/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
With reference to the Linden Steel decking drawings the reviewer's comment references general note DK-5 - Distribute steel studs uniformly over beam span unless otherwise noted on drawings. Maximum spacing of ¾ inch headed studs shall not exceed 24" on center (one stud every 2 feet) unless otherwise noted.			With reference to the Linden Steel decking drawings the reviewer's comment references general note DK-5 - Distribute steel studs uniformly over beam span unless otherwise noted on drawings. Maximum spacing of ¾ inch headed studs shall not exceed 24" on center (one stud every 2 feet) unless otherwise noted.			



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	<p>As per note 1 on 1/S1-5000 - "See general notes for headed stud size and maximum spacing. Number of studs is indicated on the framing plan." Linden has modeled their drawings as per note 1, referring to the general notes for size and spacing requirements and the framing plans for stud quantities indicated on each member as per the Steel Beam Legend on S1-2302.</p> <p>If a stud quantity is not indicated on a beam member, no studs are provided. Please confirm this interpretation is correct. If additional studs are required on members that are not currently identified please provide revised drawings identifying beams and quantities required.</p>					
T-1389.1	<p><b>SSS - Stud Comment Clarification on Decking Drawings</b></p> <p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>According to Note 1 on 1/S1-5000, the number of headed studs is indicated on the framing plans. As per the steel beam legend on the framing plans, the number of studs is indicated in (*) following the beam designation (see SK1). It has come to our attention that the design team requires studs on certain members that currently do not show a stud quantity.</p> <p>Please provide a document that shows which members, not identified as requiring studs on the framing plans, shall have studs installed so that we may incorporate into our decking shop drawing submittal packages.</p>	Closed	CR	06/10/2014	06/20/2014	06/26/2014
	<p><b>ANSWER:</b></p> <p>According to Note 1 on 1/S1-5000, the number of headed studs is indicated on the framing plans. As per the steel beam legend on the framing plans, the number of studs is indicated in (*) following the beam designation (see SK1). It has come to our attention that the design team requires studs on certain members that currently do not show a stud quantity.</p> <p>Please provide a document that shows which members, not identified as requiring studs on the framing plans, shall have studs installed so that we may incorporate into our decking shop drawing submittal packages.</p>					



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<b>T-1390</b>	<b>SSS - Stiffener Details at Roof Spandrel GL 1.4-B</b>	<b>Closed</b>	<b>CR</b>	<b>05/13/2014</b>	<b>05/23/2014</b>	<b>05/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Detail 4 on S1-8002 indicates the stiffener requirements at grid line 1.4/B at the perimeter roof girder. Please confirm the following (reference CD RFI 407 SK1 through SK3):			Detail 4 on S1-8002 indicates the stiffener requirements at grid line 1.4/B at the perimeter roof girder. Please confirm the following (reference CD RFI 407 SK1 through SK3):			
1. Confirm the stiffener plates and welding are per detail 2/S1-4205.			1. Confirm the stiffener plates and welding are per detail 2/S1-4205.			
2. Detail 4E/S1-4205 graphically indicates a beam framing into the perimeter roof girder, however there is no beam on Grid 1.4 at Grid B per S1-2602. Please confirm it is acceptable to provide a stiffener per 2/S1-4205 on each side.			2. Detail 4E/S1-4205 graphically indicates a beam framing into the perimeter roof girder, however there is no beam on Grid 1.4 at Grid B per S1-2602. Please confirm it is acceptable to provide a stiffener per 2/S1-4205 on each side.			
<b>T-1391</b>	<b>SSS - Approval Comment Clarifications, Beams at Stiffener Locations (GL 15-16)</b>	<b>Closed</b>	<b>CR</b>	<b>05/13/2014</b>	<b>05/23/2014</b>	<b>05/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 418 SK1 & SK2 for items 1 & 2:			See attached CD RFI # 418 SK1 & SK2 for items 1 & 2:			
1) Please confirm the welding as shown is correct.			1) Please confirm the welding as shown is correct.			
2) The added stiffeners per 11/S1-7630 at (8) locations will result in the (4) beams connecting at these locations not being erectable. Confirm the connection may be changed to a shear plate per 1/S1-5011 except with the bolts pulled			2) The added stiffeners per 11/S1-7630 at (8) locations will result in the (4) beams connecting at these locations not being erectable. Confirm the connection may be changed to a shear plate per 1/S1-5011 except with the bolts pulled			
outside the profile of the beams to allow access to the bolts. If not, supply an alternate solution.			outside the profile of the beams to allow access to the bolts. If not, supply an alternate solution.			
<b>T-1392</b>	<b>SSS - Missing Brace Locations (GL 3)</b>	<b>Closed</b>	<b>CR</b>	<b>05/13/2014</b>	<b>05/23/2014</b>	<b>05/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 419 SK1:			See attached CD RFI # 419 SK1:			
It is not clear what is meant by the information inside the box, please clarify. If the information is meant to locate the braces per 1/S1-7661, it seems they will not work as they will foul the beams. Please provide the locations on			It is not clear what is meant by the information inside the box, please clarify. If the information is meant to locate the braces per 1/S1-7661, it seems they will not work as they will foul the beams. Please provide the			



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	plan for the braces per 1/S1-7661.					locations on plan for the braces per 1/S1-7661.
<b>T-1393</b>	<b>SSS - Missing Beam Location (GL 15)</b>	<b>Closed</b>	<b>CR</b>	<b>05/13/2014</b>	<b>05/23/2014</b>	<b>05/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 420 SK1 & SK2 for items 1 to 3:  1) Supply the missing dimensions to locate the beam.  2) Confirm the short W16x26 is located 1'-1 1/4 south of Grid D as shown.  3) If the response to item 2 above is yes, confirm the east end of the short W16x26 may be connected with a shear plate to avoid fouling the double angle connection for the supporting W16x26.						<b>ANSWER:</b> See attached CD RFI # 420 SK1 & SK2 for items 1 to 3:  1) Supply the missing dimensions to locate the beam.  2) Confirm the short W16x26 is located 1'-1 1/4 south of Grid D as shown.  3) If the response to item 2 above is yes, confirm the east end of the short W16x26 may be connected with a shear plate to avoid fouling the double angle connection for the supporting W16x26.
<b>T-1394</b>	<b>BGP - Stem-Walls With Elevated Slab - Mat Slab Level</b>	<b>Closed</b>	<b>01</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>05/23/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> See attached Contract Documents.  Sheets A1-2124 through A1-2127 show 20 stem-wall cells with elevated slabs @ -32'-1" and -30'-0" TOC added in ASI 107 (clouded). Note 1 on mentioned sheets calls out "reinforced concrete wall refer to structural drawings", the S1-9000 series does not provide rebar schedules or details for stem-walls in question. Please confirm if typical wall rebar should be used where wall thickness is covered by the typical details. Or provide rebar schedules for vertical/horizontal rebar in stem-walls and the elevated slab. Also provide corner, intersection, and stem-wall to elevated slab connection details.						<b>ANSWER:</b> See attached Contract Documents.  Sheets A1-2124 through A1-2127 show 20 stem-wall cells with elevated slabs @ -32'-1" and -30'-0" TOC added in ASI 107 (clouded). Note 1 on mentioned sheets calls out "reinforced concrete wall refer to structural drawings", the S1-9000 series does not provide rebar schedules or details for stem-walls in question. Please confirm if typical wall rebar should be used where wall thickness is covered by the typical details. Or provide rebar schedules for vertical/horizontal rebar in stem-walls and the elevated slab. Also provide corner, intersection, and stem-wall to elevated slab connection details.
<b>T-1395</b>	<b>BGP - Lower Concourse Beams Intersecting Columns</b>	<b>Closed</b>	<b>01</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>05/19/2014</b>





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<b>T-1396.1</b>	<b>SSS - ConnectionErection Clarifications at W-13 System</b>	<b>Closed</b>	<b>CR</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/26/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 422.2 SK1 & SK2 for items 1 to 3: 1) Detail 3/S1-5028 shows 8 pairs of bolts and RFI T-1396 shows 6 pairs. Confirm 6 pairs is correct. 2) RFI T-1396 gave the 4" horizontal spacing without being specific on the bolt locations. As shown, the 4" spacing will not work because the 2" shear plate with 1/2" fillet welds will foul the 1 1/2" bolts. Please clarify the bolt spacing. 3) The connections foul each other with the revised beam spacing as shown on SK1. Confirm it is acceptable to connect the beams parallel to Grid E.2 with a single shear plate per 1/S1-5011 or supply an alternate solution.			See attached CD RFI # 422.2 SK1 & SK2 for items 1 to 3: 1) Detail 3/S1-5028 shows 8 pairs of bolts and RFI T-1396 shows 6 pairs. Confirm 6 pairs is correct. 2) RFI T-1396 gave the 4" horizontal spacing without being specific on the bolt locations. As shown, the 4" spacing will not work because the 2" shear plate with 1/2" fillet welds will foul the 1 1/2" bolts. Please clarify the bolt spacing. 3) The connections foul each other with the revised beam spacing as shown on SK1. Confirm it is acceptable to connect the beams parallel to Grid E.2 with a single shear plate per 1/S1-5011 or supply an alternate solution.			
<b>T-1397</b>	<b>SSS - Missing information at PE301 &amp; PE302 System</b>	<b>Closed</b>	<b>CR</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>05/30/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 423 SK1 to SK3 for items 1 to 8: 1) Confirm the HSS6x6x5/8 posts are aligned with the HSS12x6x5/8 posts in the north-south direction as supplied in RFI T-1090. 2) Supply the dimension to the center of the (2) HSS6x6x5/8 posts. 3) Confirm the correct reference is 1/S1-7600. 4) Supply a connection detail for the bottom of these posts at the corner of the slab opening. 5) It appears here and in detail B/S1-7132 (SK3) that the east-west HSS12x6 beam connects to the concrete wall but the north-south HSS12x6 beams are continuous. Please clarify the steel framing. 6) Supply missing dimensions. 7) Supply missing elevations. 8) Supply a connection detail for HSS beams to concrete wall.			See attached CD RFI # 423 SK1 to SK3 for items 1 to 8: 1) Confirm the HSS6x6x5/8 posts are aligned with the HSS12x6x5/8 posts in the north-south direction as supplied in RFI T-1090. 2) Supply the dimension to the center of the (2) HSS6x6x5/8 posts. 3) Confirm the correct reference is 1/S1-7600. 4) Supply a connection detail for the bottom of these posts at the corner of the slab opening. 5) It appears here and in detail B/S1-7132 (SK3) that the east-west HSS12x6 beam connects to the concrete wall but the north-south HSS12x6 beams are continuous. Please clarify the steel framing. 6) Supply missing dimensions. 7) Supply missing elevations. 8) Supply a connection detail for HSS beams to concrete wall.			



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<b>T-1398</b>	<b>SSS - W40X264 Connection clarifications (GL 32)</b>	<b>Closed</b>	<b>CR</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>05/28/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI # 424 SK1 & SK2:  Confirm it is acceptable to cope the top flange of the W40x264 as shown to clear the TPG1		<b>ANSWER:</b>  See attached CD RFI # 424 SK1 & SK2:  Confirm it is acceptable to cope the top flange of the W40x264 as shown to clear the TPG1				
<b>T-1399</b>	<b>SSS - W40X249 Schedule (GL 20-21)</b>	<b>Closed</b>	<b>CR</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>06/03/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI # 425 SK1:  The W40x249 is not listed in the schedule. Confirm it is acceptable to insert it in the 2nd row from the top.		<b>ANSWER:</b>  See attached CD RFI # 425 SK1:  The W40x249 is not listed in the schedule. Confirm it is acceptable to insert it in the 2nd row from the top.				
<b>T-1400</b>	<b>SSS - W40X149 Connection Clarification (GL 24)</b>	<b>Closed</b>	<b>CR</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>05/27/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  See attached CD RFI # 426 SK1 & SK2:  As shown on attached SK2 & SK2, it is not possible to connect the skewed W40x149 using 'W' when 'b' = 2". Confirm the connection as shown on SK2 is acceptable or supply an alternate solution.		<b>ANSWER:</b>  See attached CD RFI # 426 SK1 & SK2:  As shown on attached SK2 & SK2, it is not possible to connect the skewed W40x149 using 'W' when 'b' = 2". Confirm the connection as shown on SK2 is acceptable or supply an alternate solution.				
<b>T-1401</b>	<b>SSS - Access Hole at CJP Termination on TG</b>	<b>Closed</b>	<b>CR</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>05/28/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  Please confirm it is acceptable to incorporate two weld access holes (as per AWS D1.1, section 5.17.1) as indicated on the attached SK2 to allow TMF to properly terminate the CJP welds after the slotted intermediate flange (p774) is welded to the web.  See attached SK1 & SK2 for clarification.		<b>ANSWER:</b>  Please confirm it is acceptable to incorporate two weld access holes (as per AWS D1.1, section 5.17.1) as indicated on the attached SK2 to allow TMF to properly terminate the CJP welds after the slotted intermediate flange (p774) is welded to the web.  See attached SK1 & SK2 for clarification.				





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<b>T-1402</b>	<b>SSS - Bolt Accessibility GL 14 and 15</b>	<b>Closed</b>	<b>CR</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>05/30/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> See attached CD RFI # 415 SK1 to SK3 for item 2.  2) The noted bolts for the W27x84 beams are not accessible from the back side as shown in SK3. Please supply an alternate detail.					<b>ANSWER:</b> See attached CD RFI # 415 SK1 to SK3 for item 2.  2) The noted bolts for the W27x84 beams are not accessible from the back side as shown in SK3. Please supply an alternate detail.	
<b>T-1403</b>	<b>BGP - Partition Wall Construction Joints</b>	<b>Closed</b>	<b>01</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>05/23/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Please confirm the following items are acceptable regarding partition wall construction joints.  1. To provide the 3/8" gap for the vertical CJ's, SCCI intends to place felt board in the joint with a 3/4" chamfer (see attached drawing). Once the walls on either side of the CJ have been poured, the felt board will be trimmed down to the end of the chamfer and the edges of the joint will be caulked. Please confirm this is acceptable.  2. With the exception of the tank walls, SCCI intends to pour the Train Level Partition Walls that go all the way to the Lower Concourse (~28'-11" tall) in two lifts with one horizontal CJ. SCCI proposes prepping the joint by roughening it with 1/4" amplitude. Please confirm this is acceptable.					<b>ANSWER:</b> Please confirm the following items are acceptable regarding partition wall construction joints.  1. To provide the 3/8" gap for the vertical CJ's, SCCI intends to place felt board in the joint with a 3/4" chamfer (see attached drawing). Once the walls on either side of the CJ have been poured, the felt board will be trimmed down to the end of the chamfer and the edges of the joint will be caulked. Please confirm this is acceptable.  2. With the exception of the tank walls, SCCI intends to pour the Train Level Partition Walls that go all the way to the Lower Concourse (~28'-11" tall) in two lifts with one horizontal CJ. SCCI proposes prepping the joint by roughening it with 1/4" amplitude. Please confirm this is acceptable.	
<b>T-1404</b>	<b>SCS - Transfer Girder Clarification</b>	<b>Closed</b>	<b>01</b>	<b>05/15/2014</b>	<b>05/25/2014</b>	<b>05/29/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Please clarify the designer's intent:  Sheet SI-2303 indicates that the south end of Transfer Girder TR9 extends beyond the B87 and B88 beams southern edge and partially into the intersecting MFBI beam which is angular to the B87 beam. Section 8/S1-3701 indicates that there are welded rebar couplers at					<b>ANSWER:</b> Please clarify the designer's intent:  Sheet SI-2303 indicates that the south end of Transfer Girder TR9 extends beyond the B87 and B88 beams southern edge and partially into the intersecting MFBI beam which is angular to the B87 beam. Section 8/S1-3701 indicates that there are welded rebar couplers at	





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T-1405	SCS - TR7 Transfer Girder	Closed	01	05/16/2014	05/16/2014	05/29/2014
	<p>top flange of the TR9 girder to match the B78 beam reinforcing, but the B78 beam ends at the B87/B88 intersection prior to the southern end of the TR9 girder.</p> <p><b>From:</b> Webcor Construction LP      Claude Titcher</p> <p><b>REQUEST:</b> Please clarify the designer's intent.</p> <p>Sheet S1-2303 indicated that the south end of Transfer Girder TR7 extends into the B106 beam (60"w X 72"h). Section 8/S1-3702 and 11/S1-3703 indicate a substantially shallower beam section.</p>					
T-1406	SSS - Edge Plate Clarification at W-13 Opening	Closed	CR	05/16/2014	05/26/2014	05/30/2014
	<p><b>From:</b> Webcor Construction LP      Gregory Kemerer</p> <p><b>REQUEST:</b> See attached CD RFI # 421 SK1 &amp; SK2 for items 1 &amp; 2:</p> <p>1) Please supply a detail showing how the segmented edge plate is to be fabricated.</p> <p>2) Please supply a detail showing how the segmented edge plate is to be fabricated.</p>					



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	<p>The 2 southern 72 "x40" CBM beams east of Grid 1.4 are not indicated as sloping and reference Section 1 /S1- 5023 that indicates that the beams are sloping. Please clarify the details for the welded couplers and #11 bars for the 72"x40" CBM beams at east side of TR1 .4. Section 2/S1-7605 indicates 17-#11 bars top and bottom, one added set of 17-#11 bars 3" below the top bars, and 4-#11 side bars. The details shown in Sections 1/SI-5023 and 9/SI-3700 indicate that the top and bottom 17-# 11 bars are connected to the transfer girder by welded couplers or bars. Please verify that only the top and bottom #11 bars connect to the transfer girder by welded couplers or bars.</p>					<p>CBM beams are level. The 2 southern 72 "x40" CBM beams east of Grid 1.4 are not indicated as sloping and reference Section 1 /S1- 5023 that indicates that the beams are sloping. Please clarify the details for the welded couplers and #11 bars for the 72"x40" CBM beams at east side of TR1 .4. Section 2/S1-7605 indicates 17-#11 bars top and bottom, one added set of 17-#11 bars 3" below the top bars, and 4-#11 side bars. The details shown in Sections 1/SI-5023 and 9/SI-3700 indicate that the top and bottom 17-# 11 bars are connected to the transfer girder by welded couplers or bars. Please verify that only the top and bottom #11 bars connect to the transfer girder by welded couplers or bars.</p>
T-1410	<p><b>BGP - Updated Concrete Wall Elevation and Lower Concourse SKA's</b></p> <p>From: Webcor Construction LP                      Claude Titche</p> <p><b>REQUEST:</b></p> <p>On May 20, 2014 WOJV received an email from Adamson-Associates containing 28 SKA's with the request that WOJV incorporate the SKA's into the TCB construction documents via the RFI Process.</p> <p>Reference attached sketches:</p> <ul style="list-style-type: none"> <li>- SKA-3322 TO SKA-3325 (CONC WALL ELEVATIONS)</li> <li>- SKA-3326 TO SKA-3329 (CONC WALL ELEVATIONS)</li> <li>- ZONE 2- SKA-3330, SKA-3334, SKA-3338, SKA-3357</li> <li>- ZONE 3 - SKA-3331, SKA-3335, SKA-3339</li> <li>- ZONE 4 - SKA-3359, SKA-3360, SKA-3361</li> <li>- ZONE 5 - SKA-3322, SKA-3336, SKA-3340</li> <li>- ZONE 10 - SKA-3333, SKA-3337, SKA-3341</li> </ul> <p>Please confirm the attached architectural SKA's shall be incorporated in the TCB construction documents. Please note these unissued SKA's have been referred to in submittal package "TG0600-341 - REBAR - Shop Drawings - Partition Walls" returned to WOJV 5/21/2014.</p> <p>Also, in the attached, there are two versions of the SKA's. Please clarify which shall be deemed "current" for use in the project.</p>	Closed	01	05/22/2014	06/01/2014	06/05/2014
						<p><b>ANSWER:</b></p> <p>On May 20, 2014 WOJV received an email from Adamson-Associates containing 28 SKA's with the request that WOJV incorporate the SKA's into the TCB construction documents via the RFI Process.</p> <p>Reference attached sketches:</p> <ul style="list-style-type: none"> <li>- SKA-3322 TO SKA-3325 (CONC WALL ELEVATIONS)</li> <li>- SKA-3326 TO SKA-3329 (CONC WALL ELEVATIONS)</li> <li>- ZONE 2- SKA-3330, SKA-3334, SKA-3338, SKA-3357</li> <li>- ZONE 3 - SKA-3331, SKA-3335, SKA-3339</li> <li>- ZONE 4 - SKA-3359, SKA-3360, SKA-3361</li> <li>- ZONE 5 - SKA-3322, SKA-3336, SKA-3340</li> <li>- ZONE 10 - SKA-3333, SKA-3337, SKA-3341</li> </ul> <p>Please confirm the attached architectural SKA's shall be incorporated in the TCB construction documents. Please note these unissued SKA's have been referred to in submittal package "TG0600-341 - REBAR - Shop Drawings - Partition Walls" returned to WOJV 5/21/2014.</p> <p>Also, in the attached, there are two versions of the</p>



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SKA's. Please clarify which shall be deemed "current" for use in the project.						
T-1411	<b>SSS - Missing dimensions PE403 and PE404</b>	Closed	CR	05/23/2014	06/02/2014	05/30/2014
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 427 SK1: Confirm the noted dimensions are correct for PE403 & PE404 at 2nd level and Bus deck level.			See attached CD RFI # 427 SK1: Confirm the noted dimensions are correct for PE403 & PE404 at 2nd level and Bus deck level.			
T-1412	<b>SSS - Pipe Wall Thickness Tolerance</b>	Closed	CR	05/23/2014	06/02/2014	05/28/2014
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per conversation during "Transbay structural issues review" meeting on 5-20-2014, the wall thickness tolerance of spec 05 15 22 was discussed. Per design team, this tolerance is intended to apply to the ends where the pipe would be welded, and a less restrictive tolerance would be acceptable throughout the length of the pipe, provided that the wall thickness is greater than nominal, not less.			Per conversation during "Transbay structural issues review" meeting on 5-20-2014, the wall thickness tolerance of spec 05 15 22 was discussed. Per design team, this tolerance is intended to apply to the ends where the pipe would be welded, and a less restrictive tolerance would be acceptable throughout the length of the pipe, provided that the wall thickness is greater than nominal, not less.			
The manufacturer of the spun-cast pipe has produced all product to date with dimensions that meet the 05 15 22 spec for 12" at each end. Throughout the remainder of the pipe, the wall thickness is greater than nominal (typically by about 0.090"), and in all cases falling within the tolerance of API-5L.			The manufacturer of the spun-cast pipe has produced all product to date with dimensions that meet the 05 15 22 spec for 12" at each end. Throughout the remainder of the pipe, the wall thickness is greater than nominal (typically by about 0.090"), and in all cases falling within the tolerance of API-5L.			
See attached depiction of the pipe tolerances.			See attached depiction of the pipe tolerances.			
Please confirm that wall thickness tolerance noted in spec 05 15 22 is intended to apply to the ends only, and that the wall thickness throughout the remainder of the pipe may be greater by as much as is allowed by API5L, but shall be no less than the nominal wall thickness.			Please confirm that wall thickness tolerance noted in spec 05 15 22 is intended to apply to the ends only, and that the wall thickness throughout the remainder of the pipe may be greater by as much as is allowed by API5L, but shall be no less than the nominal wall thickness.			



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T-1413	SSS - Second Level Popout Verifications	Closed	CR	05/23/2014	06/02/2014	05/30/2014
From: Webcor Construction LP Stephanie Azzolino						
REQUEST: Please verify the two clouded dimensions at the Second Level popouts as indicated on SK1 & 2.		ANSWER: Please verify the two clouded dimensions at the Second Level popouts as indicated on SK1 & 2.				
T-1414	SSS - Step in Slab at GL21 C&G	Closed	CR	05/23/2014	06/02/2014	06/04/2014
From: Webcor Construction LP Stephanie Azzolino						
REQUEST: At GL 21 Ground Level where the slab makes a 25" step, detail 11/S1-5004 is cut along the step at the knock-out slab locations and a WT and support beam are indicated. Between the perimeter beam and the W24x55 framing beams at C & G there are no support beams or WTs indicated to support the step in slab. See SK 1 & 2 for clarification.  Please provide a detail section through this step indicating how the slab is supported.		ANSWER: At GL 21 Ground Level where the slab makes a 25" step, detail 11/S1-5004 is cut along the step at the knock-out slab locations and a WT and support beam are indicated. Between the perimeter beam and the W24x55 framing beams at C & G there are no support beams or WTs indicated to support the step in slab. See SK 1 & 2 for clarification.  Please provide a detail section through this step indicating how the slab is supported.				
T-1415	SSS - Exposed Flange at Step in Slab GL5-6	Closed	CR	05/23/2014	06/02/2014	07/01/2014
From: Webcor Construction LP Stephanie Azzolino						
REQUEST: See attached CD RFI # 429 SK1 & SK2: The flange of the W33x118 will extend outside the concrete as shown. Please advise.		ANSWER: See attached CD RFI # 429 SK1 & SK2: The flange of the W33x118 will extend outside the concrete as shown. Please advise.				
T-1416	SSS - Seal Weld at Edge of Backing Bar	Closed	CR	05/23/2014	06/02/2014	05/28/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: Please see OIW shop drawing 2771-RN151. The termination of backing plate d151 and the finished ends of the CJP welds produces an inconsistent looking finish.  It is OIW's intent to provide a continuous 5/16 seal weld at the end of the backing plate and the node web plates(see		ANSWER: Please see OIW shop drawing 2771-RN151. The termination of backing plate d151 and the finished ends of the CJP welds produces an inconsistent looking finish.  It is OIW's intent to provide a continuous 5/16 seal				



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	<p>attached photo). We believe this closure weld is within the allowances of AWS D1.1, and will improve the overall finish of this AESS area.</p> <p>Please confirm that the noted seal weld is acceptable for all welded roof nodes on OIW detail drawings RN151 through RN164.</p>					<p>weld at the end of the backing plate and the node web plates(see attached photo). We believe this closure weld is within the allowances of AWS D1.1, and will improve the overall finish of this AESS area.</p> <p>Please confirm that the noted seal weld is acceptable for all welded roof nodes on OIW detail drawings RN151 through RN164.</p>
T-1417	<b>SSS - Missing OCS Switch Information</b>	Closed	CR	05/23/2014	06/02/2014	06/11/2014
	<p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p> <p><b>REQUEST:</b></p> <p>This is a follow-up RFI to RFI SK 399 (CD 296) See attached CD RFI # 296.1 SK1 to SK7 for items 1 to 18:</p> <ol style="list-style-type: none"><li>1) 2/S1-9101 shows the framing at 14'-2 C/C UON but no start location for the steel is supplied. Please supply location dimensions for the vertical HSS4x4 hangers per 2/S1-9101.</li><li>2) Supply missing clouded dimensions (10) locations.</li><li>3) Supply elevation to top of 3/8" galvanized.</li><li>4) Supply missing clouded dimensions (12) locations.</li><li>5) Supply elevation to top of 3/8" galvanized plate.</li><li>6) Clarify how detail 2/S1-9101 applies at the noted location. Should the reference be to detail 3/S1-9101?</li><li>7) Supply (5) clouded dimensions.</li><li>8) Supply a connection detail.</li><li>9) Confirm the brace connection is as shown in detail 3/S1-9101 (SK5).</li><li>10) Supply a detail for the bottom of the HSS4x4 posts.</li><li>11) Supply a connection detail for the HSS4x4 posts to horizontal HSS4x4.</li><li>12) Supply elevation to underside of posts.</li><li>13) Supply the weld for PL3/8" to HSS4x4.</li><li>14) Supply this dimension for each HSS4x4 post.</li><li>15) This detail will not work as it is not known at the detailing stage where the deck flutes will be. The detail also does not work when the deck spans parallel with the brace. Please confirm it is acceptable to proceed per the information in RFI T-1067.2 (SK 230.2, CD 181.2).</li><li>16) Supply the thickness and welding for the stiffeners.</li><li>17) Supply elevation to establish stiffener locations.</li></ol>					<p><b>ANSWER:</b></p> <p>This is a follow-up RFI to RFI SK 399 (CD 296) See attached CD RFI # 296.1 SK1 to SK7 for items 1 to 18:</p> <ol style="list-style-type: none"><li>1) 2/S1-9101 shows the framing at 14'-2 C/C UON but no start location for the steel is supplied. Please supply location dimensions for the vertical HSS4x4 hangers per 2/S1-9101.</li><li>2) Supply missing clouded dimensions (10) locations.</li><li>3) Supply elevation to top of 3/8" galvanized.</li><li>4) Supply missing clouded dimensions (12) locations.</li><li>5) Supply elevation to top of 3/8" galvanized plate.</li><li>6) Clarify how detail 2/S1-9101 applies at the noted location. Should the reference be to detail 3/S1-9101?</li><li>7) Supply (5) clouded dimensions.</li><li>8) Supply a connection detail.</li><li>9) Confirm the brace connection is as shown in detail 3/S1-9101 (SK5).</li><li>10) Supply a detail for the bottom of the HSS4x4 posts.</li><li>11) Supply a connection detail for the HSS4x4 posts to horizontal HSS4x4.</li><li>12) Supply elevation to underside of posts.</li><li>13) Supply the weld for PL3/8" to HSS4x4.</li><li>14) Supply this dimension for each HSS4x4 post.</li><li>15) This detail will not work as it is not known at the detailing stage where the deck flutes will be. The detail also does not work when the deck spans parallel with the brace. Please confirm it is acceptable to proceed per the information in RFI T-1067.2 (SK 230.2, CD 181.2).</li></ol>



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18) Supply a connection for the HSS4x4 to the PL3/8".						16) Supply the thickness and welding for the stiffeners . 17) Supply elevation to establish stiffener locations. 18) Supply a connection for the HSS4x4 to the PL3/8".
T-1417.1	<b>SSS - Missing OCS Switch Information</b>  <b>From:</b> Webcor Construction LP  <b>REQUEST:</b> The responses to the following items were not sufficient for Candraft to proceed. 7) The response notes one of the requested dimensions as "VARIES". Please provide criteria for this varying dimension. 12) Detail 3/A1-8551 does not provide the bottom of the HSS post elevations. Please supply the elevation at each post. 13) Per the structural details only the 1/2" plate is to be galvanized. Confirm this is correct and supply weld required for PL3/8" to HSS4x4. 14) Supply the requested missing dimension as shown at each post. 17) Please supply the elevations for the stiffeners in the columns. 18b) Please provide the size, radius, hole location and dimension on the 1/2" x 8" plate. Note: See response sketches SK1 through SK7	Closed	CR	06/27/2014	07/07/2014	07/11/2014  <b>ANSWER:</b> The responses to the following items were not sufficient for Candraft to proceed. 7) The response notes one of the requested dimensions as "VARIES". Please provide criteria for this varying dimension. 12) Detail 3/A1-8551 does not provide the bottom of the HSS post elevations. Please supply the elevation at each post. 13) Per the structural details only the 1/2" plate is to be galvanized. Confirm this is correct and supply weld required for PL3/8" to HSS4x4. 14) Supply the requested missing dimension as shown at each post. 17) Please supply the elevations for the stiffeners in the columns. 18b) Please provide the size, radius, hole location and dimension on the 1/2" x 8" plate. Note: See response sketches SK1 through SK7





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T-1417.2	SSS - Missing OCS Switch Information	Closed	CR	07/15/2014	07/25/2014	07/28/2014
<div><div><div>From: Webcor Construction LP</div><div>Gregory Kemerer</div></div><div><div>REQUEST:</div><div>See attached CD RFI # 296.4 SK1 to SK3 for items 1 to 4: Skanska plans the following erection aids for the OCS. Please review and confirm there are no adverse structural implications. 1) Confirm the concept of the erection aid detail as shown is acceptable to accommodate the field welding required. 2) Confirm welding for cap plate as shown is acceptable. 3) Confirm the concept of the erection aid detail as shown is acceptable to accommodate the field welding required. 4) Confirm it is acceptable to supply (4) 15/16 diameter holes per plate for erection lifting holes as shown.</div></div><div><div>ANSWER:</div><div>See attached CD RFI # 296.4 SK1 to SK3 for items 1 to 4: Skanska plans the following erection aids for the OCS. Please review and confirm there are no adverse structural implications. 1) Confirm the concept of the erection aid detail as shown is acceptable to accommodate the field welding required. 2) Confirm welding for cap plate as shown is acceptable. 3) Confirm the concept of the erection aid detail as shown is acceptable to accommodate the field welding required. 4) Confirm it is acceptable to supply (4) 15/16 diameter holes per plate for erection lifting holes as shown.</div></div></div>						
T-1417.3	SSS - Missing OCS Switch Information	Closed	CR	07/25/2014	08/04/2014	07/28/2014
<div><div><div>From: Webcor Construction LP</div><div>Gregory Kemerer</div></div><div><div>REQUEST:</div><div>This is a follow-up to RFI T-1417.1 (SK 399.3A, CD 296.3)  Skanska: The responses to items 7, 12, 14 &amp; 17 require further clarification. Please review the items below. Item 7 ~ the response and reference to A1-8550 does not provide the requested dimensions. Please provide the dimensions to locate the posts. Item 12 ~ the response tells us that the Universal Spacer Bar is to be 18'-0" above the ultimate constructed sloping grade with the bottom of the post being +/- 4" above that per 3/A1-8151. Several unknowns remain: a) What does "ultimate constructed sloping grade" mean? b) The top of slab information on drawings S1-2306, A1-2306 &amp; A1-2866 is not clear c) What does the +/- 4" in detail 3/A1-8151 mean? Item 14 ~ same as item 12 Item 17 ~ same as item 12</div></div><div><div>ANSWER:</div><div>This is a follow-up to RFI T-1417.1 (SK 399.3A, CD 296.3) Skanska: The responses to items 7, 12, 14 &amp; 17 require further clarification. Please review the items below. Item 7 ~ the response and reference to A1-8550 does not provide the requested dimensions. Please provide the dimensions to locate the posts. Item 12 ~ the response tells us that the Universal Spacer Bar is to be 18'-0" above the ultimate constructed sloping grade with the bottom of the post being +/- 4" above that per 3/A1-8151. Several unknowns remain: a) What does "ultimate constructed sloping grade" mean? b) The top of slab information on drawings S1-2306, A1-2306 &amp; A1-2866 is not clear c) What does the +/- 4" in detail 3/A1-8151 mean? Item 14 ~ same as item 12 Item 17 ~ same as item 12</div></div></div>						
T-1417.4	SSS - Missing OCS Switch Information	Closed	CR	09/02/2014	09/12/2014	09/15/2014





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<hr/>						
<b>From:</b> Webcor Construction LP		Gregory Kemerer				
<b>REQUEST:</b>			<b>ANSWER:</b>			
Following the OCS RFI coordination meeting held 8/28/2014, please provide the following information as discussed:			Following the OCS RFI coordination meeting held 8/28/2014, please provide the following information as discussed:			
Item 7 - the response and reference to A1-8550 does not provide the requested dimensions. Please see SK1 and provide the dimensions to locate each post.			Item 7 - the response and reference to A1-8550 does not provide the requested dimensions. Please see SK1 and provide the dimensions to locate each post.			
Item 12 - the response tells us that the Universal Spacer Bar is to be 18'-0" above the ultimate constructed sloping grade with the bottom of the post being +/- 4" above that per 3/A1-8151. Please provide the elevations for the bottom of each support post.			Item 12 - the response tells us that the Universal Spacer Bar is to be 18'-0" above the ultimate constructed sloping grade with the bottom of the post being +/- 4" above that per 3/A1-8151. Please provide the elevations for the bottom of each support post.			
Item 17 - please supply the elevations for the stiffeners in the columns.			Item 17 - please supply the elevations for the stiffeners in the columns.			
Item 18b - please provide the size, radius, hole diameter and hole location on the 1/2" x8" plate.			Item 18b - please provide the size, radius, hole diameter and hole location on the 1/2" x8" plate.			
<b>T-1417.5</b>	<b>SSS - Missing OCS Switch Information</b>	<b>Closed</b>	<b>CR</b>	<b>10/23/2014</b>	<b>10/23/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP		Gregory Kemerer				
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 296.8 SK1 to SK2 for items 1 to 8:			See attached CD RFI # 296.8 SK1 to SK2 for items 1 to 8:			
1) Confirm if 1/2" thick Galv plate is required and provide dimensions & elevation.			1) Confirm if 1/2" thick Galv plate is required and provide dimensions & elevation.			
2) Confirm items 65 & 66 are typical 2/S1-9101 hangers.			2) Confirm items 65 & 66 are typical 2/S1-9101 hangers.			
3) Confirm if 1/2" thick Galv plate is required and provide dimensions & elevation.			3) Confirm if 1/2" thick Galv plate is required and provide dimensions & elevation.			
4) Confirm items 67 & 68 are typical 2/S1-9101 hangers.			4) Confirm items 67 & 68 are typical 2/S1-9101 hangers.			
5) S1-2407 shows a reference to 6/S1-9101 between grids 31.7-32. Please supply the location & elevation of the plate/hangers if required.			5) S1-2407 shows a reference to 6/S1-9101 between grids 31.7-32. Please supply the location & elevation of the plate/hangers if required.			
6) Confirm items 71 & 72 are typical 2/S1-9101 hangers.			6) Confirm items 71 & 72 are typical 2/S1-9101 hangers.			
7) Confirm if 1/2" thick Galv plate is required and provide dimensions & elevation.			7) Confirm if 1/2" thick Galv plate is required and provide dimensions & elevation.			
8) Confirm if reference to 6/S1-9101 between grids 31.7-32 is required. Please supply the location & elevation of the plate/hangers.			8) Confirm if reference to 6/S1-9101 between grids 31.7-32 is required. Please supply the location & elevation of the plate/hangers.			





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T-1417.7C	SSS - Missing OCS Information at East End Column	Open	CR	02/01/2015	02/11/2015	RFI 296.15 SK1 does not reflect the actual condition.
						Refer to the attached CD RFI 296.15 SK2 section B-B and confirm the connection as shown is correct.
T-1417.7D	SSS - Missing OCS Information at East End Column	Open	CR	02/01/2015	02/11/2015	Refer to the attached CD RFI 296.15 SK1 for the following:  3. Supply dimension at CL PL 1 1/2" @ CL column. 4. Supply dimension. 5. Supply dimension. 6. Supply angle. 8. Supply elevation at CL PL 1 1/2" @ CL column. 9. Supply angle.
						Refer to the attached CD RFI 296.15 SK1 for the following:  3. Supply dimension at CL PL 1 1/2" @ CL column. 4. Supply dimension. 5. Supply dimension. 6. Supply angle. 8. Supply elevation at CL PL 1 1/2" @ CL column. 9. Supply angle.



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<b>T-1417.7E</b>	<b>SSS - Missing OCS Information at East End Column</b>	<b>Open</b>	<b>CR</b>	<b>02/01/2015</b>	<b>02/11/2015</b>	
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> Contract Doc: N/A Location: Zone 4 Gridline: C.3/32.4 Add'l Doc Ref's: CD RFI 296.15 SK1, RFI T-1417.6  10. The note on CD RFI 296.15 SK1 states "T/O plate ELEV. 36'-7 " and "CL of plate ELEV. 35'-9".  It is not clear what these elevations are referring to.  Please clarify.						<b>ANSWER:</b> Contract Doc: N/A Location: Zone 4 Gridline: C.3/32.4 Add'l Doc Ref's: CD RFI 296.15 SK1, RFI T-1417.6  10. The note on CD RFI 296.15 SK1 states "T/O plate ELEV. 36'-7 " and "CL of plate ELEV. 35'-9".  It is not clear what these elevations are referring to.  Please clarify.
<b>T-1418</b>	<b>SSS - PE403 &amp; PE404 Missing Connection Detail</b>	<b>Closed</b>	<b>CR</b>	<b>05/23/2014</b>	<b>06/02/2014</b>	<b>06/04/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 428 SK1:  Supply a connection detail for the flat MC10 to HSS posts.						<b>ANSWER:</b> See attached CD RFI # 428 SK1:  Supply a connection detail for the flat MC10 to HSS posts.
<b>T-1419</b>	<b>SSS - Bus Deck Cast Node Dimension Confirmation</b>	<b>Closed</b>	<b>CR</b>	<b>05/23/2014</b>	<b>06/02/2014</b>	<b>06/04/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> The structural issues meeting on 5/15/2014 reviewed potential changes to the cast node machine shop drawings. Due to the sand inclusions in the 35A and 35B cast nodes, further machining of the pad surfaces is required. This will result in a dimension of 16" from the W0 work point in lieu of the initial dimension of 17" (see attached sketch). Cast Connex has proceeded to make the 16" revision to the cast node machine shop drawings.  Please confirm this is acceptable, and update the structural drawings to match.						<b>ANSWER:</b> The structural issues meeting on 5/15/2014 reviewed potential changes to the cast node machine shop drawings. Due to the sand inclusions in the 35A and 35B cast nodes, further machining of the pad surfaces is required. This will result in a dimension of 16" from the W0 work point in lieu of the initial dimension of 17" (see attached sketch). Cast Connex has proceeded to make the 16" revision to the cast node machine shop drawings.  Please confirm this is acceptable, and update the structural drawings to match.



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<b>T-1420</b>	<b>SSS - Missing Connection Detail BU-60 GL 4-5</b>	<b>Closed</b>	<b>CR</b>	<b>05/27/2014</b>	<b>06/06/2014</b>	<b>06/10/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 430 SK1 : Please supply a connection detail for the BU-60 to the BU-60's.		<b>ANSWER:</b>  See attached CD RFI # 430 SK1 : Please supply a connection detail for the BU-60 to the BU-60's.				
<b>T-1421</b>	<b>SSS - ST401 Slab Opening Dimension Discrepancies</b>	<b>Closed</b>	<b>CR</b>	<b>05/27/2014</b>	<b>06/06/2014</b>	<b>05/30/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 431 SK1 & SK2 : The black dimensions hi-lited in purple on SK2 are per 1/S1-7009 and the red dimensions are per A1-2864. The current model and drawings match 1/S1-7009. Please advise which dimensions are correct.		<b>ANSWER:</b>  See attached CD RFI # 431 SK1 & SK2 : The black dimensions hi-lited in purple on SK2 are per 1/S1-7009 and the red dimensions are per A1-2864. The current model and drawings match 1/S1-7009. Please advise which dimensions are correct.				
<b>T-1422</b>	<b>SSS - Bolt to Edge Dimension Clarification GL 16</b>	<b>Closed</b>	<b>CR</b>	<b>05/27/2014</b>	<b>06/06/2014</b>	<b>06/10/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 432 SK1: Confirm the 1 7/16" edge distance for the 1 1/2" dia. bolts per 1/S1-8000 is acceptable or supply a solution.		<b>ANSWER:</b>  See attached CD RFI # 432 SK1: Confirm the 1 7/16" edge distance for the 1 1/2" dia. bolts per 1/S1-8000 is acceptable or supply a solution.				
<b>T-1423</b>	<b>SSS - Girder Flashing Max Gap Dimension</b>	<b>Closed</b>	<b>CR</b>	<b>05/27/2014</b>	<b>06/06/2014</b>	<b>06/04/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  With reference to details 6, 7 & 9/S1-5000, a 9" max dimension is shown from the edge of the girder flashing to the edge of the deck with a 1-1/2" minimum bearing onto the girder, indicating the flashing gauge is to match the gauge of the deck. Based on the preceding, most locations would require 18 gauge flashing with the max gap between beam and deck being 7-1/2".  Please confirm it is acceptable to increase the max gap to 9" as the deck flutes are at 12" centers and at some locations this gap will exceed 7-1/2". To accommodate the increased gap, the decker is proposing to use 16 gauge flashing at all locations, connecting the flashing to the		<b>ANSWER:</b>  With reference to details 6, 7 & 9/S1-5000, a 9" max dimension is shown from the edge of the girder flashing to the edge of the deck with a 1-1/2" minimum bearing onto the girder, indicating the flashing gauge is to match the gauge of the deck. Based on the preceding, most locations would require 18 gauge flashing with the max gap between beam and deck being 7-1/2".  Please confirm it is acceptable to increase the max gap to 9" as the deck flutes are at 12" centers and at some locations this gap will exceed 7-1/2". To accommodate the increased gap, the decker is				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	beam with ¾" diameter puddle welds at 12" OC and connecting the side lap with button punches at 12" OC or top seam welds at 24" OC.  See attached SK1 for clarifications.			proposing to use 16 gauge flashing at all locations, connecting the flashing to the beam with ¾" diameter puddle welds at 12" OC and connecting the side lap with button punches at 12" OC or top seam welds at 24" OC.  See attached SK1 for clarifications.		
<b>T-1425</b>	<b>SSS - Edge of Slab Angle at Seismic Joint GL10 &amp; 20</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>05/29/2014</b>	<b>06/08/2014</b>	<b>06/10/2014</b>
	<b>REQUEST:</b>  Section 3/S1-3282 details the typical concrete wall at expansion joint on the Roof Deck Level. Please confirm an edge of slab bent plate and deck support as per 9/S1-5000 is not required. See SK1 & SK2 for reference.			<b>ANSWER:</b>  Section 3/S1-3282 details the typical concrete wall at expansion joint on the Roof Deck Level. Please confirm an edge of slab bent plate and deck support as per 9/S1-5000 is not required. See SK1 & SK2 for reference.		
<b>T-1425.1</b>	<b>SSS - Edge of Slab Angle at Seismic Joint GL10 &amp; 20</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>06/30/2014</b>	<b>07/10/2014</b>	<b>07/09/2014</b>
	<b>REQUEST:</b>  See attached CD RFI # 433.1 SK1 & SK2 for items 1 & 2: 1) Confirm the outline of top of slab/edge plate at grids B & H at the expansion joints. 2) Confirm the edge plate support detail at the 1/1/4" connection plate is acceptable as shown or supply an alternate detail. 3) The 10" on SK2 indicates top of roof floor slab. Confirm the bent plate will stop at top of slab OR is an additional 8" required to the concrete key location. 4) For detail 9/S1-5000 now required at the Seismic joints, verify the 8'-0 Max spacing for the angles can be increased to 8'-6 in order to line up with the back span beam spacing per design.			<b>ANSWER:</b>  See attached CD RFI # 433.1 SK1 & SK2 for items 1 & 2: 1) Confirm the outline of top of slab/edge plate at grids B & H at the expansion joints. 2) Confirm the edge plate support detail at the 1/1/4" connection plate is acceptable as shown or supply an alternate detail. 3) The 10" on SK2 indicates top of roof floor slab. Confirm the bent plate will stop at top of slab OR is an additional 8" required to the concrete key location. 4) For detail 9/S1-5000 now required at the Seismic joints, verify the 8'-0 Max spacing for the angles can be increased to 8'-6 in order to line up with the back span beam spacing per design.		
<b>T-1425.2</b>	<b>SSS - Edge of Slab Angle at Seismic Joint GL10 &amp; 20</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/09/2014</b>





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<b>T-1428</b>	<b>SSS - High Slab Support at Ground Level GL4F</b>	<b>Closed</b>	<b>CR</b>	<b>05/29/2014</b>	<b>06/08/2014</b>	<b>06/09/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 442 SK1 for items 1 & 2: 1) Confirm the BU-WT's at these locations should be extended as shown. 2) Confirm BU-WT's are required at the noted locations to support the high slab.						<b>ANSWER:</b> See attached CD RFI # 442 SK1 for items 1 & 2: 1) Confirm the BU-WT's at these locations should be extended as shown.  2) Confirm BU-WT's are required at the noted locations to support the high slab.
<b>T-1429</b>	<b>SSS - Framing Interference at GL 5D Ground Level</b>	<b>Closed</b>	<b>CR</b>	<b>05/29/2014</b>	<b>06/08/2014</b>	<b>06/10/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 441 SK1: 1) The double angle connection is not possible as it will foul the stiffener plate. Confirm it is acceptable to connect the W16 beam with a full depth shear plate with bolts, plate thickness and welding per 1/S1-5011. 2) Confirm the connection above may be applied at other similar locations.						<b>ANSWER:</b> See attached CD RFI # 441 SK1: 1) The double angle connection is not possible as it will foul the stiffener plate. Confirm it is acceptable to connect the W16 beam with a full depth shear plate with bolts, plate thickness and welding per 1/S1-5011. 2) Confirm the connection above may be applied at other similar locations.
<b>T-1430</b>	<b>BGP - Lower Concourse E/W Bottom Deck Bar at GL 9 MFB</b>	<b>Closed</b>	<b>01</b>	<b>05/30/2014</b>	<b>06/09/2014</b>	<b>06/04/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> Please confirm per field conversation between structural EOR and Gerdau that it is acceptable to install the concourse slab bottom East-West reinforcing at GL 9 per the attached sketch.						<b>ANSWER:</b> Please confirm per field conversation between structural EOR and Gerdau that it is acceptable to install the concourse slab bottom East-West reinforcing at GL 9 per the attached sketch.
<b>T-1431</b>	<b>SSS - PE403 &amp; PE404 Missing Connection Detail at Posts</b>	<b>Closed</b>	<b>CR</b>	<b>05/30/2014</b>	<b>06/09/2014</b>	<b>06/12/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 434 SK1 & SK2: The connection per RFI T-1105 shown above will not work at the noted locations as the base plate for the HSS10x10 is in the way. Please supply an alternate connection for the HSS12x6.						<b>ANSWER:</b> See attached CD RFI # 434 SK1 & SK2: The connection per RFI T-1105 shown above will not work at the noted locations as the base plate for the HSS10x10 is in the way. Please supply an alternate connection for the HSS12x6.





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<b>T-1432</b>	<b>SSS - PE403 &amp; PE404 Fouling Connection Detail</b>	<b>Closed</b>	<b>CR</b>	<b>05/30/2014</b>	<b>06/09/2014</b>	<b>06/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 435 SK1 & SK2: The upper angle per 5/S1-7600 will foul the gusset plate for the brace. Please supply an alternate detail.					<b>ANSWER:</b> See attached CD RFI # 435 SK1 & SK2: The upper angle per 5/S1-7600 will foul the gusset plate for the brace. Please supply an alternate detail.	
<b>T-1433</b>	<b>SSS - Conflicting ST401 Dimensions</b>	<b>Closed</b>	<b>CR</b>	<b>05/30/2014</b>	<b>06/09/2014</b>	<b>06/10/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 437 SK1 to SK3: The approval instructions on drawing 3265 in the CS6 approval submittal (Package Number TG0701-78) require a clarification. S1-7011 and A1-7011 show conflicting dimensions to locate the posts as shown on SK2 & SK3. Currently the model reflects S1-7011. Please confirm that S1-7011 is correct or supply clarifying direction on which ST403 post locations are correct.					<b>ANSWER:</b> See attached CD RFI # 437 SK1 to SK3: The approval instructions on drawing 3265 in the CS6 approval submittal (Package Number TG0701-78) require a clarification. S1-7011 and A1-7011 show conflicting dimensions to locate the posts as shown on SK2 & SK3. Currently the model reflects S1-7011. Please confirm that S1-7011 is correct or supply clarifying direction on which ST403 post locations are correct.	
<b>T-1433.1</b>	<b>SSS - Conflicting ST401 Dimensions</b>	<b>Closed</b>	<b>CR</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/18/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 437.1 SK1 & SK2: The result of the varying dimensions on S1-7011 and SKA-3379 (A1-7011) is that we do not know where to locate the posts on the north side of stair ST403. Please clarify the discrepancies.					<b>ANSWER:</b> See attached CD RFI # 437.1 SK1 & SK2: The result of the varying dimensions on S1-7011 and SKA-3379 (A1-7011) is that we do not know where to locate the posts on the north side of stair ST403. Please clarify the discrepancies.	
<b>T-1434</b>	<b>SSS - Conflicting ST401 Dimensions at Post Connections</b>	<b>Closed</b>	<b>CR</b>	<b>05/30/2014</b>	<b>06/09/2014</b>	<b>06/12/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 438 SK1 & SK2: This approval instruction on drawing 3264AB in the CS6 approval submittal (Package Number TG0701-78) requires a clarification. As shown on SK2, 4/A1-7011 shows conflicting dimensions to locate the posts for Stair ST401. Currently the model reflects S1-7009 & RFI T-1189 (SK					<b>ANSWER:</b> See attached CD RFI # 438 SK1 & SK2: This approval instruction on drawing 3264AB in the CS6 approval submittal (Package Number TG0701-78) requires a clarification. As shown on SK2, 4/A1-7011 shows conflicting dimensions to locate the posts for Stair ST401. Currently the model reflects S1-7009	



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	419, CD 413). Please confirm that S1-7009 and RFI T-1189 are correct or supply clarifying direction on which ST401 post locations are correct.					& RFI T-1189 (SK 419, CD 413). Please confirm that S1-7009 and RFI T-1189 are correct or supply clarifying direction on which ST401 post locations are correct.
<b>T-1435</b>	<b>SSS - ST401 Stiffener Plate Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>05/30/2014</b>	<b>06/09/2014</b>	<b>06/11/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 445 SK1 to SK3 for items 1 to 5: 1) It is not possible to supply a stiffener plate per 11/S1-7630 as requested in the CS6 approval submittal because it will foul the sloping beam connection. Confirm that is acceptable. 2) It is not possible to supply a stiffener plate per 11/S1-7630 as requested in the CS6 approval submittal because it will foul the sloping beam connection. Confirm that is acceptable. 3) It is not possible to supply a full depth stiffener plate per 1/S1-7600. Confirm that it is acceptable to terminate the stiffener 1" below the beam. 4) It is not possible to supply a stiffener plate at this location per 11/S1-7630 as requested in the CS6 approval submittal because the beam will not be erectable with the stiffener below the beam. Confirm that is acceptable. 5) It is not possible to supply a 12" high stiffener plate at this location per 11/S1-7630 because it will foul the bolts for the beam connection on the far side as shown. Confirm it is acceptable to terminate the stiffener as shown.			See attached CD RFI # 445 SK1 to SK3 for items 1 to 5: 1) It is not possible to supply a stiffener plate per 11/S1-7630 as requested in the CS6 approval submittal because it will foul the sloping beam connection. Confirm that is acceptable. 2) It is not possible to supply a stiffener plate per 11/S1-7630 as requested in the CS6 approval submittal because it will foul the sloping beam connection. Confirm that is acceptable. 3) It is not possible to supply a full depth stiffener plate per 1/S1-7600. Confirm that it is acceptable to terminate the stiffener 1" below the beam. 4) It is not possible to supply a stiffener plate at this location per 11/S1-7630 as requested in the CS6 approval submittal because the beam will not be erectable with the stiffener below the beam. Confirm that is acceptable. 5) It is not possible to supply a 12" high stiffener plate at this location per 11/S1-7630 because it will foul the bolts for the beam connection on the far side as shown. Confirm it is acceptable to terminate the stiffener as shown.			





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<b>T-1439</b>	<b>SSS - Oversized Holes in Cruciform Column Base Plate</b>	<b>Closed</b>	<b>CR</b>	<b>06/04/2014</b>	<b>06/14/2014</b>	<b>06/12/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Due to the thickness of the cruciform column base plate and the transfer girder top flange connection (7-1/2" to 9-1/2" thick). Please confirm it is acceptable to provide oversized holes in the base plate only at the slip critical connection as per AISC (see SK1). This will allow additional tolerance for threading the 2-1/2" diameter post tensioned rods varying from 14' to 38' long through the 7-1/2" to 9-1/2" thick steel. Note: The transfer girder top flange will remain a standard size hole (d+1/16").						<b>ANSWER:</b> Due to the thickness of the cruciform column base plate and the transfer girder top flange connection (7-1/2" to 9-1/2" thick). Please confirm it is acceptable to provide oversized holes in the base plate only at the slip critical connection as per AISC (see SK1). This will allow additional tolerance for threading the 2-1/2" diameter post tensioned rods varying from 14' to 38' long through the 7-1/2" to 9-1/2" thick steel. Note: The transfer girder top flange will remain a standard size hole (d+1/16").
<b>T-1440</b>	<b>SSS - Moment Frame Column Protected Zone</b>	<b>Closed</b>	<b>CR</b>	<b>06/04/2014</b>	<b>06/14/2014</b>	<b>06/10/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> After reviewing the contract documents for the moment frame protected zones, please reference attached SK1/SK2 and note the following: For the built-up moment frame beams, details 1, 3/S1-4201 have a note that directs Skanska to detail 10/S1-4202, which depicts the yellow striping and warning signage to be installed. However, the protected zone detail for the moment frame columns, 2/S1-4201, does not have any such requirement noted. Please confirm that detail 10/S14202 does not apply to the moment frame column protected zone shown on detail 2/S1-4201.						<b>ANSWER:</b> After reviewing the contract documents for the moment frame protected zones, please reference attached SK1/SK2 and note the following: For the built-up moment frame beams, details 1, 3/S1-4201 have a note that directs Skanska to detail 10/S1-4202, which depicts the yellow striping and warning signage to be installed. However, the protected zone detail for the moment frame columns, 2/S1-4201, does not have any such requirement noted. Please confirm that detail 10/S14202 does not apply to the moment frame column protected zone shown on detail 2/S1-4201.
<b>T-1441</b>	<b>SSS - Discrepancy for Slab Opening Location at ST301</b>	<b>Closed</b>	<b>CR</b>	<b>06/04/2014</b>	<b>06/14/2014</b>	<b>06/12/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 447 SK1: Based on the beam locations from grid D on S1-2403 and the slab opening location on A1-2883 the noted dimension should read 9 1/8". Please confirm or clarify the discrepancy.						<b>ANSWER:</b> See attached CD RFI # 447 SK1: Based on the beam locations from grid D on S1-2403 and the slab opening location on A1-2883 the noted dimension should read 9 1/8". Please confirm or clarify the discrepancy.



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T-1442	SSS - Discrepancies for Elevator Slab Opening Location PE202	Closed	CR	06/05/2014	06/15/2014	06/12/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:			ANSWER:			
See attached CD RFI # 448 SK1 & SK2 for items 1 & 2:			See attached CD RFI # 448 SK1 & SK2 for items 1 & 2:			
1) The location dimensions for the elevator pit shown in 2/S1-7101 and A1-2862 do not match as indicated by dimensions hi-lited in red. Please clarify which dimensions are correct.			1) The location dimensions for the elevator pit shown in 2/S1-7101 and A1-2862 do not match as indicated by dimensions hi-lited in red. Please clarify which dimensions are correct.			
2) The location dimensions for the elevator opening shown in 4/S1-7101 and A1-2882 do not match as indicated by dimensions hi-lited in red. Please clarify which dimensions are correct.			2) The location dimensions for the elevator opening shown in 4/S1-7101 and A1-2882 do not match as indicated by dimensions hi-lited in red. Please clarify which dimensions are correct.			





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<b>T-1444</b>	<b>SSS - Location of Grout &amp; Vent Holes</b>	<b>Closed</b>	<b>CR</b>	<b>06/09/2014</b>	<b>06/19/2014</b>	<b>06/17/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>As a follow-up to the review comment on drawing 1202 within Submittal Package TG0701-73.1, the elevation of the grout/vent holes on the pipe basket columns is called out to be 6'-0" above finished floor per S1-4003 (SK1). Skanska's modeler has taken the finished floor to be the top of steel dimension provided in the structural drawings (EL 56'-4") plus the slab thickness provided on S1-5000 (SK2). Based on this, the model currently reflects the grout/vent holes at EL 63'-6", 6'-0" above the structural slab high point. (TOS EL 56'-4" + 1'-2" [S8 slab] + 6'-0" = EL 63'-6") (SK3).</p> <p>Please confirm a grout/vent hole elevation of 63'-6" at the Bust Deck Level is acceptable. Note that shop drawings packages CS1 through CS7 have been released for fabrication based on the current model.</p>						
						<b>ANSWER:</b> <p>As a follow-up to the review comment on drawing 1202 within Submittal Package TG0701-73.1, the elevation of the grout/vent holes on the pipe basket columns is called out to be 6'-0" above finished floor per S1-4003 (SK1). Skanska's modeler has taken the finished floor to be the top of steel dimension provided in the structural drawings (EL 56'-4") plus the slab thickness provided on S1-5000 (SK2). Based on this, the model currently reflects the grout/vent holes at EL 63'-6", 6'-0" above the structural slab high point. (TOS EL 56'-4" + 1'-2" [S8 slab] + 6'-0" = EL 63'-6") (SK3).</p> <p>Please confirm a grout/vent hole elevation of 63'-6" at the Bust Deck Level is acceptable. Note that shop drawings packages CS1 through CS7 have been released for fabrication based on the current model.</p>
<b>T-1445</b>	<b>BGP - Foundation Wall Mix Placed in Partition Walls</b>	<b>Closed</b>	<b>01</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>06/12/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> <p>Please reference RFI T-1093 and cast-in-place mix designs TG0600-203.1 (#1557216 Foundation Walls) and TG0600-204.2 (#1558218 Slabs, Beams, and Shear Walls).</p> <p>The Foundation Wall cast-in-place mix design satisfies all requirements prescribed in Table 2-1 "Concrete Properties" (03 30 20.2.1) for the "Slabs, Beams, and Shear Walls" cast-in-place mix design. Please confirm both concrete mix designs (#1557216 and #1558218) can be used for Partition Walls.</p>						
						<b>ANSWER:</b> <p>Please reference RFI T-1093 and cast-in-place mix designs TG0600-203.1 (#1557216 Foundation Walls) and TG0600-204.2 (#1558218 Slabs, Beams, and Shear Walls).</p> <p>The Foundation Wall cast-in-place mix design satisfies all requirements prescribed in Table 2-1 "Concrete Properties" (03 30 20.2.1) for the "Slabs, Beams, and Shear Walls" cast-in-place mix design. Please confirm both concrete mix designs (#1557216 and #1558218) can be used for Partition Walls.</p>
<b>T-1446</b>	<b>SSS - PE 202 Dimension Discrepancies</b>	<b>Closed</b>	<b>CR</b>	<b>06/10/2014</b>	<b>06/20/2014</b>	<b>06/18/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>See attached CD RFI # 454 SK1 for items 1 &amp; 2:</p> <p>1) The dimensions locating the edge of slab shown in 4/S1-7101 &amp; A1-2892 do not match as noted by the hi-</p>						
						<b>ANSWER:</b> <p>See attached CD RFI # 454 SK1 for items 1 &amp; 2:</p> <p>1) The dimensions locating the edge of slab shown in 4/S1-7101 &amp; A1-2892 do not match as noted by the hi-</p>











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<b>T-1451</b>	<b>SSS - SE401 Missing Post Locations</b>	<b>Closed</b>	<b>CR</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>06/25/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 459 SK1:  The elevator post locations are not shown on the structural drawings. They are currently located in the model per the dimensions shown. Confirm the dimensions are acceptable or supply alternate dimensions.					<b>ANSWER:</b>  See attached CD RFI # 459 SK1:  The elevator post locations are not shown on the structural drawings. They are currently located in the model per the dimensions shown. Confirm the dimensions are acceptable or supply alternate dimensions.	
<b>T-1452</b>	<b>SSS - Beam Location Verification at Roof Level GL 12</b>	<b>Closed</b>	<b>CR</b>	<b>06/11/2014</b>	<b>06/21/2014</b>	<b>09/04/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  On attached sketch CD RFI # 458 SK1 on S1-2603 near grids 12/F verify the W12x14 beam is located 12'-0 5/16 from grid line 12 taken from the Revit model. Note per the architectural drawings there is no wall or curb etc to locate this beam.					<b>ANSWER:</b>  On attached sketch CD RFI # 458 SK1 on S1-2603 near grids 12/F verify the W12x14 beam is located 12'-0 5/16 from grid line 12 taken from the Revit model. Note per the architectural drawings there is no wall or curb etc to locate this beam.	
<b>T-1453</b>	<b>SSS - ASI 108 - Limits of Light Column Grout Port</b>	<b>Closed</b>	<b>CR</b>	<b>06/12/2014</b>	<b>06/22/2014</b>	<b>06/17/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  ASI 108 S1-6005 / Detail A specifies grout port holes drilled at one of four light column pipes at train platform level and grout port holes drilled at one of eight light column pipes above lower concourse level. Please confirm design intent is to drill holes in light column pipes at all four pipes at train box (2Ea x 4 pipes) and all eight pipes above concourse level (2Ea x 8 pipes).					<b>ANSWER:</b>  ASI 108 S1-6005 / Detail A specifies grout port holes drilled at one of four light column pipes at train platform level and grout port holes drilled at one of eight light column pipes above lower concourse level. Please confirm design intent is to drill holes in light column pipes at all four pipes at train box (2Ea x 4 pipes) and all eight pipes above concourse level (2Ea x 8 pipes).	
<b>T-1454</b>	<b>BGP - Lower Concourse Temporary Manlift</b>	<b>Closed</b>	<b>01</b>	<b>06/13/2014</b>	<b>06/23/2014</b>	<b>06/20/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>  In order to provide access to the first area of steel which is going to be erected on GL 10.1. it is necessary to install a temporary manlift on the concourse moment frame beam at GL-9.9 & D for approximately 2 months, please find					<b>ANSWER:</b>  In order to provide access to the first area of steel which is going to be erected on GL 10.1. it is necessary to install a temporary manlift on the concourse moment frame beam at GL-9.9 & D for	



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	<p>attached a layout plan, section and some typical manlift manufactures information. If it is acceptable to have the manlift footprint on the moment frame beam at Gl 9.9 -10.1, Webcor will prepare a formal submittal for the design team to review.</p> <p>Please confirm if this location would be acceptable.</p>					<p>approximately 2 months, please find attached a layout plan, section and some typical manlift manufactures information. If it is acceptable to have the manlift footprint on the moment frame beam at Gl 9.9 -10.1, Webcor will prepare a formal submittal for the design team to review.</p> <p>Please confirm if this location would be acceptable.</p>
<b>T-1455</b>	<b>BGP - Top of Shear Wall Lift 2, Beam Block-Out Pour Back</b>	<b>Closed</b>	<b>01</b>	<b>06/16/2014</b>	<b>06/26/2014</b>	<b>06/18/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>Please reference attached comprehensive lift drawing for walls Area 1 to 7 2nd lift (Drawings: W290A.1, W290A.2, W290B.1, W290B.2 W290C.1, W290C.2, W290D.1, W290D.2, W290E.1, W290E.2), and Gerdau wall rebar shop drawings SW-1, SW-2, and SW-3.</p> <p>Please reference clouded section on all drawings above. Due to constructability of formwork to build this portion of the block-out clouded on the attached comprehensive drawings, SCCI propose to pour this section back with the concourse. The difficulty arises due to forming through the embedded column steel at these locations in the shear walls.</p> <p>Please confirm this is acceptable.</p>					<p><b>ANSWER:</b></p> <p>Please reference attached comprehensive lift drawing for walls Area 1 to 7 2nd lift (Drawings: W290A.1, W290A.2, W290B.1, W290B.2 W290C.1, W290C.2, W290D.1, W290D.2, W290E.1, W290E.2), and Gerdau wall rebar shop drawings SW-1, SW-2, and SW-3.</p> <p>Please reference clouded section on all drawings above. Due to constructability of formwork to build this portion of the block-out clouded on the attached comprehensive drawings, SCCI propose to pour this section back with the concourse. The difficulty arises due to forming through the embedded column steel at these locations in the shear walls.</p> <p>Please confirm this is acceptable.</p>



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<b>T-1456</b>	<b>BSE - Concourse Deck Loading</b>	<b>Closed</b>	<b>01</b>	<b>06/16/2014</b>	<b>06/26/2014</b>	<b>06/17/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  See attached axle loadings for Hyundai forklift model 70DS-7E. The forklift is intended to be used on the concourse deck for removal of bracing and installation of rebracing.  Please confirm if it is acceptable to use the noted forklift on top of the concourse slab.						<b>ANSWER:</b>  See attached axle loadings for Hyundai forklift model 70DS-7E. The forklift is intended to be used on the concourse deck for removal of bracing and installation of rebracing.  Please confirm if it is acceptable to use the noted forklift on top of the concourse slab.
<b>T-1457</b>	<b>SSS - PE403 &amp; PE404 Framing Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>06/16/2014</b>	<b>06/26/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 457 SK1 & SK2 for items 1 to 7: 1) Confirm edge plate per 8/S1-5000 is required at 4 sides of the S7 slab shown in yellow. 2) Supply dimension. 3) T/Wall (shown in green) is at EL 86'-9 per A,B/S1-7134 & A/S1-7135. Please confirm. 4) Confirm the top of S7 slab shown in yellow is at EL 86'-10 3/4 based on the T/STEEL 86'-6 + 4 3/4" slab thickness. 5) Supply dimension. 6) Supply dimension. 7) Confirm edge plate per 8/S1-5000 is required at 4 sides for the 4 3/4" S7 slab and the curbs per A1-2965 will be a separate pour.						<b>ANSWER:</b>  See attached CD RFI # 457 SK1 & SK2 for items 1 to 7: 1) Confirm edge plate per 8/S1-5000 is required at 4 sides of the S7 slab shown in yellow. 2) Supply dimension. 3) T/Wall (shown in green) is at EL 86'-9 per A,B/S1-7134 & A/S1-7135. Please confirm. 4) Confirm the top of S7 slab shown in yellow is at EL 86'-10 3/4 based on the T/STEEL 86'-6 + 4 3/4" slab thickness. 5) Supply dimension. 6) Supply dimension. 7) Confirm edge plate per 8/S1-5000 is required at 4 sides for the 4 3/4" S7 slab and the curbs per A1-2965 will be a separate pour.
<b>T-1458</b>	<b>SSS - Work Points (WP) Drilled at Webs</b>	<b>Closed</b>	<b>CR</b>	<b>06/16/2014</b>	<b>06/26/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  Please see OIW shop drawing 2771-SK115. Following previous discussions between OIW and Skanska, it is apparent that survey Work Points accessible from both sides of a Roof Beam are required. It is our intent to provide a 1/2" dia. through hole at the web directly above the roof pins.  Please confirm that the drilled holes as shown on the attached sketch are acceptable for all roof beams.						<b>ANSWER:</b>  Please see OIW shop drawing 2771-SK115. Following previous discussions between OIW and Skanska, it is apparent that survey Work Points accessible from both sides of a Roof Beam are required. It is our intent to provide a 1/2" dia. through hole at the web directly above the roof pins.  Please confirm that the drilled holes as shown on the attached sketch are acceptable for all roof beams.



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T-1459	SSS - Paint Limits on Machined Cap Plate	Closed	CR	06/16/2014	06/26/2014	06/25/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:		ANSWER:				
Please confirm the paint limits indicated on SK1 are acceptable for the coating required on the machined surface of the cap plates as per RFI T-1230.1.		Please confirm the paint limits indicated on SK1 are acceptable for the coating required on the machined surface of the cap plates as per RFI T-1230.1.				
T-1460	BGP - Telecom Sweep Conflict	Closed	01	06/16/2014	06/26/2014	06/19/2014
From: Webcor Construction LP Claude Titcher						
REQUEST:		ANSWER:				
At Gridline 8.9/A one of the 4" 90 degree telecom sweeps conflicts with an existing internal bracing strut (See attached photo). This sweep was changed from an original 30" radius sweep to a 36" radius sweep.		At Gridline 8.9/A one of the 4" 90 degree telecom sweeps conflicts with an existing internal bracing strut (See attached photo). This sweep was changed from an original 30" radius sweep to a 36" radius sweep.				
Please confirm which option SCCI is to proceed with: 1)The first telecom sweep could be installed 7'-1" West of GL 9 (instead of 5'-7" shown in comprehensive layout drawing) and the remaining 2 installed with the specified 1' -6" offset.		Please confirm which option SCCI is to proceed with: 1)The first telecom sweep could be installed 7'-1" West of GL 9 (instead of 5'-7" shown in comprehensive layout drawing) and the remaining 2 installed with the specified 1' -6" offset.				
2)The 36" radius telecom sweep could be replaced with the original 30" radius telecom sweep at this location only.		2)The 36" radius telecom sweep could be replaced with the original 30" radius telecom sweep at this location only.				
T-1461	BGP - CMU Pier Sizing	Closed	01	06/17/2014	06/27/2014	06/25/2014
From: Webcor Construction LP Claude Titcher						
REQUEST:		ANSWER:				
Detail 9 on plan sheet S1-9000 shows the CMU pier sizing based on wall height and door opening width. The height of the CMU wall significantly affects the CMU pier sizing. The AI-9240 drawings which were provided in the response to RFI T-1410 are the only drawings which show possible CMU wall heights, however those heights are only based on scale dimensions. Please provide the heights of the CMU walls or confirm SCCI is to assume the maximum CMU wall height of 26'-8" for all CMU walls.		Detail 9 on plan sheet S1-9000 shows the CMU pier sizing based on wall height and door opening width. The height of the CMU wall significantly affects the CMU pier sizing. The AI-9240 drawings which were provided in the response to RFI T-1410 are the only drawings which show possible CMU wall heights, however those heights are only based on scale dimensions. Please provide the heights of the CMU walls or confirm SCCI is to assume the maximum CMU wall height of 26'-8" for all CMU walls.				

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<b>T-1462</b>	<b>SSS - TR5 Vertical Shear Studs at GL5C</b>	<b>Closed</b>	<b>CR</b>	<b>06/17/2014</b>	<b>06/27/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b> Please reference S1-2302, grid 5/C. At this location and similar conditions, section 2/S1-3707 is shown. Through this detail section 6/S1-3702 is cut. This detail calls for 6 rows of 6 studs @ 6" placed on the top flange of the transfer girder. At conditions where the end of the transfer girder is 4'-8" from the grid line there is not enough room to install the studs as indicated. See attached SK-1.  Note: This condition also occurred at GL9/C and detail 5/S1-3707 was provided with field order 027 to show 4 rows of 4 studs. Confirm this detail is also acceptable at 2/S1-3707.		<b>ANSWER:</b> Please reference S1-2302, grid 5/C. At this location and similar conditions, section 2/S1-3707 is shown. Through this detail section 6/S1-3702 is cut. This detail calls for 6 rows of 6 studs @ 6" placed on the top flange of the transfer girder. At conditions where the end of the transfer girder is 4'-8" from the grid line there is not enough room to install the studs as indicated. See attached SK-1.  Note: This condition also occurred at GL9/C and detail 5/S1-3707 was provided with field order 027 to show 4 rows of 4 studs. Confirm this detail is also acceptable at 2/S1-3707.				
<b>T-1463</b>	<b>SSS - Shear Studs and Rebar Holes at TR4 GL4G</b>	<b>Closed</b>	<b>CR</b>	<b>06/17/2014</b>	<b>06/27/2014</b>	<b>06/24/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 460 SK1 & SK2 for items 1 & 2: 1) It is not clear how many studs are required. See SK2 and confirm the location and number of studs. 2) It is not clear how many 2" diameter holes are required. See SK2 and confirm the location and number of holes.		<b>ANSWER:</b> See attached CD RFI # 460 SK1 & SK2 for items 1 & 2: 1) It is not clear how many studs are required. See SK2 and confirm the location and number of studs. 2) It is not clear how many 2" diameter holes are required. See SK2 and confirm the location and number of holes.				
<b>T-1464</b>	<b>BRP - Copper Drain Pipe Details</b>	<b>Closed</b>	<b>01</b>	<b>06/18/2014</b>	<b>06/28/2014</b>	<b>07/08/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titche						
<b>REQUEST:</b> Please reference the attached drawings, A1-2817 and A1-8881. Please provide further details regarding the added 2" diameter copper overflow drain pipe clouded in the attached drawings. -  1. Please specify the type of valve needed for the copper drain pipe. 2. Is the copper pipe insulated? If so, please provide specific details on insulation. 3. What type of install method should be used such as, is the connection brazed, soldered, butt welded, mechanical,		<b>ANSWER:</b> Please reference the attached drawings, A1-2817 and A1-8881. Please provide further details regarding the added 2" diameter copper overflow drain pipe clouded in the attached drawings. -  1. Please specify the type of valve needed for the copper drain pipe. 2. Is the copper pipe insulated? If so, please provide specific details on insulation. 3. What type of install method should be used such as, is the connection brazed, soldered, butt welded,				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>etc? 4. Please provide further details on how the pipe terminates at the SJ joint. 5. Please clarify how the 2" diameter hose adapters connect to the 2" diameter copper drain pipes and how they are sealed to flex flashing and SJ assembly.</p>					<p>mechanical, etc? 4. Please provide further details on how the pipe terminates at the SJ joint. 5. Please clarify how the 2" diameter hose adapters connect to the 2" diameter copper drain pipes and how they are sealed to flex flashing and SJ assembly.</p>
<b>T-1465</b>	<b>SSS - Seal Weld at Edge of Backing Bar</b>	<b>Closed</b>	<b>CR</b>	<b>06/18/2014</b>	<b>06/28/2014</b>	<b>06/25/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>Reference attached RFI OIW 033. The same configuration of weld joint and backing bar used on the fabricated roof nodes is also used on the bus-deck nodes for each pair of shear plates. It is our intent to provide a continuous seal weld at the end of the backing plate and the shear plates.</p> <p>Please confirm that the noted seal weld is acceptable for all shear plate connections to bus-deck nodes.</p>						<b>ANSWER:</b> <p>Reference attached RFI OIW 033. The same configuration of weld joint and backing bar used on the fabricated roof nodes is also used on the bus-deck nodes for each pair of shear plates. It is our intent to provide a continuous seal weld at the end of the backing plate and the shear plates.</p> <p>Please confirm that the noted seal weld is acceptable for all shear plate connections to bus-deck nodes.</p>
<b>T-1465.1</b>	<b>SSS - Seal Weld at Edge of Backing Bar</b>	<b>Closed</b>	<b>CR</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/15/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>Reference attached RFI OIW 033. The same configuration of weld joint and backing bar used on the fabricated roof nodes is also used on the bus-deck nodes for each pair of shear plates. It is our intent to provide a continuous seal weld at the end of the backing plate and the shear plates.</p> <p>Please confirm that the noted seal weld is acceptable for all shear plate connections to bus-deck nodes.</p>						<b>ANSWER:</b> <p>Reference attached RFI OIW 033. The same configuration of weld joint and backing bar used on the fabricated roof nodes is also used on the bus-deck nodes for each pair of shear plates. It is our intent to provide a continuous seal weld at the end of the backing plate and the shear plates.</p> <p>Please confirm that the noted seal weld is acceptable for all shear plate connections to bus-deck nodes.</p>
<b>T-1466</b>	<b>SSS - Exposed Flange at Step in Slab</b>	<b>Closed</b>	<b>CR</b>	<b>06/18/2014</b>	<b>06/28/2014</b>	<b>06/30/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						









<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1468</b>	<b>SCS - WPS Qualification - Lenton Weldable Couplers</b>	<b>Closed</b>	<b>01</b>	<b>06/23/2014</b>	<b>07/03/2014</b>	<b>07/03/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
The Lenton welding information provides guidance for qualification of processes; however there are inconsistencies when trying to fulfill AWS D1.1 (2010) requirements. The materials used for manufacturing weldable couplers size #9-11 are Grade 1030/1035. These grades of material are not listed in AWS D1.1 (2010) Table 3. 1, although Grade 1030 is listed in AWS B 2.1 Table D.1 as a Group 2 material (table attached). In order to move forward with welding the Lenton weldable couplers to the Transfer Girders, TMF proposes that FCA W Grade 70 filler material to be used manufacturer ESAB 7100 Ultra, Classification E71 T-9, AWS Specification AWS 5.20"D". One mockup of the actual coupler welded to a Group 2 plate and macro etched would be provided as a supplement.		The Lenton welding information provides guidance for qualification of processes; however there are inconsistencies when trying to fulfill AWS D1.1 (2010) requirements. The materials used for manufacturing weldable couplers size #9-11 are Grade 1030/1035. These grades of material are not listed in AWS D1.1 (2010) Table 3. 1, although Grade 1030 is listed in AWS B 2.1 Table D.1 as a Group 2 material (table attached). In order to move forward with welding the Lenton weldable couplers to the Transfer Girders, TMF proposes that FCA W Grade 70 filler material to be used manufacturer ESAB 7100 Ultra, Classification E71 T-9, AWS Specification AWS 5.20"D". One mockup of the actual coupler welded to a Group 2 plate and macro etched would be provided as a supplement.				
Please confirm this is acceptable.		Please confirm this is acceptable.				
<b>T-1469</b>	<b>BGP - Seismic Joint Rebar and Continuous Plate Conflict</b>	<b>Closed</b>	<b>01</b>	<b>06/23/2014</b>	<b>07/03/2014</b>	<b>06/26/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please reference the attached drawings, S1-3010 and S1-3206. The continuous plate and rebar clouded in the attached drawings is conflicting. Please provide a minimum clearance between the rebar and continuous plate.		Please reference the attached drawings, S1-3010 and S1-3206. The continuous plate and rebar clouded in the attached drawings is conflicting. Please provide a minimum clearance between the rebar and continuous plate.				
<b>T-1470</b>	<b>BGP - Steel Jacket at Column GL1.4/D.4</b>	<b>Closed</b>	<b>01</b>	<b>06/24/2014</b>	<b>07/04/2014</b>	<b>06/25/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference attached drawings. The column at Gridlines 1.4/D.4 requires steel jacketing as called out in AAI comment in Submittal Package TG0600-141 sheet G101.0. During the TG0600-141 coordination meeting held on 6/18/14 it was confirmed that the typical sleeve detail as shown on 3/A1-9208 will apply. Sheet A1-2102 implies a one-sided sleeve on the south face of		Reference attached drawings. The column at Gridlines 1.4/D.4 requires steel jacketing as called out in AAI comment in Submittal Package TG0600-141 sheet G101.0. During the TG0600-141 coordination meeting held on 6/18/14 it was confirmed that the typical sleeve detail as shown on 3/A1-9208 will apply. Sheet A1-2102 implies a one-				



<i>Number</i>	<i>Subject</i>	<i>Status</i>	<i>Choice Required</i>	<i>Date Created</i>	<i>Date Required</i>	<i>Date Answered</i>
	<p>column, sheet A1-9214 implies a two piece sleeve, and sheet 3/A1-9208 shows the typical detail with no modifications. Please confirm a detail specific to this location showing how the jacket is to be wrapped and interfaced with adjacent partition walls.</p>					<p>sided sleeve on the south face of column, sheet A1-9214 implies a two piece sleeve, and sheet 3/A1-9208 shows the typical detail with no modifications. Please confirm a detail specific to this location showing how the jacket is to be wrapped and interfaced with adjacent partition walls.</p>
T-1471	<p><b>BGP - Seismic Joint Bottom Embed Plate at Pits</b></p> <p><b>From:</b> Webcor Construction LP                      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>Please reference attached Contract Drawings and photos.</p> <p>The east face of the north pit along GL 35 is scaled at 6'-8" from face of CDSM wall (S1-2027 attached). The as-built dimension ranges from 5'-0" to 5'-11" (S1-2027). Also, the center pit is scaled at 5'-8" to face of CDSM wall while the as-built is 5'-3" (S1-2027).</p> <p>Detail 4 on S1-3010 includes a scaled dimension of 5'-0" from face of CDSM wall to the inside face of the west half of the seismic embed (See S1-3010 attached). Therefore, the bottom embed plate will need to extend 2'-6" beyond the 5' dimension per Detail 7 of AI -8881 (attached).</p> <p>Since the as-built dimensions of the edge of pits range from 5'-0" to 5'-11" from face of CDSM wall, the bottom embed plate will overhang the pit depression 2'-6" to 1'-7". The plate could be reduced up to 1'-11" (2'-6" minus the 8" for clamping detail) to accommodate the edge of pits and allow for waterproofing clamping detail. Is this acceptable?</p> <p>Please provide further details at locations where encroaching pits do not allow for sufficient room for bottom embed plate and clamping assembly.</p>	Closed	01	06/24/2014	07/04/2014	07/07/2014
						<p><b>ANSWER:</b></p> <p>Please reference attached Contract Drawings and photos.</p> <p>The east face of the north pit along GL 35 is scaled at 6'-8" from face of CDSM wall (S1-2027 attached). The as-built dimension ranges from 5'-0" to 5'-11" (S1-2027). Also, the center pit is scaled at 5'-8" to face of CDSM wall while the as-built is 5'-3" (S1-2027).</p> <p>Detail 4 on S1-3010 includes a scaled dimension of 5'-0" from face of CDSM wall to the inside face of the west half of the seismic embed (See S1-3010 attached). Therefore, the bottom embed plate will need to extend 2'-6" beyond the 5' dimension per Detail 7 of AI -8881 (attached).</p> <p>Since the as-built dimensions of the edge of pits range from 5'-0" to 5'-11" from face of CDSM wall, the bottom embed plate will overhang the pit depression 2'-6" to 1'-7". The plate could be reduced up to 1'-11" (2'-6" minus the 8" for clamping detail) to accommodate the edge of pits and allow for waterproofing clamping detail. Is this acceptable?</p> <p>Please provide further details at locations where encroaching pits do not allow for sufficient room for bottom embed plate and clamping assembly.</p>
T-1471.1	<p><b>BGP - Seismic Joint Bottom Embed Plate at Pits</b></p> <p><b>From:</b> Webcor Construction LP                      Claude Titcher</p>	Closed	01	07/28/2014	08/07/2014	08/06/2014





<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1473</b>	<b>BGP - Lower Concourse Service Corridor Dimensions on West Wall</b>	<b>Closed</b>	<b>01</b>	<b>06/25/2014</b>	<b>07/05/2014</b>	<b>07/02/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI T-0576 regarding the offset of the West Foundation Walls to account for the CDSM Encroachment. The west wall (GL 1) was offset 3 1/8" to the east while the southwest wall (GL XI-1) was offset 1 7/8" to the northeast. Currently there are no dimensions explicitly shown in the contract drawings for the width of the service corridor that runs along the very western edge of the Lower Concourse in Area 1 and 3. However there is a dimension shown on A1-3006 & A1-7004 that shows the eastern face of the service corridor at 5' to the west of GL 1.4 which translates into a service corridor width of 9' -8 7/8". Is it the designers intent to offset the service corridor walls similar to the foundation walls to maintain the 10' corridor width, or is it the designers intent to shrink the width of the service corridor and keep the interior walls in place? Please provide dimensions for the service corridor width.			Please reference RFI T-0576 regarding the offset of the West Foundation Walls to account for the CDSM Encroachment. The west wall (GL 1) was offset 3 1/8" to the east while the southwest wall (GL XI-1) was offset 1 7/8" to the northeast. Currently there are no dimensions explicitly shown in the contract drawings for the width of the service corridor that runs along the very western edge of the Lower Concourse in Area 1 and 3. However there is a dimension shown on A1-3006 & A1-7004 that shows the eastern face of the service corridor at 5' to the west of GL 1.4 which translates into a service corridor width of 9' -8 7/8". Is it the designers intent to offset the service corridor walls similar to the foundation walls to maintain the 10' corridor width, or is it the designers intent to shrink the width of the service corridor and keep the interior walls in place? Please provide dimensions for the service corridor width.			
<b>T-1473.1</b>	<b>BGP - Lower Concourse Service Corridor Dimensions on West Wall</b>	<b>Closed</b>	<b>01</b>	<b>07/07/2014</b>	<b>07/17/2014</b>	<b>07/15/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
With the response from RFI T-1473 and the contract drawings, SCCI has enough dimensions to model the service corridor running north-south in Area 3. However there is not enough information to model the alignment of the service corridor running northwest-southeast. Please provide a work point with dimensions somewhere along the east face of the service corridor (preferably at the southernmost point of the service corridor at Shear Wall 390A) or provide an angle at which the service corridor changes direction above Gridline K.			With the response from RFI T-1473 and the contract drawings, SCCI has enough dimensions to model the service corridor running north-south in Area 3. However there is not enough information to model the alignment of the service corridor running northwest-southeast. Please provide a work point with dimensions somewhere along the east face of the service corridor (preferably at the southernmost point of the service corridor at Shear Wall 390A) or provide an angle at which the service corridor changes direction above Gridline K.			
<b>T-1474</b>	<b>BGP - Seismic Joint Fire and Smoke Barrier</b>	<b>Closed</b>	<b>01</b>	<b>06/26/2014</b>	<b>07/06/2014</b>	<b>06/26/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>Please reference the attached drawing, AI-8881. Per ASI-107, a fire and smoke barrier is added at the top of Gridline 35 on 3/AI-8881. Please confirm: -</p> <p>1. The fire and smoke barrier is not to be incorporated in TG06 Contract.</p> <p>2. There will be no embeds associated with the fire and smoke barrier that will need to be installed in TG06 Contract.</p> <p>3. Please verify who will be supplying the flexible drain to be installed in the top wall poured in TG06 Contract.</p>					
	<p>Please reference the attached drawing, AI-8881. Per ASI-107, a fire and smoke barrier is added at the top of Gridline 35 on 3/AI-8881. Please confirm: -</p> <p>1. The fire and smoke barrier is not to be incorporated in TG06 Contract.</p> <p>2. There will be no embeds associated with the fire and smoke barrier that will need to be installed in TG06 Contract.</p> <p>3. Please verify who will be supplying the flexible drain to be installed in the top wall poured in TG06 Contract.</p>					
<b>T-1475</b>	<b>BGP - Plumbing Penetration Sizes in Concourse Area 8</b>	<b>Void</b>	<b>01</b>	<b>06/27/2014</b>	<b>07/07/2014</b>	
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>						
SCCI is in receipt of response to Submittal TG0600-122 for Comprehensive Layout Drawings at the Lower Concourse A8-9 on 6/20/2014. On this submittal response, it was noted on pg D2.33: "For future washroom wall layout and plumbing penetrations- Refer to forthcoming Confirming RFI."						
RFI T-1410 revised the plumbing penetrations in this area. Please provide concourse blackout sizes for these penetrations (including "future plbg").						
Sketch is based on A1-2844						
<b>ANSWER:</b>						
SCCI is in receipt of response to Submittal TG0600-122 for Comprehensive Layout Drawings at the Lower Concourse A8-9 on 6/20/2014. On this submittal response, it was noted on pg D2.33: "For future washroom wall layout and plumbing penetrations- Refer to forthcoming Confirming RFI."						
RFI T-1410 revised the plumbing penetrations in this area. Please provide concourse blackout sizes for these penetrations (including "future plbg").						
Sketch is based on A1-2844						



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-1476</b>	<b>SCS - Concrete Form Support Using CDSM Piles</b>	<b>Closed</b>	<b>01</b>	<b>06/30/2014</b>	<b>07/10/2014</b>	<b>07/09/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Shimmick is planning to use a similar form system for the fourth lift of the foundation wall to that used on the second wall lift. However, the second wall lift form system is a cantilever system and is supported by 1 row of anchors, excerpting significant force on the wall lift below. Shimmick proposes to add a tie to the top of the forms and connect to the CDSM soldier piles in addition to using the third lift anchor to provide better support. The soldier piles extend above the ground level and this added tie would not affect the waterproofing (installed by others). See the attached marked up drawings for reference			Shimmick is planning to use a similar form system for the fourth lift of the foundation wall to that used on the second wall lift. However, the second wall lift form system is a cantilever system and is supported by 1 row of anchors, excerpting significant force on the wall lift below. Shimmick proposes to add a tie to the top of the forms and connect to the CDSM soldier piles in addition to using the third lift anchor to provide better support. The soldier piles extend above the ground level and this added tie would not affect the waterproofing (installed by others). See the attached marked up drawings for reference			
Please confirm it is acceptable for the fourth lift form system to tie to the CDSM soldier piles.			Please confirm it is acceptable for the fourth lift form system to tie to the CDSM soldier piles.			
<b>T-1477</b>	<b>SSS - Comment on TR19.1 Package CS09</b>	<b>Closed</b>	<b>CR</b>	<b>06/30/2014</b>	<b>07/10/2014</b>	<b>07/03/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 465 SK1: The 1/2" fillet welds are per details C/S1-4304 to 1/S1-4354 to 7/S14354 to 2/S1-4350. Note 5 on 1/S1-5052 does not apply as detail 1/S1-5052 does not apply at the column connections to Transfer Girder connections on Grid 19.1. Please confirm no further action is required.			See attached CD RFI # 465 SK1: The 1/2" fillet welds are per details C/S1-4304 to 1/S1-4354 to 7/S14354 to 2/S1-4350. Note 5 on 1/S1-5052 does not apply as detail 1/S1-5052 does not apply at the column connections to Transfer Girder connections on Grid 19.1. Please confirm no further action is required.			
<b>T-1478</b>	<b>SSS - Paint Boundaries at Roof Level Beams</b>	<b>Closed</b>	<b>CR</b>	<b>06/30/2014</b>	<b>07/10/2014</b>	<b>07/11/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1. At the Roof Level EBF Link beams, the AESS fabricated roof nodes weld to the underside of the built-up beam protected zone as indicated in the sketch attached. Based the thickness of the SFRM coating and the profile of the node-to-flange weld, the coating boundary line between the protected zone paint and AESS primer is proposed at an elevation 3" below the bottom flange. This will allow for a clear delineation between the two coatings			1. At the Roof Level EBF Link beams, the AESS fabricated roof nodes weld to the underside of the built-up beam protected zone as indicated in the sketch attached. Based the thickness of the SFRM coating and the profile of the node-to-flange weld, the coating boundary line between the protected zone paint and AESS primer is proposed at an elevation 3" below the bottom flange. This will allow for a clear			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
	<p>by avoiding the weld area, as well as allow for compatibility between the SFRM coating and prime coat. Please confirm this is acceptable.</p> <p>2. The same elevation for the AESS to SFRM boundary is proposed at the Roof Level Brace beams; however, rather than the protected zone paint, the area above the AESS primer will be bare steel to receive SFRM by others. Please confirm it is acceptable to terminate the AESS paint 3" below the bottom flange of the Roof Level Brace beams.</p>				<p>delineation between the two coatings by avoiding the weld area, as well as allow for compatibility between the SFRM coating and prime coat. Please confirm this is acceptable.</p> <p>2. The same elevation for the AESS to SFRM boundary is proposed at the Roof Level Brace beams; however, rather than the protected zone paint, the area above the AESS primer will be bare steel to receive SFRM by others. Please confirm it is acceptable to terminate the AESS paint 3" below the bottom flange of the Roof Level Brace beams.</p>	
<b>T-1479</b>	<b>SSS - Cylindrical Plate Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>06/30/2014</b>	<b>07/10/2014</b>	<b>07/18/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> <p>1) It appears the structural drawings and the architectural drawings do not supply consistent information for the location of the AESS enclosures. Please clarify where the AESS enclosures are required.</p> <p>2) The AESS enclosure plate on A1-8692 is weldable only on one vertical side where it contacts the node. Confirm that is the intent.</p> <p>3) The AESS enclosure plate on A1-8693 is weldable only on one vertical sides and the bottom side where it contacts the node. Confirm that is the intent.</p> <p>4) Confirm all AESS enclosure plates are A572 GR50 material.</p>					<b>ANSWER:</b> <p>1) It appears the structural drawings and the architectural drawings do not supply consistent information for the location of the AESS enclosures. Please clarify where the AESS enclosures are required.</p> <p>2) The AESS enclosure plate on A1-8692 is weldable only on one vertical side where it contacts the node. Confirm that is the intent.</p> <p>3) The AESS enclosure plate on A1-8693 is weldable only on one vertical sides and the bottom side where it contacts the node. Confirm that is the intent.</p> <p>4) Confirm all AESS enclosure plates are A572 GR50 material.</p>	
<b>T-1480</b>	<b>SSS - ST301 Shear Plate to HSS Connection</b>	<b>Closed</b>	<b>CR</b>	<b>06/30/2014</b>	<b>07/10/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> <p>See attached CD RFI # 471 SK1: The shear plate for the W10x54 only partially attaches to the HSS12x6. Confirm that is acceptable or supply an alternate detail.</p>					<b>ANSWER:</b> <p>See attached CD RFI # 471 SK1: The shear plate for the W10x54 only partially attaches to the HSS12x6. Confirm that is acceptable or supply an alternate detail.</p>	





Webcor/Obayashi Joint Venture

PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

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Date: 02/11/2015  
Time: 07:01 AM  
Job: 30100

<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1481</b>	<b>Pier and Opening Sizes on Deck D214-D215</b>	<b>Closed</b>	<b>01</b>	<b>06/30/2014</b>	<b>07/10/2014</b>	<b>07/08/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Because the specific sizes are not noted on the Contract Drawings provided, and wall heights are not shown on the Contract Drawing (Refer to RFI T-1461), please confirm Pier and Opening Sizes for Deck D214 to D215 are per the attached drawings.						<b>ANSWER:</b> Because the specific sizes are not noted on the Contract Drawings provided, and wall heights are not shown on the Contract Drawing (Refer to RFI T-1461), please confirm Pier and Opening Sizes for Deck D214 to D215 are per the attached drawings.
<b>T-1482</b>	<b>SSS - Slab Opening Location at Roof Park GL5D</b>	<b>Closed</b>	<b>CR</b>	<b>06/30/2014</b>	<b>07/10/2014</b>	<b>07/15/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 472 SK1: Please supply the size and location for the noted slab opening.						<b>ANSWER:</b> See attached CD RFI # 472 SK1: Please supply the size and location for the noted slab opening.
<b>T-1483</b>	<b>BGP - Dimension to Door on North Vehicle Ramp Wall</b>	<b>Closed</b>	<b>01</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/07/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> The door in question is at the east end of the north Vehicle Ramp Wall. There is a dimension on AI-2853, dimensioned at 7'-5 7/4" from GL 6 to the end of the wall and it is shown without a door attached. On all other drawings a door is shown at the east end of the wall, but there are no other dimensions indicating the east-west location of the door. Please confirm if it is the designer's intent to have the door placed on the end of the wall shown in AI-2853 so the east face of the door is 6'-5 1/4" off of GL 6 or if the door is to be placed per scale at 6'-3" off of GL 6.						<b>ANSWER:</b> The door in question is at the east end of the north Vehicle Ramp Wall. There is a dimension on AI-2853, dimensioned at 7'-5 7/4" from GL 6 to the end of the wall and it is shown without a door attached. On all other drawings a door is shown at the east end of the wall, but there are no other dimensions indicating the east-west location of the door. Please confirm if it is the designer's intent to have the door placed on the end of the wall shown in AI-2853 so the east face of the door is 6'-5 1/4" off of GL 6 or if the door is to be placed per scale at 6'-3" off of GL 6.
<b>T-1484</b>	<b>SSS - Missing Slab Opening Locations GL16D</b>	<b>Closed</b>	<b>CR</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/14/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 478 SK1: Please supply the missing slab opening locations.						<b>ANSWER:</b> See attached CD RFI # 478 SK1: Please supply the missing slab opening locations.





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<b>T-1485</b>	<b>SSS - East and West End Hole Dimension Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/14/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> Please confirm that grout hole "5" as shown on the attached sketch, SK1, is required on the west side only on grid 1 and on the east side only on grid 33.5. If not please clarify locations.					<b>ANSWER:</b> Please confirm that grout hole "5" as shown on the attached sketch, SK1, is required on the west side only on grid 1 and on the east side only on grid 33.5. If not please clarify locations.	
<b>T-1485.1</b>	<b>SSS - East and West End Hole Dimension Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 463.1 SK1 for items 1 to 3: 1) Confirm Hole 5 is on the west side of the columns on Grid 33.5 and on the east side of the columns on Grid 1. 2) Supply elevation for Hole 1. 3) Supply elevation for Hole 1.					<b>ANSWER:</b> See attached CD RFI # 463.1 SK1 for items 1 to 3: 1) Confirm Hole 5 is on the west side of the columns on Grid 33.5 and on the east side of the columns on Grid 1. 2) Supply elevation for Hole 1. 3) Supply elevation for Hole 1.	
<b>T-1486</b>	<b>SSS - Missing Hole Locations for CP3, CP4 &amp; CP8 Connections</b>	<b>Closed</b>	<b>CR</b>	<b>07/01/2014</b>	<b>07/11/2014</b>	<b>07/14/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> Please supply the size and location for the bolt holes at the CP3, CP4, and CP8 connections as indicated on the attached SK1, SK2, SK3, and SK4 drawings.					<b>ANSWER:</b> Please supply the size and location for the bolt holes at the CP3, CP4, and CP8 connections as indicated on the attached SK1, SK2, SK3, and SK4 drawings.	
<b>T-1486.1</b>	<b>SSS - Missing Hole Locations for CP3, CP4 &amp; CP8 Connections</b>	<b>Closed</b>	<b>CR</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>07/30/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 476.1 SK1 for item 1:  1) 5" diameter access holes are provided for the outer bolts but no access holes are shown to get access to the inner bolts. Please review and advise.					<b>ANSWER:</b> See attached CD RFI # 476.1 SK1 for item 1:  1) 5" diameter access holes are provided for the outer bolts but no access holes are shown to get access to the inner bolts. Please review and advise.	
<b>T-1486.2</b>	<b>SSS - Missing Hole Locations for CP3, CP4 &amp; CP8 Connections</b>	<b>Closed</b>	<b>CR</b>	<b>08/15/2014</b>	<b>08/25/2014</b>	<b>09/09/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						



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T-1487	SSS - Shear Plate Weld Connection Clarification	Closed	CR	07/02/2014	07/12/2014	07/14/2014
<div><div>From: Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
REQUEST:			ANSWER:			
For roof level shear plate connection details at GL11D & GL11F, S1-2603 refers to 3/S1-5013. This detail calls for a 3sided PJP weld at the shear plates, with a t-1/8 bevel (t=shear plate thickness). The shear plate located at the center of the web has a thickness of 2 ½", which results in a 2 3/8" bevel. The column web at this location has a thickness of 1 ¼"; the stiffener plates top and bottom have a thickness of ½". A 2 3/8" PJP weld will greatly distort the ½" stiffener plates. The weld size should not exceed the thickness of the thinnest member. Also, Detail 3/S1-5013 refers to Connection Detail 4/S1-5013 which in turn refers to Shear Connection Detail 3/S1-5013. Since both sides of the shear plate are accessible (note 1), is a ½" fillet both sides as shown in 3/S1-5011 acceptable for this location?			For roof level shear plate connection details at GL11D & GL11F, S1-2603 refers to 3/S1-5013. This detail calls for a 3sided PJP weld at the shear plates, with a t-1/8 bevel (t=shear plate thickness). The shear plate located at the center of the web has a thickness of 2 ½", which results in a 2 3/8" bevel. The column web at this location has a thickness of 1 ¼"; the stiffener plates top and bottom have a thickness of ½". A 2 3/8" PJP weld will greatly distort the ½" stiffener plates. The weld size should not exceed the thickness of the thinnest member. Also, Detail 3/S1-5013 refers to Connection Detail 4/S1-5013 which in turn refers to Shear Connection Detail 3/S1-5013. Since both sides of the shear plate are accessible (note 1), is a ½" fillet both sides as shown in 3/S1-5011 acceptable for this location?			
T-1488	SSS - NDT of Centrifugally Cast Pipes	Closed	CR	07/02/2014	07/12/2014	07/11/2014
<div><div>From: Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
REQUEST:			ANSWER:			
Specification 05 15 22, section 2.1.A.3 indicates magnetic particle testing (MT) shall be performed according to the criteria of ASTM A903, which in turn indicates that liquid dye penetrant testing (PT) may be performed, not in lieu of but in addition to MT, to give further information in cases where the MT results may be unclear.  Please confirm if PT is an acceptable NDT method per ASTM A903, in addition to the MT already prescribed. For clarification, no reduction in the amount of MT is requested.			Specification 05 15 22, section 2.1.A.3 indicates magnetic particle testing (MT) shall be performed according to the criteria of ASTM A903, which in turn indicates that liquid dye penetrant testing (PT) may be performed, not in lieu of but in addition to MT, to give further information in cases where the MT results may be unclear.  Please confirm if PT is an acceptable NDT method per ASTM A903, in addition to the MT already prescribed. For clarification, no reduction in the amount of MT is requested.			
T-1489	SSS - Pin Coating Clarification	Closed	CR	07/02/2014	07/12/2014	07/14/2014
<div><div>From: Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
REQUEST:			ANSWER:			
Structural Steel Specification - 05 10 00, states that the pins for the basket columns, a Type 2(M) drag connection, are to be galvanized to class A or Class B-1 per ASTM			Structural Steel Specification - 05 10 00, states that the pins for the basket columns, a Type 2(M) drag connection, are to be galvanized to class A or Class			



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	<p>A153. After reading this note, and looking through all of the applicable details, Skanska interpreted all of the remaining drag connection pins to be provided as bare steel. In the response to the submittal package TG0701-60.1 SSS - PIN Drawings (Central Zone), drawing A9312, a Type 2 (N2) drag connection at GL 11, was marked to receive a galvanized coating. Similar pin connections occur at GL 21 &amp; GL 22 (Type 2-P) and GL 28 (Type 2-N2). Does the design team want these pins galvanized too? If so, please provide the standard required.</p>					<p>B-1 per ASTM A153. After reading this note, and looking through all of the applicable details, Skanska interpreted all of the remaining drag connection pins to be provided as bare steel. In the response to the submittal package TG0701-60.1 SSS - PIN Drawings (Central Zone), drawing A9312, a Type 2 (N2) drag connection at GL 11, was marked to receive a galvanized coating. Similar pin connections occur at GL 21 &amp; GL 22 (Type 2-P) and GL 28 (Type 2-N2). Does the design team want these pins galvanized too? If so, please provide the standard required.</p>
<b>T-1490</b>	<b>BGP - IDF Room B1545 on Concourse Level</b>	<b>Closed</b>	<b>CR</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/08/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> <p>On concourse wall and floor plans (SKA-3332 and SKA-3336) the southern wall of IDF room B1545 is shown as RCW. However, on detail 3 of sheet A1-3009 it is shown as 0.10CMU. Please clarify the type of wall.</p>						<b>ANSWER:</b> <p>On concourse wall and floor plans (SKA-3332 and SKA-3336) the southern wall of IDF room B1545 is shown as RCW. However, on detail 3 of sheet A1-3009 it is shown as 0.10CMU. Please clarify the type of wall.</p>
<b>T-1491</b>	<b>BGP - Extent of Ramp Wall Reinforcement Embedded in Lower Concourse</b>	<b>Closed</b>	<b>CR</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/11/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> <p>Please confirm the western extent of the north vehicle ramp wall that is to embed into the Lower Concourse. The five main contract drawings pertaining to this issue show five different layouts for the end of the wall embedded into the Lower Concourse. Please provide details or confirm which drawing shows the correct layout for the extent of the north Vehicle Ramp Wall that is to have reinforcement embedded into the Lower Concourse.</p>						<b>ANSWER:</b> <p>Please confirm the western extent of the north vehicle ramp wall that is to embed into the Lower Concourse. The five main contract drawings pertaining to this issue show five different layouts for the end of the wall embedded into the Lower Concourse. Please provide details or confirm which drawing shows the correct layout for the extent of the north Vehicle Ramp Wall that is to have reinforcement embedded into the Lower Concourse.</p>



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<b>T-1492</b>	<b>BGP - Gridline W Ramp Wall Pier Dimensions</b>	<b>Closed</b>	<b>CR</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/18/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> Please provide dimensions for 3 of the piers located along the wall south of Gridline L and west of Gridline W. A1-2230 refers to A1-2850 for wall dimensions, but none are shown for the piers.					<b>ANSWER:</b> Please provide dimensions for 3 of the piers located along the wall south of Gridline L and west of Gridline W. A1-2230 refers to A1-2850 for wall dimensions, but none are shown for the piers.	
<b>T-1493</b>	<b>BGP - Two Future Plumbing Penetrations West of GL15</b>	<b>Closed</b>	<b>CR</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/10/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> On Plan Sheet SKA-3361_R1, two(2) future plumbing penetrations are shown 2'-5 1/2" West of Gridline 15. The Moment Frame Beam at this location is 6' wide. This places both plumbing penetrations in the MFB.  Please provide details on how SCCI is to provide plumbing penetrations through MFB on GL 15 or provide new offsets for these plumbing penetrations which are outside the MFB.					<b>ANSWER:</b> On Plan Sheet SKA-3361_R1, two(2) future plumbing penetrations are shown 2'-5 1/2" West of Gridline 15. The Moment Frame Beam at this location is 6' wide. This places both plumbing penetrations in the MFB.  Please provide details on how SCCI is to provide plumbing penetrations through MFB on GL 15 or provide new offsets for these plumbing penetrations which are outside the MFB.	
<b>T-1494</b>	<b>BGP - Minna St. West Transformer Vault Pier Dimensions</b>	<b>Closed</b>	<b>CR</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/16/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> Please provide dimensions to the center piers in the Minna St. West Transformer Vault. Refer to the attached drawings.					<b>ANSWER:</b> Please provide dimensions to the center piers in the Minna St. West Transformer Vault. Refer to the attached drawings.	
<b>T-1495</b>	<b>BGP - CMU Wall and B9/B11 Beam Dimensions D204 at GL4-J</b>	<b>Closed</b>	<b>CR</b>	<b>07/03/2014</b>	<b>07/13/2014</b>	<b>07/08/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> Please provide dimension or confirmation for the CMU wall as well as the B9 and B 11 beam just north of GL Jin between GL X and 5. AI-2202 gives a dimension to the east-west alignment of the wall, but no north-south dimension. The wall is not in any of the AI-3000 detail drawings and no dimension is shown on AI-2222. In SI-2202 the wall appears to be centered on a B9 beam, which appears to be aligned with the BI 1 beam to the					<b>ANSWER:</b> Please provide dimension or confirmation for the CMU wall as well as the B9 and B 11 beam just north of GL Jin between GL X and 5. AI-2202 gives a dimension to the east-west alignment of the wall, but no north-south dimension. The wall is not in any of the AI-3000 detail drawings and no dimension is shown on AI-2222. In SI-2202 the wall appears to be centered on a B9 beam, which appears to be aligned with the BI 1 beam	



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T-1496	<b>SSS - Exposed Flange at Second Level Slab GL 15</b>  From: Webcor Construction LPGregory Kemerer  <b>REQUEST:</b> See attached CD RFI # 477 SK1: The noted dimension is shown as 15'-5 1/2 on A1-2864 but the flange of the W16x26 will extend 1/2" inside the slab opening. Confirm the slab opening may be located 15'-6 east of grid 15 as shown. If not, supply an alternate solution.	Closed	CR	07/07/2014	07/17/2014	07/14/2014
						<b>ANSWER:</b> See attached CD RFI # 477 SK1: The noted dimension is shown as 15'-5 1/2 on A1-2864 but the flange of the W16x26 will extend 1/2" inside the slab opening. Confirm the slab opening may be located 15'-6 east of grid 15 as shown. If not, supply an alternate solution.
T-1497	<b>SSS - Brace Connection Clarification at W1 - Facade System</b>  From: Webcor Construction LPGregory Kemerer  <b>REQUEST:</b> See attached CD RFI #464. Please confirm the connection as submitted and shown on the attached sketch, SK3 as approved in RFI T-0923.2 is typically acceptable.	Closed	CR	07/07/2014	07/17/2014	07/18/2014
						<b>ANSWER:</b> See attached CD RFI #464. Please confirm the connection as submitted and shown on the attached sketch, SK3 as approved in RFI T-0923.2 is typically acceptable.







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	running east-west. The group of walls also do not show up on any other architectural or structural drawings. Please provide dimensions to the east-west future CMU walls coming off the south radius wall on D206 near Gridline 5.5-K as marked in the drawings attached.					walls running east-west. The group of walls also do not show up on any other architectural or structural drawings. Please provide dimensions to the east-west future CMU walls coming off the south radius wall on D206 near Gridline 5.5-K as marked in the drawings attached.
T-1501	SSS - Edge of Slab Plate Termination Clarification at GL 7	Closed	CR	07/08/2014	07/18/2014	07/18/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: See attached CD RFI # 466 SK1 & SK2: With the sloping slab per A/A1-7106 (SK2), it is not clear where to terminate the edge plates per 8/S1-5000. Please clarify and supply the missing dimension.						ANSWER: See attached CD RFI # 466 SK1 & SK2: With the sloping slab per A/A1-7106 (SK2), it is not clear where to terminate the edge plates per 8/S1-5000. Please clarify and supply the missing dimension.
T-1502	SSS - Decking Around Bus Deck Level Crash Rails	Void	CR	07/09/2014	07/19/2014	
From: Webcor Construction LP Gregory Kemerer						
REQUEST: With reference to the Bus Deck Level crash rail post detail 1/S1-8000, the post base plate is 18"x30" and spans or overhangs the width of the supporting beam allowing no bearing for the deck. As a result along grid lines B & H where the deck runs parallel to the support beam, our decker is required to provide a gauge support angle and flashing where required around the base plate, see details 1 & 2 on SK1. At grid lines 1.1 & 33.5 where the deck runs perpendicular to the support beam, a beam flange extension is required to achieve the 3" end bearing requirements for the S8 slab, see detail 3 on SK1.  Please confirm these details are acceptable or provide an alternative detail for the deck support around the crash rail base plate.						ANSWER: With reference to the Bus Deck Level crash rail post detail 1/S1-8000, the post base plate is 18"x30" and spans or overhangs the width of the supporting beam allowing no bearing for the deck. As a result along grid lines B & H where the deck runs parallel to the support beam, our decker is required to provide a gauge support angle and flashing where required around the base plate, see details 1 & 2 on SK1. At grid lines 1.1 & 33.5 where the deck runs perpendicular to the support beam, a beam flange extension is required to achieve the 3" end bearing requirements for the S8 slab, see detail 3 on SK1.  Please confirm these details are acceptable or provide an alternative detail for the deck support around the crash rail base plate.
T-1503	BGP - Lower Concourse Grounding Grid Alternate Detail - Room B1441 GL15/G.5	Closed	01	07/09/2014	07/19/2014	07/10/2014







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<b>T-1504</b>	<b>BGP - Lower Concourse Future Wall FS Conflicts</b>	<b>Closed</b>	<b>01</b>	<b>07/09/2014</b>	<b>07/19/2014</b>	<b>07/09/2014</b>
<b>From:</b> Shimmick Construction Company, Inc. Sylvia Hartanto						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per attached correspondence, WOJV has directed SCCI to install future wall formsavers according to the details attached that clarify the Contract Drawings. However, due to congestion created by the increased amount of reinforcing within the beam components, ample space to install the future wall formsaver bars at the design spacing does not always exist. See attached pictures.			Per attached correspondence, WOJV has directed SCCI to install future wall formsavers according to the details attached that clarify the Contract Drawings. However, due to congestion created by the increased amount of reinforcing within the beam components, ample space to install the future wall formsaver bars at the design spacing does not always exist. See attached pictures.			
Because the CMU contract has not been solicited or awarded, please confirm that it is acceptable to offset the bars out of alignment +/- 4" in attempts to maintain proper spacing, and/or increase or decrease the specified spacing +/- 6" such that the future wall formsaver bars can be installed and the correct number of dowels is maintained. The same flexibility is also requested for the "ends, intersection and corner" detail per the attached document.			Because the CMU contract has not been solicited or awarded, please confirm that it is acceptable to offset the bars out of alignment +/- 4" in attempts to maintain proper spacing, and/or increase or decrease the specified spacing +/- 6" such that the future wall formsaver bars can be installed and the correct number of dowels is maintained. The same flexibility is also requested for the "ends, intersection and corner" detail per the attached document.			
<b>T-1505</b>	<b>SSS - Stiffener Plate Location at TR13</b>	<b>Closed</b>	<b>CR</b>	<b>07/09/2014</b>	<b>07/09/2014</b>	<b>07/18/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 473 SK1: The 2 1/2" plate has been located 1 1/8" above the 2" diameter rebar holes to allow room for the 1" fillet welds. If the plate is located per the response to RFI T-0889 (SK 145, CD 107) item 1, it will cover the 2" diameter rebar holes. Confirm the plate location as shown is acceptable.			See attached CD RFI # 473 SK1: The 2 1/2" plate has been located 1 1/8" above the 2" diameter rebar holes to allow room for the 1" fillet welds. If the plate is located per the response to RFI T-0889 (SK 145, CD 107) item 1, it will cover the 2" diameter rebar holes. Confirm the plate location as shown is acceptable.			
<b>T-1506</b>	<b>BGP - Room B1441 Grounding Grid Approval and Inspection Jurisdiction</b>	<b>Closed</b>	<b>01</b>	<b>07/11/2014</b>	<b>07/21/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per breakout meeting held after the TTC progress meeting on 7/9 @ 10:30am, please confirm that PG&E will have no jurisdiction over approval and inspection of the grounding grid in electrical room B1441 GL 15/G.5.			Per breakout meeting held after the TTC progress meeting on 7/9 @ 10:30am, please confirm that PG&E will have no jurisdiction over approval and inspection of the grounding grid in electrical room B1441 GL 15/G.5.			



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<b>T-1507</b>	<b>BGP - Seismic Joint Removable Corbel Connection Variance</b>	<b>Closed</b>	<b>01</b>	<b>07/11/2014</b>	<b>07/21/2014</b>	<b>07/23/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Per contract drawing S1-3205, the removable W14x61 steel corbel and WT8X28.5 angles it connects with are to be fabricated with a 2" vertical slotted hole.		Per contract drawing S1-3205, the removable W14x61 steel corbel and WT8X28.5 angles it connects with are to be fabricated with a 2" vertical slotted hole.				
In order to eliminate the lead time of field measured fabrication of the corbels after concrete formwork removal from the SJ assembly, the contractor is proposing either or preferably both of the following modifications:		In order to eliminate the lead time of field measured fabrication of the corbels after concrete formwork removal from the SJ assembly, the contractor is proposing either or preferably both of the following modifications:				
1. Change the slotted hole from vertical to horizontal (east to west) direction on the angles and or the steel corbels		1. Change the slotted hole from vertical to horizontal (east to west) direction on the angles and or the steel corbels				
2. Change the finish of the removable W14X61 steel corbel only from galvanized to primed steel (all other embedded metals would remain galvanized per the contract drawing).		2. Change the finish of the removable W14X61 steel corbel only from galvanized to primed steel (all other embedded metals would remain galvanized per the contract drawing).				
Please confirm if either of these proposals would be acceptable?		Please confirm if either of these proposals would be acceptable?				
<b>T-1508</b>	<b>SSS - Additional Stud Requirements on ASI 112</b>	<b>Closed</b>	<b>CR</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>07/24/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>		<b>ANSWER:</b>				
ASI 112 added "UON, see general note DK-5 for added studs on beams" on partial stair plan drawings S1-7002 & S1-7003. It is unclear what beams require studs on these plans. Please provide stud quantities for each beam as per the steel beam legend on S1-2302 so Skanska can provide accurate pricing for this ASI.		ASI 112 added "UON, see general note DK-5 for added studs on beams" on partial stair plan drawings S1-7002 & S1-7003. It is unclear what beams require studs on these plans. Please provide stud quantities for each beam as per the steel beam legend on S1-2302 so Skanska can provide accurate pricing for this ASI.				
<b>T-1509</b>	<b>SCS - Lower Concourse Partition Walls Drawing Conflicts</b>	<b>Closed</b>	<b>01</b>	<b>07/14/2014</b>	<b>07/24/2014</b>	<b>07/18/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
1. There is a conflicting detail on Sheet No A1-2202 and S1-2202. A1-2202 shows the wall on grid line B.8-1.4 as a concrete wall but on S1-2202, the same wall is shown as a		1. There is a conflicting detail on Sheet No A1-2202 and S1-2202. A1-2202 shows the wall on grid line B.8-1.4 as a concrete wall but on S1-2202, the same wall				



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	concrete masonry wall. Please verify					
	2. There is a conflicting detail on Sheet No A1-2202 and S1-2202 and corresponding details on A1-3008 and B/A1-9241. The dimensions of the concrete walls on grid line F-3 are different. Doors have different locations. Please verify					
	3. There is a conflicting detail on Sheet No A1-2202 (and corresponding details on A1-3001 and B/A1-9235) and S1-2202. The architectural drawings show a concrete wall along grid line 3, there is no wall in S1-2202. Please verify					
	4. There is a conflicting detail on Sheet No A1-2203 (and corresponding detail on A1-2250) and S1-2203. The scaled dimensions of the concrete wall on grid line K.1-7.5 are different. Please verify. Please also provide Sheet A1-7405 with actual dimensions.					
	5. There is a conflicting detail on Sheet No A1-2203 (and corresponding details on A1-3002 and E/A1-9236) and S1-2203. In S1-2203, grid line B.6-11 there are two gaps in the concrete wall, there is only one gap in the same wall in A1-2203. Please verify					
	6. There is a conflicting detail on Sheet No A1-2204 (and corresponding details 2/A1-3008 and A1-9240) and S1-2204. The door locations for the EAST FSR on grid line C.3-15 and B.1-15 are in different positions. Please verify					
	7. There is a conflicting detail on Sheet No A1-2205 (and corresponding details 1/A1-3003 and E/A1-9237) and S1-2205. The door locations on the concrete wall on grid line G.5-21 is in a different position. Please verify					
	is shown as a concrete masonry wall. Please verify					
	2. There is a conflicting detail on Sheet No A1-2202 and S1-2202 and corresponding details on A1-3008 and B/A1-9241. The dimensions of the concrete walls on grid line F-3 are different. Doors have different locations. Please verify					
	3. There is a conflicting detail on Sheet No A1-2202 (and corresponding details on A1-3001 and B/A1-9235) and S1-2202. The architectural drawings show a concrete wall along grid line 3, there is no wall in S1-2202. Please verify					
	4. There is a conflicting detail on Sheet No A1-2203 (and corresponding detail on A1-2250) and S1-2203. The scaled dimensions of the concrete wall on grid line K.1-7.5 are different. Please verify. Please also provide Sheet A1-7405 with actual dimensions.					
	5. There is a conflicting detail on Sheet No A1-2203 (and corresponding details on A1-3002 and E/A1-9236) and S1-2203. In S1-2203, grid line B.6-11 there are two gaps in the concrete wall, there is only one gap in the same wall in A1-2203. Please verify					
	6. There is a conflicting detail on Sheet No A1-2204 (and corresponding details 2/A1-3008 and A1-9240) and S1-2204. The door locations for the EAST FSR on grid line C.3-15 and B.1-15 are in different positions. Please verify					
	7. There is a conflicting detail on Sheet No A1-2205 (and corresponding details 1/A1-3003 and E/A1-9237) and S1-2205. The door locations on the concrete wall on grid line G.5-21 is in a different position. Please verify					
T-1510	SSS - W-1 Brace Details at Bus Level	Closed	CR	07/14/2014	07/24/2014	07/24/2014
From: Webcor Construction LP		Gregory Kemerer				
REQUEST:			ANSWER:			
See attached CD RFI # 470 SK1 to SK3:			See attached CD RFI # 470 SK1 to SK3:			



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T-1511	<p>Revised detail 4/S1-8003 from ASI 109 added (1) bolt at each end of the brace. The connections for the braces per detail 4/S1-8003 were therefore applied per the information in detail 8/S1-5015 and RFI's T-0919, T-0919.1 &amp; T-0919.2 (SK 173, 173.1 &amp; 173.2, CD 127, 127.1 &amp; 127.2). Please confirm the connection as submitted and shown on SK3 is typically acceptable.</p> <p>Note: no comment was made for the same connection on drawings 4207 &amp; 4208 in sequence CS8.</p> <p><b>BGP - Unistrut at Lower Concourse E-W Spandrel Beams</b></p> <p><b>From:</b> Webcor Construction LP                      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>Please confirm it is acceptable to install unistrut at the E-W and Spandrel Beam Bottom and Sides at the Lower Concourse construction joints (Reference attached sketch). This unistrut would serve as the bottom edge form, be left in place and cast in the Lower Concourse with the subsequent deck pour.</p>	Closed	01	07/15/2014	07/25/2014	07/15/2014
	<p>Revised detail 4/S1-8003 from ASI 109 added (1) bolt at each end of the brace. The connections for the braces per detail 4/S1-8003 were therefore applied per the information in detail 8/S1-5015 and RFI's T-0919, T-0919.1 &amp; T-0919.2 (SK 173, 173.1 &amp; 173.2, CD 127, 127.1 &amp; 127.2). Please confirm the connection as submitted and shown on SK3 is typically acceptable.</p> <p>Note: no comment was made for the same connection on drawings 4207 &amp; 4208 in sequence CS8.</p> <p><b>ANSWER:</b></p> <p>Please confirm it is acceptable to install unistrut at the E-W and Spandrel Beam Bottom and Sides at the Lower Concourse construction joints (Reference attached sketch). This unistrut would serve as the bottom edge form, be left in place and cast in the Lower Concourse with the subsequent deck pour.</p>					



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<b>T-1512</b>	<b>BGP - Mechanical Pump Room B2761 Electrical Sump Feeds</b>	<b>Closed</b>	<b>01</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>07/28/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Plan Sheet EI-2027 shows two(2) sump controllers in Mechanical Pump Room B2761, one for SE-B2- D-1,2 and the other for SPG-B2-D-I,2. There will be 2" conduits from these controllers to two of the three sump pits in the mechanical room for the pump leads. Plan Sheet EI-2027 indicates the two(2) eastern-most pits get the pump lead conduits( see attached), however Detail 4 on Plan Sheet PI-3006 identifies the western-most pit as "SPG" and the middle pit as "SE"( see attached).  Please confirm: 1) Detail 4 on plan sheet P1-3006 is correct. 2) The pump lead conduits from SE-B2-D-1,2 will lead to the pit identified as "SE" on plan sheet P1-3006. 3) The pump lead conduits from SPG-B2-D-1,2 will lead to the pit identified as "SPG" on plan sheet P1-3006.						
						<b>ANSWER:</b>  Plan Sheet EI-2027 shows two(2) sump controllers in Mechanical Pump Room B2761, one for SE-B2- D-1,2 and the other for SPG-B2-D-I,2. There will be 2" conduits from these controllers to two of the three sump pits in the mechanical room for the pump leads. Plan Sheet EI-2027 indicates the two(2) eastern-most pits get the pump lead conduits( see attached), however Detail 4 on Plan Sheet PI-3006 identifies the western-most pit as "SPG" and the middle pit as "SE"( see attached).  Please confirm: 1) Detail 4 on plan sheet P1-3006 is correct. 2) The pump lead conduits from SE-B2-D-1,2 will lead to the pit identified as "SE" on plan sheet P1-3006. 3) The pump lead conduits from SPG-B2-D-1,2 will lead to the pit identified as "SPG" on plan sheet P1-3006.
<b>T-1513</b>	<b>SSS - Corner Drag Beam Framing at Bus Deck Level</b>	<b>Closed</b>	<b>CR</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>07/28/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 383.2 SK1 & SK2: The revised framing shown on attached SK1 & SK2 is per discussions in a conference call following the response to RFI T-1322.1. Please review and confirm all boxed notes on SK1 & SK2.						
						<b>ANSWER:</b>  See attached CD RFI # 383.2 SK1 & SK2: The revised framing shown on attached SK1 & SK2 is per discussions in a conference call following the response to RFI T-1322.1. Please review and confirm all boxed notes on SK1 & SK2.
<b>T-1514</b>	<b>SSS - Bus Deck Level Stiffener Conflict at CP8 Near GL 1.4</b>	<b>Closed</b>	<b>CR</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>07/28/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 484 SK1. The 1 1/2" stiffener on the far side will cover the hole in the bottom flange for CP8. Please advise.						
						<b>ANSWER:</b>  See attached CD RFI # 484 SK1. The 1 1/2" stiffener on the far side will cover the hole in the bottom flange for CP8. Please advise.



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<b>T-1514.1</b>	<b>SSS - Bus Deck Level Stiffener Conflict at CP8 Near GL 1.4</b>	<b>Closed</b>	<b>CR</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/13/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 484.1 SK1: Providing two 2-L3x3x3/8 braces per T-1514 does not work as the brace close to the W36x231 will foul the beam. Confirm it is acceptable to supply one 2-L3x3x3/8 as shown.						<b>ANSWER:</b> See attached CD RFI # 484.1 SK1: Providing two 2-L3x3x3/8 braces per T-1514 does not work as the brace close to the W36x231 will foul the beam. Confirm it is acceptable to supply one 2-L3x3x3/8 as shown.
<b>T-1515</b>	<b>SSS - Connection Clarifications for Stiffeners and Plate at BU Beam at GL 1.4-2 &amp; I Closed</b>		<b>CR</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>07/24/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 485 SK1 & SK2 for items 1 to 3: 1) The 1/2" x 9" stiffeners foul the built-up beam as shown. Please provide a solution. 2) The 1/2" x 9" stiffeners foul the connection per detail 2/S1804 as shown. Please provide a solution. 3) The hole locations in detail 6E/S1-8003 are not known and have been requested in RFI CD 476 but it appears the  PL 2" will obstruct the flange holes. Please advise.						<b>ANSWER:</b> See attached CD RFI # 485 SK1 & SK2 for items 1 to 3: 1) The 1/2" x 9" stiffeners foul the built-up beam as shown. Please provide a solution. 2) The 1/2" x 9" stiffeners foul the connection per detail 2/S1804 as shown. Please provide a solution. 3) The hole locations in detail 6E/S1-8003 are not known and have been requested in RFI CD 476 but it appears the PL 2" will obstruct the flange holes. Please advise.
<b>T-1516</b>	<b>SSS - Bolt Requirement for BU Beam at Roof Perimeter Corner</b>	<b>Closed</b>	<b>CR</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>07/24/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 488 SK1 & SK2: 11 bolts are required per 1/S1-5010 for W40 beams. However, due to the weld access holes in the BU-40x22, we can only fit in 9 bolts in the one row as shown. The other row will have 11 bolts. Confirm that this detail is acceptable or please supply an alternate detail.						<b>ANSWER:</b> See attached CD RFI # 488 SK1 & SK2: 11 bolts are required per 1/S1-5010 for W40 beams. However, due to the weld access holes in the BU-40x22, we can only fit in 9 bolts in the one row as shown. The other row will have 11 bolts. Confirm that this detail is acceptable or please supply an alternate detail.
<b>T-1517</b>	<b>SSS - Weld Detail for Architectural Plate</b>	<b>Closed</b>	<b>CR</b>	<b>07/15/2014</b>	<b>07/25/2014</b>	<b>07/24/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Please confirm the attached weld detail is acceptable for fi						<b>ANSWER:</b> Please confirm the attached weld detail is acceptable f





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	eld welding the 3/8" architectural cylindrical plate.					or field welding the 3/8" architectural cylindrical plate.
T-1518	BGP - Partition Wall Pilaster Ties	Closed	01	07/15/2014	07/25/2014	07/21/2014
From: Webcor Construction LP Claude Titcher						
REQUEST:						ANSWER:
As discussed onsite with the SEOR, please confirm the following for partition walls:						As discussed onsite with the SEOR, please confirm the following for partition walls:
1. All isolated pilasters have ties over their entire height						1. All isolated pilasters have ties over their entire height
2. All pilasters thicker than the adjacent integral wall have ties over their entire height- Please note that this is added scope to TG06 Contract						2. All pilasters thicker than the adjacent integral wall have ties over their entire height- Please note that this is added scope to TG06 Contract
3. Pilaster that are the same thickness as the adjacent integral wall require ties only for the lengths defined in Note 3 of Detail 9/SI-9050						3. Pilaster that are the same thickness as the adjacent integral wall require ties only for the lengths defined in Note 3 of Detail 9/SI-9050
4. Where ties are present at a pilaster base, the ties are not required to penetrate the supporting slab or foundation.						4. Where ties are present at a pilaster base, the ties are not required to penetrate the supporting slab or foundation.
5. Ties may be built up from stirrups and hairpins containing 135 hooks.						5. Ties may be built up from stirrups and hairpins containing 135 hooks.
Please reference the attached drawings, S1-9050.						Please reference the attached drawings, S1-9050.





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<b>T-1519</b>	<b>BGP - Formsavers for Ramp Walls Embedded in Lower Concourse</b>	<b>Closed</b>	<b>01</b>	<b>07/17/2014</b>	<b>07/27/2014</b>	<b>07/21/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Please confirm that it is acceptable to use formsavers for the Ramp Walls that are to be embedded in the Lower Concourse in lieu of dowels shown in A/S1-3203 or 3/S1-3502. This is done to improve construction access.  Locations for the proposed formsavers are per attached documents- highlighted in red.						<b>ANSWER:</b>  Please confirm that it is acceptable to use formsavers for the Ramp Walls that are to be embedded in the Lower Concourse in lieu of dowels shown in A/S1-3203 or 3/S1-3502. This is done to improve construction access.  Locations for the proposed formsavers are per attached documents- highlighted in red.
<b>T-1520</b>	<b>SSS - Moment Weld at Bottom Flange of W30X90</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  Please supply a detail showing how to make the moment weld for the bottom flange of the W30x90.						<b>ANSWER:</b>  Please supply a detail showing how to make the moment weld for the bottom flange of the W30x90.
<b>T-1521</b>	<b>SSS - Beam Location Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/24/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  The noted weld is shown for a beam that is not shown on plans S1-2602 & S1-2607. Confirm it is shown in error on this detail.						<b>ANSWER:</b>  The noted weld is shown for a beam that is not shown on plans S1-2602 & S1-2607. Confirm it is shown in error on this detail.
<b>T-1522</b>	<b>SSS - PE201 Connection Clarification at HSS Post</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 496 SK1 & SK2: Detail 6/S1-7604 fouls the connection per 8/S1-7604 when they occur at the same location as the horizontal PL 3/4" per 6/S1-7604 is to extend 1/2" past the profile of the HSS post. Confirm it is acceptable to keep the PL 3/4" flush with the HSS post and HSS horizontal beam as shown to be able to attach the L6x6x5/8 per 8/S1-7604 to the post.						<b>ANSWER:</b>  See attached CD RFI # 496 SK1 & SK2: Detail 6/S1-7604 fouls the connection per 8/S1-7604 when they occur at the same location as the horizontal PL 3/4" per 6/S1-7604 is to extend 1/2" past the profile of the HSS post. Confirm it is acceptable to keep the PL 3/4" flush with the HSS post and HSS horizontal beam as shown to be able to attach the L6x6x5/8 per 8/S1-7604 to the post.



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<b>T-1523</b>	<b>SSS - Welding Clarifications at Bus Ramp Support GL1.4-3</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/24/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 497 SK1: Confirm the welding for thick flange to thin flange is acceptable as shown or supply details.					<b>ANSWER:</b> See attached CD RFI # 497 SK1: Confirm the welding for thick flange to thin flange is acceptable as shown or supply details.	
<b>T-1524</b>	<b>SSS - ST201 Missing Dimensions and Slab Location</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/28/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 501 SK1 for items 1 to 3:  1) Supply missing dimension.  2) Confirm EQ/EQ spacing or supply the dimensions.  3) Supply the edge of slab location on the north, west and south sides at T/SLAB 85'-0" as A1-2902 seems to show different information than 2/S1-7003. (The east side is shown in detail 6/S1-7602).					<b>ANSWER:</b> See attached CD RFI # 501 SK1 for items 1 to 3:  1) Supply missing dimension.  2) Confirm EQ/EQ spacing or supply the dimensions.  3) Supply the edge of slab location on the north, west and south sides at T/SLAB 85'-0" as A1-2902 seems to show different information than 2/S1-7003. (The east side is shown in detail 6/S1-7602).	
<b>T-1524.1</b>	<b>SSS - ST201 Missing Dimensions and Slab Location</b>	<b>Closed</b>	<b>CR</b>	<b>08/07/2014</b>	<b>08/17/2014</b>	<b>08/22/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 501.1 SK1 for items 1 to 3: 1) The response RFI T-1524 (SK 691, CD 501) did not supply a response to item 1. 2/S1-7003 clearly shows the steel framing as shown on SK1 but does not supply this dimension to locate the beam. Please supply this missing dimension. 2) Edge plate per 8/S1-5000. Confirm. 3) Supply a detail for the edge of slab plate if required.					<b>ANSWER:</b> See attached CD RFI # 501.1 SK1 for items 1 to 3: 1) The response RFI T-1524 (SK 691, CD 501) did not supply a response to item 1. 2/S1-7003 clearly shows the steel framing as shown on SK1 but does not supply this dimension to locate the beam. Please supply this missing dimension. 2) Edge plate per 8/S1-5000. Confirm. 3) Supply a detail for the edge of slab plate if required.	
<b>T-1525</b>	<b>SSS - Plate Extending Past End of Girder at Roof Level</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 513 SK1 & SK2: The 3" plate per Type 61 on S1-5132 extends 3" past the end of the Girder. Please confirm that is the intent or					<b>ANSWER:</b> See attached CD RFI # 513 SK1 & SK2: The 3" plate per Type 61 on S1-5132 extends 3" past the end of the Girder. Please confirm that is the intent	



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	supply a solution.					or supply a solution.
<b>T-1526</b>	<b>SSS - Dimensional Adjustment to Avoid Fouling BU-Box Beam at Bus Ramp Supp</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 515 SK1 & SK2 for items 1 & 2: 1) The web reinforcing plate will foul the BU-Box Beam 'B' if extended past the flange cope 1'-0 per S1-5017. Confirm the 1'-0 dimension may be reduced to 9 3/4" as shown or supply an alternate solution.  2) The web reinforcing plate will foul the end plate for the WF beam if extended past the flange cope 1'-0 per S1-5017. Confirm the 1'-0 dimension may be reduced to 6 1/4" on both sides or supply an alternate solution.						<b>ANSWER:</b> See attached CD RFI # 515 SK1 & SK2 for items 1 & 2: 1) The web reinforcing plate will foul the BU-Box Beam 'B' if extended past the flange cope 1'-0 per S1-5017. Confirm the 1'-0 dimension may be reduced to 9 3/4" as shown or supply an alternate solution.  2) The web reinforcing plate will foul the end plate for the WF beam if extended past the flange cope 1'-0 per S1-5017. Confirm the 1'-0 dimension may be reduced to 6 1/4" on both sides or supply an alternate solution.
<b>T-1527</b>	<b>SSS - PE201 &amp; ST201A Welding Clarifications at WF</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/30/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 494 SK1 for items 1 to 3:  1) Confirm weld prep.  2) The noted fillet weld cannot be made with the 3/8" gap per item 1. Confirm this weld will also be a CJP weld to match the T & B weld.  3) This weld cannot be made with the 3/8" gap. Supply an alternate weld.						<b>ANSWER:</b> See attached CD RFI # 494 SK1 for items 1 to 3:  1) Confirm weld prep.  2) The noted fillet weld cannot be made with the 3/8" gap per item 1. Confirm this weld will also be a CJP weld to match the T & B weld.  3) This weld cannot be made with the 3/8" gap. Supply an alternate weld.



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<b>T-1528</b>	<b>SSS - ST201A Missing Moment Connection Details at HSS Posts</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/30/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 498 SK1: Clarify how the top and bottom flange of the W12x19 are to be moment welded as they do not align with the top and bottom plates for the HSS10x8 per 6/S1-7604 as shown.						<b>ANSWER:</b> See attached CD RFI # 498 SK1: Clarify how the top and bottom flange of the W12x19 are to be moment welded as they do not align with the top and bottom plates for the HSS10x8 per 6/S1-7604 as shown.
<b>T-1529</b>	<b>SSS - ST201 W16 to HSS Post Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>07/18/2014</b>	<b>07/28/2014</b>	<b>07/30/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 503 SK1: Confirm it is acceptable to connect the W16's to the HSS posts per 1/S1-5011.						<b>ANSWER:</b> See attached CD RFI # 503 SK1: Confirm it is acceptable to connect the W16's to the HSS posts per 1/S1-5011.
<b>T-1529.1</b>	<b>SSS - ST201 W16 to HSS Post Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>10/14/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 503.1 SK1: The connection for the HSS10x8 per 10/S1-7605 no longer works with the W14x311 terminating at the underside of slab at T/Slab 86'-9 per RFI T-1529 (SK 693, CD 503). Please supply a new detail.						<b>ANSWER:</b> See attached CD RFI # 503.1 SK1: The connection for the HSS10x8 per 10/S1-7605 no longer works with the W14x311 terminating at the underside of slab at T/Slab 86'-9 per RFI T-1529 (SK 693, CD 503). Please supply a new detail.
<b>T-1529.2</b>	<b>SSS - ST201 W16 to HSS Post Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>10/31/2014</b>	<b>11/09/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 503.2 SK1: 1). The connection detail is not possible as the beam will not be erectable with the HSS beam extending to the web of the W14 column as shown by the (2) end views of the HSS10x8x1/2. Please supply a new detail that permits the erection of the (2) noted beams. 2). For the added prep. weld to the top of the HSS per the response T-1529.1 verify this is a PJP weld with zero clearance and if so review with question # 1 for erection of these members in question.						<b>ANSWER:</b> See attached CD RFI # 503.2 SK1: 1). The connection detail is not possible as the beam will not be erectable with the HSS beam extending to the web of the W14 column as shown by the (2) end views of the HSS10x8x1/2. Please supply a new detail that permits the erection of the (2) noted beams.  2). For the added prep. weld to the top of the HSS per the response T-1529.1 verify this is a PJP weld with zero clearance and if so review with question # 1 for



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erection of these members in question.						
<b>T-1530</b>	<b>SSS - ST201 Edge of Slab Locations at Landings</b>	<b>Closed</b>	<b>CR</b>	<b>07/19/2014</b>	<b>07/29/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The edge of slab locations for the ST201A landings are missing. Please refer to the list below and supply the edge of slab locations for items 1, 3, 5, 6 & 8: 1) 2/S1-7001 ~ supply edge of slab information 2) 3/S1-7001 ~ edge of slab information is shown on A1-2882 3) 1/S1-7002 ~ supply edge of slab information 4) 2/S1-7002 ~ edge of slab information is shown on A1-2892 5) 3/S1-7002 ~ supply edge of slab information 6) 1/S1-7003 ~ supply edge of slab information 7) 2/S1-7003 ~ edge of slab information is shown on A1-2902 8) 3/S1-7003 ~ supply edge of slab information			The edge of slab locations for the ST201A landings are missing. Please refer to the list below and supply the edge of slab locations for items 1, 3, 5, 6 & 8: 1) 2/S1-7001 ~ supply edge of slab information 2) 3/S1-7001 ~ edge of slab information is shown on A1-2882 3) 1/S1-7002 ~ supply edge of slab information 4) 2/S1-7002 ~ edge of slab information is shown on A1-2892 5) 3/S1-7002 ~ supply edge of slab information 6) 1/S1-7003 ~ supply edge of slab information 7) 2/S1-7003 ~ edge of slab information is shown on A1-2902 8) 3/S1-7003 ~ supply edge of slab information			
<b>T-1531</b>	<b>SSS - ST201 Bolt and Cap Plate Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>07/19/2014</b>	<b>07/29/2014</b>	<b>07/31/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 504 SK1 & SK2 for items 1 & 2:  1) The bolts will not be erectable if left inside the profile of the column as shown in detail 4/S1-5033. Confirm it is acceptable to locate the bolts as shown.  2) Confirm it is acceptable to locate the cap plate per 11/S1-7605 as shown and weld the L4x4x1/4 per 4/S1-5033 as shown.			See attached CD RFI # 504 SK1 & SK2 for items 1 & 2:  1) The bolts will not be erectable if left inside the profile of the column as shown in detail 4/S1-5033. Confirm it is acceptable to locate the bolts as shown.  2) Confirm it is acceptable to locate the cap plate per 11/S1-7605 as shown and weld the L4x4x1/4 per 4/S1-5033 as shown.			



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<b>T-1532</b>	<b>SSS - Deck Support Angle at Column Base Plate</b>	<b>Closed</b>	<b>CR</b>	<b>07/19/2014</b>	<b>07/29/2014</b>	<b>07/24/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Per detail 1/S1-5052 when the column base plate and the transfer girder flange is the same width, there is no deck support for the length of the column base plate when the deck sits at the same level as the girder.			Per detail 1/S1-5052 when the column base plate and the transfer girder flange is the same width, there is no deck support for the length of the column base plate when the deck sits at the same level as the girder.			
Please confirm it is acceptable to provide a gage angle to support the deck around the column base plate. The gauge angle will be welded after the column to girder connection is made to avoid the angle and weld interfering with the connection. See attached SK1 & 2 for clarification.			Please confirm it is acceptable to provide a gage angle to support the deck around the column base plate. The gauge angle will be welded after the column to girder connection is made to avoid the angle and weld interfering with the connection. See attached SK1 & 2 for clarification.			
<b>T-1532.1</b>	<b>SSS - Deck Support Angle at Column Base Plate</b>	<b>Closed</b>	<b>CR</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/18/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
This is a follow-up to the response of RFI T-1532.			This is a follow-up to the response of RFI T-1532.			
As per the response, restricting welding to only the girder flange creates several issues: 1. See detail 17 on SK1 - an additional 4" would be required on the vertical leg of the angle to allow welding to the girder flange. This creates a 12" vertical leg, welded at the bottom toe to the girder and supporting a 13" concrete slab. Our decker does not believe this is adequate support. This occurs at 9 locations.  2. See detail 5 & 17 on SK1 - overhead welding will be required from the underside to weld to the girder flange. Detail 5 occurs at 67 locations and 17 occurs at 9 locations. Our decker will incur additional labor and equipment costs for this work.  If welding to the base plate is prohibited, an alternative solution which will have lower additional cost impact is to use Hilti pins to attach the gauge angle to base plate or girder flange. See attached product data and application details. See attached SK2 for clarification.  Please confirm if welding to the column base plate is prohibited, Skanska can use Hilti pins as per SK2.			As per the response, restricting welding to only the girder flange creates several issues: 1. See detail 17 on SK1 - an additional 4" would be required on the vertical leg of the angle to allow welding to the girder flange. This creates a 12" vertical leg, welded at the bottom toe to the girder and supporting a 13" concrete slab. Our decker does not believe this is adequate support. This occurs at 9 locations.  2. See detail 5 & 17 on SK1 - overhead welding will be required from the underside to weld to the girder flange. Detail 5 occurs at 67 locations and 17 occurs at 9 locations. Our decker will incur additional labor and equipment costs for this work.  If welding to the base plate is prohibited, an alternative solution which will have lower additional cost impact is to use Hilti pins to attach the gauge angle to base plate or girder flange. See attached product data and application details. See attached SK2 for clarification.  Please confirm if welding to the column base plate is prohibited, Skanska can use Hilti pins as per SK2.			



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<b>T-1532.2</b>	<b>SSS - Deck Support Angle at Column Base Plate</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/15/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>This is a follow-up to the response of RFI T-1532.1</p> <p>As per the response to the above RFI, direction to drawing A1-9317 (which was provided as part of FO 27) indicates a deck closure/support angle separated 1" from the column base plate, with no structural information provided. As a detail for deck support around the column base plate is not provided in the structural drawings and Skanska's proposals have been rejected in previous RFI responses, this remains an open issue. We require a detail in order to proceed with the detailing of shop drawings. Please note this condition occurs at CS1 and throughout the central zone and is currently holding up the resubmittal of the metal deck shop drawings for the central zone.</p> <p>Additionally, the RFI response identified a "no weld zone" that was not shown on the design documents. As this was not called out on the contract drawings, this zone is not being identified with paint strips as per the protected zone requirements. This may lead to trade workers accidentally welding in the field. Please provide direction for the identification requirements for the base plate "no weld zone".</p>						<b>ANSWER:</b> <p>This is a follow-up to the response of RFI T-1532.1</p> <p>As per the response to the above RFI, direction to drawing A1-9317 (which was provided as part of FO 27) indicates a deck closure/support angle separated 1" from the column base plate, with no structural information provided. As a detail for deck support around the column base plate is not provided in the structural drawings and Skanska's proposals have been rejected in previous RFI responses, this remains an open issue. We require a detail in order to proceed with the detailing of shop drawings. Please note this condition occurs at CS1 and throughout the central zone and is currently holding up the resubmittal of the metal deck shop drawings for the central zone.</p> <p>Additionally, the RFI response identified a "no weld zone" that was not shown on the design documents. As this was not called out on the contract drawings, this zone is not being identified with paint strips as per the protected zone requirements. This may lead to trade workers accidentally welding in the field. Please provide direction for the identification requirements for the base plate "no weld zone".</p>
<b>T-1532.3</b>	<b>SSS - Deck Support Angle at Column Base Plate</b>	<b>Closed</b>	<b>CR</b>	<b>10/07/2014</b>	<b>10/17/2014</b>	<b>10/17/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>Please provide a structural detail indicating the designer's requirements for the deck support/closure angle around the column base plate to top of transfer girder connection on all sides of the base plate.</p>						<b>ANSWER:</b> <p>Please provide a structural detail indicating the designer's requirements for the deck support/closure angle around the column base plate to top of transfer girder connection on all sides of the base plate.</p>
<b>T-1533</b>	<b>BGP - Additional Reinforcing at MS 113</b>	<b>Closed</b>	<b>01</b>	<b>07/21/2014</b>	<b>07/31/2014</b>	<b>07/23/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> <p>Due to sagging of the top mat reinforcing at Mat Slab MS</p>						<b>ANSWER:</b> <p>Due to sagging of the top mat reinforcing at Mat Slab</p>





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	113, there would be excessive concrete cover.  Per field conversation with SEOR, please confirm it is acceptable to install #5 reinforcing at 8" OC( either on dobies or other reinforcing) at locations where this sagging occurs to decrease the upper cover to within the allowable limits.					MS 113, there would be excessive concrete cover.  Per field conversation with SEOR, please confirm it is acceptable to install #5 reinforcing at 8" OC( either on dobies or other reinforcing) at locations where this sagging occurs to decrease the upper cover to within the allowable limits.
<b>T-1534</b>	<b>SCS - Transfer Girder Elevation &amp; Detail - G Line - TR19.9, 20.1, 22, 24</b>	<b>Closed</b>	<b>01</b>	<b>07/22/2014</b>	<b>08/01/2014</b>	<b>08/06/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>  Find attached Harris RFI dated July 17, 2014 and SK2 and SK3 from response to RFI #T-1408 with added blue markups/comments.  SK3 with the response to RFI #T-1408 shows the elevation of the top of steel to be above the elevation of the top of concrete.  1. Please confirm the top of steel is intended to come out of the top of concrete and provide a revised 9/SI-3702 detail.  a. Please clarify the detail for the welded couplers at the top flange.  For TR19.9 and 20.1, a plate for the bottom welded couplers are not shown on SI-4304.  2. Please clarify the location of the lower set of couplers. Does the lower set of couplers weld to the bottom flange?						<b>ANSWER:</b>  Find attached Harris RFI dated July 17, 2014 and SK2 and SK3 from response to RFI #T-1408 with added blue markups/comments.  SK3 with the response to RFI #T-1408 shows the elevation of the top of steel to be above the elevation of the top of concrete.  1. Please confirm the top of steel is intended to come out of the top of concrete and provide a revised 9/SI-3702 detail.  a. Please clarify the detail for the welded couplers at the top flange.  For TR19.9 and 20.1, a plate for the bottom welded couplers are not shown on SI-4304.  2. Please clarify the location of the lower set of couplers. Does the lower set of couplers weld to the bottom flange?





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<b>T-1535</b>	<b>SSS - PE201 Connection Clarifications at Roof Level</b>	<b>Closed</b>	<b>CR</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>08/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 505 SK1 for items 1 to 6: 1) Supply the elevation of the (4) HSS12x6x1/4. 2a) Supply a connection detail. 3) Supply a connection detail. 4) Supply missing dimension. 6) The noted slab edge detail is referenced but no slab information is shown. Please clarify.						<b>ANSWER:</b> See attached CD RFI # 505 SK1 for items 1 to 6: 1) Supply the elevation of the (4) HSS12x6x1/4. 2a) Supply a connection detail. 3) Supply a connection detail. 4) Supply missing dimension. 6) The noted slab edge detail is referenced but no slab information is shown. Please clarify.
<b>T-1536</b>	<b>SCS - Concrete Cover &amp; Tolerance on Foundation Wall Terminator Rebar</b>	<b>Closed</b>	<b>01</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>08/04/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> Drawing SI-3201 and other similar foundation wall sections call out a 2" concrete cover on the terminators of the foundation wall vertical rebar. These vertical bars have no adjustability because one end is a terminator and the other end is a male coupler end. The elevation of the top of terminator will be dependent on the coupler elevation above the Lift 3 Foundation Wall. The ground floor slab is 10" thick and sloped along the top of foundation wall.  To allow for maximum deviation to accommodate for the slope of the slab, tolerances in rebar fabrication and location from the previous wall lifts and the final wall lift: 1. Shimmick proposes to change the concrete cover from 2" to 5" and increase the tolerance to 2.5". Please confirm this is acceptable. 2. If the proposal is not acceptable, please provide a concrete cover and tolerance that is acceptable.						<b>ANSWER:</b> Drawing SI-3201 and other similar foundation wall sections call out a 2" concrete cover on the terminators of the foundation wall vertical rebar. These vertical bars have no adjustability because one end is a terminator and the other end is a male coupler end. The elevation of the top of terminator will be dependent on the coupler elevation above the Lift 3 Foundation Wall. The ground floor slab is 10" thick and sloped along the top of foundation wall.  To allow for maximum deviation to accommodate for the slope of the slab, tolerances in rebar fabrication and location from the previous wall lifts and the final wall lift: 1. Shimmick proposes to change the concrete cover from 2" to 5" and increase the tolerance to 2.5". Please confirm this is acceptable. 2. If the proposal is not acceptable, please provide a concrete cover and tolerance that is acceptable.
<b>T-1537</b>	<b>SSS - ST202 Connection Clarification at HSS Posts</b>	<b>Closed</b>	<b>CR</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>08/07/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 508 SK1: Confirm the connection as shown is acceptable or supply a new detail.						<b>ANSWER:</b> See attached CD RFI # 508 SK1: Confirm the connection as shown is acceptable or supply a new detail.



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<b>T-1538</b>	<b>SSS - ST202 &amp; PE203 Connection Clarification and Missing Connection</b>	<b>Closed</b>	<b>CR</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>08/06/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 509 SK1 & SK2 for items 1 to 3:  1) Please confirm the noted dimension.  2) Please supply a connection detail for the HSS post to the underside of slab beside B71.  3) Please confirm detail 9/S1-7600 applies at the noted location.						<b>ANSWER:</b> See attached CD RFI # 509 SK1 & SK2 for items 1 to 3:  1) Please confirm the noted dimension.  2) Please supply a connection detail for the HSS post to the underside of slab beside B71.  3) Please confirm detail 9/S1-7600 applies at the noted location.
<b>T-1539</b>	<b>SSS - SE201 &amp; SE202 Framing Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>08/06/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 514 SK1: Confirm the intent is to have the noted beam as a W40x431. If yes, supply a connection detail for the W40 to the W16x26.						<b>ANSWER:</b> See attached CD RFI # 514 SK1: Confirm the intent is to have the noted beam as a W40x431. If yes, supply a connection detail for the W40 to the W16x26.
<b>T-1540</b>	<b>SSS - Stiffener Connection Clarification at Bus Deck Support Beam</b>	<b>Closed</b>	<b>CR</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>08/06/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 521 SK1: This PL1/2x6 stiffener per 2/S1-8014 cannot be welded to the top flange of the W30x391 because the flange has been coped for the skewed W40x277 and it blocks the field moment welding at the bottom flange of the W30x391. Confirm the stiffener may be omitted or supply an alternate solution.						<b>ANSWER:</b> See attached CD RFI # 521 SK1: This PL1/2x6 stiffener per 2/S1-8014 cannot be welded to the top flange of the W30x391 because the flange has been coped for the skewed W40x277 and it blocks the field moment welding at the bottom flange of the W30x391. Confirm the stiffener may be omitted or supply an alternate solution.
<b>T-1541</b>	<b>SSS - ST202 &amp; PE203 Framing Clarifications at Lower Level Concourse</b>	<b>Closed</b>	<b>CR</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>09/10/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 511 SK1 & SK2: 2/S1-7004 or any other details on S1-7004 do not appear to show the framing at the Lower Concourse Level as						<b>ANSWER:</b> See attached CD RFI # 511 SK1 & SK2: 2/S1-7004 or any other details on S1-7004 do not appear to show the framing at the Lower Concourse



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	shown on A/A1-7004 (SK1). Please clarify.					Level as shown on A/A1-7004 (SK1). Please clarify.
T-1542	SSS - Steel Plate Availability	Closed	CR	07/24/2014	08/03/2014	08/21/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: After reviewing the specifications and standards for ASTM A572 Gr. 50 steel, along with speaking with our steel suppliers, Herrick has found that the aforementioned grade of material is not manufactured below 3/16". Please confirm that A606 Type 4 Mod is acceptable where A572 Gr 50 is specified at 1/8" (Please see attached SK1).						ANSWER: After reviewing the specifications and standards for ASTM A572 Gr. 50 steel, along with speaking with our steel suppliers, Herrick has found that the aforementioned grade of material is not manufactured below 3/16". Please confirm that A606 Type 4 Mod is acceptable where A572 Gr 50 is specified at 1/8" (Please see attached SK1).
T-1542.1	SSS - Steel Plate Availability	Closed	CR	07/28/2014	08/07/2014	08/15/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: After reviewing the specifications and standards for ASTM A572 Gr. 50 steel, along with speaking with our steel suppliers, Herrick has found that the aforementioned grade of material is not manufactured below 3/16". Please advise.						ANSWER: After reviewing the specifications and standards for ASTM A572 Gr. 50 steel, along with speaking with our steel suppliers, Herrick has found that the aforementioned grade of material is not manufactured below 3/16". Please advise.



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<b>T-1542.2</b>	<b>SSS - Steel Plate Availability</b>	<b>Closed</b>	<b>CR</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>09/04/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> The Type 3 drag connection detail, 1/S1-5018, calls for shim plates as required at connections between beams of different depth. The contract documents specify, "Shims/Fillers: Match Material Strength of Connecting Members", which in this case would be ASTM A572 Gr. 50.  After reviewing the specifications and standards for ASTM A572 Gr. 50 steel, along with speaking with our steel suppliers, we have found that the aforementioned grade of material is not manufactured below 3/16".  Please advise.						
						<b>ANSWER:</b> The Type 3 drag connection detail, 1/S1-5018, calls for shim plates as required at connections between beams of different depth. The contract documents specify, "Shims/Fillers: Match Material Strength of Connecting Members", which in this case would be ASTM A572 Gr. 50.  After reviewing the specifications and standards for ASTM A572 Gr. 50 steel, along with speaking with our steel suppliers, we have found that the aforementioned grade of material is not manufactured below 3/16".  Please advise.
<b>T-1543</b>	<b>SSS - ST201A Missing Information</b>	<b>Closed</b>	<b>CR</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>08/07/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 500 SK1 for items 1 to 4: 1) Confirm the missing beam size should read W12x26. 2) Supply missing dimension. 3) Supply the slab type. 4) The noted (2) davits are shown located on Grid 'D' but A1-7003 shows the davits located north of Grid 'D' with providing the location. Please provide the north/south location of the davits.						
						<b>ANSWER:</b> See attached CD RFI # 500 SK1 for items 1 to 4: 1) Confirm the missing beam size should read W12x26. 2) Supply missing dimension. 3) Supply the slab type. 4) The noted (2) davits are shown located on Grid 'D' but A1-7003 shows the davits located north of Grid 'D' with providing the location. Please provide the north/south location of the davits.
<b>T-1544</b>	<b>SSS - SE201 &amp; SE202 Connection Clarification at W16X36 Beams</b>	<b>Closed</b>	<b>CR</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 522 SK1 & SK2: Confirm it is acceptable to connect the noted W16 beams per 1/S1-5011 as 1/S1-5010 will not work with the connection on the opposite side.						
						<b>ANSWER:</b> See attached CD RFI # 522 SK1 & SK2: Confirm it is acceptable to connect the noted W16 beams per 1/S1-5011 as 1/S1-5010 will not work with the connection on the opposite side.



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<b>T-1545</b>	<b>SSS - SE201 &amp; SE202 Beam Connection and Location Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>08/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 523 SK1 for items 1 & 2:  1) Confirm it is acceptable to locate the beam 6" from the edge of slab.  2) Confirm it is acceptable to connect the beam per 1/S1-5011 at both ends to avoid fouling the HSS beam connection.						<b>ANSWER:</b> See attached CD RFI # 523 SK1 for items 1 & 2:  1) Confirm it is acceptable to locate the beam 6" from the edge of slab.  2) Confirm it is acceptable to connect the beam per 1/S1-5011 at both ends to avoid fouling the HSS beam connection.
<b>T-1546</b>	<b>SSS - Brace Connection Clarificationat GL 3 W40X149 Beam</b>	<b>Closed</b>	<b>CR</b>	<b>07/24/2014</b>	<b>08/03/2014</b>	<b>08/07/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 526 SK1: Confirm it is acceptable to omit the brace per 5/S1-5015 as there is insufficient room below the noted W40x149.						<b>ANSWER:</b> See attached CD RFI # 526 SK1: Confirm it is acceptable to omit the brace per 5/S1-5015 as there is insufficient room below the noted W40x149.
<b>T-1547</b>	<b>BGP - Seismic Joint Deformed Bar Anchor Variance</b>	<b>Closed</b>	<b>01</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>07/25/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> Please reference Detail 2, Sheet S 1-3205.  Please clarify the following deformed bar questions:  1. 1" diameter deformed bar is a non-standard diameter for deformed bar. This will require the bar to be specially manufactured which could take up to 4 weeks to procure. 3/4" diameter deformed bar is the largest standard size readily available. Is it acceptable to use 3/4" diameter in lieu of 1" diameter deformed bar?  2. The detail depicts a head on the end of the deformed bar. Headed deformed bar is also non-standard (see attached Nelson Stud Deformed Bar Product Data) and currently found to be unattainable. Is it the intent of the engineer to include a headed deformed bar anchor? Is it acceptable to use a standard A 108 bar per ASTM A496 as detailed in the attached product data?						<b>ANSWER:</b> Please reference Detail 2, Sheet S 1-3205.  Please clarify the following deformed bar questions:  1. 1" diameter deformed bar is a non-standard diameter for deformed bar. This will require the bar to be specially manufactured which could take up to 4 weeks to procure. 3/4" diameter deformed bar is the largest standard size readily available. Is it acceptable to use 3/4" diameter in lieu of 1" diameter deformed bar?  2. The detail depicts a head on the end of the deformed bar. Headed deformed bar is also non-standard (see attached Nelson Stud Deformed Bar Product Data) and currently found to be unattainable. Is it the intent of the engineer to include a headed deformed bar anchor? Is it acceptable to use a standard A 108 bar per ASTM A496 as detailed in the



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T-1548	<div>BGP - Protection Slab at Seismic Joint</div> <div>From: Webcor Construction LP</div> <div>Claude Titche</div> <div>REQUEST: Please reference Detail 3 of A1-8881, S1-2027, and the attached Drawings.  Please confirm the protection slab is to stop 5" east of GL 35 as scaled on attached Detail 3 of Sheet AI - 8881.  Please confirm it is acceptable to eliminate the 12" section of protection slab against the east CDSM wall.  Please reference attached drawing S1-2027. If the protection slab is required to end 5" east of GL 35, the slab will be poured halfway up the slope on the center and north pits as depicted in S1-2027. Is it the engineer's intent to end the slab halfway up the pit? If not, please provide details of protection slab location at the seismic joint at the center and north pits.</div>	Closed	01	07/25/2014	08/04/2014	07/29/2014
						<div>attached product data?</div> <div>3. If the 3/4" diameter deformed bar is unacceptable, is it acceptable to use 1" (#8) welded rebar in lieu of the 1" deformed bar? Furthermore, will the rebar require headed reinforcement?</div> <div>ANSWER: Please reference Detail 3 of A1-8881, S1-2027, and the attached Drawings.  Please confirm the protection slab is to stop 5" east of GL 35 as scaled on attached Detail 3 of Sheet AI - 8881.  Please confirm it is acceptable to eliminate the 12" section of protection slab against the east CDSM wall.  Please reference attached drawing S1-2027. If the protection slab is required to end 5" east of GL 35, the slab will be poured halfway up the slope on the center and north pits as depicted in S1-2027. Is it the engineer's intent to end the slab halfway up the pit? If not, please provide details of protection slab location at the seismic joint at the center and north pits.</div>



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<b>T-1549</b>	<b>BGP - 3rd Wall Lift Vertical Unistrut Spacing Tolerance</b>	<b>Closed</b>	<b>01</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b> Vertical Unistrut embedded in the 3rd wall lift is spaced at 10' O.C. per plan drawing AI -6231. Due to locations of wall bulkheads, form work panel ribs, and other embedded items, the contractor requests a tolerance of plus/minus 4" off the 10' O.C. spacing. Please confirm if this is acceptable and if not, please provide an acceptable lateral tolerance.						<b>ANSWER:</b> Vertical Unistrut embedded in the 3rd wall lift is spaced at 10' O.C. per plan drawing AI -6231. Due to locations of wall bulkheads, form work panel ribs, and other embedded items, the contractor requests a tolerance of plus/minus 4" off the 10' O.C. spacing. Please confirm if this is acceptable and if not, please provide an acceptable lateral tolerance.
<b>T-1550</b>	<b>SSS - PE203 Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 512 SK1 for items 1 & 2:  1) Please confirm the noted connection (similar to RFI T-1105, SK 240.1, CD 183.1) is acceptable or supply a new detail.  2) The connection per 1/S1-7630 (shown in item 1) and the connection per 1/S1-7600 foul each other at this location. Please supply a solution.						<b>ANSWER:</b> See attached CD RFI # 512 SK1 for items 1 & 2:  1) Please confirm the noted connection (similar to RFI T-1105, SK 240.1, CD 183.1) is acceptable or supply a new detail.  2) The connection per 1/S1-7630 (shown in item 1) and the connection per 1/S1-7600 foul each other at this location. Please supply a solution.
<b>T-1551</b>	<b>SSS - SE201 &amp; SE202 Connection Clarification at HSS Posts</b>	<b>Closed</b>	<b>CR</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/07/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 516 SK1 to SK3: The L8x4x1/2 is insufficient for the HSS10x10 posts in SE201 & SE202. Please supply an alternate detail.						<b>ANSWER:</b> See attached CD RFI # 516 SK1 to SK3: The L8x4x1/2 is insufficient for the HSS10x10 posts in SE201 & SE202. Please supply an alternate detail.
<b>T-1552</b>	<b>BGP - Pit Rebar in South Pit at GL 35</b>	<b>Closed</b>	<b>01</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>07/29/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b> Please reference Contract Drawings SI-2027, SI-3010, SI-3004, and the attached drawings.  Sheet S1-2027 depicts the east edge of the south pit 2'-5"						<b>ANSWER:</b> Please reference Contract Drawings SI-2027, SI-3010, SI-3004, and the attached drawings.  Sheet S1-2027 depicts the east edge of the south pit



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>west of GL 35. As-built location of east edge of south pit is 1'-0" west of GL 35. The south pit at GL 35 has typical pit rebar per Detail 4, S1-3004. Typically, the pit rebar extends 4' beyond the edge of pit and 4' in height (see attached drawing SI- 3010). At the as-built location of the pit, pit rebar will extend into the seismic joint gap by approximately 14" as depicted in the attached drawing. Please provide alternate pit rebar detail for the south pit at GL 35.</p> <p>Please note, pit rebar for all pits in Area 16 have been fabricated.</p>					
	<p>2'-5" west of GL 35. As-built location of east edge of south pit is 1'-0" west of GL 35. The south pit at GL 35 has typical pit rebar per Detail 4, S1-3004. Typically, the pit rebar extends 4' beyond the edge of pit and 4' in height (see attached drawing SI- 3010). At the as-built location of the pit, pit rebar will extend into the seismic joint gap by approximately 14" as depicted in the attached drawing. Please provide alternate pit rebar detail for the south pit at GL 35.</p> <p>Please note, pit rebar for all pits in Area 16 have been fabricated.</p>					
<b>T-1553</b>	<b>SSS - SE201 &amp; SE202 Roof Level Framing Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/07/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>See attached CD RFI # 524 SK1 for items 1 to 4:</p> <p>1) These (2) beams have been located on center of the HSS10x10 posts above per 1/S1-7102. This results in the beams being 1'-0 from the edge of wall as shown but 8/S1-7631 indicates 10". Confirm 1'-0 is acceptable.</p> <p>2) Detail 11/S1-7631 does not indicate that slab edge plate per 8/S1-5000 is required. Confirm that is the intent.</p> <p>3) 6/S1-7631 is shown at the west side of the slab openings but it is not clear if slab edge plate per 8/S1-5000 is required. Clarify the slab edge requirement on the east and west sides of the openings.</p> <p>4) Depending on the response to item 3, supply a connection detail for the HSS beam.</p>			<p>See attached CD RFI # 524 SK1 for items 1 to 4:</p> <p>1) These (2) beams have been located on center of the HSS10x10 posts above per 1/S1-7102. This results in the beams being 1'-0 from the edge of wall as shown but 8/S1-7631 indicates 10". Confirm 1'-0 is acceptable.</p> <p>2) Detail 11/S1-7631 does not indicate that slab edge plate per 8/S1-5000 is required. Confirm that is the intent.</p> <p>3) 6/S1-7631 is shown at the west side of the slab openings but it is not clear if slab edge plate per 8/S1-5000 is required. Clarify the slab edge requirement on the east and west sides of the openings.</p> <p>4) Depending on the response to item 3, supply a connection detail for the HSS beam.</p>			





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<b>T-1553.1</b>	<b>SSS - SE201 &amp; SE202 Stud Length Requirement and Dimension Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/08/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 524.1 SK1 for items 1 & 2: 1) Please update the stud length requirements to suit the revised wall thicknesses/beam locations. 2) This 5" dimension is shown as 1'-9 3/4 on SKS-0383 in RFI T-1544 (SK 712, CD 522). Please clarify which dimension is correct.			See attached CD RFI # 524.1 SK1 for items 1 & 2: 1) Please update the stud length requirements to suit the revised wall thicknesses/beam locations. 2) This 5" dimension is shown as 1'-9 3/4 on SKS-0383 in RFI T-1544 (SK 712, CD 522). Please clarify which dimension is correct.			
<b>T-1553.2</b>	<b>SSS - SE201 &amp; SE202 Roof Level Framing Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 524.2 SK1 for items 1 to 3: 1) This dimension is 11" per 10/A1-2969. Please confirm that is correct or supply the dimensions at the (2) hi-lited clouds to locate the slab opening. 2) It is not clear where the noted beam is located. This dimension is not shown anywhere except RFI T-1544 (SK 712, CD 522) SKS-0411, which shows the beam off-set 1'-5 3/4 from the edge of the opening but this detail appears to place the beam below the wall whose thickness is not known. Please supply the location for the beam. 3) The wall is shown as 1'-6 thick in detail 10/A1-2969. If that is the case the 6" long studs will extend outside the concrete. Please clarify.			See attached CD RFI # 524.2 SK1 for items 1 to 3: 1) This dimension is 11" per 10/A1-2969. Please confirm that is correct or supply the dimensions at the (2) hi-lited clouds to locate the slab opening. 2) It is not clear where the noted beam is located. This dimension is not shown anywhere except RFI T-1544 (SK 712, CD 522) SKS-0411, which shows the beam off-set 1'-5 3/4 from the edge of the opening but this detail appears to place the beam below the wall whose thickness is not known. Please supply the location for the beam. 3) The wall is shown as 1'-6 thick in detail 10/A1-2969. If that is the case the 6" long studs will extend outside the concrete. Please clarify.			
<b>T-1553.3</b>	<b>SSS - SE201 &amp; SE202 Roof Level Framing Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/28/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 524.3 SK1: The W16x26 beam has been moved to 1'-9 3/4 south of the slab edge in ASI-127. The result is that the beam will be very close to the beam on Grid F with only 3/4" clear between the flanges and the connection per 1/S1-5010 is not possible. Please supply a solution.			See attached CD RFI # 524.3 SK1: The W16x26 beam has been moved to 1'-9 3/4 south of the slab edge in ASI-127. The result is that the beam will be very close to the beam on Grid F with only 3/4" clear between the flanges and the connection per 1/S1-5010 is not possible. Please supply a solution.			



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<b>T-1554</b>	<b>SSS - PE201 Missing Dimensions</b>	<b>Closed</b>	<b>CR</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 491 SK1 for items 1 to 7: 1) Supply the missing clouded dimension and confirm the outriggers are spaced in 4 equal spaces. 2) Supply the missing clouded dimension and confirm the outriggers are spaced in 3 equal spaces. 3) Supply the missing clouded dimension and confirm the outriggers are spaced in 3 equal spaces. 4) Supply missing clouded dimension. 5) Supply missing clouded dimension. 6) Supply missing clouded dimension. 7) Confirm items 1 to 6 may be applied at details 2,3/S1/S1-7001; 1,2,3/S1-7002 & 1,2,3/S1-7003 unless the framing is clearly shown otherwise.						<b>ANSWER:</b> See attached CD RFI # 491 SK1 for items 1 to 7: 1) Supply the missing clouded dimension and confirm the outriggers are spaced in 4 equal spaces. 2) Supply the missing clouded dimension and confirm the outriggers are spaced in 3 equal spaces. 3) Supply the missing clouded dimension and confirm the outriggers are spaced in 3 equal spaces. 4) Supply missing clouded dimension. 5) Supply missing clouded dimension. 6) Supply missing clouded dimension. 7) Confirm items 1 to 6 may be applied at details 2,3/S1/S1-7001; 1,2,3/S1-7002 & 1,2,3/S1-7003 unless the framing is clearly shown otherwise.
<b>T-1555</b>	<b>SSS - PE201 HSS Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/06/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 492 SK1 & SK2 for items 1 & 2:  1) Detail 2/S1-7602 as shown does not apply at this location. Confirm the modified detail as shown is acceptable.  2) Confirm it is acceptable to cut the vertical leg of the angle as shown to fit below the cantilevered horizontal HSS10x8.						<b>ANSWER:</b> See attached CD RFI # 492 SK1 & SK2 for items 1 & 2:  1) Detail 2/S1-7602 as shown does not apply at this location. Confirm the modified detail as shown is acceptable.  2) Confirm it is acceptable to cut the vertical leg of the angle as shown to fit below the cantilevered horizontal HSS10x8.
<b>T-1556</b>	<b>SSS - E201 &amp; E202 Connection Clarification with Current Framing</b>	<b>Closed</b>	<b>CR</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 527 SK1: With the framing shown it is not clear how detail 3/S1-7603 is to be applied. Please clarify.						<b>ANSWER:</b> See attached CD RFI # 527 SK1: With the framing shown it is not clear how detail 3/S1-7603 is to be applied. Please clarify.
<b>T-1556.1</b>	<b>SSS - E201 &amp; E202 Connection Clarification with Current Framing</b>	<b>Open</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/15/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						



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	<b>REQUEST:</b> See attached CD RFI # 525.1 SK1 for items 1 to 3: 1) The WT22x167.5 fouls the continuity plates. Please provide a detail showing the solution. 2) Supply hole size for 2 1/2" dia. pretensioned rod thru the continuity plates 3) The WT22x167.5 fouls the continuity plates. Please provide a detail showing the solution.					<b>ANSWER:</b> See attached CD RFI # 525.1 SK1 for items 1 to 3: 1) The WT22x167.5 fouls the continuity plates. Please provide a detail showing the solution. 2) Supply hole size for 2 1/2" dia. pretensioned rod thru the continuity plates 3) The WT22x167.5 fouls the continuity plates. Please provide a detail showing the solution.
T-1557	<b>SSS - Domestic Material Availability</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	Void	CR	07/25/2014	08/04/2014	
	<b>REQUEST:</b> At the Second Level Shaw Alley Bridge we have HSS 5 X .500 posts (Detail 6 on S1-5013). This size is not available domestically. Please confirm the substitution to HSS 5.563 X .500 as to meet the USA requirement is acceptable. Otherwise, provide an alternate solution.					<b>ANSWER:</b> At the Second Level Shaw Alley Bridge we have HSS 5 X .500 posts (Detail 6 on S1-5013). This size is not available domestically. Please confirm the substitution to HSS 5.563 X .500 as to meet the USA requirement is acceptable. Otherwise, provide an alternate solution.
T-1557.1	<b>SSS - Domestic Material Availability</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	Closed	CR	07/28/2014	08/07/2014	08/13/2014
	<b>REQUEST:</b> Detail 6 on S1-5013 calls for HSS 5 X .500 posts at the Second Level Shaw Alley Bridge. This size is not available domestically. Please advise.					<b>ANSWER:</b> Detail 6 on S1-5013 calls for HSS 5 X .500 posts at the Second Level Shaw Alley Bridge. This size is not available domestically. Please advise.



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<b>T-1558</b>	<b>BGP - Bracing removal sequence on the East end of Zone 4</b>	<b>Pending</b>	<b>01</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Bracing removal sequence on the East end of Zone 4 WOJV is proposing the following sequence for the bracing removal for the east side of Zone 4. See sketch SK1, attached.  Sequence 1. Remove level D struts and walers from within the green clouded area up to GL- 32.2 once the mat slab beneath has reached adequate strength. 2. Remove level D struts within the Blue clouded area STD-65 to 74, 82 & 83 and all corresponding walers once the mat slab beneath has reached adequate strength, the sequence for de-stressing will be the diagonals struts should be all de-stressing prior to the 4 remaining cross lot struts (STD-65,66,67 & 68).  For the remaining levels A, B and C WOJV is proposing to follow a similar removal sequence as Level D  Please confirm if this sequence would be acceptable.						<b>ANSWER:</b>  Bracing removal sequence on the East end of Zone 4 WOJV is proposing the following sequence for the bracing removal for the east side of Zone 4. See sketch SK1, attached.  Sequence 1. Remove level D struts and walers from within the green clouded area up to GL- 32.2 once the mat slab beneath has reached adequate strength. 2. Remove level D struts within the Blue clouded area STD-65 to 74, 82 & 83 and all corresponding walers once the mat slab beneath has reached adequate strength, the sequence for de-stressing will be the diagonals struts should be all de-stressing prior to the 4 remaining cross lot struts (STD-65,66,67 & 68).  For the remaining levels A, B and C WOJV is proposing to follow a similar removal sequence as Level D  Please confirm if this sequence would be acceptable.
<b>T-1559</b>	<b>SSS - TPG1 Stiffener Connection Clarification at GL 3</b>	<b>Closed</b>	<b>CR</b>	<b>07/25/2014</b>	<b>08/04/2014</b>	<b>08/05/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 529 SK1 & SK2 for items 1a & 1b:  1a) The typical connection for the TPG1 to the perimeter BU Beam is per 6/S1-5010 as shown on SK2 but at this location a pull-out shear plate is shown. Supply thickness, bolt and weld information for the shear plate connection if that is the connection to be used at the noted location. 1b) The stiffener will foul the bolts for the top plate per 7/S1-5032 if the connection per 6/S1-5010 is used. Please provide a solution.						<b>ANSWER:</b>  See attached CD RFI # 529 SK1 & SK2 for items 1a & 1b: 1a) The typical connection for the TPG1 to the perimeter BU Beam is per 6/S1-5010 as shown on SK2 but at this location a pull-out shear plate is shown. Supply thickness, bolt and weld information for the shear plate connection if that is the connection to be used at the noted location. 1b) The stiffener will foul the bolts for the top plate per 7/S1-5032 if the connection per 6/S1-5010 is used. Please provide a solution.
<b>T-1559.1</b>	<b>SSS - TPG1 Stiffener Connection Clarification at GL 3</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/08/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						





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<b>T-1562</b>	<b>SSS - Protected Zone Signage</b>	<b>Closed</b>	<b>CR</b>	<b>07/28/2014</b>	<b>08/07/2014</b>	<b>07/30/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Detail 10/S1-4202 shows the protected zone signage attached directly to both sides of the beam. The response to submittal package TG0701-37A includes a note directing the contractor to remove the signage before SFRM application, and then reattaching it after the SFRM is applied (See SK1). Please confirm this note is to be ignored.		<b>ANSWER:</b> Detail 10/S1-4202 shows the protected zone signage attached directly to both sides of the beam. The response to submittal package TG0701-37A includes a note directing the contractor to remove the signage before SFRM application, and then reattaching it after the SFRM is applied (See SK1). Please confirm this note is to be ignored.				
<b>T-1563</b>	<b>SSS - Erection Access Confirmation at TPG1</b>	<b>Closed</b>	<b>CR</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/18/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> There is a 3 7/16" gap between the flanges of the TPG1 and W40x264. The north end of the W40x264 is connected to the W40x297 per 1/S1-5010 and the south end is connected to the BU-40x28 per 1/S1-8001 including a field moment weld at the top and bottom flange of the W40x264. Please review and supply any changes that are required for erection access.  Alternatively, please confirm that it is acceptable to move CP1D (shown on S1-4002) 6-inches away from TPG01.		<b>ANSWER:</b> There is a 3 7/16" gap between the flanges of the TPG1 and W40x264. The north end of the W40x264 is connected to the W40x297 per 1/S1-5010 and the south end is connected to the BU-40x28 per 1/S1-8001 including a field moment weld at the top and bottom flange of the W40x264. Please review and supply any changes that are required for erection access.  Alternatively, please confirm that it is acceptable to move CP1D (shown on S1-4002) 6-inches away from TPG01.				
<b>T-1564</b>	<b>SSS - Bus Ramp Support Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 489 SK1 for items 1 & 2:  1) Please provide the hole diameter and hole size as noted.  2) Please provide the weld size for the 2" web plate to the 2" end plate.		<b>ANSWER:</b> See attached CD RFI # 489 SK1 for items 1 & 2:  1) Please provide the hole diameter and hole size as noted.  2) Please provide the weld size for the 2" web plate to the 2" end plate.				



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<b>T-1565</b>	<b>SSS - ST403 Embedded Angle Elevation &amp; Orientation and Curb Dimensions</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/13/2014</b>
<b>REQUEST:</b>  See attached CD RFI # 481 SK1 & SK2: 1). At Stair # 403 at the train platform level verify the embedded angles for the elevator posts per detail 11/S1-7600 are at elevation 29'-0 and confirm the angles orientation onto the concrete curb. 2). Confirm the 5'-4 dimension is to inside face of curb shown on SK1. 3). Confirm the 5'-10 & 6'-1 dimensions are to inside face of curb shown on SK2.		<b>ANSWER:</b>  See attached CD RFI # 481 SK1 & SK2: 1). At Stair # 403 at the train platform level verify the embedded angles for the elevator posts per detail 11/S1-7600 are at elevation 29'-0 and confirm the angles orientation onto the concrete curb. 2). Confirm the 5'-4 dimension is to inside face of curb shown on SK1. 3). Confirm the 5'-10 & 6'-1 dimensions are to inside face of curb shown on SK2.				
<b>T-1566</b>	<b>SSS - Pretension Rod Connection Clarification at Second Level GL 3</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/06/2014</b>
<b>REQUEST:</b>  See attached CD RFI # 525 SK1: The columns on Grid 3 indicate that the WT's/Pretensioned Rods in the web of the columns are to extend up to the Bus Deck Level. If this is the intent, supply a detail showing how the continuity plates per 4/S1-5012 interface with the WT/Pretensioned Rods at the Second Level.		<b>ANSWER:</b>  See attached CD RFI # 525 SK1: The columns on Grid 3 indicate that the WT's/Pretensioned Rods in the web of the columns are to extend up to the Bus Deck Level. If this is the intent, supply a detail showing how the continuity plates per 4/S1-5012 interface with the WT/Pretensioned Rods at the Second Level.				
<b>T-1566.1</b>	<b>SSS - Pretension Rod Connection Clarification at Second Level GL 3</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>09/03/2014</b>	<b>09/13/2014</b>	<b>09/16/2014</b>
<b>REQUEST:</b>  See attached CD RFI # 525.1 SK1 for items 1 to 3: 1) The WT22x167.5 fouls the continuity plates. Please provide a detail showing the solution. 2) Supply hole size for 2 1/2" dia. pretensioned rod thru the continuity plates 3) The WT22x167.5 fouls the continuity plates. Please provide a detail showing the solution.		<b>ANSWER:</b>  See attached CD RFI # 525.1 SK1 for items 1 to 3: 1) The WT22x167.5 fouls the continuity plates. Please provide a detail showing the solution. 2) Supply hole size for 2 1/2" dia. pretensioned rod thru the continuity plates 3) The WT22x167.5 fouls the continuity plates. Please provide a detail showing the solution.				
<b>T-1566.2</b>	<b>SSS - Pretension Rod Connection Clarification at Second Level GL 3</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>10/01/2014</b>	<b>10/11/2014</b>	<b>10/09/2014</b>







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<b>T-1567.1</b>	<b>BGP - Substitutue Fly-Ash Product - Salt River - Attached Cemex Letter</b>	<b>Closed</b>	<b>01</b>	<b>08/11/2014</b>	<b>08/21/2014</b>	<b>08/11/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						
Please reference attached CEMEX Letter and product data sheet for the Salt River brand Fly-ash, product data sheet for Headwaters brand Fly-ash and product comparison spreadsheet.						
If needed, due to a shortage in supply of the Headwaters brand fly-ash, please confirm it is acceptable to substitute Salt River brand fly-ash for use in all TG06.0 cast-in-place mix designs.						
If Salt River Fly-ash is acceptable, will the design team require trial batches?						
<b>ANSWER:</b>						
Please reference attached CEMEX Letter and product data sheet for the Salt River brand Fly-ash, product data sheet for Headwaters brand Fly-ash and product comparison spreadsheet.						
If needed, due to a shortage in supply of the Headwaters brand fly-ash, please confirm it is acceptable to substitute Salt River brand fly-ash for use in all TG06.0 cast-in-place mix designs.						
If Salt River Fly-ash is acceptable, will the design team require trial batches?						
<b>T-1568</b>	<b>BGP - Pile Location Discrepancy at GL E/34.5 - Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/12/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						
SCCI is in receipt of RFI response T-0741. RFI response T-0741 references RFI response T-0264.3, which states: " ... Note that a pile in this location will require the pile to be cut off at a lower elevation than the typical detail, which will involve a larger block-out..."						
1. Please provide an elevation, referenced in the above mentioned RPI response, which the pile will be cut.						
RFI response T-0264.3 also states: " ... the mat shall be re-braced at the block-out by TG03 ... "						
2. Please provide a detailed description of this required re-brace and how it will effect TG06 formwork, rebar, etc.						
<b>ANSWER:</b>						
SCCI is in receipt of RFI response T-0741. RFI response T-0741 references RFI response T-0264.3, which states: " ... Note that a pile in this location will require the pile to be cut off at a lower elevation than the typical detail, which will involve a larger block-out..."						
1. Please provide an elevation, referenced in the above mentioned RPI response, which the pile will be cut.						
RFI response T-0264.3 also states: " ... the mat shall be re-braced at the block-out by TG03 ... "						
2. Please provide a detailed description of this required re-brace and how it will effect TG06 formwork, rebar, etc.						
<b>T-1568.1</b>	<b>BGP - Pile Location Discrepancy at GL E/34.5 in Zone 4</b>	<b>Closed</b>	<b>01</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>09/02/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						
<b>ANSWER:</b>						



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>Please reference RPI response T-1568, attached photo of bridge pier/sleeve at Grid Lines 34.5/E and attached sketch.</p> <p>SCCI is in receipt of RFI response T-1568. The bridge pier in question is not fully contained within the sumps pits referenced in RPI response T-1568 (see photo attached). The galvanized pile sleeve encroaches into the Eastern most sump pit approximately 5/8".</p> <p>In order to facilitate the installation of this sump pit the provided contract details will need to be modified. The pile will need to be cut off at a lower elevation than the typical detail, which will require a larger block-out. The typical contract rebar details will need to be altered, as well as the typical waterproofing details.</p> <p>1. Please confirm the attached sketch showing revised rebar and formwork details is acceptable. 2. In order to install a deeper blockout, the galvanized sleeve will be cut down to 4" above bottom Of blockout. This cut will remove the sleeve flange. Please confirm this is acceptable.</p> <p>All waterproofing detail revisions will be performed per Grace's recommendation and referenced mat slab "Ready-to-Pour" sign-off document for Mat Slab 16</p>					<p>Please reference RPI response T-1568, attached photo of bridge pier/sleeve at Grid Lines 34.5/E and attached sketch.</p> <p>SCCI is in receipt of RFI response T-1568. The bridge pier in question is not fully contained within the sumps pits referenced in RPI response T-1568 (see photo attached). The galvanized pile sleeve encroaches into the Eastern most sump pit approximately 5/8".</p> <p>In order to facilitate the installation of this sump pit the provided contract details will need to be modified. The pile will need to be cut off at a lower elevation than the typical detail, which will require a larger block-out. The typical contract rebar details will need to be altered, as well as the typical waterproofing details.</p> <p>1. Please confirm the attached sketch showing revised rebar and formwork details is acceptable. 2. In order to install a deeper blockout, the galvanized sleeve will be cut down to 4" above bottom Of blockout. This cut will remove the sleeve flange. Please confirm this is acceptable.</p> <p>All waterproofing detail revisions will be performed per Grace's recommendation and referenced mat slab "Ready-to-Pour" sign-off document for Mat Slab 16</p>
<b>T-1568.2</b>	<b>BGP - Pile Location Discrepancy at GL E/34.5 in Zone 4</b>	<b>Closed</b>	<b>CR</b>	<b>09/15/2014</b>	<b>09/25/2014</b>	<b>09/25/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>						<b>ANSWER:</b>
SCCI is in receipt of RFI response T-1568.1. Please see attached sketch for reference.						SCCI is in receipt of RFI response T-1568.1. Please see attached sketch for reference.
SCCI offers this response for Item #2.						SCCI offers this response for Item #2.
1. Galvanizing will be removed from surface of the installed sleeve in order to weld a new identical flange centered 19" down from the top of sleeve. Weld will be a ¼" fillet weld, both top and bottom of flange all the way around the sleeve. The sleeve will then be repaired with approved cold galv spray.						1. Galvanizing will be removed from surface of the installed sleeve in order to weld a new identical flange centered 19" down from the top of sleeve. Weld will be a ¼" fillet weld, both top and bottom of flange all the way around the sleeve. The sleeve will then be repaired with approved cold galv spray.



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	<p>2. All waterproofing details will remain typical.</p> <p>Please confirm this is acceptable.</p>					<p>2. All waterproofing details will remain typical.</p> <p>Please confirm this is acceptable.</p>
<b>T-1569</b>	<b>BGP - 3rd Lift Foundation Walls - Area 1 CJ Layout</b>	<b>Closed</b>	<b>CR</b>	<b>07/29/2014</b>	<b>08/08/2014</b>	<b>08/04/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Please reference attached sketch that illustrates a proposed CJ layout for the 3rd wall lift foundation walls located in Zone 1, above the knock-out wall locations.  Please confirm this layout is acceptable?  Please note: This layout, if deemed acceptable, will be submitted in the comprehensive layout drawings for 3rd wall lift area 1 through 5.						<b>ANSWER:</b>  Please reference attached sketch that illustrates a proposed CJ layout for the 3rd wall lift foundation walls located in Zone 1, above the knock-out wall locations.  Please confirm this layout is acceptable?  Please note: This layout, if deemed acceptable, will be submitted in the comprehensive layout drawings for 3rd wall lift area 1 through 5.
<b>T-1570</b>	<b>SSS - PE201 Missing Information at Roof Level</b>	<b>Closed</b>	<b>CR</b>	<b>07/30/2014</b>	<b>08/09/2014</b>	<b>08/11/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 506 SK1 for items 1 to 4: 1) Supply location and size for slab opening. 2) Supply missing dimensions. 3) Confirm the C8x11.5's per 12/S1-5003 connect to the HSS beams per 6/S1-5011. 4) The davit base plate per 12/S1-7600 will foul the top plate in detail 6/S1-7604. 5) Detail 9/S1-5000 shows the slab support connecting to WF beams. Please supply a detail showing how to connect the braces at the HSS beams.						<b>ANSWER:</b>  See attached CD RFI # 506 SK1 for items 1 to 4: 1) Supply location and size for slab opening. 2) Supply missing dimensions. 3) Confirm the C8x11.5's per 12/S1-5003 connect to the HSS beams per 6/S1-5011. 4) The davit base plate per 12/S1-7600 will foul the top plate in detail 6/S1-7604. 5) Detail 9/S1-5000 shows the slab support connecting to WF beams. Please supply a detail showing how to connect the braces at the HSS beams.
<b>T-1571</b>	<b>SSS - PE302 Angle Brace Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/13/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<b>REQUEST:</b> See attached CD RFI # 532 SK1 & SK2: Per RFI T-0973 (SK 061A, CD 070A) the angle braces per 5/S1-5015 are not possible in the space. Confirm the alternate connection at this location is acceptable or supply a new detail.					<b>ANSWER:</b> See attached CD RFI # 532 SK1 & SK2: Per RFI T-0973 (SK 061A, CD 070A) the angle braces per 5/S1-5015 are not possible in the space. Confirm the alternate connection at this location is acceptable or supply a new detail.
T-1572	<b>SSS - Post Location Clarifications GL 19.1-19.9</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	Closed	CR	07/31/2014	08/10/2014	08/11/2014
	<b>REQUEST:</b> See attached CD RFI # 534 SK1 for items 1 & 2: 1) 3 spaces at 11'-8 3/4 equals 35'-2 1/4. Please clarify which dimensions are correct. 2) The HSS 10x10 post will be located east of Grid 19.1. The result is that the east wall of the HSS 10x10 cannot be fillet welded to the beam below as it extends beyond the beam flange as shown. Confirm that is acceptable or supply a solution.					<b>ANSWER:</b> See attached CD RFI # 534 SK1 for items 1 & 2: 1) 3 spaces at 11'-8 3/4 equals 35'-2 1/4. Please clarify which dimensions are correct. 2) The HSS 10x10 post will be located east of Grid 19.1. The result is that the east wall of the HSS 10x10 cannot be fillet welded to the beam below as it extends beyond the beam flange as shown. Confirm that is acceptable or supply a solution.
T-1573	<b>SSS - Dimension Clarifications for W-3 Support Beams on Ground Level</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	Closed	CR	07/31/2014	08/10/2014	08/11/2014
	<b>REQUEST:</b> See attached CD RFI # 537 SK1: Confirm the dimensions on B/S1-6050 are the dimensions to be used to locate the beams on S1-2305 noted thus * (beams indicated by bold red line on SK1).					<b>ANSWER:</b> See attached CD RFI # 537 SK1: Confirm the dimensions on B/S1-6050 are the dimensions to be used to locate the beams on S1-2305 noted thus * (beams indicated by bold red line on SK1).



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<b>T-1573.1</b>	<b>SSS - Dimension Clarifications for W-3 Support Beams on Ground Level</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/09/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 537.1 SK1: It is not clear where the row of beams west of Grid 19.1 & east of Grid 24.9 are located. Per B/S1-050 they are located 3'-8 off the noted Grids. S1-305 appears to show the beams centered under the 1'-3 wide thick portion of the slab and detail 9/S1-5022 does not provide a dimension. Confirm the beams are located 3'-8 off the noted Grids.						<b>ANSWER:</b> See attached CD RFI # 537.1 SK1: It is not clear where the row of beams west of Grid 19.1 & east of Grid 24.9 are located. Per B/S1-050 they are located 3'-8 off the noted Grids. S1-305 appears to show the beams centered under the 1'-3 wide thick portion of the slab and detail 9/S1-5022 does not provide a dimension. Confirm the beams are located 3'-8 off the noted Grids.
<b>T-1573.2</b>	<b>SSS - Dimension Clarifications for W-3</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 537.2 SK1 for items 1 to 3: 1) The reference to S1-8005 in RFI T-1573.1 (SK 732.1, CD 537.1) is not helpful in locating the W14x68* as dimensions are not provided. Supply the locations for the W14x68* beams from Grids 19.1 & 24.9. 2) S1-2305 locates the W12x14 beams 4'-3 from Grid 19.1 & 24.9. Confirm we are to proceed to revise the beam locations to center them under the 1'-3 slab step per RFI T-1573.1 (SK 732.1, CD 537.1). 3) Depending on the amount of off-set between the W12x14's and the W14x68, confirm it is acceptable to connect the W12x14 per 1/S1-5011 at one end.						<b>ANSWER:</b> See attached CD RFI # 537.2 SK1 for items 1 to 3: 1) The reference to S1-8005 in RFI T-1573.1 (SK 732.1, CD 537.1) is not helpful in locating the W14x68* as dimensions are not provided. Supply the locations for the W14x68* beams from Grids 19.1 & 24.9. 2) S1-2305 locates the W12x14 beams 4'-3 from Grid 19.1 & 24.9. Confirm we are to proceed to revise the beam locations to center them under the 1'-3 slab step per RFI T-1573.1 (SK 732.1, CD 537.1). 3) Depending on the amount of off-set between the W12x14's and the W14x68, confirm it is acceptable to connect the W12x14 per 1/S1-5011 at one end.
<b>T-1574</b>	<b>SSS - Dimension Discrepancies at W-3 Support Beams on Bus Deck Level</b>	<b>Closed</b>	<b>CR</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 539 SK1: The dimensions on S1-2607 (SK2) do not match the dimension on A/S1-6058 (SK1). Please clarify.						<b>ANSWER:</b> See attached CD RFI # 539 SK1: The dimensions on S1-2607 (SK2) do not match the dimension on A/S1-6058 (SK1). Please clarify.
<b>T-1575</b>	<b>SSS - Locations for W-3 Support Beams on Ground Level</b>	<b>Closed</b>	<b>CR</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						





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<b>T-1578</b>	<b>SSS - Pipe Grade Requirement</b>	<b>Closed</b>	<b>CR</b>	<b>07/31/2014</b>	<b>08/10/2014</b>	<b>08/06/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At Gridline 21 & 22 between D & F, S1-2505 shows two pipes 16 X 1.031, that are to be supplied as API 5L GR X52. Herrick has been unable to locate this grade of material that stays within the Buy America requirement. Please advise.			At Gridline 21 & 22 between D & F, S1-2505 shows two pipes 16 X 1.031, that are to be supplied as API 5L GR X52. Herrick has been unable to locate this grade of material that stays within the Buy America requirement. Please advise.			
<b>T-1579</b>	<b>BGP - Seismic Joint Stepped Embed Clarifications</b>	<b>Closed</b>	<b>01</b>	<b>08/01/2014</b>	<b>08/11/2014</b>	<b>08/19/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference Contract Drawing S1-3010 and Submittal TG0600-051 .1.			Please reference Contract Drawing S1-3010 and Submittal TG0600-051 .1.			
Please clarify the following:			Please clarify the following:			
1. Sheet 8.01 of Submittal TG0600-05 l.1 calls for a M30x4-1/2" stud welded to the embed. Due to the unavailability of M30x4-1/2" studs, please confirm it is acceptable to fillet welding M30x4-1/2" long cut pieces of fully threaded rod.			1. Sheet 8.01 of Submittal TG0600-05 l.1 calls for a M30x4-1/2" stud welded to the embed. Due to the unavailability of M30x4-1/2" studs, please confirm it is acceptable to fillet welding M30x4-1/2" long cut pieces of fully threaded rod.			
2. Submittal TG0600-051.1 requested design to determine bar lengths for the hooked #6 welded rebar. Please provide missing lengths for the hooked welded #6 rebar.			2. Submittal TG0600-051.1 requested design to determine bar lengths for the hooked #6 welded rebar. Please provide missing lengths for the hooked welded #6 rebar.			
3. Please clarify the (. 11 7) at the double flare bevel weld on the hooked #6 rebar and the deformed #6 bar. It is SCCI's understanding the 3/8" is the weld size and the 5 is the weld length. What is the (.117) referring to?			3. Please clarify the (. 11 7) at the double flare bevel weld on the hooked #6 rebar and the deformed #6 bar. It is SCCI's understanding the 3/8" is the weld size and the 5 is the weld length. What is the (.117) referring to?			
4. See attached SI-3010. The east embed top vertical leg appears to be welded to the horizontal leg with a square bevel and convex weld with a fillet on the other side. Typically, a square bevel has a given dimension that depicts the gap between plates. As no dimension is given and the other side is a fillet weld, it appears to be a flat fillet laid down on the joint, with no gap. Please confirm this is the design intent.			4. See attached SI-3010. The east embed top vertical leg appears to be welded to the horizontal leg with a square bevel and convex weld with a fillet on the other side. Typically, a square bevel has a given dimension that depicts the gap between plates. As no dimension is given and the other side is a fillet weld, it appears to be a flat fillet laid down on the joint, with no gap. Please confirm this is the design intent.			





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<b>T-1580</b>	<b>SSS - Stiffener Fouling 2" Plate at GL21G</b>	<b>Closed</b>	<b>CR</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 541 SK1: With the actual location of MFB11 relative to TR21 as shown, the PL2 1/2" x 14 x 2'-6 will foul the vertical stiffeners. Confirm it is acceptable to locate the PL 2 1/2" as shown and terminate the vertical stiffener plates 1" below MFB11 as shown. If not, please clarify detail 8/S1-3702 at Grid G/21.						<b>ANSWER:</b> See attached CD RFI # 541 SK1: With the actual location of MFB11 relative to TR21 as shown, the PL2 1/2" x 14 x 2'-6 will foul the vertical stiffeners. Confirm it is acceptable to locate the PL 2 1/2" as shown and terminate the vertical stiffener plates 1" below MFB11 as shown. If not, please clarify detail 8/S1-3702 at Grid G/21.
<b>T-1581</b>	<b>SSS - Inaccessible Welds at Corner GL Cast Nodes</b>	<b>Closed</b>	<b>CR</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/22/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 474 SK1 to SK6 for item 1 to 10: 1- 8) Several of the shop and field welds required at the ground level corner cast nodes are inaccessible once the cast node is set in position. Please see items 1 through 8 on SK1 through SK8 for clarification and provide direction. 9) Supply the welding for the north flange plates of the BU column to the node. 10) The welds required at this external corner create an unweldable condition, See SK5 & SK6 for clarification. Please provide direction.						<b>ANSWER:</b> See attached CD RFI # 474 SK1 to SK6 for item 1 to 10: 1- 8) Several of the shop and field welds required at the ground level corner cast nodes are inaccessible once the cast node is set in position. Please see items 1 through 8 on SK1 through SK8 for clarification and provide direction. 9) Supply the welding for the north flange plates of the BU column to the node. 10) The welds required at this external corner create an unweldable condition, See SK5 & SK6 for clarification. Please provide direction.
<b>T-1581.1</b>	<b>SSS - Inaccessible Welds at Corner GL Cast Nodes</b>	<b>Closed</b>	<b>CR</b>	<b>09/08/2014</b>	<b>09/18/2014</b>	<b>09/22/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> This is a follow up to RFI T-1581:  1-7) Confirm the backing bar may be left in place. 8)The "vertical stiffener" referenced in the response is the transfer girder web as per section 5B. This weld is inaccessible once the node is set in place. Confirm this section of 2" PJP can be omitted. 9) The weld detail requested was the vertical edge of the built-up column flange to node, not the column field weld						<b>ANSWER:</b> This is a follow up to RFI T-1581:  1-7) Confirm the backing bar may be left in place. 8)The "vertical stiffener" referenced in the response is the transfer girder web as per section 5B. This weld is inaccessible once the node is set in place. Confirm this section of 2" PJP can be omitted. 9) The weld detail requested was the vertical edge of the built-up column flange to node, not the column





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	indicated in item #4 as referenced in the response. Please provide weld detail or confirm weld is not required. 10a & b) Confirm access holes as per SK4 are acceptable.					field weld indicated in item #4 as referenced in the response. Please provide weld detail or confirm weld is not required. 10a & b) Confirm access holes as per SK4 are acceptable.
<b>T-1582</b>	<b>BGP - Geothermal and Temperature Probe Manifold Sleeve Detail</b>	<b>Open</b>	<b>01</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>  The CDSM wall has significant variation at the geothermal and temperature probe sleeve locations.  Please confirm the front flange and nelson stud at the manifold sleeve can be removed, as shown in the attached shop drawings. This would allow the sleeve to be cut to take the CDSM encroachment into consideration while still installing the sleeve flush to the face of the wall.						<b>ANSWER:</b>  The CDSM wall has significant variation at the geothermal and temperature probe sleeve locations.  Please confirm the front flange and nelson stud at the manifold sleeve can be removed, as shown in the attached shop drawings. This would allow the sleeve to be cut to take the CDSM encroachment into consideration while still installing the sleeve flush to the face of the wall.
<b>T-1582.1</b>	<b>BGP - Geothermal and Temperature Probe Manifold Sleeve</b>	<b>Closed</b>	<b>01</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/14/2014</b>
<b>From:</b> Webcor Construction LP Claude Titché						
<b>REQUEST:</b>  The CDSM wall has significant variation at the geothermal and temperature probe sleeve locations.  Please confirm the front flange and nelson stud at the manifold sleeve can be removed, as shown in the attached shop drawings. This would allow the sleeve to be cut to take the CDSM encroachment into consideration while still installing the sleeve flush to the face of the wall.  Note: The original sleeve shop drawings have also been included for comparison purposes.						<b>ANSWER:</b>  The CDSM wall has significant variation at the geothermal and temperature probe sleeve locations.  Please confirm the front flange and nelson stud at the manifold sleeve can be removed, as shown in the attached shop drawings. This would allow the sleeve to be cut to take the CDSM encroachment into consideration while still installing the sleeve flush to the face of the wall.  Note: The original sleeve shop drawings have also been included for comparison purposes.



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<b>T-1583</b>	<b>BGP - CB15 Added in ASI 121- Lower Concourse GL 7-8</b>	<b>Closed</b>	<b>01</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/04/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
SCCI received ASI 121 on 8/1/2014. Part of the change includes an added beam in Area 5 between GL7-8. Upon receipt of ASI 121 , the deck has been formed and rebar has been fully installed.			SCCI received ASI 121 on 8/1/2014. Part of the change includes an added beam in Area 5 between GL7-8. Upon receipt of ASI 121 , the deck has been formed and rebar has been fully installed.			
To accommodate the late beam addition, please confirm that CB-15 issued in ASI 121 on SI-2203 between Grids 7 and 8 near Grid D.8 may be constructed per the attached sketches in order to accommodate schedule and work in place.			To accommodate the late beam addition, please confirm that CB-15 issued in ASI 121 on SI-2203 between Grids 7 and 8 near Grid D.8 may be constructed per the attached sketches in order to accommodate schedule and work in place.			
<b>T-1584</b>	<b>SCS - Wall Construction Joint - Moment Frame Beam Conflict</b>	<b>Closed</b>	<b>01</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/18/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached diagram of a wall lift construction joint overlay on the ground floor plan. Per Specification Section 03 30 02 - 3.2, Foundation wall and ground floor construction joints shall align with the location of joint below in TG06 and will not impair the strength of the structure. A conflict occurs at the noted areas of the attached diagram, where the wall joint placed according to specification, will intersect at a moment frame beam. SCCI proposes to move wall construction joints in order to avoid conflicting with the moment frame beams.			See attached diagram of a wall lift construction joint overlay on the ground floor plan. Per Specification Section 03 30 02 - 3.2, Foundation wall and ground floor construction joints shall align with the location of joint below in TG06 and will not impair the strength of the structure. A conflict occurs at the noted areas of the attached diagram, where the wall joint placed according to specification, will intersect at a moment frame beam. SCCI proposes to move wall construction joints in order to avoid conflicting with the moment frame beams.			
Please confirm shifting construction joints to not align with the construction joints below is acceptable and provide permissible parameters to assign new construction joints for SCCI to reflect upon and modify the construction joint layout. A conceptual sketch is attached reflecting the proposed shift in vertical construction joint.			Please confirm shifting construction joints to not align with the construction joints below is acceptable and provide permissible parameters to assign new construction joints for SCCI to reflect upon and modify the construction joint layout. A conceptual sketch is attached reflecting the proposed shift in vertical construction joint.			
<b>T-1586</b>	<b>SSS - Telecom Vault - ToS Elevation</b>	<b>Closed</b>	<b>CR</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/13/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			



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	See attached CD RFI # 545 SK1: Please supply the top of slab elevation.					See attached CD RFI # 545 SK1: Please supply the top of slab elevation.
<b>T-1587</b>	<b>SSS - ST501 &amp; ST502 Missing Dimensions</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/22/2014</b>
	<b>REQUEST:</b> See attached CD RFI # 543 SK1 for items 1 to 3:  1) Confirm the set-back from edge of slab to center of beam is 6" on (3) sides noted or supply the missing dimensions.  2) Confirm the set-back from edge of slab to center of beam is 6" on (3) sides noted or supply the missing dimensions.  3) Confirm the 8" dimension offset to match the opening at Grid C or supply the missing dimension.					<b>ANSWER:</b> See attached CD RFI # 543 SK1 for items 1 to 3:  1) Confirm the set-back from edge of slab to center of beam is 6" on (3) sides noted or supply the missing dimensions.  2) Confirm the set-back from edge of slab to center of beam is 6" on (3) sides noted or supply the missing dimensions.  3) Confirm the 8" dimension offset to match the opening at Grid C or supply the missing dimension.
<b>T-1588</b>	<b>SSS - E510-E512 Framing Clarification at GL 22</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/22/2014</b>
	<b>REQUEST:</b> See attached CD RFI # 544 SK1: It appears a beam is missing on the east side of the dropped slab to support the low steel. If yes, supply the size and location of the beam. If no, supply a connection for the east end of the (3) low W24x68 beams.					<b>ANSWER:</b> See attached CD RFI # 544 SK1: It appears a beam is missing on the east side of the dropped slab to support the low steel. If yes, supply the size and location of the beam. If no, supply a connection for the east end of the (3) low W24x68 beams.
<b>T-1589</b>	<b>SSS - Missing Beam Locations at OCS Posts</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/15/2014</b>
	<b>REQUEST:</b> See attached CD RFI # 546 SK1: Supply the missing dimensions to locate the beams. A1-2307 does not supply the dimensions.					<b>ANSWER:</b> See attached CD RFI # 546 SK1: Supply the missing dimensions to locate the beams. A1-2307 does not supply the dimensions.



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<b>T-1590</b>	<b>SCS - Concrete Form Support Kicked To Temp Bridge</b>	<b>Closed</b>	<b>CR</b>	<b>08/04/2014</b>	<b>08/14/2014</b>	<b>08/09/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>A unique support system is necessary for the foundation wall under the temporary bridges (First, Fremont, Beale). Shimmick proposes that in these areas, the form work will kick to support brackets mounted to the underside of the bridges to provide better support. Without this, the form system will be a cantilever system supported by 1 row of anchors which will exert a significant force on the wall lift below.</p> <p>Please confirm it is acceptable for the fourth lift form system to be supported by the bridge underside.</p>			<p>A unique support system is necessary for the foundation wall under the temporary bridges (First, Fremont, Beale). Shimmick proposes that in these areas, the form work will kick to support brackets mounted to the underside of the bridges to provide better support. Without this, the form system will be a cantilever system supported by 1 row of anchors which will exert a significant force on the wall lift below.</p> <p>Please confirm it is acceptable for the fourth lift form system to be supported by the bridge underside.</p>			
<b>T-1591</b>	<b>SCS - Pendulum Bearing Test Results</b>	<b>Closed</b>	<b>01</b>	<b>08/05/2014</b>	<b>08/15/2014</b>	<b>08/15/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Per Specification Section 03 20 02-1.4-A-7, "test results for [pendulum] bearings with similar size" are to be submitted for approval. Per Mageba, they have test data on pendulum bearings (isolators) with similar sizes which they believe have been tested under more severe conditions than the ones required for this project. See attached email correspondence with Mageba for further explanation and more specific details in regards to their test and how the results apply to satisfy the requirements in the specifications. The referenced Annex documents in the email are also attached. Only a sample of the data has been included for preliminary approval; full test data will be submitted once this data is confirmed acceptable.</p> <p>Please confirm that the mentioned test reports could serve as evidence of the proper functioning of mageba pendulum bearings, as well as the fulfillment for the requirements in the specifications.</p>			<p>Per Specification Section 03 20 02-1.4-A-7, "test results for [pendulum] bearings with similar size" are to be submitted for approval. Per Mageba, they have test data on pendulum bearings (isolators) with similar sizes which they believe have been tested under more severe conditions than the ones required for this project. See attached email correspondence with Mageba for further explanation and more specific details in regards to their test and how the results apply to satisfy the requirements in the specifications. The referenced Annex documents in the email are also attached. Only a sample of the data has been included for preliminary approval; full test data will be submitted once this data is confirmed acceptable.</p> <p>Please confirm that the mentioned test reports could serve as evidence of the proper functioning of mageba pendulum bearings, as well as the fulfillment for the requirements in the specifications.</p>			



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<b>REQUEST:</b> Contract Drawings A1-2057 dimensions the removal steel corbels at 5' OC UON. Reference attached sketch. Please confirm it is acceptable to space the first and last corbels at 5'-1 ¼ from the north and south interior wall edges. This will also confirm a total of 33 removable corbels at the seismic joint.					
	<b>ANSWER:</b> Contract Drawings A1-2057 dimensions the removal steel corbels at 5' OC UON. Reference attached sketch. Please confirm it is acceptable to space the first and last corbels at 5'-1 ¼ from the north and south interior wall edges. This will also confirm a total of 33 removable corbels at the seismic joint.					
<b>T-1595</b>	<b>SSS - Ring Elevation Clarification in Light Tower</b>	<b>Closed</b>	<b>CR</b>	<b>08/11/2014</b>	<b>08/21/2014</b>	<b>08/13/2014</b>
	<b>From:</b> Webcor Construction LP Gregory Kemerer					
	<b>REQUEST:</b> See attached CD RFI # 550 SK1: Verify the Ring Elevation to be used is EL. 56'-7 3/8 to match the elevation shown on detail B/S1-6006 (SK2). Thornton Tomasetti's certified Tekla Geometry of the Light Tower also shows a ring EL. 56'-7 3/8.					
	<b>ANSWER:</b> See attached CD RFI # 550 SK1: Verify the Ring Elevation to be used is EL. 56'-7 3/8 to match the elevation shown on detail B/S1-6006 (SK2). Thornton Tomasetti's certified Tekla Geometry of the Light Tower also shows a ring EL. 56'-7 3/8.					
<b>T-1596</b>	<b>SSS - Light Tower Cast Node Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>08/11/2014</b>	<b>08/21/2014</b>	<b>08/13/2014</b>
	<b>From:</b> Webcor Construction LP Gregory Kemerer					
	<b>REQUEST:</b> See attached CD RFI # 551 SK1 for items 1 & 2: 1) It is not possible to use 9 3/4" at the noted locations as the edges of the lug plates will not completely land on the 3" thick round cap plate. Verify the 8 7/16" dimension is acceptable in lieu of 9 3/4" 2) Verify clouded dimensions.					
	<b>ANSWER:</b> See attached CD RFI # 551 SK1 for items 1 & 2: 1) It is not possible to use 9 3/4" at the noted locations as the edges of the lug plates will not completely land on the 3" thick round cap plate. Verify the 8 7/16" dimension is acceptable in lieu of 9 3/4" 2) Verify clouded dimensions.					



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
T-1597	SSS - Transfer Girder Top Flange Stiffeners at GL3	Closed	CR	08/11/2014	08/21/2014	09/16/2014
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
At either side of transfer girder TR3, the lower slab elevation requires the deck to be cut around each of the girder stiffeners compromising the rigidity of the deck, see attached SK1 for clarification.			At either side of transfer girder TR3, the lower slab elevation requires the deck to be cut around each of the girder stiffeners compromising the rigidity of the deck, see attached SK1 for clarification.			
Please confirm it is acceptable to shorten the short stiffeners under both of the center columns to 1" above the top flange of the deck.			Please confirm it is acceptable to shorten the short stiffeners under both of the center columns to 1" above the top flange of the deck.			



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-1598	<b>SSS - Supplemental Pin Testing</b>	Void	CR	08/11/2014	08/21/2014	
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Based on review of structural and referenced specification section it appears that Supplemental destructive testing of galvanized pins is not required after pins are galvanized per para 2.3.J. As per our previous QAQC meeting of June 17, 2014, it was requested by the TJPA representative that additional galvanized steel pin samples (ASTM A668 Class M) with associated nuts for each pin size be provided for destructive testing by TJPA representative testing lab and as follow:			Based on review of structural and referenced specification section it appears that Supplemental destructive testing of galvanized pins is not required after pins are galvanized per para 2.3.J. As per our previous QAQC meeting of June 17, 2014, it was requested by the TJPA representative that additional galvanized steel pin samples (ASTM A668 Class M) with associated nuts for each pin size be provided for destructive testing by TJPA representative testing lab and as follow:			
1. 7" dia. X 2'-1" (A1551 as shown on 1551 of Pin shop drawings)			1. 7" dia. X 2'-1" (A1551 as shown on 1551 of Pin shop drawings)			
2. 7" dia. X 1'-2 3/16" (A1553 as shown on 1553 of Pin shop drawings).			2. 7" dia. X 1'-2 3/16" (A1553 as shown on 1553 of Pin shop drawings).			
3. 7" dia. X 1'-6 7/8" (A1554 as shown on 1554 of Pin shop drawings).			3. 7" dia. X 1'-6 7/8" (A1554 as shown on 1554 of Pin shop drawings).			
4. 8" dia. X 2'-7" (A1555 as shown on 1555 of Pin shop drawings).			4. 8" dia. X 2'-7" (A1555 as shown on 1555 of Pin shop drawings).			
5. 6" dia. X 1'- 8 13/16" (A13001 as shown on 13001 of Pin shop drawings).			5. 6" dia. X 1'- 8 13/16" (A13001 as shown on 13001 of Pin shop drawings).			
Please provide the following:			Please provide the following:			
1. Provide testing criteria to the testing lab agency after pins have been galvanized and in addition to the specified forged steel pins ASTM 668 Class M testing requirements.			1. Provide testing criteria to the testing lab agency after pins have been galvanized and in addition to the specified forged steel pins ASTM 668 Class M testing requirements.			
2. Provide WOJV with a CR to supply and ship these additional pins and nuts to the testing lab.			2. Provide WOJV with a CR to supply and ship these additional pins and nuts to the testing lab.			
3. If additional samples of other pins on the project are required, please specify.			3. If additional samples of other pins on the project are required, please specify.			
4. BRB pins are not included in this RFI to be tested. Please confirm.			4. BRB pins are not included in this RFI to be tested. Please confirm.			





Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-1598.1	SSS - Supplemental Pin Testing	Closed	CR	08/12/2014	08/22/2014	08/22/2014
From: Webcor Construction LP		Gregory Kemerer				
REQUEST:		ANSWER:				
<p>Based on review of structural and referenced specification section it appears that Supplemental destructive testing of galvanized pins is not required after pins are galvanized per 05 10 00 para 2.3.J. As per our previous QAQC meeting of June 17, 2014, it was requested by the TJPA representative that additional galvanized steel pin samples (ASTM A668 Class M) with associated nuts for each pin size be provided for destructive testing by TJPA representative testing lab and as follow:</p> <p>1. 7" dia. X 2'-1" (A1551 as shown on 1551 of Pin shop drawings)</p> <p>2. 7" dia. X 1'-2 3/16" (A1553 as shown on 1553 of Pin shop drawings).</p> <p>3. 7" dia. X 1'-6 7/8" (A1554 as shown on 1554 of Pin shop drawings).</p> <p>4. 8" dia. X 2'-7" (A1555 as shown on 1555 of Pin shop drawings).</p> <p>5. 6" dia. X 1'- 8 13/16" (A13001 as shown on 13001 of Pin shop drawings).</p> <p>Please provide the following:</p> <p>1. Provide testing criteria to the testing lab agency after pins have been galvanized and in addition to the specified forged steel pins ASTM 668 Class M testing requirements.</p> <p>2. Provide WOJV with a CR to supply and ship these additional pins and nuts to the testing lab.</p> <p>3. If additional samples of other pins on the project are required, please specify.</p> <p>4. BRB pins are not included in this RFI to be tested. Please confirm.</p>		<p>Based on review of structural and referenced specification section it appears that Supplemental destructive testing of galvanized pins is not required after pins are galvanized per 05 10 00 para 2.3.J. As per our previous QAQC meeting of June 17, 2014, it was requested by the TJPA representative that additional galvanized steel pin samples (ASTM A668 Class M) with associated nuts for each pin size be provided for destructive testing by TJPA representative testing lab and as follow:</p> <p>1. 7" dia. X 2'-1" (A1551 as shown on 1551 of Pin shop drawings)</p> <p>2. 7" dia. X 1'-2 3/16" (A1553 as shown on 1553 of Pin shop drawings).</p> <p>3. 7" dia. X 1'-6 7/8" (A1554 as shown on 1554 of Pin shop drawings).</p> <p>4. 8" dia. X 2'-7" (A1555 as shown on 1555 of Pin shop drawings).</p> <p>5. 6" dia. X 1'- 8 13/16" (A13001 as shown on 13001 of Pin shop drawings).</p> <p>Please provide the following:</p> <p>1. Provide testing criteria to the testing lab agency after pins have been galvanized and in addition to the specified forged steel pins ASTM 668 Class M testing requirements.</p> <p>2. Provide WOJV with a CR to supply and ship these additional pins and nuts to the testing lab.</p> <p>3. If additional samples of other pins on the project are required, please specify.</p> <p>4. BRB pins are not included in this RFI to be tested. Please confirm.</p>				
T-1599	SSS - Top of Slab at Roof Park Level GL F	Closed	CR	08/11/2014	08/21/2014	08/15/2014
From: Webcor Construction LP		Gregory Kemerer				
REQUEST:		ANSWER:				
<p>See attached CD RFI # 549 SK1:</p> <p>Confirm the note should read 80'-7 1/2 to match S1-2605.</p>		<p>See attached CD RFI # 549 SK1:</p> <p>Confirm the note should read 80'-7 1/2 to match S1-2605.</p>				



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<b>T-1600</b>	<b>SCS - Concrete Form Support Kicked to Temp Bridge</b>  <b>From:</b> Webcor Construction LP                      Claude Titcher	<b>Closed</b>	<b>01</b>	<b>08/11/2014</b>	<b>08/21/2014</b>	
<b>REQUEST:</b>  A unique support system is necessary for the foundation wall under the temporary bridges (First, Fremont, Beale). Shimmick proposes that in these areas, the form work will kick to support brackets mounted to the underside of the bridges to provide better support. Without this, the form system will be a cantilever system supported by 1 row of anchors which will exert a significant force on the wall lift below.  Please confirm it is acceptable for the fourth lift form system to be supported by the bridge underside.		<b>ANSWER:</b>  A unique support system is necessary for the foundation wall under the temporary bridges (First, Fremont, Beale). Shimmick proposes that in these areas, the form work will kick to support brackets mounted to the underside of the bridges to provide better support. Without this, the form system will be a cantilever system supported by 1 row of anchors which will exert a significant force on the wall lift below.  Please confirm it is acceptable for the fourth lift form system to be supported by the bridge underside.				
<b>T-1601</b>	<b>SCS - Concrete Slabs Between Lower Concourse and Ground Level</b>  <b>From:</b> Shimmick Construction Company, Inc. Henry Chiang	<b>Closed</b>	<b>01</b>	<b>08/11/2014</b>	<b>08/21/2014</b>	<b>08/11/2014</b>
<b>REQUEST:</b>  See Detail 2/S1-2252 from ASI 117. On grid Line H - 23 there is a concrete slab between the Lower Concourse and Ground Level (elevation 1'7"). The Slab is connected to a concrete partition wall on the south. On the north of the slab there is a concrete wall starting at the slab and connecting at the Ground Level Slab. Detail 2/S1-2252 was revised to add detail 2/S1-3504 in ASI 117. 1 . Is TG07.2 to add this scope of work and provide a price?  See Detail 1/S1-2252. On grid Line B - 23 there is a concrete slab between the Lower Concourse and Ground Level (elevation 1'7"). The Slab is connected to a CMU wall on the north side. On the south of the slab there is a CMU wall starting at the slab elevation and ending at the Ground Level Slab. 2. Please confirm this slab does not belong to the TG07 .2 Package. a. If this slab does belong to the TG07.2 Package, please advice on the sequence of work with the CMU wall and steel.		<b>ANSWER:</b>  See Detail 2/S1-2252 from ASI 117. On grid Line H - 23 there is a concrete slab between the Lower Concourse and Ground Level (elevation 1'7"). The Slab is connected to a concrete partition wall on the south. On the north of the slab there is a concrete wall starting at the slab and connecting at the Ground Level Slab. Detail 2/S1-2252 was revised to add detail 2/S1-3504 in ASI 117. 1 . Is TG07.2 to add this scope of work and provide a price?  See Detail 1/S1-2252. On grid Line B - 23 there is a concrete slab between the Lower Concourse and Ground Level (elevation 1'7"). The Slab is connected to a CMU wall on the north side. On the south of the slab there is a CMU wall starting at the slab elevation and ending at the Ground Level Slab. 2. Please confirm this slab does not belong to the TG07 .2 Package. a. If this slab does belong to the TG07.2 Package, please advice on the sequence of work with the CMU wall and steel.				
<b>T-1602</b>	<b>SCS - Gas Line Lateral Pipe Size at GL 7.3/A</b>	<b>Closed</b>	<b>CR</b>	<b>08/12/2014</b>	<b>08/22/2014</b>	<b>08/22/2014</b>







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<b>T-1604</b>	<b>SSS - Light Tower Cast Node Detail LC-702 Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>08/12/2014</b>	<b>08/22/2014</b>	<b>08/26/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 559 SK1 for items 1 to 3:			See attached CD RFI # 559 SK1 for items 1 to 3:			
1) Provide welding detail for the contact plate to lug plates.			1) Provide welding detail for the contact plate to lug plates.			
2) Verify dimensions.			2) Verify dimensions.			
3) Confirm the dimensions in item 2 can be used at similar conditions e.g. 3&5/S1-6005, 4&6/S1-6006.			3) Confirm the dimensions in item 2 can be used at similar conditions e.g. 3&5/S1-6005, 4&6/S1-6006.			
<b>T-1605</b>	<b>SSS - Light Tower Cast Node LC-302W Detail Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>08/12/2014</b>	<b>08/22/2014</b>	<b>08/22/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 560 SK1 for items 1 & 2:			See attached CD RFI # 560 SK1 for items 1 & 2:			
1) The CJP weld cannot be applied at the sharp edges of the 2 3/4" thick (2) plates and the 3" thick connection plate. Verify it is acceptable to shape the edges of plates as shown.			1) The CJP weld cannot be applied at the sharp edges of the 2 3/4" thick (2) plates and the 3" thick connection plate. Verify it is acceptable to shape the edges of plates as shown.			
2) Confirm the solution in item 1 can be applied to similar conditions e.g. 4/S1-6006 and 3&5/S1-6005.			2) Confirm the solution in item 1 can be applied to similar conditions e.g. 4/S1-6006 and 3&5/S1-6005.			
<b>T-1606</b>	<b>SSS - Type Drag Connection Geometry at Bus Level Cast Node 29A-01 and 29B-01</b>	<b>Closed</b>	<b>CR</b>	<b>08/12/2014</b>	<b>08/22/2014</b>	<b>08/19/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please refer to attached SK RFI 744 SK01.			Please refer to attached SK RFI 744 SK01.			
The shear plates at drag connection type 1-A at cast node 29B-01 and 29A-01 land on the edge of the welding pad. This condition does not make allowance for as-cast tolerances on the welding pad edge and may impact our ability to achieve effective weld size. Please advise.			The shear plates at drag connection type 1-A at cast node 29B-01 and 29A-01 land on the edge of the welding pad. This condition does not make allowance for as-cast tolerances on the welding pad edge and may impact our ability to achieve effective weld size. Please advise.			
Skanska acknowledges the response to SK RFI 004.1 (T-0738) and 004.2 (T-0738.1). Following these RFIs, Skanska increased shear plate spacing to give a 1/8" gap on the inside of each shear plate for erection clearance. Even had this gap not been added, Skanska would still flag this issue for			Skanska acknowledges the response to SK RFI 004.1 (T-0738) and 004.2 (T-0738.1). Following these RFIs, Skanska increased shear plate spacing to give a 1/8" gap on the inside of each shear plate for erection clearance. Even had this			



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<div>designers review as 1/8" still does not allow for as-cast tolerances at these locations.</div> <div>gap not been added, Skanska would still flag this issue for designers review as 1/8" still does not allow for as-cast tolerances at these locations.</div>						
T-1607	SCS - Fill Material at ST201	Closed	CR	08/13/2014	08/23/2014	08/22/2014
<div>From: Webcor Construction LP</div> <div>Claude Titcher</div>						
REQUEST:			ANSWER:			
Drawing Sheets AI-7101 and A1-8714 shows backfill material between the stair tower grade beams.			Drawing Sheets AI-7101 and A1-8714 shows backfill material between the stair tower grade beams.			
1) SCCI suggests to neat cut the stair tower excavation to the bottom of the grade beams at ST201 in order to eliminate the need to form grade beams and backfill. Please confirm this is acceptable.			1) SCCI suggests to neat cut the stair tower excavation to the bottom of the grade beams at ST201 in order to eliminate the need to form grade beams and backfill. Please confirm this is acceptable.			
2) What fill does the hatching in the drawings indicate? Please confirm that the hatching refers to acceptable material in Specification 31 00 00 (Earthwork) and do not have special requirements.			2) What fill does the hatching in the drawings indicate? Please confirm that the hatching refers to acceptable material in Specification 31 00 00 (Earthwork) and do not have special requirements.			



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<b>T-1608</b>	<b>SSS - Light Tower Pin Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>08/13/2014</b>	<b>08/23/2014</b>	<b>08/27/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 558 SK1 for items 1 to 11: 1) Confirm dimensions are correct. 2) Confirm dimensions are correct. 3) Confirm dimensions indicated can be used on all pin diameters. 4) Verify material grade for the cover plate is A572 Gr. 50. 5) 3,4&6/S1-6006 shows 3 1/8" hole size for 3" dia pin. Please confirm. 6) 5&7/S1-6005 shows 7 3/16" hole size for 7" dia pin. Please confirm. 7) Verify 5/8" &Oslash; ASTM bolt can be used for cap screw. Please provide specification. 8) Provide thread pitch to use for the threaded hole. 9) Clarify the intent of the noted dimensions. 10) Confirm it is acceptable to use 11/16" thick plate. 11) Confirm hole diameters are correct.			See attached CD RFI # 558 SK1 for items 1 to 11: 1) Confirm dimensions are correct. 2) Confirm dimensions are correct. 3) Confirm dimensions indicated can be used on all pin diameters. 4) Verify material grade for the cover plate is A572 Gr. 50. 5) 3,4&6/S1-6006 shows 3 1/8" hole size for 3" dia pin. Please confirm. 6) 5&7/S1-6005 shows 7 3/16" hole size for 7" dia pin. Please confirm. 7) Verify 5/8" &Oslash; ASTM bolt can be used for cap screw. Please provide specification. 8) Provide thread pitch to use for the threaded hole. 9) Clarify the intent of the noted dimensions. 10) Confirm it is acceptable to use 11/16" thick plate. 11) Confirm hole diameters are correct.			
<b>T-1609</b>	<b>SCS - Loads Imposed on Rebrace Struts by Wall Formwork</b>	<b>Void</b>	<b>01</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached Brierley Associates calculations.  Regarding imposed loads on rebracing struts due to fourth lift formwork, please confirm that the additional calculated loads on rebracing struts are acceptable.			See attached Brierley Associates calculations.  Regarding imposed loads on rebracing struts due to fourth lift formwork, please confirm that the additional calculated loads on rebracing struts are acceptable.			
<b>T-1610</b>	<b>SSS - W14 Connection at Perimeter Framing GL20.1</b>	<b>Closed</b>	<b>CR</b>	<b>08/14/2014</b>	<b>08/24/2014</b>	<b>09/04/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 567 SK1 & SK2 for items 1, 2 & 3:  1) Work with RFI T-1573 and supply the clouded dimensions. 2) The connection per 1/S1-5028 does not work for the W14x68. The seat angles do not work due to the 8" MIN. criteria and there is insufficient room to fit the angles with (2) bolts. Please advise. 3) Confirm a W14x68 beam is required at the step in slab			See attached CD RFI # 567 SK1 & SK2 for items 1, 2 & 3: 1) Work with RFI T-1573 and supply the clouded dimensions. 2) The connection per 1/S1-5028 does not work for the W14x68. The seat angles do not work due to the 8" MIN. criteria and there is insufficient room to fit the angles with (2) bolts. Please advise. 3) Confirm a W14x68 beam is required at the step in			





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	or provide a deck support detail.					slab or provide a deck support detail.
	Note: the same condition occurs on the north side at Grid 'C'					Note: the same condition occurs on the north side at Grid 'C'
T-1611	SSS - Mis-Drilled Holes in Transfer Girder A628	Closed	CR	08/14/2014	08/24/2014	08/28/2014
From: Webcor Construction LPGregory Kemerer						
REQUEST:			ANSWER:			
Skanska's transfer girder fabricator, TMF, has drilled girder A628's top flange hole pattern 5/16" north of the designed location. Please confirm it is acceptable to offset the hole pattern and machined grooves on the column base plate to account for the offset on the girder. This will allow the column to be positioned at the required location while accommodating the offset holes. The girder short stiffeners will also need to be shifted 5/16" to allow bolt clearance. See attached 628AC & 628AB for as-built hole locations, also see attached p4284 for proposed offset on column base plate.			Skanska's transfer girder fabricator, TMF, has drilled girder A628's top flange hole pattern 5/16" north of the designed location. Please confirm it is acceptable to offset the hole pattern and machined grooves on the column base plate to account for the offset on the girder. This will allow the column to be positioned at the required location while accommodating the offset holes. The girder short stiffeners will also need to be shifted 5/16" to allow bolt clearance. See attached 628AC & 628AB for as-built hole locations, also see attached p4284 for proposed offset on column base plate.			
T-1612	SSS - Transfer Girder Stud Clarifications at GL6G	Closed	CR	08/15/2014	08/25/2014	08/15/2014
From: Webcor Construction LPGregory Kemerer						
REQUEST:			ANSWER:			
See attached CD RFI # 568 SK1 & SK2 for items 1 & 2: 1) Please clarify what is meant by "SIM". 2) Please supply the missing stud information.			See attached CD RFI # 568 SK1 & SK2 for items 1 & 2: 1) Please clarify what is meant by "SIM". 2) Please supply the missing stud information.			



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T-1613	SSS - Deck Support at TR3 Grid C & G	Closed	CR	08/15/2014	08/25/2014	08/27/2014
<div><div>From: Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
REQUEST:			ANSWER:			
<p>At grid C3 the BU perimeter beam butts into the transfer girder TR3. The deck support angle in the web of the BU beam stops back 4' 9-5/16" from the center of the girder. This results in a region of about 4'-4" without deck support at an end bearing.</p> <p>Design drawings do not provide a specific detail for this condition. Deck support at column detail 1/S1-5001 should not be used as this detail typically only supports one or two flutes of deck. The condition described above has 4 ½ unsupported flutes in an end bearing condition with 10" of nominal weight concrete. This is too much for a 12 gauge angle. Please advise.</p>			<p>At grid C3 the BU perimeter beam butts into the transfer girder TR3. The deck support angle in the web of the BU beam stops back 4' 9-5/16" from the center of the girder. This results in a region of about 4'-4" without deck support at an end bearing.</p> <p>Design drawings do not provide a specific detail for this condition. Deck support at column detail 1/S1-5001 should not be used as this detail typically only supports one or two flutes of deck. The condition described above has 4 ½ unsupported flutes in an end bearing condition with 10" of nominal weight concrete. This is too much for a 12 gauge angle. Please advise.</p>			
T-1614	BGP - Gridline L/X Moment Frame Beam Bottom Reinforcing	Closed	CR	08/15/2014	08/25/2014	08/22/2014
<div><div>From: Webcor Construction LP</div><div>Claude Titcher</div></div>						
REQUEST:			ANSWER:			
<p>Per conversations with the SEOR, for MFB-X near Grid L, please confirm that it is acceptable to place the southern MFB bottom bars with the heads on the column side and hooks on the wall side provided that matching hooks are spliced onto all of the MFB bottom bars so oriented. Please also confirm that skin reinforcing is required to wrap around both exposed faces of the MFB below the reduced MFB section.</p>			<p>Per conversations with the SEOR, for MFB-X near Grid L, please confirm that it is acceptable to place the southern MFB bottom bars with the heads on the column side and hooks on the wall side provided that matching hooks are spliced onto all of the MFB bottom bars so oriented. Please also confirm that skin reinforcing is required to wrap around both exposed faces of the MFB below the reduced MFB section.</p>			
T-1615	BGP - Gridline 4.5 Elevator Pit Top Layer Hook Orientation	Closed	01	08/15/2014	08/25/2014	08/22/2014
<div><div>From: Webcor Construction LP</div><div>Claude Titcher</div></div>						
REQUEST:			ANSWER:			
<p>For the Lower Concourse suspended elevator pit between Grids 4 and 5 near Grid E, please confirm that it is acceptable to install the top layer of elevator slab reinforcing with the hooks oriented up provided that the</p>			<p>For the Lower Concourse suspended elevator pit between Grids 4 and 5 near Grid E, please confirm that it is acceptable to install the top layer of elevator slab reinforcing with the hooks oriented up provided</p>			



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	hooks positively engage the exterior wall horizontal reinforcing or a nosing bar of equivalent diameter.					that the hooks positively engage the exterior wall horizontal reinforcing or a nosing bar of equivalent diameter.
<b>T-1616</b>	<b>SSS - Drag Beam Web Stiffener Plate Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>08/18/2014</b>	<b>08/28/2014</b>	<b>09/16/2014</b>
	<b>From:</b> Webcor Construction LP Gregory Kemerer					
	<b>REQUEST:</b> See attached CD RFI # 572 SK1: Confirm it is acceptable to end the web stiffener plate as shown to avoid fouling the shear plate for the beam connection.					<b>ANSWER:</b> See attached CD RFI # 572 SK1: Confirm it is acceptable to end the web stiffener plate as shown to avoid fouling the shear plate for the beam connection.
<b>T-1617</b>	<b>SSS - Drag Beam Connection Symbol Clarifications GL 20</b>	<b>Closed</b>	<b>CR</b>	<b>08/18/2014</b>	<b>08/28/2014</b>	<b>09/04/2014</b>
	<b>From:</b> Webcor Construction LP Gregory Kemerer					
	<b>REQUEST:</b> See attached CD RFI # 570 SK1 for items 1 to 3: 1) Confirm the kicker brace symbol is not required as the beams are the same size. 2) Confirm it is the intent to show the symbol indicating shear plate connections. 3) Confirm the kicker brace symbol is not required as the beams are the same size					<b>ANSWER:</b> See attached CD RFI # 570 SK1 for items 1 to 3: 1) Confirm the kicker brace symbol is not required as the beams are the same size. 2) Confirm it is the intent to show the symbol indicating shear plate connections. 3) Confirm the kicker brace symbol is not required as the beams are the same size
<b>T-1618</b>	<b>SSS - E610 &amp; E611 Dimension Discrepancies at Ground Level</b>	<b>Closed</b>	<b>CR</b>	<b>08/18/2014</b>	<b>08/28/2014</b>	<b>09/16/2014</b>
	<b>From:</b> Webcor Construction LP Gregory Kemerer					
	<b>REQUEST:</b> See attached CD RFI # 569 SK1: Please clarify the discrepancy in dimensions for the escalator pit. Supply/confirm the clouded dimensions if A1-2866 is correct.					<b>ANSWER:</b> See attached CD RFI # 569 SK1: Please clarify the discrepancy in dimensions for the escalator pit. Supply/confirm the clouded dimensions if A1-2866 is correct.
<b>T-1619</b>	<b>SSS - E510 &amp; E511 Edge Plate Support Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>08/18/2014</b>	<b>08/28/2014</b>	<b>08/27/2014</b>
	<b>From:</b> Webcor Construction LP Gregory Kemerer					





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<b>T-1622</b>	<b>BGP - Top of Vertical Wall Interface with Ground Level MFB</b>	<b>Closed</b>	<b>01</b>	<b>08/18/2014</b>	<b>08/28/2014</b>	<b>08/22/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
The current design indicates the top of the # 11 vertical wall reinforcing terminating with a terminator maintaining 2" concrete clear cover above the bar as seen in Section 1 on S-3201 .		The current design indicates the top of the # 11 vertical wall reinforcing terminating with a terminator maintaining 2" concrete clear cover above the bar as seen in Section 1 on S-3201 .				
A # 11 555 t-head is 11/ 16" thick and a #11 Lenton Terminator is 11/16" thick. The Ground Moment Frame Beams appear in Section 5 of S1-3600. The concrete clear cover indicated over the stirrup is 2 3/4".		A # 11 555 t-head is 11/ 16" thick and a #11 Lenton Terminator is 11/16" thick. The Ground Moment Frame Beams appear in Section 5 of S1-3600. The concrete clear cover indicated over the stirrup is 2 3/4".				
Due to the designed concrete clear cover called for, the thicknesses of the different terminator products and the allowable tolerance in fabrication and placement per ACI a conflict with the two interfacing componets is expected to occur at the top bars of the MFB.		Due to the designed concrete clear cover called for, the thicknesses of the different terminator products and the allowable tolerance in fabrication and placement per ACI a conflict with the two interfacing componets is expected to occur at the top bars of the MFB.				
Please confirm if the tenninator on the vertical bar can terminate below the ground level MFB top bars to avoid the expected conflict similiar to that allowed between the column and concourse MFB as indicated within RFI T-0917.		Please confirm if the tenninator on the vertical bar can terminate below the ground level MFB top bars to avoid the expected conflict similiar to that allowed between the column and concourse MFB as indicated within RFI T-0917.				
Should this proposed solution not be acceptable please provide direction how to proceed.		Should this proposed solution not be acceptable please provide direction how to proceed.				
<b>T-1623</b>	<b>SSS - ST601B Missing Beam Location</b>	<b>Closed</b>	<b>CR</b>	<b>08/18/2014</b>	<b>08/28/2014</b>	<b>08/22/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached CD RFI # 547 SK1: Supply the missing dimension (not shown on Arch. drawings).		See attached CD RFI # 547 SK1: Supply the missing dimension (not shown on Arch. drawings).				
<b>T-1624</b>	<b>BGP - CMU Wall Dimensions Deck 220 GL20-H</b>	<b>Closed</b>	<b>01</b>	<b>08/18/2014</b>	<b>08/28/2014</b>	<b>08/20/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please provide dimensions for CMU walls near Gridline 20-H. See attached marked-up contract drawings for the		Please provide dimensions for CMU walls near Gridline 20-H. See attached marked-up contract				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	dimensions needed.				drawings for the dimensions needed.	
<b>T-1625</b>	<b>SCS - Embedded Column Inner Vertical Bars at Foundation Walls</b>	<b>Closed</b>	<b>01</b>	<b>08/19/2014</b>	<b>08/29/2014</b>	<b>08/22/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> See attached marked up contract drawings.  The TG0600 rebar subcontractor is using HRC 420 friction welded standard couplers at the top end of the 3rd lift foundation walls and embedded columns C21, C33, C34 vertical bars. The inner set of embedded column vertical bars are indicated in sections as having a 90 degree SH tail at the top of bar. It isn't possible to install the bar with the 90 tail into the 3rd lift HRC 420 standard coupler as the tail of the bar needs to spin circular to engage the threads into the coupler with the final tail orientation dictated by torquing of the bar into the coupler. A possible solution to this issue is to utilize the HRC 555 Forged head in lieu of the 90 degree tail. The top of the inner vertical bar 555 head could be held 4" below the top of the outer vertical bar 555 head for concrete encapsulation.  Please confirm that the proposed solution is acceptable for the typical embedded column in foundation walls.						
					<b>ANSWER:</b> See attached marked up contract drawings.  The TG0600 rebar subcontractor is using HRC 420 friction welded standard couplers at the top end of the 3rd lift foundation walls and embedded columns C21, C33, C34 vertical bars. The inner set of embedded column vertical bars are indicated in sections as having a 90 degree SH tail at the top of bar. It isn't possible to install the bar with the 90 tail into the 3rd lift HRC 420 standard coupler as the tail of the bar needs to spin circular to engage the threads into the coupler with the final tail orientation dictated by torquing of the bar into the coupler. A possible solution to this issue is to utilize the HRC 555 Forged head in lieu of the 90 degree tail. The top of the inner vertical bar 555 head could be held 4" below the top of the outer vertical bar 555 head for concrete encapsulation.  Please confirm that the proposed solution is acceptable for the typical embedded column in foundation walls.	
<b>T-1626</b>	<b>BGP - Column 101E and 101F Concrete Elevation</b>	<b>Closed</b>	<b>01</b>	<b>08/20/2014</b>	<b>08/30/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Column 101E and 101F was poured 1 ft below shoring elevation.  Is it acceptable to use concourse slab concrete mix 1558218 to pour back the remaining top foot concurrently with concourse deck D201?						
					<b>ANSWER:</b> Column 101E and 101F was poured 1 ft below shoring elevation.  Is it acceptable to use concourse slab concrete mix 1558218 to pour back the remaining top foot concurrently with concourse deck D201?	



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<b>T-1627</b>	<b>SCS - Bridge Blockout CIDH Piles</b>	<b>Closed</b>	<b>01</b>	<b>08/21/2014</b>	<b>08/31/2014</b>	<b>09/05/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Please provide details for blockout(s) at the concrete roadway section as shown in the attached sketch. A Blockout will be necessary to allow the CIDH pier(s) to be removed from the ground level, pulled up through the deck, after the concrete bridge(s) section has reached design strength. Once the CIDH piers have been removed the circular section will need to have rebar installed, concrete poured back, and waterproofing repaired prior to receiving infill pursuant to contract documents; 10 sets of CIDH piers, per temporary structure, will require Blockouts at First, Fremont, and Beal street.						<b>ANSWER:</b> Please provide details for blockout(s) at the concrete roadway section as shown in the attached sketch. A Blockout will be necessary to allow the CIDH pier(s) to be removed from the ground level, pulled up through the deck, after the concrete bridge(s) section has reached design strength. Once the CIDH piers have been removed the circular section will need to have rebar installed, concrete poured back, and waterproofing repaired prior to receiving infill pursuant to contract documents; 10 sets of CIDH piers, per temporary structure, will require Blockouts at First, Fremont, and Beal street.
<b>T-1628</b>	<b>SSS - Weld Columns to TG-33.2</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/24/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> See SK-1 attached 1) Ref.: 3/S1-4357 Confirm no weld at 1-inch plate to web as shown 2) Ref.: 3/S1-4357 Confirm no weld at 2-inch plate to TG-33.2 inside of pipe column as shown 3) Ref.: 3/S1-4357 Cannot back gouge weld as shown - access is not provided please resolve 4) Ref.: 4/S1-4357 Clarify 5/16 fillet weld shown - provide complete extents for all-around weld. No weld show for WF flange to TG-33.2 shown please confirm 5) Ref. S1-4357 Suggest use of base plate to resolve weld issues						<b>ANSWER:</b> See SK-1 attached 1) Ref.: 3/S1-4357 Confirm no weld at 1-inch plate to web as shown 2) Ref.: 3/S1-4357 Confirm no weld at 2-inch plate to TG-33.2 inside of pipe column as shown 3) Ref.: 3/S1-4357 Cannot back gouge weld as shown - access is not provided please resolve 4) Ref.: 4/S1-4357 Clarify 5/16 fillet weld shown - provide complete extents for all-around weld. No weld show for WF flange to TG-33.2 shown please confirm  5) Ref. S1-4357 Suggest use of base plate to resolve weld issues
<b>T-1629</b>	<b>SSS - Shop Prime Coat Thickness for Carbozinc 859</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/08/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> Specification 05 10 00 3.2.P.1.b.1 requires a primer of DFT 2.5 to 3.4 mils. The product data sheet for the approved Carbozinc 859 specifies a DFT of 3-5 mils. OIW coating instruction CI-2770-14-04 notes the 3-5 mils as recommended by the coating manufacturer.						<b>ANSWER:</b> Specification 05 10 00 3.2.P.1.b.1 requires a primer of DFT 2.5 to 3.4 mils. The product data sheet for the approved Carbozinc 859 specifies a DFT of 3-5 mils. OIW coating instruction CI-2770-14-04 notes the 3-5 mils as recommended by the coating manufacturer.



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	<p>Please confirm that a prime coat of 3-5 mils of Carbozinc 859 is acceptable.</p>					<p>Please confirm that a prime coat of 3-5 mils of Carbozinc 859 is acceptable.</p>
<b>T-1630</b>	<b>SSS - ST201 Weld Details for Access at W14x311</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/08/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p>					
	<p><b>REQUEST:</b></p> <p>For erectability and weld access, the following is proposed: stiffener plates will be shop welded while the HSS will be field welded (as far back as allowable with only a 2-5/16" gap). Please confirm sketch is acceptable; otherwise, provide alternative.</p>					<p><b>ANSWER:</b></p> <p>For erectability and weld access, the following is proposed: stiffener plates will be shop welded while the HSS will be field welded (as far back as allowable with only a 2-5/16" gap). Please confirm sketch is acceptable; otherwise, provide alternative.</p>
<b>T-1631</b>	<b>SCS - Elevation at Transformer Vaults Room</b>	<b>Closed</b>	<b>01</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>08/27/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Claude Titcher</p>					
	<p><b>REQUEST:</b></p> <p>On Sheet AI-2203, on the slab around the transformer vaults room, grid lines 12-C, the elevation is indicated as TOFF -7'-9". On Plan 1/A1-3002, there is an elevation point corresponding to the same slab at grid lines 12.1-B.5 that indicates an elevation of TOC -7'-9". On the detail A/AI-9236 same point has an TOC elevation of -8'-2". Please confirm that elevation -7-9" on Detail 1/A1-3002 corresponds to TOFF instead of TOC.</p>					<p><b>ANSWER:</b></p> <p>On Sheet AI-2203, on the slab around the transformer vaults room, grid lines 12-C, the elevation is indicated as TOFF -7'-9". On Plan 1/A1-3002, there is an elevation point corresponding to the same slab at grid lines 12.1-B.5 that indicates an elevation of TOC -7'-9". On the detail A/AI-9236 same point has an TOC elevation of -8'-2". Please confirm that elevation -7-9" on Detail 1/A1-3002 corresponds to TOFF instead of TOC.</p>



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<b>T-1632</b>	<b>SSS - Ground Level Kicker Brace Detail Clarification at GL 20</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/08/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 562 SK1: The kicker braces will extend thru the lower deck but still remain within the slab as shown. Confirm that is acceptable or supply a detail showing the design intent. Note: the condition is the same at Grid 'C'.						<b>ANSWER:</b> See attached CD RFI # 562 SK1: The kicker braces will extend thru the lower deck but still remain within the slab as shown. Confirm that is acceptable or supply a detail showing the design intent. Note: the condition is the same at Grid 'C'.
<b>T-1633</b>	<b>SSS - Shear Plate Connection Clarification at GL 20.1</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/08/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 574 SK1: Confirm it is acceptable to connect the W12x14 to the W16x26 with a shear plate per 1/S1-5011 as the double angle connection will extend past the edge of slab 4" north of the W12. The same condition occurs on the symmetrical framing at Grid D.4.						<b>ANSWER:</b> See attached CD RFI # 574 SK1: Confirm it is acceptable to connect the W12x14 to the W16x26 with a shear plate per 1/S1-5011 as the double angle connection will extend past the edge of slab 4" north of the W12. The same condition occurs on the symmetrical framing at Grid D.4.
<b>T-1634</b>	<b>SSS - Crash Rail Post Hole Clarification GL 21-22</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/16/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 576 SK1: This hole for the crash rail post per S1-8000 cannot be supplied as indicated. Confirm it may be omitted or supply a solution.						<b>ANSWER:</b> See attached CD RFI # 576 SK1: This hole for the crash rail post per S1-8000 cannot be supplied as indicated. Confirm it may be omitted or supply a solution.
<b>T-1635</b>	<b>SSS - Exit Mezzanine Framing and Connection Clarifications at South Exit</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/08/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 554 SK1 to SK3 for items 1 to 14: 1) Supply missing dimensions. 2) Confirm dimensions. 3) Supply location for the MC4. 4) Confirm the vertical hanger is interrupted at the upper beam. Please clarify. 5) Clarify where the MC4x13.8 is located and supply a						<b>ANSWER:</b> See attached CD RFI # 554 SK1 to SK3 for items 1 to 14: 1) Supply missing dimensions. 2) Confirm dimensions. 3) Supply location for the MC4. 4) Confirm the vertical hanger is interrupted at the upper beam. Please clarify.





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	connection to the W12 & CMU wall. 6) Supply missing dimension. 7) Connection per 9/S1-5028 will not work at the noted location. Supply an alternate detail. 8) Confirm off-center connection per 9/S1-5028 is acceptable or supply an alternate detail. 9) Supply the elevation for the W12x40 at the landing and depending on the elevation, supply a connection detail at each end. 10) Supply missing dimension. 11) Supply length of weld. 12) Supply a connection detail for brace to hanger/beam. 13) Confirm dimension for plate orientation. 14) Confirm detail 9/S1-5028 also applies to the top of the hangers. 15) Confirm a 2-sided fillet is acceptable as the welding access of the end of the brace is not sufficient.					
	5) Clarify where the MC4x13.8 is located and supply a connection to the W12 & CMU wall. 6) Supply missing dimension. 7) Connection per 9/S1-5028 will not work at the noted location. Supply an alternate detail. 8) Confirm off-center connection per 9/S1-5028 is acceptable or supply an alternate detail. 9) Supply the elevation for the W12x40 at the landing and depending on the elevation, supply a connection detail at each end. 10) Supply missing dimension. 11) Supply length of weld. 12) Supply a connection detail for brace to hanger/beam. 13) Confirm dimension for plate orientation. 14) Confirm detail 9/S1-5028 also applies to the top of the hangers. 15) Confirm a 2-sided fillet is acceptable as the welding access of the end of the brace is not sufficient.					
T-1635.1	SSS - Exit Mezzanine Framing and Connection Clarifications at South Exit	Closed	CR	09/26/2014	10/06/2014	10/09/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: See attached CD RFI # 554.1 SK1: The W12x40 is currently not supported on the right end. Supply a connection for the hanger at the Ground Level or supply a connection detail to the 2-L6x6x1/2 hanger.						
ANSWER: See attached CD RFI # 554.1 SK1: The W12x40 is currently not supported on the right end. Supply a connection for the hanger at the Ground Level or supply a connection detail to the 2-L6x6x1/2 hanger.						



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<b>T-1636</b>	<b>SSS - Exit Mezzanine Framing and Connection Clarifications at North Exit</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/08/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 555 SK1 for items 1 to 9: 1) Supply/confirm all clouded dimensions (9 clouds total). 2) Verify dimension. 3) Connection per 9/S1-5028 will not work at the noted location. Supply an alternate detail. 4) Confirm off-center connection per 9/S1-5028 is acceptable or supply an alternate detail. 5) Confirm the noted dimensions may vary to match the W-3 supporting beam locations above. 6) Confirm the orientation for the MC4. 7) Confirm the vertical hanger is interrupted at the upper beam. Please clarify. 8) Clarify where the MC4x13.8 is located and supply a connection to the W12 & CMU wall. 9) Supply the elevation for the W12x40 at the landing and depending on the elevation, supply a connection detail at each end.			See attached CD RFI # 555 SK1 for items 1 to 9: 1) Supply/confirm all clouded dimensions (9 clouds total). 2) Verify dimension. 3) Connection per 9/S1-5028 will not work at the noted location. Supply an alternate detail. 4) Confirm off-center connection per 9/S1-5028 is acceptable or supply an alternate detail. 5) Confirm the noted dimensions may vary to match the W-3 supporting beam locations above. 6) Confirm the orientation for the MC4. 7) Confirm the vertical hanger is interrupted at the upper beam. Please clarify. 8) Clarify where the MC4x13.8 is located and supply a connection to the W12 & CMU wall. 9) Supply the elevation for the W12x40 at the landing and depending on the elevation, supply a connection detail at each end.			



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<b>T-1637</b>	<b>SSS - North Exit Mezzanine Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/11/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 561 SK1 to SK4 for items 1 to 20: 1) Confirm all dimensions shown in a green rectangle box. 2) The noted W12x40 appears to be shown as one continuous beam from west of Grid 22 to east of grid 24. Supply splice locations and a splice detail. 3) Confirm the CMU wall aligns with the west edge of the slab opening or supply more information. 4) Confirm it is acceptable to locate the MC4's 1" from edge of slab openings. 5) It is not clear where the MC4 is located. Is it backed against the CMU wall? Please provide more information and connection details. 6) The location of the hangers are not correct at the locations noted as a red rectangle box based on the concrete beam locations per RFI T-1010 (SK 287, CD 230). Confirm the updated dimensions are correct. 7) Confirm connection is per 9/S1-5028. 8) Supply a connection detail. 9) Supply a connection detail for MC4 to MC4. 10) Confirm all W12x40's and all MC4x13.8's are located vertically per 4/S1-5032 (SK3). If not, supply more information. 11) The location of the hangers are not correct at the locations noted as a red rectangle box based on the concrete beam locations per RFI T-1010 (SK 287, CD 230). Confirm the updated dimensions are correct. 12) Confirm the CMU wall aligns with the east edge of the slab opening or supply more information. 13) Confirm dimension to edge of slab. 14) Confirm MC4 is located 1" west from the edge of slab or supply more information. 15) Confirm the underside of the slab is typically at the top of the bottom flange of the W12x40's or supply more information. 16) Supply a connection detail for the MC4 to the W12x40.  17) Supply a connection detail for the MC4 to the CMU wall. 18) Please supply dimensions for hooked deformed bars. 19) Confirm stud spacing to avoid the bending radius. 20) Confirm 1 1/4" I/S cold bending radius.			See attached CD RFI # 561 SK1 to SK4 for items 1 to 20: 1) Confirm all dimensions shown in a green rectangle box. 2) The noted W12x40 appears to be shown as one continuous beam from west of Grid 22 to east of grid 24. Supply splice locations and a splice detail. 3) Confirm the CMU wall aligns with the west edge of the slab opening or supply more information. 4) Confirm it is acceptable to locate the MC4's 1" from edge of slab openings. 5) It is not clear where the MC4 is located. Is it backed against the CMU wall? Please provide more information and connection details. 6) The location of the hangers are not correct at the locations noted as a red rectangle box based on the concrete beam locations per RFI T-1010 (SK 287, CD 230). Confirm the updated dimensions are correct. 7) Confirm connection is per 9/S1-5028. 8) Supply a connection detail. 9) Supply a connection detail for MC4 to MC4. 10) Confirm all W12x40's and all MC4x13.8's are located vertically per 4/S1-5032 (SK3). If not, supply more information. 11) The location of the hangers are not correct at the locations noted as a red rectangle box based on the concrete beam locations per RFI T-1010 (SK 287, CD 230). Confirm the updated dimensions are correct. 12) Confirm the CMU wall aligns with the east edge of the slab opening or supply more information. 13) Confirm dimension to edge of slab. 14) Confirm MC4 is located 1" west from the edge of slab or supply more information. 15) Confirm the underside of the slab is typically at the top of the bottom flange of the W12x40's or supply more information. 16) Supply a connection detail for the MC4 to the W12x40. 17) Supply a connection detail for the MC4 to the CMU wall. 18) Please supply dimensions for hooked deformed bars. 19) Confirm stud spacing to avoid the bending radius. 20) Confirm 1 1/4" I/S cold bending radius.			



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<b>T-1638</b>	<b>SSS - Framing Beam Location at GL20.1G &amp; C</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/08/2014</b>
<b>REQUEST:</b>  See attached CD RFI # 577 SK1 for items 1 & 2: 1) Supply location for W14x68. 2) Supply welding information for L4x4x3/8 deck support a ngle flush with the top of the plate on the Transfer Girder.		<b>ANSWER:</b>  See attached CD RFI # 577 SK1 for items 1 & 2: 1) Supply location for W14x68. 2) Supply welding information for L4x4x3/8 deck supp ort angle flush with the top of the plate on the Transfer Girder.				
<b>T-1638.1</b>	<b>SSS - Framing Beam Location at GL20.1G &amp; C</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>10/01/2014</b>
<b>REQUEST:</b>  See attached CD RFI # 577.1 SK1: Confirm it is acceptable to connect the W14x68 with a shear plate per 1/S1-5011 as a double angle connection per 1/S1-5010 will foul the shear plate connection for the W33x118. NOTE: The same condition occurs at Grid C		<b>ANSWER:</b>  See attached CD RFI # 577.1 SK1: Confirm it is acceptable to connect the W14x68 with a shear plate per 1/S1-5011 as a double angle connection per 1/S1-5010 will foul the shear plate connection for the W33x118. NOTE: The same condition occurs at Grid C				
<b>T-1639</b>	<b>SSS - Light Column LC-301 Detail Clarifications</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/04/2014</b>
<b>REQUEST:</b>  See attached CD RFI # 581 SK1 for items 1 to 8: 1) Verify dimension. 2) Verify dimension. 3) Provide elevation. 4) Provide dimension. 5) Confirm the noted dimension should be 8 3/4" to match the dimension shown on 7/S1-6005. 6) Verify dimension. 7) Verify dimensions. 8) Verify dimension.		<b>ANSWER:</b>  See attached CD RFI # 581 SK1 for items 1 to 8: 1) Verify dimension. 2) Verify dimension. 3) Provide elevation. 4) Provide dimension. 5) Confirm the noted dimension should be 8 3/4" to match the dimension shown on 7/S1-6005. 6) Verify dimension. 7) Verify dimensions. 8) Verify dimension.				
<b>T-1639.1</b>	<b>SSS - Light Column LC-301 Detail Clarifications</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>10/02/2014</b>
<b>REQUEST:</b>  See attached CD RFI # 581.1 SK1 for items 1 to 3: 1) Verify elevation. 2) Verify dimensions.		<b>ANSWER:</b>  See attached CD RFI # 581.1 SK1 for items 1 to 3: 1) Verify elevation. 2) Verify dimensions.				

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T-1641	BGP - Embedded Column Base Plates	Closed	01	08/25/2014	09/04/2014	08/27/2014
From: Webcor Construction LP Claude Titcher						
REQUEST: Please reference detail 6, section B of S1-3503 and attached sketch.  Please confirm concrete column collar plate can be fabricated out of continuous plate at the concourse and bike/vehicle ramp level.						ANSWER: Please reference detail 6, section B of S1-3503 and attached sketch.  Please confirm concrete column collar plate can be fabricated out of continuous plate at the concourse and bike/vehicle ramp level.
T-1642	BGP - Electrical Junction Box at GL 2.8/V.3 Beam	Closed	01	08/25/2014	09/04/2014	08/27/2014
From: Webcor Construction LP Claude Titcher						
REQUEST: Plan sheet AI-2850 shows an electrical junction box located at GL 2.8/V.3 which is directly in the B18 Beam. Please confirm this location is correct or provide revised locating dimensions for this EJB.						ANSWER: Plan sheet AI-2850 shows an electrical junction box located at GL 2.8/V.3 which is directly in the B18 Beam. Please confirm this location is correct or provide revised locating dimensions for this EJB.
T-1643	SSS - W40 Moment Connection Clarification GL 21-22	Closed	CR	08/25/2014	09/04/2014	09/08/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: See attached CD RFI # 583 SK1: The required 11 bolts per 1/S1-5010 for the W40x149 does not leave room for the weld access hole at the bottom flange for the moment connection. Confirm it is acceptable to supply 10 bolts or supply an alternate solution.						ANSWER: See attached CD RFI # 583 SK1: The required 11 bolts per 1/S1-5010 for the W40x149 does not leave room for the weld access hole at the bottom flange for the moment connection. Confirm it is acceptable to supply 10 bolts or supply an alternate solution.
T-1644	SSS - Steel Fouling at Telecom Vault GL 33	Closed	CR	08/25/2014	09/04/2014	09/08/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: See attached CD RFI # 585 SK1: The W12x53 is flush with the underside of the sloping WT40x53 on the left end of the W12x53 and fouls the WT40x503 by 1" at the right end of the WT. Please provide a solution. Note: The same condition occurs on Grid C.3.						ANSWER: See attached CD RFI # 585 SK1: The W12x53 is flush with the underside of the sloping WT40x53 on the left end of the W12x53 and fouls the WT40x503 by 1" at the right end of the WT. Please provide a solution. Note: The same condition occurs on Grid C.3.



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<b>T-1645</b>	<b>SSS - SE201 &amp; SE202 W16 Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/09/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 587 SK1: Please supply a connection for the W16x36 to avoid fouling the connection per 9/S1-5032 on the opposite side.			See attached CD RFI # 587 SK1: Please supply a connection for the W16x36 to avoid fouling the connection per 9/S1-5032 on the opposite side.			
<b>T-1646</b>	<b>SSS - TPG1 Moment Connection Detail Clarification GL 21</b>	<b>Closed</b>	<b>CR</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>09/09/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 588 SK1: The bottom flange moment weld per 8/S1-5013 is not possible as the flange of the W44x230 only partially contacts the sloping flange of the TPG1. Please supply a detail.			See attached CD RFI # 588 SK1: The bottom flange moment weld per 8/S1-5013 is not possible as the flange of the W44x230 only partially contacts the sloping flange of the TPG1. Please supply a detail.			
<b>T-1647</b>	<b>SCS - 12" Thick Partition Walls over 24' height</b>	<b>Closed</b>	<b>01</b>	<b>08/25/2014</b>	<b>09/04/2014</b>	<b>08/27/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Partition walls taller than 24'-0", as indicated in Section 3/S1-9050, have vertical reinforcing of #7 bars @12" centers. Section 4/S -9051 indicates that the vertical bars will have a 90 degree tail at the top end of the bar. The standard tail length for #7 bars is 14" and will not fit within the 12" thick walls. Rotating the #7 bar tails to clear the concrete face by 1 1/2" clash with one another. Harris Rebar requests permission to utilize #7 straight vertical bars with a #5 'U' shaped cap with 30" lap@ 6" centers at the top of walls. Is this acceptable?			Partition walls taller than 24'-0", as indicated in Section 3/S1-9050, have vertical reinforcing of #7 bars @12" centers. Section 4/S -9051 indicates that the vertical bars will have a 90 degree tail at the top end of the bar. The standard tail length for #7 bars is 14" and will not fit within the 12" thick walls. Rotating the #7 bar tails to clear the concrete face by 1 1/2" clash with one another. Harris Rebar requests permission to utilize #7 straight vertical bars with a #5 'U' shaped cap with 30" lap@ 6" centers at the top of walls. Is this acceptable?			
In order to minimize multiple varying lengths of the #7 vertical wall bars, the #7 threaded vertical dowel bar length will be increased from 49" to 56". The #7 wall vertical bar will be held up to the top of wall elevation, less clear cover, and maintain a minimum 49" lap splice at the Concourse Level. Is this acceptable?			In order to minimize multiple varying lengths of the #7 vertical wall bars, the #7 threaded vertical dowel bar length will be increased from 49" to 56". The #7 wall vertical bar will be held up to the top of wall elevation, less clear cover, and maintain a minimum 49" lap splice at the Concourse Level. Is this acceptable?			





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<b>T-1648</b>	<b>SSS - Deck Support Angle at Drum Café Roof</b>	<b>Closed</b>	<b>CR</b>	<b>08/26/2014</b>	<b>09/05/2014</b>	<b>08/29/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 584 SK1: Supply the size, length and spacing for the expansion and hors shown on SK1.					<b>ANSWER:</b> See attached CD RFI # 584 SK1: Supply the size, length and spacing for the expansion anchors shown on SK1.	
<b>T-1649</b>	<b>SSS - Review Comments on Embed Angles</b>	<b>Closed</b>	<b>CR</b>	<b>08/26/2014</b>	<b>09/05/2014</b>	<b>09/08/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 589 SK1 for items 1 & 2: 1) 5" long studs are correct per detail 1/S1-7600. Confirm no further action is required. (The same applies to drawing 9519 & 9526). 2) 3 3/4" is correct to place the pipe collar on the centerline of the HSS8x8 post per 1/S1-7600 above as shown. Confirm no further action is required.					<b>ANSWER:</b> See attached CD RFI # 589 SK1 for items 1 & 2: 1) 5" long studs are correct per detail 1/S1-7600. Confirm no further action is required. (The same applies to drawing 9519 & 9526). 2) 3 3/4" is correct to place the pipe collar on the centerline of the HSS8x8 post per 1/S1-7600 above as shown. Confirm no further action is required.	
<b>T-1650</b>	<b>SCS - Requesting Approval for Precast Panels in Lieu of Cast-In-Place</b>	<b>Closed</b>	<b>01</b>	<b>08/27/2014</b>	<b>09/06/2014</b>	<b>09/15/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> See attached Exhibits A through D as a possible means of precast installation method. Using a precast yard facility in lieu of concrete cast in place will reduce the construction duration for the scallop, perimeter, and sky light elliptical wall operations since production can start prior to the construction start date. Precasting concrete elements also results in a better final product. Quality advantages of using a precast yard facility over cast in place methods are improved formwork design, curing procedures, temperature control, and form striping methods since work space, formwork weight, formwork relocation and reassembly, and hoisting equipment restrictions are greatly reduced if not eliminated.  shimmick requests approval from the engineer to allow precast manufactured panels in lieu of the cast in place walls shown on S1-2752 & S1-3282 scallop walls, S1- 2602 stair well, S1-2603 elliptical skylight, S1-2604 light column opening, S1-2606 stair shaft, and S1-2607 elevator shaft.					<b>ANSWER:</b> See attached Exhibits A through D as a possible means of precast installation method. Using a precast yard facility in lieu of concrete cast in place will reduce the construction duration for the scallop, perimeter, and sky light elliptical wall operations since production can start prior to the construction start date. Precasting concrete elements also results in a better final product. Quality advantages of using a precast yard facility over cast in place methods are improved formwork design, curing procedures, temperature control, and form striping methods since work space, formwork weight, formwork relocation and reassembly, and hoisting equipment restrictions are greatly reduced if not eliminated.  shimmick requests approval from the engineer to allow precast manufactured panels in lieu of the cast in place walls shown on S1-2752 & S1-3282 scallop walls, S1-2602 stair well, S1-2603 elliptical skylight, S1-2604 light column opening, S1-2606 stair shaft, and S1-2607 elevator shaft.	





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<p>Please confirm that the concept drawing is acceptable as well.</p>						
<b>T-1651</b>	<b>SCS - Concrete Anchor Inserts at Temp Bridges</b>	<b>Closed</b>	<b>01</b>	<b>08/27/2014</b>	<b>09/06/2014</b>	
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached photo and concept sketch.		See attached photo and concept sketch.				
A unique support system is necessary for the foundation wall under the temporary bridges (First, Fremont, and Beale). A top tie is required to avoid the significant forces caused by a cantilever system supported by 1 row of anchors. In the attached photo, a concrete ledge can be seen along the CDSM wall under the temporary bridges. SCCI proposes to utilize these concrete ledges to install anchors for tie points.		A unique support system is necessary for the foundation wall under the temporary bridges (First, Fremont, and Beale). A top tie is required to avoid the significant forces caused by a cantilever system supported by 1 row of anchors. In the attached photo, a concrete ledge can be seen along the CDSM wall under the temporary bridges. SCCI proposes to utilize these concrete ledges to install anchors for tie points.				
Please confirm it is acceptable to install anchors into the concrete ledges underneath the temporary bridges and provide rebar layout and concrete strength information for the concrete.		Please confirm it is acceptable to install anchors into the concrete ledges underneath the temporary bridges and provide rebar layout and concrete strength information for the concrete.				
<b>T-1652</b>	<b>BGP - B45 Beam Proposed Move Between GL 20.1 and 21</b>	<b>Closed</b>	<b>01</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>09/02/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Due to trestle pile conflict with Beam B45 between GL 20.1 and 21 at two locations, SCCI proposes that two B45 Beams be moved per attached drawings.		Due to trestle pile conflict with Beam B45 between GL 20.1 and 21 at two locations, SCCI proposes that two B45 Beams be moved per attached drawings.				
Please confirm that this is acceptable.		Please confirm that this is acceptable.				
<b>T-1653</b>	<b>SCS - Partition Wall Elevation</b>	<b>Closed</b>	<b>01</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>09/04/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						



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	<div><div>REQUEST:</div><div>On sheet A1-2203 and A1-3002, at the Transformer Vaults Room located about GL 11, Section detail A/A1-9236, regarding the elevation of the top of the partition walls.  1. Is the gap between the top of the wall and the Ground Level slab constant along the wall? This would mean that the top of wall would slope with the ground level. Please confirm. 2. Please clarify how to obtain the elevation of the top of the partition walls. Note: The elevations obtained through those two methods below conflict. a.Should be obtained using the ground slab elevation points on plan S1-2303 and a constant slope between two consecutive elevation points? -OR-. b.Scale the elevation from the section detail in AI A1-9236? 3. Please confirm if the above answers are applicable to all similar partition walls.</div></div>					
			<div><div>ANSWER:</div><div>On sheet A1-2203 and A1-3002, at the Transformer Vaults Room located about GL 11, Section detail A/A1-9236, regarding the elevation of the top of the partition walls.  1. Is the gap between the top of the wall and the Ground Level slab constant along the wall? This would mean that the top of wall would slope with the ground level. Please confirm. 2. Please clarify how to obtain the elevation of the top of the partition walls. Note: The elevations obtained through those two methods below conflict. a.Should be obtained using the ground slab elevation points on plan S1-2303 and a constant slope between two consecutive elevation points? -OR-. b.Scale the elevation from the section detail in AI A1-9236? 3. Please confirm if the above answers are applicable to all similar partition walls.</div></div>			
T-1654	SSS - Cast Node Work Points	Closed	CR	08/28/2014	09/07/2014	09/10/2014
	From: Webcor Construction LPGregory Kemerer					
	<div><div>REQUEST:</div><div>Candraft have identified several dimensional discrepancies between the Structural Engineer's certified Tekla model and the cast node shop drawings. Please verify correct dimensions queried in attached sketches</div></div>		<div><div>ANSWER:</div><div>Candraft have identified several dimensional discrepancies between the Structural Engineer's certified Tekla model and the cast node shop drawings. Please verify correct dimensions queried in attached sketches</div></div>			



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T-1655	SCS - Additional Information Needed to Cut CDSM Soldier Pile Beam.	Closed	01	08/28/2014	09/07/2014	08/28/2014
<b>From:</b> Shimmick Construction Company, Inc. Henry Chiang						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Direction provided is limited to: EXHIBIT A - TRADE SUBCONTRACTOR BID PACKAGE MANUAL AND FORMS, IV. SCOPE OF PACKAGE, D - 13. pg-13 - "Cut Interior Flange and Web of the CDSM Shoring Wall Steel Soldier Pile. Assume that the cutting of the pile will occur between 2' and 8' below the finish grade surface. The precise elevations for the cutting of the pile beams will be determined by a future design requirement."</p> <p>In order to perform this work the elevation of the cut is needed.</p> <p>Shimmick was informally provided a copy of the attached drawing from Webcor Obayashi Joint Venture which only identifies a vertical distance in elevation from finished grade elevations. Shimmick was then informed by WOJV that Thornton Tomassetti is currently making corrections to the site grading civil drawings C1-1000's to C1-8000's Rev 0 Dated 03/31/14 which show top of curb elevations. The civil drawings do not provide complete curb elevations to determine the elevation of the cut at all piles.</p> <p>1. Confirm that the attached drawing which identifies a vertical distance in elevation from finished grade elevation is to be used.</p> <p>2. Are the top of curb elevations in the civil drawings to be used as finish grade elevations for the depth of cut on the CDSM piles?</p> <p>a. Provide finished grade elevations for cutting the interior flange and web of the CDSM piles or provide a height from top of concrete elevations shown on the ground level deck on plans sheet A1- 2862 to A1-2781. Also, please provide a drawing identifying the type of cut required ie., horizontal, angling up, or angling down.</p>			<p>Direction provided is limited to: EXHIBIT A - TRADE SUBCONTRACTOR BID PACKAGE MANUAL AND FORMS, IV. SCOPE OF PACKAGE, D - 13. pg-13 - "Cut Interior Flange and Web of the CDSM Shoring Wall Steel Soldier Pile. Assume that the cutting of the pile will occur between 2' and 8' below the finish grade surface. The precise elevations for the cutting of the pile beams will be determined by a future design requirement."</p> <p>In order to perform this work the elevation of the cut is needed.</p> <p>Shimmick was informally provided a copy of the attached drawing from Webcor Obayashi Joint Venture which only identifies a vertical distance in elevation from finished grade elevations. Shimmick was then informed by WOJV that Thornton Tomassetti is currently making corrections to the site grading civil drawings C1-1000's to C1-8000's Rev 0 Dated 03/31/14 which show top of curb elevations. The civil drawings do not provide complete curb elevations to determine the elevation of the cut at all piles.</p> <p>1. Confirm that the attached drawing which identifies a vertical distance in elevation from finished grade elevation is to be used.</p> <p>2. Are the top of curb elevations in the civil drawings to be used as finish grade elevations for the depth of cut on the CDSM piles?</p> <p>a. Provide finished grade elevations for cutting the interior flange and web of the CDSM piles or provide a height from top of concrete elevations shown on the ground level deck on plans sheet A1- 2862 to A1-2781. Also, please provide a drawing identifying the type of cut required ie., horizontal, angling up, or angling down.</p>			



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<b>T-1656</b>	<b>SSS - Beam Spacing Clarification at W-4 System GL20.1-22</b>	<b>Closed</b>	<b>CR</b>	<b>08/28/2014</b>	<b>09/07/2014</b>	<b>09/17/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> The beam spacing does not meet the 6'-0 max. spacing requirement in 10/S1-5004. Please clarify the beam locations to match 10/S1-5004.						<b>ANSWER:</b> The beam spacing does not meet the 6'-0 max. spacing requirement in 10/S1-5004. Please clarify the beam locations to match 10/S1-5004.
<b>T-1657</b>	<b>SCS - Lower Concourse MEP Room Construction Joints</b>	<b>Closed</b>	<b>01</b>	<b>09/03/2014</b>	<b>09/13/2014</b>	<b>09/12/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> ASI 121 issued new wall CJ locations. Reference sheet AI-2203, Detail I/AI-3008 and B/AI-9240.  A. On the south wall of the room at GL 13-B.2 there is a construction joint (see plan attached indicated as location "A") running through one of the MEP blocks outs. That creates a cantilever section below the beams cutout and above the MEP block out. Is that acceptable? If not, where should the CJ be relocated?  B. On the same wall there is a construction joint ( see plan attached indicated as location "B"), the distance between the MEP block and the construction joint is 5' 5/8", which is not enough to place diagonal trim bars on the comers of the MEP block out that are closer to the construction joint. Please provide detail for this location.  C. In the same wall, the construction joints are not shown in the area cover by the spall plate, do the CJ's start at the top of the wall plates? Please confirm intent and provide further direction.						<b>ANSWER:</b> ASI 121 issued new wall CJ locations. Reference sheet AI-2203, Detail I/AI-3008 and B/AI-9240.  A. On the south wall of the room at GL 13-B.2 there is a construction joint (see plan attached indicated as location "A") running through one of the MEP blocks outs. That creates a cantilever section below the beams cutout and above the MEP block out. Is that acceptable? If not, where should the CJ be relocated?  B. On the same wall there is a construction joint ( see plan attached indicated as location "B"), the distance between the MEP block and the construction joint is 5' 5/8", which is not enough to place diagonal trim bars on the comers of the MEP block out that are closer to the construction joint. Please provide detail for this location.  C. In the same wall, the construction joints are not shown in the area cover by the spall plate, do the CJ's start at the top of the wall plates? Please confirm intent and provide further direction.
<b>T-1658</b>	<b>SCS - ST201 Intersection of grade beams, shoring wall, and foundation wall</b>	<b>Closed</b>	<b>CR</b>	<b>09/04/2014</b>	<b>09/14/2014</b>	<b>09/15/2014</b>
<b>From:</b> Webcor Construction LP Stephanie Azzolino						
<b>REQUEST:</b> The contract drawings do not provide a section or detail for the intersection between the foundation wall, stair tower grade beams, and ground floor deck slab. A rebar detail for the grade beams at the connection is not provided. The bottom of the stair tower walls are not provided. Shimmick						<b>ANSWER:</b> The contract drawings do not provide a section or detail for the intersection between the foundation wall, stair tower grade beams, and ground floor deck slab. A rebar detail for the grade beams at the connection is not provided. The bottom of the stair tower walls are



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	<p>has been provided with updated sketches that show the connection of the stair tower grade beams and foundation wall.</p> <p>1) Please confirm that the SKS-400 details show the intersection between the foundation wall, stair tower grade beams, and ground floor deck slab as intended per the contract drawings.</p> <p>2) Please confirm that the SKS-400 details show the rebar detail at the grade beams as intended per the contract drawings.</p> <p>3) Please confirm that the bottom of wall elevations called out in SKS-400 match the intended elevations per the contract drawings.</p>					
<b>T-1659</b>	<b>SCS - ST201 Sloping of grade beams</b>	<b>Closed</b>	<b>CR</b>	<b>09/04/2014</b>	<b>09/14/2014</b>	<b>09/15/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>						
<p>Drawing S 1-2302 at Stair Tower 201 call only the grade beams north of GL E as sloping and the grade beams south of GL E are not identified to have a slope. A section view is not provided to confirm the slopes for either set of grade beams. Shimmick has been provided with updated sketches that show the slopes for each set of grade beams.</p> <p>1) Please confirm that SKS-400 shows the slopes for each grade beam at Stair Tower 201.</p>						
<b>ANSWER:</b>						
<p>Drawing S 1-2302 at Stair Tower 201 call only the grade beams north of GL E as sloping and the grade beams south of GL E are not identified to have a slope. A section view is not provided to confirm the slopes for either set of grade beams. Shimmick has been provided with updated sketches that show the slopes for each set of grade beams.</p> <p>1) Please confirm that SKS-400 shows the slopes for each grade beam at Stair Tower 201.</p>						



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<b>T-1660</b>	<b>SCS - ST201 Thickened foundation wall</b>	<b>Closed</b>	<b>CR</b>	<b>09/04/2014</b>	<b>09/14/2014</b>	<b>09/12/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The contract drawings do not provide a section view for the foundation wall at Stair Tower 201. S1-2302 calls out the foundation wall as a uniform 3' thickness. S1-3201 provides a typical foundation wall section, shown with uniform thickness. Shimmick has been provided with updated sketches that call out a thickened foundation wall of 4' 3 1/2" at Stair Tower 201 as opposed to the uniform 3' thickness.			The contract drawings do not provide a section view for the foundation wall at Stair Tower 201. S1-2302 calls out the foundation wall as a uniform 3' thickness. S1-3201 provides a typical foundation wall section, shown with uniform thickness. Shimmick has been provided with updated sketches that call out a thickened foundation wall of 4' 3 1/2" at Stair Tower 201 as opposed to the uniform 3' thickness.			
1) Please confirm that, at Stair Tower 201 between Grid Line D and E.6, the foundation wall will be thickened to 4' 3 1/2" per the sketches provided.			1) Please confirm that, at Stair Tower 201 between Grid Line D and E.6, the foundation wall will be thickened to 4' 3 1/2" per the sketches provided.			
<b>T-1661</b>	<b>SCS - ST201 Wall thicknesses A1-2960 with SKS 0398</b>	<b>Closed</b>	<b>CR</b>	<b>09/04/2014</b>	<b>09/14/2014</b>	<b>09/12/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Shimmick has been provided with sketches, namely SKS-0398, calling out thickness of the walls at the foundation of Stair Tower 201. The wall thickness called out in the sketches do not match the thickness provided in the contract drawing A1-2960 nor SKA-2991 as provided in Bid Addendum 3.			Shimmick has been provided with sketches, namely SKS-0398, calling out thickness of the walls at the foundation of Stair Tower 201. The wall thickness called out in the sketches do not match the thickness provided in the contract drawing A1-2960 nor SKA-2991 as provided in Bid Addendum 3.			
1) Please confirm which set of drawing wall thickness dimensions are correct.			1) Please confirm which set of drawing wall thickness dimensions are correct.			
<b>T-1662</b>	<b>SCS - CDSM Shoring Wall Pre-Cut</b>	<b>Closed</b>	<b>01</b>	<b>09/04/2014</b>	<b>09/14/2014</b>	<b>09/17/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please see attached documents.			Please see attached documents.			
Pursuant to the response given in T-0774 WOJV is proposing the following means and method for precutting the interior flange of the CDSM beams in order to prevent damage to the waterproofing. Attached is a narrative along with a map of the locations, and depth wherein beams will be notched prior to installation of the waterproofing.			Pursuant to the response given in T-0774 WOJV is proposing the following means and method for precutting the interior flange of the CDSM beams in order to prevent damage to the waterproofing. Attached is a narrative along with a map of the locations, and depth wherein beams will be notched prior to installation of the waterproofing.			











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<b>T-1667</b>	<b>SCS - Foundation Wall Conduit Penetration Locations</b>	<b>Closed</b>	<b>CR</b>	<b>09/08/2014</b>	<b>09/18/2014</b>	<b>09/08/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Reference drawings: A1-2862 to A1-2871, E1-2862 to E1-2871 and E1-3208 to E1-3210.  Please see attached marked up drawings noting atypical set of conduit penetrations. A1-2862 provides a lateral distance relative to a grid line to set of conduits; however 3/E1-3208 details the set of conduits as not aligned. On top of this, the referenced drawings do not provide an elevation for the conduit penetrations. SCCO requires the locations of these penetrations to proceed with foundation wall rebar detailing.  Please provide information on the location of the electrical conduit foundation wall penetrations.						
						<b>ANSWER:</b>  Reference drawings: A1-2862 to A1-2871, E1-2862 to E1-2871 and E1-3208 to E1-3210.  Please see attached marked up drawings noting atypical set of conduit penetrations. A1-2862 provides a lateral distance relative to a grid line to set of conduits; however 3/E1-3208 details the set of conduits as not aligned. On top of this, the referenced drawings do not provide an elevation for the conduit penetrations. SCCO requires the locations of these penetrations to proceed with foundation wall rebar detailing.  Please provide information on the location of the electrical conduit foundation wall penetrations.
<b>T-1668</b>	<b>SSS - Weld Access Hole at Transfer Girder</b>	<b>Closed</b>	<b>CR</b>	<b>09/08/2014</b>	<b>09/18/2014</b>	<b>09/15/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  Please confirm it is acceptable to provide weld access holes as indicated on 628BC per AWS D1.1 to adequately terminate the CJP weld to cast node. For plates not fabricated, confirm it is acceptable to omit the hole closest to the proposed access hole to maintain AISC min edge distance. This typically occurs at all cast node to girder connections.						
						<b>ANSWER:</b>  Please confirm it is acceptable to provide weld access holes as indicated on 628BC per AWS D1.1 to adequately terminate the CJP weld to cast node. For plates not fabricated, confirm it is acceptable to omit the hole closest to the proposed access hole to maintain AISC min edge distance. This typically occurs at all cast node to girder connections.
<b>T-1669</b>	<b>SSS - Insufficient Cope for Erection Clearance</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2014</b>	<b>09/19/2014</b>	<b>09/15/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>  Please refer to beams highlighted in S1-2503 (attached) and work with this RFI.  The 7" cope shown in 2/S1-5017 is not sufficient for erection clearance. The connections on the other end of these beams dictate that the beams cannot be dropped vertically into position during erection (Please refer to attached SK RFI 813 SK1 and SK RFI 813 SK2). Beams						
						<b>ANSWER:</b>  Please refer to beams highlighted in S1-2503 (attached) and work with this RFI.  The 7" cope shown in 2/S1-5017 is not sufficient for erection clearance. The connections on the other end of these beams dictate that the beams cannot be dropped vertically into position during erection (Please refer to attached SK RFI 813 SK1 and SK RFI 813



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	<p>have either a gravity moment connection (per 1/S1-8003) at odd gridlines or a shear plate connection (per 1/S1-8000) at even gridlines. These beams connect into the bus deck perimeter beams which must be installed first in our erection sequence due to the constraints of knife/pin connections at either end at the bus deck level cast nodes.</p> <p>Please advise.</p>					<p>SK2). Beams have either a gravity moment connection (per 1/S1-8003) at odd gridlines or a shear plate connection (per 1/S1-8000) at even gridlines. These beams connect into the bus deck perimeter beams which must be installed first in our erection sequence due to the constraints of knife/pin connections at either end at the bus deck level cast nodes.</p> <p>Please advise.</p>
<b>T-1670</b>	<b>SSS - Reviewer Comments - Decking Dwgs GL 1-4</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2014</b>	<b>09/19/2014</b>	<b>09/17/2014</b>
	<p><b>From:</b> Webcor Construction LP Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>Reference metal decking shop drawings submittal package TG0701-92D &amp; SK1</p> <p>1) On sheet 24, reviewer's comment requires studs on transfer girder. Note at TR3 the concrete cover on top of girder flange is 3". T/slab=21.344, T/steel=21.19) The shortest available stud is 3" after welding, resulting in insufficient concrete coverage. Please confirm shear studs are not required at TR3.</p> <p>2) On sheet D16, detail 4, reviewer's comment requires 3" of bearing at S3 slabs per 2/S1-5000. Note, detail 4 depicts a deck flute parallel condition, note 2 on 2/S1-5000 indicates 2" bearing (3" bearing where noted) at supporting members perpendicular to deck span and 1-1/2" at members parallel. As this is a parallel condition the minimum bearing required is 1-1/2". Confirm detail is acceptable as is.</p>					<p><b>ANSWER:</b></p> <p>Reference metal decking shop drawings submittal package TG0701-92D &amp; SK1</p> <p>1) On sheet 24, reviewer's comment requires studs on transfer girder. Note at TR3 the concrete cover on top of girder flange is 3". T/slab=21.344, T/steel=21.19) The shortest available stud is 3" after welding, resulting in insufficient concrete coverage. Please confirm shear studs are not required at TR3.</p> <p>2) On sheet D16, detail 4, reviewer's comment requires 3" of bearing at S3 slabs per 2/S1-5000. Note, detail 4 depicts a deck flute parallel condition, note 2 on 2/S1-5000 indicates 2" bearing (3" bearing where noted) at supporting members perpendicular to deck span and 1-1/2" at members parallel. As this is a parallel condition the minimum bearing required is 1-1/2". Confirm detail is acceptable as is.</p>





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	<p>XKT has welded each type of weld joint we will see for the 130 Moment Columns. XKT's repair percentages have been well below the required limit of 5%, to demonstrate the quality of our welders.</p> <p>Per this section of specifications, the amount of UT and MT testing is permitted to be reduced if approved by the engineer of record.</p> <p>To facilitate production and project schedule, XKT is requesting the engineer of record to implement this reduction to be performed by ISI QA Inspection Services.</p>					
	<p>XKT has welded each type of weld joint we will see for the 130 Moment Columns. XKT's repair percentages have been well below the required limit of 5%, to demonstrate the quality of our welders.</p> <p>Per this section of specifications, the amount of UT and MT testing is permitted to be reduced if approved by the engineer of record.</p> <p>To facilitate production and project schedule, XKT is requesting the engineer of record to implement this reduction to be performed by ISI QA Inspection Services.</p>					
T-1673.1	SSS - Structural Steel Additional Seismic Requirements	Closed	CR	12/18/2014	12/28/2014	01/06/2015
	From: Webcor Construction LP  Gregory Kemerer					
	REQUEST:  Per the response to SK RFI 812 (T-1673), the EOR made recommendations that allowed for certain reductions in NDT. The final decision rested with TJPA/Turner and at that time they decided to not permit the reduction of any NDT testing.					
	However, as per the meeting held at XKT Engineering on 11/14/14, it was discussed that TJPA/Turner would revisit the NDT reduction request from XKT Engineering. XKT has continued to demonstrate the quality of their welders, keeping repair percentages at 2%, well below the required limit of 5% as per AISC. At this time, XKT has completed 16 moment frame columns with another 20 columns currently in fabrication (see attached status report). This experience has allowed them to substantiate their repair percentages and the overall quality of their work.					
	Please confirm that the EOR's recommendation to SK RFI 812 (T-1673) is acceptable and the NDT reduction may occur at the XKT Engineering facility.					



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<b>T-1674</b>	<b>SSS - PE502 &amp; PE503 Ground Level Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2014</b>	<b>09/19/2014</b>	<b>09/19/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 597 SK1 for items 1 & 2: 1) The W12x19 & W24x55 cannot both be connected with double angle connections per 1 /S1-5010 because of insufficient space. Confirm it is acceptable to connect the W12x19 per 1/S1-5011. 2) The W12x19 & W16x26 cannot both be connected with double angle connections per 1 /S1-5010 because of insufficient space. Confirm it is acceptable to connect the W12x19 per 1/S1-5011.						<b>ANSWER:</b> See attached CD RFI # 597 SK1 for items 1 & 2: 1) The W12x19 & W24x55 cannot both be connected with double angle connections per 1 /S1-5010 because of insufficient space. Confirm it is acceptable to connect the W12x19 per 1/S1-5011. 2) The W12x19 & W16x26 cannot both be connected with double angle connections per 1 /S1-5010 because of insufficient space. Confirm it is acceptable to connect the W12x19 per 1/S1-5011.
<b>T-1675</b>	<b>SSS - Roof Level Moment Connection Clarifications GL 22</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2014</b>	<b>09/19/2014</b>	<b>09/19/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 598 SK1 for items 1 TO 3: 1) Confirm the prep. shown on the 2 1/2" thick flange is acceptable for the moment connection or supply an alternate detail. 2) The flange of the BU-beam does not align with the flange of the W44x230 for the moment connection. Please advise. 3) Confirm the prep. shown on the 1 3/4" thick flange is acceptable for the moment connection or supply an alternate detail.						<b>ANSWER:</b> See attached CD RFI # 598 SK1 for items 1 TO 3: 1) Confirm the prep. shown on the 2 1/2" thick flange is acceptable for the moment connection or supply an alternate detail. 2) The flange of the BU-beam does not align with the flange of the W44x230 for the moment connection. Please advise. 3) Confirm the prep. shown on the 1 3/4" thick flange is acceptable for the moment connection or supply an alternate detail.
<b>T-1676</b>	<b>SSS - Missing Weld Access Hole Information at Roof Level GL 28</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2014</b>	<b>09/19/2014</b>	<b>09/22/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 600 SK1: Confirm the weld access hole as shown is acceptable or supply the missing information.						<b>ANSWER:</b> See attached CD RFI # 600 SK1: Confirm the weld access hole as shown is acceptable or supply the missing information.
<b>T-1677</b>	<b>SSS - TPG1 Connection Clarification GL 32</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2014</b>	<b>09/19/2014</b>	<b>09/19/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 601 SK1: Please confirm the skewed TPG1 is to be connected to						<b>ANSWER:</b> See attached CD RFI # 601 SK1: Please confirm the skewed TPG1 is to be connected



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	the BU-40x28x1x2.25 per 8/S1-5032 (skewed). If not, supply the missing detail.					to the BU-40x28x1x2.25 per 8/S1-5032 (skewed). If not, supply the missing detail.
<b>T-1678</b>	<b>SSS - TR23 Missing Moment Connection Detail</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2014</b>	<b>09/19/2014</b>	<b>09/19/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 602 SK1: The W30x90 connects to the BU-60x24 per 1/S1-5010 with the flanges moment welded but no detail is provided for the moment connection of the Transfer Girder to the BU-60x24 connect. Please provide a detail.						<b>ANSWER:</b> See attached CD RFI # 602 SK1: The W30x90 connects to the BU-60x24 per 1/S1-5010 with the flanges moment welded but no detail is provided for the moment connection of the Transfer Girder to the BU-60x24 connect. Please provide a detail.
<b>T-1679</b>	<b>SSS - TR29 Bolted Connection Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2014</b>	<b>09/19/2014</b>	<b>09/19/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 604 SK1 & SK2 for items 1 & 2: 1) Condition at Transfer Girders with 4" thick flange: It is not possible to fit in the number of bolts as requested in 1/S1-5010 with the 4" thick Transfer Girder flange even with minimizing the end distance on the angles to 1 1/4" as shown. This condition occurs at W40x183, W36x135, W33x118, W30x108, W30x90 & W21x44 on S1-2606 & S1-2607. Confirm it is acceptable to reduce the number of bolts by one or supply an alternate connection. 2) Condition at Transfer Girders with 3" thick flange: It is not possible to fit in the number of bolts as requested in 1/S1-5010 with the 3" thick Transfer Girder flange unless we minimize the end distance on the angles to 1 1/4" as shown. This condition occurs at W40x183, W36x135, W33x118, W30x108, W30x90 & W21x44 on S1-2606 & S1-2607. Confirm this is acceptable or supply an alternate connection.						<b>ANSWER:</b> See attached CD RFI # 604 SK1 & SK2 for items 1 & 2: 1) Condition at Transfer Girders with 4" thick flange: It is not possible to fit in the number of bolts as requested in 1/S1-5010 with the 4" thick Transfer Girder flange even with minimizing the end distance on the angles to 1 1/4" as shown. This condition occurs at W40x183, W36x135, W33x118, W30x108, W30x90 & W21x44 on S1-2606 & S1-2607. Confirm it is acceptable to reduce the number of bolts by one or supply an alternate connection. 2) Condition at Transfer Girders with 3" thick flange: It is not possible to fit in the number of bolts as requested in 1/S1-5010 with the 3" thick Transfer Girder flange unless we minimize the end distance on the angles to 1 1/4" as shown. This condition occurs at W40x183, W36x135, W33x118, W30x108, W30x90 & W21x44 on S1-2606 & S1-2607. Confirm this is acceptable or supply an alternate connection.





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<b>T-1680</b>	<b>SSS - ST301 Post Location Discrepancy</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2014</b>	<b>09/19/2014</b>	<b>09/11/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 591 SK1 & SK2: The dimensions locating the HSS posts shown in detail 2/S1-7007 and detail 3/A1/7006 do not match as shown. Please clarify the discrepancy.					<b>ANSWER:</b>  See attached CD RFI # 591 SK1 & SK2: The dimensions locating the HSS posts shown in detail 2/S1-7007 and detail 3/A1/7006 do not match as shown. Please clarify the discrepancy.	
<b>T-1681</b>	<b>SCS Clarification of Amount of Camber in BU-40x28x1x2.25 Beam</b>	<b>Closed</b>	<b>01</b>	<b>09/09/2014</b>	<b>09/19/2014</b>	<b>09/22/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Shimmick is requesting the engineer to provide the amount of camber that the roof top perimeter beams BU-40-28x1x2.25 will have before the perimeter walls are installed and after the roof is fully loaded. Refer to S1-2602 for location of BU-40-28x1x2.25.  Shimmick is requesting the engineer to provide the amount of vertical tolerance that should be anticipated in the perimeter beams BU-40-28x1x2.25 at the connection points.  Shimmick is requesting the engineer to provide the amount of vertical tolerance that will be acceptable for the roof top perimeter walls.  See attached Exhibit A for visual aid.					<b>ANSWER:</b>  Shimmick is requesting the engineer to provide the amount of camber that the roof top perimeter beams BU-40-28x1x2.25 will have before the perimeter walls are installed and after the roof is fully loaded. Refer to S1-2602 for location of BU-40-28x1x2.25.  Shimmick is requesting the engineer to provide the amount of vertical tolerance that should be anticipated in the perimeter beams BU-40-28x1x2.25 at the connection points.  Shimmick is requesting the engineer to provide the amount of vertical tolerance that will be acceptable for the roof top perimeter walls.  See attached Exhibit A for visual aid.	
<b>T-1683</b>	<b>SSS - Stair Opening Framing Clarification GL 16-16.9</b>	<b>Closed</b>	<b>CR</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/24/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 593 SK1: The new slab opening shown on A1-2884 in ASI 121 is not shown on the current version of S1-2404 and will foul the steel as shown. Please clarify.					<b>ANSWER:</b>  See attached CD RFI # 593 SK1: The new slab opening shown on A1-2884 in ASI 121 is not shown on the current version of S1-2404 and will foul the steel as shown. Please clarify.	





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<b>T-1684</b>	<b>BGP - Ramp Dimensions to RPC-308V and RPC-309V</b>	<b>Closed</b>	<b>01</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/18/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please provide missing dimensions for RPC-308V and RPC-309V. On AI-2870 there are N-S dimensions between RPC-308V, RPC-309V, RPC-310V, and RPC 3111V, but no reference dimension to anything else. Please provide N-S dimension to a reference point (i.e. Gridline L).			Please provide missing dimensions for RPC-308V and RPC-309V. On AI-2870 there are N-S dimensions between RPC-308V, RPC-309V, RPC-310V, and RPC 3111V, but no reference dimension to anything else. Please provide N-S dimension to a reference point (i.e. Gridline L).			
The E-W dimension for RPC-309V is given to the slab centerline. Please provide E-W dimension for RPC-309V to a valid reference point (i.e. Gridline 3).			The E-W dimension for RPC-309V is given to the slab centerline. Please provide E-W dimension for RPC-309V to a valid reference point (i.e. Gridline 3).			
<b>T-1685</b>	<b>SSS - PE301 &amp; PE302 Brace to BU-WT Connection</b>	<b>Closed</b>	<b>CR</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/22/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 467 SK1 & SK2: Please clarify how the braces per 1/S1-7600 are to be applied at the noted locations with the close proximity of the beams and the BU-WT's on top of the beams.			See attached CD RFI # 467 SK1 & SK2: Please clarify how the braces per 1/S1-7600 are to be applied at the noted locations with the close proximity of the beams and the BU-WT's on top of the beams.			
<b>T-1686</b>	<b>SSS - Stiffener Interference at Ground Level GL 24</b>	<b>Void</b>	<b>CR</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 595 SK1: Confirm the beam location dimensions per S1-6050 & RFI T-1573 shown in red may be adjusted as shown in blue in order to connect the noted W44x230's to the 2" stiffener plate below the column flange. If not, supply an alternate solution.			See attached CD RFI # 595 SK1: Confirm the beam location dimensions per S1-6050 & RFI T-1573 shown in red may be adjusted as shown in blue in order to connect the noted W44x230's to the 2" stiffener plate below the column flange. If not, supply an alternate solution.			
<b>T-1687</b>	<b>SSS - Light Column Pipe Ring Bending Radius at EL 12ft</b>	<b>Closed</b>	<b>CR</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/18/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The light column ring at EL. 12'-0" is specified as 6'-11 3/4" diameter (measured from center of pipe), from 14" diameter pipe with a wall thickness of 1.97". Refer to			The light column ring at EL. 12'-0" is specified as 6'-11 3/4" diameter (measured from center of pipe), from 14" diameter pipe with a wall thickness of 1.97". Refer			



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	<p>section C on S1-6006.</p> <p>The minimum achievable bending radius for a pipe this size and wall thickness is 42". This exceeds the actual designed radius by 1/8". We are requesting permission to change the diameter of this ring to 7' which will allow the minimum bending radius to be achieved. The difference in ring member size will be accommodated in the connection plates which join the ring to the cast nodes.</p> <p>Please confirm acceptance for this proposal</p>					
	<p>to section C on S1-6006.</p> <p>The minimum achievable bending radius for a pipe this size and wall thickness is 42". This exceeds the actual designed radius by 1/8". We are requesting permission to change the diameter of this ring to 7' which will allow the minimum bending radius to be achieved. The difference in ring member size will be accommodated in the connection plates which join the ring to the cast nodes.</p> <p>Please confirm acceptance for this proposal</p>					
<b>T-1688</b>	<b>SSS - Connection Clarification at GL 21.5</b>	<b>Closed</b>	<b>CR</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/19/2014</b>
<div><div><p><b>From:</b> Webcor Construction LP</p><p><b>REQUEST:</b></p><p>See attached CD RFI # 610 SK1 for items 1 &amp; 2:</p><p>1) The top flange of the W40x503 is cut 1/2" past the PL 2 1/2". The shear plate for the W30x231 is only 3/8" past the end of the PL 2 1/2" and can therefore only be welded to the top flange on one side. Confirm if acceptable or supply an alternate solution.</p><p>2) The shear plate is 3/8" past the end of the PL 2 1/2" and the 5/16" fillet weld for the shear plate will clear the PL 2 1/2" by 1/16". Confirm that is acceptable or supply an alternate solution.</p></div><div><p><b>ANSWER:</b></p><p>See attached CD RFI # 610 SK1 for items 1 &amp; 2:</p><p>1) The top flange of the W40x503 is cut 1/2" past the PL 2 1/2". The shear plate for the W30x231 is only 3/8" past the end of the PL 2 1/2" and can therefore only be welded to the top flange on one side. Confirm if acceptable or supply an alternate solution.</p><p>2) The shear plate is 3/8" past the end of the PL 2 1/2" and the 5/16" fillet weld for the shear plate will clear the PL 2 1/2" by 1/16". Confirm that is acceptable or supply an alternate solution.</p></div></div>						
<b>T-1688.1</b>	<b>SSS - Connection Clarification at GL 21.5</b>	<b>Closed</b>	<b>CR</b>	<b>09/26/2014</b>	<b>10/06/2014</b>	<b>10/06/2014</b>
<div><div><p><b>From:</b> Webcor Construction LP</p><p><b>REQUEST:</b></p><p>See attached CD RFI # 610.1 SK1:</p><p>The short W36x231 cannot be erected with the shear plate on the east side of the web as shown in RFI T-1688 (SK 819, CD 610). To place the shear plate on the west side as shown will require the beam to be moved as shown. Confirm that is acceptable.</p></div><div><p><b>ANSWER:</b></p><p>See attached CD RFI # 610.1 SK1:</p><p>The short W36x231 cannot be erected with the shear plate on the east side of the web as shown in RFI T-1688 (SK 819, CD 610). To place the shear plate on the west side as shown will require the beam to be moved as shown. Confirm that is acceptable.</p></div></div>						



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T-1689	SSS - Nut Grade Clarification	Closed	CR	09/12/2014	09/22/2014	09/23/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: Please see attached SK1 and note the following:  After reviewing structural design drawing S-0007, structural note SS-2 calls for nuts to be provided as A563 DH. It is industry standard, as well as permissible by both AISC and RCSC, for nuts to be supplied also as A194 2H. Please confirm this is acceptable.		ANSWER: Please see attached SK1 and note the following:  After reviewing structural design drawing S-0007, structural note SS-2 calls for nuts to be provided as A563 DH. It is industry standard, as well as permissible by both AISC and RCSC, for nuts to be supplied also as A194 2H. Please confirm this is acceptable.				
T-1690	SSS - W30 Connection Clarifications at W40 GL 33.2	Closed	CR	09/12/2014	09/22/2014	09/23/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: See attached CD RFI # 609 SK1: The W40x503's are web vertical and the W30x90's are canted to match the slope. This makes it difficult to supply double angle connections per 1/S1-6010 for both beams. Confirm it is acceptable to connect the W30x90 per 1/S1-5011at the noted locations.		ANSWER: See attached CD RFI # 609 SK1: The W40x503's are web vertical and the W30x90's are canted to match the slope. This makes it difficult to supply double angle connections per 1/S1-6010 for both beams. Confirm it is acceptable to connect the W30x90 per 1/S1-5011at the noted locations.				
T-1691	SSS - PE201 Connection Clarifications at Moment Symbols	Closed	CR	09/12/2014	09/22/2014	09/23/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: See attached CD RFI # 495 SK1: The moment symbol is shown at the noted locations while detail 12/S1-7604 is being referenced. Note that the moment symbols are not shown on plans 2,3/S1-7001 & 1,2,3/S1-7002 with detail 12/S1-7604 also being referenced. Confirm the moment symbols may be ignored and detail 12/S1-7604 applies at all locations where it is referenced.		ANSWER: See attached CD RFI # 495 SK1: The moment symbol is shown at the noted locations while detail 12/S1-7604 is being referenced. Note that the moment symbols are not shown on plans 2,3/S1-7001 & 1,2,3/S1-7002 with detail 12/S1-7604 also being referenced. Confirm the moment symbols may be ignored and detail 12/S1-7604 applies at all locations where it is referenced.				
T-1692	SSS - PE201 Missing Connection Details	Closed	CR	09/12/2014	09/22/2014	09/24/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: See attached CD RFI # 493 SK1 & SK2 for items 1 & 2:		ANSWER: See attached CD RFI # 493 SK1 & SK2 for items 1 &				



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	<p>1) Detail 12/S1-7604 cannot be applied at this corner because the column size is HSS6x6x1/4. See attached SK2 and supply a detail showing how to connect the members at this intersection. The same condition occurs in details 1,2,3/S1-7002 &amp; 1,2,3/S1-7003.</p> <p>2) Supply a slab edge detail.</p>					
			2:			
			<p>1) Detail 12/S1-7604 cannot be applied at this corner because the column size is HSS6x6x1/4. See attached SK2 and supply a detail showing how to connect the members at this intersection. The same condition occurs in details 1,2,3/S1-7002 &amp; 1,2,3/S1-7003.</p> <p>2) Supply a slab edge detail.</p>			
<b>T-1692.1</b>	<b>SSS - PE201 Missing Connection Details</b>	<b>Closed</b>	<b>CR</b>	<b>10/17/2014</b>	<b>10/17/2014</b>	<b>10/31/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>See attached CD RFI # 493.1 SK1 &amp; SK2 for items 1 to 3:</p> <p>1) The response in RFI T-1692 (SK 681, CD 493) is a revision to the contract documents as it is clearly stated in the details on S1-7001, S1-7002 &amp; S1-7003 that detail 12/S1-7604 is to be applied at 4 corners of PE201. The response will require revisions to the model and the material order.</p> <p>Please supply a splice location and a splice detail for the HSS6x6x1/4 column that is now to be continuous.</p> <p>2) Detail 8/S1-7604 cannot be applied as shown. Confirm it is acceptable to clip the corners as shown or supply a new detail.</p> <p>3) Details 6 &amp; 8/S1-7604 cannot be applied as shown. Confirm it is acceptable to cope the corners as shown or supply a new detail.</p>			<p>See attached CD RFI # 493.1 SK1 &amp; SK2 for items 1 to 3:</p> <p>1) The response in RFI T-1692 (SK 681, CD 493) is a revision to the contract documents as it is clearly stated in the details on S1-7001, S1-7002 &amp; S1-7003 that detail 12/S1-7604 is to be applied at 4 corners of PE201. The response will require revisions to the model and the material order.</p> <p>Please supply a splice location and a splice detail for the HSS6x6x1/4 column that is now to be continuous.</p> <p>2) Detail 8/S1-7604 cannot be applied as shown. Confirm it is acceptable to clip the corners as shown or supply a new detail.</p> <p>3) Details 6 &amp; 8/S1-7604 cannot be applied as shown. Confirm it is acceptable to cope the corners as shown or supply a new detail.</p>			







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	<p><b>REQUEST:</b></p> <p>Per S1-3282 detail 7 and section B sows continuous horizontal bars 3-#4 rebar each face and 2-#10 rebar in the fin wall with tails embedded into the scallop wall.</p> <p>Shimmick requests approval to have the option of a lap splice for the scallop wall to fin wall connection and or the use of Dayton Form Savers at the location. See attachment A.</p>					
	<p><b>ANSWER:</b></p> <p>Per S1-3282 detail 7 and section B sows continuous horizontal bars 3-#4 rebar each face and 2-#10 rebar in the fin wall with tails embedded into the scallop wall.</p> <p>Shimmick requests approval to have the option of a lap splice for the scallop wall to fin wall connection and or the use of Dayton Form Savers at the location. See attachment A.</p>					
<b>T-1697</b>	<b>SCS - Request To Use Snap Ties for Roof Top</b>	<b>Closed</b>	<b>01</b>	<b>09/15/2014</b>	<b>09/25/2014</b>	<b>10/01/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Claude Titche</p> <p><b>REQUEST:</b></p> <p>Per specification section 03 10 01 Part 2 - Products 7. H. Snap-Off Ties: Use for concrete walls below grade and walls which will not remain exposed to view and are not scheduled for architectural finishes.</p> <p>Drawing A 1-8608 shows the scallop and perimeter walls and A 1-8402 shows the skylight walls not being visible from the sides because of the fascia details and the earth backfill. These walls are not called out as architectural walls on the architectural drawings.</p> <p>Please confirm that typical snap ties are acceptable at perimeter wall, scallop wall, and sky light wall locations.</p>					
	<p><b>ANSWER:</b></p> <p>Per specification section 03 10 01 Part 2 - Products 7. H. Snap-Off Ties: Use for concrete walls below grade and walls which will not remain exposed to view and are not scheduled for architectural finishes.</p> <p>Drawing A 1-8608 shows the scallop and perimeter walls and A 1-8402 shows the skylight walls not being visible from the sides because of the fascia details and the earth backfill. These walls are not called out as architectural walls on the architectural drawings.</p> <p>Please confirm that typical snap ties are acceptable at perimeter wall, scallop wall, and sky light wall locations.</p>					





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<b>T-1698</b>	<b>SCS - Wall Opening Detail Conflicts for Horizontal and Diagonal Bars</b>	<b>Closed</b>	<b>01</b>	<b>09/15/2014</b>	<b>09/25/2014</b>	<b>09/18/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Please reference detail 4/S1-3207 and RFI T-1666 about foundation wall opening conflicts. Since submitting RFI T-1666, more concerns regarding 4/SI-3207 have been brought to our attention.  One of the concerns mentioned in RFI T-1666 was the case of additional vertical bars potentially conflicting with moment frame beam rebar. This issue can also occur with the diagonal cross bars and additional horizontal rebar.  1. Please provide an alternative detail for cases where either additional horizontal bars and diagonal cross bars conflict with moment frame beam rebar.  Due to the length of additional horizontal bars, wall openings near foundation wall CJs will cause the constructability issue of requiring rebar to be built out further to proceed with concrete pours.  2. Please confirm that the alternate wall opening detail (requested in RFI T-1666) for additional vertical bar lengths can also be applied to additional horizontal bars.						
						<b>ANSWER:</b>  Please reference detail 4/S1-3207 and RFI T-1666 about foundation wall opening conflicts. Since submitting RFI T-1666, more concerns regarding 4/SI-3207 have been brought to our attention.  One of the concerns mentioned in RFI T-1666 was the case of additional vertical bars potentially conflicting with moment frame beam rebar. This issue can also occur with the diagonal cross bars and additional horizontal rebar.  1. Please provide an alternative detail for cases where either additional horizontal bars and diagonal cross bars conflict with moment frame beam rebar.  Due to the length of additional horizontal bars, wall openings near foundation wall CJs will cause the constructability issue of requiring rebar to be built out further to proceed with concrete pours.  2. Please confirm that the alternate wall opening detail (requested in RFI T-1666) for additional vertical bar lengths can also be applied to additional horizontal bars.
<b>T-1699</b>	<b>SSS - Drag Beam Web Stiffener Plate Connection Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>09/15/2014</b>	<b>09/25/2014</b>	<b>09/26/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 572 SK1: Confirm it is acceptable to end the web stiffener plate as shown to avoid fouling the shear plate for the beam connection.						<b>ANSWER:</b>  See attached CD RFI # 572 SK1: Confirm it is acceptable to end the web stiffener plate as shown to avoid fouling the shear plate for the beam connection.
<b>T-1700</b>	<b>SSS - Shaw Alley Bent Plate Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>09/15/2014</b>	<b>09/25/2014</b>	<b>09/17/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 533 SK1 for item 2:  2) Confirm the 3/8" bent plate (50 ksi) terminates on top of						<b>ANSWER:</b>  See attached CD RFI # 533 SK1 for item 2:  2) Confirm the 3/8" bent plate (50 ksi) terminates on

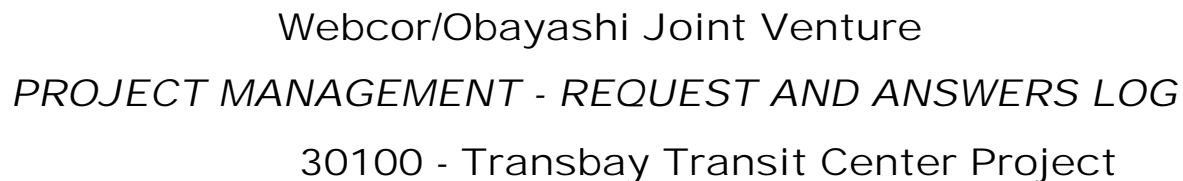


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T-1701	<b>SSS - Insufficient Cope for Erection Clearance</b>  From: Webcor Construction LP Gregory Kemerer  <b>REQUEST:</b>  Please refer to beams highlighted in S1-2503 (SK1) and work with this RFI.  The 1' 11 ½" cope is not sufficient for erection clearance as shown on A4644, A4645 (SK2, SK3). The Type 4 (V) drag connection on the other end of the beam limits the position in which the beam can be erected (See SK4, SK5). The beam needs to be erected from below and positioned through the shear plate on the moment frame column. As you slowly lift the back side that is currently coped, a ¼" clash transpires between the top flange of W40x327 and the shear plates on the bus deck casting.  Please advise.	Closed	CR	09/15/2014	09/25/2014	09/22/2014
						<b>ANSWER:</b>  Please refer to beams highlighted in S1-2503 (SK1) and work with this RFI.  The 1' 11 ½" cope is not sufficient for erection clearance as shown on A4644, A4645 (SK2, SK3). The Type 4 (V) drag connection on the other end of the beam limits the position in which the beam can be erected (See SK4, SK5). The beam needs to be erected from below and positioned through the shear plate on the moment frame column. As you slowly lift the back side that is currently coped, a ¼" clash transpires between the top flange of W40x327 and the shear plates on the bus deck casting.  Please advise.





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<b>T-1702.1</b>	<b>SSS - Weld Terminations on Moment Frame Column Shafts</b>	<b>Closed</b>	<b>CR</b>	<b>10/01/2014</b>	<b>10/11/2014</b>	<b>10/17/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  As a follow-up to SK RFI 821 (T-1702.1), please note the following:  It is unclear on how to exactly fix the weld terminations that currently do not conform with Section 5.17.1- Figure 5.2 from the AWS D1.1 Structural Welding Code.  Skanska proposes two options that would allow the transitions to be permissible as per AWS code. Option 1: Remove material from the web/CJP that would allow for a proper transition down to the flange. See SK1. Option 2: Add additional weld metal to the CJP that would allow for a proper transition down to the flange. See SK2.  Please confirm that both options are acceptable & it is at the fabricators discretion to choose which method.						
						<b>ANSWER:</b>  As a follow-up to SK RFI 821 (T-1702.1), please note the following:  It is unclear on how to exactly fix the weld terminations that currently do not conform with Section 5.17.1- Figure 5.2 from the AWS D1.1 Structural Welding Code.  Skanska proposes two options that would allow the transitions to be permissible as per AWS code. Option 1: Remove material from the web/CJP that would allow for a proper transition down to the flange. See SK1. Option 2: Add additional weld metal to the CJP that would allow for a proper transition down to the flange. See SK2.  Please confirm that both options are acceptable & it is at the fabricators discretion to choose which method.
<b>T-1703</b>	<b>SSS - Fouling Issues for CP8</b>	<b>Closed</b>	<b>CR</b>	<b>09/15/2014</b>	<b>09/25/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 611 SK1 to SK4 for items 1 to 4: 1) This hole at the bottom flange for CP8 fouls the full depth gusset plate for the CP6 brace per 4/S1-8003. Please provide a solution. 2) The noted hole at the bottom flange for CP8 fouls the full depth gusset plate for the W40x277 beam on Grid 1.4. Please provide a solution. 3) The noted 4 holes will not be accessible as they are within the plate closure for CP5 per 1/S1-8003. Please provide a solution. 4) The noted holes are too close to the 2" thick stiffeners for the CP5 connection per 1/S1-8003. Please provide a solution.						
						<b>ANSWER:</b>  See attached CD RFI # 611 SK1 to SK4 for items 1 to 4: 1) This hole at the bottom flange for CP8 fouls the full depth gusset plate for the CP6 brace per 4/S1-8003. Please provide a solution. 2) The noted hole at the bottom flange for CP8 fouls the full depth gusset plate for the W40x277 beam on Grid 1.4. Please provide a solution. 3) The noted 4 holes will not be accessible as they are within the plate closure for CP5 per 1/S1-8003. Please provide a solution. 4) The noted holes are too close to the 2" thick stiffeners for the CP5 connection per 1/S1-8003. Please provide a solution.
<b>T-1704</b>	<b>SSS - Roof Connection Discrepancy</b>	<b>Closed</b>	<b>CR</b>	<b>09/15/2014</b>	<b>09/25/2014</b>	<b>09/26/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						





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<b>T-1706</b>	<b>BGP - Construction Joint in Vehicle and Bike Ramp Walls</b>	<b>Closed</b>	<b>01</b>	<b>09/16/2014</b>	<b>09/26/2014</b>	<b>09/26/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Spec section 03 30 20-16-3.2 states that the maximum spacing for interior walls and ramp slab is 60 feet. In the response to the Area 1-4 and Area 5 Comprehensive Drawings, TT indicated that no movement joints were required in the Vehicle and Bicycle Ramps below the ramp.			Spec section 03 30 20-16-3.2 states that the maximum spacing for interior walls and ramp slab is 60 feet. In the response to the Area 1-4 and Area 5 Comprehensive Drawings, TT indicated that no movement joints were required in the Vehicle and Bicycle Ramps below the ramp.			
To facilitate construction, please confirm:			To facilitate construction, please confirm:			
1) The bicycle ramp and associated ramp walls can be poured monolithically from the start of the walls to the wall perpendicular to the ramp walls under the slab(See attached). Due to access issues, SCCI has to use lost formwork (styrofoam) for the underside of ramp formwork. SCCI will use this same form work for the inside of wall form.			1) The bicycle ramp and associated ramp walls can be poured monolithically from the start of the walls to the wall perpendicular to the ramp walls under the slab(See attached). Due to access issues, SCCI has to use lost formwork (styrofoam) for the underside of ramp formwork. SCCI will use this same form work for the inside of wall form.			
2) The maximum joint spacing for the Ramp and Ramp Walls can be increased to 90 feet as no movement joints are required below the vehicle or bike ramps.			2) The maximum joint spacing for the Ramp and Ramp Walls can be increased to 90 feet as no movement joints are required below the vehicle or bike ramps.			
<b>T-1707</b>	<b>BGP - Conflict Between Mezzanine and Vehicle Ramp Slab</b>	<b>Closed</b>	<b>01</b>	<b>09/17/2014</b>	<b>09/27/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached drawing showing the Mezzanine Slab conflicting with the Vehicle Ramp Slab. There is nothing in the contract drawings showing a detail of this intersection, or acknowledging this intersection occurs. Please confirm this intersection of the Mezzanine and Vehicle Ramp Slab is the designer's intent. If this is the designer's intent, please provide further drawing details on the interface of the two components.			See attached drawing showing the Mezzanine Slab conflicting with the Vehicle Ramp Slab. There is nothing in the contract drawings showing a detail of this intersection, or acknowledging this intersection occurs. Please confirm this intersection of the Mezzanine and Vehicle Ramp Slab is the designer's intent. If this is the designer's intent, please provide further drawing details on the interface of the two components.			



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<b>T-1708</b>	<b>BGP - Structural Detail for Mezzanine to Foundation Wall Connection</b>	<b>Closed</b>	<b>01</b>	<b>09/17/2014</b>	<b>09/27/2014</b>	<b>09/25/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached marked up drawing SI-2250 showing Shimmick's assumption of which drawing details apply to the various edges on the Mezzanine Slab. There does not appear to be a structural detail for the connection between the Foundation Wall and Mezzanine Slab. Please provide a structural detail showing the connection between the Mezzanine Slab and Foundation Wall.		See attached marked up drawing SI-2250 showing Shimmick's assumption of which drawing details apply to the various edges on the Mezzanine Slab. There does not appear to be a structural detail for the connection between the Foundation Wall and Mezzanine Slab. Please provide a structural detail showing the connection between the Mezzanine Slab and Foundation Wall.				
<b>T-1709</b>	<b>BGP - Area 16 Placement Issues-West Side of Seismic Joint</b>	<b>Closed</b>	<b>01</b>	<b>09/18/2014</b>	<b>09/28/2014</b>	<b>09/19/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Detail 4/S1-3010 indicates the threading of the (3) #8 Horizontal Rebar through various closed loops of reinforcing created by the mat reinforcing intersecting welded joint reinforcing directly below the top clamp assembly on the west side of the seismic joint. Please confirm the following that has been discussed with the Design Team's Structural Field Representative and included in the attached sketch:  1) The topmost #8's below the top clamp assembly may be a continuous #5 in lieu.  2) The middle and bottom #8's below the top clamp assembly contained within the U-bar need not be continuous but may be placed end to end with a 0" splice. Each segment of #8 shall be positioned as indicated in detail 4/S 1-3010 and shall continuously engage no fewer than ( 4) #8 U-bars. 0" splices shall be located midway between adjacent U-Bars.		Detail 4/S1-3010 indicates the threading of the (3) #8 Horizontal Rebar through various closed loops of reinforcing created by the mat reinforcing intersecting welded joint reinforcing directly below the top clamp assembly on the west side of the seismic joint. Please confirm the following that has been discussed with the Design Team's Structural Field Representative and included in the attached sketch:  1) The topmost #8's below the top clamp assembly may be a continuous #5 in lieu.  2) The middle and bottom #8's below the top clamp assembly contained within the U-bar need not be continuous but may be placed end to end with a 0" splice. Each segment of #8 shall be positioned as indicated in detail 4/S 1-3010 and shall continuously engage no fewer than ( 4) #8 U-bars. 0" splices shall be located midway between adjacent U-Bars.				
<b>T-1710</b>	<b>SCS - Roof Scallop Wall Construction Joint Relocation</b>	<b>Closed</b>	<b>CR</b>	<b>09/19/2014</b>	<b>09/29/2014</b>	<b>09/26/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Per drawing SI-3282 detail 4 and 5 the base of the scallop wall requires a chamfered starter wall that transitions from		Per drawing SI-3282 detail 4 and 5 the base of the scallop wall requires a chamfered starter wall that				



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	<p>a 1 to 1 angle, 3 to 2 angle, and a 4 to 1 angle throughout the geometry of the scallops.Shimmick requests approval to relocate the construction joint from the wall to the deck and pour the full height of the wall including the deck beneath. See attachment "A" for clarification. This change would only occur at the scallops and detail 2 "Typical walls at the roof north &amp; south edges" would not change.</p> <p>Please confirm the joint location is acceptable.</p>					<p>transitions from a 1 to 1 angle, 3 to 2 angle, and a 4 to 1 angle throughout the geometry of the scallops.Shimmick requests approval to relocate the construction joint from the wall to the deck and pour the full height of the wall including the deck beneath. See attachment "A" for clarification. This change would only occur at the scallops and detail 2 "Typical walls at the roof north &amp; south edges" would not change.</p> <p>Please confirm the joint location is acceptable.</p>
T-1711	SCS - Roof Shear Key and Pylon Rebar Epoxy Connection	Closed	CR	09/19/2014	09/29/2014	09/29/2014
From: Webcor Construction LP                      Stephanie Azzolino						
REQUEST:		ANSWER:				
<p>Per S1-3282 detail 9 "SHEAR KEY AT ROOF" calls for vertical #4 rebar dowels to be cast with the structural deck. Per S1-3281 detail 7 "SIGNAGE PYLON FOOTING DETAIL" calls for vertical #5 rebar dowels to be cast with structural deck. For constructability reasons it is essential that Shimmick's operations maintain open access for roof top small cranes and man lifts for all trades. Per specification section 03 20 02 Part 2.3 F. 3. Adhesive for dowels in existing concrete b. HIT-RE 500_SD injectable adhesive by HIL TI, Inc., is an approved method. The minimum embedment depth requirement for #4 rebar is 2.75 inches and the concrete thickness requirement is 4 inches at the shear keys. The minimum embedment depth requirement for #5 rebar is 3.125 inches and the concrete thickness requirement is 4.375 inches for the signage pylon. The roof deck is 7" with 3" tall flutes leaving a clear space of 4 inches. Shimmick's solution for the 4.375 inch concrete thickness requirement is to pour an additional .5 inches of concrete at the concrete deck where the pylons will be installed. See drawing attachment "A" and product data attachment "B" on page 4 for rebar drill and epoxy requirements.</p> <p>Shimmick requests approval for the shear keys and the pylons be drill and epoxy rebar doweled connections after the roof top deck has been poured and larger operations have been completed.</p>		<p>Per S1-3282 detail 9 "SHEAR KEY AT ROOF" calls for vertical #4 rebar dowels to be cast with the structural deck. Per S1-3281 detail 7 "SIGNAGE PYLON FOOTING DETAIL" calls for vertical #5 rebar dowels to be cast with structural deck. For constructability reasons it is essential that Shimmick's operations maintain open access for roof top small cranes and man lifts for all trades. Per specification section 03 20 02 Part 2.3 F. 3. Adhesive for dowels in existing concrete b. HIT-RE 500_SD injectable adhesive by HIL TI, Inc., is an approved method. The minimum embedment depth requirement for #4 rebar is 2.75 inches and the concrete thickness requirement is 4 inches at the shear keys. The minimum embedment depth requirement for #5 rebar is 3.125 inches and the concrete thickness requirement is 4.375 inches for the signage pylon. The roof deck is 7" with 3" tall flutes leaving a clear space of 4 inches. Shimmick's solution for the 4.375 inch concrete thickness requirement is to pour an additional .5 inches of concrete at the concrete deck where the pylons will be installed. See drawing attachment "A" and product data attachment "B" on page 4 for rebar drill and epoxy requirements.</p> <p>Shimmick requests approval for the shear keys and the pylons be drill and epoxy rebar doweled</p>				





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					connections after the roof top deck has been poured and larger operations have been completed.	
T-1712	BGP - Bike Ramp Blockout for Steel Column at GL 5-G.7	Closed	CR	09/19/2014	09/29/2014	10/01/2014
From: Webcor Construction LP Claude Titcher						
REQUEST:						ANSWER:
The structural details for the blackout in the Bike Ramp for the steel column at GL 5-G.7 is called out in S1-2202. The drawing calls out for the ramp walls to be blocked out 2'-0" to the east and west of Gridline 5.						The structural details for the blackout in the Bike Ramp for the steel column at GL 5-G.7 is called out in S1-2202. The drawing calls out for the ramp walls to be blocked out 2'-0" to the east and west of Gridline 5.
The architectural details for the blackout in the Bike Ramp for the steel column at GL 5-G.7 is called out in A 1-2202 and A 1-9209. The drawings on A 1-9209 show a plan view detail at 3 different levels with no supplementary elevation views. Detail 4-A 1-9209 appears to call out for the ramp walls to be blocked out 2'-1" to the east and west of Gridline 5 at the base of the column. Detail 5-A 1-9209 appears to call out for the ramp walls and slab at the ramp slab level to be blocked out 1'-1 1/2" to the east and west and 1'-2" to the south of the center of the column and perpendicular to the ramp wall.						The architectural details for the blackout in the Bike Ramp for the steel column at GL 5-G.7 is called out in A 1-2202 and A 1-9209. The drawings on A 1-9209 show a plan view detail at 3 different levels with no supplementary elevation views. Detail 4-A 1-9209 appears to call out for the ramp walls to be blocked out 2'-1" to the east and west of Gridline 5 at the base of the column. Detail 5-A 1-9209 appears to call out for the ramp walls and slab at the ramp slab level to be blocked out 1'-1 1/2" to the east and west and 1'-2" to the south of the center of the column and perpendicular to the ramp wall.
Please provide an elevation detail for the column-ramp interface at GL 5-G.7 that clarifies the discrepancy between S 1-2202 and A1-9209 as well as clarifies the elevation at which the wall blackout changes from Detail 4-A1-9209 to Detail 5-A1-9209 (if A1-9209 is correct).						Please provide an elevation detail for the column-ramp interface at GL 5-G.7 that clarifies the discrepancy between S 1-2202 and A1-9209 as well as clarifies the elevation at which the wall blackout changes from Detail 4-A1-9209 to Detail 5-A1-9209 (if A1-9209 is correct).



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<b>T-1713</b>	<b>SSS - Location of BU-40 Splice Locations</b>	<b>Closed</b>	<b>CR</b>	<b>09/19/2014</b>	<b>09/29/2014</b>	<b>09/29/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 615 SK1 & SK2:  Current splice locations for BU-40 members foul beam connections. Please confirm it is acceptable to relocate the splices as shown.						<b>ANSWER:</b>  See attached CD RFI # 615 SK1 & SK2:  Current splice locations for BU-40 members foul beam connections. Please confirm it is acceptable to relocate the splices as shown.
<b>T-1713.1</b>	<b>SSS - Location of BU-40 Splice Locations</b>	<b>Closed</b>	<b>CR</b>	<b>10/07/2014</b>	<b>10/17/2014</b>	<b>10/16/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 615 SK1 & SK2: Following confirmation from design team on moving splice locations, we have learned that the affected girder blanks have already been fabricated and cut to length. Confirm it is acceptable to relocate the W40 x 264 8" south and the W16 x 26 7 11/16" north.						<b>ANSWER:</b>  See attached CD RFI # 615 SK1 & SK2: Following confirmation from design team on moving splice locations, we have learned that the affected girder blanks have already been fabricated and cut to length. Confirm it is acceptable to relocate the W40 x 264 8" south and the W16 x 26 7 11/16" north.
<b>T-1714</b>	<b>SCS - Roof Seismic Joint Gap 22" or 24"</b>	<b>Closed</b>	<b>01</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>09/22/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  The roof top seismic joint shown on sheets AI-2603, AI-8897 detail 2 and AI-8898 detail 1 section elevation shows the gap between the cast in place walls at line 10 and at line 20 to be 24 inches. A1- 8613 detail 1 is a section view of the seismic joint for the exterior wall for line 10 & 20 shows the gap between the cast in place wall at line 10 and at line 20 to be 22 inches.  Please clarify if the gap between the cast in place walls at the seismic joints needs to be 24 inches or 22 inches.						<b>ANSWER:</b>  The roof top seismic joint shown on sheets AI-2603, AI-8897 detail 2 and AI-8898 detail 1 section elevation shows the gap between the cast in place walls at line 10 and at line 20 to be 24 inches. A1- 8613 detail 1 is a section view of the seismic joint for the exterior wall for line 10 & 20 shows the gap between the cast in place wall at line 10 and at line 20 to be 22 inches.  Please clarify if the gap between the cast in place walls at the seismic joints needs to be 24 inches or 22 inches.
<b>T-1715</b>	<b>SCS - Foundation Wall Mix Design</b>	<b>Closed</b>	<b>01</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>09/23/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	<p>The answer to QBD 18 for the TG07.2 Trade Package, which asked if the "slab and beam mix can be used for the fourth wall lift", was "Confirmed. The slab and beam mix can be used for the fourth wall lift." Submittal TG0702-118 submitted the slab and beam mix and included the foundation wall as one of the usage locations. The review comment stated "Mix design for Foundations Walls shall be as approved in Submittal TG0600-203.I. Omission of SRA is not acceptable" which is the Foundation Wall mix.</p> <p>Please confirm if the Submittal Response to use the Foundation Wall Mix is superseding the answer to QBD 18 which confirmed the use of the Slab and Beam Mix design on the fourth lift foundation wall.</p>					<p>The answer to QBD 18 for the TG07.2 Trade Package, which asked if the "slab and beam mix can be used for the fourth wall lift", was "Confirmed. The slab and beam mix can be used for the fourth wall lift." Submittal TG0702-118 submitted the slab and beam mix and included the foundation wall as one of the usage locations. The review comment stated "Mix design for Foundations Walls shall be as approved in Submittal TG0600-203.I. Omission of SRA is not acceptable" which is the Foundation Wall mix.</p> <p>Please confirm if the Submittal Response to use the Foundation Wall Mix is superseding the answer to QBD 18 which confirmed the use of the Slab and Beam Mix design on the fourth lift foundation wall.</p>
<b>T-1716</b>	<b>SSS - Field Weld Shear Plate Detail</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>10/03/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> <p>Detail 3/S1-7632 requires a field fillet weld for the shear plate to beam connection. Positioning an electrode or gooseneck to achieve the 7/16" weld on the back side will not be possible in the field due to the limited gap between the weld and vertical HSS member.</p> <p>Please confirm it is acceptable to weld the shear plate to beam web on three sides on the near side and omit the far side weld. See attached SK1 for clarification.</p>						<b>ANSWER:</b> <p>Detail 3/S1-7632 requires a field fillet weld for the shear plate to beam connection. Positioning an electrode or gooseneck to achieve the 7/16" weld on the back side will not be possible in the field due to the limited gap between the weld and vertical HSS member.</p> <p>Please confirm it is acceptable to weld the shear plate to beam web on three sides on the near side and omit the far side weld. See attached SK1 for clarification.</p>



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T-1716.1	<b>SSS - Field Weld Shear Plate Detail</b>	Closed	CR	10/03/2014	10/13/2014	10/16/2014
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Following further review of this detail it was found the varying web thickness of the framing beams are significantly less than the 7/16" fillet required as per 3/S1-7632: Typical framing beams are as follows: - W12x44 - tw 3/16" (max 3/16" fillet as per D1.1 Fig 2.1) - W16x26 - tw 1/4" (max 3/16" fillet as per D1.1 Fig 2.1) - W18x65 - tw 7/16" (max 3/8" fillet as per D1.1 Fig 2.1) - W21x44 - tw 3/8" (max 5/16" fillet as per D1.1 Fig 2.1)  Confirm these weld sizes are acceptable or alternatively if welding the shear tab on 3 sides (7/16" fillet) as originally requested is acceptable.			Following further review of this detail it was found the varying web thickness of the framing beams are significantly less than the 7/16" fillet required as per 3/S1-7632: Typical framing beams are as follows: - W12x44 - tw 3/16" (max 3/16" fillet as per D1.1 Fig 2.1) - W16x26 - tw 1/4" (max 3/16" fillet as per D1.1 Fig 2.1) - W18x65 - tw 7/16" (max 3/8" fillet as per D1.1 Fig 2.1) - W21x44 - tw 3/8" (max 5/16" fillet as per D1.1 Fig 2.1)  Confirm these weld sizes are acceptable or alternatively if welding the shear tab on 3 sides (7/16" fillet) as originally requested is acceptable.			
T-1717	<b>SSS - Missing Weld Details at GL Cast Node</b>	Closed	CR	09/22/2014	10/02/2014	10/03/2014
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 619 SK1 & SK2 for items 1 to 3: 1) All welds shown in red are from details on drawing S1-4351. Confirm they are acceptable where shown or supply the missing welds. 2) Provide weld for W14 to cast node. 3) All welds shown in red are from details on drawing S1-4351. Confirm they are acceptable where shown or supply the missing welds.			See attached CD RFI # 619 SK1 & SK2 for items 1 to 3: 1) All welds shown in red are from details on drawing S1-4351. Confirm they are acceptable where shown or supply the missing welds. 2) Provide weld for W14 to cast node. 3) All welds shown in red are from details on drawing S1-4351. Confirm they are acceptable where shown or supply the missing welds.			
T-1718	<b>SSS - LC Beam Flange Location Verification at EOS</b>	Closed	CR	09/22/2014	10/02/2014	10/03/2014
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached sketches CD RFI 607 SK1 & SK2 and confirm acceptable the beam flange will protrude past the curved edge of slab by 5/16".			See attached sketches CD RFI 607 SK1 & SK2 and confirm acceptable the beam flange will protrude past the curved edge of slab by 5/16".			



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<b>T-1719</b>	<b>SSS - LC Connection Verifications at Skewed Beams</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached sketches CD RFI 608 SK1 & SK2 and verify / confirm proposed connections for skewed beams.		<b>ANSWER:</b>  See attached sketches CD RFI 608 SK1 & SK2 and verify / confirm proposed connections for skewed beams.				
<b>T-1720</b>	<b>SSS - W12 Connection Clarifications at Roof Level GL 31.7-33.2</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 613 SK1: Confirm the locations for the W12x305's may be modified as shown as hi-lited in order to connect the beams to the stiffeners in detail 1/S1-4205 with bolts per 1/S1-5011.		<b>ANSWER:</b>  See attached CD RFI # 613 SK1: Confirm the locations for the W12x305's may be modified as shown as hi-lited in order to connect the beams to the stiffeners in detail 1/S1-4205 with bolts per 1/S1-5011.				
<b>T-1721</b>	<b>SSS - MF Beam Connection Clarification GL 24</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 617 SK1 & SK2: The radius is to be 1'-1 per 7/S1-5027 but due to the flange thickness of the MF beam on Grid 24, the maximum radius on the plate is 1'-0 1/2. Confirm the connection is acceptable as shown or supply an alternate solution.		<b>ANSWER:</b>  See attached CD RFI # 617 SK1 & SK2: The radius is to be 1'-1 per 7/S1-5027 but due to the flange thickness of the MF beam on Grid 24, the maximum radius on the plate is 1'-0 1/2. Confirm the connection is acceptable as shown or supply an alternate solution.				
<b>T-1722</b>	<b>SSS - PE403 &amp; PE404 Conflicting Approval Information From CS7 Package</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 620 SK1: The dimension between the edge of slab and center of beam on drawing 3490 Section A-A & B-B and drawing 4151 Section A-A in Package Number: TG0701-079.1 has been changed from 7" to 5". This does not agree with the latest issued drawings as shown on attached SK1, which shows the edge of slab to center of beam as 7" on the north, east and south sides. Note that drawings 3486, 3492, 3493, 4095, 9044 & 9083 in the same area have not been marked at approval.		<b>ANSWER:</b>  See attached CD RFI # 620 SK1: The dimension between the edge of slab and center of beam on drawing 3490 Section A-A & B-B and drawing 4151 Section A-A in Package Number: TG0701-079.1 has been changed from 7" to 5". This does not agree with the latest issued drawings as shown on attached SK1, which shows the edge of slab to center of beam as 7" on the north, east and south sides. Note that drawings 3486, 3492, 3493, 4095, 9044 & 9083 in the same area have not been marked				



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<b>T-1725</b>	<b>SSS - Missing Skewed Connection Details at Light Column</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 612 SK1: Details 7 & 8/S1-5010 do not give the necessary information for the connection when skewed connections occur on both sides. This information was requested in RFI T-1087 (SK 122, CD 094) but the requested typical connection information was not provided.  Confirm the connection as shown is acceptable.			See attached CD RFI # 612 SK1: Details 7 & 8/S1-5010 do not give the necessary information for the connection when skewed connections occur on both sides. This information was requested in RFI T-1087 (SK 122, CD 094) but the requested typical connection information was not provided.  Confirm the connection as shown is acceptable.			
<b>T-1725.1</b>	<b>SSS - Missing Skewed Connection Details at Light Column</b>	<b>Closed</b>	<b>CR</b>	<b>10/07/2014</b>	<b>10/17/2014</b>	<b>10/10/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The previous response to this RFI (T-1725) does not take into account the difference in geometry between the light column and W27x84 on the north side of GL E when compared to the W27x84 on the south side of GL E. Please refer to SK RFI 052.1 (T-0770.1), which provided different dimensions for the two locations. In addition, this connection clarification was requested in SK RFI 122 (T-1087).  See attached CD RFI # 612 SK1: Details 7 & 8/S1-5010 do not give the necessary information for the connection when skewed connections occur on both sides. This information was requested in RFI T-1087 (SK 122, CD 094) but the requested typical connection information was not provided. Confirm the connection as shown is acceptable.			The previous response to this RFI (T-1725) does not take into account the difference in geometry between the light column and W27x84 on the north side of GL E when compared to the W27x84 on the south side of GL E. Please refer to SK RFI 052.1 (T-0770.1), which provided different dimensions for the two locations. In addition, this connection clarification was requested in SK RFI 122 (T-1087).  See attached CD RFI # 612 SK1: Details 7 & 8/S1-5010 do not give the necessary information for the connection when skewed connections occur on both sides. This information was requested in RFI T-1087 (SK 122, CD 094) but the requested typical connection information was not provided. Confirm the connection as shown is acceptable.			
<b>T-1726</b>	<b>SCS - Roof Seismic Connection Details</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>09/25/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
After reviewing the seismic joint details on sheets AI-2603, AI-8897 detail 2, AI-8898 detail 1, and AI-8613 detail 1 it is Shimmick's understanding that the seismic joint system only requires a vertical wall corner embedded angle at building lines B 10 & 20 and H 10 & 20. All other means of			After reviewing the seismic joint details on sheets AI-2603, AI-8897 detail 2, AI-8898 detail 1, and AI-8613 detail 1 it is Shimmick's understanding that the seismic joint system only requires a vertical wall corner embedded angle at building lines B 10 & 20			





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	<div>attachment for the seismic joint will be drill anchors at a later date.</div> <div>Please confirm.</div>					<div>and H 10 &amp; 20. All other means of attachment for the seismic joint will be drill anchors at a later date.</div> <div>Please confirm.</div>
T-1727	SCS - Roof Sensors, Sky Light Drain, and Hose Bib Deck Connection	Closed	CR	09/22/2014	10/02/2014	10/13/2014
	<div>From: Webcor Construction LP</div> <div>Stephanie Azzolino</div>					
	<div>REQUEST:</div> <div>Drawing L-2602 to 2627 shows moisture sensors, skylight one trench drain, and hose bibs. The drawings do not show the details for these items and whether or not they require a rebar or embedded anchor connection to the roof top structural deck.</div> <div>Please provide details and whether or not they require a cast in place connection to the roof top deck.</div>					<div>ANSWER:</div> <div>Drawing L-2602 to 2627 shows moisture sensors, skylight one trench drain, and hose bibs. The drawings do not show the details for these items and whether or not they require a rebar or embedded anchor connection to the roof top structural deck.</div> <div>Please provide details and whether or not they require a cast in place connection to the roof top deck.</div>
T-1728	SCS - Roof Top Pylon Rebar Connection	Closed	CR	09/22/2014	10/02/2014	09/30/2014
	<div>From: Webcor Construction LP</div> <div>Stephanie Azzolino</div>					
	<div>REQUEST:</div> <div>S1-3281 detail 4 shows the roof top security pole with a bolted connection to the structural deck. Yet all security and lighting references on A1-2602 to A1-2607 and E1-2602 to E1-2607 are shown as pylons which is consistent with detail 7 that was added per addendum 5 issue for construction.</div> <div>Please identify where detail 4 is used on the A1-2602 to A1-2607 roof park level plan sheets.</div> <div>For constructability reasons it is essential that access remains open for roof top operations with small cranes and man lifts for all trades.Shimmick requests approval to use drill and epoxy rebar connections after the roof top deck has been poured for the detail 7 pylon.</div>					<div>ANSWER:</div> <div>S1-3281 detail 4 shows the roof top security pole with a bolted connection to the structural deck. Yet all security and lighting references on A1-2602 to A1-2607 and E1-2602 to E1-2607 are shown as pylons which is consistent with detail 7 that was added per addendum 5 issue for construction.</div> <div>Please identify where detail 4 is used on the A1-2602 to A1-2607 roof park level plan sheets.</div> <div>For constructability reasons it is essential that access remains open for roof top operations with small cranes and man lifts for all trades.Shimmick requests approval to use drill and epoxy rebar connections after the roof top deck has been poured for the detail 7 pylon.</div>
T-1729	SCS - Roof Walking Path Foundation Rebar Connection	Closed	CR	09/22/2014	10/02/2014	10/07/2014





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<div><div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div><div><b>REQUEST:</b><p>Per Drawing L1-2625 and L1-7661 detail 2 precast concrete walking path subslab units are supported by a CMU wall on a CMU support footing on a protection slab (all by others) and presumable on the roof top slab. This detail does not show rebar.</p><p>Please confirm the support wall footing/protection slab does not require rebar to be embedded into the roof top structural deck.</p></div><div><b>ANSWER:</b><p>Per Drawing L1-2625 and L1-7661 detail 2 precast concrete walking path subslab units are supported by a CMU wall on a CMU support footing on a protection slab (all by others) and presumable on the roof top slab. This detail does not show rebar.</p><p>Please confirm the support wall footing/protection slab does not require rebar to be embedded into the roof top structural deck.</p></div></div>						
<b>T-1730</b>	<b>SCS - Roof Top CIPAC Walls</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>09/26/2014</b>
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div><div><b>REQUEST:</b><p>Per specification section 03 33 00 Part 1 - 1.1 SUMMARY B. The extent of Cast-In-Place Architectural Concrete is indicated on the Drawings by the designation "CIPAC" A1-8645 to A1-8654 shows architectural concrete walls but Shimmick has not found the notation "CIPAC" for the architectural concrete walls.</p><p>Please clarify if any additional roof top walls fall with in "CIPAC" requirements.</p></div><div><b>ANSWER:</b><p>Per specification section 03 33 00 Part 1 - 1.1 SUMMARY B. The extent of Cast-In-Place Architectural Concrete is indicated on the Drawings by the designation "CIPAC" A1-8645 to A1-8654 shows architectural concrete walls but Shimmick has not found the notation "CIPAC" for the architectural concrete walls.</p><p>Please clarify if any additional roof top walls fall with in "CIPAC" requirements.</p></div></div>						
<b>T-1731</b>	<b>SCS - Roof Top Park Building Rebar Connection to Structural Deck</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>09/23/2014</b>
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div><div><b>REQUEST:</b><p>Architectural drawings A1-2602 to A1-2603 &amp; A1-6012 Note: "Roof Park building not in current scope of work. Refer to mechanical, electrical, plumbing and telecom drawings for extent of work to be included in this package." Structural drawings S1-2602 and S1-4103 show minimal details for roof park building walls.</p><p>Please provide, if any, rebar needs in the roof structural deck placement for the roof park building.</p></div><div><b>ANSWER:</b><p>Architectural drawings A1-2602 to A1-2603 &amp; A1-6012 Note: "Roof Park building not in current scope of work. Refer to mechanical, electrical, plumbing and telecom drawings for extent of work to be included in this package." Structural drawings S1-2602 and S1-4103 show minimal details for roof park building walls.</p><p>Please provide, if any, rebar needs in the roof structural deck placement for the roof park building.</p></div></div>						
<b>T-1732</b>	<b>SCS - Roof Circular Cafe Foundation Rebar Connection to Deck</b>	<b>Closed</b>	<b>CR</b>	<b>09/22/2014</b>	<b>10/02/2014</b>	<b>09/23/2014</b>





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<b>T-1733.1</b>	<b>SCS - Bus Deck Embeds: Hose Bibb HB-4 at GL 10-13</b>	<b>Closed</b>	<b>01</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>10/13/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference RFI T-1733 and also attached marked up drawings.			Please reference RFI T-1733 and also attached marked up drawings.			
Drawing PI-2503 notes a hose bibb type HB-4 which leads directly to a 3/4" CW pipe through the bus deck level. However as this is a 3", this sleeve is not displayed in drawing AI-2893 which does not locate sleeves smaller than 4".			Drawing PI-2503 notes a hose bibb type HB-4 which leads directly to a 3/4" CW pipe through the bus deck level. However as this is a 3", this sleeve is not displayed in drawing AI-2893 which does not locate sleeves smaller than 4".			
Please provide the dimensions for locating the noted bus deck sleeve.			Please provide the dimensions for locating the noted bus deck sleeve.			
<b>T-1734</b>	<b>SCS - Roof Architectural Stair 301 Joint Orientation</b>	<b>Closed</b>	<b>CR</b>	<b>09/24/2014</b>	<b>10/04/2014</b>	<b>09/29/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Specification Section 03 33 00 - 2.2 A. 3. furnish sizes required by joint pattern indicated on Drawings. Per A1-2603 stair structure 301/roof park landform the wall elevations are called out on A1-8645. A1-8645 detail 3 and 4 shows the high density overlay plywood layout as being vertical. Both detail 3 and 4 refer you to section 2 and section 2 refers you to elevation 1. Elevation 1 shows the high density overlay plywood layout as being horizontal. Shimmick intends to use full height aluminum gang panels skinned with the high density overlay plywood running vertical.			Specification Section 03 33 00 - 2.2 A. 3. furnish sizes required by joint pattern indicated on Drawings. Per A1-2603 stair structure 301/roof park landform the wall elevations are called out on A1-8645. A1-8645 detail 3 and 4 shows the high density overlay plywood layout as being vertical. Both detail 3 and 4 refer you to section 2 and section 2 refers you to elevation 1. Elevation 1 shows the high density overlay plywood layout as being horizontal. Shimmick intends to use full height aluminum gang panels skinned with the high density overlay plywood running vertical.			
Please confirm that markings from vertical plywood joints (as shown in 3/A1-8645) is the correct interpretation.			Please confirm that markings from vertical plywood joints (as shown in 3/A1-8645) is the correct interpretation.			
<b>T-1735</b>	<b>SCS - Roof Architectural Stair 301 Joint Chamfer</b>	<b>Closed</b>	<b>CR</b>	<b>09/24/2014</b>	<b>10/04/2014</b>	<b>09/29/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
A1-8645 shows horizontal and vertical lines between plywood sheets bolder than what is created by plywood alone. Specification Section 03 33 00 - 2.2 B Chamfer Strips: calls for 3/4" chamfer to be "used for horizontal			A1-8645 shows horizontal and vertical lines between plywood sheets bolder than what is created by plywood alone. Specification Section 03 33 00 - 2.2 B Chamfer Strips: calls for 3/4" chamfer to be "used for			



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	<p>joints between pours" Shimmick is assuming that the vertical lines are chamfer strips as well. Vertical chamfer strips would provide a weakened plane and help confine any vertical shrinkage cracks to the chamfer location.</p> <p>1. Please confirm only vertical 3/4" chamfer strips at the plywood joints is being depicted by the wall elevations on A1-8645. 2. Please provide additional direction if anything is required at the vertical and horizontal joints.</p>					<p>horizontal joints between pours" Shimmick is assuming that the vertical lines are chamfer strips as well. Vertical chamfer strips would provide a weakened plane and help confine any vertical shrinkage cracks to the chamfer location.</p> <p>1. Please confirm only vertical 3/4" chamfer strips at the plywood joints is being depicted by the wall elevations on A1-8645. 2. Please provide additional direction if anything is required at the vertical and horizontal joints.</p>
<b>T-1736</b>	<b>SCS - Roof Architectural Stair 301 Construction Joint Spacing</b>	<b>Closed</b>	<b>CR</b>	<b>09/24/2014</b>	<b>10/04/2014</b>	<b>10/13/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> <p>Specification Section 03 33 00 - 3.4 A. "Install joints..., at locations indicated on Drawings or as approved by TJPA Representative." Construction joints are not called out on A1-2603, A1-8645, or S1-2603.</p> <p>Please confirm there is no restriction for the maximum construction joint spacing for architectural wall, otherwise provide a maximum allowable length for architectural wall pours.</p>						<b>ANSWER:</b> <p>Specification Section 03 33 00 - 3.4 A. "Install joints..., at locations indicated on Drawings or as approved by TJPA Representative." Construction joints are not called out on A1-2603, A1-8645, or S1-2603.</p> <p>Please confirm there is no restriction for the maximum construction joint spacing for architectural wall, otherwise provide a maximum allowable length for architectural wall pours.</p>
<b>T-1737</b>	<b>SCS - Joint Spacing for Roof Top Earth Retaining Walls</b>	<b>Closed</b>	<b>CR</b>	<b>09/24/2014</b>	<b>10/04/2014</b>	<b>10/07/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> <p>03 30 02 - 3.2 JOINTS IN CONCRETE A. 3. "Maximum construction joint spacing in foundation wall, ground floor concrete slab, and interior walls is 60 feet." This statement does not make reference to roof top earth retaining non-architectural walls. S1-2602 through 7 and A1-2602 through 7 does not identify joint locations for the perimeter/scallop, seismic, sky light, and stair/elevator walls.</p> <p>Please provide maximum length of walls for roof top earth retaining non-architectural walls.</p>						<b>ANSWER:</b> <p>03 30 02 - 3.2 JOINTS IN CONCRETE A. 3. "Maximum construction joint spacing in foundation wall, ground floor concrete slab, and interior walls is 60 feet." This statement does not make reference to roof top earth retaining non-architectural walls. S1-2602 through 7 and A1-2602 through 7 does not identify joint locations for the perimeter/scallop, seismic, sky light, and stair/elevator walls.</p> <p>Please provide maximum length of walls for roof top earth retaining non-architectural walls.</p>



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<b>T-1738</b>	<b>SCS - Construction Joint Relocation at Roof Seismic Walls</b>	<b>Closed</b>	<b>CR</b>	<b>09/24/2014</b>	<b>10/04/2014</b>	<b>10/07/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  S1-3282 detail 3 Concrete wall detail at seismic joint. Shows a construction joint above the chamfer. This seismic wall must accommodate both the roof top structural deck change in elevation of 1'-1 1/2" and a top of wall continuous elevation with a pop out detail. Per these restrictions, the control and change in elevation must occur at the bottom of the wall form and requires a starter wall above the chamfer. Refer to drawing attachment A for further explanation.  Please confirm that a starter wall is acceptable.						<b>ANSWER:</b>  S1-3282 detail 3 Concrete wall detail at seismic joint. Shows a construction joint above the chamfer. This seismic wall must accommodate both the roof top structural deck change in elevation of 1'-1 1/2" and a top of wall continuous elevation with a pop out detail. Per these restrictions, the control and change in elevation must occur at the bottom of the wall form and requires a starter wall above the chamfer. Refer to drawing attachment A for further explanation.  Please confirm that a starter wall is acceptable.
<b>T-1739</b>	<b>SCS - Roof Top Exterior Walls and Scallop Walls Updated Drawings</b>	<b>Closed</b>	<b>CR</b>	<b>09/24/2014</b>	<b>10/04/2014</b>	<b>09/30/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  Per Meeting with WOJV and Designers on 9/2/2014 regarding options for the scallop wall changes in ASI 124, additional changes in the design for the rooftop exterior walls, including the scallop walls, were forthcoming.  Please provide updated details and drawings.						<b>ANSWER:</b>  Per Meeting with WOJV and Designers on 9/2/2014 regarding options for the scallop wall changes in ASI 124, additional changes in the design for the rooftop exterior walls, including the scallop walls, were forthcoming.  Please provide updated details and drawings.
<b>T-1740</b>	<b>SCS - Confirming Construction Joint Increased Spacing</b>	<b>Closed</b>	<b>CR</b>	<b>09/24/2014</b>	<b>10/04/2014</b>	<b>10/07/2014</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  Please reference specification section 03 30 02 3.2.A.3, RFI #T-1104, and rejected submittal number TG0702-503.0, in reference to Ground Level CJ layout.  The specification calls out 60' maximum CJ spacing in the ground floor concrete slab. RFI #T-1104 permitted TG 06.0 to use a maximum CJ spacing of 96'. The rejected submittal includes comments suggesting to reduce the number of pours in Comment #2 on Sheet CJ-000, specifically in the metal deck section. Further comments would require a 97' 6" span at First and Fremont Streets and a potential 120' span between GL 19 and Wall 421. In order to match both suggestions, TG 07.2 would need to						<b>ANSWER:</b>  Please reference specification section 03 30 02 3.2.A.3, RFI #T-1104, and rejected submittal number TG0702-503.0, in reference to Ground Level CJ layout.  The specification calls out 60' maximum CJ spacing in the ground floor concrete slab. RFI #T-1104 permitted TG 06.0 to use a maximum CJ spacing of 96'. The rejected submittal includes comments suggesting to reduce the number of pours in Comment #2 on Sheet CJ-000, specifically in the metal deck section. Further comments would require a 97' 6" span at First and Fremont Streets and a potential 120' span between





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	approval.					An attachment demonstrates the change in the mix layout on ground level. Final CJ location is pending submittal approval.
T-1742	SCS - ST201 Slope of Grade Beams	Closed	CR	09/25/2014	10/05/2014	10/07/2014
From: Webcor Construction LP		Stephanie Azzolino				
REQUEST:		ANSWER:				
Please see attached SKA-4052 (per RFI #T-1661), SKS-0398 & SKS-0400 (confirmed in RFI #T-1661and 1658), and Markup A1-2960A (per email from TT on 8/8). SKS-0400 visually shows the grade beams sloping but does not provide the exact percent of slope. Cross referencing the available sheets provides conflicting slopes for each beam.		Please see attached SKA-4052 (per RFI #T-1661), SKS-0398 & SKS-0400 (confirmed in RFI #T-1661and 1658), and Markup A1-2960A (per email from TT on 8/8). SKS-0400 visually shows the grade beams sloping but does not provide the exact percent of slope. Cross referencing the available sheets provides conflicting slopes for each beam.				
The grade beams GBI and GB2 (see SKS-0398 for naming) are indicated on Mark up A1-2960A as "Constant Slope: Projection of Ground Level Slab". SKA-4052 calls out a single west-east slab slope of 1.2%. However, SKS-0398 provides two unique end of beam top elevations for GB1 and GB2, suggesting that the beams have two different slopes. The grade beams GB3 and GB4 (see SKS-0398 for naming) are indicated on Mark up A1-2960A as "Slope to Foundation Wall", where the grade break occurs at the west face of the foundation wall at EL + 15' 7". 3/SKS-0400 visually indicates the grade break at the east face of the foundation wall. No elevation on the east end of GB3 or GB4 is provided. The top of slab is provided, but 1/S1-5023 does not show a difference between top of slab and top of beam.		The grade beams GBI and GB2 (see SKS-0398 for naming) are indicated on Mark up A1-2960A as "Constant Slope: Projection of Ground Level Slab". SKA-4052 calls out a single west-east slab slope of 1.2%. However, SKS-0398 provides two unique end of beam top elevations for GB1 and GB2, suggesting that the beams have two different slopes. The grade beams GB3 and GB4 (see SKS-0398 for naming) are indicated on Mark up A1-2960A as "Slope to Foundation Wall", where the grade break occurs at the west face of the foundation wall at EL + 15' 7". 3/SKS-0400 visually indicates the grade break at the east face of the foundation wall. No elevation on the east end of GB3 or GB4 is provided. The top of slab is provided, but 1/S1-5023 does not show a difference between top of slab and top of beam.				
1) What are the slopes for GBI, GB2, GB3, and GB4? 2) What creates a difference in top elevation between GB 1 and GB2? 3) Where does the grade break for GB3 and GB4 occur? 4) What is the top elevation of GB3 and GB4 at the east face of the foundation wall? 5) What is the top elevation of GB3 and GB4 at GL 1.4, per 1/S 1-5023?		1) What are the slopes for GBI, GB2, GB3, and GB4? 2) What creates a difference in top elevation between GB 1 and GB2? 3) Where does the grade break for GB3 and GB4 occur? 4) What is the top elevation of GB3 and GB4 at the east face of the foundation wall? 5) What is the top elevation of GB3 and GB4 at GL 1.4, per 1/S 1-5023?				





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<b>T-1743</b>	<b>SSS - PE502 &amp; PE503 Fouling Connection at Bus Deck Level</b>	<b>Closed</b>	<b>CR</b>	<b>09/25/2014</b>	<b>10/05/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 627 SK1: The connection for the elevator post per 1/S1-7600 and the post foul the beam as shown. Please supply an alternate connection detail.			See attached CD RFI # 627 SK1: The connection for the elevator post per 1/S1-7600 and the post foul the beam as shown. Please supply an alternate connection detail.			
<b>T-1743.1</b>	<b>SSS - PE502 &amp; PE503 Fouling Connection at Bus Deck Level</b>	<b>Closed</b>	<b>CR</b>	<b>10/29/2014</b>	<b>11/08/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 627.1 SK1: The horizontal connection angle for the HSS18x6x5/8 per SKS-0418 is inside the elevator opening, typical 4 locations. Confirm that is the intent or supply a solution.			See attached CD RFI # 627.1 SK1: The horizontal connection angle for the HSS18x6x5/8 per SKS-0418 is inside the elevator opening, typical 4 locations. Confirm that is the intent or supply a solution.			
<b>T-1743.2</b>	<b>SSS - PE502 and PE503 Erection Clarification at Bus Deck Level</b>	<b>Closed</b>	<b>01</b>	<b>01/16/2015</b>	<b>01/26/2015</b>	<b>01/28/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: 4/S1-7108 Location: GL 24-25 Bus Deck Level Add'l Doc Ref: T-1743.1 (SK 841.1, CD 627.1), attached 3D model  The response in RFI T-1743.1 (SK 841.1, CD 627.1) creates an access issue. The nut end of the 1 in diameter through bolt will not be accessible with the current proposed layout. Please provide a solution.			Contract Doc Ref: 4/S1-7108 Location: GL 24-25 Bus Deck Level Add'l Doc Ref: T-1743.1 (SK 841.1, CD 627.1), attached 3D model  The response in RFI T-1743.1 (SK 841.1, CD 627.1) creates an access issue. The nut end of the 1 in diameter through bolt will not be accessible with the current proposed layout. Please provide a solution.			
<b>T-1744</b>	<b>SSS - Approval Comment Clarification at Cruciform Column GL 5</b>	<b>Closed</b>	<b>CR</b>	<b>09/25/2014</b>	<b>10/05/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 628 SK1: Columns A1777 & A1778 occur on Grid 5 and according to Note 3 in detail 4/S1-5011, the connection as shown and submitted is correct. Confirm it is the intent of the approval comment on drawings 1777 & 1778 to revise the			See attached CD RFI # 628 SK1: Columns A1777 & A1778 occur on Grid 5 and according to Note 3 in detail 4/S1-5011, the connection as shown and submitted is correct. Confirm it is the intent of the approval comment on			





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	connection at columns A1777 & A1778.				drawings 1777 & 1778 to revise the connection at columns A1777 & A1778.	
<b>T-1745</b>	<b>BGP - Elevator Sill Support Angle Conflict</b>	<b>Closed</b>	<b>01</b>	<b>09/25/2014</b>	<b>10/05/2014</b>	<b>10/05/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Please find attached drawings and picture. During the installation of Elevator Sill Support Angle at GL4.2/D.4, it was noticed there is a portion of L6x4x3/8 embed per detail 4/AI -7576 that will not be able to be installed due to the conflict with horizontal bracing member (see picture).  Please confirm that it will be acceptable to discontinue the Elevator Sill Support Angle at the conflict location. The embed will be discontinuous within 6" proximity of the bracing member.					<b>ANSWER:</b> Please find attached drawings and picture. During the installation of Elevator Sill Support Angle at GL4.2/D.4, it was noticed there is a portion of L6x4x3/8 embed per detail 4/AI -7576 that will not be able to be installed due to the conflict with horizontal bracing member (see picture).  Please confirm that it will be acceptable to discontinue the Elevator Sill Support Angle at the conflict location. The embed will be discontinuous within 6" proximity of the bracing member.	
<b>T-1746</b>	<b>BGP - Bar Layering Issues at Beam Intersection</b>	<b>Closed</b>	<b>01</b>	<b>09/26/2014</b>	<b>10/06/2014</b>	<b>10/01/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Please verify that per detail 1/S1-3600, the call out for 1 ½" - 2" clear between top and 2nd layer bars is a minimum distance between bars.  As demonstrated in the attached picture, at the intersection of GL-3, GL-G, and GL-X, the 1 ½" - 2" clear cover is not attainable due to layering issues with intersection beams.					<b>ANSWER:</b> Please verify that per detail 1/S1-3600, the call out for 1 ½" - 2" clear between top and 2nd layer bars is a minimum distance between bars.  As demonstrated in the attached picture, at the intersection of GL-3, GL-G, and GL-X, the 1 ½" - 2" clear cover is not attainable due to layering issues with intersection beams.	



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<b>T-1747</b>	<b>SSS - ST201A Approval Comment Clarification at Bus Deck Level</b>	<b>Closed</b>	<b>CR</b>	<b>09/26/2014</b>	<b>10/06/2014</b>	<b>10/02/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 630 SK1: The note on S1-2502 states "SEE 1/S1-5010 (SIM) FOR CONNECTION". This is insufficient information to detail these connections as detail 1/S1-5010 does not show a double angle connection inside the column profile. The beam will not be erectable and the bolts thru the beam web will not be accessible inside the column profile. The current connection is per typical detail 2/S1-5012.			See attached CD RFI # 630 SK1: The note on S1-2502 states "SEE 1/S1-5010 (SIM) FOR CONNECTION". This is insufficient information to detail these connections as detail 1/S1-5010 does not show a double angle connection inside the column profile. The beam will not be erectable and the bolts thru the beam web will not be accessible inside the column profile. The current connection is per typical detail 2/S1-5012.			
Confirm the connections at the noted locations are acceptable or supply a new detail.			Confirm the connections at the noted locations are acceptable or supply a new detail.			
<b>T-1748</b>	<b>SSS - Conflicting Instructions at Second Level Beam GL 1.4</b>	<b>Closed</b>	<b>CR</b>	<b>09/26/2014</b>	<b>10/06/2014</b>	<b>10/02/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 632 SK1: The comment conflicts with the instructions issued with RFI T-1427 (SK 605 , CD 433) item 2, which requested a new beam to support the unsupported deck. Which instruction is correct?			See attached CD RFI # 632 SK1: The comment conflicts with the instructions issued with RFI T-1427 (SK 605 , CD 433) item 2, which requested a new beam to support the unsupported deck. Which instruction is correct?			
<b>T-1749</b>	<b>SSS - PE502 &amp; PE503 Conflicting Connection Details</b>	<b>Closed</b>	<b>CR</b>	<b>09/26/2014</b>	<b>10/06/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 633 SK1 for items 1 to 4: 1) Confirm the various types of base plates per RFI T-0941.1 (SK 193.2, CD 146.1) may be applied here depending on the beam flange widths. 2) The beam to beam connections per 1/S1-5010 & 1/S1-5013 and the post connection with stiffener plates per 10/S1-7630 cannot both be applied as they will foul each other. Supply a new connection detail at (6) locations. 3) The beam to beam connections per S1-5019 and the post connection with stiffener plates per 10/S1-7630 cannot both be applied as they will foul each other. Supply a new connection detail. 4) The beam to beam connections per 1/S1-5010 and the			See attached CD RFI # 633 SK1 for items 1 to 4: 1) Confirm the various types of base plates per RFI T-0941.1 (SK 193.2, CD 146.1) may be applied here depending on the beam flange widths. 2) The beam to beam connections per 1/S1-5010 & 1/S1-5013 and the post connection with stiffener plates per 10/S1-7630 cannot both be applied as they will foul each other. Supply a new connection detail at (6) locations. 3) The beam to beam connections per S1-5019 and the post connection with stiffener plates per 10/S1-7630 cannot both be applied as they will foul each other. Supply a new connection detail.			



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	post connection with stiffener plates per 10/S1-7630 cannot both be applied as they foul each other. Supply a new detail at (11) locations.					4) The beam to beam connections per 1/S1-5010 and the post connection with stiffener plates per 10/S1-7630 cannot both be applied as they foul each other. Supply a new detail at (11) locations.
T-1749.1	SSS - PE502 & PE503 Conflicting Connection Details	Closed	CR	10/29/2014	11/08/2014	11/06/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:						ANSWER:
See attached CD RFI # 633.1 SK1 to SK4 for items 1 to 6:						See attached CD RFI # 633.1 SK1 to SK4 for items 1 to 6:
1) The connection per 9/S1-5032 fouls the stiffener plates per 10 & 11/S1-7630. Provide a solution.						1) The connection per 9/S1-5032 fouls the stiffener plates per 10 & 11/S1-7630. Provide a solution.
2) Confirm the shear plate per 1/S1-5013 may be pulled out as shown to be able to erect the beam as the other end of the beam has a double angle connection per 1/S1-5010 between the stiffeners per 10 & 11/S1/7630.						2) Confirm the shear plate per 1/S1-5013 may be pulled out as shown to be able to erect the beam as the other end of the beam has a double angle connection per 1/S1-5010 between the stiffeners per 10 & 11/S1/7630.
3) Confirm the stiffener plate per 10/S1-7630 may be located as shown to substitute as a shear plate per 1/S1-5011 or supply an alternate connection detail.						3) Confirm the stiffener plate per 10/S1-7630 may be located as shown to substitute as a shear plate per 1/S1-5011 or supply an alternate connection detail.
4) Confirm the shear plate per 1/S1-5013 may be pulled out as shown to be able to erect the beam as the other end of the beam has the same connection.						4) Confirm the shear plate per 1/S1-5013 may be pulled out as shown to be able to erect the beam as the other end of the beam has the same connection.
5) Confirm stiffener plates per 10/S1-7630 are not required at the noted locations.						5) Confirm stiffener plates per 10/S1-7630 are not required at the noted locations.
6) Confirm stiffener plates per 10/S1-7630 are not required at the noted locations.						6) Confirm stiffener plates per 10/S1-7630 are not required at the noted locations.



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-1750</b>	<b>SSS - Added Splice Due to Crane Capacity</b>	<b>Closed</b>	<b>CR</b>	<b>09/26/2014</b>	<b>10/06/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Information Requested: Please verify it is acceptable to add an additional shop splice to the Built Up Members as shown in the attached sketches due to crane capacity.  THC crane capacity is 40 Ton. By adding the additional splice we can build the members in two pieces allowing the Subarc welding to be completed inside along with all secondary fit up. When this is complete THC will move them outside and splice the two pieces together.						<b>ANSWER:</b>  Information Requested: Please verify it is acceptable to add an additional shop splice to the Built Up Members as shown in the attached sketches due to crane capacity.  THC crane capacity is 40 Ton. By adding the additional splice we can build the members in two pieces allowing the Subarc welding to be completed inside along with all secondary fit up. When this is complete THC will move them outside and splice the two pieces together.
<b>T-1751</b>	<b>SSS - Upper Basket Column Pipes at Gridline 1</b>	<b>Closed</b>	<b>CR</b>	<b>09/26/2014</b>	<b>10/06/2014</b>	<b>10/08/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Upper basket column pipes located at gridline 1/G and 1/C are shown to be concrete filled to top of pipe in S1-4018 Rev 4. Details shown in B/S1-4018 require a 2" diameter vent hole in the roof level cast nodes. Roof level cast nodes at these locations are 72-36 and 72-37 and according to machining drawing CN-0127 Rev 4 (issued 12/19/13), neither of these cast nodes have vent or grout holes.  Currently, pipes are detailed with (2) - 5/8" vent holes approx. 7" from the top of the pipe so the pipes could be filled to this point. Please confirm currently detailed pipe vent holes can remain as is or provide new vent hole details for the pipe and/or cast nodes.  Please note cast nodes 72-36 and 72-37 have already been released to Oregon Iron Works by Bradken.						<b>ANSWER:</b>  Upper basket column pipes located at gridline 1/G and 1/C are shown to be concrete filled to top of pipe in S1-4018 Rev 4. Details shown in B/S1-4018 require a 2" diameter vent hole in the roof level cast nodes. Roof level cast nodes at these locations are 72-36 and 72-37 and according to machining drawing CN-0127 Rev 4 (issued 12/19/13), neither of these cast nodes have vent or grout holes.  Currently, pipes are detailed with (2) - 5/8" vent holes approx. 7" from the top of the pipe so the pipes could be filled to this point. Please confirm currently detailed pipe vent holes can remain as is or provide new vent hole details for the pipe and/or cast nodes.  Please note cast nodes 72-36 and 72-37 have already been released to Oregon Iron Works by Bradken.



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<b>T-1752</b>	<b>SSS - Missing Stiffeners Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>09/26/2014</b>	<b>10/06/2014</b>	<b>10/13/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  1) May it be assumed that the missing stiffeners are the ones hi-lited in yellow in detail 7/S1-8002? If yes, please supply the missing information for thickness, location and welding for the stiffeners.  2) May it be assumed that the missing stiffeners are the ones hi-lited in yellow in detail 11/S1-8002? If yes, please supply the missing information for thickness, location and welding for the stiffeners.						<b>ANSWER:</b>  1) May it be assumed that the missing stiffeners are the ones hi-lited in yellow in detail 7/S1-8002? If yes, please supply the missing information for thickness, location and welding for the stiffeners.  2) May it be assumed that the missing stiffeners are the ones hi-lited in yellow in detail 11/S1-8002? If yes, please supply the missing information for thickness, location and welding for the stiffeners.
<b>T-1753</b>	<b>SSS - Stiffener Fouling Box Beam at GL 2-3</b>	<b>Closed</b>	<b>CR</b>	<b>09/26/2014</b>	<b>10/06/2014</b>	<b>10/06/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 631 SK1: The full depth stiffeners per 8/S1-8003 foul the Box beam as shown. Please supply a solution.						<b>ANSWER:</b>  See attached CD RFI # 631 SK1: The full depth stiffeners per 8/S1-8003 foul the Box beam as shown. Please supply a solution.
<b>T-1754</b>	<b>SSS - ST202 Approval Comment Clarifications at Second Level</b>	<b>Closed</b>	<b>CR</b>	<b>09/29/2014</b>	<b>10/09/2014</b>	<b>10/21/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 629 SK1 The model has been built per the dimensions in 3 & 5/S1-7004. This approval comment directs us to confirm locations per A1-7004 in AS1-121 and introduces conflicting dimensions as A1-7004 shows different post locations than S1-7004. Per the American Institute of Steel Construction Code of Standard Practice Section 3. Design Drawings and Specifications, subsection 3.1, the structural drawings are the document Skanska used to locate the stair posts. Please clarify the conflicting information.						<b>ANSWER:</b>  See attached CD RFI # 629 SK1 The model has been built per the dimensions in 3 & 5/S1-7004. This approval comment directs us to confirm locations per A1-7004 in AS1-121 and introduces conflicting dimensions as A1-7004 shows different post locations than S1-7004. Per the American Institute of Steel Construction Code of Standard Practice Section 3. Design Drawings and Specifications, subsection 3.1, the structural drawings are the document Skanska used to locate the stair posts. Please clarify the conflicting information.
<b>T-1755</b>	<b>SCS - Electrical Conduit in Foundation Wall</b>	<b>Closed</b>	<b>01</b>	<b>09/29/2014</b>	<b>10/09/2014</b>	<b>10/19/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						



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	<p><b>REQUEST:</b></p> <p>Reference: ES-2107(Dated: 04/24/14) , A1-3010(Dated: 07/18/14)</p> <p>Detail A &amp; B on ES-2107 provides the section view for PG&amp;E 12 KV Main Service coming into North Electrical Room (B1289) but A1-3010 doesn't refer to an Elevation view identifying these sleeves in the walls. Please confirm the section views and the elevation of the bottom of the conduits in Detail A&amp;B/ES-2107 are correct.</p> <p>Reference : A1-9244(Dated: 07/18/14), ES-2107 to ES-2111(Dated: 04/24/14)</p> <p>Detail D/A1-9244 specifies center to center vertical spacing between 2" and 6" electrical conduits as 1'-5"(See attached). Please confirm this spacing is applicable to all the section details in ES-2107 to ES-2111 entering the building, if not please provide the center to center vertical spacing between 2' and 6" conduits called out in ES-2107 to ES-2111.</p> <p>Reference: ES-2107 to ES-2111(Dated: 04/24/14)</p> <p>The section view on ES-2107 to ES-2111 calls out the bottom of the conduit elevations for 2" and 6" conduits. Please confirm these elevations are accurate for all the ES drawings.</p>					
	<p><b>ANSWER:</b></p> <p>Reference: ES-2107(Dated: 04/24/14) , A1-3010(Dated: 07/18/14)</p> <p>Detail A &amp; B on ES-2107 provides the section view for PG&amp;E 12 KV Main Service coming into North Electrical Room (B1289) but A1-3010 doesn't refer to an Elevation view identifying these sleeves in the walls. Please confirm the section views and the elevation of the bottom of the conduits in Detail A&amp;B/ES-2107 are correct.</p> <p>Reference : A1-9244(Dated: 07/18/14), ES-2107 to ES-2111(Dated: 04/24/14)</p> <p>Detail D/A1-9244 specifies center to center vertical spacing between 2" and 6" electrical conduits as 1'-5"(See attached). Please confirm this spacing is applicable to all the section details in ES-2107 to ES-2111 entering the building, if not please provide the center to center vertical spacing between 2' and 6" conduits called out in ES-2107 to ES-2111.</p> <p>Reference: ES-2107 to ES-2111(Dated: 04/24/14)</p> <p>The section view on ES-2107 to ES-2111 calls out the bottom of the conduit elevations for 2" and 6" conduits. Please confirm these elevations are accurate for all the ES drawings.</p>					
T-1756	BGP - Area 3 Concourse Rebar- Displacement of Top Added Bar Due to Congestion	Closed	01	09/29/2014	10/09/2014	10/01/2014
	From: Webcor Construction LP Claude Titcher					
	<p><b>REQUEST:</b></p> <p>Per the attached picture, please verify that is acceptable that a 2nd layer was created for top added bars due to congestion at beam and column intersection. This occurs between GL-3 &amp; GL- 4 at GL-K.3.</p>					
	<p><b>ANSWER:</b></p> <p>Per the attached picture, please verify that is acceptable that a 2nd layer was created for top added bars due to congestion at beam and column intersection. This occurs between GL-3 &amp; GL- 4 at GL-K.3.</p>					
T-1757	SCS - Rebar Shop Drawing Discrepancy at GL 32.4	Closed	01	09/30/2014	10/10/2014	09/30/2014



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<b>From:</b> Shimmick Construction Company, Inc. Henry Chiang						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please see attached marked up drawings.			Please see attached marked up drawings.			
Submittal TG0600-318.2 and RFI T-1218.I note vertical rebar spacing of "Rebar option 2 SK3 #11@5" OC" on the south foundation wall at GL 32.4. RFI T-1372 notes "Contract Rebar WR2 #11@6" OC" vertical rebar spacing on the south foundation wall at GL 32.4.			Submittal TG0600-318.2 and RFI T-1218.I note vertical rebar spacing of "Rebar option 2 SK3 #11@5" OC" on the south foundation wall at GL 32.4. RFI T-1372 notes "Contract Rebar WR2 #11@6" OC" vertical rebar spacing on the south foundation wall at GL 32.4.			
Please confirm which document to follow for planning and pricing of additional costs incurred by the changes if any.			Please confirm which document to follow for planning and pricing of additional costs incurred by the changes if any.			
<b>T-1758</b>	<b>SSS - Shear Plate Fouling Bolts at TR24.9</b>	<b>Closed</b>	<b>CR</b>	<b>10/01/2014</b>	<b>10/11/2014</b>	<b>10/13/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 639-SK1: The shear plate will foul the flange bolts in TR24.9 for the column above if the beam is located per design dimension s. Confirm it is acceptable to re-locate the beams per dimensions shown in blue or supply an alternate solution.			See attached CD RFI # 639-SK1: The shear plate will foul the flange bolts in TR24.9 for the column above if the beam is located per design dimensions. Confirm it is acceptable to re-locate the beams per dimensions shown in blue or supply an alternate solution.			
<b>T-1759</b>	<b>SSS - Transfer Girder Fillet Weld on Intermediate Flange</b>	<b>Closed</b>	<b>CR</b>	<b>10/01/2014</b>	<b>10/11/2014</b>	<b>10/09/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 635 SK1: On the approval return of WS07 transfer girder 664, the design team has asked for a CJP weld at the left end where the intermediate flange plate slots into the web. The typical detail for the intermediate flange as it slots around the web calls out a fillet weld to the girder web, both sides. The contract drawing detail B/S1-4301 does not show the typical fillet weld but it is indicated on the right end of this girder as a 5/8" fillet. Please confirm the 5/8" fillet is the design intent and no revision to 664AB is required.			See attached CD RFI # 635 SK1: On the approval return of WS07 transfer girder 664, the design team has asked for a CJP weld at the left end where the intermediate flange plate slots into the web. The typical detail for the intermediate flange as it slots around the web calls out a fillet weld to the girder web, both sides. The contract drawing detail B/S1-4301 does not show the typical fillet weld but it is indicated on the right end of this girder as a 5/8" fillet. Please confirm the 5/8" fillet is the design intent and no revision to 664AB is required.			





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<b>T-1760</b>	<b>SSS - Field Splices at Curved Light Column Beam</b>	<b>Closed</b>	<b>CR</b>	<b>10/01/2014</b>	<b>10/11/2014</b>	<b>10/09/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> It is the intent to shop-attach the short cantilevered beams (shown in red on the attached SK1) to the supporting beams to assist in erecting the W27 curved beams. In addition, for erectability three field splices will need to be introduced as shown in Blue.  Please confirm that the field splices will be acceptable.						<b>ANSWER:</b> It is the intent to shop-attach the short cantilevered beams (shown in red on the attached SK1) to the supporting beams to assist in erecting the W27 curved beams. In addition, for erectability three field splices will need to be introduced as shown in Blue.  Please confirm that the field splices will be acceptable.
<b>T-1761</b>	<b>SSS - Transfer Girder Stiffener Plate Access Holes</b>	<b>Closed</b>	<b>CR</b>	<b>10/01/2014</b>	<b>10/11/2014</b>	<b>10/09/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> With reference to the transfer girder stiffener plates with C JP welds, please confirm it acceptable to terminate the weld as per D1.8 Figure C-6.3 and SK1 attached.						<b>ANSWER:</b> With reference to the transfer girder stiffener plates with CJP welds, please confirm it acceptable to terminate the weld as per D1.8 Figure C-6.3 and SK1 attached.
<b>T-1762</b>	<b>SSS - E510, E511, &amp; E512 Angle Connection Clarification at Erection Tab</b>	<b>Closed</b>	<b>CR</b>	<b>10/01/2014</b>	<b>10/11/2014</b>	<b>10/09/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 594 SK1: The L5x5 connection angle fouls the bolt on the erection tab as shown. Confirm it is acceptable to connect the W12x14 to the W12x14 per 1/S1-5011 or supply an alternate solution. This condition occurs at (6) locations.						<b>ANSWER:</b> See attached CD RFI # 594 SK1: The L5x5 connection angle fouls the bolt on the erection tab as shown. Confirm it is acceptable to connect the W12x14 to the W12x14 per 1/S1-5011 or supply an alternate solution. This condition occurs at (6) locations.
<b>T-1763</b>	<b>SSS - Moment Connection Clarification GL 22</b>	<b>Closed</b>	<b>CR</b>	<b>10/01/2014</b>	<b>10/11/2014</b>	<b>10/09/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 603 SK1: It is not possible to connect the BU beam to the column flange per 4/S1-5012 because the 6" gap between the web doubler plates and the column flange is insufficient space for the shear plate. Confirm it acceptable to shop weld the web to the column flange as shown or supply an alternate solution.						<b>ANSWER:</b> See attached CD RFI # 603 SK1: It is not possible to connect the BU beam to the column flange per 4/S1-5012 because the 6" gap between the web doubler plates and the column flange is insufficient space for the shear plate. Confirm it acceptable to shop weld the web to the column flange as shown or supply an alternate solution.







<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
T-1767	<b>SSS - Weld Clearance Hole Size at Moment Frame Column Cruciform</b>  From: Webcor Construction LP Gregory Kemerer	Closed	CR	10/01/2014	10/11/2014	10/13/2014
<b>REQUEST:</b>  Several Moment Frame Columns have cruciform sections that weld over column web splice joints. The splice joint provides a transition from thick web to thin web. In a majority of these cases, the cruciform web as detailed has a 1" radius cutout to clear the column web splice joint. The 1" radius cutout is insufficient to adequately clear the tapered double bevel splice joint and provide space for runoff tabs necessary for the CJP weld.  Please confirm it is acceptable to provide a weld clearance hole in the cruciform section wide enough to clear the bevels of the column web splice joint plus 1" on each side of the bevel for weld runoff tabs. See attached sketch. If confirmed, this information needs to be shown on detail drawings.		<b>ANSWER:</b>  Several Moment Frame Columns have cruciform sections that weld over column web splice joints. The splice joint provides a transition from thick web to thin web. In a majority of these cases, the cruciform web as detailed has a 1" radius cutout to clear the column web splice joint. The 1" radius cutout is insufficient to adequately clear the tapered double bevel splice joint and provide space for runoff tabs necessary for the CJP weld.  Please confirm it is acceptable to provide a weld clearance hole in the cruciform section wide enough to clear the bevels of the column web splice joint plus 1" on each side of the bevel for weld runoff tabs. See attached sketch. If confirmed, this information needs to be shown on detail drawings.				



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-1768</b>	<b>SSS - SLRS Bracing Connection Confirmations GL 22-24</b>	<b>Closed</b>	<b>CR</b>	<b>10/01/2014</b>	<b>10/11/2014</b>	<b>10/17/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 618 SK1:			See attached CD RFI # 618 SK1:			
The following is a summary of consequences for the varying slopes on Grids 22, 23 & 24 as the diagonal bracing will no longer be in the same plane similar to the issues in RFI's T-1322, T-1322.2 & T-1513 (SK 515, 515.1 & 515.2, CD 382, 382.1 & 382.2.):			The following is a summary of consequences for the varying slopes on Grids 22, 23 & 24 as the diagonal bracing will no longer be in the same plane similar to the issues in RFI's T-1322, T-1322.2 & T-1513 (SK 515, 515.1 & 515.2, CD 382, 382.1 & 382.2.):			
1) The two ends of the W40x297's will not be flush with the connection ends due to varying slope between Grid 22 to the slope on Grid 23. Please provide a detail to address this issue.			1) The two ends of the W40x297's will not be flush with the connection ends due to varying slope between Grid 22 to the slope on Grid 23. Please provide a detail to address this issue.			
2) The 1 3/4" gusset plates per 1A/S1-5018 will have two beams coming in at compound angles. Please provide a solution.			2) The 1 3/4" gusset plates per 1A/S1-5018 will have two beams coming in at compound angles. Please provide a solution.			
3) A shim plate will be required on top of the W40x277 & W40x297's to lift the 1 3/4" gusset above the flange of the W40x362's.			3) A shim plate will be required on top of the W40x277 & W40x297's to lift the 1 3/4" gusset above the flange of the W40x362's.			
4) Shims beveled in both directions, varying in thickness at each corner will be required on top and bottom of the W40x362's as the flanges are not parallel to the flanges of the W40x277 & W40x297's.			4) Shims beveled in both directions, varying in thickness at each corner will be required on top and bottom of the W40x362's as the flanges are not parallel to the flanges of the W40x277 & W40x297's.			
5) The holes for the 1 1/2" A490-X-SC bolts will require oversize holes as the surfaces of the flanges and the 1 3/4" gusset plate are not parallel.			5) The holes for the 1 1/2" A490-X-SC bolts will require oversize holes as the surfaces of the flanges and the 1 3/4" gusset plate are not parallel.			
Confirm this is the design intent.			Confirm this is the design intent.			



<i>Number</i>	<i>Subject</i>	<i>Status</i>	<i>Choice Required</i>	<i>Date Created</i>	<i>Date Required</i>	<i>Date Answered</i>
T-1768.1	SSS - SLRS Bracing Connection Confirmations GL 22-24	Closed	CR	12/09/2014	12/19/2014	12/22/2014
<div><div><div>From: Webcor Construction LP</div><div>Gregory Kemerer</div></div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<div>Contract Doc Ref: S1-2505 Location: Zone 4, Bus Deck Grid Line: A &amp; 23, H &amp; 23 Add'l Doc Ref's: SK1, SK2, SK3, RFI T-1768 Response</div>			<div>Contract Doc Ref: S1-2505 Location: Zone 4, Bus Deck Grid Line: A &amp; 23, H &amp; 23 Add'l Doc Ref's: SK1, SK2, SK3, RFI T-1768 Response</div>			
<div>The response to RFI T-1768 confirmed that detail 1/S1-5018 calls for shim plates as required. The response also indicated that it is acceptable to slope Grid 23 to match Grids 22 and 24 - in order to avoid slope in two directions.</div>			<div>The response to RFI T-1768 confirmed that detail 1/S1-5018 calls for shim plates as required. The response also indicated that it is acceptable to slope Grid 23 to match Grids 22 and 24 - in order to avoid slope in two directions.</div>			
<div>Skanska has indicated that the second option is preferable, as the complex shims (as detailed in SK1 and SK1) would likely cause erection problems.</div>			<div>Skanska has indicated that the second option is preferable, as the complex shims (as detailed in SK1 and SK1) would likely cause erection problems.</div>			
<div>Please confirm the top of steel elevations as shown in SK3, which would result from sloping Grid 23 to match Grids 22 and 24.</div>			<div>Please confirm the top of steel elevations as shown in SK3, which would result from sloping Grid 23 to match Grids 22 and 24.</div>			
T-1769	SCS - Foundation Wall Embeds: Plumbing at GL 6-10	Closed	01	10/01/2014	10/11/2014	10/01/2014
<div><div><div>From: Shimmick Construction Company, Inc. Henry Chiang</div></div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<div>See attached marked up drawing.</div>			<div>See attached marked up drawing.</div>			
<div>The noted natural gas pipe does not have a specified diameter. The pipe diameter is required to determine sleeve size and center elevation for sleeve placement.</div>			<div>The noted natural gas pipe does not have a specified diameter. The pipe diameter is required to determine sleeve size and center elevation for sleeve placement.</div>			
<div>Please provide the diameter for noted natural gas pipe.</div>			<div>Please provide the diameter for noted natural gas pipe.</div>			
T-1770	BGP - C30 Column at GL 4/J	Closed	01	10/02/2014	10/12/2014	10/06/2014
<div><div><div>From: Webcor Construction LP</div><div>Claude Titcher</div></div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<div>The C30 Column at GL 4/J was installed 4" West of the planned location along the correct Bicycle Ramp Alignment (See attached As-Built comparison)</div>			<div>The C30 Column at GL 4/J was installed 4" West of the planned location along the correct Bicycle Ramp Alignment (See attached As-Built comparison)</div>			



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	Please confirm this as-built location is acceptable.				Please confirm this as-built location is acceptable.	
<b>T-1771</b>	<b>BGP - Geothermal Sleeve Reinforcement at GL 13</b>	<b>Closed</b>	<b>01</b>	<b>10/02/2014</b>	<b>10/12/2014</b>	<b>10/15/2014</b>
<hr/>						
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Detail 4 on plan sheet S1-3207 provides the trim detail for wall penetrations. Note 1 states that wall penetrations not shown in the structural drawings are not allowed unless approved by the SEOR.		Detail 4 on plan sheet S1-3207 provides the trim detail for wall penetrations. Note 1 states that wall penetrations not shown in the structural drawings are not allowed unless approved by the SEOR.				
1) Please confirm the geothermal sleeves may be installed in the foundation walls as shown in the Architectural drawings.		1) Please confirm the geothermal sleeves may be installed in the foundation walls as shown in the Architectural drawings.				
2) Currently the Geothermal Sleeves conflict with the foundation walls at two locations, GL 13(See attached photo). Please confirm two(2) bars at each location (one in the front curtain and one in the back curtain) can be abandoned to facilitate the installation of the Geothermal Sleeves.		2) Currently the Geothermal Sleeves conflict with the foundation walls at two locations, GL 13(See attached photo). Please confirm two(2) bars at each location (one in the front curtain and one in the back curtain) can be abandoned to facilitate the installation of the Geothermal Sleeves.				
<hr/>						
<b>T-1772</b>	<b>BGP - Partition Wall -Top of Pier Bracing</b>	<b>Void</b>	<b>CR</b>	<b>10/02/2014</b>	<b>10/12/2014</b>	
<hr/>						
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please see the attached clouded drawings, S1-9050, S1-9002, and S1-9001. Note 5 on detail 9/S1-9050 refers to detail 1/S1-9002 for the typical top of pier bracing details. Detail 3/S1-9001 depicts the bracing of an interior CMU/Concrete Pier below a metal deck. There are no other details shown on the contract drawings that depict bracing of interior CMU/Concrete pier below concrete concourse deck. Please confirm it is acceptable to omit the 3/8" thick plate on Detail 3/S1-9001 for bracing below concrete concourse deck. Otherwise, please provide a detail for bracing the top of an interior concrete pier to the concrete deck.		Please see the attached clouded drawings, S1-9050, S1-9002, and S1-9001. Note 5 on detail 9/S1-9050 refers to detail 1/S1-9002 for the typical top of pier bracing details. Detail 3/S1-9001 depicts the bracing of an interior CMU/Concrete Pier below a metal deck. There are no other details shown on the contract drawings that depict bracing of interior CMU/Concrete pier below concrete concourse deck. Please confirm it is acceptable to omit the 3/8" thick plate on Detail 3/S1-9001 for bracing below concrete concourse deck. Otherwise, please provide a detail for bracing the top of an interior concrete pier to the concrete deck.				



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T-1773.1	SSS - AB Analysis - TB Column Base Plates	Closed	CR	10/20/2014	10/20/2014	10/22/2014
<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p>						
<p><b>REQUEST:</b></p> <p>This is a confirming RFI following our October 9, 2014 structural coordination meeting. The following items / actions were discussed:</p> <ol style="list-style-type: none"> <li>1. For anchor bolts that do not clash with the column base plates but are out of AISC tolerance thus limiting the ability to adjust for structural steel fabrication and erection tolerances, the EOR takes no exception to modifying the base plate holes in the shop or field to suit the as-built anchor bolt position.</li> <li>2. The EOR takes no exception to reducing the size of the plate washers above and below the horizontal stiffener/anchor plate in the column. This will be done to clear the fillet weld for the vertical stiffener below the horizontal anchor plate and to allow for consistent plate washer sizes. The plate washers will be larger than the major diameter of the anchor nuts. The attached spreadsheet details the sizes we will use at each location.</li> <li>3. The EOR takes no exception to adding a spacer plate below the horizontal anchor plate and above the nut at the 2-1/2" anchor rod locations where necessary. This is necessary to accommodate out of tolerance anchor bolts so that the nut can clear the fillet weld between the horizontal anchor plate and the vertical stiffener below.</li> </ol> <p>Please confirm the above items.</p>			<p><b>ANSWER:</b></p> <p>This is a confirming RFI following our October 9, 2014 structural coordination meeting. The following items / actions were discussed:</p> <ol style="list-style-type: none"> <li>1. For anchor bolts that do not clash with the column base plates but are out of AISC tolerance thus limiting the ability to adjust for structural steel fabrication and erection tolerances, the EOR takes no exception to modifying the base plate holes in the shop or field to suit the as-built anchor bolt position.</li> <li>2. The EOR takes no exception to reducing the size of the plate washers above and below the horizontal stiffener/anchor plate in the column. This will be done to clear the fillet weld for the vertical stiffener below the horizontal anchor plate and to allow for consistent plate washer sizes. The plate washers will be larger than the major diameter of the anchor nuts. The attached spreadsheet details the sizes we will use at each location.</li> <li>3. The EOR takes no exception to adding a spacer plate below the horizontal anchor plate and above the nut at the 2-1/2" anchor rod locations where necessary. This is necessary to accommodate out of tolerance anchor bolts so that the nut can clear the fillet weld between the horizontal anchor plate and the vertical stiffener below.</li> </ol> <p>Please confirm the above items.</p>			









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<div>Please specify the cure time required for the 10" structural slab before the 4" structural topping slab can be placed.</div> <div>Please specify the cure time required for the 10" structural slab before the 4" structural topping slab can be placed.</div>						
<b>T-1777</b>	<b>SCS - Concrete Placement Sequence GL 10-13</b>	<b>Closed</b>	<b>01</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>10/14/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference attached sketch.		Reference attached sketch.				
Please confirm,		Please confirm,				
1. Once A1 is poured and cured, is it acceptable to place A2-C2 in one pour prior to B1 and C1 being placed?		1. Once A1 is poured and cured, is it acceptable to place A2-C2 in one pour prior to B1 and C1 being placed?				
2. If #1 is confirmed, is it acceptable to then place A3-C3 in one pour, following A2-C2?		2. If #1 is confirmed, is it acceptable to then place A3-C3 in one pour, following A2-C2?				
3. If #2 is confirmed, is it acceptable to then place A4-C4 in one pour, Following A3-C3?		3. If #2 is confirmed, is it acceptable to then place A4-C4 in one pour, Following A3-C3?				



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<b>T-1778</b>	<b>SCS - Ground, Second, Bus, and Roof Level Concrete Placement Sequence</b>	<b>Closed</b>	<b>01</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>10/13/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>						
Reference attached sketch.						
Steel erection begins in Area C01, which is between gridlines 10-11.						
1) The construction Joints in the trainbox walls do not line up with gridlines. Is it acceptable to align the fourth lift foundation wall construction joints with gridlines? Note that ground level slabs will also follow this layout as shown in the attached sketch.						
2) WOJV believes that the formed and metal deck concrete at ground level GL 10-11 needs to be placed before pouring any other metal decks above this level. Please confirm this is correct. If required to place more east/west concrete at ground level prior to placing decks above, please provide requirements.						
3) Please provide the required cure time of ground floor concrete before the placing of concrete in metal decks above ground level can be poured.						
4) Once the ground level concrete reaches required strength is it acceptable to pour the 2nd level, bus deck level (1st 10" thick slab only), and roof level consecutively with no cure time requirement between deck pours?						
<b>ANSWER:</b>						
Reference attached sketch.						
Steel erection begins in Area C01, which is between gridlines 10-11.						
1) The construction Joints in the trainbox walls do not line up with gridlines. Is it acceptable to align the fourth lift foundation wall construction joints with gridlines? Note that ground level slabs will also follow this layout as shown in the attached sketch.						
2) WOJV believes that the formed and metal deck concrete at ground level GL 10-11 needs to be placed before pouring any other metal decks above this level. Please confirm this is correct. If required to place more east/west concrete at ground level prior to placing decks above, please provide requirements.						
3) Please provide the required cure time of ground floor concrete before the placing of concrete in metal decks above ground level can be poured.						
4) Once the ground level concrete reaches required strength is it acceptable to pour the 2nd level, bus deck level (1st 10" thick slab only), and roof level consecutively with no cure time requirement between deck pours?						
<b>T-1779</b>	<b>SSS - Cast Node PJP to Girder Flange</b>	<b>Closed</b>	<b>CR</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>10/09/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>						
The PJP for the cast node base to girder intermediate flange runs over the radius of the slotted flange. Please confirm it is acceptable to use a 3/8" run off tab fitted into the slot to allow the PJP to continue around the node as required.						
See SK1 & SK2 for clarification.						
<b>ANSWER:</b>						
The PJP for the cast node base to girder intermediate flange runs over the radius of the slotted flange. Please confirm it is acceptable to use a 3/8" run off tab fitted into the slot to allow the PJP to continue around the node as required.						
See SK1 & SK2 for clarification.						



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<b>T-1780</b>	<b>SSS - ST501 &amp; ST502 Paint Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>10/22/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 641 SK1: Confirm the noted beams require IRFM-1 per D & E/A1-8663 and supply dimensions on plan to show the extent of the paint as A1-8663 & A1-9230 do not supply this information.						<b>ANSWER:</b>  See attached CD RFI # 641 SK1: Confirm the noted beams require IRFM-1 per D & E/A1-8663 and supply dimensions on plan to show the extent of the paint as A1-8663 & A1-9230 do not supply this information.
<b>T-1781</b>	<b>SSS - W21 and W24 Connection Clarification GL 27.1</b>	<b>Closed</b>	<b>CR</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>11/11/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 636 SK1: The W21x48 north of Grid D.4 and the W24x76 south of Grid E.6 can't be connected with double angles per 1/S1-5010 as the angles will foul the web stiffener plates per 2/S1-5016. Please supply a new detail.						<b>ANSWER:</b>  See attached CD RFI # 636 SK1: The W21x48 north of Grid D.4 and the W24x76 south of Grid E.6 can't be connected with double angles per 1/S1-5010 as the angles will foul the web stiffener plates per 2/S1-5016. Please supply a new detail.
<b>T-1782</b>	<b>SSS - Stair Elevator Weld Clarification from WS4 Package</b>	<b>Void</b>	<b>CR</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  The noted weld symbol has been interpreted as a weld only across the toe of the angles based on the single fillet shown and "BOTH SIDES" referring to both angles. This was marked the first time by TT in the WS4 approval submittal, requesting a double fillet weld. All previous sequences have been shop issued with a single fillet weld. Confirm that one single fillet per angle is correct.						<b>ANSWER:</b>  The noted weld symbol has been interpreted as a weld only across the toe of the angles based on the single fillet shown and "BOTH SIDES" referring to both angles. This was marked the first time by TT in the WS4 approval submittal, requesting a double fillet weld. All previous sequences have been shop issued with a single fillet weld. Confirm that one single fillet per angle is correct.
<b>T-1782.1</b>	<b>SSS - Stair Elevator Weld Clarification from WS4 Package</b>	<b>Closed</b>	<b>CR</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>10/13/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 642 SK1: The noted weld symbol has been interpreted as a weld only across the toe of the angles based on the single fillet shown and "BOTH SIDES" referring to both angles. This was marked the first time by TT in the WS4 approval submittal, requesting a double fillet weld. All previous						<b>ANSWER:</b>  See attached CD RFI # 642 SK1: The noted weld symbol has been interpreted as a weld only across the toe of the angles based on the single fillet shown and "BOTH SIDES" referring to both angles. This was marked the first time by TT in the WS4 approval submittal, requesting a double fillet



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	sequences have been shop issued with a single fillet weld.  Confirm that one single fillet per angle is correct.					weld. All previous sequences have been shop issued with a single fillet weld. Confirm that one single fillet per angle is correct.
<b>T-1783</b>	<b>BGP - Partition Wall - Top of Pier Bracing</b>  <b>From:</b> Webcor Construction LP                      Claude Titcher	<b>Closed</b>	<b>01</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>10/16/2014</b>
	<b>REQUEST:</b>  Please see the attached clouded drawings, S1-9050, S1-9002, and S1-9001. Note 5 on detail 9/S1-9050 refers to detail 1/S1-9002 for the typical top of pier bracing details. Detail 3/S1-9001 depicts the bracing of an interior CMU/Concrete Pier below a metal deck. There are no other details shown on the contract drawings that depict bracing of interior CMU/Concrete pier below concrete concourse deck. Please confirm it is acceptable to omit the 3/8" thick plate on Detail 3/S 1-9001 for bracing below concrete concourse deck. Otherwise, please provide a detail for bracing the top of an interior concrete pier to the concrete deck.					<b>ANSWER:</b>  Please see the attached clouded drawings, S1-9050, S1-9002, and S1-9001. Note 5 on detail 9/S1-9050 refers to detail 1/S1-9002 for the typical top of pier bracing details. Detail 3/S1-9001 depicts the bracing of an interior CMU/Concrete Pier below a metal deck. There are no other details shown on the contract drawings that depict bracing of interior CMU/Concrete pier below concrete concourse deck. Please confirm it is acceptable to omit the 3/8" thick plate on Detail 3/S 1-9001 for bracing below concrete concourse deck. Otherwise, please provide a detail for bracing the top of an interior concrete pier to the concrete deck.
<b>T-1784</b>	<b>SSS - Shear Plate Connection Clarifications at Bus Deck Level GL 23.5</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	<b>Closed</b>	<b>CR</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>10/16/2014</b>
	<b>REQUEST:</b>  See attached CD RFI # 638 SK1: Confirm the connections per 7 & 8/S1-5010 with the shear plate per 1/S1-5011 for the W21x50 welded to the plates are acceptable as shown or supply a new detail.					<b>ANSWER:</b>  See attached CD RFI # 638 SK1: Confirm the connections per 7 & 8/S1-5010 with the shear plate per 1/S1-5011 for the W21x50 welded to the plates are acceptable as shown or supply a new detail.



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T-1785	BGP - Area 2 South Splice Bar Correction	Closed	01	10/06/2014	10/16/2014	10/13/2014
From: Webcor Construction LP Claude Titché						
REQUEST: The southern #9A42 splice bars that are being corrected in the field should have been placed during the previous pour (D205). During the correction for Decks C203 and D204, it was discovered that there was not enough room to insert these bars from the East side of GL-X due to the proximity of the pour joint. Per the attached drawings, please confirm the constructible reconfiguration of the splice bars acceptable.						ANSWER: The southern #9A42 splice bars that are being corrected in the field should have been placed during the previous pour (D205). During the correction for Decks C203 and D204, it was discovered that there was not enough room to insert these bars from the East side of GL-X due to the proximity of the pour joint. Per the attached drawings, please confirm the constructible reconfiguration of the splice bars acceptable.
T-1786	SSS - Weld Detail Comment Clarification at Drag Beam GL 2	Closed	CR	10/06/2014	10/16/2014	10/13/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: See attached CD RFI # 637 SK1: The approval comment is requesting verification for information that is not shown in detail 9/S1-8014. Confirm the weld shown applies to (4) sides.						ANSWER: See attached CD RFI # 637 SK1: The approval comment is requesting verification for information that is not shown in detail 9/S1-8014. Confirm the weld shown applies to (4) sides.
T-1787	SSS - WS5 Beam Paint Clarifications	Closed	CR	10/06/2014	10/16/2014	10/07/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST: See attached CD RFI # 643 SK1 for items 1 & 2: 1) Beams A11249 & A12166 in WS5 are not shown in drawings A1-8660 thru A1-8663 as requiring AESS/IFRM-1. Please clarify which architectural drawings indicate the noted finishes. 2) Supply specific information showing the extent of the AESS/IFRM-1 finishes on the noted (2) beams.						ANSWER: See attached CD RFI # 643 SK1 for items 1 & 2: 1) Beams A11249 & A12166 in WS5 are not shown in drawings A1-8660 thru A1-8663 as requiring AESS/IFRM-1. Please clarify which architectural drawings indicate the noted finishes. 2) Supply specific information showing the extent of the AESS/IFRM-1 finishes on the noted (2) beams.
T-1788	BGP - Drawing Discrepancy With Wall Above Bike and Vehicle Ramp Slab	Closed	CR	10/06/2014	10/16/2014	10/14/2014
From: Webcor Construction LP Claude Titché						
REQUEST: The Architectural Drawings including AI-7404, AI-7405, and AI-7407, shows TG0600 having no Vehicle or Bike Ramp walls above the ramp slabs. However, SI-2251 calls out Detail 5 on SI-3210, which shows a partition wall below						ANSWER: The Architectural Drawings including AI-7404, AI-7405, and AI-7407, shows TG0600 having no Vehicle or Bike Ramp walls above the ramp slabs. However, SI-2251 calls out Detail 5 on SI-3210, which shows a partition



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T-1789	the TG0600 scope line going from the top of the Vehicle Ramp slab to the bottom of the beams on the Ground Level. Please confirm which drawing set shows the correct scope line.	Closed	CR	10/06/2014	10/16/2014	10/17/2014
	SSS - ST401 Brace Connection and Dimension Clarifications					
From: Webcor Construction LP Gregory Kemerer						
REQUEST:						
See attached CD RFI # 586 SK1 & SK2 for items 1 & 2: 1) These braces extend up to the underside of the W27x84 on S1-2604 as shown in snapshot from the model. Confirm no further action is required. 2) Confirm the red approval dimensions shown on E51 (SK1) are intended to be a revision to the information supplied previously as noted.						
ANSWER:						
See attached CD RFI # 586 SK1 & SK2 for items 1 & 2: 1) These braces extend up to the underside of the W27x84 on S1-2604 as shown in snapshot from the model. Confirm no further action is required. 2) Confirm the red approval dimensions shown on E51 (SK1) are intended to be a revision to the information supplied previously as noted.						
T-1790		Closed	CR	10/06/2014	10/16/2014	10/13/2014
	SSS - Weld Clearance Holes					
From: Webcor Construction LP Gregory Kemerer						
REQUEST:						
XKT Engineering requests that the weld clearance holes located at column flange to web, weld transition points (full penetration to fillet welds), be eliminated (see attached drawing P1022).						
There is no code requirement for these holes. Welds would be detailed and welded in accordance with AWS D1.1, Section 2.8.2.						
Please confirm this is acceptable.						
ANSWER:						
XKT Engineering requests that the weld clearance holes located at column flange to web, weld transition points (full penetration to fillet welds), be eliminated (see attached drawing P1022).						
There is no code requirement for these holes. Welds would be detailed and welded in accordance with AWS D1.1, Section 2.8.2.						
Please confirm this is acceptable.						

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<b>T-1790.1</b>	<b>SSS - Weld Clearance Holes</b>	<b>Closed</b>	<b>CR</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/12/2014</b>
<div> <div> <b>From:</b> Webcor Construction LP Gregory Kemerer </div> <div> <b>REQUEST:</b> <p>This RFI addresses the information requested and the response given to SK RFI 867 (T-1790). To clarify the applicability of the reference code, Skanska's fabricator has provided details on the fabrication of the column assemblies:</p> <ul style="list-style-type: none"> <li>· The web welds (splices) are welded first and NDT is performed- no access holes needed at ends of welds.</li> <li>· Web plates are fit/assembled to flanges.</li> <li>· Flange to web welds are made - again, no access holes are required.</li> <li>· There is no welding being performed through access holes for these conditions.</li> </ul> <p>There is no code or project requirement to install weld access holes given the order of operations. AWS D1.1, Section 2.8.2 addresses weld transition from full penetration to fillet welds as required on these flange to web welds.</p> <p>Given that the three welds are not done concurrently, please confirm that it is acceptable to eliminate unnecessary access holes.</p> </div> <div> <b>ANSWER:</b> <p>This RFI addresses the information requested and the response given to SK RFI 867 (T-1790). To clarify the applicability of the reference code, Skanska's fabricator has provided details on the fabrication of the column assemblies:</p> <ul style="list-style-type: none"> <li>· The web welds (splices) are welded first and NDT is performed- no access holes needed at ends of welds.</li> <li>· Web plates are fit/assembled to flanges.</li> <li>· Flange to web welds are made - again, no access holes are required.</li> <li>· There is no welding being performed through access holes for these conditions.</li> </ul> <p>There is no code or project requirement to install weld access holes given the order of operations. AWS D1.1, Section 2.8.2 addresses weld transition from full penetration to fillet welds as required on these flange to web welds.</p> <p>Given that the three welds are not done concurrently, please confirm that it is acceptable to eliminate unnecessary access holes.</p> </div> </div>						
<b>T-1791</b>	<b>SCS - Foundation Wall Telecom Sleeve Elevations</b>	<b>Closed</b>	<b>01</b>	<b>10/06/2014</b>	<b>10/16/2014</b>	<b>10/15/2014</b>
<div> <div> <b>From:</b> Webcor Construction LP Claude Titche </div> <div> <b>REQUEST:</b> <p>Reference 4/TE1-5200, 1/TE1-8007 and 2/TE1-8007</p> <p>Attached slab edge drawing note lateral locations of telecom sleeves through the foundation wall. Section detail 2/TE1-8007 depicts two embedded sleeves, one directly above the other, alongside the moment frame beams. Details 4/TE1-5200 and 1/TE1-8007 detail the corresponding opening for TSER room to lie between the moment frame beams. Elevations for the foundation wall telecom sleeves are not provided in the above drawings.</p> <p>Please provide elevations for the foundation wall telecom sleeves.</p> </div> <div> <b>ANSWER:</b> <p>Reference 4/TE1-5200, 1/TE1-8007 and 2/TE1-8007</p> <p>Attached slab edge drawing note lateral locations of telecom sleeves through the foundation wall. Section detail 2/TE1-8007 depicts two embedded sleeves, one directly above the other, alongside the moment frame beams. Details 4/TE1-5200 and 1/TE1-8007 detail the corresponding opening for TSER room to lie between the moment frame beams. Elevations for the foundation wall telecom sleeves are not provided in the above drawings.</p> <p>Please provide elevations for the foundation wall telecom sleeves.</p> </div> </div>						



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<b>T-1792</b>	<b>BGP - Eastern Wall Rebar at Seismic Joint</b>	<b>Closed</b>	<b>01</b>	<b>10/07/2014</b>	<b>10/17/2014</b>	<b>10/13/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Please confirm that the attached drawings for rebar placement is acceptable for horizontal rebar placement at eastern wall - seismic joint location.						<b>ANSWER:</b> Please confirm that the attached drawings for rebar placement is acceptable for horizontal rebar placement at eastern wall - seismic joint location.
<b>T-1793</b>	<b>SCS - Exposed Concrete Roof Perimeter Wall Per ASI 0124</b>	<b>Closed</b>	<b>01</b>	<b>10/07/2014</b>	<b>10/17/2014</b>	<b>10/10/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> RFI # T-1697 requested confirmation for the use of snap ties for perimeter wall, scallop wall, and sky light wall locations. TJPA answer: "The Design Team does not object to the use of Snap-off ties only at skylight wall locations." Spec 031001 - 2.2 B. 7. h. allows the use of snap ties for walls that will not be exposed. Architectural drawings AI-5002 and AI-8604 -through 8613 shows the roof level perimeter walls and seismic walls as not being exposed due to the GFRC fascia prior to ASI 0124. Questions:  1. Please confirm that per ASI 0124 the concrete roof perimeter walls/scallops will now be required to meet the specification requirements for exposed concrete walls.  2. Please confirm that the concrete roof perimeter walls/scallops will not need to meet requirements for cast-in-place architectural concrete.						<b>ANSWER:</b> RFI # T-1697 requested confirmation for the use of snap ties for perimeter wall, scallop wall, and sky light wall locations. TJPA answer: "The Design Team does not object to the use of Snap-off ties only at skylight wall locations." Spec 031001 - 2.2 B. 7. h. allows the use of snap ties for walls that will not be exposed. Architectural drawings AI-5002 and AI-8604 -through 8613 shows the roof level perimeter walls and seismic walls as not being exposed due to the GFRC fascia prior to ASI 0124. Questions:  1. Please confirm that per ASI 0124 the concrete roof perimeter walls/scallops will now be required to meet the specification requirements for exposed concrete walls.  2. Please confirm that the concrete roof perimeter walls/scallops will not need to meet requirements for cast-in-place architectural concrete.
<b>T-1794</b>	<b>SSS - Manlift Leave Out Beams</b>	<b>Open</b>	<b>CR</b>	<b>10/07/2014</b>	<b>10/17/2014</b>	
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b> The Contractor intends to install (4) temporary manlifts during the construction of the building. To accomodate these manlifts, some structural beams and associated decking will need to be temporarily left out of the structure. These items will be installed after the manlifts are removed. Please review the attached sketches and confirm no additional temporary structural support will be						<b>ANSWER:</b> The Contractor intends to install (4) temporary manlifts during the construction of the building. To accomodate these manlifts, some structural beams and associated decking will need to be temporarily left out of the structure. These items will be installed after the manlifts are removed. Please review the attached sketches and confirm no additional temporary





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	<div><div>required within the leave out areas while the temporary manlifts are in place.</div><div>Beams to be temporarily left out are hilighted in yellow Decking areas to be temporarily left out are denoted with a red box</div></div>					
	<div><div>structural support will be required within the leave out areas while the temporary manlifts are in place.</div><div>Beams to be temporarily left out are hilighted in yellow Decking areas to be temporarily left out are denoted with a red box</div></div>					
T-1795	<div>SCS - Information Needed for Fascia Embed Layout</div> <div>From: Shimmick Construction Company, Inc. Henry Chiang</div> <div>REQUEST: The perimeter wall for both the rooftop and bus level GFRC fascia panel and support steel found on A 1-8601 to A 1-8611 do not show vertical dimensions or horizontal layout for the embed locations. Please provide layout with vertical dimensions, horizontal layout, and embed plate size for the embed locations equivalent to what is provided for the roof park railing details on sheets AI-8680 to AI-8685.</div>	Closed	01	10/07/2014	10/17/2014	10/07/2014
	<div>ANSWER: The perimeter wall for both the rooftop and bus level GFRC fascia panel and support steel found on A 1-8601 to A 1-8611 do not show vertical dimensions or horizontal layout for the embed locations. Please provide layout with vertical dimensions, horizontal layout, and embed plate size for the embed locations equivalent to what is provided for the roof park railing details on sheets AI-8680 to AI-8685.</div>					

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<b>REQUEST:</b>						
Section 07 09 16 - Part 1.6.E describes the requirements of the jobsite constructed Seismic Joint Assembly Mock-up. As discussed during TJPA Program QA/QC Coordination Meetings, confirm it is acceptable to employ "First-In-Place" interim inspection reviews of the actual seismic joint assembly installation in lieu of a job-site constructed mock-up.						
<b>ANSWER:</b>						
Section 07 09 16 - Part 1.6.E describes the requirements of the jobsite constructed Seismic Joint Assembly Mock-up. As discussed during TJPA Program QA/QC Coordination Meetings, confirm it is acceptable to employ "First-In-Place" interim inspection reviews of the actual seismic joint assembly installation in lieu of a job-site constructed mock-up.						
T-1801	BGP - D201&202 RCS-1 & RCS-7 Reinforcing Steel Field Change Confirmation	Closed	01	10/09/2014	10/19/2014	10/14/2014
From: Webcor Construction LP Claude Titche						
<b>REQUEST:</b>						
Please confirm the following: 1. Arrangement of RCS-1 top reinforcing hooks at the west foundation wall may be per the attached sketch.  2. Matching hooks may be spliced to RCS-7 top reinforcing above the knockout wall to conform to Detail 4/S1-3204.  3. Minimum splice length for RCS7 hooks is acceptable at 76" based on a 23" embedment of the typical bar into the foundation wall.  4. RCS-7 hooks may be omitted where congestion results in inadequate clear spacing. All such locations will be reviewed by the Structural Field Representative before inspection.  5. Top formsaver in Detail 4/S1-3204 may be relocated immediately below the top reinforcing of B60 to reduce top steel congestion in RCS-7.						
<b>ANSWER:</b>						
Please confirm the following: 1. Arrangement of RCS-1 top reinforcing hooks at the west foundation wall may be per the attached sketch.  2. Matching hooks may be spliced to RCS-7 top reinforcing above the knockout wall to conform to Detail 4/S1-3204.  3. Minimum splice length for RCS7 hooks is acceptable at 76" based on a 23" embedment of the typical bar into the foundation wall.  4. RCS-7 hooks may be omitted where congestion results in inadequate clear spacing. All such locations will be reviewed by the Structural Field Representative before inspection.  5. Top formsaver in Detail 4/S1-3204 may be relocated immediately below the top reinforcing of B60 to reduce top steel congestion in RCS-7.						



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<b>T-1802</b>	<b>BGP - Area 16- East Wall Vertical Reinforcing- CDSM Encroachment</b>	<b>Closed</b>	<b>01</b>	<b>10/09/2014</b>	<b>10/19/2014</b>	<b>10/14/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titche</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Portions of the East Wall have CDSM encroachments -per RFI T-1372 and approved submittals require 6" spacing of vertical reinforcing.			Portions of the East Wall have CDSM encroachments -per RFI T-1372 and approved submittals require 6" spacing of vertical reinforcing.			
The original call out was for 17 Verts each face between GL C & C.3 and between GL G & G.2 (at 6" OC). We installed 13 each face in both sections (at 8" OC)- see attached drawing. Please confirm it is acceptable for the 8" spacing to remain and there is no further action needed.			The original call out was for 17 Verts each face between GL C & C.3 and between GL G & G.2 (at 6" OC). We installed 13 each face in both sections (at 8" OC)- see attached drawing. Please confirm it is acceptable for the 8" spacing to remain and there is no further action needed.			
<b>T-1803</b>	<b>SCS - Roof Edge of Deck Bent Plate</b>	<b>Closed</b>	<b>01</b>	<b>10/09/2014</b>	<b>10/19/2014</b>	<b>10/15/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titche</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
S1-5000 detail 8 & 9 shows a bent plate as the typical edge of deck detail. The bent plate is also shown on SI-8008 for the sky light opening. SI-3282 details do not show a bent plate for the rooftop seismic detail 3, perimeter detail 2, and scallop wall detail 4 & 5.			S1-5000 detail 8 & 9 shows a bent plate as the typical edge of deck detail. The bent plate is also shown on SI-8008 for the sky light opening. SI-3282 details do not show a bent plate for the rooftop seismic detail 3, perimeter detail 2, and scallop wall detail 4 & 5.			
1. Please confirm that the roof top seismic, perimeter, and scallop walls will have a bent plate per detail 8&9/S1-5000.			1. Please confirm that the roof top seismic, perimeter, and scallop walls will have a bent plate per detail 8&9/S1-5000.			
2. Please provide detail on what the scallop bent plate will look like ( 4&5/S1-3282).			2. Please provide detail on what the scallop bent plate will look like ( 4&5/S1-3282).			
3. Please confirm that it is acceptable for Shimmick to place a 1 inch hole in the bent plate to install a removable tie for overhang brackets. Holes for overhang brackets will be roughly 6' O.C.			3. Please confirm that it is acceptable for Shimmick to place a 1 inch hole in the bent plate to install a removable tie for overhang brackets. Holes for overhang brackets will be roughly 6' O.C.			
<b>T-1804</b>	<b>SSS - Kicker Brace Fouling TR31.7 Stiffener</b>	<b>Closed</b>	<b>CR</b>	<b>10/10/2014</b>	<b>10/20/2014</b>	<b>10/21/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 645 SK1: It is not possible to fit the erection bolt and make the weld			See attached CD RFI # 645 SK1: It is not possible to fit the erection bolt and make the weld			



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	at the bottom of the kicker between the gusset plate per 5/S1-5015 and the PL 2 1/2" stiffener above the column flange. Confirm it is acceptable to omit the fouling kicker angle and detail a single angle kicker at this location and other similar locations.					
T-1805	SSS - Shear Plate Fouling Column Bolts at TR24	Closed	CR	10/10/2014	10/20/2014	10/21/2014
	From: Webcor Construction LP Gregory Kemerer					
	REQUEST: See attached CD RFI # 646 SK1: The shear plate will foul the bolts in the bottom flange of TR24 if the beams are spaced equally as shown on S1-2305. To avoid the fouling, confirm it is acceptable to move the noted W16x26 north by 1 3/4". If not, supply an alternate solution.					
T-1806	SSS - Shear Plate Connection Confirmation at Roof Level GL 24	Closed	CR	10/10/2014	10/20/2014	10/21/2014
	From: Webcor Construction LP Gregory Kemerer					
	REQUEST: See attached CD RFI # 624 SK1:  Confirm the shear plate connections for the W21x50's as shown on plan are welded to the PL 2 1/2" per 7/S1-5027 & 8/S1-5025 as shown. If not, supply a new detail.					





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	<p>Per conference call with Thornton Tomasetti on 10/9/14, the clearance for the top bars would need to be 2.75"(CLR) + .50"(#4 STIRRUP) + 1 .50"(#11 REBAR-MAX SIZE) = 4.75". 4.75" top rebar clearance can be applied as a typical for the top bar in 4/S 1-5022.</p> <p>Please confirm.</p>					<p>Per conference call with Thornton Tomasetti on 10/9/14, the clearance for the top bars would need to be 2.75"(CLR) + .50"(#4 STIRRUP) + 1 .50"(#11 REBAR-MAX SIZE) = 4.75". 4.75" top rebar clearance can be applied as a typical for the top bar in 4/S 1-5022.</p> <p>Please confirm.</p>
<b>T-1810</b>	<b>SSS - Grouting Train Box Column Base Plates</b>	<b>Closed</b>	<b>CR</b>	<b>10/13/2014</b>	<b>10/13/2014</b>	<b>10/16/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>Structural Steel specification part 3.3E, requires the column base plates to be grouted immediately after plumbing the first tier of columns.</p> <p>Skanska's erection plans prepared by our structural engineer indicate that the grouting of the columns shall be done after erecting the EBF columns and tie-in steel at the bus deck and prior to setting the ancillary bus deck fill-in steel.</p> <p>See EP3.0 and EP3.1 attached for reference. This is done to allow for flexibility when plumbing and aligning the EBF steel.</p> <p>Please confirm that Skanska has the option to follow the EP3.0 notes for the timing of the column grouting operation.</p>						<b>ANSWER:</b> <p>Structural Steel specification part 3.3E, requires the column base plates to be grouted immediately after plumbing the first tier of columns.</p> <p>Skanska's erection plans prepared by our structural engineer indicate that the grouting of the columns shall be done after erecting the EBF columns and tie-in steel at the bus deck and prior to setting the ancillary bus deck fill-in steel.</p> <p>See EP3.0 and EP3.1 attached for reference. This is done to allow for flexibility when plumbing and aligning the EBF steel.</p> <p>Please confirm that Skanska has the option to follow the EP3.0 notes for the timing of the column grouting operation.</p>
<b>T-1811</b>	<b>SSS - Missing Connection Details at Roof Level GL 27.1-29</b>	<b>Closed</b>	<b>CR</b>	<b>10/13/2014</b>	<b>10/13/2014</b>	<b>10/23/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> <p>See attached CD RFI # 644 SK1: Confirm the connection as shown is acceptable or supply a new detail.</p>						<b>ANSWER:</b> <p>See attached CD RFI # 644 SK1: Confirm the connection as shown is acceptable or supply a new detail.</p>





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<b>T-1812</b>	<b>SSS - PE503 Fouling Brace at Slab Opening</b>	<b>Closed</b>	<b>CR</b>	<b>10/13/2014</b>	<b>10/13/2014</b>	<b>10/28/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 649 SK1: The brace per 1/S1-7600 fouls the slab opening. Please provide a solution.					<b>ANSWER:</b> See attached CD RFI # 649 SK1: The brace per 1/S1-7600 fouls the slab opening. Please provide a solution.	
<b>T-1813</b>	<b>SSS - SE201 &amp; SE202 Edge Plate Approval Comment Clarification GL 4-5</b>	<b>Closed</b>	<b>CR</b>	<b>10/13/2014</b>	<b>10/13/2014</b>	<b>10/23/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 651 SK1: A slab opening is not shown on S1-2602 or A1-2892. 7/A1-2969 shows a series of circular openings but they are not included in the edge plate requirement per 8/S1-5002. Confirm the edge plate is not required or supply revised contract documents.					<b>ANSWER:</b> See attached CD RFI # 651 SK1: A slab opening is not shown on S1-2602 or A1-2892. 7/A1-2969 shows a series of circular openings but they are not included in the edge plate requirement per 8/S1-5002. Confirm the edge plate is not required or supply revised contract documents.	
<b>T-1814</b>	<b>SSS - Approval Comment Clarification at Connection GL 4-5</b>	<b>Closed</b>	<b>CR</b>	<b>10/13/2014</b>	<b>10/13/2014</b>	<b>10/28/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 652 SK1: The connections as submitted are correct per 4/S1-5011 Note 3. Is the intent of the comment to revise the connections?					<b>ANSWER:</b> See attached CD RFI # 652 SK1: The connections as submitted are correct per 4/S1-5011 Note 3. Is the intent of the comment to revise the connections?	
<b>T-1815</b>	<b>SSS - SLRS Missing Connection Details GL 27</b>	<b>Closed</b>	<b>CR</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 653 SK1 & SK2 for items 1 to 4: 1) Supply a connection detail for the L6x6x3/8 at the Type 3 drag connection per S1-5018. 2) 1/S1-5018 does not provide a detail for beams of varying depth. Supply a new detail. 3) Confirm it is acceptable to reduce the edge distance to 4" to avoid the plates fouling each other or supply a new detail. 4) The diagonal gusset plate per 1C/S1-5018 and the square gusset plate per 1A/S1-5018 foul each other as					<b>ANSWER:</b> See attached CD RFI # 653 SK1 & SK2 for items 1 to 4: 1) Supply a connection detail for the L6x6x3/8 at the Type 3 drag connection per S1-5018. 2) 1/S1-5018 does not provide a detail for beams of varying depth. Supply a new detail. 3) Confirm it is acceptable to reduce the edge distance to 4" to avoid the plates fouling each other or supply a new detail. 4) The diagonal gusset plate per 1C/S1-5018 and the	



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	shown. Supply a new detail.					square gusset plate per 1A/S1-5018 foul each other as shown. Supply a new detail.
T-1815.1	SSS - SLRS Missing Connection Details GL 27	Closed	CR	11/14/2014	11/24/2014	11/24/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:			ANSWER:			
This is a follow-up RFI to RFI T-1815 (SK 878, CD 653) See attached CD RFI # 653.1 SK1 for items 1 & 2: 1) The response to item 1 in RFI T-1815 (SK 878, CD 653) does not work because the vertical leg of the angle cannot be placed between the 1 1/2 dia. bolts per S1-5018 and the angle cannot be welded to the beam flange as it fouls the web stiffener plates per S1-5016. Review and supply a workable solution.  2) The (3) noted bolts for the bottom tie plate per 1/S1-5018 extend past the end of the coped bottom beam flange or do not meet the required edge distance. Provide a solution.			This is a follow-up RFI to RFI T-1815 (SK 878, CD 653) See attached CD RFI # 653.1 SK1 for items 1 & 2: 1) The response to item 1 in RFI T-1815 (SK 878, CD 653) does not work because the vertical leg of the angle cannot be placed between the 1 1/2 dia. bolts per S1-5018 and the angle cannot be welded to the beam flange as it fouls the web stiffener plates per S1-5016. Review and supply a workable solution.  2) The (3) noted bolts for the bottom tie plate per 1/S1-5018 extend past the end of the coped bottom beam flange or do not meet the required edge distance. Provide a solution.			



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<b>T-1815.2</b>	<b>SSS - SLRS Missing Connection Details GL 27</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2014</b>	<b>12/20/2014</b>	<b>12/24/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Contract Doc Ref: S1-2506 Location: Zone 4, Bus Deck Grid Line: C & 27.1 Add'l Doc Ref's: CD RFI # 653.2 SK1  Drawing S1-2506 shows W40 members connecting near grid lines C and 27.1.  Connection information is missing for the tie plate to gusset plates as indicated below.  1) Supply details for the field weld between the 1 3/4" tie plate to the 1 3/4" gusset plate. 2) Supply details for the field weld between the 2 1/4" tie plate to the 1 3/4" gusset plate. Note that the plates do not align due to the varying beam depth. NOTE: this RFI applies to all similar connections between Grid 27.1 - 33.5 on the Bus Deck Level.						<b>ANSWER:</b> Contract Doc Ref: S1-2506 Location: Zone 4, Bus Deck Grid Line: C & 27.1 Add'l Doc Ref's: CD RFI # 653.2 SK1  Drawing S1-2506 shows W40 members connecting near grid lines C and 27.1.  Connection information is missing for the tie plate to gusset plates as indicated below.  1) Supply details for the field weld between the 1 3/4" tie plate to the 1 3/4" gusset plate. 2) Supply details for the field weld between the 2 1/4" tie plate to the 1 3/4" gusset plate. Note that the plates do not align due to the varying beam depth. NOTE: this RFI applies to all similar connections between Grid 27.1 - 33.5 on the Bus Deck Level.
<b>T-1816</b>	<b>SSS - Conflicting Information at North &amp; South Exit Mezzanines GL 20.1</b>	<b>Closed</b>	<b>CR</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/28/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 655 SK1: This note states that the hangers are to be located below the beams on the Ground Level. Per S1-6050 and RFI T-1573 (SK 732, CD 537) the beam is located 8'-10 3/4" as is shown in this snapshot from the model. However, the yellow dimension notes that the dimension is not to exceed 8'-0". Please clarify this conflicting information.						<b>ANSWER:</b> See attached CD RFI # 655 SK1: This note states that the hangers are to be located below the beams on the Ground Level. Per S1-6050 and RFI T-1573 (SK 732, CD 537) the beam is located 8'-10 3/4" as is shown in this snapshot from the model. However, the yellow dimension notes that the dimension is not to exceed 8'-0". Please clarify this conflicting information.
<b>T-1817</b>	<b>SSS - Conflicting Dimensions at Second Level GL 14-15</b>	<b>Closed</b>	<b>CR</b>	<b>10/13/2014</b>	<b>10/23/2014</b>	<b>10/16/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 656 SK2 for item 2:  2) This string of dimensions does not add up to 42'-6". Confirm the dimensions as marked are acceptable as this steel is currently being fabricated.						<b>ANSWER:</b> See attached CD RFI # 656 SK2 for item 2:  2) This string of dimensions does not add up to 42'-6". Confirm the dimensions as marked are acceptable as this steel is currently being fabricated.



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1818</b>	<b>SCS - Roof Top Door Opening</b>	<b>Closed</b>	<b>CR</b>	<b>10/14/2014</b>	<b>10/14/2014</b>	<b>10/16/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
A1-7013 Detail 1 plan view for stair 401 calls out door 03401 found on A 1-9706. The door height is 7'-0" and the width is 7'-0". A-0035 shows the door frames as 2" thick. Detail 1 of A1-0035 shows a 3/16 inch gap between the frame and the concrete wall.			A1-7013 Detail 1 plan view for stair 401 calls out door 03401 found on A 1-9706. The door height is 7'-0" and the width is 7'-0". A-0035 shows the door frames as 2" thick. Detail 1 of A1-0035 shows a 3/16 inch gap between the frame and the concrete wall.			
1. Please confirm that Detail 1 is the correct detail for the roof top concrete wall door openings.			1. Please confirm that Detail 1 is the correct detail for the roof top concrete wall door openings.			
2. Please confirm that the rough opening for door 401 is 7'- 4 3/8" wide and 7'- 2 3/16" tall.			2. Please confirm that the rough opening for door 401 is 7'- 4 3/8" wide and 7'- 2 3/16" tall.			
3. Please confirm that roof top door openings in concrete walls will always be 4 3/8 inch width and 2 3/16 inch height larger than the doors called out on the door schedule.			3. Please confirm that roof top door openings in concrete walls will always be 4 3/8 inch width and 2 3/16 inch height larger than the doors called out on the door schedule.			
<b>T-1819</b>	<b>SCS - Grout Testing Requirements</b>	<b>Closed</b>	<b>01</b>	<b>10/14/2014</b>	<b>10/24/2014</b>	<b>10/15/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
According to Exhibit A, the contractor for TG07.2 will" Grout all TG07.1 R Structural Steel column base plates and anchor bolts ... ".			According to Exhibit A, the contractor for TG07.2 will" Grout all TG07.1 R Structural Steel column base plates and anchor bolts ... ".			
On Section 03 30 02 of the Specifications, title 2.8 MISCELLANEOUS CONCRETE PRODUCTS, it is indicated " Grout for base plate grouting shall be flowable consistency and have minimum compressive strength of 8000 psi at 28 days or as indicated on drawings" No information regarding testing requirements was found.			On Section 03 30 02 of the Specifications, title 2.8 MISCELLANEOUS CONCRETE PRODUCTS, it is indicated " Grout for base plate grouting shall be flowable consistency and have minimum compressive strength of 8000 psi at 28 days or as indicated on drawings" No information regarding testing requirements was found.			
Please provide information about testing requirements for Grout on Column Base Plates.			Please provide information about testing requirements for Grout on Column Base Plates.			
<b>T-1820</b>	<b>SSS - Fouling Stiffeners at CP8 Connection</b>	<b>Closed</b>	<b>CR</b>	<b>10/15/2014</b>	<b>10/25/2014</b>	<b>10/30/2014</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Gregory Kemerer</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please confirm if it is acceptable to eliminate the (6) 1" thick stiffeners at the CP8 connection due to their			Please confirm if it is acceptable to eliminate the (6) 1" thick stiffeners at the CP8 connection due to their			





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1822</b>	<b>BGP - Seismic Joint Vertical Wall Reinforcing</b>	<b>Closed</b>	<b>01</b>	<b>10/15/2014</b>	<b>10/25/2014</b>	<b>10/29/2014</b>
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div><div><b>REQUEST:</b><p>The configuration of welded reinforcing on the train box side of the seismic joint is installed as if there were an adjacent horizontal trough similar to the horizontal leg of the joint at the mat slab. There is no such trough on the walls. To accommodate the existing vertical west embed please see the following:</p><ol style="list-style-type: none"><li>1. An additional #7 hook will be added to the foundation wall that will engage the welded angled bar and the welded deformed anchor - See Sketch #1</li><li>2. As necessary to provide 2" clear to the waterproofing or as is required to place foundation wall reinforcing, the welded angled hook may be field bent. Bending will be controlled as to not stress the bar end weld and bend radius will not be less than ACI minimum. See Sketch #1</li><li>3. Termination of north and south foundation wall horizontal reinforcing shall be hooked. Hooks shall be oriented per the attached sketch. See Sketch # 1.</li><li>4. The interior leg of 5/8" plate terminating at Grid A may be omitted and replaced with a formed edge-See Sketch # 1, reference AI-8881 detail 5.</li><li>5. Please confirm that the South East Termination Bars can be installed per attached Sketch #2 .</li><li>6. Please confirm that the North East Comer of the East Wall: Within the projection of the seismic joint toward the eastern CDSM wall, the interior foundation wall cross ties were not installed. Please confirm that this field condition is acceptable and that a uniformly detailed east foundation wall will resume in the second lift per the attached sketch #3.</li></ol><p>S1-3010 does not depict detail of terminated horizontal bars along North and South wall and Seismic Joint embed. Please approve the following sketches.</p></div><div><b>ANSWER:</b><p>The configuration of welded reinforcing on the train box side of the seismic joint is installed as if there were an adjacent horizontal trough similar to the horizontal leg of the joint at the mat slab. There is no such trough on the walls. To accommodate the existing vertical west embed please see the following:</p><ol style="list-style-type: none"><li>1. An additional #7 hook will be added to the foundation wall that will engage the welded angled bar and the welded deformed anchor - See Sketch #1</li><li>2. As necessary to provide 2" clear to the waterproofing or as is required to place foundation wall reinforcing, the welded angled hook may be field bent. Bending will be controlled as to not stress the bar end weld and bend radius will not be less than ACI minimum. See Sketch #1</li><li>3. Termination of north and south foundation wall horizontal reinforcing shall be hooked. Hooks shall be oriented per the attached sketch. See Sketch # 1.</li><li>4. The interior leg of 5/8" plate terminating at Grid A may be omitted and replaced with a formed edge-See Sketch # 1, reference AI-8881 detail 5.</li><li>5. Please confirm that the South East Termination Bars can be installed per attached Sketch #2 .</li><li>6. Please confirm that the North East Comer of the East Wall: Within the projection of the seismic joint toward the eastern CDSM wall, the interior foundation wall cross ties were not installed. Please confirm that this field condition is acceptable and that a uniformly detailed east foundation wall will resume in the second lift per the attached sketch #3.</li></ol><p>S1-3010 does not depict detail of terminated horizontal bars along North and South wall and Seismic Joint embed. Please approve the following sketches.</p></div></div>						
<b>T-1823</b>	<b>BGP - Upturned Beam Conflict at GL 21</b>	<b>Closed</b>	<b>01</b>	<b>10/16/2014</b>	<b>10/26/2014</b>	<b>10/17/2014</b>



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T-1824	<b>From:</b> Webcor Construction LP  <b>REQUEST:</b>  Reference: various attached drawings.  Sheet S1-2205 shows an upturned beam at GL 21. Due to a conflict with the internal bracing system please confirm it is acceptable to pour the upturned beam portion separately following the D221 pour with a horizontal cold joint between D221 and the upturned beam.  Skanska's Anchor Bolt Layout Plan TG0702-070.4 sheet AB14 detail 18 shows the upturned beam column anchor bolts embedded 4'-0 1/2" into concrete. Detail 5 on S1-3621 shows the height of a typical upturned beam at 3'-3" above top of Lower Concourse concrete. This results in the anchor bolts being embedded through the upturned beam and the Lower Concourse. Please confirm it is acceptable to utilize couplers as seen in attached SKA to facilitate anchor bolt installation in this specified location.	Closed	CR	10/16/2014	10/16/2014	10/17/2014
	<b>ANSWER:</b>  Reference: various attached drawings.  Sheet S1-2205 shows an upturned beam at GL 21. Due to a conflict with the internal bracing system please confirm it is acceptable to pour the upturned beam portion separately following the D221 pour with a horizontal cold joint between D221 and the upturned beam.  Skanska's Anchor Bolt Layout Plan TG0702-070.4 sheet AB14 detail 18 shows the upturned beam column anchor bolts embedded 4'-0 1/2" into concrete. Detail 5 on S1-3621 shows the height of a typical upturned beam at 3'-3" above top of Lower Concourse concrete. This results in the anchor bolts being embedded through the upturned beam and the Lower Concourse. Please confirm it is acceptable to utilize couplers as seen in attached SKA to facilitate anchor bolt installation in this specified location.					
T-1824	<b>SSS - Column Base Plate Grout Pads</b>  <b>From:</b> Webcor Construction LP  <b>REQUEST:</b>  Skanska and their construction engineer, Hassett have evaluated several options for setting and securing the train box columns in position until the grouting operation has been complete. Based on Hassett's calculations the only feasible option to adequately stabilize the column is to utilize shim packs at each corner of the base plate.  Due to the congestion of rebar at the base plate block-out, there is currently insufficient room to set shim packs on a flat surface, therefore Skanska propose to pour 10"x10" grout pads at each corner to achieve the required uniformed surface for shim packs, see attached SK-5.0 for clarification.  Skanska propose to use TG07.2's Dayton Superior grout, previously approved in submittal TG0702-500.  Please confirm this is acceptable.	Closed	CR	10/16/2014	10/16/2014	10/17/2014
	<b>ANSWER:</b>  Skanska and their construction engineer, Hassett have evaluated several options for setting and securing the train box columns in position until the grouting operation has been complete. Based on Hassett's calculations the only feasible option to adequately stabilize the column is to utilize shim packs at each corner of the base plate.  Due to the congestion of rebar at the base plate block-out, there is currently insufficient room to set shim packs on a flat surface, therefore Skanska propose to pour 10"x10" grout pads at each corner to achieve the required uniformed surface for shim packs, see attached SK-5.0 for clarification.  Skanska propose to use TG07.2's Dayton Superior grout, previously approved in submittal TG0702-500.  Please confirm this is acceptable.					





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<b>T-1825</b>	<b>SSS - Connection Clarifications at Roof Level GL 27</b>	<b>Closed</b>	<b>CR</b>	<b>10/17/2014</b>	<b>10/17/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 654 SK1 & SK2 for items 1 to 2: 1) Since the PL 3" bend at Grid E.6 on Grid 30, the column will have to drop as shown. Confirm it is acceptable to drop the continuity plate as shown and moment weld the beam to the PL 3 1/2" or supply a new detail. 2) Confirm the PL 3" per 8/S1-5020 is acceptable as shown on Grid 28.			See attached CD RFI # 654 SK1 & SK2 for items 1 to 2: 1) Since the PL 3" bend at Grid E.6 on Grid 30, the column will have to drop as shown. Confirm it is acceptable to drop the continuity plate as shown and moment weld the beam to the PL 3 1/2" or supply a new detail. 2) Confirm the PL 3" per 8/S1-5020 is acceptable as shown on Grid 28.			
<b>T-1826</b>	<b>SSS - Fouling Stiffeners at East Zone Roof Level</b>	<b>Closed</b>	<b>CR</b>	<b>10/20/2014</b>	<b>10/20/2014</b>	<b>11/03/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 659 SK1 to SK3 for items 1 to 5: 1) The web stiffener plates per 2/S1-5016 with revised length per ASI-127 will extend into the noted beam connections per 1/S1-5010. Please supply a new detail. 2) The web stiffener plates per 2/S1-5016 with revised length per ASI-127 will extend into the noted beam connections per 1/S1-5010. Please supply a new detail. 3) The web stiffener plates per 2/S1-5016 with revised length per ASI-127 will extend into the noted beam connections per 1/S1-5010. Please supply a new detail. 4) The web stiffener plates per 2/S1-5016 with revised length per ASI-127 will extend into the noted beam connections per 1/S1-5010. Please supply a new detail. 5) The web stiffener plates per 2/S1-5016 with revised length per ASI-127 will extend into the noted beam connections per 1/S1-5010. Please supply a new detail.			See attached CD RFI # 659 SK1 to SK3 for items 1 to 5: 1) The web stiffener plates per 2/S1-5016 with revised length per ASI-127 will extend into the noted beam connections per 1/S1-5010. Please supply a new detail. 2) The web stiffener plates per 2/S1-5016 with revised length per ASI-127 will extend into the noted beam connections per 1/S1-5010. Please supply a new detail. 3) The web stiffener plates per 2/S1-5016 with revised length per ASI-127 will extend into the noted beam connections per 1/S1-5010. Please supply a new detail. 4) The web stiffener plates per 2/S1-5016 with revised length per ASI-127 will extend into the noted beam connections per 1/S1-5010. Please supply a new detail. 5) The web stiffener plates per 2/S1-5016 with revised length per ASI-127 will extend into the noted beam connections per 1/S1-5010. Please supply a new detail.			
<b>T-1826.1</b>	<b>SSS - Fouling Stiffeners at East Zone Roof Level</b>	<b>Closed</b>	<b>CR</b>	<b>12/15/2014</b>	<b>12/25/2014</b>	<b>01/06/2015</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			





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	<p>Contract Doc Ref: S1-2606 Location: Zone 4, Roof Grid Line: E.6 &amp; 31, Similar condition at GL 24.9 Add'l Doc Ref's: SK1, RFI T-1826 Response</p> <p>Drawing S1-2606 shows a tapered girder (TPG1) connection near GL E.6 and 31.</p> <p>The web stiffeners at this location will foul additional beams as shown in SK1.</p> <p>Confirm the web stiffeners may be reduced by 2" as indicated by the dotted line to avoid fouling additional beams. This will also apply to Grid 24.9.</p>					
	<p>Contract Doc Ref: S1-2606 Location: Zone 4, Roof Grid Line: E.6 &amp; 31, Similar condition at GL 24.9 Add'l Doc Ref's: SK1, RFI T-1826 Response</p> <p>Drawing S1-2606 shows a tapered girder (TPG1) connection near GL E.6 and 31.</p> <p>The web stiffeners at this location will foul additional beams as shown in SK1.</p> <p>Confirm the web stiffeners may be reduced by 2" as indicated by the dotted line to avoid fouling additional beams. This will also apply to Grid 24.9.</p>					
<b>T-1826.2</b>	<b>SSS - Fouling Stiffeners at East Zone Roof Level</b>	<b>Closed</b>	<b>CR</b>	<b>12/19/2014</b>	<b>12/29/2014</b>	<b>12/27/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: S1-2606 Location: Zone 4, Roof Grid Line: F &amp; 31 Add'l Doc Ref's: CD RFI # 659.1 SK1, RFI T-1826 Response, Response to Skanska Internal RFI 888.1A</p> <p>The response to RFI T-1826 provided clarification for web stiffener plates. This response reduced the clearance between the top and bottom edges of the deeper web stiffener plates and the flange to web welds of the Tapered Roof Girders.</p> <p>Per this response, the original 7/16" fillet weld will not work due to lack of clearance between the edge of the deeper stiffener plates and the flange to web weld of the tapered roof girder.</p> <p>Please advise.</p>					
	<p>Contract Doc Ref: S1-2606 Location: Zone 4, Roof Grid Line: F &amp; 31 Add'l Doc Ref's: CD RFI # 659.1 SK1, RFI T-1826 Response, Response to Skanska Internal RFI 888.1A</p> <p>The response to RFI T-1826 provided clarification for web stiffener plates. This response reduced the clearance between the top and bottom edges of the deeper web stiffener plates and the flange to web welds of the Tapered Roof Girders.</p> <p>Per this response, the original 7/16" fillet weld will not work due to lack of clearance between the edge of the deeper stiffener plates and the flange to web weld of the tapered roof girder.</p> <p>Please advise.</p>					
<b>T-1827</b>	<b>SSS - Missing Connection Detail at Bus Level Protected Zone GL 2</b>	<b>Closed</b>	<b>CR</b>	<b>10/20/2014</b>	<b>10/30/2014</b>	<b>10/31/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p>					



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	<b>REQUEST:</b> See attached CD RFI # 660 SK1: Supply a connection detail for the C8x11.5 to the beam inside the protected zone.					<b>ANSWER:</b> See attached CD RFI # 660 SK1: Supply a connection detail for the C8x11.5 to the beam inside the protected zone.
T-1828	<b>SSS - Elevator Post Baseplate Clearance</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	Closed	CR	10/20/2014	10/20/2014	11/03/2014
	<b>REQUEST:</b> Please see attached SK1, 11/S1-7600 and note the following:  The hole at the bottom of the HSS post is for a 3" pipe with a 3 1/2" O.D. Currently a 3 17/32" hole is detailed in the baseplate leaving a 1/32" clearance. This clearance tolerance will cause erection difficulty.  Please confirm the hole can be enlarged to 3 3/4" in the baseplate.					<b>ANSWER:</b> Please see attached SK1, 11/S1-7600 and note the following:  The hole at the bottom of the HSS post is for a 3" pipe with a 3 1/2" O.D. Currently a 3 17/32" hole is detailed in the baseplate leaving a 1/32" clearance. This clearance tolerance will cause erection difficulty.  Please confirm the hole can be enlarged to 3 3/4" in the baseplate.
T-1829	<b>SSS - ST201A &amp; PE201 Missing Base Plate Information at Ground Level</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	Closed	CR	10/20/2014	10/20/2014	11/03/2014
	<b>REQUEST:</b> See attached CD RFI # 663 SK1 for items 1 to 3: 1) Confirm the noted (3) column locations will have base plate TYPE I A orientated as shown. 2) Confirm the noted (3) column locations will have base plate TYPE I B as shown. 3) It is not clear what the orientation of the noted column base plate is. Please supply the missing base plate orientation for TYPE II A.					<b>ANSWER:</b> See attached CD RFI # 663 SK1 for items 1 to 3: 1) Confirm the noted (3) column locations will have base plate TYPE I A orientated as shown. 2) Confirm the noted (3) column locations will have base plate TYPE I B as shown. 3) It is not clear what the orientation of the noted column base plate is. Please supply the missing base plate orientation for TYPE II A.



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<b>T-1830</b>	<b>SSS - SE502 &amp; SE503 Weld Connection Clarification at HSS Connections</b>	<b>Closed</b>	<b>CR</b>	<b>10/20/2014</b>	<b>10/20/2014</b>	<b>11/03/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> The HSS18x6x5/8 does not totally bear on the HSS12x6x5/8 as shown and therefore the fillet weld on both sides per 2/S1-7630 is not possible. Confirm a fillet weld on one side is sufficient or supply a new detail.						<b>ANSWER:</b> The HSS18x6x5/8 does not totally bear on the HSS12x6x5/8 as shown and therefore the fillet weld on both sides per 2/S1-7630 is not possible. Confirm a fillet weld on one side is sufficient or supply a new detail.
<b>T-1831</b>	<b>SSS - PE201 Missing Beam Information at Roof Level</b>	<b>Closed</b>	<b>CR</b>	<b>10/20/2014</b>	<b>10/20/2014</b>	<b>11/03/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 665 SK1: Identify the (4) members hi-lited in yellow.						<b>ANSWER:</b> See attached CD RFI # 665 SK1: Identify the (4) members hi-lited in yellow.
<b>T-1832</b>	<b>SSS - ST201A Slab Opening Fouling Steel</b>	<b>Closed</b>	<b>CR</b>	<b>10/20/2014</b>	<b>10/20/2014</b>	<b>11/03/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 666 SK1: The slab opening as shown on A1-2964 fouls the steel. Please provide a solution.						<b>ANSWER:</b> See attached CD RFI # 666 SK1: The slab opening as shown on A1-2964 fouls the steel. Please provide a solution.
<b>T-1833</b>	<b>SSS - SLRS Missing Connection Details at Second Level GL 2</b>	<b>Closed</b>	<b>CR</b>	<b>10/20/2014</b>	<b>10/30/2014</b>	<b>10/28/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 667 SK1: S1-5010 does not provide a detail for a skewed double angle connection when the connection occurs on each side as details 7 & 8/S1-5010 are not symmetrical. Confirm the connection as shown is acceptable or supply a new detail.						<b>ANSWER:</b> See attached CD RFI # 667 SK1: S1-5010 does not provide a detail for a skewed double angle connection when the connection occurs on each side as details 7 & 8/S1-5010 are not symmetrical. Confirm the connection as shown is acceptable or supply a new detail.



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	<p>described in RFI T 1558.</p> <p>2) The east end rebracing sequence is also the sequence performed successfully in west end of the transit center excavation Zone 1.</p> <p>3) The east end rebracing will have raker bracing similar to the west end, the east end does again have additional reinforcing bar in structural concrete walls where rebracing rakers are supported per details on sheet SH-5301 in Submittal TG0300-553.</p> <p>4) The east end rebracing cross-lot or perpendicular sections are in accordance with submittal TG0300-553 and as has previously and successfully re-braced in zones 1, 2, and 3.</p> <p>If acceptable, please specify 1) the movement tolerance for a given waler removed, 2) the observation period for analyzing movement, and 3) the acceptable means for monitoring for movement (survey, inclinometer, or both).</p>					<p>previously been implemented in Zone 4 for Level D bracing removal as described in RFI T 1558.</p> <p>2) The east end rebracing sequence is also the sequence performed successfully in west end of the transit center excavation Zone 1.</p> <p>3) The east end rebracing will have raker bracing similar to the west end, the east end does again have additional reinforcing bar in structural concrete walls where rebracing rakers are supported per details on sheet SH-5301 in Submittal TG0300-553.</p> <p>4) The east end rebracing cross-lot or perpendicular sections are in accordance with submittal TG0300-553 and as has previously and successfully re-braced in zones 1, 2, and 3.</p> <p>If acceptable, please specify 1) the movement tolerance for a given waler removed, 2) the observation period for analyzing movement, and 3) the acceptable means for monitoring for movement (survey, inclinometer, or both).</p>
T-1835.2	BGP - Removal Sequence of Internal Bracing System Within Zone 4	Closed	01	10/31/2014	11/10/2014	10/31/2014
<div><div>From: Webcor Construction LP</div><div>Claude Titcher</div></div>						
REQUEST:			ANSWER:			
<p>Sequence:</p> <p>1) Remove level A, B, and C struts and walers (two struts at a time, following discussion present in "monitoring" section below) area up to gridline 31.2 once the concrete has reached adequate strength.</p> <p>2) Remove level A, B, and C struts within area (ST-65 to 74, 82, and 83) and all corresponding walers. once the concrete has reached adequate strength. The diagonal struts should all be de-stressed prior to the four remaining cross lot struts (ST 65, 66, 67, and 68).</p> <p>Monitoring:</p> <p>Remove the walers sections (continuous waler is actually friction connected together) in a sequence referred to as the observation method where one waler on the north and one on the south as well as the two integral cross lot struts attached to the walers would be de-stressed and removed at any one time.</p>			<p>Sequence:</p> <p>1) Remove level A, B, and C struts and walers (two struts at a time, following discussion present in "monitoring" section below) area up to gridline 31.2 once the concrete has reached adequate strength.</p> <p>2) Remove level A, B, and C struts within area (ST-65 to 74, 82, and 83) and all corresponding walers. once the concrete has reached adequate strength. The diagonal struts should all be de-stressed prior to the four remaining cross lot struts (ST 65, 66, 67, and 68).</p> <p>Monitoring:</p> <p>Remove the walers sections (continuous waler is actually friction connected together) in a sequence referred to as the observation method where one waler on the north and one on the south as well as the two integral cross lot struts attached to the walers</p>			



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	<p>Once the first pair of cross lot struts have been de-stressed and removed, the WOJV installed survey points (targets) on the inside face of the soldier beams of the CDSM shoring wall, the Global Analyzer and Arup inclinometers would measure internal bracing performance over the following 24 hours after de-stressing. If little or no movement has been recorded, the contractor can then proceed and begin de-stressing and removing the next pair of wailers and struts to the east. Contract Specification 31-09-13 would be followed in case of movement, and the work would stop and the MRP Movement Review Panel would be convened.</p> <p>Please note the following:</p> <p>1) The east end rebracing sequence indicated in this RFI is the same rebracing sequence that has previously been implemented in Zone 4 for Level D bracing removal as described in RFI T 1558.</p> <p>2) The east end rebracing sequence is also the sequence performed successfully in west end of the transit center excavation Zone 1.</p> <p>3) The east end rebracing will have raker bracing similar to the west end, the east end does again have additional reinforcing bar in structural concrete walls where rebracing rakers are supported per details on sheet SH-5301 in Submittal TG0300-553.</p> <p>4) The east end rebracing cross-lot or perpendicular sections are in accordance with submittal TG0300-553 and as has previously and successfully re-braced in zones 1, 2, and 3.</p> <p>If acceptable, please specify 1) the movement tolerance for a given waler removed, 2) the observation period for analyzing movement, and 3) the acceptable means for monitoring for movement (survey, inclinometer, or both).</p>					<p>would be de-stressed and removed at any one time. Once the first pair of cross lot struts have been de-stressed and removed, the WOJV installed survey points (targets) on the inside face of the soldier beams of the CDSM shoring wall, the Global Analyzer and Arup inclinometers would measure internal bracing performance over the following 24 hours after de-stressing. If little or no movement has been recorded, the contractor can then proceed and begin de-stressing and removing the next pair of wailers and struts to the east. Contract Specification 31-09-13 would be followed in case of movement, and the work would stop and the MRP Movement Review Panel would be convened.</p> <p>Please note the following:</p> <p>1) The east end rebracing sequence indicated in this RFI is the same rebracing sequence that has previously been implemented in Zone 4 for Level D bracing removal as described in RFI T 1558.</p> <p>2) The east end rebracing sequence is also the sequence performed successfully in west end of the transit center excavation Zone 1.</p> <p>3) The east end rebracing will have raker bracing similar to the west end, the east end does again have additional reinforcing bar in structural concrete walls where rebracing rakers are supported per details on sheet SH-5301 in Submittal TG0300-553.</p> <p>4) The east end rebracing cross-lot or perpendicular sections are in accordance with submittal TG0300-553 and as has previously and successfully re-braced in zones 1, 2, and 3.</p> <p>If acceptable, please specify 1) the movement tolerance for a given waler removed, 2) the observation period for analyzing movement, and 3) the acceptable means for monitoring for movement (survey, inclinometer, or both).</p>
T-1837	SSS - Cast Node Repair LAST Temperature	Void	CR	10/21/2014	10/21/2014	
From: Webcor Construction LP Gregory Kemerer						
REQUEST:			ANSWER:			





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	<p>The AWS D1.8 Structural Welding Code - Seismic Supplement, and the Contract specifications, require that either:</p> <p>1) Each lot of weld electrodes (filler metals) used to make demand critical welds (DCW) must be tested in accordance with AWS D1.8 Annex A</p> <p>2) Filler metals may be exempt from individual lot testing, provided the electrode manufacture perform three (3) separate lot tests in accordance with Section 6.3.8.1</p> <p>Additionally, the testing specified shall be not less than the Lowest Anticipated Service Temperature (L.A.S.T.) plus 20 degrees F (AWS D1.8 Section 6.3.6).</p> <p>Bradken has made repairs to the ground node castings utilizing Hobart Brothers weld electrode in one, or both, types, of TM-771 and FabCO RXR (data and test reports attached). The weld repair(s) were not considered DCW welds, and were made in accordance with an approved repair procedure in conformance to the contract specifications.</p> <p>The attachment of the Ground Node to the Transfer Girder is specified in the drawing at the girder flange/web to casting interface to be a DCW. The required weld will intersect with this previously repaired weld metal. The Fabricator has qualified a welding procedure for the specified base metal combination (ASTM A572-50 to ASTM A216WCC). It is the understanding of the Code that this additional weld filler metal of the casting repair to be included in the final girder to casting weld must comply with the AWS D1.8 Seismic requirements in order to be in conformance to the Code provisions.</p> <p>The manufacturer has lot tested three (3) individual lots for each respective type in accordance with the AWS D1.8 requirements. (documents attached). However, the testing temperature was 70 degrees F, not at the Contract required L.A.S.T. of 25 degrees F plus 20 degrees F (45 degrees F).</p> <p>Please confirm that the three (3) ASW D1.8 Lot Tests for the TM-771 and the FabCO RXR weld electrode performed at a L.A.S.T. temperature of 70 degrees F, in lieu of the required 45 degree F, for the weld repair are acceptable:</p> <p>1) Specifically to Ground Cast Node 5-02</p>					
	<p>The AWS D1.8 Structural Welding Code - Seismic Supplement, and the Contract specifications, require that either:</p> <p>1) Each lot of weld electrodes (filler metals) used to make demand critical welds (DCW) must be tested in accordance with AWS D1.8 Annex A</p> <p>2) Filler metals may be exempt from individual lot testing, provided the electrode manufacture perform three (3) separate lot tests in accordance with Section 6.3.8.1</p> <p>Additionally, the testing specified shall be not less than the Lowest Anticipated Service Temperature (L.A.S.T.) plus 20 degrees F (AWS D1.8 Section 6.3.6).</p> <p>Bradken has made repairs to the ground node castings utilizing Hobart Brothers weld electrode in one, or both, types, of TM-771 and FabCO RXR (data and test reports attached). The weld repair(s) were not considered DCW welds, and were made in accordance with an approved repair procedure in conformance to the contract specifications.</p> <p>The attachment of the Ground Node to the Transfer Girder is specified in the drawing at the girder flange/web to casting interface to be a DCW. The required weld will intersect with this previously repaired weld metal. The Fabricator has qualified a welding procedure for the specified base metal combination (ASTM A572-50 to ASTM A216WCC). It is the understanding of the Code that this additional weld filler metal of the casting repair to be included in the final girder to casting weld must comply with the AWS D1.8 Seismic requirements in order to be in conformance to the Code provisions.</p> <p>The manufacturer has lot tested three (3) individual lots for each respective type in accordance with the AWS D1.8 requirements. (documents attached). However, the testing temperature was 70 degrees F, not at the Contract required L.A.S.T. of 25 degrees F plus 20 degrees F (45 degrees F).</p> <p>Please confirm that the three (3) ASW D1.8 Lot Tests for the TM-771 and the FabCO RXR weld electrode performed at a L.A.S.T. temperature of 70 degrees F,</p>					



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	And 2) To any future repairs in the DCW weld zone of the ground cast nodes					in lieu of the required 45 degree F , for the weld repair are acceptable: 1) Specifically to Ground Cast Node 5-02 And 2) To any future repairs in the DCW weld zone of the ground cast nodes
T-1837.1	SSS - Cast Node Repair LAST Temperature  From: Webcor Construction LP  Gregory Kemerer	Closed	CR	10/22/2014	11/01/2014	10/22/2014
	REQUEST:  The AWS D1.8 Structural Welding Code - Seismic Supplement, and the Contract specifications, require that either: 1) Each lot of weld electrodes (filler metals) used to make demand critical welds (DCW) must be tested in accordance with AWS D1.8 Annex A 2) Filler metals may be exempt from individual lot testing, provided the electrode manufacture perform three (3) separate lot tests in accordance with Section 6.3.8.1 Additionally, the testing specified shall be not less than the Lowest Anticipated Service Temperature (L.A.S.T.) plus 20 degrees F (AWS D1.8 Section 6.3.6).  Bradken has made repairs to the ground node castings utilizing Hobart Brothers weld electrode in one, or both, types, of TM-771 and FabCO RXR (data and test reports attached). The weld repair(s) were not considered DCW welds, and were made in accordance with an approved repair procedure in conformance to the contract specifications.  The attachment of the Ground Node to the Transfer Girder is specified in the drawing at the girder flange/web to casting interface to be a DCW. The required weld will intersect with this previously repaired weld metal. The Fabricator has qualified a welding procedure for the specified base metal combination (ASTM A572-50 to ASTM A216WCC). It is the understanding of the Code that this additional weld filler metal of the casting repair to be					ANSWER:  The AWS D1.8 Structural Welding Code - Seismic Supplement, and the Contract specifications, require that either: 1) Each lot of weld electrodes (filler metals) used to make demand critical welds (DCW) must be tested in accordance with AWS D1.8 Annex A 2) Filler metals may be exempt from individual lot testing, provided the electrode manufacture perform three (3) separate lot tests in accordance with Section 6.3.8.1 Additionally, the testing specified shall be not less than the Lowest Anticipated Service Temperature (L.A.S.T.) plus 20 degrees F (AWS D1.8 Section 6.3.6).  Bradken has made repairs to the ground node castings utilizing Hobart Brothers weld electrode in one, or both, types, of TM-771 and FabCO RXR (data and test reports attached). The weld repair(s) were not considered DCW welds, and were made in accordance with an approved repair procedure in conformance to the contract specifications.  The attachment of the Ground Node to the Transfer Girder is specified in the drawing at the girder flange/web to casting interface to be a DCW. The required weld will intersect with this previously repaired weld metal. The Fabricator has qualified a welding procedure for the



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	<p>included in the final girder to casting weld must comply with the AWS D1.8 Seismic requirements in order to be in conformance to the Code provisions.</p> <p>The manufacturer has lot tested three (3) individual lots for each respective type in accordance with the AWS D1.8 requirements. (documents attached). However, the testing temperature was 70 degrees F, not at the Contract required L.A.S.T. of 25 degrees F plus 20 degrees F (45 degrees F).</p> <p>Please confirm that the three (3) ASW D1.8 Lot Tests for the TM-771 and the FabCO RXR weld electrode performed at a L.A.S.T. temperature of 70 degrees F, in lieu of the required 45 degree F, for the weld repair are acceptable: 1) Specifically to Ground Cast Node 5-02 And 2) To any future repairs in the DCW weld zone of the ground cast nodes</p>		<p>specified base metal combination (ASTM A572-50 to ASTM A216WCC). It is the understanding of the Code that this additional weld filler metal of the casting repair to be included in the final girder to casting weld must comply with the AWS D1.8 Seismic requirements in order to be in conformance to the Code provisions.</p> <p>The manufacturer has lot tested three (3) individual lots for each respective type in accordance with the AWS D1.8 requirements. (documents attached). However, the testing temperature was 70 degrees F, not at the Contract required L.A.S.T. of 25 degrees F plus 20 degrees F (45 degrees F).</p> <p>Please confirm that the three (3) ASW D1.8 Lot Tests for the TM-771 and the FabCO RXR weld electrode performed at a L.A.S.T. temperature of 70 degrees F, in lieu of the required 45 degree F, for the weld repair are acceptable: 1) Specifically to Ground Cast Node 5-02 And 2) To any future repairs in the DCW weld zone of the ground cast nodes</p>			
T-1837.2	SSS - Cast Node Repair LAST Temperature	Closed	CR	10/31/2014	11/10/2014	11/04/2014
<p>From: Webcor Construction LP                      Gregory Kemerer</p>						
REQUEST:			ANSWER:			
<p>The AWS D1.8 Structural Welding Code - Seismic Supplement, and the Contract specifications, require that either: 1) Each lot of weld electrodes (filler metals) used to make demand critical welds (DCW) must be tested in accordance with AWS D1.8 Annex A 2) Filler metals may be exempt from individual lot testing, provided the electrode manufacturer perform three (3) separate lot tests in accordance with Section 6.3.8.1 Additionally, the testing specified shall be not more than</p>			<p>The AWS D1.8 Structural Welding Code - Seismic Supplement, and the Contract specifications, require that either: 1) Each lot of weld electrodes (filler metals) used to make demand critical welds (DCW) must be tested in accordance with AWS D1.8 Annex A 2) Filler metals may be exempt from individual lot testing, provided the electrode manufacturer perform three (3) separate lot tests in accordance with Section 6.3.8.1</p>			



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	<p>the Lowest Anticipated Service Temperature (L.A.S.T.) plus 20 degrees F (AWS D1.8 Section 6.3.6).</p> <p>Bradken has made repairs to the ground node casting 5-02 utilizing Hobart Brothers weld electrode in TM-771 (data and test reports attached). The weld repair(s) were not considered DCW welds, and were made in accordance with an approved repair procedure in conformance to the contract specifications.</p> <p>The attachment of the Ground Node to the Transfer Girder is specified in the drawing at the girder flange/web to casting interface to be a DCW. The required weld will intersect with this previously repaired weld metal. The Fabricator has qualified a welding procedure for the specified base metal combination (ASTM A572-50 to ASTM A216WCC). It is the understanding of the Code that this additional weld filler metal of the casting repair to be included in the final girder to casting weld must comply with the AWS D1.8 Seismic requirements in order to be in conformance to the Code provisions.</p> <p>The manufacturer has lot tested three (3) individual lots of TM-771 in accordance with the AWS D1.8 requirements. (documents attached). In addition to the test results provided at 70 degrees F, Hobart has provided additional test data from three (3) lots at 0 degrees F. These additional test results exceed the contract required L.A.S.T. of 25 degrees F plus 20 degrees F (45 degrees F).</p> <p>Please confirm that the three (3) ASW D1.8 Lot Tests for the TM-771 weld electrode performed at a L.A.S.T. temperature of 0 degrees F for the weld repair are acceptable:</p> <ol style="list-style-type: none"><li>1) Specifically to Ground Cast Node 5-02</li></ol> <p>And</p> <ol style="list-style-type: none"><li>2) To any future repairs in the DCW weld zone of the ground cast nodes</li></ol> <p>Note: Additional test results for Fabco RXR weld electrodes will be provided when they are available.</p>					<p>Additionally, the testing specified shall be not more than the Lowest Anticipated Service Temperature (L.A.S.T.) plus 20 degrees F (AWS D1.8 Section 6.3.6).</p> <p>Bradken has made repairs to the ground node casting 5-02 utilizing Hobart Brothers weld electrode in TM-771 (data and test reports attached). The weld repair(s) were not considered DCW welds, and were made in accordance with an approved repair procedure in conformance to the contract specifications.</p> <p>The attachment of the Ground Node to the Transfer Girder is specified in the drawing at the girder flange/web to casting interface to be a DCW. The required weld will intersect with this previously repaired weld metal. The Fabricator has qualified a welding procedure for the specified base metal combination (ASTM A572-50 to ASTM A216WCC). It is the understanding of the Code that this additional weld filler metal of the casting repair to be included in the final girder to casting weld must comply with the AWS D1.8 Seismic requirements in order to be in conformance to the Code provisions.</p> <p>The manufacturer has lot tested three (3) individual lots of TM-771 in accordance with the AWS D1.8 requirements. (documents attached). In addition to the test results provided at 70 degrees F, Hobart has provided additional test data from three (3) lots at 0 degrees F. These additional test results exceed the contract required L.A.S.T. of 25 degrees F plus 20 degrees F (45 degrees F).</p> <p>Please confirm that the three (3) ASW D1.8 Lot Tests for the TM-771 weld electrode performed at a L.A.S.T. temperature of 0 degrees F for the weld repair are acceptable:</p> <ol style="list-style-type: none"><li>1) Specifically to Ground Cast Node 5-02</li></ol> <p>And</p> <ol style="list-style-type: none"><li>2) To any future repairs in the DCW weld zone of the ground cast nodes</li></ol> <p>Note: Additional test results for Fabco RXR weld electrodes will be provided when they are available.</p>



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<b>T-1838</b>	<b>SCS - 4th Lift Foundation Wall - Moment Frame Beam Alignment</b>	<b>Closed</b>	<b>01</b>	<b>10/22/2014</b>	<b>11/01/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>  See attached sketch. This RFI is related to RFI T-1467.  Ground level moment frame beams tie into the foundation wall. Due to level shoring towers, the moment frame beams will be step in elevation. The level moment frame beams will be interfacing with the sloped 4th lift foundation wall causing issues in alignment and constructability in the remaining foundation wall. SCCI proposes to have the top of 4th lift be 3'-11" below top of slab. This method will clear the alignment issues, provide a starter wall edge and will result in a 1" ± 1/4" encroachment of 4th lift wall into the moment frame beam.  Please confirm it is acceptable for the 4th lift encroach into the moment frame beam by 1" ± 1/4"		<b>ANSWER:</b>  See attached sketch. This RFI is related to RFI T-1467.  Ground level moment frame beams tie into the foundation wall. Due to level shoring towers, the moment frame beams will be step in elevation. The level moment frame beams will be interfacing with the sloped 4th lift foundation wall causing issues in alignment and constructability in the remaining foundation wall. SCCI proposes to have the top of 4th lift be 3'-11" below top of slab. This method will clear the alignment issues, provide a starter wall edge and will result in a 1" ± 1/4" encroachment of 4th lift wall into the moment frame beam.  Please confirm it is acceptable for the 4th lift encroach into the moment frame beam by 1" ± 1/4"				
<b>T-1839</b>	<b>SCS - Roof Top Bracket Attachment at Protected Zone</b>	<b>Closed</b>	<b>01</b>	<b>10/22/2014</b>	<b>11/01/2014</b>	<b>10/31/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						
<b>REQUEST:</b>  SI-4205 Detail I EBF LINK BEAM DETAIL Note, "Welded, bolted, screwed or shot-in attachments for perimeter edge angles exterior facades, Partitions, duct work, piping or other construction shall not be placed within protected zone.  Please confirm that it is acceptable to use Dayton C68-TY-Down Hanger's hooked to the back of the EFB link beam at the protected zone. This would provide an attachment point for temporary access Dayton C49 brackets. Once the deck and wall are poured the attachment coil rod will be removed and the C68 will remain in the concrete hooked to the EFB link beam.		<b>ANSWER:</b>  SI-4205 Detail I EBF LINK BEAM DETAIL Note, "Welded, bolted, screwed or shot-in attachments for perimeter edge angles exterior facades, Partitions, duct work, piping or other construction shall not be placed within protected zone.  Please confirm that it is acceptable to use Dayton C68-TY-Down Hanger's hooked to the back of the EFB link beam at the protected zone. This would provide an attachment point for temporary access Dayton C49 brackets. Once the deck and wall are poured the attachment coil rod will be removed and the C68 will remain in the concrete hooked to the EFB link beam.				
<b>T-1840</b>	<b>BGP - Area 11-16 HRC 555 T-Head Lieu of Standard Hooks In EW Beams</b>	<b>Closed</b>	<b>01</b>	<b>10/22/2014</b>	<b>11/01/2014</b>	<b>10/29/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titché						

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	<b>REQUEST:</b> Please confirm it is acceptable to substitute HRC-555 T-Heads in lieu of standard hooks at 4-ft deep East-West Beams where they frame into Moment Frame Beams in Areas 11-16. Please reference the attached drawing for example.					<b>ANSWER:</b> Please confirm it is acceptable to substitute HRC-555 T-Heads in lieu of standard hooks at 4-ft deep East-West Beams where they frame into Moment Frame Beams in Areas 11-16. Please reference the attached drawing for example.
<b>T-1841</b>	<b>SST - Stair 304 Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>10/28/2014</b>	<b>10/28/2014</b>	<b>11/11/2014</b>
	<b>From:</b> Webcor Construction LP Gregory Kemerer					
	<b>REQUEST:</b> Q1: At the second level final landing, the stair stringers are to be supported per connection 3/S1-7601. There is no supporting beam on plan. Please clarify missing information for beam size.  Q2: There is a stair landing elevation discrepancy: A/7007, 1/A1-7506 show the stair landing elevation at 39'-9. This will put the missing beam noted in Q1 at elevation EL. 39'-8 3/4"for the required 1/4" landing plate shown in the detail. 3/S1-7006 shows the stair landing at top of slab 39'-4 1/2" and a landing support 7 1/2" below at EL. 38'-9. Please clarify missing beam elevation.  Q3: The architectural detail 1/A1-7506 and structural detail 3/S1-7601 show completely different support details. Please clarify which of the architectural or structural stair support details should take precedence -typical for all stairs.					<b>ANSWER:</b> Q1: At the second level final landing, the stair stringers are to be supported per connection 3/S1-7601. There is no supporting beam on plan. Please clarify missing information for beam size.  Q2: There is a stair landing elevation discrepancy: A/7007, 1/A1-7506 show the stair landing elevation at 39'-9. This will put the missing beam noted in Q1 at elevation EL. 39'-8 3/4"for the required 1/4" landing plate shown in the detail. 3/S1-7006 shows the stair landing at top of slab 39'-4 1/2" and a landing support 7 1/2" below at EL. 38'-9. Please clarify missing beam elevation.  Q3: The architectural detail 1/A1-7506 and structural detail 3/S1-7601 show completely different support details. Please clarify which of the architectural or structural stair support details should take precedence -typical for all stairs.
<b>T-1842</b>	<b>SST - Stair Stringer to Landing Beam Conn at the Offset WT</b>	<b>Closed</b>	<b>CR</b>	<b>10/28/2014</b>	<b>11/07/2014</b>	<b>11/06/2014</b>
	<b>From:</b> Webcor Construction LP Gregory Kemerer					
	<b>REQUEST:</b> (comments are revised in blue on attached sketch)  Channel end plates are usually flush with the back and the tow of the channel. Olson has projected the end plate 1/2" past on both sides for welding to the supporting WT. Please confirm this is acceptable.					<b>ANSWER:</b> (comments are revised in blue on attached sketch)  Channel end plates are usually flush with the back and the tow of the channel. Olson has projected the end plate 1/2" past on both sides for welding to the supporting WT. Please confirm this is acceptable.





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1843</b>	<b>SST - Edge of Slab for Stair Opening at Stair 403</b>	<b>Closed</b>	<b>CR</b>	<b>10/28/2014</b>	<b>11/07/2014</b>	<b>11/03/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Q1: Stair 403 design is setup for 5'-6 stair clearances at ascending and descending flights and a 9 1/2" guardrail clearance. This creates a total wall to wall clearance of 11'-9 1/2". The slab opening at the concourse level in the north to south direction is currently at 12'-0 1/2". But the opening must be 12'-1 1/2" for the required clearances. See attached sketches for layout information. Please advise.						<b>ANSWER:</b> Q1: Stair 403 design is setup for 5'-6 stair clearances at ascending and descending flights and a 9 1/2" guardrail clearance. This creates a total wall to wall clearance of 11'-9 1/2". The slab opening at the concourse level in the north to south direction is currently at 12'-0 1/2". But the opening must be 12'-1 1/2" for the required clearances. See attached sketches for layout information. Please advise.
<b>T-1844</b>	<b>SST - Checkered Plate Stair Tread Clearance</b>	<b>Closed</b>	<b>CR</b>	<b>10/28/2014</b>	<b>10/28/2014</b>	<b>10/30/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Q1: Architectural design for the setup of checkered plate treads shows the vertical bent lip fitting up tight with the bottom of tread's riser. As illustrated in the attached sketch, the vertical bend of the lip creates a natural radius that will clash with the riser if you were to create this tight fit. Please advise if it's acceptable to back-off the riser portion of the tread by 3/8" to create a gap that clears the lip radius.						<b>ANSWER:</b> Q1: Architectural design for the setup of checkered plate treads shows the vertical bent lip fitting up tight with the bottom of tread's riser. As illustrated in the attached sketch, the vertical bend of the lip creates a natural radius that will clash with the riser if you were to create this tight fit. Please advise if it's acceptable to back-off the riser portion of the tread by 3/8" to create a gap that clears the lip radius.
<b>T-1845</b>	<b>SST - Floating Stair Connection Issue at Stair 401</b>	<b>Closed</b>	<b>CR</b>	<b>10/28/2014</b>	<b>11/07/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Q1: Per the stair layout dimensions provided on 2/A1-7012, stair 401 nosing location at landing elevation 39'-9 will go 7" past the supporting column and beam per 3/S1-7009. As a result, the designed conneciton called out 10/S1-7601 will not work. The cantilevered beam has no purpose in this scenario. Please see included sketch and advise.						<b>ANSWER:</b> Q1: Per the stair layout dimensions provided on 2/A1-7012, stair 401 nosing location at landing elevation 39'-9 will go 7" past the supporting column and beam per 3/S1-7009. As a result, the designed conneciton called out 10/S1-7601 will not work. The cantilevered beam has no purpose in this scenario. Please see included sketch and advise.
<b>T-1846</b>	<b>SST - Cantilevered Support Beam Length at Stair 403</b>	<b>Closed</b>	<b>CR</b>	<b>10/28/2014</b>	<b>10/28/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Q1: PER STAIR SETUP DIMENSION ON 5/A1-7011,						<b>ANSWER:</b> Q1: PER STAIR SETUP DIMENSION ON 5/A1-7011,



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T-1847	<b>SST - Ground Level Stair Landing at Curb &amp; Built Up Slab</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	Closed	CR	10/28/2014	11/07/2014	11/07/2014
	<p>THERE IS A 2'-4" OFFSET BETWEEN LANDING SUPPORT BEAM AND FIRST TREAD. THIS CREATES THE NEED FOR A 2'-10½" CANTILEVERED BEAM. 10/S1-7601 SPECIFIES 2'-0" MAX . PLEASE ADVISE SEE INCLUDED LAYOUT FOR MORE INFORMATION.</p>					<p>THERE IS A 2'-4" OFFSET BETWEEN LANDING SUPPORT BEAM AND FIRST TREAD. THIS CREATES THE NEED FOR A 2'-10½" CANTILEVERED BEAM. 10/S1-7601 SPECIFIES 2'-0" MAX . PLEASE ADVISE SEE INCLUDED LAYOUT FOR MORE INFORMATION.</p>
T-1848	<b>SST - Missing Landing Information at Stair 501</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer	Closed	CR	10/28/2014	11/07/2014	11/24/2014
	<p><b>REQUEST:</b></p> <p>Q1: PER S1-2204, THERE ARE B8 (36" DEEP) CONCRETE BEAMS AT THE NORTH AND SOUTH SIDES OF THE STAIR 403 LOWER CONCOURSE OPENING. THEN THERE IS A B4 (30" DEEP) CONCRETE BEAM AT THE EAST OPENING. THIS CREATES A 6" STEP ON THE UNDERSIDE WHERE THESE THREE BEAMS INTERSECT. THERE ARE (2) HSS6X6 LANDING SUPPORT BEAMS CONNECTING TO THE CONCRETE UNDERSIDE WITH AN EMBED HERE PER 11/S1-7600. BECAUSE OF THE STEP, THE EMBED DETAIL WILL NOT WORK. PLEASE REFER TO INCLUDED SKETCH AND ADVISE ON A SOLUTION.</p>					<p><b>ANSWER:</b></p> <p>Q1: PER S1-2204, THERE ARE B8 (36" DEEP) CONCRETE BEAMS AT THE NORTH AND SOUTH SIDES OF THE STAIR 403 LOWER CONCOURSE OPENING. THEN THERE IS A B4 (30" DEEP) CONCRETE BEAM AT THE EAST OPENING. THIS CREATES A 6" STEP ON THE UNDERSIDE WHERE THESE THREE BEAMS INTERSECT. THERE ARE (2) HSS6X6 LANDING SUPPORT BEAMS CONNECTING TO THE CONCRETE UNDERSIDE WITH AN EMBED HERE PER 11/S1-7600. BECAUSE OF THE STEP, THE EMBED DETAIL WILL NOT WORK. PLEASE REFER TO INCLUDED SKETCH AND ADVISE ON A SOLUTION.</p>
T-1848.1	<b>SST - Stair 501 Missing Beam to Beam Support Detail</b>	Closed	CR	12/22/2014	01/01/2015	01/07/2015
	<p>Q1: STRUCTURAL FRAMING SUPPORT PLANS ARE MISSING FOR LANDINGS AT -3'-1" AND +1'-7" -PLEASE FURNISH THIS MISSING INFORMATION.</p> <p>Q2: VERIFY LANDING AT EL. -11'-9" IS TRUE TO AGREE WITH ARCHITECTURAL ELEVATIONS. 4/S1-7016 CALLS THIS LANDING OUT AT -11'-9 1/8".</p>					<p>Q1: STRUCTURAL FRAMING SUPPORT PLANS ARE MISSING FOR LANDINGS AT -3'-1" AND +1'-7" - PLEASE FURNISH THIS MISSING INFORMATION.</p> <p>Q2: VERIFY LANDING AT EL. -11'-9" IS TRUE TO AGREE WITH ARCHITECTURAL ELEVATIONS. 4/S1-7016 CALLS THIS LANDING OUT AT -11'-9 1/8".</p>





Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-1849.1	<p><b>REQUEST:</b></p> <p>1) There does not appear to be any specific detailing that allows for transverse drift to occur, without inducing loads in the stairs themselves. Is there an expectation that the stairs can handle this displacement-induced load demand?</p> <p>2) We would like to modify the base detail of the typical stairs in order to add a second base plate with a horizontal slot in the transverse direction, in order to accommodate transverse drift without inducing load on the stairs. Do you have any issues or concerns with that?</p> <p>3) We would like to ensure that there are adequate seismic gaps at the perimeter of the stair framing, in order to avoid inducing any horizontal seismic loading on the building framing members at the stair perimeter. In the process of accomplishing this, we may need to reduce slightly the overall widths or lengths of the stairs themselves ¿ possibly by one or several inches. Are there any specific reasons that this could not be done? Would this be acceptable to the design team?</p> <p>4) We will list the vertical and horizontal stair reactions at the perimeter of the stair framing. Do you have any specific format that we should attempt to use for this effort?</p> <p>5) We understand that the review of the stair drawings and calculations will not proceed through the City and County of San Francisco, but instead has been delegated to the design team. Is this correct? Please elaborate.</p> <p>6) Please let us know the best persons to coordinate our efforts with, both at Thornton Tomasetti and at Pelli Clarke Pelli Architects.</p>	Closed	CR	11/17/2014	11/27/2014	12/01/2014
	<p><b>ANSWER:</b></p> <p>1) There does not appear to be any specific detailing that allows for transverse drift to occur, without inducing loads in the stairs themselves. Is there an expectation that the stairs can handle this displacement-induced load demand?</p> <p>2) We would like to modify the base detail of the typical stairs in order to add a second base plate with a horizontal slot in the transverse direction, in order to accommodate transverse drift without inducing load on the stairs. Do you have any issues or concerns with that?</p> <p>3) We would like to ensure that there are adequate seismic gaps at the perimeter of the stair framing, in order to avoid inducing any horizontal seismic loading on the building framing members at the stair perimeter. In the process of accomplishing this, we may need to reduce slightly the overall widths or lengths of the stairs themselves ¿ possibly by one or several inches. Are there any specific reasons that this could not be done? Would this be acceptable to the design team?</p> <p>4) We will list the vertical and horizontal stair reactions at the perimeter of the stair framing. Do you have any specific format that we should attempt to use for this effort?</p> <p>5) We understand that the review of the stair drawings and calculations will not proceed through the City and County of San Francisco, but instead has been delegated to the design team. Is this correct? Please elaborate.</p> <p>6) Please let us know the best persons to coordinate our efforts with, both at Thornton Tomasetti and at Pelli Clarke Pelli Architects.</p>					
T-1849.1	<p><b>From:</b> Webcor Construction LP</p> <p><b>Subject:</b> SST - Coffman Engineers Stair Design Questions for SEOR</p> <p><b>Request:</b></p> <p>The response to item #5 on RFI T-1849 is incomplete. As the TJP is the Authority Having Jurisdiction (AHJ) for the remaining code permissions, please confirm they will be providing the required permits and reviews for the stair package shop drawings (TG07.5). If this is not the case, please confirm that no permit is required.</p>	Closed	CR	11/17/2014	11/27/2014	12/01/2014
	<p><b>Response:</b></p> <p>The response to item #5 on RFI T-1849 is incomplete. As the TJP is the Authority Having Jurisdiction (AHJ) for the remaining code permissions, please confirm they will be providing the required permits and reviews for the stair package shop drawings (TG07.5). If this is not the case, please confirm that no permit is required.</p>					



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			required.			
<b>T-1850</b>	<b>BGP - Sleeve Penetrations Below Bicycle/Vehicle Ramp</b>	<b>Closed</b>	<b>01</b>	<b>10/28/2014</b>	<b>11/07/2014</b>	<b>11/07/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> Per coordination meeting held on 10/9/14 between WOJV and the Design Team regarding sheet A1-7401 and utility sleeve changes under bicycle/vehicle ramps issued in ASI 127, please issue revised architectural layout and structural sketches detailing conduit penetrations under the bicycle/vehicle ramp.			<b>ANSWER:</b> Per coordination meeting held on 10/9/14 between WOJV and the Design Team regarding sheet A1-7401 and utility sleeve changes under bicycle/vehicle ramps issued in ASI 127, please issue revised architectural layout and structural sketches detailing conduit penetrations under the bicycle/vehicle ramp.			
<b>T-1851</b>	<b>SSS - Crash Rail Stiffener Plate Location</b>	<b>Closed</b>	<b>CR</b>	<b>10/29/2014</b>	<b>11/08/2014</b>	<b>11/07/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Confirm the vertical stiffener can remain in the same location with the concrete crash rail per ASI-127 as it was in the steel crash rail in F.O. 027. If not, provide the location.			<b>ANSWER:</b> Confirm the vertical stiffener can remain in the same location with the concrete crash rail per ASI-127 as it was in the steel crash rail in F.O. 027. If not, provide the location.			
<b>T-1852</b>	<b>SSS - Field Welds at Light Column</b>	<b>Closed</b>	<b>CR</b>	<b>10/29/2014</b>	<b>11/08/2014</b>	<b>11/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Details 3-7/S1-6005 show connection lug plates field welded to base plates after field survey. Welding plates this size in position is not practical. Please confirm these welds can be carried out in the shop. We consider this a contractor's option.			<b>ANSWER:</b> Details 3-7/S1-6005 show connection lug plates field welded to base plates after field survey. Welding plates this size in position is not practical. Please confirm these welds can be carried out in the shop. We consider this a contractor's option.			
<b>T-1853</b>	<b>SSS - PE502 &amp; PE503 Beam Flange Removal</b>	<b>Closed</b>	<b>CR</b>	<b>10/29/2014</b>	<b>11/08/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 661 SK1:			<b>ANSWER:</b> See attached CD RFI # 661 SK1:			



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<div><div><p>The flanges at the noted W6x25's will be removed full length per 3/S1-7630 due to their limited length. Confirm that is the intent.</p></div><div><p><b>T-1854</b></p><p><b>SCS - Roof Top Pole Foundation Bolted Connection</b></p><p><b>From:</b> Webcor Construction LP      Claude Titcher</p><p><b>REQUEST:</b></p><p>S1-3281 Detail 4 Roof park security pole and mast light foundation is anchored using bolts going through an angle iron and metal deck. For constructability reasons it is essential that access remains open on the roof top for operations needing crane and man lift support.</p><p>Please confirm that it is acceptable for Shimmick to place the anchor bolts through the deck after the concrete deck placement and not before the concrete is placed resulting in access obstructions.</p></div></div> <div><div><p>The flanges at the noted W6x25's will be removed full length per 3/S1-7630 due to their limited length. Confirm that is the intent.</p></div><div><p><b>ANSWER:</b></p><p>S1-3281 Detail 4 Roof park security pole and mast light foundation is anchored using bolts going through an angle iron and metal deck. For constructability reasons it is essential that access remains open on the roof top for operations needing crane and man lift support.</p><p>Please confirm that it is acceptable for Shimmick to place the anchor bolts through the deck after the concrete deck placement and not before the concrete is placed resulting in access obstructions.</p></div></div>						
		<b>Closed</b>	<b>02</b>	<b>10/29/2014</b>	<b>11/08/2014</b>	<b>11/06/2014</b>
<div><div><p><b>T-1855</b></p><p><b>BGP - Lower Concourse GL16 Escalator Opening As-Built Locations</b></p><p><b>From:</b> Webcor Construction LP      Claude Titcher</p><p><b>REQUEST:</b></p><p>Please see the attached surveyed As-Built Locations of the Escalator Blockouts at GL 16.</p><p>As shown the thinner portion of the Escalator BO at GL 16/C is shifted approximately 4 1/2" South of the planned location( centered on larger portion of BO). Similarly, the thinner portion of the Escalator BO at GL 16/G is shifted approximately 5 5/8" North of its planned location.</p><p>Please confirm the As-Built locations are acceptable.</p></div><div><p><b>ANSWER:</b></p><p>Please see the attached surveyed As-Built Locations of the Escalator Blockouts at GL 16.</p><p>As shown the thinner portion of the Escalator BO at GL 16/C is shifted approximately 4 1/2" South of the planned location( centered on larger portion of BO). Similarly, the thinner portion of the Escalator BO at GL 16/G is shifted approximately 5 5/8" North of its planned location.</p><p>Please confirm the As-Built locations are acceptable.</p></div></div>						
		<b>Closed</b>	<b>01</b>	<b>10/29/2014</b>	<b>11/08/2014</b>	<b>10/31/2014</b>
<div><div><p><b>T-1856</b></p><p><b>BGP - Knee Wall Mix Design</b></p></div><div></div></div>						
		<b>Closed</b>	<b>01</b>	<b>10/29/2014</b>	<b>11/08/2014</b>	<b>11/06/2014</b>





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<b>T-1858</b>	<b>SCS - Opening blockouts through Ground Level</b>	<b>Closed</b>	<b>01</b>	<b>10/30/2014</b>	<b>11/09/2014</b>	<b>11/12/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Due to the North to South shoring along the Lower Concourse Level, the concrete partition walls cannot be built until the Ground Level is poured and struts have been removed. Since the Ground Level will be completed before the partition walls are built, SCCI proposes installing blockouts in the Ground Level above the walls in order to complete the concrete pours and vibrate for the partition walls in GL 10 to GL 13. The blockouts will be 8" diameter circle, the rebar detail will be the one shown at 9/S1-5003. The blockouts will not be farther than 5' O.C. or closer than 2' 6" O.C. See the attached drawing for blockouts layout. Please confirm this is acceptable.			Due to the North to South shoring along the Lower Concourse Level, the concrete partition walls cannot be built until the Ground Level is poured and struts have been removed. Since the Ground Level will be completed before the partition walls are built, SCCI proposes installing blockouts in the Ground Level above the walls in order to complete the concrete pours and vibrate for the partition walls in GL 10 to GL 13. The blockouts will be 8" diameter circle, the rebar detail will be the one shown at 9/S1-5003. The blockouts will not be farther than 5' O.C. or closer than 2' 6" O.C. See the attached drawing for blockouts layout. Please confirm this is acceptable.			
<b>T-1859</b>	<b>SCS - Vertical Trim Bar Termination</b>	<b>Closed</b>	<b>01</b>	<b>10/30/2014</b>	<b>11/09/2014</b>	<b>11/05/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference drawing 4/S1-3207, RFI T-1666, RFI T-1698 and see attached spreadsheet listing out the length of trim bar to be embedded into poured spans of 3rd lift for relevant openings.  The foundation wall penetration detail 4/S1-3207 requires vertical trim bar of LTS + W above and below the opening. RFI T-1666 and RFI T-1695 responses have noted that a hook or head may be used to terminate trim bars where bar length exceeds height of wall. However the third lift will require embedded trim bars for all foundation wall openings where this detail is applied. seer proposes to terminate trim bars with hooks at the bottom of the 4th lift, similar to RFI T- 1666's response for the top of wall.  Please confirm if terminating trim bars with hooks at the bottom of the 4th lift is acceptable or provide parameters for cases where this would be acceptable.			Please reference drawing 4/S1-3207, RFI T-1666, RFI T-1698 and see attached spreadsheet listing out the length of trim bar to be embedded into poured spans of 3rd lift for relevant openings.  The foundation wall penetration detail 4/S1-3207 requires vertical trim bar of LTS + W above and below the opening. RFI T-1666 and RFI T-1695 responses have noted that a hook or head may be used to terminate trim bars where bar length exceeds height of wall. However the third lift will require embedded trim bars for all foundation wall openings where this detail is applied. seer proposes to terminate trim bars with hooks at the bottom of the 4th lift, similar to RFI T- 1666's response for the top of wall.  Please confirm if terminating trim bars with hooks at the bottom of the 4th lift is acceptable or provide parameters for cases where this would be acceptable.			



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<b>T-1860</b>	<b>SSS - WS09 Approval Comment Clarification at GL 4</b>	<b>Closed</b>	<b>CR</b>	<b>10/31/2014</b>	<b>10/31/2014</b>	<b>11/11/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 650 SK1: The connection as shown conforms to the requirements of 2/S1-5010 and the request for an additional bolt is not possible as shown in the snapshot. Please supply a new detail if the intent is to revise this connection.			See attached CD RFI # 650 SK1: The connection as shown conforms to the requirements of 2/S1-5010 and the request for an additional bolt is not possible as shown in the snapshot. Please supply a new detail if the intent is to revise this connection.			
<b>T-1861</b>	<b>SSS - PE503 Fouling Brace at Bus Deck Slab Opening</b>	<b>Closed</b>	<b>CR</b>	<b>10/31/2014</b>	<b>11/10/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 668 SK1: The brace per 1/S1-7600 fouls the slab opening. Please confirm that is acceptable or provide a solution.			See attached CD RFI # 668 SK1: The brace per 1/S1-7600 fouls the slab opening. Please confirm that is acceptable or provide a solution.			
<b>T-1862</b>	<b>SSS - PE503 Fouling Brace at Roof Level Slab Opening</b>	<b>Closed</b>	<b>CR</b>	<b>10/31/2014</b>	<b>11/10/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 669 SK1: The brace per 1/S1-7600 fouls the slab opening. Please confirm that is acceptable or provide a solution.			See attached CD RFI # 669 SK1: The brace per 1/S1-7600 fouls the slab opening. Please confirm that is acceptable or provide a solution.			
<b>T-1863</b>	<b>SSS - E510, E511, &amp; E512 Missing Slab Information</b>	<b>Closed</b>	<b>CR</b>	<b>10/31/2014</b>	<b>11/10/2014</b>	<b>12/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 673 SK1 for items 1 to 4: 1) The model and drawings match the elevation shown on S1-2305 which shows T/SLAB 12.73'. Confirm the TOC 12.58' shown on A1-2865 is correct and the model/drawings are to be revised. 2) Confirm the loose 1/4" bent plate over the beams is an acceptable edge between the knock-out and permanent slab since a structural detail is not provided. 3) Detail 6/S1-5004 noted on S1-2305 does not address the knock-out slab condition at a dropped slab condition			See attached CD RFI # 673 SK1 for items 1 to 4: 1) The model and drawings match the elevation shown on S1-2305 which shows T/SLAB 12.73'. Confirm the TOC 12.58' shown on A1-2865 is correct and the model/drawings are to be revised. 2) Confirm the loose 1/4" bent plate over the beams is an acceptable edge between the knock-out and permanent slab since a structural detail is not provided. 3) Detail 6/S1-5004 noted on S1-2305 does not			





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>as occurs here. Provide a detail showing the edge of slab requirements at both the top and bottom slabs. 4) Supply a detail showing the edge of slab requirement for the low slab.</p>					<p>address the knock-out slab condition at a dropped slab condition as occurs here. Provide a detail showing the edge of slab requirements at both the top and bottom slabs. 4) Supply a detail showing the edge of slab requirement for the low slab.</p>
<b>T-1864</b>	<b>SSS - Missing Kicker Brace Information at Ground Level GL 21-22</b>	<b>Closed</b>	<b>CR</b>	<b>10/31/2014</b>	<b>11/09/2014</b>	<b>11/18/2014</b>
<p><b>From:</b> Webcor Construction LP Gregory Kemerer</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>See attached CD RFI # 674 SK1: The reference to detail 1/S1-7661 is not shown on S1-2305 and the kicker locations are not shown as is on S1-2403 at ST307 between Grids 11-12. Please supply the necessary information if these beams are to be revised prior to fabrication.</p>			<p>See attached CD RFI # 674 SK1: The reference to detail 1/S1-7661 is not shown on S1-2305 and the kicker locations are not shown as is on S1-2403 at ST307 between Grids 11-12. Please supply the necessary information if these beams are to be revised prior to fabrication.</p>			
<b>T-1865</b>	<b>SSS - Incorrect Weld Size at HSS Connections</b>	<b>Closed</b>	<b>CR</b>	<b>10/31/2014</b>	<b>11/10/2014</b>	<b>11/07/2014</b>
<p><b>From:</b> Webcor Construction LP Gregory Kemerer</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>See attached CD RFI # 676 SK1: The noted weld size does not work for the HSS12x6x1/4's in details 4 &amp; 5/S1-7003. Confirm it is acceptable to proceed with a 1/8" PJP weld or supply an alternate weld.</p>			<p>See attached CD RFI # 676 SK1: The noted weld size does not work for the HSS12x6x1/4's in details 4 &amp; 5/S1-7003. Confirm it is acceptable to proceed with a 1/8" PJP weld or supply an alternate weld.</p>			





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<b>T-1866</b>	<b>SSS - Grout Hole Size at Light Tower Column Base Plate</b>	<b>Closed</b>	<b>CR</b>	<b>10/31/2014</b>	<b>11/10/2014</b>	<b>11/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Please see attached SK1 for questions 1 & 2:  1. Grout hole diameter is not specified on the design drawings. Verify use of 1" diameter grout holes is acceptable.  2. Verify 1'-7" dimension from center of column to center of grout hole. Typical at 3 places.					<b>ANSWER:</b> Please see attached SK1 for questions 1 & 2:  1. Grout hole diameter is not specified on the design drawings. Verify use of 1" diameter grout holes is acceptable.  2. Verify 1'-7" dimension from center of column to center of grout hole. Typical at 3 places.	
<b>T-1867</b>	<b>SSS - Flange Cut Flush Requirement</b>	<b>Closed</b>	<b>CR</b>	<b>10/31/2014</b>	<b>11/09/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Shop drawings have been detailed with a note to "cut flange flush with web." Where there are web doubler plates (web stiffener plates) installed, the thickness of the web plus doubler plates controls erection clearances. Please confirm that there is no need for the flange to be ground flush with the web in these locations, and that flange metal can sit proud of the web but not thicker than the thickness of the web plus doubler plates.					<b>ANSWER:</b> Shop drawings have been detailed with a note to "cut flange flush with web." Where there are web doubler plates (web stiffener plates) installed, the thickness of the web plus doubler plates controls erection clearances. Please confirm that there is no need for the flange to be ground flush with the web in these locations, and that flange metal can sit proud of the web but not thicker than the thickness of the web plus doubler plates.	
<b>T-1868</b>	<b>SSS - Clarification of Moment Frame Column Designation</b>	<b>Closed</b>	<b>CR</b>	<b>10/31/2014</b>	<b>11/10/2014</b>	<b>11/06/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Are the 130 Moment Frame Columns (GL1.4 through GL33.2, Ground Level to Roof Park Level) designated as SFRS (Seismic Force Resisting System)? If yes, please clarify where they are identified as such.					<b>ANSWER:</b> Are the 130 Moment Frame Columns (GL1.4 through GL33.2, Ground Level to Roof Park Level) designated as SFRS (Seismic Force Resisting System)? If yes, please clarify where they are identified as such.	
<b>T-1869</b>	<b>BKN - Light Column Cast Node Clarification</b>	<b>Open</b>	<b>CR</b>	<b>10/31/2014</b>	<b>11/10/2014</b>	
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> ASI 127 provided updates to AESS requirements for the light column cast nodes, indicating that the lower light column cast nodes (500, 600, 700 series) are to be AESS-					<b>ANSWER:</b> ASI 127 provided updates to AESS requirements for the light column cast nodes, indicating that the lower light column cast nodes (500, 600, 700 series) are to	



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	<p>1. As these lower light column cast nodes are to be coated with IFRM, please confirm that Bradken may forgo the SCRATA A1 Reference Comparator for these cast nodes, pursuant to the requirements of the cast node specification 05 15 21.</p> <p>If these cast nodes are to be changed, please provide an updated cast node specification 05 15 21. Please note that these cast nodes are in various stages of production, and some have already shipped.</p>					<p>be AESS-1. As these lower light column cast nodes are to be coated with IFRM, please confirm that Bradken may forgo the SCRATA A1 Reference Comparator for these cast nodes, pursuant to the requirements of the cast node specification 05 15 21.</p> <p>If these cast nodes are to be changed, please provide an updated cast node specification 05 15 21. Please note that these cast nodes are in various stages of production, and some have already shipped.</p>







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<b>T-1871</b>	<b>SCS - Rebar Detail for Upturned Beams at GL 18, 26, 34, and 35</b>	<b>Closed</b>	<b>01</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/13/2014</b>
<div><div><b>From:</b> Webcor Construction LP  <b>REQUEST:</b> Please reference S1-3660 through 3663, and TG03 Balfour Beatty Bridge submittal TG0300-201.  Shimmick is proposing a rebar detail change at the ground level roadway upturned beams to facilitate rebar placement. Per BBII's detail 6/SH-5101 , the upturned beam's vertical clearance is 7"-12" to the W18 steel, and the horizontal beam clearance to the cap beam is 19 .5".  Details S1-3660 to 3663 show the ground level upturned beam rebar detail with closed stirrups at every beam. In lieu of the closed stirrup, Shimmick proposes to use an open stirrup and a cap (similar to Detail 2/S1-3600), with the opening and cap placed on the side of the beam.  Please confirm that the proposed open stirrup and cap in lieu of the closed stirrup is acceptable.</div><div><b>ANSWER:</b> Please reference S1-3660 through 3663, and TG03 Balfour Beatty Bridge submittal TG0300-201.  Shimmick is proposing a rebar detail change at the ground level roadway upturned beams to facilitate rebar placement. Per BBII's detail 6/SH-5101 , the upturned beam's vertical clearance is 7"-12" to the W18 steel, and the horizontal beam clearance to the cap beam is 19 .5".  Details S1-3660 to 3663 show the ground level upturned beam rebar detail with closed stirrups at every beam. In lieu of the closed stirrup, Shimmick proposes to use an open stirrup and a cap (similar to Detail 2/S1-3600), with the opening and cap placed on the side of the beam.  Please confirm that the proposed open stirrup and cap in lieu of the closed stirrup is acceptable.</div></div>						
<b>T-1872</b>	<b>SCS - Roof Top Skylight Construction Joint Relocation</b>	<b>Closed</b>	<b>01</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>11/13/2014</b>
<div><div><b>From:</b> Webcor Construction LP  <b>REQUEST:</b> Per drawing S1-8008 detail 4 the deck at the sky light overhangs the beams on a bent plate and the distance of the overhang varies. Please see attached drawing for further explanation.  Shimmick requests approval for the following: 1. To relocate the construction joint from above the bent plate to the top of the slab elevation. 2. To cut a 1" diameter hole roughly 2 feet on center through the bent plate to allow the use of embedded wall form supports to support the inside wall panel after the deck pour. 3. Please confirm that the skylight over hang bent plate will support the wet concrete deck weight.</div><div><b>ANSWER:</b> Per drawing S1-8008 detail 4 the deck at the sky light overhangs the beams on a bent plate and the distance of the overhang varies. Please see attached drawing for further explanation.  Shimmick requests approval for the following: 1. To relocate the construction joint from above the bent plate to the top of the slab elevation. 2. To cut a 1" diameter hole roughly 2 feet on center through the bent plate to allow the use of embedded wall form supports to support the inside wall panel after the deck pour. 3. Please confirm that the skylight over hang bent plate will support the wet concrete deck weight.</div></div>						
<b>T-1873</b>	<b>DAT - Missing Data outlet for LED Screen in W-5 System</b>	<b>Closed</b>	<b>01</b>	<b>11/03/2014</b>	<b>11/13/2014</b>	<b>12/08/2014</b>



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	<div><div>From: Webcor Construction LP</div><div>Aseem Goyal</div></div> <div>REQUEST:<div>Alternate #18 calls for installation of LED screens into the W-5 Wall System of the Beale Street Lobby at Grid line 32.4. See Sheet A1-5301B(03/31/14). Power and data connection are required for LED Screen to function. E1-2307(02/21/14) calls for providing power to the screens but TE1-2307(02/21/14) doesn't depict data outlet for the same. Please confirm TE1-2307(02/21/14) is missing a data outlet for the LED screen and provide a data outlet for the same.</div></div> <div>ANSWER:<div>Alternate #18 calls for installation of LED screens into the W-5 Wall System of the Beale Street Lobby at Grid line 32.4. See Sheet A1-5301B(03/31/14). Power and data connection are required for LED Screen to function. E1-2307(02/21/14) calls for providing power to the screens but TE1-2307(02/21/14) doesn't depict data outlet for the same. Please confirm TE1-2307(02/21/14) is missing a data outlet for the LED screen and provide a data outlet for the same.</div></div>					
T-1874	SSS - Mis-drilled Hole in TR12 Web	Closed	CR	11/03/2014	11/13/2014	11/12/2014
	<div><div>From: Webcor Construction LP</div><div>Gregory Kemerer</div></div> <div>REQUEST:<div>Skanska's transfer girder fabricator TMF has mis-drilled (1) hole in the web of transfer girder TR12, girder segment A631. The as built hole centers are noted on SK1. Please confirm it is acceptable to drill the 15/16" hole at the correct location and provide a 1-15/16" slotted hole.</div></div> <div>ANSWER:<div>Skanska's transfer girder fabricator TMF has mis-drilled (1) hole in the web of transfer girder TR12, girder segment A631. The as built hole centers are noted on SK1. Please confirm it is acceptable to drill the 15/16" hole at the correct location and provide a 1-15/16" slotted hole.</div></div> <div>Note all other holes on the hole pattern are correctly located and drilled.</div>					
T-1875	SSS - ST401 Revised Beam Locations at Ground Level	Closed	CR	11/04/2014	11/14/2014	11/07/2014
	<div><div>From: Webcor Construction LP</div><div>Gregory Kemerer</div></div> <div>REQUEST:<div>The dimensions shown in Field Order 027 have been revised in ASI-127 as shown. Please confirm that the beam locations remain the same and only the bent plate for the edge of slab shall be modified to accommodate the revised opening dimensions.</div></div> <div>ANSWER:<div>The dimensions shown in Field Order 027 have been revised in ASI-127 as shown. Please confirm that the beam locations remain the same and only the bent plate for the edge of slab shall be modified to accommodate the revised opening dimensions.</div></div>					



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<b>T-1875.1</b>	<b>SSS - ST401 Slab Opening Dimension Clarifications</b>	<b>Closed</b>	<b>01</b>	<b>01/22/2015</b>	<b>02/01/2015</b>	<b>01/30/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: T-1875, S1-2304 (ASI 128) Location: GL 15-16 Add'l Doc Ref: CD RFI 672.1 SK1  The noted W33 beams were located 9" from the edge of slab opening. ASI-127 revised the dimensions from the edge of opening to the W33's as shown. RFI T-1875 confirmed that the beams may remain where they were prior to ASI-127 but the edge plates are to be moved to suit ASI-127. That will result in the slab opening dimensions changing to what is shown in blue.  Confirm that is the intent.		<b>ANSWER:</b>  Contract Doc Ref: T-1875, S1-2304 (ASI 128) Location: GL 15-16 Add'l Doc Ref: CD RFI 672.1 SK1  The noted W33 beams were located 9" from the edge of slab opening. ASI-127 revised the dimensions from the edge of opening to the W33's as shown. RFI T-1875 confirmed that the beams may remain where they were prior to ASI-127 but the edge plates are to be moved to suit ASI-127. That will result in the slab opening dimensions changing to what is shown in blue.  Confirm that is the intent.				
<b>T-1876</b>	<b>SSS - Fouling Connection at Ground Level Operating Booths GL 30-31</b>	<b>Closed</b>	<b>CR</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>12/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 677 SK1: The connection for the W16x26 to the W40 is currently per detail 1/S1-5028 but it interferes with the connection for the W16x26 to W16x26 as shown. Please provide a connection for the W16 to W40 and W16 to W16.		<b>ANSWER:</b>  See attached CD RFI # 677 SK1: The connection for the W16x26 to the W40 is currently per detail 1/S1-5028 but it interferes with the connection for the W16x26 to W16x26 as shown. Please provide a connection for the W16 to W40 and W16 to W16.				
<b>T-1877</b>	<b>SSS - PE704 &amp; PE705 Missing Information at Ground Level</b>	<b>Closed</b>	<b>CR</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/18/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 679 SK1 for items 1 & 2: 1) Supply a deck support detail. 2) Supply a slab edge plate detail.		<b>ANSWER:</b>  See attached CD RFI # 679 SK1 for items 1 & 2: 1) Supply a deck support detail. 2) Supply a slab edge plate detail.				
<b>T-1877.1A</b>	<b>SSS - PE704 &amp; PE705 Missing Connection Details at Ground Level</b>	<b>Closed</b>	<b>CR</b>	<b>12/18/2014</b>	<b>12/18/2014</b>	<b>12/27/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  Contract Doc Ref: 2/S1-7111 & A,B,C/S1-7139		<b>ANSWER:</b>  Contract Doc Ref: 2/S1-7111 & A,B,C/S1-7139				







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<b>REQUEST:</b> Contract Doc Ref: 2/S1-7111 & A,B,C/S1-7139 Location: Zone 4, Ground Level Gridline: E/32-32.4 Add'l Doc Ref's: RFI T-1877, CD RFI #679.1 SK1  Please see the attached Skanska RFI 913.1  3.) Elevator PE704 & PE705 at Ground level GL E/32-32.4 shown in CD RFI #679.1 SK1 (drawing S1-7111 detail 2) has a horizontal HSS beam per B/S1-7139.  Please confirm the HSS beam is to remain horizontal.			<b>ANSWER:</b> Contract Doc Ref: 2/S1-7111 & A,B,C/S1-7139 Location: Zone 4, Ground Level Gridline: E/32-32.4 Add'l Doc Ref's: RFI T-1877, CD RFI #679.1 SK1  Please see the attached Skanska RFI 913.1  3.) Elevator PE704 & PE705 at Ground level GL E/32-32.4 shown in CD RFI #679.1 SK1 (drawing S1-7111 detail 2) has a horizontal HSS beam per B/S1-7139.  Please confirm the HSS beam is to remain horizontal.			
<b>T-1878</b>	<b>SSS - PE502 &amp; PE503 Conflicting Dimensions at Roof Park Level</b>	<b>Closed</b>	<b>CR</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/13/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 681 SK1 to SK3 for items 1 to 6: 1) The noted dimensions do not match. Please clarify. 2) Confirm the beams are centered on the HSS columns. 3) Confirm the beams are centered on the HSS columns. 4) The noted dimensions do not match. Please clarify. 5) Confirm the beams are centered on the HSS columns. 6) Confirm the dimensions added to detail 2/S1-7110 are correct based on the 12'-1 dimension in detail 3/A1-2966. If not, clarify the dimensions.			<b>ANSWER:</b> See attached CD RFI # 681 SK1 to SK3 for items 1 to 6: 1) The noted dimensions do not match. Please clarify.  2) Confirm the beams are centered on the HSS columns. 3) Confirm the beams are centered on the HSS columns. 4) The noted dimensions do not match. Please clarify.  5) Confirm the beams are centered on the HSS columns. 6) Confirm the dimensions added to detail 2/S1-7110 are correct based on the 12'-1 dimension in detail 3/A1-2966. If not, clarify the dimensions.			



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<b>T-1879</b>	<b>SSS - PE502 &amp; PE503 Conflicting Elevations at Roof Park Level</b>	<b>Closed</b>	<b>CR</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 682 SK1: The noted T/STEEL elevations do not match. Which elevation is correct?						<b>ANSWER:</b> See attached CD RFI # 682 SK1: The noted T/STEEL elevations do not match. Which elevation is correct?
<b>T-1880</b>	<b>SSS - PE502 &amp; PE503 Connection Clarifications at Gusset Plates</b>	<b>Closed</b>	<b>CR</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 683 SK1 & SK2 for items 1 & 2: 1) The gusset plate fouls the HSS members. Please provide a solution. 2) Confirm the weld length for the HSS8.625X0.322 brace gusset plate can match the HSS10.750X0.500 to avoid splitting the column at an additional location						<b>ANSWER:</b> See attached CD RFI # 683 SK1 & SK2 for items 1 & 2: 1) The gusset plate fouls the HSS members. Please provide a solution. 2) Confirm the weld length for the HSS8.625X0.322 brace gusset plate can match the HSS10.750X0.500 to avoid splitting the column at an additional location
<b>T-1881</b>	<b>SCS - Roof Top Added Deck Construction Joint at Seismic Wall</b>	<b>Closed</b>	<b>01</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/14/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> Per drawing S1-3282 detail 3 the roof top concrete deck overhangs the 10.1 beam line by 2 feet. Delaying the installation of placing the overhang portion of the concrete deck would allow Shimmick to place the roof top concrete deck sooner. This construction joint would be in addition to the current construction joint in the seismic wall per RFI T-1738. Please see attached drawing for further explanation. Shimmick requests approval to add a construction joint in the deck on the 10.1 beam line.						<b>ANSWER:</b> Per drawing S1-3282 detail 3 the roof top concrete deck overhangs the 10.1 beam line by 2 feet. Delaying the installation of placing the overhang portion of the concrete deck would allow Shimmick to place the roof top concrete deck sooner. This construction joint would be in addition to the current construction joint in the seismic wall per RFI T-1738. Please see attached drawing for further explanation. Shimmick requests approval to add a construction joint in the deck on the 10.1 beam line.
<b>T-1882</b>	<b>SCS - Geothermal Manifold Conflict at Field 7, 8, and 11</b>	<b>Closed</b>	<b>01</b>	<b>11/04/2014</b>	<b>11/14/2014</b>	<b>11/12/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> Please Reference RFI's T-1042 and T-1179						<b>ANSWER:</b> Please Reference RFI's T-1042 and T-1179



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	<p>As shown in the attached mark up, the geothermal manifolds penetrate the 4th lift walls. RFI's T-1042 &amp; T-1179 confirmed elevations for the geothermal manifolds at GL 17.5 (Field 7 &amp; 8) and GL 26.5 (Field 11).</p> <p>Per the confirmed manifold elevations, the upper sleeves at both locations enter the 32" roadway slab.</p> <p>Please provide the designers' intent for these manifolds to enter the roadway slab at only these locations. Please provide direction.</p>					
	<p>As shown in the attached mark up, the geothermal manifolds penetrate the 4th lift walls. RFI's T-1042 &amp; T-1179 confirmed elevations for the geothermal manifolds at GL 17.5 (Field 7 &amp; 8) and GL 26.5 (Field 11).</p> <p>Per the confirmed manifold elevations, the upper sleeves at both locations enter the 32" roadway slab.</p> <p>Please provide the designers' intent for these manifolds to enter the roadway slab at only these locations. Please provide direction.</p>					
T-1883	SSS - Bus Deck Crash Rail Detail Clarification	Closed	CR	11/05/2014	11/15/2014	11/12/2014
	From: Webcor Construction LP Gregory Kemerer					
	REQUEST:  The bus deck crash rail details (S1-8000) were updated in ASI 127, and eliminated the bolted connections previously shown. Sheet S1-5031 shows field welds at the top flanges to accomodate the crash rail bolts. Please confirm that these welds are no longer required.					





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T-1888	ELV - Missing designation for the step down transformer	Closed	01	11/05/2014	11/15/2014	12/08/2014
From: Webcor Construction LP                      Aseem Goyal						
REQUEST:						ANSWER:
Reference: E1-3402						Reference: E1-3402
Sheet E1-3402 depicts a step down transformer in Electrical Room 02226 without any designation, please see attached sketch. Please confirm that a transformer is required at that location. If yes please provide a designation for the same.						Sheet E1-3402 depicts a step down transformer in Electrical Room 02226 without any designation, please see attached sketch. Please confirm that a transformer is required at that location. If yes please provide a designation for the same.
T-1889	SCS - Roof Skylight Shear Key Clarification for RFI T-1281.3	Closed	CR	11/05/2014	11/05/2014	11/08/2014
From: Webcor Construction LP                      Claude Titcher						
REQUEST:						ANSWER:
The detail provided in RFI T-1281.3 on SKS-0422 only shows the geometry for central shear key pocket locations that are square to the skylight ellipse.						The detail provided in RFI T-1281.3 on SKS-0422 only shows the geometry for central shear key pocket locations that are square to the skylight ellipse.
Please provide a detail for the grid line 11 and 28 shear key pockets that do not sit square to the ellipse as seen on detail 3 of S1-8008. Please see attached drawing for further explanation.						Please provide a detail for the grid line 11 and 28 shear key pockets that do not sit square to the ellipse as seen on detail 3 of S1-8008. Please see attached drawing for further explanation.
T-1890	ELV - Missing power source for Electric Water Heater EWH-2-A-2 & EWH-2-B-1	Closed	01	11/05/2014	11/15/2014	11/19/2014
From: Webcor Construction LP                      Aseem Goyal						
REQUEST:						ANSWER:
Reference: E1-0052 dated 06/20/14						Reference: E1-0052 dated 06/20/14
Plumbing Equipment Connection Schedule on E1-0052 dated 06/20/14 doesn't depict power source for EWH-2-A-2 & EWH-2-B-1. Please confirm power source for EWH-2-A-2 & EWH-2-B-1.						Plumbing Equipment Connection Schedule on E1-0052 dated 06/20/14 doesn't depict power source for EWH-2-A-2 & EWH-2-B-1. Please confirm power source for EWH-2-A-2 & EWH-2-B-1.
T-1891	SCS - Foundation Wall Plumbing Sleeve Sizes GL 1 to 6	Open	01	11/06/2014	11/16/2014	11/13/2014
From: Webcor Construction LP                      Claude Titcher						
REQUEST:						ANSWER:
See attached marked up drawings.						See attached marked up drawings.



Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
	<p>Noted 8" SAN and 8" F plumbing pipes do not have a corresponding sleeve size on the plumbing sleeve schedule. SCCI requires the sleeve sizes for wall penetrations to adjust rebar layout accordingly.</p> <p>Please provide corresponding sleeve sizes for the noted 8" SAN and 8" F plumbing pipes.</p>					
T-1892	<p><b>SCS - CDSM follow up question in response to RFI#T-1655 answer</b></p> <p><b>From:</b> Shimmick Construction Company, Inc. Henry Chiang</p> <p><b>REQUEST:</b></p> <p>RFI T-1655 SCS -0013 Response provided the following:</p> <p>A. Confirmed notching depths with an updated sketch.</p> <p>B. Referenced ASI 123 roadway at curb (low point) elevations to be used to calculate notching elevation.</p> <p>Shimmick was the able to determine cut elevations for Minna Street from grid line 1 to grid line 18. CI-2001 shows extent of below grade train box and shoring wall at STA: 2+30.85. Using the same reference point on CI-4001 to determine the North West corner of the shoring wall in relation to roadway stationing Shimmick was then able to follow the south flow line proposed grade profile as the bottom of curb elevation for the south side of Minna Street elevations.</p> <p>I. Please confirm that GL 1 correlates with STA: 2+32.85.</p> <p>2. There isn't an equivalent elevation for Natoma Street (south shoring wall), Shimmick requests that reference point with stationing be provided and added to the C 1-4004 drawing.</p> <p>Some areas lack a CI Flow Line Profile. Shimmick requests that a different reference point, such as top of ground level deck, be provided for</p> <p>3. the west shoring wall from grid line A to X,</p> <p>4. the south shoring wall from grid line X to start of CI-4004 which is the first drawing sheet for Natoma Street, and</p> <p>5. First street to Fremont for the south shoring wall.</p>	Closed	01	11/06/2014	11/16/2014	11/06/2014
	<p><b>ANSWER:</b></p> <p>RFI T-1655 SCS -0013 Response provided the following:</p> <p>A. Confirmed notching depths with an updated sketch.</p> <p>B. Referenced ASI 123 roadway at curb (low point) elevations to be used to calculate notching elevation.</p> <p>Shimmick was the able to determine cut elevations for Minna Street from grid line 1 to grid line 18. CI-2001 shows extent of below grade train box and shoring wall at STA: 2+30.85. Using the same reference point on CI-4001 to determine the North West corner of the shoring wall in relation to roadway stationing Shimmick was then able to follow the south flow line proposed grade profile as the bottom of curb elevation for the south side of Minna Street elevations.</p> <p>I. Please confirm that GL 1 correlates with STA: 2+32.85.</p> <p>2. There isn't an equivalent elevation for Natoma Street (south shoring wall), Shimmick requests that reference point with stationing be provided and added to the C 1-4004 drawing.</p> <p>Some areas lack a CI Flow Line Profile. Shimmick requests that a different reference point, such as top of ground level deck, be provided for</p> <p>3. the west shoring wall from grid line A to X,</p> <p>4. the south shoring wall from grid line X to start of CI-4004 which is the first drawing sheet for Natoma Street, and</p> <p>5. First street to Fremont for the south shoring wall.</p>					











<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-1897	<b>SSS - Connection Clarifications at Bus Deck Perimeter Beams GL 27-33.5</b>  <b>From:</b> Webcor Construction LP  <b>REQUEST:</b>  See attached CD RFI # 686 SK1 & SK2 for items 1 to 6: The design intent in this revised connection for the W40 beams east of Grid 26 is not clear as it contradict various other details as listed below. The noted beams are currently connected to the perimeter beams per S1-8003. Please review and confirm for items 1 to 5: 1) Indicates W40 beam supported by perimeter beam and W30 at the opposite end. a) Confirm the connection to the perimeter beam is per 1/S1-8003 at the CP5 location. b) Confirm the other end of the W40 may be connected to the W30 per 1/S1-5011 except with a full depth shear plate and 7 bolts as there is insufficient room for a double angle connection per S1-5010. See enlarged typical detail on SK1 2) Confirm the connection for the W40 to the perimeter beam is per 1 & 1B/S1-8003 at the CP5 location.  3) Indicates W40 beam supported by perimeter beam and BU-40 at the opposite end. a) Confirm the connection to the perimeter beam is per 1/S1-8003 at the CP5 location. b) Confirm the other end of the W40 may be connected to the BU-40 per 1/S1-5011 except with a full depth shear plate and 11 bolts.  4) Indicates W40 beam supported by perimeter beam and W24 at the opposite end. a) Confirm the connection to the perimeter beam is per 1/S1-8003 at the CP5 location. b) Confirm the other end of the W40 may be connected to the W24 per 1/S1-5011 except with a full depth shear plate and 6 bolts.	Closed	CR	11/08/2014	11/18/2014	11/21/2014
	<b>ANSWER:</b>  See attached CD RFI # 686 SK1 & SK2 for items 1 to 6: The design intent in this revised connection for the W40 beams east of Grid 26 is not clear as it contradict various other details as listed below. The noted beams are currently connected to the perimeter beams per S1-8003. Please review and confirm for items 1 to 5: 1) Indicates W40 beam supported by perimeter beam and W30 at the opposite end. a) Confirm the connection to the perimeter beam is per 1/S1-8003 at the CP5 location. b) Confirm the other end of the W40 may be connected to the W30 per 1/S1-5011 except with a full depth shear plate and 7 bolts as there is insufficient room for a double angle connection per S1-5010. See enlarged typical detail on SK1 2) Confirm the connection for the W40 to the perimeter beam is per 1 & 1B/S1-8003 at the CP5 location.  3) Indicates W40 beam supported by perimeter beam and BU-40 at the opposite end. a) Confirm the connection to the perimeter beam is per 1/S1-8003 at the CP5 location. b) Confirm the other end of the W40 may be connected to the BU-40 per 1/S1-5011 except with a full depth shear plate and 11 bolts.  4) Indicates W40 beam supported by perimeter beam and W24 at the opposite end. a) Confirm the connection to the perimeter beam is per 1/S1-8003 at the CP5 location. b) Confirm the other end of the W40 may be					



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
	<p>5) Indicates W40 beam supported by perimeter beam and W40 at the opposite end. a) Confirm the connection to the perimeter beam is per 2/S1-8003 at the CP6 location with the brace omitted and the W40 connected to the 1 1/2" thick plate with 11 bolts per 1/S1-5011. b) Confirm the other end of the W40 may be connected to the W40 per 1/S1-5011 except with a full depth shear plate and 11 bolts.</p> <p>6) If the noted connections are to be revised to suit S1-8000, please note that the connections are not possible with the "LEV" &amp; "S" values as shown. Confirm these values may be adjusted as necessary to fit in the required bolts.</p>					
	<p>connected to the W24 per 1/S1-5011 except with a full depth shear plate and 6 bolts.</p> <p>5) Indicates W40 beam supported by perimeter beam and W40 at the opposite end. a) Confirm the connection to the perimeter beam is per 2/S1-8003 at the CP6 location with the brace omitted and the W40 connected to the 1 1/2" thick plate with 11 bolts per 1/S1-5011. b) Confirm the other end of the W40 may be connected to the W40 per 1/S1-5011 except with a full depth shear plate and 11 bolts.</p> <p>6) If the noted connections are to be revised to suit S1-8000, please note that the connections are not possible with the "LEV" &amp; "S" values as shown. Confirm these values may be adjusted as necessary to fit in the required bolts.</p>					
<b>T-1898</b>	<b>SSS - Light Column - Holes for Electric Lighting System</b>	<b>Closed</b>	<b>CR</b>	<b>11/08/2014</b>	<b>11/18/2014</b>	<b>11/11/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Please provide hole locations and orientations required for the lighting system cabling on the top ring of the light column (EL 98'- 9"). No detail is provided in A1-4650.						
			<b>ANSWER:</b>  Please provide hole locations and orientations required for the lighting system cabling on the top ring of the light column (EL 98'- 9"). No detail is provided in A1-4650.			





<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-1902	<b>SST - Terrazzo Tread Clarification</b>  <b>From:</b> Webcor Construction LP  <b>REQUEST:</b>  One of the specified terrazzo tread suppliers, Wausau, has indicated that using only marble chips in the aggregate mix for the terrazzo stair treads is "unusual" as per Spec Section 09 66 14, section 2.4.C. Olson Steel priced the terrazzo stair treads from Wausau per this spec. Please confirm the design intent does not include any glass/mirror into the aggregate mix.  Under Spec section 09 66 23 Epoxy Terrazzo Flooring, section 2.3.B.5 the terrazzo stair landings are specified to use an aggregate mix of 50% recycled glass/mirror and 50% natural marble/stone. The aggregate mix for the landings differs from the tread; please confirm this was the design intent.	Closed	CR	11/08/2014	11/18/2014	12/11/2014
	<b>ANSWER:</b>  One of the specified terrazzo tread suppliers, Wausau, has indicated that using only marble chips in the aggregate mix for the terrazzo stair treads is "unusual" as per Spec Section 09 66 14, section 2.4.C. Olson Steel priced the terrazzo stair treads from Wausau per this spec. Please confirm the design intent does not include any glass/mirror into the aggregate mix.  Under Spec section 09 66 23 Epoxy Terrazzo Flooring, section 2.3.B.5 the terrazzo stair landings are specified to use an aggregate mix of 50% recycled glass/mirror and 50% natural marble/stone. The aggregate mix for the landings differs from the tread; please confirm this was the design intent.					



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
<b>T-1903</b>	<b>SST - Stair 401 Structural Framing Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>11/08/2014</b>	<b>11/18/2014</b>	<b>11/20/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Q1. From the General Structural Notes note CD-13 on sheet S-0006, it is our understanding that the stair structure should not impose torsion on the superstructure members. However, for Stair 401 sheet A1-7111 shows the stair columns located eccentrically from the centerlines of the superstructure beams. In addition, Structural sheet S1-2404 calls out detail 9/S1-5027 which shows the superstructure beam supporting the stair column eccentrically. Is it acceptable to detail this connection as shown in 9/S1-5027 even though it applies torsion to the beam?			Q1. From the General Structural Notes note CD-13 on sheet S-0006, it is our understanding that the stair structure should not impose torsion on the superstructure members. However, for Stair 401 sheet A1-7111 shows the stair columns located eccentrically from the centerlines of the superstructure beams. In addition, Structural sheet S1-2404 calls out detail 9/S1-5027 which shows the superstructure beam supporting the stair column eccentrically. Is it acceptable to detail this connection as shown in 9/S1-5027 even though it applies torsion to the beam?			
Q2. Should the 3/8" side plate shown in detail 9/S1-5027 be installed for the full length of the beam?			Q2. Should the 3/8" side plate shown in detail 9/S1-5027 be installed for the full length of the beam?			
<b>T-1904</b>	<b>BGP - Partition Wall Top Brace Layout</b>	<b>Closed</b>	<b>01</b>	<b>11/11/2014</b>	<b>11/21/2014</b>	<b>11/21/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached for SCCI's interpretation of the layout of the top braces for the Train Level Partition Walls in Area 3 and 4 based on the structural details in the S1-9000' s. Please confirm that the locations and spacing of the braces shown in the layout drawing are acceptable. Also please confirm or provide direction on several instances that are noted on the layout where SCCI was either unsure of the layout or had to space the braces beyond the specified maximum per the structural details.			See attached for SCCI's interpretation of the layout of the top braces for the Train Level Partition Walls in Area 3 and 4 based on the structural details in the S1-9000' s. Please confirm that the locations and spacing of the braces shown in the layout drawing are acceptable. Also please confirm or provide direction on several instances that are noted on the layout where SCCI was either unsure of the layout or had to space the braces beyond the specified maximum per the structural details.			
<b>T-1905</b>	<b>SCS - Roof Top ASI-124 Perimeter and Scallop Wall Rebar Update</b>	<b>Closed</b>	<b>01</b>	<b>11/13/2014</b>	<b>11/23/2014</b>	<b>12/25/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The revised roof perimeter wall drawings provided in ASI-0124 and RFI T-1739 and T-1857 are architectural only and do not show how the change in height affects the rebar design.			The revised roof perimeter wall drawings provided in ASI-0124 and RFI T-1739 and T-1857 are architectural only and do not show how the change in height affects the rebar design.			
Please provide updated rebar details 2, 4 and 5 of S1-			Please provide updated rebar details 2, 4 and 5 of S1-			



Webcor/Obayashi Joint Venture  
*PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG*  
30100 - Transbay Transit Center Project

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Date: 02/11/2015  
Time: 07:01 AM  
Job: 30100

<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
3282.			3282.			
<b>T-1906</b>	<b>SCS - RCS 2 Splice Layout Requirements</b>	<b>Closed</b>	<b>01</b>	<b>11/13/2014</b>	<b>11/23/2014</b>	<b>11/21/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  See attached markup of S1-3500 including detail 1/S1-3001 showing the rebar splice schedule. S1-3500 has been marked up with dimensions showing typical MFB span (6 eq spans in between 42' 6" grids). CJs will be placed in the span west of each grid. The requirements for rebar splices and CJ placement are shown. Two conflicts are noted below, involving the RCS 2 slab's #5 top layer of rebar.  1) Midspan LCS of 1' 7" is greater than the available 1/3rd span (per spec) for CJs of 1' 6 1/4". Please confirm it is acceptable for the splice length to exceed the central third of the slab's span.  2)Reference CAD mock up combining CJ spec requirements and S1-3500 splice requirements. Splice length of 1'7" conflicts with required 0.3LN spacing intended to center the splice. Please confirm it is acceptable for the LCS to extend into the 0.3LN span.						
						<b>ANSWER:</b>  See attached markup of S1-3500 including detail 1/S1-3001 showing the rebar splice schedule. S1-3500 has been marked up with dimensions showing typical MFB span (6 eq spans in between 42' 6" grids). CJs will be placed in the span west of each grid. The requirements for rebar splices and CJ placement are shown. Two conflicts are noted below, involving the RCS 2 slab's #5 top layer of rebar.  1) Midspan LCS of 1' 7" is greater than the available 1/3rd span (per spec) for CJs of 1' 6 1/4". Please confirm it is acceptable for the splice length to exceed the central third of the slab's span.  2)Reference CAD mock up combining CJ spec requirements and S1-3500 splice requirements. Splice length of 1'7" conflicts with required 0.3LN spacing intended to center the splice. Please confirm it is acceptable for the LCS to extend into the 0.3LN span.







<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1909</b>	<b>TTC - Frame 4 Falsework Ground Loading</b>	<b>Open</b>	<b>01</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Drawing S-1200 shows installation of falsework for Frame 4. To complete the Frame 4 Bridge Falsework design, adjacent to the TTC Train Box, please provide the following:			Drawing S-1200 shows installation of falsework for Frame 4. To complete the Frame 4 Bridge Falsework design, adjacent to the TTC Train Box, please provide the following:			
1. Maximum allowable lateral loading of the CDSM wall/Train Box Wall after the concourse level has been poured and cured.			1. Maximum allowable lateral loading of the CDSM wall/Train Box Wall after the concourse level has been poured and cured.			
2. Maximum allowable lateral loading of the CDSM wall/Train Box Wall after the third wall lift has been poured and cured.			2. Maximum allowable lateral loading of the CDSM wall/Train Box Wall after the third wall lift has been poured and cured.			
3. Maximum allowable lateral loading of the CDSM wall/Train Box Wall after the fourth wall lift has been poured and cured.			3. Maximum allowable lateral loading of the CDSM wall/Train Box Wall after the fourth wall lift has been poured and cured.			
4. Maximum allowable lateral loading of the CDSM wall/Train Box Wall after the ground level has been poured and cured.			4. Maximum allowable lateral loading of the CDSM wall/Train Box Wall after the ground level has been poured and cured.			
Please note that Specification Section 31 00 00 (Earthwork) Section 3.19 B. 1. does not specify any loading limitations for falsework.			Please note that Specification Section 31 00 00 (Earthwork) Section 3.19 B. 1. does not specify any loading limitations for falsework.			





<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>T-1910</b>	<b>BGP - As-Built Condition of Rebar Dowels for Shear Walls W390B and W390D</b>	<b>Closed</b>	<b>01</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>11/17/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
SCCI recently performed an as-built survey of the western-most Shear Wall rebar dowels for the 3rd Lift that were cast into the 2nd Lift Shear Walls and Lower Concourse. These dowels dictate the western face of the 3rd Lift Shear Walls as well as a couple of columns on the West Throat Slab and a B130/B131 Beam alignment. Out of the walls that were surveyed, two of the walls had rebar dowels that were several inches to the west of where they should have been. Based on the results shown in the attached drawings, please confirm that the resulting as-built conditions are acceptable for construction:			SCCI recently performed an as-built survey of the western-most Shear Wall rebar dowels for the 3rd Lift that were cast into the 2nd Lift Shear Walls and Lower Concourse. These dowels dictate the western face of the 3rd Lift Shear Walls as well as a couple of columns on the West Throat Slab and a B130/B131 Beam alignment. Out of the walls that were surveyed, two of the walls had rebar dowels that were several inches to the west of where they should have been. Based on the results shown in the attached drawings, please confirm that the resulting as-built conditions are acceptable for construction:			
1. Encroachment of the western faces of W390B and W390D into the adjacent corridor.			1. Encroachment of the western faces of W390B and W390D into the adjacent corridor.			
2. New location of C19 Column above W390B (roughly 8" to the southwest in alignment with the western face of W390B below).			2. New location of C19 Column above W390B (roughly 8" to the southwest in alignment with the western face of W390B below).			
3. New alignment of B130/B131 Beam below the Vehicle Ramp as a result of the new C19 Column location above W390B			3. New alignment of B130/B131 Beam below the Vehicle Ramp as a result of the new C19 Column location above W390B			







<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1914</b>	<b>SSS - PE201 Splice Location and Weld Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>12/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 493.3 SK1 & SK2: The response to RFI T-1692 (SK 681, CD 493) re the HSS6x6 to be continuous was implemented in the model and the Mill Order has been revised to maintain current submittal schedules as sis shown on attached SK2. The response in RFI T 1692.1 (SK 681.1, CD 493.1) confused this direction. At this time, for Skanska to move forward, they require a splice location and splice weld for the continuous HSS6x6x1/4 column. Confirm the proposed splice location and weld as shown on SK2 are acceptable.			See attached CD RFI # 493.3 SK1 & SK2: The response to RFI T-1692 (SK 681, CD 493) re the HSS6x6 to be continuous was implemented in the model and the Mill Order has been revised to maintain current submittal schedules as sis shown on attached SK2. The response in RFI T 1692.1 (SK 681.1, CD 493.1) confused this direction. At this time, for Skanska to move forward, they require a splice location and splice weld for the continuous HSS6x6x1/4 column. Confirm the proposed splice location and weld as shown on SK2 are acceptable.			
<b>T-1915</b>	<b>SSS - Moment Columns Stiffener and Continuity Plates</b>	<b>Closed</b>	<b>CR</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
With reference to the Moment Columns stiffener and continuity plates with CJP welds, please confirm if acceptable to terminate the weld as per D1.8 figure C-6.3. This will be the same as requested and confirmed on Webcor RFI T-1761 for the transfer beams (see attached).			With reference to the Moment Columns stiffener and continuity plates with CJP welds, please confirm if acceptable to terminate the weld as per D1.8 figure C-6.3. This will be the same as requested and confirmed on Webcor RFI T-1761 for the transfer beams (see attached).			
<b>T-1916</b>	<b>SSS - ST601A Fouling Connections</b>	<b>Closed</b>	<b>CR</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 688 SK1: The W12x14 fouls the connection per 3/S1-5019. Please provide a solution.			See attached CD RFI # 688 SK1: The W12x14 fouls the connection per 3/S1-5019. Please provide a solution.			
<b>T-1917</b>	<b>SSS - Missing Connection Detail at Second Level Protected Zone GL 30</b>	<b>Closed</b>	<b>CR</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 689 SK1: The C8x11.5 per 12/S1-5003 cannot be connected to the beam with a protected zone on Grid 30. Provide a solution.			See attached CD RFI # 689 SK1: The C8x11.5 per 12/S1-5003 cannot be connected to the beam with a protected zone on Grid 30. Provide a solution.			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-1918</b>	<b>SSS - Conflicting Gusset Plate Details at Kicker Brace Connections</b>	<b>Closed</b>	<b>CR</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Detail 8/S1-3704 is referenced for the kicker braces east of Grid 27.1 north of Grid C.3 and south of Grid F.7 on the Ground Level. The detail shows a non full depth gusset plate and refers to detail 3/S1-3503, which shows a full depth gusset plate but also refers to 8/S1-5015. Detail 8/S1-5015 shows a non full-depth gusset plate. Confirm the gusset plates in detail 8/S1-3704 are non full depth as shown.						<b>ANSWER:</b> Detail 8/S1-3704 is referenced for the kicker braces east of Grid 27.1 north of Grid C.3 and south of Grid F.7 on the Ground Level. The detail shows a non full depth gusset plate and refers to detail 3/S1-3503, which shows a full depth gusset plate but also refers to 8/S1-5015. Detail 8/S1-5015 shows a non full-depth gusset plate. Confirm the gusset plates in detail 8/S1-3704 are non full depth as shown.
<b>T-1919</b>	<b>SSS - Clashing Steel at SLRS Connection GL 33.2</b>	<b>Closed</b>	<b>CR</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 691 SK1: As the noted W40x297 is not in plane with the bracing steel, the gusset plate at the top will foul with the beam and there will be a variable thickness gap at the bottom flange as shown. Confirm the beam may be moved south to avoid fouling the gusset plate similar to the west side or supply an alternate solution.						<b>ANSWER:</b> See attached CD RFI # 691 SK1: As the noted W40x297 is not in plane with the bracing steel, the gusset plate at the top will foul with the beam and there will be a variable thickness gap at the bottom flange as shown. Confirm the beam may be moved south to avoid fouling the gusset plate similar to the west side or supply an alternate solution.
<b>T-1919.1A</b>	<b>SSS - Bus Deck Level Connection Clarifications GL 33.2</b>	<b>Closed</b>	<b>CR</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>01/06/2015</b>
<b>From:</b> Webcor Construction LP Stephanie Azzolino						
<b>REQUEST:</b> Contract Doc Ref: S1-2507 Location: Zone 4, Bus Deck Gridline: C/33.2 Add'l Doc Ref's: RFI T-1919, CD RFI # 691.1 SK1  Please see the attached Skanska RFI 934.1.  1.) RFI T-1919 revised the beam location shown in CD RFI # 691.1 SK1  Please confirm the gusset plate is to be revised to the shape as shown in CD RFI # 691.1 SK1 per the response in RFI T-1919.						<b>ANSWER:</b> Contract Doc Ref: S1-2507 Location: Zone 4, Bus Deck Gridline: C/33.2 Add'l Doc Ref's: RFI T-1919, CD RFI # 691.1 SK1  Please see the attached Skanska RFI 934.1.  1.) RFI T-1919 revised the beam location shown in CD RFI # 691.1 SK1  Please confirm the gusset plate is to be revised to the shape as shown in CD RFI # 691.1 SK1 per the response in RFI T-1919.

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<b>T-1919.1D</b>	<b>SSS - Bus Deck Level Connection Clarifications GL 33.2</b>	<b>Closed</b>	<b>CR</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>01/07/2015</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  Contract Doc Ref: S1-2507, 2/S1-5011, 2/S1-5017 Location: Zone 4, Bus Deck Gridline: C/33.2 Add'l Doc Ref's: RFI T-1919, CD RFI # 691.1 SK3  Please see the attached CD RFI # 691.1 SK3:  4.) Confirm it is acceptable to connect the W40x149 per 1/S1-5011 and weld the shear plate to the PL 2" per 2/S1- 5017 or supply an alternate solution.						<b>ANSWER:</b>  Contract Doc Ref: S1-2507, 2/S1-5011, 2/S1-5017 Location: Zone 4, Bus Deck Gridline: C/33.2 Add'l Doc Ref's: RFI T-1919, CD RFI # 691.1 SK3  Please see the attached CD RFI # 691.1 SK3:  4.) Confirm it is acceptable to connect the W40x149 per 1/S1-5011 and weld the shear plate to the PL 2" per 2/S1-5017 or supply an alternate solution.
<b>T-1919.1E</b>	<b>SSS - Bus Deck Level Connection Clarifications GL 33.2</b>	<b>Closed</b>	<b>CR</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>01/06/2015</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  Contract Doc Ref: S1-2507, 2/S1-5011, 2/S1-5017 Location: Zone 4, Bus Deck Gridline: C/33.2 Add'l Doc Ref's: RFI T-1919, CD RFI # 691.1 SK3  Please see the attached CD RFI # 691.1 SK3:  5.) Confirm it is acceptable to connect the W40x149 per 1/S1-5011 SIM. to the web reinforcing plate per 1/S1-5017 as shown or supply an alternate solution						<b>ANSWER:</b>  Contract Doc Ref: S1-2507, 2/S1-5011, 2/S1-5017 Location: Zone 4, Bus Deck Gridline: C/33.2 Add'l Doc Ref's: RFI T-1919, CD RFI # 691.1 SK3  Please see the attached CD RFI # 691.1 SK3:  5.) Confirm it is acceptable to connect the W40x149 per 1/S1-5011 SIM. to the web reinforcing plate per 1/S1-5017 as shown or supply an alternate solution
<b>T-1919.2</b>	<b>SSS - Bus Deck Level Connection Clarifications</b>	<b>Closed</b>	<b>01</b>	<b>01/19/2015</b>	<b>01/29/2015</b>	<b>02/02/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: T-1919.1 Location: GL 33.2 Add'l Doc Ref: Attached 3D model  The response to RFI T-1919.1D tells us to move the noted W40x149 north and connect it to the PL 1" per 2/S1-5017 but this will result in the W40x297 and the W40x149 connecting to the same plate. Please review and provide another response to RFI T-1919.D						<b>ANSWER:</b>  Contract Doc Ref: T-1919.1 Location: GL 33.2 Add'l Doc Ref: Attached 3D model  The response to RFI T-1919.1D tells us to move the noted W40x149 north and connect it to the PL 1" per 2/S1-5017 but this will result in the W40x297 and the W40x149 connecting to the same plate. Please review and provide another response to RFI T-1919.D





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<b>T-1920</b>	<b>SSS - PE704 &amp; PE705 Missing Beam Locations</b>	<b>Closed</b>	<b>CR</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 693 SK1 for items 1 to 3: 1) The (4) noted beams are shown as not aligning with the W36 beams between Grid D.4-E.6 but locating dimensions are not provided. Confirm the (4) W12x14 beams may be aligned with the W36 beams or provide the locating dimensions. 2) The noted beam is shown located 5" north of the slab opening but the slab opening does not exist per A1-2897. Provide the dimension to locate the W12x14 beam. 3) Provide the dimension to locate the W12x14 beam.			See attached CD RFI # 693 SK1 for items 1 to 3: 1) The (4) noted beams are shown as not aligning with the W36 beams between Grid D.4-E.6 but locating dimensions are not provided. Confirm the (4) W12x14 beams may be aligned with the W36 beams or provide the locating dimensions. 2) The noted beam is shown located 5" north of the slab opening but the slab opening does not exist per A1-2897. Provide the dimension to locate the W12x14 beam. 3) Provide the dimension to locate the W12x14 beam.			
<b>T-1921</b>	<b>SSS - Inadequate Connections at Roof Level GL 28</b>	<b>Closed</b>	<b>CR</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 694 SK1 for items 1 & 2: 1) There is insufficient room to connect the skewed W21 to the MF beam with double angles per S1-5010. Confirm it is acceptable to provide connections per 1/S1-5011 or supply alternate solution. 2) The 3/8 x 6 x 6 stiffener plate is not possible at 8 locations since Grid 28 only has drag plates as shown. Confirm that is the intent.			See attached CD RFI # 694 SK1 for items 1 & 2: 1) There is insufficient room to connect the skewed W21 to the MF beam with double angles per S1-5010. Confirm it is acceptable to provide connections per 1/S1-5011 or supply alternate solution. 2) The 3/8 x 6 x 6 stiffener plate is not possible at 8 locations since Grid 28 only has drag plates as shown. Confirm that is the intent.			
<b>T-1922</b>	<b>SSS - Roof Park Restaurant Missing Information GL 4-6</b>	<b>Closed</b>	<b>CR</b>	<b>11/14/2014</b>	<b>11/24/2014</b>	<b>11/24/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 698 SK1 for items 1 & 2: 1) Please supply the missing dimensions noted with a cloud (6 total). 2) For dimensions noted with an oval (4 total), confirm the given dimensions as currently shown in the model based on pre-Field Order 34 information may remain or supply the revised locations. 3) Confirm the noted red dimension line extensions are correct or supply the dimensions to locate the posts.			See attached CD RFI # 698 SK1 for items 1 & 2: 1) Please supply the missing dimensions noted with a cloud (6 total). 2) For dimensions noted with an oval (4 total), confirm the given dimensions as currently shown in the model based on pre-Field Order 34 information may remain or supply the revised locations. 3) Confirm the noted red dimension line extensions are correct or supply the dimensions to locate the posts.			







<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-1929	SSS - ST601 Conflicting and Insufficient Information	Closed	CR	11/18/2014	11/28/2014	12/08/2014
<div> <div> <b>From:</b> Webcor Construction LP Gregory Kemerer </div> </div>						
<b>REQUEST:</b> In order to locate the connections to the stair posts/vertical bracing, please provide the following information:  See attached CD RFI # 684 SK1 to SK2 for items 1 to 10: 1) Dimensions hi-lited in yellow match 2/A1-7020 and the current model. Dimensions hi-lited in purple do not match 2/A1-7020 and the current model. Note that the structural drawings do not locate the posts supporting the stair landings closer to Grid 31at all. Please clarify the discrepancy. (The posts not located on the structural drawings are currently located to suit the Revit model) 3) Clarify the (3) braces indicated in blue and supply more information if they are required. 4) Braces are indicated on plan (green lines) but sizes and connection details are missing. Supply sizes and connection details for each end of every brace. 5) These dimensions do not match 2/A1-7509 (SK3). Please clarify where the steel is located. 7) Supply connection details for the HSS12x6 & HSS6x6 hangers to the underside of beams. 8) These posts are shown as single posts on S1-7012. Please clarify the discrepancy. 9) These dimensions do not match 3/S1-7012 (SK1). Please clarify where the steel is located. 10) 3/S1-7012 shows (4) vertical posts. Clarify why this detail shows (6) posts.			<b>ANSWER:</b>  In order to locate the connections to the stair posts/vertical bracing, please provide the following information:  See attached CD RFI # 684 SK1 to SK2 for items 1 to 10: 1) Dimensions hi-lited in yellow match 2/A1-7020 and the current model. Dimensions hi-lited in purple do not match 2/A1-7020 and the current model. Note that the structural drawings do not locate the posts supporting the stair landings closer to Grid 31at all. Please clarify the discrepancy. (The posts not located on the structural drawings are currently located to suit the Revit model) 3) Clarify the (3) braces indicated in blue and supply more information if they are required. 4) Braces are indicated on plan (green lines) but sizes and connection details are missing. Supply sizes and connection details for each end of every brace. 5) These dimensions do not match 2/A1-7509 (SK3). Please clarify where the steel is located. 7) Supply connection details for the HSS12x6 & HSS6x6 hangers to the underside of beams. 8) These posts are shown as single posts on S1-7012. Please clarify the discrepancy. 9) These dimensions do not match 3/S1-7012 (SK1). Please clarify where the steel is located. 10) 3/S1-7012 shows (4) vertical posts. Clarify why this detail shows (6) posts.			



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<b>T-1929.1</b>	<b>SSS - ST601 Conflicting and Insufficient Information</b>	<b>Closed</b>	<b>CR</b>	<b>01/06/2015</b>	<b>01/16/2015</b>	<b>01/16/2015</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 684.1 SK1 to SK2 for follow up on items 1, 2, 5, 6 and 9 :			See attached CD RFI # 684.1 SK1 to SK2 for follow up on items 1, 2, 5, 6 and 9 :			
1) The response to item 1 in T-1929 SKA-4323 has provided dimensions to locate the stair posts on the Second Level (4/S1-7012) but it remains unclear where the post locations are in details 5 & 6/S1-7012. Where do the dimensions in 4/SKA-4323 apply? Note dimensions of nearest HSS 12x6x5/8" from GL 30 in detail 2 and 4 are not consistent (13'-5" from Detail 4 and 13'-10 1/4" from detail 2. Please clarify.			1) The response to item 1 in T-1929 SKA-4323 has provided dimensions to locate the stair posts on the Second Level (4/S1-7012) but it remains unclear where the post locations are in details 5 & 6/S1-7012. Where do the dimensions in 4/SKA-4323 apply? Note dimensions of nearest HSS 12x6x5/8" from GL 30 in detail 2 and 4 are not consistent (13'-5" from Detail 4 and 13'-10 1/4" from detail 2. Please clarify.			
2) Braces are indicated on plan (orange lines) but connection details are missing. Supply connection details for each end of each brace.			2) Braces are indicated on plan (orange lines) but connection details are missing. Supply connection details for each end of each brace.			
3) Previous response is complete			3) Previous response is complete			
4) Previous response is complete			4) Previous response is complete			
5) SKA-4324 provides revised east/west locations for the hangers below the second level. Confirm the north/south dimensions in 3/S1-7012 are correct.			5) SKA-4324 provides revised east/west locations for the hangers below the second level. Confirm the north/south dimensions in 3/S1-7012 are correct.			
6) The post to beam connection per 1/S1-7600 does not work at the double posts. Supply a new connection detail.			6) The post to beam connection per 1/S1-7600 does not work at the double posts. Supply a new connection detail.			
7) Previous response is complete			7) Previous response is complete			
8) Previous response is complete			8) Previous response is complete			
9) The response is not clear please provide a revised structural plan			9) The response is not clear please provide a revised structural plan			
10) Previous response is complete			10) Previous response is complete			





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<b>T-1930.1</b>	<b>SSS - Stud Spacing Detail</b>	<b>Closed</b>	<b>CR</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>12/29/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Contract Doc Ref: 1/S1-5000 Location: Bus Deck Grid Line: Throughout Add'l Doc Ref's:  The response to RFI T-1930 item 2(C) indicated that if studs are required where there is no rib over the beam due to flange plates, studs should be welded to the flange plates and centered on the beam.  On the gusset plate on the SLRS framing at the bus level, the 12" studs indicated will project through the topping slab. The decker can provide 10" studs at no additional cost but these will only project 1-3/4" into the topping slab and not meet the 2" min requirement. If a custom stud length is required the decker will have additional material and labor costs as the stud welding equipment will need to be modified to shoot the custom length stud.  Please confirm it is acceptable to use 10" studs at the flange plates.		<b>ANSWER:</b>  Contract Doc Ref: 1/S1-5000 Location: Bus Deck Grid Line: Throughout Add'l Doc Ref's:  The response to RFI T-1930 item 2(C) indicated that if studs are required where there is no rib over the beam due to flange plates, studs should be welded to the flange plates and centered on the beam.  On the gusset plate on the SLRS framing at the bus level, the 12" studs indicated will project through the topping slab. The decker can provide 10" studs at no additional cost but these will only project 1-3/4" into the topping slab and not meet the 2" min requirement. If a custom stud length is required the decker will have additional material and labor costs as the stud welding equipment will need to be modified to shoot the custom length stud.  Please confirm it is acceptable to use 10" studs at the flange plates.				
<b>T-1931</b>	<b>SSS - Connection Detail for OCS Support Steel</b>	<b>Closed</b>	<b>CR</b>	<b>11/18/2014</b>	<b>11/28/2014</b>	<b>12/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Please provide a connection detail for the HSS shown in SK1.		<b>ANSWER:</b>  Please provide a connection detail for the HSS shown in SK1.				
<b>T-1932</b>	<b>SSS - Weld Detail Clarifications at TR33.2</b>	<b>Closed</b>	<b>CR</b>	<b>11/18/2014</b>	<b>11/28/2014</b>	<b>12/01/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  See attached CD RFI # 704 SK1 for items 1 to 9: 1) Confirm the weld symbol is acceptable or clarify. 2) Confirm the weld symbol is acceptable or clarify. 3) Confirm the effective weld size is 2" as shown. 4) Confirm the added notes are acceptable or clarify. 5) Confirm the weld symbol is acceptable or clarify. 6) Confirm the effective weld size is 1" as shown.		<b>ANSWER:</b>  See attached CD RFI # 704 SK1 for items 1 to 9: 1) Confirm the weld symbol is acceptable or clarify. 2) Confirm the weld symbol is acceptable or clarify. 3) Confirm the effective weld size is 2" as shown. 4) Confirm the added notes are acceptable or clarify. 5) Confirm the weld symbol is acceptable or clarify. 6) Confirm the effective weld size is 1" as shown.				



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T-1932.1	<p>7) Confirm if double prep is the intent on this weld similar to 5B/S1-4351 &amp; 5B/S1-4355. 8) Confirm the boxed note is correct or clarify. 9) Confirm the arrows as shown are correct to designate weld to the joints or supply the missing welds.</p> <p><b>SSS - Weld Detail Clarifications at TR33.2</b></p> <p><b>From:</b> Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: S1-4352 Location: Cast Node to Transfer Girder Connections Gridline: Ground Level Add'l Doc Ref's: Attached sketch, RFI T-1932</p> <p>RFI T-1932, item 2, requested confirmation that a 2" PJP weld would be acceptable for the two outer stiffener plates at the cast node to transfer girder connections (see attached sketch). The response indicates that a CJP weld is required.</p> <p>A CJP weld will work for the center plate, however the indicated locations (outside plates) will not work as only the outside plate edge is accessible.</p> <p>Please confirm the 2" PJP weld is acceptable at the outer stiffener plates or supply an alternate workable weld.</p>	Closed	CR	01/09/2015	01/19/2015	01/26/2015
	<p>7) Confirm if double prep is the intent on this weld similar to 5B/S1-4351 &amp; 5B/S1-4355. 8) Confirm the boxed note is correct or clarify. 9) Confirm the arrows as shown are correct to designate weld to the joints or supply the missing welds.</p> <p><b>ANSWER:</b></p> <p>Contract Doc Ref: S1-4352 Location: Cast Node to Transfer Girder Connections Gridline: Ground Level Add'l Doc Ref's: Attached sketch, RFI T-1932</p> <p>RFI T-1932, item 2, requested confirmation that a 2" PJP weld would be acceptable for the two outer stiffener plates at the cast node to transfer girder connections (see attached sketch). The response indicates that a CJP weld is required.</p> <p>A CJP weld will work for the center plate, however the indicated locations (outside plates) will not work as only the outside plate edge is accessible.</p> <p>Please confirm the 2" PJP weld is acceptable at the outer stiffener plates or supply an alternate workable weld.</p>					





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<b>T-1933</b>	<b>ESC - Escalator Handrail Clearance</b>	<b>Closed</b>	<b>CR</b>	<b>11/18/2014</b>	<b>11/28/2014</b>	<b>11/25/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  ASME A17.1-2004 Safety Code for Elevators & Escalators Section 6.1.3.2.2 states:  "The handrail shall be a minimum of 100 mm (4 in.) horizontally and 25 mm (1 in.) vertically away from adjacent surfaces, except that round fillets or beveled sides of the handrail stand are permitted to reduce the 25 mm (1 in.) clearance between the handrail and the point where the handrail stand is connected to the balustrade."  Detail 3/A1-7550 shows a 3" clearance between the escalator handrail and the glass rail. This detail only references Escalators E409/E410, but may affect the other escalators on the project. Please confirm that the design will be adjusted to meet code for all the escalators on the job.		<b>ANSWER:</b>  ASME A17.1-2004 Safety Code for Elevators & Escalators Section 6.1.3.2.2 states:  "The handrail shall be a minimum of 100 mm (4 in.) horizontally and 25 mm (1 in.) vertically away from adjacent surfaces, except that round fillets or beveled sides of the handrail stand are permitted to reduce the 25 mm (1 in.) clearance between the handrail and the point where the handrail stand is connected to the balustrade."  Detail 3/A1-7550 shows a 3" clearance between the escalator handrail and the glass rail. This detail only references Escalators E409/E410, but may affect the other escalators on the project. Please confirm that the design will be adjusted to meet code for all the escalators on the job.				
<b>T-1934</b>	<b>SCS - Basket Column Fill</b>	<b>Closed</b>	<b>01</b>	<b>11/19/2014</b>	<b>11/29/2014</b>	<b>12/24/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>  Please reference drawings S1-4000 through S1-4018 regarding filling basket columns.  Currently, basket columns are shown to be filled with both concrete and non-shrink grout. SCCI proposes to fill all basket columns to the elevations shown with self-consolidating concrete. Please see attached submittal TG0702-123.1 for approved mix design.  Please confirm this is acceptable.		<b>ANSWER:</b>  Please reference drawings S1-4000 through S1-4018 regarding filling basket columns.  Currently, basket columns are shown to be filled with both concrete and non-shrink grout. SCCI proposes to fill all basket columns to the elevations shown with self-consolidating concrete. Please see attached submittal TG0702-123.1 for approved mix design.  Please confirm this is acceptable.				
<b>T-1934.1</b>	<b>SCS - Basket Colum Fill</b>	<b>Closed</b>	<b>01</b>	<b>12/05/2014</b>	<b>12/15/2014</b>	<b>12/10/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>  Contract Doc Ref: S1-4000 - S1-4018 Location: N/A Grid Line: N/A Add'l Doc Ref's: RFI T-1934		<b>ANSWER:</b>  Contract Doc Ref: S1-4000 - S1-4018 Location: N/A Grid Line: N/A Add'l Doc Ref's: RFI T-1934				







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<b>T-1936</b>	<b>BGP - Steel Jacket GL 1.4/D.4 - Ring Base Plate</b>	<b>Closed</b>	<b>01</b>	<b>11/21/2014</b>	<b>12/01/2014</b>	<b>12/25/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The column at Gridlines 1.4/D.4 requires a post installed steel jacket. Please reference the attached clouded detail 6/SI-3503. Due to the added reinforcement around the column and field experience with drill and epoxy rebar dowels in this area, the possibility of post installing the ring base plate with postinstalled expansion anchors at shop drilled holes location is highly unlikely. Please confirm it is acceptable to exclude the ring base plate or please provide an additional detail that is more constructible for post-installed column jacket base plate attachment.			The column at Gridlines 1.4/D.4 requires a post installed steel jacket. Please reference the attached clouded detail 6/SI-3503. Due to the added reinforcement around the column and field experience with drill and epoxy rebar dowels in this area, the possibility of post installing the ring base plate with postinstalled expansion anchors at shop drilled holes location is highly unlikely. Please confirm it is acceptable to exclude the ring base plate or please provide an additional detail that is more constructible for post-installed column jacket base plate attachment.			
<b>T-1937</b>	<b>BGP - Zone 1 C-19 and C-38 Column in Conflict with Level A Struts</b>	<b>Closed</b>	<b>01</b>	<b>11/21/2014</b>	<b>12/01/2014</b>	<b>11/25/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please see attached S1-2250, and attached rebar shop drawings.			Please see attached S1-2250, and attached rebar shop drawings.			
Please confirm it is acceptable to eliminate the stagger of the column dowels referenced on the attached drawings, in order to eliminate the conflict with level A struts.			Please confirm it is acceptable to eliminate the stagger of the column dowels referenced on the attached drawings, in order to eliminate the conflict with level A struts.			
<b>T-1938</b>	<b>SSS - Missing OCS Switch Information</b>	<b>Closed</b>	<b>CR</b>	<b>11/24/2014</b>	<b>12/04/2014</b>	<b>12/16/2014</b>
<b>From:</b> Webcor Construction LP      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached CD RFI # 296.11 SK1 & SK2 for items 1 & 2: 1) This brace per 3/S1-9101 fouls the corner post above the plate. Confirm this one brace may be omitted or supply an alternate solution. 2) The diagonal brace per 3/S1-9101 fouls the horizontal HSS4x4x1/4 per 2/S1-9101. Confirm it is acceptable to adjust the angle of the brace to clear to HSS4x4 as required at this location and other similar locations or supply an alternate typical solution.			See attached CD RFI # 296.11 SK1 & SK2 for items 1 & 2: 1) This brace per 3/S1-9101 fouls the corner post above the plate. Confirm this one brace may be omitted or supply an alternate solution. 2) The diagonal brace per 3/S1-9101 fouls the horizontal HSS4x4x1/4 per 2/S1-9101. Confirm it is acceptable to adjust the angle of the brace to clear to HSS4x4 as required at this location and other similar locations or supply an alternate typical solution.			





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T-1941	<b>SSS - ST603 Missing Information at Second Level Stringers</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer  <b>REQUEST:</b>  To coordinate connections with Second Level Framing, please answer the following questions:  See attached CD RFI # 697 SK1 for items 1 & 2: 1) Please clarify what is meant by "SIM" as the connection in detail 3/S1-7601 is for channel stringers. 2) Supply the missing dimensions to locate the stair stringers (not shown on 3/A1-7022).	Closed	CR	11/24/2014	12/04/2014	12/08/2014
	<b>ANSWER:</b>  To coordinate connections with Second Level Framing, please answer the following questions:  See attached CD RFI # 697 SK1 for items 1 & 2: 1) Please clarify what is meant by "SIM" as the connection in detail 3/S1-7601 is for channel stringers.  2) Supply the missing dimensions to locate the stair stringers (not shown on 3/A1-7022).					
T-1942	<b>SSS - PE704 &amp; PE705 Kicker Brace Fouling Slab Opening</b>  <b>From:</b> Webcor Construction LP                      Gregory Kemerer  <b>REQUEST:</b>  See attached CD RFI # 706 SK1: The kicker brace per 1/S1-7600 fouls the slab opening. Please provide a solution.	Closed	CR	11/26/2014	12/06/2014	12/10/2014
	<b>ANSWER:</b>  See attached CD RFI # 706 SK1: The kicker brace per 1/S1-7600 fouls the slab opening. Please provide a solution.					



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<b>T-1943</b>	<b>SSS - ST601B Fouling Connections</b>	<b>Closed</b>	<b>CR</b>	<b>11/26/2014</b>	<b>12/06/2014</b>	<b>12/10/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 707 SK1 for items 1 to 3: 1) A 1/16" gap remains between the beam flanges when they are centered under the curbs. Confirm this is the design intent or supply revised beam locations. 2) The double angle connections per 1/S1-5010 will not fit at the noted locations. Confirm it is acceptable to supply shear plate connections per 1/S1-5011 except they will be welded to the web stiffeners and the underside of the beam flange. If not, supply a new connection detail. 3) Confirm a shear plate per 1/S1-5011 may be used at the noted locations as 1/S1-5010 will not work opposite the connection per 9/S1-5032. If not, supply a new connection detail.						<b>ANSWER:</b> See attached CD RFI # 707 SK1 for items 1 to 3: 1) A 1/16" gap remains between the beam flanges when they are centered under the curbs. Confirm this is the design intent or supply revised beam locations. 2) The double angle connections per 1/S1-5010 will not fit at the noted locations. Confirm it is acceptable to supply shear plate connections per 1/S1-5011 except they will be welded to the web stiffeners and the underside of the beam flange. If not, supply a new connection detail. 3) Confirm a shear plate per 1/S1-5011 may be used at the noted locations as 1/S1-5010 will not work opposite the connection per 9/S1-5032. If not, supply a new connection detail.
<b>T-1944</b>	<b>SSS - Dimension Discrepancy at Glass Floor GL 20.1-21</b>	<b>Closed</b>	<b>CR</b>	<b>12/01/2014</b>	<b>12/11/2014</b>	<b>12/02/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> See attached CD RFI # 648 SK1: 19'-11 3/4 x 4 = 47'-3. Please clarify the discrepancy as shown on SK1. This impacts the post locations per 1/S1-8016.						<b>ANSWER:</b> See attached CD RFI # 648 SK1: 19'-11 3/4 x 4 = 47'-3. Please clarify the discrepancy as shown on SK1. This impacts the post locations per 1/S1-8016.
<b>T-1945</b>	<b>SCS - Foundation Wall Inner Vertical Bar Detail 4/S1-3206</b>	<b>Closed</b>	<b>CR</b>	<b>12/01/2014</b>	<b>12/11/2014</b>	<b>12/04/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b> Please reference RFI T-1625 & T-1894  See attached marked up drawing of detail 4/S1-3206. Similar to the embedded column detail mentioned in RFI T-1625 & T-1894, detail 4/S1-3206 is not constructible with HRC 420 couplers. SCCI proposes to utilize the method approved for embedded columns in RFI T-1894, using Bartee position couplers in lieu of the HRC 400 series on the rebar with hook. As before, note that Bartee is not listed in spec section 03 20 02 acceptable products, but is an or equal.  Please confirm if Bartee couplers are acceptable to be						<b>ANSWER:</b> Please reference RFI T-1625 & T-1894  See attached marked up drawing of detail 4/S1-3206. Similar to the embedded column detail mentioned in RFI T-1625 & T-1894, detail 4/S1-3206 is not constructible with HRC 420 couplers. SCCI proposes to utilize the method approved for embedded columns in RFI T-1894, using Bartee position couplers in lieu of the HRC 400 series on the rebar with hook. As before, note that Bartee is not listed in spec section 03 20 02 acceptable products, but is an or equal.  Please confirm if Bartee couplers are acceptable to be

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	used at detail 4/S1-3206 to allow hook installation.					
T-1946	PLG - Missing Load Requirement for Pumps CP-3-A-1 & CP-B1-A-1	Closed	CR	12/02/2014	12/12/2014	12/10/2014
	<p><b>From:</b> Webcor Construction LP                      Aseem Goyal</p> <p><b>REQUEST:</b></p> <p>Reference: P1-0050 &amp; E1-0052 dated 06/20/14 &amp; Specification 26 24 16/APA</p> <p>Neither the Equipment Schedule (P1-0050) nor the Electrical Panel Schedule (26 24 16/APA) depict load requirements for CP-3-A-1 &amp; CP-B1-A-1. Please provide required load requirements.</p>					<p><b>ANSWER:</b></p> <p>Reference: P1-0050 &amp; E1-0052 dated 06/20/14 &amp; Specification 26 24 16/APA</p> <p>Neither the Equipment Schedule (P1-0050) nor the Electrical Panel Schedule (26 24 16/APA) depict load requirements for CP-3-A-1 &amp; CP-B1-A-1. Please provide required load requirements.</p>

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<b>T-1947</b>	<b>BGP - Embedded C-17 Column in Throat Wall 391 @ GL V/H</b>	<b>Closed</b>	<b>01</b>	<b>12/02/2014</b>	<b>12/12/2014</b>	<b>12/04/2014</b>
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>						
Please reference attached photo (1 page) and sketches (3 pages). Due to missing #10 column vert's in Throat Wall 391 at GL V/H, SCCI proposes the following corrective actions:						
1. In Throat Wall 391:						
- Add #10 dowels with approved HRC couplers, drill and epoxied into concourse deck below per sketches attached, using approved HILTI RE-500 SD Epoxy.						
- Add additional ties and salvage existing ties per sketches attached.						
- Pull horizontal bars into western most end of Throat Wall boundary while maintaining a minimum 81" lap splice.						
- Cast additional #10 column verts into Throat Wall which dowl into column above per contract plans and sketch attached.						
2. In column above:						
- Abandon the three (3) eastern most #10 column verts as shown on sketch.						
- All other bars installed per contract plans and/or sketches attached.						
Please confirm this is acceptable.						
<b>T-1948</b>		<b>Closed</b>	<b>CR</b>	<b>12/02/2014</b>	<b>12/12/2014</b>	<b>12/10/2014</b>
<b>PLG - Missing Load requirements for Pump CP-3-B-1</b>						
<b>From:</b> Webcor Construction LP Aseem Goyal						
<b>REQUEST:</b>						
Reference: E1-0052 dated 06/20/14 & 26 24 16/APA-92						
Neither Plumbing Equipment Connection Schedule (E1-0052) nor the Electrical Panel Schedule(26 24 16/APA-92) specify the load for CP-3-B-1. Please provide required load requirements.						
<b>T-1949</b>		<b>Closed</b>	<b>CR</b>	<b>12/02/2014</b>	<b>12/12/2014</b>	<b>12/08/2014</b>
<b>PLG - Missing Load Requirement for EWH-B1-A-3, EWH-1-C-1, EWH-1-C-2 &amp; EWH-1-C-3</b>						
<b>From:</b> Webcor Construction LP Aseem Goyal						
<b>REQUEST:</b>						
Reference: E1-0052 dated 06/20/14 & 26 24 16/APA-92						
Neither Plumbing Equipment Connection Schedule (E1-0052) nor the Electrical Panel Schedule(26 24 16/APA-92) specify the load for CP-3-B-1. Please provide required load requirements.						
<b>ANSWER:</b>						
Please reference attached photo (1 page) and sketches (3 pages). Due to missing #10 column vert's in Throat Wall 391 at GL V/H, SCCI proposes the following corrective actions:						
1. In Throat Wall 391:						
- Add #10 dowels with approved HRC couplers, drill and epoxied into concourse deck below per sketches attached, using approved HILTI RE-500 SD Epoxy.						
- Add additional ties and salvage existing ties per sketches attached.						
- Pull horizontal bars into western most end of Throat Wall boundary while maintaining a minimum 81" lap splice.						
- Cast additional #10 column verts into Throat Wall which dowl into column above per contract plans and sketch attached.						
2. In column above:						
- Abandon the three (3) eastern most #10 column verts as shown on sketch.						
- All other bars installed per contract plans and/or sketches attached.						
Please confirm this is acceptable.						







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<b>T-1952</b>	<b>ELV - Clarification on Numbered Notes for Junction Box on 5/E1-3204</b>	<b>Closed</b>	<b>CR</b>	<b>12/02/2014</b>	<b>12/12/2014</b>	<b>12/09/2014</b>
<b>From:</b> Webcor Construction LP      Aseem Goyal						
<b>REQUEST:</b>  Reference: 5/E1-3204 dated 06/20/2014  5/E1-3204 specifies Note 6 for the junction box on GL 1.4 & D.4 (See attached). Please confirm Note 6 does not apply to this junction box and should be replaced by Note 4 as it is relevant to the junction box in discussion.						<b>ANSWER:</b>  Reference: 5/E1-3204 dated 06/20/2014  5/E1-3204 specifies Note 6 for the junction box on GL 1.4 & D.4 (See attached). Please confirm Note 6 does not apply to this junction box and should be replaced by Note 4 as it is relevant to the junction box in discussion.
<b>T-1953</b>	<b>ELV - Clarification on the tags of the Distribution Panel MS-B1-D</b>	<b>Closed</b>	<b>CR</b>	<b>12/02/2014</b>	<b>12/02/2014</b>	<b>12/09/2014</b>
<b>From:</b> Webcor Construction LP      Aseem Goyal						
<b>REQUEST:</b>  Reference: E1-5010 dated 06/20/2014 & E1-5003 dated 06/20/2014  E1-5010 depicts MS-B1-D calling out the same panel (DPH-B1-C-12) thrice, see attached. Please confirm power distribution through MS-B1-D specified in E1-5003 is correct and to supersede E1-5010.						<b>ANSWER:</b>  Reference: E1-5010 dated 06/20/2014 & E1-5003 dated 06/20/2014  E1-5010 depicts MS-B1-D calling out the same panel (DPH-B1-C-12) thrice, see attached. Please confirm power distribution through MS-B1-D specified in E1-5003 is correct and to supersede E1-5010.
<b>T-1954</b>	<b>ELV - Clarification on the magnitude of current specified in Power Distribution Acc Closed</b>		<b>CR</b>	<b>12/02/2014</b>	<b>12/12/2014</b>	<b>12/08/2014</b>
<b>From:</b> Webcor Construction LP      Aseem Goyal						
<b>REQUEST:</b>  Reference: Spec Section 26 07 00-13 dated 03/21/2014  Specification Section 26 07 00-13 specifies the magnitude of current as 12000 amp in the Power Distribution Acceptance Test (See attached). Please confirm the magnitude of current should be 1200 amp in lieu of 12000 amp.						<b>ANSWER:</b>  Reference: Spec Section 26 07 00-13 dated 03/21/2014  Specification Section 26 07 00-13 specifies the magnitude of current as 12000 amp in the Power Distribution Acceptance Test (See attached). Please confirm the magnitude of current should be 1200 amp in lieu of 12000 amp.
<b>T-1955</b>	<b>BGP - West Foundation Wall As-Built Location</b>	<b>Closed</b>	<b>01</b>	<b>12/02/2014</b>	<b>12/12/2014</b>	<b>12/04/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						





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T-1957	SCS - Foundation Wall 4th Lift Vertical Unistrut Spacing Tolerance	Closed	01	12/02/2014	12/12/2014	12/03/2014
From: Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>  Please reference T-1549 3rd Wall Lift Vertical Unistrut Spacing Tolerance.  Per RFI T-1549, the vertical unistrut tolerances of plus/minus 4" off the 10' O.C. spacing was allowed on all 3rd lift foundation walls.  Please confirm: 1. That the same tolerances apply to the foundation walls 4th lift. 2. That TG07.2 is to as-built unistrut layout in the field and install unistrut in the 4th lift to match.						<b>ANSWER:</b>  Please reference T-1549 3rd Wall Lift Vertical Unistrut Spacing Tolerance.  Per RFI T-1549, the vertical unistrut tolerances of plus/minus 4" off the 10' O.C. spacing was allowed on all 3rd lift foundation walls.  Please confirm: 1. That the same tolerances apply to the foundation walls 4th lift. 2. That TG07.2 is to as-built unistrut layout in the field and install unistrut in the 4th lift to match.
T-1958	FSP - Missing Fire/Smoke Dampers (FSD) on M1-2104	Closed	CR	12/02/2014	12/12/2014	12/05/2014
From: Webcor Construction LP Aseem Goyal						
<b>REQUEST:</b>  Reference: M1-0053(06/20/2014) & M1-2104(01/23/2014)  FSD B2-B-5 is identified in Fire Smoke Dampers Schedule (M1-0053) but not on M1-2104. Please confirm if FSD is required and, if so, provide updated floor plan or sketch						<b>ANSWER:</b>  Reference: M1-0053(06/20/2014) & M1-2104(01/23/2014)  FSD B2-B-5 is identified in Fire Smoke Dampers Schedule (M1-0053) but not on M1-2104. Please confirm if FSD is required and, if so, provide updated floor plan or sketch
T-1959	FSP - Missing FireSmoke Dampers (FSD) on M1-2205	Closed	CR	12/02/2014	12/12/2014	12/12/2014
From: Webcor Construction LP Aseem Goyal						
<b>REQUEST:</b>  Reference: M1-0053(06/20/2014) & M1-2205(06/20/2014)  FSD B1-C-26 is identified in Fire Smoke Dampers Schedule (M1-0053) but not on M1-2205. Please confirm if FSD is required and, if so, provide updated floor plan or sketch						<b>ANSWER:</b>  Reference: M1-0053(06/20/2014) & M1-2205(06/20/2014)  FSD B1-C-26 is identified in Fire Smoke Dampers Schedule (M1-0053) but not on M1-2205. Please confirm if FSD is required and, if so, provide updated floor plan or sketch



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T-1960	FSP - Missing FireSmoke Dampers (FSD) on M1-2506	Closed	CR	12/02/2014	12/12/2014	12/12/2014
From: Webcor Construction LP Aseem Goyal						
REQUEST: Reference: M1-0053(06/20/2014) & M1-2506(06/20/2014)  FSD 3-D-7 is identified in Fire Smoke Dampers Schedule (M1-0053) but not on M1-2506. Please confirm if FSD is required and, if so, provide updated floor plan or sketch		ANSWER:  Reference: M1-0053(06/20/2014) & M1-2506(06/20/2014)  FSD 3-D-7 is identified in Fire Smoke Dampers Schedule (M1-0053) but not on M1-2506. Please confirm if FSD is required and, if so, provide updated floor plan or sketch				
T-1961	SCS - Sand Oil Interceptor and Baffle Wall	Closed	01	12/02/2014	12/12/2014	12/08/2014
From: Webcor Construction LP Claude Titcher						
REQUEST: Exhibit A - TG07 .2, section 3. Item 6a indicates Furnish and install the sand oil interceptor and baffle wall below Lower Concourse Level. See RFI #T-0677.  RFI #T-0677 (See attached) makes reference to the sand oil interceptor and baffle, however there is no Information or details about the location of the element.  Please provide details for the sand oil interceptor and baffle wall.		ANSWER:  Exhibit A - TG07 .2, section 3. Item 6a indicates Furnish and install the sand oil interceptor and baffle wall below Lower Concourse Level. See RFI #T-0677.  RFI #T-0677 (See attached) makes reference to the sand oil interceptor and baffle, however there is no Information or details about the location of the element.  Please provide details for the sand oil interceptor and baffle wall.				
T-1962	FSP - Missing Duct Smoke Detector for FSD B2-A-1	Closed	CR	12/02/2014	12/12/2014	12/12/2014
From: Webcor Construction LP Aseem Goyal						
REQUEST: Reference: M1-0053(06/20/2014), M1-2102(01/23/2014) & Location: Room No-B2280  Per M1-0053 FSD B2-A-1 requires a duct smoke detector but floor plan M1-2102 doesn't depict one. Please confirm Smoke Detector is required and provide applicable updated floor plan or sketch.		ANSWER:  Reference: M1-0053(06/20/2014), M1-2102(01/23/2014) & Location: Room No-B2280  Per M1-0053 FSD B2-A-1 requires a duct smoke detector but floor plan M1-2102 doesn't depict one. Please confirm Smoke Detector is required and provide applicable updated floor plan or sketch.				



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<b>T-1963</b>	<b>FSP - Missing Duct Smoke Detector for FSD B2-B-5</b>	<b>Closed</b>	<b>CR</b>	<b>12/02/2014</b>	<b>12/12/2014</b>	<b>12/12/2014</b>
<b>From:</b> Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>  Reference: M1-0053(06/20/2014), M1-2104(01/23/2014)  Per M1-0053 FSD B2-B-5 requires a duct smoke detector but floor plan M1-2104 doesn't depict one. Please confirm Smoke Detector is required and provide applicable updated floor plan or sketch.						<b>ANSWER:</b>  Reference: M1-0053(06/20/2014), M1-2104(01/23/2014)  Per M1-0053 FSD B2-B-5 requires a duct smoke detector but floor plan M1-2104 doesn't depict one. Please confirm Smoke Detector is required and provide applicable updated floor plan or sketch.
<b>T-1964</b>	<b>FSP - Missing Duct Smoke Detector for FSD 1-A-10</b>	<b>Closed</b>	<b>CR</b>	<b>12/02/2014</b>	<b>12/12/2014</b>	<b>12/12/2014</b>
<b>From:</b> Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>  Reference: M1-0053(06/20/2014), M1-2302 (02/21/2014) & Location: Room No-01221  Per M1-0053 FSD 1-A-10 requires a duct smoke detector but floor plan M1-2302 doesn't depict one. Please confirm Smoke Detector is required and provide applicable updated floor plan or sketch.						<b>ANSWER:</b>  Reference: M1-0053(06/20/2014), M1-2302 (02/21/2014) & Location: Room No-01221  Per M1-0053 FSD 1-A-10 requires a duct smoke detector but floor plan M1-2302 doesn't depict one. Please confirm Smoke Detector is required and provide applicable updated floor plan or sketch.
<b>T-1965</b>	<b>FSP - Missing Duct Smoke Detector for FSD 3-D-3</b>	<b>Closed</b>	<b>CR</b>	<b>12/02/2014</b>	<b>12/12/2014</b>	<b>12/17/2014</b>
<b>From:</b> Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>  Reference: M1-0053(06/20/2014), M1-2506(06/20/2014) & Location: Room No-03540  Per M1-0053 FSD 3-D-3 requires a duct smoke detector but floor plan M1-2506 doesn't depict one. Please confirm Smoke Detector is required and provide applicable updated floor plan or sketch.						<b>ANSWER:</b>  Reference: M1-0053(06/20/2014), M1-2506(06/20/2014) & Location: Room No-03540  Per M1-0053 FSD 3-D-3 requires a duct smoke detector but floor plan M1-2506 doesn't depict one. Please confirm Smoke Detector is required and provide applicable updated floor plan or sketch.
<b>T-1966</b>	<b>SSS - ST201A Inaccessible Weld Access at HSS</b>	<b>Closed</b>	<b>CR</b>	<b>12/08/2014</b>	<b>12/18/2014</b>	<b>12/19/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Contract Doc Ref: 2 & 3/S1-7003 Location: Zone 1, Roof Grid Line: E.6 & 1						<b>ANSWER:</b>  Contract Doc Ref: 2 & 3/S1-7003 Location: Zone 1, Roof Grid Line: E.6 & 1





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<b>T-1968</b>	<b>SSS - ASI 127 Missing Beam Locations at Ground Level GL 21</b>	<b>Closed</b>	<b>CR</b>	<b>12/08/2014</b>	<b>12/18/2014</b>	<b>12/18/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: S1-2307, S1-6050 Location: Zone 3, Ground Level Grid Line: C & 21, G & 21 Add'l Doc Ref's: CD RFI # 709 SK1			Contract Doc Ref: S1-2307, S1-6050 Location: Zone 3, Ground Level Grid Line: C & 21, G & 21 Add'l Doc Ref's: CD RFI # 709 SK1			
ASI 127 added several beams shown on S1-2305.			ASI 127 added several beams shown on S1-2305.			
Locations for these beams were not provided. Similar beams have been located to suit the W-3 system as noted on S1-6050.			Locations for these beams were not provided. Similar beams have been located to suit the W-3 system as noted on S1-6050.			
Please provide the dimensions to locate the beams.			Please provide the dimensions to locate the beams.			
<b>T-1969</b>	<b>SSS - FO 34 Missing Post Size at Roof Park Restaurant GL 4-5</b>	<b>Closed</b>	<b>CR</b>	<b>12/08/2014</b>	<b>12/18/2014</b>	<b>12/19/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: D & 4 Add'l Doc Ref's: CD RFI # 711 SK1			Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: D & 4 Add'l Doc Ref's: CD RFI # 711 SK1			
Drawing S1-2701 provides location and sizing information for HSS posts.			Drawing S1-2701 provides location and sizing information for HSS posts.			
A post near grid line D & 4 does not have a size indicated.			A post near grid line D & 4 does not have a size indicated.			
Confirm the noted missing post size is HSS5x5x1/2 to match the south side of S1-2701.			Confirm the noted missing post size is HSS5x5x1/2 to match the south side of S1-2701.			
<b>T-1970</b>	<b>SSS - SE201 &amp; SE202, FO 34 Fouling Connection Details</b>	<b>Closed</b>	<b>CR</b>	<b>12/08/2014</b>	<b>12/18/2014</b>	<b>12/19/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: E & 4 Add'l Doc Ref's: CD RFI # 712 SK1			Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: E & 4 Add'l Doc Ref's: CD RFI # 712 SK1			
Drawing S1-2701 shows (4) connections near grid lines E			Drawing S1-2701 shows (4) connections near grid			







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<b>T-1972</b>	<b>PLG - 8" Storm Water sleeve size conflict</b>	<b>Closed</b>	<b>CR</b>	<b>12/04/2014</b>	<b>12/14/2014</b>	<b>12/08/2014</b>
<b>From:</b> Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference : P1-0051 dated 02/21/2014, Table of Foundation Wall Penetrations (TG07.2 IFC Documents) & Submittal Package No TG0702-022			Reference : P1-0051 dated 02/21/2014, Table of Foundation Wall Penetrations (TG07.2 IFC Documents) & Submittal Package No TG0702-022			
Submittal Package No TG0702-022 was returned with markups calling out for 12" sleeves through the foundation wall for 8" Storm Water line. The Table of Foundation Wall Penetrations also specifies 12" sleeve for the same (See attached) . However a 10" sleeve is called out for 8" Storm Water pipe in the Pipe Sleeve Schedule P1-005. Please confirm 12" sleeve is required, if so provide an updated Pipe Sleeve Schedule.			Submittal Package No TG0702-022 was returned with markups calling out for 12" sleeves through the foundation wall for 8" Storm Water line. The Table of Foundation Wall Penetrations also specifies 12" sleeve for the same (See attached) . However a 10" sleeve is called out for 8" Storm Water pipe in the Pipe Sleeve Schedule P1-005. Please confirm 12" sleeve is required, if so provide an updated Pipe Sleeve Schedule.			
<b>T-1973</b>	<b>SCS - Foundation Wall Pipe Sleeve Thickness and Base Metal</b>	<b>Closed</b>	<b>01</b>	<b>12/04/2014</b>	<b>12/14/2014</b>	<b>12/12/2014</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please reference drawings 4/A1-8712 and specification section 22 05 29 2.3B.			Please reference drawings 4/A1-8712 and specification section 22 05 29 2.3B.			
Detail 4/A1-8712 calls out ¼" Thick Stainless Steel Pipe sleeve, specification section 22 05 29 2.3B states to "Provide standard weight galvanized steel pipe sleeves". Standard weight would govern the pipe thickness according to ANSI specifications derived from pipe diameter.			Detail 4/A1-8712 calls out ¼" Thick Stainless Steel Pipe sleeve, specification section 22 05 29 2.3B states to "Provide standard weight galvanized steel pipe sleeves". Standard weight would govern the pipe thickness according to ANSI specifications derived from pipe diameter.			
There is a discrepancy in base metals used and thickness between the drawing and specification.			There is a discrepancy in base metals used and thickness between the drawing and specification.			
Please confirm what document governs.			Please confirm what document governs.			
<b>T-1974</b>	<b>SCS - Vehicle Ramp West Wall</b>	<b>Open</b>	<b>01</b>	<b>12/05/2014</b>	<b>12/15/2014</b>	<b>12/05/2014</b>
<b>From:</b> Shimmick Construction Company, Inc. Henry Chiang						
<b>REQUEST:</b>			<b>ANSWER:</b>			
RFI T-1788 stated that the wall on the west of the Bus Ramp is part of TG07.2 scope of work. Regarding this			RFI T-1788 stated that the wall on the west of the Bus Ramp is part of TG07.2 scope of work. Regarding this			



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	wall:  1. Confirm if formsavers or dowels will be provided by TG06.0 for this wall 2. Confirm Alum Vertical Louver is not part of TG07.2					wall:  1. Confirm if formsavers or dowels will be provided by TG06.0 for this wall 2. Confirm Alum Vertical Louver is not part of TG07.2
<b>T-1975</b>	<b>SCS - Vehicle Ramp West Wall Louver Blockout</b>	<b>Closed</b>	<b>01</b>	<b>12/05/2014</b>	<b>12/15/2014</b>	<b>12/18/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Contract Doc Ref: A1-7407 Location: Area 1/2 Grid Line: N/A Add'l Doc Ref's: RFI T-1788  Aluminum vertical louvers are shown in the vehicle ramp wall (A1-7407 detail A).  Details for the louver blockout cannot be located.  Please provide details for the alum louver blockout including rebar details. Please pay special attention to the southernmost upper most southern corner of the louver where the concrete wall is less then 2" thick.						<b>ANSWER:</b>  Contract Doc Ref: A1-7407 Location: Area 1/2 Grid Line: N/A Add'l Doc Ref's: RFI T-1788  Aluminum vertical louvers are shown in the vehicle ramp wall (A1-7407 detail A).  Details for the louver blockout cannot be located.  Please provide details for the alum louver blockout including rebar details. Please pay special attention to the southernmost upper most southern corner of the louver where the concrete wall is less then 2" thick.



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T-1976	SCS - Roof Pole Foundations That Conflict With Shear Key Locations	Closed	01	12/08/2014	12/18/2014	12/23/2014
<div><div><div>From: Webcor Construction LP</div><div>Claude Titché</div></div><div><div>REQUEST:</div><div>Contract Doc Ref: A1-2902, A1-2903,A1-2904, A1-2906, A1-2907 Location: Roof Park Grid Line: N/A Add'l Doc Ref's:  Several light and camera pole foundations are in conflict with shear key locations. See attached highlighted drawings for further explanation.  Please identify if the shear key of the pole foundation needs to be relocated or provide a detail for the current condition.</div></div><div><div>ANSWER:</div><div>Contract Doc Ref: A1-2902, A1-2903,A1-2904, A1-2906, A1-2907 Location: Roof Park Grid Line: N/A Add'l Doc Ref's:  Several light and camera pole foundations are in conflict with shear key locations. See attached highlighted drawings for further explanation.  Please identify if the shear key of the pole foundation needs to be relocated or provide a detail for the current condition.</div></div></div>						
T-1977	SCS - Roof Ellipse Sky Light Brace Wall Layout Dimensions	Closed	01	12/08/2014	12/18/2014	12/10/2014
<div><div><div>From: Webcor Construction LP</div><div>Claude Titché</div></div><div><div>REQUEST:</div><div>Contract Doc Ref: A1-2903, A1-2906 Location: Roof Park Grid Line: 11, 28 Add'l Doc Ref's:  The ellipse sky lights at Grid Lines 11 and 28 have sloping brace walls around the perimeter as seen on sheet A1-2903, A1-2906, and S1-8008. The north, south, east, and west brace walls that fall directly on grid lines have layout dimensions but the remaining 4 walls do not. Please see attached for further clarification.  Please provide layout dimensions for the ellipse sky light sloping brace walls.</div></div><div><div>ANSWER:</div><div>Contract Doc Ref: A1-2903, A1-2906 Location: Roof Park Grid Line: 11, 28 Add'l Doc Ref's:  The ellipse sky lights at Grid Lines 11 and 28 have sloping brace walls around the perimeter as seen on sheet A1-2903, A1-2906, and S1-8008. The north, south, east, and west brace walls that fall directly on grid lines have layout dimensions but the remaining 4 walls do not. Please see attached for further clarification.  Please provide layout dimensions for the ellipse sky light sloping brace walls.</div></div></div>						
T-1978	SCS - Roof Top Perimeter Wall and Scallop Final Design	Closed	01	12/08/2014	12/18/2014	12/15/2014
<div><div><div>From: Webcor Construction LP</div><div>Claude Titché</div></div><div><div>REQUEST:</div><div>Contract Doc Ref: SKA-3890, SKA-3049-R1</div></div><div><div>ANSWER:</div><div>Contract Doc Ref: SKA-3890, SKA-3049-R1</div></div></div>						







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<b>T-1982</b>	<b>SSS - SLRS Bolted Connection Clarifications GL 31</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2014</b>	<b>12/19/2014</b>	<b>12/22/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: S1-2506 Location: Zone 4, Bus Deck Grid Line: A & 31, J & 31 Add'l Doc Ref's: CD RFI # 730 SK1			Contract Doc Ref: S1-2506 Location: Zone 4, Bus Deck Grid Line: A & 31, J & 31 Add'l Doc Ref's: CD RFI # 730 SK1			
Detail 1/S1-5018 calls for 10 bolts per row for the connections shown near grid lines A & 31, J & 31 on S1-2506.			Detail 1/S1-5018 calls for 10 bolts per row for the connections shown near grid lines A & 31, J & 31 on S1-2506.			
1) Due to the width of the BU-40 flange width, it is not possible to get the 10 bolts per row. (Note: the same occurs on the south side)			1) Due to the width of the BU-40 flange width, it is not possible to get the 10 bolts per row. (Note: the same occurs on the south side)			
Please provide a solution.			Please provide a solution.			
2) The bottom flange is coped as shown and it is not possible to get the 10 bolts per row. (Note: the same occurs on the south side)			2) The bottom flange is coped as shown and it is not possible to get the 10 bolts per row. (Note: the same occurs on the south side)			
Please provide a solution.			Please provide a solution.			
<b>T-1983</b>	<b>SSS - FO 34 Fouling Post at Roof Level GL 6</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2014</b>	<b>12/19/2014</b>	<b>12/19/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: F & 6 Add'l Doc Ref's: CD RFI # 715 SK1			Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: F & 6 Add'l Doc Ref's: CD RFI # 715 SK1			
Drawing S1-2701 provides the location for a post near grid lines F & 6.			Drawing S1-2701 provides the location for a post near grid lines F & 6.			
The noted post fouls the 3" drag plate as shown in SK1.			The noted post fouls the 3" drag plate as shown in SK1.			
Please supply a solution and provide a new connection detail for this location.			Please supply a solution and provide a new connection detail for this location.			



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<b>T-1984</b>	<b>SSS - FO 34 Fouling Connection Details at Roof Level GL 6</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2014</b>	<b>12/19/2014</b>	<b>12/20/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: E.2 & 6 Add'l Doc Ref's: CD RFI # 716 SK1  Drawing S1-2701 provides the location for a post near grid line E.2 and 6.  The connections for this post interfere with the beam connection below.  1) Confirm it is acceptable to locate the 2 bolts as shown and widen the base plate to clear the beam connection below.  2) Confirm it is acceptable to move the stiffener and bolts to clear the beam connection.			<b>ANSWER:</b>  Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: E.2 & 6 Add'l Doc Ref's: CD RFI # 716 SK1  Drawing S1-2701 provides the location for a post near grid line E.2 and 6.  The connections for this post interfere with the beam connection below.  1) Confirm it is acceptable to locate the 2 bolts as shown and widen the base plate to clear the beam connection below.  2) Confirm it is acceptable to move the stiffener and bolts to clear the beam connection.			
<b>T-1985</b>	<b>SSS - FO 34 Fouling Connection Detail at Roof Level GL 4</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2014</b>	<b>12/19/2014</b>	<b>12/20/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: F & 4 Add'l Doc Ref's: CD RFI # 717 SK1  Drawing S1-2701 locates a W16 beam 9'-11.25" East of grid line 4.  This location causes the W16 connection to foul to connection to the BU-60x24 as shown in SK1.  Please provide a solution.			<b>ANSWER:</b>  Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: F & 4 Add'l Doc Ref's: CD RFI # 717 SK1  Drawing S1-2701 locates a W16 beam 9'-11.25" East of grid line 4.  This location causes the W16 connection to foul to connection to the BU-60x24 as shown in SK1.  Please provide a solution.			
<b>T-1985.1</b>	<b>SSS - FO 34 Fouling Connection Detail at Roof Level GL 4</b>	<b>Closed</b>	<b>01</b>	<b>01/19/2015</b>	<b>01/29/2015</b>	<b>01/29/2015</b>
<b>From:</b> Webcor Construction LP                      Andrew Kitchen						
<b>REQUEST:</b>  Contract Doc Ref: T-1985, 9/S1-5032 Location: GL 4			<b>ANSWER:</b>  Contract Doc Ref: T-1985, 9/S1-5032 Location: GL 4			



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	<div>Add'l Doc Ref: Attached CD RFI 717.1 SK1</div> <div>See attached CD RFI 717.1 SK1. Stopping the W16 short to avoid fouling the bolts is a partial solution but the shear plate also fouls the 1 1/2" diameter bolts per 9/S1-5032 as shown and cannot be welded to the web of the BU-60. Please provide a workable alternate connection detail.</div>					<div>Add'l Doc Ref: Attached CD RFI 717.1 SK1</div> <div>See attached CD RFI 717.1 SK1. Stopping the W16 short to avoid fouling the bolts is a partial solution but the shear plate also fouls the 1 1/2" diameter bolts per 9/S1-5032 as shown and cannot be welded to the web of the BU-60. Please provide a workable alternate connection detail.</div>
T-1986	<div>SSS - FO 34 Missing Connection Detail at HSS Post GL 4</div> <div>From: Webcor Construction LPGregory Kemerer</div> <div>REQUEST: Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: D &amp; 4 Add'l Doc Ref's: CD RFI # 719 SK1  Drawing S1-2701 shows a W12x19 member connection to a W16x36 near grid line D &amp; 4.  The post shown at this location appears to rest on the beam to beam connection.  Confirm the beam to beam connection is per 1/S1-5010. If yes, supply a connection detail for the post to clear the beam to beam connection.</div>	Closed	CR	12/09/2014	12/19/2014	12/20/2014
						<div>ANSWER: Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: D &amp; 4 Add'l Doc Ref's: CD RFI # 719 SK1  Drawing S1-2701 shows a W12x19 member connection to a W16x36 near grid line D &amp; 4.  The post shown at this location appears to rest on the beam to beam connection.  Confirm the beam to beam connection is per 1/S1-5010. If yes, supply a connection detail for the post to clear the beam to beam connection.</div>









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T-1992	<div>SSS - PE704 &amp; PE705 Fouling Steel and Missing Information</div> <div>From: Webcor Construction LPGregory Kemerer</div> <div>REQUEST: Contract Doc Ref: 5/S1-7112 Location: Zone 4, Roof Grid Line: E &amp; 32.4 Add'l Doc Ref's: CD RFI # 727 SK1 to SK3  Detail 5/S1-7112 shows several beam to post connections.  Some of the information conflicts, and some of the information is missing.  1) The W12x19 in detail 4/S1-7112 fouls the W16x26 in detail 5/S1-7112, as shown in SK2. Please clarify.  2) The gap between the beams noted in SK3 is 1/8". Confirm that is the intent.</div>	Closed	CR	12/09/2014	12/19/2014	12/23/2014
	<div>5 vertical bolts for the W21's and 8 vertical bolts for the W30's per 1/S1-5010 at the noted locations are not possible due to the 4" thick flange on the BU-44x36x1.5x4.  Confirm it is acceptable to use the N-1 bolts similar to 12/S1-5010 at the noted locations or supply a new detail.</div> <div>ANSWER: Contract Doc Ref: 5/S1-7112 Location: Zone 4, Roof Grid Line: E &amp; 32.4 Add'l Doc Ref's: CD RFI # 727 SK1 to SK3  Detail 5/S1-7112 shows several beam to post connections.  Some of the information conflicts, and some of the information is missing.  1) The W12x19 in detail 4/S1-7112 fouls the W16x26 in detail 5/S1-7112, as shown in SK2. Please clarify.  2) The gap between the beams noted in SK3 is 1/8". Confirm that is the intent.</div>					



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T-1993	SSS - Fill Plate on TR23 Bottom Flange	Closed	CR	12/09/2014	12/19/2014	12/19/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:		ANSWER:				
Contract Doc Ref: S1-4308 & S1-5052 Location: Zone 3, Ground Level Grid Line: A & 23 Add'l Doc Ref's: CD RFI # 723 SK1		Contract Doc Ref: S1-4308 & S1-5052 Location: Zone 3, Ground Level Grid Line: A & 23 Add'l Doc Ref's: CD RFI # 723 SK1				
The TT comments on Skanska shop drawing 706AB from the TG0701-94 submittal package note "plate not required per note 2 on 4/S1-5052." A similar condition exists on Skanska shop drawing 727 in the same submittal package.		The TT comments on Skanska shop drawing 706AB from the TG0701-94 submittal package note "plate not required per note 2 on 4/S1-5052." A similar condition exists on Skanska shop drawing 727 in the same submittal package.				
Skanska agrees that it is correct that the plates are not required at this location per Note 2 in detail 4/S1-5052. As shown in the attached sketch, the plate is shown in detail B/S1-4308.		Skanska agrees that it is correct that the plates are not required at this location per Note 2 in detail 4/S1-5052. As shown in the attached sketch, the plate is shown in detail B/S1-4308.				
Confirm the plate may remain.		Confirm the plate may remain.				
T-1994	SSS - Connection Details on TR24	Closed	CR	12/09/2014	12/19/2014	12/19/2014
From: Webcor Construction LP Gregory Kemerer						
REQUEST:		ANSWER:				
Contract Doc Ref: S1&#8208;5011 Location: Zone 4, Roof Grid Line: Typ. Connection Details Add'l Doc Ref's: CD RFI # 726 SK1		Contract Doc Ref: S1&#8208;5011 Location: Zone 4, Roof Grid Line: Typ. Connection Details Add'l Doc Ref's: CD RFI # 726 SK1				
The TT comments on Skanska shop drawing 708BC from the TG0701-121 submittal package included notes on connections.		The TT comments on Skanska shop drawing 708BC from the TG0701-121 submittal package included notes on connections.				
The noted connections are correct per 2/S1-5011 and per RFI T-0894 (SK 192, CD 143). (see SK1)		The noted connections are correct per 2/S1-5011 and per RFI T-0894 (SK 192, CD 143). (see SK1)				
1) The (2) noted connections are correct per 2/S1-5011. Confirm the intent of the comment is to revise the connections. If yes, supply a detail as it is not possible to have 4 bolts per row as shown. 2) These connections with shear plates are correct per RFI T-0894 (SK 192, CD 143). Confirm the intent of the comment is to revise the connections.		1) The (2) noted connections are correct per 2/S1-5011. Confirm the intent of the comment is to revise the connections. If yes, supply a detail as it is not possible to have 4 bolts per row as shown. 2) These connections with shear plates are correct per RFI T-0894 (SK 192, CD 143). Confirm the intent of the comment is to revise the connections.				



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<b>T-1995</b>	<b>SSS - ST601 Fouling Connections at Second Level</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2014</b>	<b>12/19/2014</b>	<b>12/19/2014</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: S1-2406 Location: Zone 4, Second Level Grid Line: D.4 & 31 Add'l Doc Ref's: CD RFI # 728 SK1		Contract Doc Ref: S1-2406 Location: Zone 4, Second Level Grid Line: D.4 & 31 Add'l Doc Ref's: CD RFI # 728 SK1				
S1-2406 shows a double angle connection near grid lines D.4 and 31.		S1-2406 shows a double angle connection near grid lines D.4 and 31.				
The double angle connection per 1/S1-5010 fouls the web stiffener per 3/S1-5019 by 1 3/16" as shown in SK1.		The double angle connection per 1/S1-5010 fouls the web stiffener per 3/S1-5019 by 1 3/16" as shown in SK1.				
Advise whether to reduce the length of the web stiffener by 2" or replace the double angle connection with a shear plate connection per 1/S1-5011.		Advise whether to reduce the length of the web stiffener by 2" or replace the double angle connection with a shear plate connection per 1/S1-5011.				
<b>T-1996</b>	<b>SSS - TPG1 Approval Comment Clarification GL 24.9</b>	<b>Closed</b>	<b>CR</b>	<b>12/09/2014</b>	<b>12/09/2014</b>	<b>12/23/2014</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: S1-2606 Location: Zone 4, Roof Park Level Grid Line: D & F/24.9 Add'l Doc Ref's: CD RFI # 736 SK1, Skanska Shop Drawings 2878 & 2910, TG0701-130		Contract Doc Ref: S1-2606 Location: Zone 4, Roof Park Level Grid Line: D & F/24.9 Add'l Doc Ref's: CD RFI # 736 SK1, Skanska Shop Drawings 2878 & 2910, TG0701-130				
The TT comment on drawing 2878 in submittal package TG701-130 noted "provide connection per 9/S1-5032" (The same condition occurs on drawing 2910)		The TT comment on drawing 2878 in submittal package TG701-130 noted "provide connection per 9/S1-5032" (The same condition occurs on drawing 2910)				
Revising the connection for A2878 & A2910 to 9/S1-5032 from 1/S1-5010 causes a connection conflict with the connection per 1/S1-5010 for the W40x294.		Revising the connection for A2878 & A2910 to 9/S1-5032 from 1/S1-5010 causes a connection conflict with the connection per 1/S1-5010 for the W40x294.				
Confirm it is acceptable to connect the (2) W40x294's per 1/S1-5011 or supply an alternate solution.		Confirm it is acceptable to connect the (2) W40x294's per 1/S1-5011 or supply an alternate solution.				





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T-1999	SSS - FO 34 New Beam Fouling Existing Connection GL 5	Closed	CR	12/09/2014	12/09/2014	12/22/2014
From: Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: F & 5 Add'l Doc Ref's: CD RFI # 735 SK1  Drawing S1-2701 shows a W12 beam near grid lines F & 5.  The added W12x19 in FO 34 fouls the existing connection per 2/S1-5018.  Please provide a solution.						
						<b>ANSWER:</b> Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: F & 5 Add'l Doc Ref's: CD RFI # 735 SK1  Drawing S1-2701 shows a W12 beam near grid lines F & 5.  The added W12x19 in FO 34 fouls the existing connection per 2/S1-5018.  Please provide a solution.
T-2000	SSS - CP6 Connection at GL 33.5 B & H	Closed	CR	12/09/2014	12/09/2014	12/22/2014
From: Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b> Contract Doc Ref: S1-2507 Location: Zone 4, Bus Deck Grid Line: H & 33.5, B & 33.5 Add'l Doc Ref's: CD RFI # 738 SK1  Drawing S1-2507 shows CP6 connections at grid lines B/H and 33.5.  The connections as shown cause a dimension to change and also cause a potential conflict with CP6 colt hole connections.  1) With the 1 1/2" stiffener plate per 4/S1-8003 centered about the CP6 connection, the W40x149 will be located 5'-9 11/16 west of Grid 33.5. Confirm that is acceptable.  2) The edge of the web reinforcing plate per 1/S1-5017 is 1 1/2" clear of the bolt holes for the CP6 connection. Confirm that is acceptable or supply a solution.						
						<b>ANSWER:</b> Contract Doc Ref: S1-2507 Location: Zone 4, Bus Deck Grid Line: H & 33.5, B & 33.5 Add'l Doc Ref's: CD RFI # 738 SK1  Drawing S1-2507 shows CP6 connections at grid lines B/H and 33.5.  The connections as shown cause a dimension to change and also cause a potential conflict with CP6 colt hole connections.  1) With the 1 1/2" stiffener plate per 4/S1-8003 centered about the CP6 connection, the W40x149 will be located 5'-9 11/16 west of Grid 33.5. Confirm that is acceptable.  2) The edge of the web reinforcing plate per 1/S1-5017 is 1 1/2" clear of the bolt holes for the CP6 connection. Confirm that is acceptable or supply a solution.
T-2001	SSS - AESS and Erection Aid Approval Comment Clarifications GL 24	Closed	CR	12/09/2014	12/09/2014	01/06/2015
From: Webcor Construction LP Stephanie Azzolino						



T-2001.1	SSS - AEISS/IFRM-1 Requirement at Roof Level GL 23E	Closed	01	01/19/2015	01/29/2015	01/21/2015
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**From:** Webcor Construction LP Andrew Kitchen

**REQUEST:**

Contract Doc Ref: T-2001, S1-2605  
Location: GL 23E

Contrary to the response in RFI T-2001, the extent of the painting for the beam on Grid E @ Grid 24 was not provided in the Submittal TG0701-121 (ES4). Supply the extent of the AESS/IFRM-1 requirement.

**ANSWER:**

Contract Doc Ref: E/A1-8660 & A/A1-8662  
Location: Zone 3  
Grid Line: E/24  
Add'l Doc Ref's: CD RFI # 734 SK1, Skanska Shop  
Drawings 6805, TG0701-121

The AAI comments on drawing 6805 in submittal package TG0701-121 noted "AESS with IFRM-1 per E/A1-8660 and A/A1-8662".

The approval comment is a revision to the contract documents as E/A1-8660 & A/A1-8662 are sections along Grid 23 and do not indicate that this beam on Grid E/24 requires AESS/IFRM-1.

If painting is required on this beam, supply the extent on this drawing and on the MF beam on Grid 24 if required.

**ANSWER:**

Contract Doc Ref: T-2001, S1-2605  
Location: GL 23E

Contrary to the response in RFI T-2001, the extent of the painting for the beam on Grid E @ Grid 24 was not provided in the Submittal TG0701-121 (ES4). Supply the extent of the AESS/IFRM-1 requirement.







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<div><div><b>REQUEST:</b><p>Contract Doc Ref: Specification 08 44 27 Section 2.2H Location: Building Exterior Add'l Doc Ref's: RWDI dated June 7, 2014</p><p>The aforementioned report includes an importance factor of 1.15 and with the material safety factors to be utilized this may result in a complicated design and cost implications.</p><p>Please confirm that the 150% is to be applied to the RWDI reported wind pressures for the structural analysis of the panels and the panels support structure.</p></div><div><b>ANSWER:</b><p>Contract Doc Ref: Specification 08 44 27 Section 2.2H Location: Building Exterior Add'l Doc Ref's: RWDI dated June 7, 2014</p><p>The aforementioned report includes an importance factor of 1.15 and with the material safety factors to be utilized this may result in a complicated design and cost implications.</p><p>Please confirm that the 150% is to be applied to the RWDI reported wind pressures for the structural analysis of the panels and the panels support structure.</p></div></div>						
<b>T-2005</b>	<b>EAW - Uniformly Distributed Loads</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2014</b>	<b>12/10/2014</b>	<b>12/15/2014</b>
<div><div><b>From:</b> Webcor/Obayashi Joint Venture      Jonathan Flaming</div><div><b>REQUEST:</b><p>Contract Doc Ref: Specification 08 44 27 Section 2.2K Location: Building Exterior</p><p>The upper capacity of the panels must be achieved under a uniformly distributed load of not more than 150 psf, acting perpendicular to the panel surface. The load acts on the net area of the perforated panel and needs to be considered in both directions (inward and outward).</p><p>It is Larson Engineering Inc (LEI's) interpretation that the panel shall not exceed 150 psf for its ultimate static capacity but can be less as long as the wind loads under ASD design provisions are obtained. Please confirm this interpretation is correct.</p><p>LEI intends to use a phi factor of 0.9 for the ultimate design capacity as a safety margin for the design of the panels and the panels' support structure. Please confirm this is acceptable.</p></div><div><b>ANSWER:</b><p>Contract Doc Ref: Specification 08 44 27 Section 2.2K Location: Building Exterior</p><p>The upper capacity of the panels must be achieved under a uniformly distributed load of not more than 150 psf, acting perpendicular to the panel surface. The load acts on the net area of the perforated panel and needs to be considered in both directions (inward and outward).</p><p>It is Larson Engineering Inc (LEI's) interpretation that the panel shall not exceed 150 psf for its ultimate static capacity but can be less as long as the wind loads under ASD design provisions are obtained. Please confirm this interpretation is correct.</p><p>LEI intends to use a phi factor of 0.9 for the ultimate design capacity as a safety margin for the design of the panels and the panels' support structure. Please confirm this is acceptable.</p></div></div>						
<b>T-2006</b>	<b>SCS - Roof Sky Light Overhang Bent Plate Brace</b>	<b>Open</b>	<b>01</b>	<b>12/10/2014</b>	<b>12/20/2014</b>	<b>12/10/2014</b>

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<b>T-2009</b>	<b>SCS - Roof Park - Notch in Scallop Wall Fins</b>	<b>Open</b>	<b>01</b>	<b>12/10/2014</b>	<b>12/10/2014</b>	
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Contract Doc Ref: S1-3282 Location: Roof Park Grid Line: N/A Add'l Doc Ref's: N/A  Detail 7/S1-3282 shows a notch in the scallop wall fin. This notch is not shown anywhere else in the Contract Documents.  Please provide locations where the notch shown on detail 7/S1-3282 occurs.						<b>ANSWER:</b>  Contract Doc Ref: S1-3282 Location: Roof Park Grid Line: N/A Add'l Doc Ref's: N/A  Detail 7/S1-3282 shows a notch in the scallop wall fin. This notch is not shown anywhere else in the Contract Documents.  Please provide locations where the notch shown on detail 7/S1-3282 occurs.
<b>T-2010</b>	<b>BGP - Extended Concrete Delivery Times - Cemex Mix Design #1574293</b>	<b>Closed</b>	<b>01</b>	<b>12/11/2014</b>	<b>12/21/2014</b>	<b>12/18/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Contract Doc Ref: 03 30 20 3.3c Location: N/A Grid Line: N/A Add'l Doc Ref's: Attached Cemex Letter, Set-Time Results.  Please reference attached set-time results and letter from Cemex QC Mangaer, Robert Foley, regarding 7 day high early mix #1574293 .  Please confirm it is acceptable to extend the delivery time of mix#1574293 to 2 hours?						<b>ANSWER:</b>  Contract Doc Ref: 03 30 20 3.3c Location: N/A Grid Line: N/A Add'l Doc Ref's: Attached Cemex Letter, Set-Time Results.  Please reference attached set-time results and letter from Cemex QC Mangaer, Robert Foley, regarding 7 day high early mix #1574293 .  Please confirm it is acceptable to extend the delivery time of mix#1574293 to 2 hours?
<b>T-2011</b>	<b>BGP - Upturned Beam at GL 21 - Shoring Removal</b>	<b>Closed</b>	<b>01</b>	<b>12/11/2014</b>	<b>12/21/2014</b>	<b>12/11/2014</b>
<b>From:</b> Shimmick Construction Company, Inc. Sylvia Hartanto						
<b>REQUEST:</b>  The response to RFI T-1823 states that it is acceptable to pour the upturned portion of the beam at GL 21 separately.  Please confirm it is acceptable to strip the Lower Concourse at this location(D22 I) prior to the pour of						<b>ANSWER:</b>  The response to RFI T-1823 states that it is acceptable to pour the upturned portion of the beam at GL 21 separately.  Please confirm it is acceptable to strip the Lower Concourse at this location(D22 I) prior to the pour of



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T-2012	<p>the upturned beam.</p> <p><b>SCS - Plate and Bearing Pad Embeds at B141 Beams</b></p> <p><b>From:</b> Shimmick Construction Company, Inc. Henry Chiang</p> <p><b>REQUEST:</b></p> <p>Please reference drawing S1-2251 and detail 10/S1-3411</p> <p>Foundation Wall and Corbels supporting B141 beams are under TG07 scope of work; however the bearing pad and plate embeds are not included in TG07 scope.</p> <p>Please confirm which package is responsible for furnishing the plate and bearing pad embeds.</p>	Closed	01	12/11/2014	12/21/2014	12/11/2014
						<p><b>ANSWER:</b></p> <p>Please reference drawing S1-2251 and detail 10/S1-3411</p> <p>Foundation Wall and Corbels supporting B141 beams are under TG07 scope of work; however the bearing pad and plate embeds are not included in TG07 scope.</p> <p>Please confirm which package is responsible for furnishing the plate and bearing pad embeds.</p>



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T-2013	FRP - UL Listing Reference for Intumescent Fire Resistive Material (IFRM-2)	Open	CR	12/12/2014	12/12/2014	
From: Webcor Construction LP      Tram Nguyen						
REQUEST:		ANSWER:				
Contract Doc Ref: Sheet A1-8662 (ASI 127 dated 9/12/14) Detail A/A1-8662 (ASI 127 dated 9/12/14) Detail A/S1-6005 (IFC Drawings for Main Package dated 3/31/14)		Contract Doc Ref: Sheet A1-8662 (ASI 127 dated 9/12/14) Detail A/A1-8662 (ASI 127 dated 9/12/14) Detail A/S1-6005 (IFC Drawings for Main Package dated 3/31/14)				
Location: Light Column - Train Plaform through Roof Park Level		Location: Light Column - Train Plaform through Roof Park Level				
Grid Line: 22-24 & D-F		Grid Line: 22-24 & D-F				
Add'l Doc Ref's: UL Design Listings (Design No. X674)		Add'l Doc Ref's: UL Design Listings (Design No. X674)				
Per the Fire Protection Matrix & Schedule/A1-8662, the UL Design Listing referenced for IFRM-2 is X674.		Per the Fire Protection Matrix & Schedule/A1-8662, the UL Design Listing referenced for IFRM-2 is X674.				
Per Detail A/A1-8662, IFRM-2 is to be applied to the light column cast pipe members.		Per Detail A/A1-8662, IFRM-2 is to be applied to the light column cast pipe members.				
Per Detail A/S1-6005, the light column cast pipe sizes are 32"x2.36", 30"x2", 28"x1.57", and 30"x1.57".		Per Detail A/S1-6005, the light column cast pipe sizes are 32"x2.36", 30"x2", 28"x1.57", and 30"x1.57".				
The specific member sizes required by the contract documents are not identified within UL Design Listing X674 as tested assemblies.		The specific member sizes required by the contract documents are not identified within UL Design Listing X674 as tested assemblies.				
Please provide direction as to how UL Listing X674 is to be applied to the light column members, or provide a standard to be used for installation of IFRM-2 at the members not listed within UL X674.		Please provide direction as to how UL Listing X674 is to be applied to the light column members, or provide a standard to be used for installation of IFRM-2 at the members not listed within UL X674.				



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<b>T-2013.1</b>	<b>FRP - IFRM Thicknesses and Third Party Assessment Requirements</b>	<b>Open</b>	<b>CR</b>	<b>12/31/2014</b>	<b>12/31/2014</b>	<b>01/20/2015</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: Specification Section 07 81 23 UL Design Listing X674						Contract Doc Ref: Specification Section 07 81 23 UL Design Listing X674
Location: N/A						Location: N/A
Closest Column Line Intersection: N/A						Closest Column Line Intersection: N/A
Add'l Doc Ref's: RFI P1-2013						Add'l Doc Ref's: RFI P1-2013
Some Light Column Steel Members in the contract documents call for IFRM but are not listed on the UL designated for those members.						Some Light Column Steel Members in the contract documents call for IFRM but are not listed on the UL designated for those members.
Per RFI T-2013 the IFRM thickness is to be determined by calculating the A/P value and using the UL table. The response also indicates the requirements are to comply with Specification Section 07 81 23 (1.5) E.						Per RFI T-2013 the IFRM thickness is to be determined by calculating the A/P value and using the UL table. The response also indicates the requirements are to comply with Specification Section 07 81 23 (1.5) E.
Specification Section 07 81 23 (1.5) E 1 requires submitting a certification that proposed fireproofing system is acceptable to Authorities having Jurisdiction.						Specification Section 07 81 23 (1.5) E 1 requires submitting a certification that proposed fireproofing system is acceptable to Authorities having Jurisdiction.
Per discussions with the manufacturer, if the A/P value of the steel is greater than what is shown on UL table, the thickness as shown on the table is acceptable and if a reduction of thickness from the UL table is desired, a third party review would be required.						Per discussions with the manufacturer, if the A/P value of the steel is greater than what is shown on UL table, the thickness as shown on the table is acceptable and if a reduction of thickness from the UL table is desired, a third party review would be required.
Please confirm that it is acceptable to TJPA to use the IFRM thickness from the UL table as long as it meets the A/P requirements, or that a third party assessment will be required only if a reduction in thickness from the UL table is proposed by the subcontractor. (Note: This RFI is being submitted at the request of Ed Sum - See attached.)						Please confirm that it is acceptable to TJPA to use the IFRM thickness from the UL table as long as it meets the A/P requirements, or that a third party assessment will be required only if a reduction in thickness from the UL table is proposed by the subcontractor. (Note: This RFI is being submitted at the request of Ed Sum - See attached.)
<b>T-2014</b>	<b>SCS - Roof Architectural Wall Mix Design</b>	<b>Closed</b>	<b>01</b>	<b>12/12/2014</b>	<b>12/22/2014</b>	<b>12/15/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						





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	<b>REQUEST:</b>  Contract Doc Ref: 03 33 00 1.1a, 03 33 00 1.5c, 03 30 02 2.1 Location: Roof Park Grid Line: N/A Add'l Doc Ref's: N/A  The Cast-In-Place Architectural Concrete specification, 03 33 00, refers you to specification 03 30 02 for concrete mix design.  The concrete properties table in specification section 03 30 02 2.1 does not specifically call out a roof architectural wall mix design.  Please confirm the 5,000 psi ¾ inch slabs, beams, and wall mix is the correct mix design for the root top architectural walls.					<b>ANSWER:</b>  Contract Doc Ref: 03 33 00 1.1a, 03 33 00 1.5c, 03 30 02 2.1 Location: Roof Park Grid Line: N/A Add'l Doc Ref's: N/A  The Cast-In-Place Architectural Concrete specification, 03 33 00, refers you to specification 03 30 02 for concrete mix design.  The concrete properties table in specification section 03 30 02 2.1 does not specifically call out a roof architectural wall mix design.  Please confirm the 5,000 psi ¾ inch slabs, beams, and wall mix is the correct mix design for the root top architectural walls.
<b>T-2015</b>	<b>SSS - Framing Clarifications at Roof Level GL 4-6</b>	<b>Closed</b>	<b>CR</b>	<b>12/12/2014</b>	<b>12/12/2014</b>	<b>12/22/2014</b>
	<b>From:</b> Webcor Construction LP  <b>REQUEST:</b>  Contract Doc Ref: S1-2602 & S1-2603 (ASI 127) Location: Zone 1, Roof Park Level Gridline: 4-4.5/C-D & 4-4.5/F-F.4 Add'l Doc Ref's: CD RFI 657 SK1 & SK2  2.) The 7 beams shown boxed on CD RFI 657 SK1 (referencing drawing S1-2602 from ASI 127) have had their location dimensions that were shown in Field Order 27 removed.  Confirm the dimensions still apply or supply the revised locations.  3.) The beam size at 4/D on CD RFI 657 SK1 (referencing drawing S1-2602 from ASI 127) has been removed.  Confirm the beam is still a W16x26.  4) The beams at 4.5-5.5/D and 4.5-5.5/F on CD RFI 657 SK1 (referencing drawing S1-2602 from ASI 127) were aligned as is shown in detail 1/S1-7102.	Stephanie Azzolino				<b>ANSWER:</b>  Contract Doc Ref: S1-2602 & S1-2603 (ASI 127) Location: Zone 1, Roof Park Level Gridline: 4-4.5/C-D & 4-4.5/F-F.4 Add'l Doc Ref's: CD RFI 657 SK1 & SK2  2.) The 7 beams shown boxed on CD RFI 657 SK1 (referencing drawing S1-2602 from ASI 127) have had their location dimensions that were shown in Field Order 27 removed.  Confirm the dimensions still apply or supply the revised locations.  3.) The beam size at 4/D on CD RFI 657 SK1 (referencing drawing S1-2602 from ASI 127) has been removed.  Confirm the beam is still a W16x26.  4) The beams at 4.5-5.5/D and 4.5-5.5/F on CD RFI 657 SK1 (referencing drawing S1-2602 from ASI



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	Clarify the revised framing and supply the location dimensions for the (4) beams on each side.  5) The 5 beams shown boxed on CD RFI 657 SK2 (referencing drawing S1-2603 from ASI 127) have had their location dimensions that were shown in Field Order 27 removed.  Confirm the dimensions still apply or supply the revised locations.				127) were aligned as is shown in detail 1/S1-7102.  Clarify the revised framing and supply the location dimensions for the (4) beams on each side.  5) The 5 beams shown boxed on CD RFI 657 SK2 (referencing drawing S1-2603 from ASI 127) have had their location dimensions that were shown in Field Order 27 removed.  Confirm the dimensions still apply or supply the revised locations.	
T-2016	<b>SSS - Moment Frame Column Testing Requirements</b>  From: Webcor Construction LP  Gregory Kemerer  <b>REQUEST:</b> Contract Doc Ref: Spec Section 05 12 10-8 Location: Moment Frames Grid Line: N/A Add'l Doc Ref's: SK1  Spec Section 05 12 10-8, Table 2.1 outlines the nondestructive testing requirements.  Please review the following, and confirm:  1) The moment frame columns that are being fabricated are currently being subjected to 100% UT and 100% MT on all weld configurations. However, Table 2.1 shows that only the demand critical welds and butt joints in the column splices should receive the aforementioned testing and that in general, the welds on the moment frame columns should be subject to MT on 25% of the joints and UT on 100% of the joints. Further, testing on the moment frame columns can be reduced as per AISC 341, Appendix Q5.2. Please see attached SK1 that highlights the testing requirements, as understood by Skanska, for the moment frame columns and confirm this is correct.  If this is not accurate, please supply the NDT	Closed	CR	12/15/2014	12/25/2014	01/06/2015
					<b>ANSWER:</b> Contract Doc Ref: Spec Section 05 12 10-8 Location: Moment Frames Grid Line: N/A Add'l Doc Ref's: SK1  Spec Section 05 12 10-8, Table 2.1 outlines the nondestructive testing requirements.  Please review the following, and confirm:  1) The moment frame columns that are being fabricated are currently being subjected to 100% UT and 100% MT on all weld configurations. However, Table 2.1 shows that only the demand critical welds and butt joints in the column splices should receive the aforementioned testing and that in general, the welds on the moment frame columns should be subject to MT on 25% of the joints and UT on 100% of the joints. Further, testing on the moment frame columns can be reduced as per AISC 341, Appendix Q5.2. Please see attached SK1 that highlights the testing requirements, as understood by Skanska, for the moment frame columns and confirm this is correct.	



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requirements for the Moment Frame Columns.

2) Please clarify the Table 2-1 remark under welds not described below, "MT 25% of joints, full length". It is our understanding that 1 out of every 4 joints will receive full length MT inspection and the next 3 joints will not receive MT. Please confirm.

If this is not accurate, please supply the NDT requirements for the Moment Frame Columns.

2) Please clarify the Table 2-1 remark under welds not described below, "MT 25% of joints, full length". It is our understanding that 1 out of every 4 joints will receive full length MT inspection and the next 3 joints will not receive MT. Please confirm.

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<b>T-2018</b>	<b>PLG - Missing information for Drain Assemblies TD-2 &amp; AD-2</b>	<b>Closed</b>	<b>CR</b>	<b>12/16/2014</b>	<b>12/26/2014</b>	<b>01/07/2015</b>
<b>From:</b> Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>  Contract Doc Ref: P1-0051 dated 02/21/14, P1-2307 dated 08/06/14 & A1-2307 dated 09/12/14 Location: Zone 7, Ground Level Closest Column Line Intersection is GL - 33.5 & E Add'l Doc Ref's: L1-2307A dated 03/31/14, & L1-7386 dated 03/31/14  Question : Drains and Cleanout Schedule (P1-0051) does not specify make and model of TD-2 and AD-2.The remarks refer to Landscape and Architectural plans (see A1-2307, L1-2307A & L1-7386) for information, but they too lack information for the same. Please provide required scheduled drain assemblies and update P1-0051 Schedule accordingly.						<b>ANSWER:</b>  Contract Doc Ref: P1-0051 dated 02/21/14, P1-2307 dated 08/06/14 & A1-2307 dated 09/12/14 Location: Zone 7, Ground Level Closest Column Line Intersection is GL - 33.5 & E Add'l Doc Ref's: L1-2307A dated 03/31/14, & L1-7386 dated 03/31/14  Question : Drains and Cleanout Schedule (P1-0051) does not specify make and model of TD-2 and AD-2.The remarks refer to Landscape and Architectural plans (see A1-2307, L1-2307A & L1-7386) for information, but they too lack information for the same. Please provide required scheduled drain assemblies and update P1-0051 Schedule accordingly.
<b>T-2019</b>	<b>BGP - Skewed Electrical Junction Box at Column C173, GL 15/D.8</b>	<b>Closed</b>	<b>CR</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>01/05/2015</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  Contract Doc Ref: N/A Location: C173 Grid Line: 15/D.8 Add'l Doc Ref's: Attached Photo  The lower Electrical Junction Box embedded in column C173 @ GL 15/D.8 was inadvertently skewed (see attached photo) during concrete placement.  Please confirm the as-built condition is acceptable.						<b>ANSWER:</b>  Contract Doc Ref: N/A Location: C173 Grid Line: 15/D.8 Add'l Doc Ref's: Attached Photo  The lower Electrical Junction Box embedded in column C173 @ GL 15/D.8 was inadvertently skewed (see attached photo) during concrete placement.  Please confirm the as-built condition is acceptable.
<b>T-2020</b>	<b>SCS - Moment Frame Beam 1 Beam Location vs Step Location</b>	<b>Closed</b>	<b>01</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>01/04/2015</b>
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>  Contract Doc Ref: A1-2864 Location: Ground Floor Grid Line: 16.9 Add'l Doc Ref's: RFI T-1010.1						<b>ANSWER:</b>  Contract Doc Ref: A1-2864 Location: Ground Floor Grid Line: 16.9 Add'l Doc Ref's: RFI T-1010.1



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	<p>Please refer to RFI T-1010.1 question #5 response.</p> <p>The response to question #5 locates MFB 1, 8'-0" west of GL 16.9 to the beam center line.</p> <p>Drawing A1-2864 gives dimension of 7'-6", from GL 16.9 to a 1'-0" slab step. This would create the center line at 7.50' (LINE 16.9 to the 12" STEP) + 2.50'/2 (MFB1 divided by 2)=8.75'.</p> <p>Please confirm the dimension from GL16.9 to beam center of MFB 1.</p> <p>If the dimension is 8'-0", please provide a revised section of detail 2/S1-3703 reflecting the 12" slab step.</p>					<p>Please refer to RFI T-1010.1 question #5 response.</p> <p>The response to question #5 locates MFB 1, 8'-0" west of GL 16.9 to the beam center line.</p> <p>Drawing A1-2864 gives dimension of 7'-6", from GL 16.9 to a 1'-0" slab step. This would create the center line at 7.50' (LINE 16.9 to the 12" STEP) + 2.50'/2 (MFB1 divided by 2)=8.75'.</p> <p>Please confirm the dimension from GL16.9 to beam center of MFB 1.</p> <p>If the dimension is 8'-0", please provide a revised section of detail 2/S1-3703 reflecting the 12" slab step.</p>
<b>T-2021</b>	<b>SCS - Confirming RFI - Foundation Wall Vertical Trim Bars</b>	<b>Closed</b>	<b>01</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>12/19/2014</b>
<b>From:</b> Webcor Construction LP		Claude Titcher				
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: S1-2307 Location: Foundation Walls Grid Line: N/A Add'l Doc Ref's: Attached.		Contract Doc Ref: S1-2307 Location: Foundation Walls Grid Line: N/A Add'l Doc Ref's: Attached.				
Please reference RFI T-1666.		Please reference RFI T-1666.				
In a breakout meeting held on 11-19-14, Shimmick was directed to detail a case-by-case solution to the foundation wall vertical trim bars that cannot be embedded in the 3rd lift, due to it already being poured. Four penetrations at GL9.9, GL12, and GL13, were detailed and sent to Thornton Tomasetti for comments.		In a breakout meeting held on 11-19-14, Shimmick was directed to detail a case-by-case solution to the foundation wall vertical trim bars that cannot be embedded in the 3rd lift, due to it already being poured. Four penetrations at GL9.9, GL12, and GL13, were detailed and sent to Thornton Tomasetti for comments.				
Per coordination with Thornton Tomasetti, please confirm that the attached sketches and comments are acceptable modifications to foundation wall trim bar reinforcing.		Per coordination with Thornton Tomasetti, please confirm that the attached sketches and comments are acceptable modifications to foundation wall trim bar reinforcing.				



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<b>T-2022</b>	<b>SSS - FO 34 Fouling Connection at HSS Post GL 4</b>	<b>Closed</b>	<b>CR</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>12/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: F & 4 Add'l Doc Ref's: CD RFI # 718 SK1  Drawing S1-2701 shows a connection between a HSS post and a beam near grid lines F & 4.  This HSS connection will not clear the beam to beam connection per 1/S1-5010 below as the bolts/stiffeners per 1/S1-2720 will foul the beam connection.  Please supply a connection detail for the HSS5x5x3/8 post.						<b>ANSWER:</b>  Contract Doc Ref: S1-2701 Location: Zone 1, Roof Grid Line: F & 4 Add'l Doc Ref's: CD RFI # 718 SK1  Drawing S1-2701 shows a connection between a HSS post and a beam near grid lines F & 4.  This HSS connection will not clear the beam to beam connection per 1/S1-5010 below as the bolts/stiffeners per 1/S1-2720 will foul the beam connection.  Please supply a connection detail for the HSS5x5x3/8 post.
<b>T-2023</b>	<b>SSS - Missing Information at Bus Deck Level GL 32.4-33.2</b>	<b>Closed</b>	<b>CR</b>	<b>12/18/2014</b>	<b>12/28/2014</b>	<b>12/27/2014</b>
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Contract Doc Ref: S1-2507 Location: Zone 4, Bus Deck Grid Line: D & 33 Add'l Doc Ref's: CD RFI # 737 SK1  Drawing S1-2507 provides information for the framing of the bus deck near grid lines D and 33.  Some of the information is missing. Please provide clarification on the following:  1) Confirm 8/S1-5003 applies between Grids D-D.4, opposite of Grids E.6-F. 2) Confirm the noted symbols indicate bracing per 8/S1-5015 at (8) locations hi-lited. 3) Supply the missing clouded dimensions at (8) locations.						<b>ANSWER:</b>  Contract Doc Ref: S1-2507 Location: Zone 4, Bus Deck Grid Line: D & 33 Add'l Doc Ref's: CD RFI # 737 SK1  Drawing S1-2507 provides information for the framing of the bus deck near grid lines D and 33.  Some of the information is missing. Please provide clarification on the following:  1) Confirm 8/S1-5003 applies between Grids D-D.4, opposite of Grids E.6-F. 2) Confirm the noted symbols indicate bracing per 8/S1-5015 at (8) locations hi-lited. 3) Supply the missing clouded dimensions at (8) locations.
<b>T-2023.1</b>	<b>SSS - Missing Dimensions at Bus Deck Level GL 32.4-33.2</b>	<b>Closed</b>	<b>01</b>	<b>01/19/2015</b>	<b>01/29/2015</b>	<b>02/10/2015</b>
<b>From:</b> Webcor Construction LP Andrew Kitchen						
<b>REQUEST:</b>						<b>ANSWER:</b>

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	<p>Contract Doc Ref: T-2023, S1-2507  Location: GL 32.4-33.2  Add'l Doc Ref: CD RFI 737.1 SK1</p> <p>Contrary to the response in RFI T-2023 item 3, the requested missing dimensions are not clearly shown on S1-6058. Confirm the (6) clouded dimensions, or supply the dimensions if the shown dimensions on SK1 are not correct.</p>					
	<p>Contract Doc Ref: T-2023, S1-2507  Location: GL 32.4-33.2  Add'l Doc Ref: CD RFI 737.1 SK1</p> <p>Contrary to the response in RFI T-2023 item 3, the requested missing dimensions are not clearly shown on S1-6058. Confirm the (6) clouded dimensions, or supply the dimensions if the shown dimensions on SK1 are not correct.</p>					
<b>T-2024</b>	<b>SSS - Transfer Girder Studs and Rebar Holes</b>	<b>Closed</b>	<b>01</b>	<b>12/12/2013</b>	<b>12/22/2013</b>	<b>03/11/2014</b>
	<p><b>From:</b> Webcor Construction LP                      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>At TR8 near grid line G refer to sketches CD RFI 220 SK1 to SK3 for items 1 to 3:</p> <p>1) Confirm the headed studs as shown are correct (work with item 2).</p> <p>2) Detail 2/S1-5023 is referenced with a "SIM" designation and it is not clear what is required on grid 8 for the additional headed studs shown in detail 2/S1-5023. Confirm the headed studs as shown on SK3 are acceptable or supply a clarifying detail specifically for this location showing the stud locations.</p> <p>3a) Confirm the 2" dia. hole locations as shown on SK3 are acceptable to clear the bolts in the bottom flange and the stiffeners.</p> <p>3b) Detail 2/S1-5023 shows the holes at 5" OC but this contradicts the 6" OC shown in detail 7/S1-3701. Confirm the spacing shown in item 3a above is acceptable.</p> <p>3c) Confirm the 3" dia holes are not required at grid 8 as they are not shown in detail 7/S1-3701. Supply location dimensions if they are required.</p>					
	<p><b>ANSWER:</b></p> <p>At TR8 near grid line G refer to sketches CD RFI 220 SK1 to SK3 for items 1 to 3:</p> <p>1) Confirm the headed studs as shown are correct (work with item 2).</p> <p>2) Detail 2/S1-5023 is referenced with a "SIM" designation and it is not clear what is required on grid 8 for the additional headed studs shown in detail 2/S1-5023. Confirm the headed studs as shown on SK3 are acceptable or supply a clarifying detail specifically for this location showing the stud locations.</p> <p>3a) Confirm the 2" dia. hole locations as shown on SK3 are acceptable to clear the bolts in the bottom flange and the stiffeners.</p> <p>3b) Detail 2/S1-5023 shows the holes at 5" OC but this contradicts the 6" OC shown in detail 7/S1-3701. Confirm the spacing shown in item 3a above is acceptable.</p> <p>3c) Confirm the 3" dia holes are not required at grid 8 as they are not shown in detail 7/S1-3701. Supply location dimensions if they are required.</p>					





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	<p>Contract Doc Ref: N/A Location: N/A Gridline: 24 Add'l Doc Ref's: RFI-T-1334, TG0701-121, Shop Drawing 6716AC &amp; 7386AC, CD RFI #733 SK1</p> <p>The attached CD RFI #733 SK1 shows the edge plate at curved slab edges approved in RFI T-1334.</p> <p>Please supply a revised detail if the edge plate is to be revised.</p>					
<b>T-2028</b>	<b>Drawing References to Deleted Sheet A1-8168</b>	<b>Closed</b>	<b>CR</b>	<b>12/22/2014</b>	<b>01/01/2015</b>	<b>01/06/2015</b>
	<p><b>From:</b> Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>Contract Document Ref: (ASI 128 dated 12/16/14) Sheet A1-2502 Sheet A1-6002 Consolidated Drawing List</p> <p>Location: Bus Deck Level</p> <p>Closet Column Line Intersection: 2-D.4</p> <p>Ass'l Doc Ref's: CR No. T-128 - ASI #128 IFC Conformed Set</p> <p>Sheet A1-2502 references Detail -/A1-8168 for Prefabricated Booth at Bus Deck Superintendent Station 03620.</p> <p>Sheet A1-6002 references Detail A/A1-8168 at Bus Deck Superintendent Station 03620.</p> <p>Per the Consolidated Drawing List, Sheet A1-8168 has been deleted from the drawing set.</p> <p>Please provide Sheet A1-8168 as called out on Sheets</p>					
	<p>Contract Doc Ref: N/A Location: N/A Gridline: 24 Add'l Doc Ref's: RFI-T-1334, TG0701-121, Shop Drawing 6716AC &amp; 7386AC, CD RFI #733 SK1</p> <p>The attached CD RFI #733 SK1 shows the edge plate at curved slab edges approved in RFI T-1334.</p> <p>Please supply a revised detail if the edge plate is to be revised.</p>					
	<p><b>ANSWER:</b></p> <p>Contract Document Ref: (ASI 128 dated 12/16/14) Sheet A1-2502 Sheet A1-6002 Consolidated Drawing List</p> <p>Location: Bus Deck Level</p> <p>Closet Column Line Intersection: 2-D.4</p> <p>Ass'l Doc Ref's: CR No. T-128 - ASI #128 IFC Conformed Set</p> <p>Sheet A1-2502 references Detail -/A1-8168 for Prefabricated Booth at Bus Deck Superintendent Station 03620.</p> <p>Sheet A1-6002 references Detail A/A1-8168 at Bus Deck Superintendent Station 03620.</p> <p>Per the Consolidated Drawing List, Sheet A1-8168 has been deleted from the drawing set.</p> <p>Please provide Sheet A1-8168 as called out on</p>					



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	A1-2502 and A1-6002 or revise the references to call out the correct sheet.					Sheets A1-2502 and A1-6002 or revise the references to call out the correct sheet.
<b>T-2029</b>	<b>BGP - Phase 2 Bike and Vehicle Ramp Details</b>	<b>Closed</b>	<b>CR</b>	<b>12/23/2014</b>	<b>12/23/2014</b>	<b>01/15/2015</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> See attached agenda with RFI Action Items from SCCI's 12/ 18/14 Phase 2 Ramp Design Walk through Meeting. Please provide confirmation and/or further details for Action Items #1-10.						<b>ANSWER:</b> See attached agenda with RFI Action Items from SCCI's 12/ 18/14 Phase 2 Ramp Design Walk through Meeting. Please provide confirmation and/or further details for Action Items #1-10.



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T-2030	SCS - Elevation of Curbs at Transformer Vault Openings	Closed	CR	12/23/2014	01/02/2015	01/13/2015
<p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p>						
<p><b>REQUEST:</b></p> <p>Contract Doc Ref: 2/A1-3001 - 3003, A1-2862, A1-2865</p> <p>Location: Zone 1 to 3, Ground Level</p> <p>Gridline: B/1.4-5, B/9.8-12, H/20.1-22</p> <p>Add'l Doc Ref's: N/A</p> <p>Please reference Detail 2 of Drawings A1-3001 - A1-3003 and See attached slab edge plan drawings with markups of slab opening curbs.</p> <p>On Drawings A1-2862, A1-2862 and A1-2865, Curbs are called out at the Transformer Vault room openings, however the top of curb elevations are not provided or called out as "Height to Be Coordinated". Detail 2 of Drawings A1-3001 - A1-3003 call out Transformer Vault curb elevations as Top of Cover. SCCI requires confirmation on curb elevations in order to appropriately plan for construction of openings on the ground level.</p> <p>Please confirm if A1-3001 - A1-3003 Detail 2, Top of Cover elevations are top of curb elevations for Transformer Vault room opening curbs, if not please provide elevations.</p>			<p><b>ANSWER:</b></p> <p>Contract Doc Ref: 2/A1-3001 - 3003, A1-2862, A1-2865</p> <p>Location: Zone 1 to 3, Ground Level</p> <p>Gridline: B/1.4-5, B/9.8-12, H/20.1-22</p> <p>Add'l Doc Ref's: N/A</p> <p>Please reference Detail 2 of Drawings A1-3001 - A1-3003 and See attached slab edge plan drawings with markups of slab opening curbs.</p> <p>On Drawings A1-2862, A1-2862 and A1-2865, Curbs are called out at the Transformer Vault room openings, however the top of curb elevations are not provided or called out as "Height to Be Coordinated". Detail 2 of Drawings A1-3001 - A1-3003 call out Transformer Vault curb elevations as Top of Cover. SCCI requires confirmation on curb elevations in order to appropriately plan for construction of openings on the ground level.</p> <p>Please confirm if A1-3001 - A1-3003 Detail 2, Top of Cover elevations are top of curb elevations for Transformer Vault room opening curbs, if not please provide elevations.</p>			







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<b>T-2033</b>	<b>SSS - Added Shear Studs on Decking Package TG701-92F</b>	<b>Closed</b>	<b>CR</b>	<b>12/24/2014</b>	<b>12/24/2014</b>	<b>02/02/2015</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: 2/A-8237 Location: Zone 3, Roof Park & Bus Deck Gridline: E/19.9 thru 24.9 Add'l Doc Ref's: TG701-92F, SK RFI 993 SK1 & SK2		Contract Doc Ref: 2/A-8237 Location: Zone 3, Roof Park & Bus Deck Gridline: E/19.9 thru 24.9 Add'l Doc Ref's: TG701-92F, SK RFI 993 SK1 & SK2				
Engineer's review comments on SK RFI 993 SK1 & SK2 from decking drawing package TG701-92F added 182 shear studs to the bus deck level and roof level		Engineer's review comments on SK RFI 993 SK1 & SK2 from decking drawing package TG701-92F added 182 shear studs to the bus deck level and roof level				
The added 182 shear studs to the bus deck level and roof level are not indicated on the latest structural drawings provided to Skanska.		The added 182 shear studs to the bus deck level and roof level are not indicated on the latest structural drawings provided to Skanska.				
Please verify if the additional shear studs are required.		Please verify if the additional shear studs are required.				
<b>T-2034</b>	<b>SSS - Conflicting Connection at GL 33.5-E</b>	<b>Closed</b>	<b>CR</b>	<b>12/24/2014</b>	<b>12/24/2014</b>	<b>01/14/2015</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: S1-2607, 1/S1-5013, 2/S1-4018, 1/S1- 8002 Location: Zone 4, Roof Park Gridline: E/33.5 Add'l Doc Ref's: CD RFI 740 SK1		Contract Doc Ref: S1-2607, 1/S1-5013, 2/S1-4018, 1/S1-8002 Location: Zone 4, Roof Park Gridline: E/33.5 Add'l Doc Ref's: CD RFI 740 SK1				
The attached CD RFI # 740 SK1 at GL E/33.5 shows the connection for the W33x118 to the BU-40x22.		The attached CD RFI # 740 SK1 at GL E/33.5 shows the connection for the W33x118 to the BU-40x22.				
The connection details for the W33x118 to the BU-40x22 show conflicting information. Per S1-2607 the connection is to be per 1/S1-5013 and per 2/S1-4018 the connection is to be per 1/S1-8002 for the CP4 connection.		The connection details for the W33x118 to the BU- 40x22 show conflicting information. Per S1-2607 the connection is to be per 1/S1-5013 and per 2/S1-4018 the connection is to be per 1/S1-8002 for the CP4 connection.				
Please supply correct detail		Please supply correct detail				
<b>T-2034.1</b>	<b>SSS - Conflicting Connection at GL 33.5-E</b>	<b>Closed</b>	<b>CR</b>	<b>01/30/2015</b>	<b>02/09/2015</b>	<b>02/06/2015</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						



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	<b>REQUEST:</b>  Contract Doc: S1-2607, S1-8002 Location: Zone 4, Roof Park Gridline: E/33.5 Add'l Doc Ref's: RFI T-2034, CD RFI 740.1 SK1  Per RFI T-2034, detail 1/S1-8002 was supplied for the W33x118 to the BU-40x22 connection at GL E/33.5.  The connection per 1/S1-8002 will not work at the noted location: > 1/S1-8002 calls for 11 bolts but only 9 bolts will fit inside the W33. > the bottom flange of the W33 cannot be moment welded.  Please see the proposed connection in the attached CD RFI 740.1 SK1.  Confirm the connection is acceptable as shown or supply a new detail.					
	<b>ANSWER:</b>  Contract Doc: S1-2607, S1-8002 Location: Zone 4, Roof Park Gridline: E/33.5 Add'l Doc Ref's: RFI T-2034, CD RFI 740.1 SK1  Per RFI T-2034, detail 1/S1-8002 was supplied for the W33x118 to the BU-40x22 connection at GL E/33.5.  The connection per 1/S1-8002 will not work at the noted location: > 1/S1-8002 calls for 11 bolts but only 9 bolts will fit inside the W33. > the bottom flange of the W33 cannot be moment welded.  Please see the proposed connection in the attached CD RFI 740.1 SK1.  Confirm the connection is acceptable as shown or supply a new detail.					





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<b>T-2035</b>	<b>SST - Stair 301 Landing Support Framing Conflict</b>	<b>Closed</b>	<b>CR</b>	<b>12/24/2014</b>	<b>01/03/2015</b>	<b>01/14/2015</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: 1/A1-7006, 1, 2/S1-7006, 6/S1-7601			Contract Doc Ref: 1/A1-7006, 1, 2/S1-7006, 6/S1-7601			
Location: Zone 1, Ground & Second Level			Location: Zone 1, Ground & Second Level			
Gridline: E/7			Gridline: E/7			
Add'l Doc Ref's: CWSD RFI #27			Add'l Doc Ref's: CWSD RFI #27			
See the attached sketch.			See the attached sketch.			
The beam and column locations at Stair 301 intermediate landing EL. 29'-0 1/8" are setup too narrow to support the outside stringers per required detail 6/S1-7601.			The beam and column locations at Stair 301 intermediate landing EL. 29'-0 1/8" are setup too narrow to support the outside stringers per required detail 6/S1-7601.			
Per the attached sketch, confirm the header beam can sit on top of the columns and extend East and West as required to support the stringers. If not, please advise on a solution.			Per the attached sketch, confirm the header beam can sit on top of the columns and extend East and West as required to support the stringers. If not, please advise on a solution.			





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Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-2038	SST - Stair 502 Stringer to Landing Support Clarification	Closed	CR	12/24/2014	01/03/2015	01/14/2015
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: 2/S1-2252, 1/S1-5034 Location: Zone 3, Lower Concourse Gridline: F.7/20.1 Add'l Doc Ref's: CWSD RFI #26		Contract Doc Ref: 2/S1-2252, 1/S1-5034 Location: Zone 3, Lower Concourse Gridline: F.7/20.1 Add'l Doc Ref's: CWSD RFI #26				
1. Drawing 2/S1-2252 calls for a W6x25 at the edge of the 7'4 landing.		1. Drawing 2/S1-2252 calls for a W6x25 at the edge of the 7'4 landing.				
The location of the beam is not clear per 1/S1-5034.		The location of the beam is not clear per 1/S1-5034.				
Please advise on where the beam should be located.		Please advise on where the beam should be located.				
2. Per drawing 1/S1-5034, the W6x25 is to be located so that the bottom flange support the 7 ½" Slab, leaving a 1 5/8" slab over the W6x26.		2. Per drawing 1/S1-5034, the W6x25 is to be located so that the bottom flange support the 7 ½" Slab, leaving a 1 5/8" slab over the W6x26.				
Since the beam is not flush with the edge of slab, connections similar to 3/S1-7601 and 2/S1-7603 will not work.		Since the beam is not flush with the edge of slab, connections similar to 3/S1-7601 and 2/S1-7603 will not work.				
Please advise.		Please advise.				
T-2039	SST - Stair 307 308 309 Mid-landing Support	Closed	CR	12/24/2014	01/03/2015	01/14/2015
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: 4/S1-7604, 7/A1-7511 Location: Terrazzo Gridline: N/A Add'l Doc Ref's: CWSD RFI #28		Contract Doc Ref: 4/S1-7604, 7/A1-7511 Location: Terrazzo Gridline: N/A Add'l Doc Ref's: CWSD RFI #28				
The attached sketch refers to Stairs 307, 308 and 309.		The attached sketch refers to Stairs 307, 308 and 309.				
Please clarify acceptable stringer setup to support Terrazzo risers and installation of pipe sleeves with electrical conduits for the illuminated rails. Per structural details 4/S1-7604 and 7/A1-7511, CAL West Steel Detailing is unable to attach pipe sleeves and their required electrical case.		Please clarify acceptable stringer setup to support Terrazzo risers and installation of pipe sleeves with electrical conduits for the illuminated rails. Per structural details 4/S1-7604 and 7/A1-7511, CAL West Steel Detailing is unable to attach pipe sleeves and their required electrical case.				
Please advise.		Please advise.				



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T-2040	PFB - Prefabricated Guard Booth Electrical Clarification per Sheet Notes	Closed	CR	12/24/2014	01/03/2015	01/12/2015
From: Webcor Construction LP      Tram Nguyen						
REQUEST:		ANSWER:				
Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 13 34 24 Sheet E1-2310 Sheet E1-2306 Sheet E1-2502		Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 13 34 24 Sheet E1-2310 Sheet E1-2306 Sheet E1-2502				
Location: Ground Level Bus Deck Level		Location: Ground Level Bus Deck Level				
Add'l Doc Ref's: N/A		Add'l Doc Ref's: N/A				
Per Specification Section 13 34 24 (2.3) N.1 Prefabricated Guard Booths, "booth shall be delivered prewired and ready for site power connections by the booth contractor", referencing Sheet E1-2310, E1-2306, and E1-2502 for booth type 1, 2 and 3, respectively.		Per Specification Section 13 34 24 (2.3) N.1 Prefabricated Guard Booths, "booth shall be delivered prewired and ready for site power connections by the booth contractor", referencing Sheet E1-2310, E1-2306, and E1-2502 for booth type 1, 2 and 3, respectively.				
1.) Note 4 of Numbered Notes on Sheet E1-2310 states, "Equipment and devices will be provided with prefab booth." However, Note 4 on Sheet E1-2310 at Prefab Guardhouse 01960 states that it is "NIC".		1.) Note 4 of Numbered Notes on Sheet E1-2310 states, "Equipment and devices will be provided with prefab booth." However, Note 4 on Sheet E1-2310 at Prefab Guardhouse 01960 states that it is "NIC".				
2.) Note 6 of Numbered Notes on Sheet E1-2306 states, "Equipment and devices will be provided with prefab booth." However, Note 6 on Sheet E1-2310 at GGT Supervisor Booth 01622 states that it is "NIC".		2.) Note 6 of Numbered Notes on Sheet E1-2306 states, "Equipment and devices will be provided with prefab booth." However, Note 6 on Sheet E1-2310 at GGT Supervisor Booth 01622 states that it is "NIC".				
3.) Note 5 of Numbered Notes on Sheet E1-2502 states, "Equipment and devices will be provided with prefab booth." However, Note 5 on Sheet E1-2502 at Bus Deck Superintendent Station 03260 states that it is "NIC".		3.) Note 5 of Numbered Notes on Sheet E1-2502 states, "Equipment and devices will be provided with prefab booth." However, Note 5 on Sheet E1-2502 at Bus Deck Superintendent Station 03260 states that it is "NIC".				
Please confirm that all electrical equipment, devices, and site power connections for prefabricated booths are to be furnished and installed by the booth contract per Specification Section 13 34 24 (2.3) N.1 or revise the sheet notes to match.		Please confirm that all electrical equipment, devices, and site power connections for prefabricated booths are to be furnished and installed by the booth contract per Specification Section 13 34 24 (2.3) N.1 or revise the sheet notes to match.				
T-2040.1	INT - Electrical Work Scope Within Prefabricated Units	Closed	CR	01/12/2015	01/22/2015	01/23/2015
From: Webcor Construction LP      Tram Nguyen						



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<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: (ASI 128 dated 12/16/2014)			Contract Doc Ref: (ASI 128 dated 12/16/2014)			
Sheet E1-2306			Sheet E1-2306			
Sheet E1-2310			Sheet E1-2310			
Sheet E1-2502			Sheet E1-2502			
Location:			Location:			
Prefabricated Buildings			Prefabricated Buildings			
Closest Column Line Intersection:			Closest Column Line Intersection:			
Add'l Doc Ref's:			Add'l Doc Ref's:			
RFI P1-2040			RFI P1-2040			
Per the response to RFI T-2040, the "NIC" within the plans is to remain.			Per the response to RFI T-2040, the "NIC" within the plans is to remain.			
During the QBD meeting on 01/09/2014, it was agreed that the electrical work within the prefabricated units is to be included in the contract work.			During the QBD meeting on 01/09/2014, it was agreed that the electrical work within the prefabricated units is to be included in the contract work.			
Please confirm that the electrical work within the prefabricated units is to be included in the contract work, and update the electrical drawings to remove the "NIC" notes at these locations. (Note: See the attached drawings showing the notes to be struck)			Please confirm that the electrical work within the prefabricated units is to be included in the contract work, and update the electrical drawings to remove the "NIC" notes at these locations. (Note: See the attached drawings showing the notes to be struck)			

<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
<b>T-2041</b>	<b>BSE - ASI 0128 Revision Narrative</b>	<b>Open</b>	<b>CR</b>	<b>12/24/2014</b>	<b>01/03/2015</b>	
<div> <div> <b>From:</b> Webcor Construction LP           Stephanie Azzolino         </div> <div> <b>REQUEST:</b> <p>Contract Doc Ref: ASI 0128 Revision Narrative (dated December 16, 2014)            Location: N/A            Gridline: N/A            Add'l Doc Ref's: N/A</p> <p>ASI 0128 Conformed IFC set was issued with a revision narrative that lists of the trade packages to which it applies. TG03 - Buttress, Shoring, and Excavation package was not included.</p> <p>Please confirm that the ASI 0128 Conformed IFC set is not incorporated into the TG03 Contract Documents.</p> </div> <div> <b>ANSWER:</b> <p>Contract Doc Ref: ASI 0128 Revision Narrative (dated December 16, 2014)            Location: N/A            Gridline: N/A            Add'l Doc Ref's: N/A</p> <p>ASI 0128 Conformed IFC set was issued with a revision narrative that lists of the trade packages to which it applies. TG03 - Buttress, Shoring, and Excavation package was not included.</p> <p>Please confirm that the ASI 0128 Conformed IFC set is not incorporated into the TG03 Contract Documents.</p> </div> </div>						



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T-2042	PFB - Make-Up Air Requirements for Restrooms 01640, 01641, 01620, and 01621	pe Closed	CR	12/29/2014	01/08/2015	01/15/2015
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:			ANSWER:			
Contract Doc Ref: (ASI 128 dated 12/16/14) Sheet M1-2306 Specification Section 13 34 24			Contract Doc Ref: (ASI 128 dated 12/16/14) Sheet M1-2306 Specification Section 13 34 24			
Location: Restrooms 01640, 01641, 01620, 01621			Location: Restrooms 01640, 01641, 01620, 01621			
Closest Column Line Intersections: 30 & 31 - C 30 & 31 - F.7			Closest Column Line Intersections: 30 & 31 - C 30 & 31 - F.7			
Add'l Doc Ref's: N/A			Add'l Doc Ref's: N/A			
Sheet M1-2306 indicates that there are exhaust fans and supply air from the heat pump within Restrooms 01640, 01641, 01620, and 01621.			Sheet M1-2306 indicates that there are exhaust fans and supply air from the heat pump within Restrooms 01640, 01641, 01620, and 01621.			
No make-up air or return plenums are indicated on the mechanical plans. No louvers are called out within Specification Section 13 34 24 Prefabricated Guard Booths or on the architectural drawings. This condition may prevent the restroom doors from properly working.			No make-up air or return plenums are indicated on the mechanical plans. No louvers are called out within Specification Section 13 34 24 Prefabricated Guard Booths or on the architectural drawings. This condition may prevent the restroom doors from properly working.			
Please confirm no make-up air is required at these locations.			Please confirm no make-up air is required at these locations.			

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-2043	PFB - Electrical Requirements for Booth Type 1 and 3	Closed	CR	12/29/2014	01/08/2015	01/07/2015
<div> <div> <b>From:</b> Webcor Construction LP           Tram Nguyen         </div> <div> <b>REQUEST:</b> <p>Contract Doc Ref: (ASI 128 dated 12/16/14)            Specification Section 13 34 24            Sheet E1-2310            Sheet E1-2502</p> <p>Location:            Ground Level            Bus Deck Level</p> <p>Add'l Doc Ref's: N/A</p> <p>Specification Section 13 34 24 (2.3) N 1 Prefabricated Guard Booths indicates that electrical equipment and devices for Booth Type 1 and 3 are per specifications and drawings E1-2310 and E1-2502, respectively.</p> <p>No receptacles are shown within Prefab Guardhouse 01960 on Sheet E1-2310 or Bus Deck Superintendent Station 03260 on Sheet E1-2502.</p> <p>Please confirm no receptacles, or any other electrical, other than the mechanical connections shown, are required within Booth Type 1 and 3.</p> </div> <div> <b>ANSWER:</b> <p>Contract Doc Ref: (ASI 128 dated 12/16/14)            Specification Section 13 34 24            Sheet E1-2310            Sheet E1-2502</p> <p>Location:            Ground Level            Bus Deck Level</p> <p>Add'l Doc Ref's: N/A</p> <p>Specification Section 13 34 24 (2.3) N 1 Prefabricated Guard Booths indicates that electrical equipment and devices for Booth Type 1 and 3 are per specifications and drawings E1-2310 and E1-2502, respectively.</p> <p>No receptacles are shown within Prefab Guardhouse 01960 on Sheet E1-2310 or Bus Deck Superintendent Station 03260 on Sheet E1-2502.</p> <p>Please confirm no receptacles, or any other electrical, other than the mechanical connections shown, are required within Booth Type 1 and 3.</p> </div> </div>						





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<b>T-2044</b>	<b>PFB - Telecommunication Requirements per Specification Section 13 34 24</b>	<b>Closed</b>	<b>CR</b>	<b>12/29/2014</b>	<b>01/08/2015</b>	<b>01/30/2015</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 13 34 24 Sheet TE1-5304 Sheet TE1-4306 Sheet TE1-4307 Sheet A1-8278		Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 13 34 24 Sheet TE1-5304 Sheet TE1-4306 Sheet TE1-4307 Sheet A1-8278				
Location: Ground Level		Location: Ground Level				
Closest Column Line Intersections: 30 & 31 - C 30 & 31 - F.7		Closest Column Line Intersections: 30 & 31 - C 30 & 31 - F.7				
Add'l Doc Ref's: N/A		Add'l Doc Ref's: N/A				
Specification Section 13 34 24 (2.3) Q.1 a and b Prefabricated Guard Booths reference Sheets TE1-5304, TE1-4306, TE1-4307, and A1-8278.		Specification Section 13 34 24 (2.3) Q.1 a and b Prefabricated Guard Booths reference Sheets TE1- 5304, TE1-4306, TE1-4307, and A1-8278.				
1.) Sheet TE1-5304 was not issued. 2.) TE1-4306 does not show any telecom information within GGT Supervisor Booth 01622 or SMTFA 01642. 3.) Sheet TE1-4307 does not show a booth. 4.) Sheet A1-8278 was not issued.		1.) Sheet TE1-5304 was not issued. 2.) TE1-4306 does not show any telecom information within GGT Supervisor Booth 01622 or SMTFA 01642. 3.) Sheet TE1-4307 does not show a booth. 4.) Sheet A1-8278 was not issued.				
Please provide the referenced telecom information, or revise the specification to the intended reference documents.		Please provide the referenced telecom information, or revise the specification to the intended reference documents.				





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Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-2046	FCL - Additional Final Cleaning Requirements per Specification Section 01 17 00	Open	CR	12/29/2014	01/08/2015	
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:			ANSWER:			
Contract Doc Ref: (IFC Drawings for Main Package dated 3/31/14)			Contract Doc Ref: (IFC Drawings for Main Package dated 3/31/14)			
Specification Section 01 17 00			Specification Section 01 17 00			
Location: N/A			Location: N/A			
Closest Column Line Intersection: N/A			Closest Column Line Intersection: N/A			
Add'l Doc Ref's: N/A			Add'l Doc Ref's: N/A			
Specification Section 01 17 00 (1.5) H Completion and Contract Close-Out indicates that there are additional final cleaning requirements in Section 01 74 23.			Specification Section 01 17 00 (1.5) H Completion and Contract Close-Out indicates that there are additional final cleaning requirements in Section 01 74 23.			
Specification Section 01 74 23 has not been issued and is not shown in Specification Section 00 01 10 Table of Contents.			Specification Section 01 74 23 has not been issued and is not shown in Specification Section 00 01 10 Table of Contents.			
Please provide Specification Section 01 74 23, or revise Specification Section 01 17 00 (1.5) H to reference the desired specification.			Please provide Specification Section 01 74 23, or revise Specification Section 01 17 00 (1.5) H to reference the desired specification.			



**ANSWER:**

Contract Doc Ref:  
Specification Section 13 34 24 (ASI 128 dated 12/16/14)  
Specification Section 00 05 20 (IFC Drawings for Main Package dated 3/31/14)

Location: N/A

Closest Column Line Intersection: N/A

Add'l Doc Ref's: N/A

Specification Section 13 34 24 (2.4) A, B & C  
Prefabricated Guard Booths call out for the use of Linoleum.

Per Specification Section 00 05 20 2.07 Agreement, sole source procurement is only acceptable with prior approval by TJPA. In addition, no specification has been provided for Linoleum.

Please confirm it is acceptable to single source this sheet vinyl, and provide a specification for it.

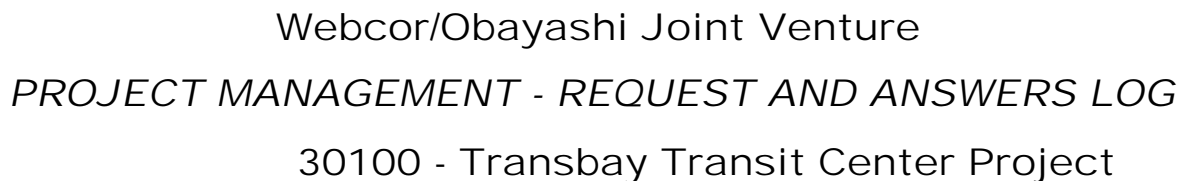


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Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-2048	PFB - Specifications for Aluminum Ceiling Tile	Closed	CR	12/29/2014	01/08/2015	01/16/2015
<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div></div>						
REQUEST:			ANSWER:			
Contract Doc Ref: (ASI 128 dated 12/16/14)			Contract Doc Ref: (ASI 128 dated 12/16/14)			
Specification Section 13 34 24			Specification Section 13 34 24			
Specification Section 09 51 00			Specification Section 09 51 00			
Location: N/A			Location: N/A			
Closest Column Line Intersection: N/A			Closest Column Line Intersection: N/A			
Add'l Doc Ref's: N/A			Add'l Doc Ref's: N/A			
Specification Section 13 34 24 (2.4) A, B & C			Specification Section 13 34 24 (2.4) A, B & C			
Prefabricated Guard Booths call out for the use of perforated aluminum acoustic ceiling tile.			Prefabricated Guard Booths call out for the use of perforated aluminum acoustic ceiling tile.			
Specification Section 09 51 00 Acoustic Ceiling Tiles does not identify an aluminum acoustic tile.			Specification Section 09 51 00 Acoustic Ceiling Tiles does not identify an aluminum acoustic tile.			
Please provide a specification for the aluminum ceiling tile called out in Specification Section 13 34 24.			Please provide a specification for the aluminum ceiling tile called out in Specification Section 13 34 24.			



**ANSWER:**

Contract Doc Ref: (ASI 128 dated 12/16/14)  
Specification Section 13 34 24  
Sheet E1-2310  
Sheet E1-2306  
Sheet E1-2502

Location: N/A

Closest Column Line Intersection: N/A

Add'l Doc Ref's: N/A

Specification Section 13 34 24 (2.3) N Prefabricated Guard Booths indicates that Sheets E1-2310, E1-2306, and E1-2502 are to be used for installation of electrical within the prefabricated guard booths.

Sheets E1-2310, E1-2306, and E1-2502 show the electrical for prefabricated guard booths as home runs to electrical panels within electrical rooms not integrated with the booths.

Specification Section 13 34 24 (3.2) C indicates that there is to be a single booth panel board.

Please revise the electrical drawings to show a single booth panel board which feeds all electrical to the booth, or revise the specification to agree with the drawings.



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<b>T-2051</b>	<b>INT - Corrosion Analysis and Engineer Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>12/31/2014</b>	<b>01/10/2015</b>	<b>01/13/2015</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 07 42 13		Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 07 42 13				
Location: N/A		Location: N/A				
Closest Column Line Intersection: N/A		Closest Column Line Intersection: N/A				
Add'l Doc Ref's: N/A		Add'l Doc Ref's: N/A				
Specification Section 07 42 13 (1.9) C Preformed Metal and Clay Panel Cladding requires a corrosion analysis by a licensed engineer.		Specification Section 07 42 13 (1.9) C Preformed Metal and Clay Panel Cladding requires a corrosion analysis by a licensed engineer.				
Per previous discussions, QBD's (for example, TG07.3-145), and RFI's (for example, P1-0418):		Per previous discussions, QBD's (for example, TG07.3-145), and RFI's (for example, P1-0418):				
1.) ASI 128 was to remove the requirement for the corrosion analysis to be performed by a corrosion engineer from all specification sections.		1.) ASI 128 was to remove the requirement for the corrosion analysis to be performed by a corrosion engineer from all specification sections.				
2.) The Contractor is to provide a component-by-component corrosion analysis report with their own forces, rather than providing an analysis by a corrosion engineer, when a corrosion analysis is called for in the specifications.		2.) The Contractor is to provide a component-by-component corrosion analysis report with their own forces, rather than providing an analysis by a corrosion engineer, when a corrosion analysis is called for in the specifications.				
Please confirm the corrosion analysis to be performed by a corrosion engineer is required per Specification Section 07 42 13 or revise the specification to be in accordance with previous discussions.		Please confirm the corrosion analysis to be performed by a corrosion engineer is required per Specification Section 07 42 13 or revise the specification to be in accordance with previous discussions.				



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Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-2052	EXP - Expansion Joint Through W-18 at GL 20	Closed	CR	12/31/2014	01/10/2015	01/15/2015
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:			ANSWER:			
Contract Doc Ref: (ASI 128 dated 12/16/14)			Contract Doc Ref: (ASI 128 dated 12/16/14)			
Sheet A1-6005			Sheet A1-6005			
Sheet A1-8880			Sheet A1-8880			
Specification Section 07 09 13			Specification Section 07 09 13			
Location:			Location:			
Bus Deck Level			Bus Deck Level			
Closest Column Line Intersection:			Closest Column Line Intersection:			
GL 20			GL 20			
Add'l Doc Ref's: N/A			Add'l Doc Ref's: N/A			
Per Sheet A1-6005, the seismic/expansion joint at GL 20 passes through the W-18 system.			Per Sheet A1-6005, the seismic/expansion joint at GL 20 passes through the W-18 system.			
No expansion joint is called out on Sheet A1-8880 and or in Specification Section 07 09 13 for a seismic joint within a cement plaster system.			No expansion joint is called out on Sheet A1-8880 and or in Specification Section 07 09 13 for a seismic joint within a cement plaster system.			
Please provide information on the seismic joint to be installed at the W-18 system.			Please provide information on the seismic joint to be installed at the W-18 system.			



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Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-2053	BGP - W140-C23 Embedded Columns Rebar	Void	01	01/05/2015	01/15/2015	
From: Webcor Construction LP Claude Titcher						
REQUEST:			ANSWER:			
Contract Doc Ref: N/A			Contract Doc Ref: N/A			
Location: Area 16			Location: Area 16			
Grid Line: 35			Grid Line: 35			
Add'l Doc Ref's: RFI T-1372			Add'l Doc Ref's: RFI T-1372			
The area 16 East End Seismic Joint on the South Wall (W140) does not allow for the placement of all 56ea #11 vertical bars @5" OC as called out in RFI T-1372. Instead, only 48ea #11 vertical bars can be installed at 5" OC before the vertical bars interfere with the seismic joint. Note that C-23 embedded column only requires 48ea #11 vertical bars.			The area 16 East End Seismic Joint on the South Wall (W140) does not allow for the placement of all 56ea #11 vertical bars @5" OC as called out in RFI T-1372. Instead, only 48ea #11 vertical bars can be installed at 5" OC before the vertical bars interfere with the seismic joint. Note that C-23 embedded column only requires 48ea #11 vertical bars.			
Please confirm that it is acceptable to eliminate 2ea vertical bars on each face (4 faces for a total of 8 vertical bars) out at the eastern most end of Wall W140 at East Seismic Joint.			Please confirm that it is acceptable to eliminate 2ea vertical bars on each face (4 faces for a total of 8 vertical bars) out at the eastern most end of Wall W140 at East Seismic Joint.			



**ANSWER:**

Contract Doc Ref: 1/S1-3700, 2/S1-3702, 4/A1-2863, and S1-1003.  
Location: Zone 1, 2  
Grid Line: G  
Add'l Doc Ref's: Attached mark-ups

Please find attached detail 1/S1-3700, 2/S1-3702, 4/A1-2863, and S1-1003.

The elevation difference between the noted bottom of step and top of slab at Gridline G averages less than 4" per detail 1/S1 3700. The elevation difference required in detail 2/S1-3702 is nearly 1". See 4/A1-2863 markups attached noting the contract elevations provided. These small, top of slab differences will require an additional set of formwork and slow the schedule.

Note that the slab steps will be later covered in 18" of asphalt as noted in S1-1003 and the concrete steps will not be seen.

Please confirm the following is acceptable:

1. Eliminate the first step at Gridline G in Zones 1 and 2 by sloping the top of CIP slab to the elevation of the metal deck slab, as shown in attached detail 1/S1-3700.
2. Eliminate the step in 2/S1-3702 by sloping the slab to meet the elevation at the top of step. This would be applicable for all locations where this detail and similar occur.



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>T-2055</b>	<b>SCS - Cast In Place Bus Crash Rail Detail</b>	<b>Closed</b>	<b>01</b>	<b>01/06/2015</b>	<b>01/16/2015</b>	<b>01/07/2015</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Contract Doc Ref: S1-8000 ASI 127 Location: Bus Deck Grid Line: N/A Add'l Doc Ref's: N/A  Reference attached markups on Sheet S1-8000 of ASI 127  1) The CIP Bus Deck Crash Rail detail is currently shown as poured monolithic with the structural topping slab of the Bus Level composite deck. For forming reasons it isn't practical to place this wall with the same pour as the deck it sits on. Please confirm if the horizontal keyed joint for the crash rail can be relocated to the top of the structural deck as shown in the attached markups or advise how else the detail may be modified to separate vertical and horizontal elements.  2) It appears the highlighted rebar callout of #5 @ 8" OC EF (Total) is a transcription typo. Please confirm the note should read #5 continuous EF?						
						<b>ANSWER:</b> Contract Doc Ref: S1-8000 ASI 127 Location: Bus Deck Grid Line: N/A Add'l Doc Ref's: N/A  Reference attached markups on Sheet S1-8000 of ASI 127  1) The CIP Bus Deck Crash Rail detail is currently shown as poured monolithic with the structural topping slab of the Bus Level composite deck. For forming reasons it isn't practical to place this wall with the same pour as the deck it sits on. Please confirm if the horizontal keyed joint for the crash rail can be relocated to the top of the structural deck as shown in the attached markups or advise how else the detail may be modified to separate vertical and horizontal elements.  2) It appears the highlighted rebar callout of #5 @ 8" OC EF (Total) is a transcription typo. Please confirm the note should read #5 continuous EF?
<b>T-2056</b>	<b>BGP - Lower Concourse Wall Tags</b>	<b>Closed</b>	<b>01</b>	<b>01/06/2015</b>	<b>01/16/2015</b>	<b>01/08/2015</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> As discussed with the Design Team, please confirm the attached SKA's are to be incorporated showing updated CMU tags to suit future CMU wall limiting heights, on Lower Concourse Wall Plans for Zones 6 and 7, to enable slight adjustment to coupler setting out at Gridlines B-C and G-H						
						<b>ANSWER:</b> As discussed with the Design Team, please confirm the attached SKA's are to be incorporated showing updated CMU tags to suit future CMU wall limiting heights, on Lower Concourse Wall Plans for Zones 6 and 7, to enable slight adjustment to coupler setting out at Gridlines B-C and G-H
<b>T-2057</b>	<b>ELV - Missing Feeder Tag Numbers</b>	<b>Closed</b>	<b>CR</b>	<b>01/07/2015</b>	<b>01/17/2015</b>	<b>01/15/2015</b>
<b>From:</b> Webcor Construction LP      Aseem Goyal						
<b>REQUEST:</b> Contract Doc Ref: (E1-2012, E1-2103, E1-2104, E1-2105, E1-2106) dated 12/16/14						
						<b>ANSWER:</b> Contract Doc Ref: (E1-2012, E1-2103, E1-2104, E1-2105, E1-2106) dated 12/16/14



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<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
	<p>Location: N/A Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>Question : ALL clouded feeder pathways on ASI 0128 drawings E1-2102, E1-2103, E1-2104, E1-2105, &amp; E1-2106 are missing their associated feeder tag numbers. Please provide these missing feeder tag numbers.</p>					
T-2058	<p><b>SSS - Erection Issue at GL 32-32.4</b></p> <p>From: Webcor Construction LP                      Gregory Kemerer</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: S1-2607 Location: Zone 4, Roof Park Gridline: B/32.2 Add'l Doc Ref's: SK1</p> <p>Drawing S1-2607 shows a diaphragm beam being intersected by a tapered girder near gridline B/32.2.</p> <p>The small diaphragm beam highlighted in attached sketch is not erectable given the specified connections (double angle both side of tapered roof girder).</p> <p>Considering the size and location of the beam, please confirm the beam can be eliminated or supply a solution</p>	Closed	CR	01/07/2015	01/17/2015	01/17/2015
	<p><b>ANSWER:</b></p> <p>Contract Doc Ref: S1-2607 Location: Zone 4, Roof Park Gridline: B/32.2 Add'l Doc Ref's: SK1</p> <p>Drawing S1-2607 shows a diaphragm beam being intersected by a tapered girder near gridline B/32.2.</p> <p>The small diaphragm beam highlighted in attached sketch is not erectable given the specified connections (double angle both side of tapered roof girder).</p> <p>Considering the size and location of the beam, please confirm the beam can be eliminated or supply a solution</p>					



<i>Number</i>	<i>Subject</i>	<i>Status</i>	<i>Choice Required</i>	<i>Date Created</i>	<i>Date Required</i>	<i>Date Answered</i>
T-2059	SCS - DBA's at Bus Level Seismic Joint	Closed	01	01/07/2015	01/17/2015	01/16/2015
<div> <div> <b>From:</b> Webcor Construction LP           Claude Titche         </div> <div> <b>REQUEST:</b> <p>Contract Doc Ref: S1-2503, S1-5005            Location: Bus Deck Level            Grid Line: 10            Add'l Doc Ref's: N/A</p> <p>Please reference S1-5005 regarding the seismic joint detail at bus level. Drawing states "5/8" DEFORMED BAR ANCHOR (TYP AS SHOWN)@ 10" OC BETWEEN JOIST BOX". The joist box locations/dimensions and further DBA spacing/dimensions are required to detail the DBAs.</p> <ol style="list-style-type: none"> <li>What is the north-south spacing between these joist boxes?</li> <li>What are the plan dimensions for the joist boxes?</li> <li>Please confirm the 10" OC DBA spacing is in the north-south direction between joist boxes.</li> <li>What is the east-west spacing between the two DBAs shown in S1-5005?</li> <li>Are the DBAs noted above (#5 at 10" o/c between joist boxes) required in the region where 2/S 1-5005 is valid (IE: at Bus Level Seismic Joints roughly between GLD and F)?</li> <li>Detail 1/S1-5005 shows 3 of the 4 DBAs penetrating the joist box. Please confirm or revise the detail.</li> </ol> </div> </div>						
<div> <b>ANSWER:</b> <p>Contract Doc Ref: S1-2503, S1-5005            Location: Bus Deck Level            Grid Line: 10            Add'l Doc Ref's: N/A</p> <p>Please reference S1-5005 regarding the seismic joint detail at bus level. Drawing states "5/8" DEFORMED BAR ANCHOR (TYP AS SHOWN)@ 10" OC BETWEEN JOIST BOX". The joist box locations/dimensions and further DBA spacing/dimensions are required to detail the DBAs.</p> <ol style="list-style-type: none"> <li>What is the north-south spacing between these joist boxes?</li> <li>What are the plan dimensions for the joist boxes?</li> <li>Please confirm the 10" OC DBA spacing is in the north-south direction between joist boxes.</li> <li>What is the east-west spacing between the two DBAs shown in S1-5005?</li> <li>Are the DBAs noted above (#5 at 10" o/c between joist boxes) required in the region where 2/S 1-5005 is valid (IE: at Bus Level Seismic Joints roughly between GLD and F)?</li> <li>Detail 1/S1-5005 shows 3 of the 4 DBAs penetrating the joist box. Please confirm or revise the detail.</li> </ol> </div>						



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T-2060	SSS - SE201 & SE202 FO 34 Missing Information	Closed	CR	01/07/2015	01/17/2015	01/16/2015
<b>From:</b> Webcor Construction LP Gregory Kemerer						
<b>REQUEST:</b>  Contract Doc Ref: S1-2701 Location: Zone 1, Roof Park Gridline: F/5 Add'l Doc Ref's: SK1  Drawing S1-2701 shows a beam supporting elevator posts near gridline F/5. This beam was initially aligned with the W12x14 beams on either side, but was moved below the future elevator post in FO 34.  The location of the moved W16x36 beam was requested in RFI T-1922, but the dimensions were not supplied and the issue was deferred to the elevator contractor.  To avoid holding up the fabrication of the beams and supporting steel, confirm it is acceptable to locate the beam aligned with the W12's and extend the new W12x19 as shown in current model.		<b>ANSWER:</b>  Contract Doc Ref: S1-2701 Location: Zone 1, Roof Park Gridline: F/5 Add'l Doc Ref's: SK1  Drawing S1-2701 shows a beam supporting elevator posts near gridline F/5. This beam was initially aligned with the W12x14 beams on either side, but was moved below the future elevator post in FO 34.  The location of the moved W16x36 beam was requested in RFI T-1922, but the dimensions were not supplied and the issue was deferred to the elevator contractor.  To avoid holding up the fabrication of the beams and supporting steel, confirm it is acceptable to locate the beam aligned with the W12's and extend the new W12x19 as shown in current model.				
T-2061	BGP - Telecom Sweeps Conflict at GL32.4	Closed	01	01/08/2015	01/18/2015	01/16/2015
<b>From:</b> Webcor Construction LP Claude Titcher						
<b>REQUEST:</b>  Contract Doc Ref: A1-2847 Location: Lower Concourse Grid Line: 33.2 Add'l Doc Ref's: N/A  Plan Sheet AI-2847 shows two(2) groups of three(3) telecom sweeps on the South Side near GL 32.4. As shown, two sweeps in the Eastern group will conflict with the Moment Frame Beam located at GL 32.4.  Please confirm this Eastern group of Telecom Sweeps can be moved 20" further East to avoid a conflict with the GL 32.4 MFB.		<b>ANSWER:</b>  Contract Doc Ref: A1-2847 Location: Lower Concourse Grid Line: 33.2 Add'l Doc Ref's: N/A  Plan Sheet AI-2847 shows two(2) groups of three(3) telecom sweeps on the South Side near GL 32.4. As shown, two sweeps in the Eastern group will conflict with the Moment Frame Beam located at GL 32.4.  Please confirm this Eastern group of Telecom Sweeps can be moved 20" further East to avoid a conflict with the GL 32.4 MFB.				
T-2062	SSS - Light Column Cast Node Ring	Closed	CR	01/08/2015	01/18/2015	01/15/2015
<b>From:</b> Webcor Construction LP Gregory Kemerer						





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	<div>REQUEST:<div>Contract Doc Ref: S1-6005, S1-6006 Location: Zone 3 Gridline: E/23 Add'l Doc Ref's: NCR 7869-03</div><div>The internal diameter of the type 701 and 702 cast nodes varies by up to 3/8in. With this condition a tight fitting backing ring cannot be installed per AWS 5.22.1.1 which requires a fit to within 1/6in.</div><div>Please confirm a PJP with an effective size of 2in can be provided in lieu of a CJP weld (Note; a CJP weld is not specifically called out in the contract drawings, however, note on S1-6006 states that all welds are to be CJP unless noted otherwise)</div><div>Please find attached NCR generated by our light column fabricator for sketches and additional details.</div></div>					
	<div>ANSWER:<div>Contract Doc Ref: S1-6005, S1-6006 Location: Zone 3 Gridline: E/23 Add'l Doc Ref's: NCR 7869-03</div><div>The internal diameter of the type 701 and 702 cast nodes varies by up to 3/8in. With this condition a tight fitting backing ring cannot be installed per AWS 5.22.1.1 which requires a fit to within 1/6in.</div><div>Please confirm a PJP with an effective size of 2in can be provided in lieu of a CJP weld (Note; a CJP weld is not specifically called out in the contract drawings, however, note on S1-6006 states that all welds are to be CJP unless noted otherwise)</div><div>Please find attached NCR generated by our light column fabricator for sketches and additional details.</div></div>					
T-2063	SSS - Transfer Girder Stiffener Weld Clarifications	Closed	CR	01/08/2015	01/18/2015	01/12/2015
	<div>From: Webcor Construction LPGregory Kemerer</div> <div>REQUEST:<div>Contract Doc Ref: S1-4354, S1-4350 Location: Gridline: 17, 19, 25 Add'l Doc Ref's: Table 1</div><div>For TR 16.9, 19.1 and 24.9, S1-4354 shows details for stiffeners and welding at the cast nodes.</div><div>Please refer to SK1 and SK 2 and stiffener highlighted. No weld details are provided in detail 1 or 7 for this stiffener. Note 1 on detail 7 refers us to detail 2/S1-4350 which shows a 1/2" fillet for this stiffener. Please confirm 1/2" is correct for this specific stiffener on TR 16.9, 19.1 and 24.9.</div><div>Please also confirm weld details for this stiffener at all locations listed in attached Table 1.</div></div>					
	<div>ANSWER:<div>Contract Doc Ref: S1-4354, S1-4350 Location: Gridline: 17, 19, 25 Add'l Doc Ref's: Table 1</div><div>For TR 16.9, 19.1 and 24.9, S1-4354 shows details for stiffeners and welding at the cast nodes.</div><div>Please refer to SK1 and SK 2 and stiffener highlighted. No weld details are provided in detail 1 or 7 for this stiffener. Note 1 on detail 7 refers us to detail 2/S1-4350 which shows a 1/2" fillet for this stiffener. Please confirm 1/2" is correct for this specific stiffener on TR 16.9, 19.1 and 24.9.</div><div>Please also confirm weld details for this stiffener at all locations listed in attached Table 1.</div></div>					



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<b>T-2064</b>	<b>SST - Clarifications to Guardrail Landing Clearances</b>	<b>Closed</b>	<b>CR</b>	<b>01/09/2015</b>	<b>01/19/2015</b>	<b>01/15/2015</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						
<b>REQUEST:</b>  Contract Doc Ref: 6/A1-7503, A1-7010 Location: Zone 3 Gridline: C.3/20, and similar Add'l Doc Ref's: Attached sketch  Detail 6/A1-7503 provides a uniform distance for the "W" dimension from face of wall to face of guard rail.  Plan dimensions, such as those for Stair 501, provide a non-uniform guardrail clearance dimension (see attached).  Please advise.		<b>ANSWER:</b>  Contract Doc Ref: 6/A1-7503, A1-7010 Location: Zone 3 Gridline: C.3/20, and similar Add'l Doc Ref's: Attached sketch  Detail 6/A1-7503 provides a uniform distance for the "W" dimension from face of wall to face of guard rail.  Plan dimensions, such as those for Stair 501, provide a non-uniform guardrail clearance dimension (see attached).  Please advise.				
<b>T-2065</b>	<b>BGP - Buried Formsavers at Vehicle/Bicycle Ramp</b>	<b>Closed</b>	<b>01</b>	<b>01/09/2015</b>	<b>01/19/2015</b>	<b>01/15/2015</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Contract Doc Ref: N/A Location: Vehicle Bike Ramp Grid Line: N/A Add'l Doc Ref's: Attached Photo  Reference attached photo  During field walk with Sean McNeill (SEOR), SCCI pointed out a limited number of Formsavers at the Ramp Walls which had been inadvertently buried or filled with concrete during the Lower Concourse pour(see attached photo).  Please confirm per field discussion that these limited number of Form Savers may be abandoned and verticals added to ensure proper location of the coupler above.		<b>ANSWER:</b>  Contract Doc Ref: N/A Location: Vehicle Bike Ramp Grid Line: N/A Add'l Doc Ref's: Attached Photo  Reference attached photo  During field walk with Sean McNeill (SEOR), SCCI pointed out a limited number of Formsavers at the Ramp Walls which had been inadvertently buried or filled with concrete during the Lower Concourse pour(see attached photo).  Please confirm per field discussion that these limited number of Form Savers may be abandoned and verticals added to ensure proper location of the coupler above.				
<b>T-2066</b>	<b>SSS - Erection Issues at GL 33.2</b>	<b>Closed</b>	<b>CR</b>	<b>01/09/2015</b>	<b>01/19/2015</b>	<b>01/22/2015</b>
<b>From:</b> Webcor Construction LP                      Gregory Kemerer						



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	<div><div>REQUEST:</div><div>Contract Doc Ref: S1-2607, 2/S1-4205 Location: Roof Level Zone 4 Gridline: B&amp;H/33.2 Add'l Doc Ref's: Attached sketch</div><div>Drawing S1-2607 shows a W40x503 beam on GL 33.2 connecting to the perimeter beams at GL B &amp; H. These perimeter beams have stiffeners per 2/S1-4205.</div><div>With the perimeter beam stiffeners shop welded, the W40x503 will not be erectable.</div><div>Please confirm it is acceptable to ship the stiffener loose, to be field welded as shown in the attached sketch, or supply an alternate solution.</div></div>					
	<div><div>ANSWER:</div><div>Contract Doc Ref: S1-2607, 2/S1-4205 Location: Roof Level Zone 4 Gridline: B&amp;H/33.2 Add'l Doc Ref's: Attached sketch</div><div>Drawing S1-2607 shows a W40x503 beam on GL 33.2 connecting to the perimeter beams at GL B &amp; H. These perimeter beams have stiffeners per 2/S1-4205.</div><div>With the perimeter beam stiffeners shop welded, the W40x503 will not be erectable.</div><div>Please confirm it is acceptable to ship the stiffener loose, to be field welded as shown in the attached sketch, or supply an alternate solution.</div></div>					
T-2067	SSS - SLRS Bolt Up Connection Clarification GL 27.1-28	Closed	CR	01/09/2015	01/19/2015	01/23/2015
	<div>From: Webcor Construction LP</div> <div>Gregory Kemerer</div> <div><div>REQUEST:</div><div>Contract Doc Ref: S1-2506, 1C/S1-5018 Location: Bus Deck, Zone 4 Gridline: B/27.1 &amp; H/27.1 Add'l Doc Ref's: RFI T-1815, attached sketch</div><div>Drawing S1-2506 shows beam connections near B/27.1 &amp; H/27.1. Per 1C/S1-5018, these connections should have 10 bolts in each row.</div><div>Due to the cut of the beam bottom flange it is not possible to fit the required 10 bolts in each row.</div><div>Confirm it is acceptable to supply a total of 15 bolts at the bottom flange as shown similar to RFI T-1815 (SK 878, CD 653).</div></div>					
	<div><div>ANSWER:</div><div>Contract Doc Ref: S1-2506, 1C/S1-5018 Location: Bus Deck, Zone 4 Gridline: B/27.1 &amp; H/27.1 Add'l Doc Ref's: RFI T-1815, attached sketch</div><div>Drawing S1-2506 shows beam connections near B/27.1 &amp; H/27.1. Per 1C/S1-5018, these connections should have 10 bolts in each row.</div><div>Due to the cut of the beam bottom flange it is not possible to fit the required 10 bolts in each row.</div><div>Confirm it is acceptable to supply a total of 15 bolts at the bottom flange as shown similar to RFI T-1815 (SK 878, CD 653).</div></div>					



<b>T-2070</b>	<b>BOL - Design Build Bollards on Roof - Layout</b>	<b>Pending</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>
<b>From:</b> Webcor Construction LP		Sihaya Roselle			



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	<div><div>REQUEST:</div><div>Contract Doc Ref: ASI 128 - L1-2602, L1-2604, L1-2605, L1-2606 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</div><div>Design Build Bollards have been added to the Roof Park in ASI 128 on sheets L1-2602, L1-2604, L1-2605 and L1-2606 at GL 5.7/G, GL 15.5/F, GL 21/G and GL 31/E.1.</div><div>Performance requirements have been indicated on these sheets for the bollards, however there are no details providing layout, finishes, footings and their attachment to structural or protection concrete slabs.</div><div>Please provide dimensions and define the layout of the design build bollards on the roof, especially near structures, walkways, plantings, and the future roof park cafe.</div></div>					
T-2071	<div><div>BOL - Design Build Bollards on Roof - Layout</div><div>From: Webcor Construction LP      Sihaya Roselle</div></div>	Closed	CR	01/13/2015	01/13/2015	02/09/2015
	<div><div>REQUEST:</div><div>Contract Doc Ref: ASI 128 - L1-2602, L1-2604, L1-2605, L1-2606 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</div><div>Design Build Bollards have been added to the Roof Park in ASI 128 on sheets L1-2602, L1-2604, L1-2605 and L1-2606 at GL 5.7/G, GL 15.5/F, GL 21/G and GL 31/E.1.</div><div>Performance requirements have been indicated on these sheets for the bollards, however there are no details providing layout, finishes, footings and their attachment to structural or protection concrete slabs.</div><div>Please provide dimensions and define the layout of the design build bollards on the roof, especially near structures, walkways, plantings, and the future roof park cafe.</div></div>					
	<div><div>ANSWER:</div><div>Contract Doc Ref: ASI 128 - L1-2602, L1-2604, L1-2605, L1-2606 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</div><div>Design Build Bollards have been added to the Roof Park in ASI 128 on sheets L1-2602, L1-2604, L1-2605 and L1-2606 at GL 5.7/G, GL 15.5/F, GL 21/G and GL 31/E.1.</div><div>Performance requirements have been indicated on these sheets for the bollards, however there are no details providing layout, finishes, footings and their attachment to structural or protection concrete slabs.</div><div>Please provide dimensions and define the layout of the design build bollards on the roof, especially near structures, walkways, plantings, and the future roof park cafe.</div></div>					



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T-2072	<b>BGP - Partition Wall G140 Misaligned Dowels</b>  From: Webcor Construction LP                      Claude Titche  <b>REQUEST:</b> Contract Doc Ref: N/A Location: Mat Slab Grid Line: 3.5/C.5 Add'l Doc Ref's: Attached  Please see the attached sketch for the proposed layout of the misaligned layer of rebar dowels on partition wall G140. Please confirm it is acceptable to cut the misaligned layer of dowels, abandon, and drill/epoxy new rebar dowels as long as proper clearance is maintained per the attached approved CAP TG0600-917.1.	Closed	01	01/13/2015	01/23/2015	01/20/2015
			park cafe.			
			<b>ANSWER:</b> Contract Doc Ref: N/A Location: Mat Slab Grid Line: 3.5/C.5 Add'l Doc Ref's: Attached  Please see the attached sketch for the proposed layout of the misaligned layer of rebar dowels on partition wall G140. Please confirm it is acceptable to cut the misaligned layer of dowels, abandon, and drill/epoxy new rebar dowels as long as proper clearance is maintained per the attached approved CAP TG0600-917.1.			

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
T-2073	BOL -Design Build Bollards on Roof - Finishes	Closed	CR	01/13/2015	01/13/2015	02/05/2015
<div><div>From: Webcor Construction LP</div><div>Sihaya Roselle</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<div>Contract Doc Ref: ASI 128 - L1-2602, L1-2604, L1-2605, L1-2606</div> <div>Location: Roof Park</div> <div>Closest Column Line Intersection: N/A</div> <div>Add'l Doc Ref's: N/A</div>			<div>Contract Doc Ref: ASI 128 - L1-2602, L1-2604, L1-2605, L1-2606</div> <div>Location: Roof Park</div> <div>Closest Column Line Intersection: N/A</div> <div>Add'l Doc Ref's: N/A</div>			
<div>Design Build Bollards have been added to the Roof Park in ASI 128 on sheets L1-2602, L1-2604, L1-2605 and L1-2606 at GL 5.7/G, GL 15.5/F, GL 21/G and GL 31/E.1.</div>			<div>Design Build Bollards have been added to the Roof Park in ASI 128 on sheets L1-2602, L1-2604, L1-2605 and L1-2606 at GL 5.7/G, GL 15.5/F, GL 21/G and GL 31/E.1.</div>			
<div>Performance requirements have been indicated on these sheets for the bollards, however there are no details providing layout, finishes, footings and their attachment to structural or protection concrete slabs.</div>			<div>Performance requirements have been indicated on these sheets for the bollards, however there are no details providing layout, finishes, footings and their attachment to structural or protection concrete slabs.</div>			
<div>Please provide details and define the finishes of the design build bollards on the roof.</div>			<div>Please provide details and define the finishes of the design build bollards on the roof.</div>			
T-2074	BOL - Design Build Bollards on Roof - Footings	Closed	CR	01/13/2015	01/23/2015	02/05/2015
<div><div>From: Webcor Construction LP</div><div>Sihaya Roselle</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<div>Contract Doc Ref: ASI 128 - L1-2602, L1-2604, L1-2605, L1-2606</div> <div>Location: Roof Park</div> <div>Closest Column Line Intersection: N/A</div> <div>Add'l Doc Ref's: N/A</div>			<div>Contract Doc Ref: ASI 128 - L1-2602, L1-2604, L1-2605, L1-2606</div> <div>Location: Roof Park</div> <div>Closest Column Line Intersection: N/A</div> <div>Add'l Doc Ref's: N/A</div>			
<div>Design Build Bollards have been added to the Roof Park in ASI 128 on sheets L1-2602, L1-2604, L1-2605 and L1-2606 at GL 5.7/G, GL 15.5/F, GL 21/G and GL 31/E.1.</div>			<div>Design Build Bollards have been added to the Roof Park in ASI 128 on sheets L1-2602, L1-2604, L1-2605 and L1-2606 at GL 5.7/G, GL 15.5/F, GL 21/G and GL 31/E.1.</div>			
<div>Performance requirements have been indicated on these sheets for the bollards, however there are no details providing layout, finishes, footings and their attachment to structural or protection concrete slabs.</div>			<div>Performance requirements have been indicated on these sheets for the bollards, however there are no details providing layout, finishes, footings and their attachment to structural or protection concrete slabs.</div>			
<div>Please provide footing details and their attachment to structural or protection concrete slabs for the design build bollards on the roof.</div>			<div>Please provide footing details and their attachment to structural or protection concrete slabs for the design build bollards on the roof.</div>			



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<b>T-2075</b>	<b>BOL - Reference to Interim Location of HPU</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>02/05/2015</b>
<b>From:</b> Webcor Construction LP                      Sihaya Roselle						
<b>REQUEST:</b>  Contract Doc Ref: ASI 128 - A1-2206 Location: Lower Concourse Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  A1-2206 at GL 27.7/H on the Lower Concourse callout states that the location provided is an interim location of the HPU for wedge barriers at ground level. Based on a previous answer to RFI P1-0011, any reference to interim was meant to be removed.  Confirm that this is not an interim location or if it is in fact an interim location, provide final location details for the HPUs currently located at GL 27.7/H on the Lower Concourse.						<b>ANSWER:</b>  Contract Doc Ref: ASI 128 - A1-2206 Location: Lower Concourse Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  A1-2206 at GL 27.7/H on the Lower Concourse callout states that the location provided is an interim location of the HPU for wedge barriers at ground level. Based on a previous answer to RFI P1-0011, any reference to interim was meant to be removed.  Confirm that this is not an interim location or if it is in fact an interim location, provide final location details for the HPUs currently located at GL 27.7/H on the Lower Concourse.
<b>T-2076</b>	<b>RPL - Tree on Café Lid</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>01/28/2015</b>
<b>From:</b> Webcor Construction LP                      Sihaya Roselle						
<b>REQUEST:</b>  Contract Doc Ref: ASI 128 - L1-2605 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  A tree has been added to the roof park at GL 21/G, on top of the stem walls and lid of the future Roof Park Café. In previous meetings regarding temporary conditions for the W-20 Roof Park Café it was agreed that trees should not be planted on the location of future expansion.  Please confirm that there is enough space for the root ball at GL 21/G or remove the tree and update/coordinate the drawings.						<b>ANSWER:</b>  Contract Doc Ref: ASI 128 - L1-2605 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  A tree has been added to the roof park at GL 21/G, on top of the stem walls and lid of the future Roof Park Café. In previous meetings regarding temporary conditions for the W-20 Roof Park Café it was agreed that trees should not be planted on the location of future expansion.  Please confirm that there is enough space for the root ball at GL 21/G or remove the tree and update/coordinate the drawings.
<b>T-2077</b>	<b>RPL - Missing Specification 06 15 35 Site Carpentry in ASI 128</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>01/28/2015</b>
<b>From:</b> Webcor Construction LP                      Sihaya Roselle						
<b>REQUEST:</b>  Contract Doc Ref: ASI 128 - 00 01 10 Table of Contents,						<b>ANSWER:</b>  Contract Doc Ref: ASI 128 - 00 01 10 Table of





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	<p>06 15 35 Site Carpentry, 12 93 00 Site Furniture Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>In ASI 128 the 01 01 10 Table of Contents does not reissue or delete specification 06 15 35 Site Carpentry and it was not included with the ASI 128 issuance of the conformed set. However, specification 12 93 00 Site Furniture references specification 06 15 35 Site Carpentry.</p> <p>Please provide specification 06 15 35 Site Carpentry or revise the reference to it within specification 12 93 00 Site Furniture.</p>					
						<p>Contents, 06 15 35 Site Carpentry, 12 93 00 Site Furniture Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>In ASI 128 the 01 01 10 Table of Contents does not reissue or delete specification 06 15 35 Site Carpentry and it was not included with the ASI 128 issuance of the conformed set. However, specification 12 93 00 Site Furniture references specification 06 15 35 Site Carpentry.</p> <p>Please provide specification 06 15 35 Site Carpentry or revise the reference to it within specification 12 93 00 Site Furniture.</p>
T-2078	RPL - Roof Park Reference to Alternate 16	Closed	CR	01/13/2015	01/13/2015	01/28/2015
	<p>From: Webcor Construction LP                      Sihaya Roselle</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: ASI 128 - L1-5627, 01 10 30/APE - Schedule of Alternates for Main Package Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>In ASI 128 specification 01 10 30/APE Schedule of Alternates for Main Package, all alternates for the Roof Park have been removed. L1-5627 still references Alternate No. 16 regarding deletion of the greywater system.</p> <p>Please confirm that Alternate 16 is no longer applicable and reference to it should be removed from L1-5627.</p>					<p><b>ANSWER:</b></p> <p>Contract Doc Ref: ASI 128 - L1-5627, 01 10 30/APE - Schedule of Alternates for Main Package Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>In ASI 128 specification 01 10 30/APE Schedule of Alternates for Main Package, all alternates for the Roof Park have been removed. L1-5627 still references Alternate No. 16 regarding deletion of the greywater system.</p> <p>Please confirm that Alternate 16 is no longer applicable and reference to it should be removed from L1-5627.</p>



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<b>T-2079</b>	<b>RPL - Reference to Roof Park Café Area of Alternate Deduct</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>01/28/2015</b>
<b>From:</b> Webcor Construction LP                      Sihaya Roselle						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: ASI 128 - L1-2633, 01 10 30/APE - Schedule of Alternates for Main Package Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A						Contract Doc Ref: ASI 128 - L1-2633, 01 10 30/APE - Schedule of Alternates for Main Package Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A
In ASI 128 specification 01 10 30/APE Schedule of Alternates for Main Package, all alternates for the Roof Park have been removed. L1-2633 still references and calls out an Area of Alternate Deduct at the W-20 Roof Park Cafe.						In ASI 128 specification 01 10 30/APE Schedule of Alternates for Main Package, all alternates for the Roof Park have been removed. L1-2633 still references and calls out an Area of Alternate Deduct at the W-20 Roof Park Cafe.
Please confirm that reference to this Area of Alternate Deduct at the W-20 Roof Park Cafe is no longer applicable and reference to it should be removed from L1-2633.						Please confirm that reference to this Area of Alternate Deduct at the W-20 Roof Park Cafe is no longer applicable and reference to it should be removed from L1-2633.
<b>T-2080</b>	<b>RPL - Play Structure Anchor Assembly Footing Callout</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>01/28/2015</b>
<b>From:</b> Webcor Construction LP                      Sihaya Roselle						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: ASI 128 - L1-5604 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A						Contract Doc Ref: ASI 128 - L1-5604 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A
L1-5604 callout for the Play Structure Anchor Assembly Footing references 1/L1-7665. This reference appears to be incorrect because detail 1/L1-7665 is for the Precast Concrete Roof Drain, however detail 2/L1-7647 shows the Play Structure Anchor Assembly Footing.						L1-5604 callout for the Play Structure Anchor Assembly Footing references 1/L1-7665. This reference appears to be incorrect because detail 1/L1- 7665 is for the Precast Concrete Roof Drain, however detail 2/L1-7647 shows the Play Structure Anchor Assembly Footing.
Please confirm that 2/L1-7647 is the correct detail reference for the callout on L1-5604 for the Play structure Anchor Assembly Footing.						Please confirm that 2/L1-7647 is the correct detail reference for the callout on L1-5604 for the Play structure Anchor Assembly Footing.
<b>T-2081</b>	<b>SCS - Play Structure Anchor Assembly Footing Connection</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>01/28/2015</b>
<b>From:</b> Webcor Construction LP                      Sihaya Roselle						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	<p>Contract Doc Ref: ASI 128 - L1-2639, 2/L1-7647, 8/S1-3281 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>2/L1-7647 shows the Play Structure Anchor Assembly Footing as part of the protection slab on the roof. 8/S1-3281 shows the Play Structure Footing as part of the structural slab on the Roof Park level. L1-2639 states that the Play Structure Footing layout is according to the play structure installers written instructions.</p> <p>Please clarify if the Play Structure Anchor Assembly Footing is part of the protection slab, as shown in 2/L1-7647 or if it is part of the structural slab as shown in 8/S1-3281. If it is part of the structural slab, provide the appropriate waterproofing details.</p>					<p>Contract Doc Ref: ASI 128 - L1-2639, 2/L1-7647, 8/S1-3281 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>2/L1-7647 shows the Play Structure Anchor Assembly Footing as part of the protection slab on the roof. 8/S1-3281 shows the Play Structure Footing as part of the structural slab on the Roof Park level. L1-2639 states that the Play Structure Footing layout is according to the play structure installers written instructions.</p> <p>Please clarify if the Play Structure Anchor Assembly Footing is part of the protection slab, as shown in 2/L1-7647 or if it is part of the structural slab as shown in 8/S1-3281. If it is part of the structural slab, provide the appropriate waterproofing details.</p>
T-2082	SCS - Play Structure Anchor Assembly Footing at the Shear Key	Closed	CR	01/13/2015	01/23/2015	01/23/2015
	From: Webcor Construction LP Sihaya Roselle					
	REQUEST: <p>Contract Doc Ref: ASI 128 - L1-2624, L1-2639, 2/L1-7647, 8/S1-3281 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>2/L1-7647 shows the Play Structure Anchor Assembly Footing as part of the protection slab on the roof. 8/S1-3281 shows the Play Structure Footing as part of the structural slab on the Roof Park level. L1-2624 shows Play Structure Footings at GL 15/G on a Shear Key. L1-2639 states to SSD for the Play Structure Post Footing at Shear Key, but the only detail in the Structural Drawings for the Play Structure Footing is the typical detail 8/L1-3281 which does not show the footing at the shear key.</p> <p>Please provide details for the Play Structure Anchor Assembly Footings at the Shear Key located at GL 15/G.</p>					ANSWER: <p>Contract Doc Ref: ASI 128 - L1-2624, L1-2639, 2/L1-7647, 8/S1-3281 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>2/L1-7647 shows the Play Structure Anchor Assembly Footing as part of the protection slab on the roof. 8/S1-3281 shows the Play Structure Footing as part of the structural slab on the Roof Park level. L1-2624 shows Play Structure Footings at GL 15/G on a Shear Key. L1-2639 states to SSD for the Play Structure Post Footing at Shear Key, but the only detail in the Structural Drawings for the Play Structure Footing is the typical detail 8/L1-3281 which does not show the footing at the shear key.</p> <p>Please provide details for the Play Structure Anchor Assembly Footings at the Shear Key located at GL</p>



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			15/G.			
<b>T-2083</b>	<b>RPL - Vine Planting in Doorway</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>01/28/2015</b>
<b>From:</b> Webcor Construction LP      Sihaya Roselle						
<b>REQUEST:</b> Contract Doc Ref: ASI 128 - L1-6606 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  L1-6606 shows a vine planting in front of the doorway at GL 25.5/E.2, outside of the planter.  Confirm that there should be a vine outside of the planter in front of the doorway at GL 25.5/E.2 and define how it should be planted, or remove the vine and update/coordinate the drawings.			<b>ANSWER:</b> Contract Doc Ref: ASI 128 - L1-6606 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  L1-6606 shows a vine planting in front of the doorway at GL 25.5/E.2, outside of the planter.  Confirm that there should be a vine outside of the planter in front of the doorway at GL 25.5/E.2 and define how it should be planted, or remove the vine and update/coordinate the drawings.			
<b>T-2084</b>	<b>RPL - Connecting Bridge at Roof Park Bus Fountain</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>01/15/2015</b>
<b>From:</b> Webcor Construction LP      Sihaya Roselle						
<b>REQUEST:</b> Contract Doc Ref: ASI - 128 - L1-2605 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  L1-2605 states that Structural Resolution is Pending at the Bridge on GL 21/B which are located within the Bus Fountain at the Roof Park.  Confirm that the structural resolution that is still pending will not impact any of the elements of the Bus Fountain and if so define what those impacts will be.			<b>ANSWER:</b> Contract Doc Ref: ASI - 128 - L1-2605 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  L1-2605 states that Structural Resolution is Pending at the Bridge on GL 21/B which are located within the Bus Fountain at the Roof Park.  Confirm that the structural resolution that is still pending will not impact any of the elements of the Bus Fountain and if so define what those impacts will be.			



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<b>T-2085</b>	<b>SSS - W-20 Uncoordinated Drawings</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>01/22/2015</b>
<b>From:</b> Webcor Construction LP                      Sihaya Roselle						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: ASI 128 - S1-2650, S1-6100, S1-6101, S1-6102 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A						Contract Doc Ref: ASI 128 - S1-2650, S1-6100, S1-6101, S1-6102 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A
W-20 is not coordinated in the Structural, Landscape and Architectural drawings. The Architectural and Landscape drawings show the W-20 stem walls with a concrete lid over the top so that the W-20 may be constructed in the future. The Structural drawings show two conflicting conditions for the Roof Park Cafe. 1/S1-2650 shows the stem walls while sheets S1-6100, S1-6101, S1-6102 show the final construction of the W-20 Roof Park Cafe in its entirety.						W-20 is not coordinated in the Structural, Landscape and Architectural drawings. The Architectural and Landscape drawings show the W-20 stem walls with a concrete lid over the top so that the W-20 may be constructed in the future. The Structural drawings show two conflicting conditions for the Roof Park Cafe. 1/S1-2650 shows the stem walls while sheets S1-6100, S1-6101, S1-6102 show the final construction of the W-20 Roof Park Cafe in its entirety.
Please clarify if the W-20 is meant to be constructed in its entirety or only the stem walls and concrete lid. Revise and coordinate the drawings with the required direction and define which drawings govern this scope.						Please clarify if the W-20 is meant to be constructed in its entirety or only the stem walls and concrete lid. Revise and coordinate the drawings with the required direction and define which drawings govern this scope.
<b>T-2086</b>	<b>SCS - Park Rail Uncoordinated Drawings</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>02/06/2015</b>
<b>From:</b> Webcor Construction LP                      Sihaya Roselle						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: ASI 128 - 1/S1-3282, A1-8685 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A						Contract Doc Ref: ASI 128 - 1/S1-3282, A1-8685 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A
Perimeter Guard Rail at Park Level is not coordinated in Structural and Architectural drawings. 1/S1-3282 is showing a different Perimeter Guard Rail at the Park Level than A1-8685.						Perimeter Guard Rail at Park Level is not coordinated in Structural and Architectural drawings. 1/S1-3282 is showing a different Perimeter Guard Rail at the Park Level than A1-8685.
Please clarify which detail is correct and revise/coordinate the drawings.						Please clarify which detail is correct and revise/coordinate the drawings.



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<b>T-2087</b>	<b>RPL - Fountain Drawings Removed in ASI 128</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>02/04/2015</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Sihaya Roselle</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - L-0000 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - L-0000 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
All fountain drawings (L1-7000 through L1-7071), except for hardscape, have been removed from the Landscape drawings. However, their removal is not indicated in L-0000.			All fountain drawings (L1-7000 through L1-7071), except for hardscape, have been removed from the Landscape drawings. However, their removal is not indicated in L-0000.			
Confirm that the fountain drawings were meant to be excluded from the newly conformed ASI 128 and that their removal will not impact the fountain hardscape details, or provide the fountain drawings.			Confirm that the fountain drawings were meant to be excluded from the newly conformed ASI 128 and that their removal will not impact the fountain hardscape details, or provide the fountain drawings.			
<b>T-2088</b>	<b>SCS - Drill Into Structural Concrete at Seismic Joint for Bus Jet Fountain</b>	<b>Closed</b>	<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>02/03/2015</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Sihaya Roselle</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - L1-7634 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - L1-7634 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
L1-7634 shows support angle bolted into the structural concrete for the glass shoe of the Bus Jet Fountain at the Seismic Joint.			L1-7634 shows support angle bolted into the structural concrete for the glass shoe of the Bus Jet Fountain at the Seismic Joint.			
Confirm that these embeds may be drilled into the structural concrete of the seismic joint wall.			Confirm that these embeds may be drilled into the structural concrete of the seismic joint wall.			
<b>T-2089</b>	<b>SCS - Drill Into Structural Concrete at Seismic Joint for Bus Jet Fountain and Plan Closed</b>		<b>CR</b>	<b>01/13/2015</b>	<b>01/23/2015</b>	<b>02/03/2015</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Sihaya Roselle</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - L1-7635 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - L1-7635 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
L1-7635 shows support angle bolted into the structural			L1-7635 shows support angle bolted into the structural			



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T-2090	<p>concrete for the glass shoe, gaskets and plant rail at the Seismic Joint.</p> <p>Confirm that these embeds may be drilled into the structural concrete of the seismic joint wall.</p> <p><b>RPL - Polystyrene Fill on Roof</b></p> <p><b>From:</b> Webcor Construction LP                      Sihaya Roselle</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: ASI 128 - L1-8623, L1-8624 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>Details 1, 2 and 3 on L1-8623 and 1/L1-8624 call out polystyrene fill.</p> <p>Confirm that polystyrene fill called out is meant to be geosynthetic fill and revise accordingly. If it is not meant to be geosynthetic fill please provide specification requirements for polystyrene fill.</p>	Closed	CR	01/14/2015	01/24/2015	01/28/2015
						<p><b>ANSWER:</b></p> <p>Contract Doc Ref: ASI 128 - L1-8623, L1-8624 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>Details 1, 2 and 3 on L1-8623 and 1/L1-8624 call out polystyrene fill.</p> <p>Confirm that polystyrene fill called out is meant to be geosynthetic fill and revise accordingly. If it is not meant to be geosynthetic fill please provide specification requirements for polystyrene fill.</p>



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T-2091	TOP - Location of Protection Slabs	Closed	CR	01/14/2015	01/24/2015	01/16/2015
<div>From: Webcor Construction LPTram Nguyen</div> <div>REQUEST: Contract Doc Ref: (ASI 128 dated 12/16/14) Sheets A1-2912 through A1-2931  Location: Roof Level Ground Level  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Per the Protection Legend on sheets A1-2912 through A1-2931, the hatch pattern indicates areas that do not require protection slab.  The Protection Legend on Sheets A1-2912 through A1-2931 does not indicate which areas do require topping Slab.  Please confirm that the areas that are not hatched require protection slab, or provide the areas that require protection slab, and revise the legend and plans to match.</div>		<div>ANSWER: Contract Doc Ref: (ASI 128 dated 12/16/14) Sheets A1-2912 through A1-2931  Location: Roof Level Ground Level  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Per the Protection Legend on sheets A1-2912 through A1-2931, the hatch pattern indicates areas that do not require protection slab.  The Protection Legend on Sheets A1-2912 through A1-2931 does not indicate which areas do require topping Slab.  Please confirm that the areas that are not hatched require protection slab, or provide the areas that require protection slab, and revise the legend and plans to match.</div>				
T-2092	BGP - Chord Bars at Vehicle and Bike Ramp	Closed	01	01/15/2015	01/25/2015	01/19/2015
<div>From: Webcor Construction LPClaude Titcher</div> <div>REQUEST: Contract Doc Ref: S1-2251 Location: Bike Ramp Z1 Grid Line: N/A Add'l Doc Ref's: Attached.  Please confirm it is the Engineer and Designer intent to install the Chord Bars at the Vehicle and Bike Ramp per the attached TT sketch and NOT per the Contract Drawings or Shop Drawings(Submittal TG0600-331.1) which were returned "Approved As Noted."</div>		<div>ANSWER: Contract Doc Ref: S1-2251 Location: Bike Ramp Z1 Grid Line: N/A Add'l Doc Ref's: Attached.  Please confirm it is the Engineer and Designer intent to install the Chord Bars at the Vehicle and Bike Ramp per the attached TT sketch and NOT per the Contract Drawings or Shop Drawings(Submittal TG0600-331.1) which were returned "Approved As Noted."</div>				
T-2093	ELV -Park Level - Tag Device HP-4-A-1 Duplication	Closed	CR	01/15/2015	01/25/2015	01/20/2015





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	<p><b>From:</b> Webcor Construction LP                      Aseem Goyal</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: (ASI 128/ E1-2602/Spec: 26 28 16) Location: Zone 02, Park Level Column Line: (D,5), (D.8,5) The attached drawing shows a motor in two separate locations with the same feeder at each location. Please confirm both the motors are in contract, if yes provide an updated schedule with appropriate designation and feeder circuit.</p>					<p><b>ANSWER:</b></p> <p>Contract Doc Ref: (ASI 128/ E1-2602/Spec: 26 28 16) Location: Zone 02, Park Level Column Line: (D,5), (D.8,5) The attached drawing shows a motor in two separate locations with the same feeder at each location. Please confirm both the motors are in contract, if yes provide an updated schedule with appropriate designation and feeder circuit.</p>
T-2094	SCS - Welded Bar Material Specification	Closed	01	01/16/2015	01/26/2015	01/21/2015
	<p><b>From:</b> Webcor Construction LP                      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: S-0007, 03 30 02 2.1A.1, 03 20 02 2.3G Location: N/A Grid Line: N/A Add'l Doc Ref's: TG0702-514, Attached.</p> <p>Please reference submittal #TG0702-514, Specification 03 30 02 2.1.A.1, Specification 03 20 02 2.3.G, and General Notes on S-0007.</p> <p>The requirements for these welded bars conflict. ASTM A 706 is required per General Notes and Specification 03 30 02 2.1.A.I, but ASTM A108 is required per Specification 03 20 02 2.3.G and submittal response to #TG0702-514. Each specification section references the same items.</p> <p>All welded bars, including those called out as Deformed Bar Anchors, will be fabricated per ASTM A496, which surpasses both A108 and A 706 requirements set by the specifications above. Please see attached letter from manufacturer.</p> <p>Please confirm that providing bars per ASTM A496 in lieu of either A706 or A108 is acceptable.</p>					<p><b>ANSWER:</b></p> <p>Contract Doc Ref: S-0007, 03 30 02 2.1A.1, 03 20 02 2.3G Location: N/A Grid Line: N/A Add'l Doc Ref's: TG0702-514, Attached.</p> <p>Please reference submittal #TG0702-514, Specification 03 30 02 2.1.A.1, Specification 03 20 02 2.3.G, and General Notes on S-0007.</p> <p>The requirements for these welded bars conflict. ASTM A 706 is required per General Notes and Specification 03 30 02 2.1.A.I, but ASTM A108 is required per Specification 03 20 02 2.3.G and submittal response to #TG0702-514. Each specification section references the same items.</p> <p>All welded bars, including those called out as Deformed Bar Anchors, will be fabricated per ASTM A496, which surpasses both A108 and A 706 requirements set by the specifications above. Please see attached letter from manufacturer.</p> <p>Please confirm that providing bars per ASTM A496 in lieu of either A706 or A108 is acceptable.</p>
T-2095	SCS - Rebar Detail at MFB/Drum Beam Connection GL C and G	Closed	01	01/16/2015	01/26/2015	01/19/2015



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<hr/>						
<b>From:</b> Webcor Construction LP		Claude Titché				
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: S1-5022 Location: Ground Level Grid Line: C and G Add'l Doc Ref's: Attached			Contract Doc Ref: S1-5022 Location: Ground Level Grid Line: C and G Add'l Doc Ref's: Attached			
Please reference Sheet S 1-5022 showing the typical steel to concrete connection at the ground level. Note the 1:1 slope from bottom of beam to bottom of flange in Detail 1B and 4A. Insufficient rebar details are available in this 1:1 slope in SI-5022. The details direct to the SI-37XX series for further rebar details. Sections are taken at various GLs at N/S Transfer Girders, but not between GLs at the E/W perimeter steel. The bottom bar embedment varies: 90 degree, 45 degree, and terminator ends are all used.			Please reference Sheet S 1-5022 showing the typical steel to concrete connection at the ground level. Note the 1:1 slope from bottom of beam to bottom of flange in Detail 1B and 4A. Insufficient rebar details are available in this 1:1 slope in SI-5022. The details direct to the SI-37XX series for further rebar details. Sections are taken at various GLs at N/S Transfer Girders, but not between GLs at the E/W perimeter steel. The bottom bar embedment varies: 90 degree, 45 degree, and terminator ends are all used.			
Shimmick requires further rebar details in the 1:1 sloped region of the MFBs between GLs at the E/W perimeter steel along GL C and G to produce shop drawings. This RFI affects Submittal TG0702-076 for Ground Level 13-19 Rebar and has potential changes for Submittal TG0702-075 for Ground Level 10-13.			Shimmick requires further rebar details in the 1:1 sloped region of the MFBs between GLs at the E/W perimeter steel along GL C and G to produce shop drawings. This RFI affects Submittal TG0702-076 for Ground Level 13-19 Rebar and has potential changes for Submittal TG0702-075 for Ground Level 10-13.			
1. Please provide a new rebar detail applicable to all MFB between grid lines, one each for GL C and GL G. Please include bent bottom bar requirements and stirrup requirements as applicable.			1. Please provide a new rebar detail applicable to all MFB between grid lines, one each for GL C and GL G. Please include bent bottom bar requirements and stirrup requirements as applicable.			
2. Are bent bars, similar to those shown in the SI-37XX markups, required in the 1:1 sloped region of the MFBs between TRs along GL C and G?			2. Are bent bars, similar to those shown in the SI-37XX markups, required in the 1:1 sloped region of the MFBs between TRs along GL C and G?			
3. If bent bars are required per #2, please provide criteria on when to apply each of the following bottom bar end conditions: 90 degree, 45 degree, and terminator ends. Shimmick would propose the criteria as the size of the 1:1 slope region.			3. If bent bars are required per #2, please provide criteria on when to apply each of the following bottom bar end conditions: 90 degree, 45 degree, and terminator ends. Shimmick would propose the criteria as the size of the 1:1 slope region.			
4. If bent bars are required per #2, Shimmick proposes that the bottom bent bars are added with a splice to facilitate placing the bar. Propose (a) splice length per splice schedule on S1-3001, to extend from start of 1:1 toward foundation wall, (b) to reduce bent bar size from matching MFB to a #5, (c) splice length per larger bar size.			4. If bent bars are required per #2, Shimmick proposes that the bottom bent bars are added with a splice to facilitate placing the bar. Propose (a) splice length per splice schedule on S1-3001, to extend from start of 1:1 toward foundation wall, (b) to reduce bent bar size from matching MFB to a #5, (c) splice length per larger bar size.			
5. Please provide a distance from the GL G and C at						



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	<p>which to stop stirrups. Alternatively, please confirm it is acceptable for stirrups to end when the MFB 1:1 slope begins.</p> <p>6. Does the 1:1 slope occur at the built up drag beams along GL C and G? Does the bottom of beam (a) slope to middle flange or (b) run straight into the steel web?</p>					<p>5. Please provide a distance from the GL G and C at which to stop stirrups. Alternatively, please confirm it is acceptable for stirrups to end when the MFB 1:1 slope begins.</p> <p>6. Does the 1:1 slope occur at the built up drag beams along GL C and G? Does the bottom of beam (a) slope to middle flange or (b) run straight into the steel web?</p>
T-2096	ELV - Confirmation on Company Switch at Park Level	Closed	CR	01/16/2015	01/26/2015	01/20/2015
	<p>From: Webcor Construction LP                      Aseem Goyal</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: (ASI 128/E-0005, E1-2602, E1-2606, E1-5001, E1-5003)</p> <p>Location: (Zone 02, Park Level), (Zone 06, Park Level)</p> <p>Closest Column: (E1-2602/D.4,4), (E1-2606/D.8,29), (E1-5001/Park), (E1-5003/Park)</p> <p>Per ASI 118, the park level company switches were deleted in zoned 02 and 06. Per ASI 128 electrical plan drawings E1-2602 &amp; E1-2606 the company switches were added back but they are still shown as "NIC" on E1-5001 &amp; E1-5003. Per E-0005, NIC notes states, "Where "NIC" is indicated on the schedules for switchboards, panelboards, relay panels and lighting cabinets, contractor to provide the indicated overcurrent device, relay or dimming module. Feeder and branch circuit connections and conductors are deferred for future work."</p> <p>Please confirm which drawing is correct in regards to the 200A company switches, the plan or the riser diagram.</p>					<p><b>ANSWER:</b></p> <p>Contract Doc Ref: (ASI 128/E-0005, E1-2602, E1-2606, E1-5001, E1-5003)</p> <p>Location: (Zone 02, Park Level), (Zone 06, Park Level)</p> <p>Closest Column: (E1-2602/D.4,4), (E1-2606/D.8,29), (E1-5001/Park), (E1-5003/Park)</p> <p>Per ASI 118, the park level company switches were deleted in zoned 02 and 06. Per ASI 128 electrical plan drawings E1-2602 &amp; E1-2606 the company switches were added back but they are still shown as "NIC" on E1-5001 &amp; E1-5003. Per E-0005, NIC notes states, "Where "NIC" is indicated on the schedules for switchboards, panelboards, relay panels and lighting cabinets, contractor to provide the indicated overcurrent device, relay or dimming module. Feeder and branch circuit connections and conductors are deferred for future work."</p> <p>Please confirm which drawing is correct in regards to the 200A company switches, the plan or the riser diagram.</p>





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<b>T-2100</b>	<b>SCS - Beam layout Confirmation at Bollard Pit</b>	<b>Closed</b>	<b>01</b>	<b>01/16/2015</b>	<b>01/26/2015</b>	<b>01/22/2015</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: S1-2304 Location: Ground Level Grid Line: 14-15 Add'l Doc Ref's: Attached.		Contract Doc Ref: S1-2304 Location: Ground Level Grid Line: 14-15 Add'l Doc Ref's: Attached.				
Reference S1-2304 for the ground level bollard pit (South of GL G, between GL 14 and 15). Per approved submittal TG0702-027, the beam soffits are typically level in the east-west direction. The slab soffit slopes in the east-west direction. In the ground level bollard pit area, maintaining both the level beam bottoms and the sloping soffit will result in a reveal in the bottom of the concrete beams. See the attached sketch noting the expected 3/8" beam reveal.		Reference S1-2304 for the ground level bollard pit (South of GL G, between GL 14 and 15). Per approved submittal TG0702-027, the beam soffits are typically level in the east-west direction. The slab soffit slopes in the east-west direction. In the ground level bollard pit area, maintaining both the level beam bottoms and the sloping soffit will result in a reveal in the bottom of the concrete beams. See the attached sketch noting the expected 3/8" beam reveal.				
Please reference the attached sketch. Please confirm the beam layout in the sketch is acceptable.		Please reference the attached sketch. Please confirm the beam layout in the sketch is acceptable.				
<b>T-2101</b>	<b>ELV - Tag ELRC-1-A-EMG Numbered Note Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>01/16/2015</b>	<b>01/26/2015</b>	<b>01/28/2015</b>
<b>From:</b> Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: (ASI 128/Detail 1,E1-3301, 26 24 16apa, panel boards) Location: (Zone 02, Ground Level) Closest Column Line: (E.6,5)		Contract Doc Ref: (ASI 128/Detail 1,E1-3301, 26 24 16apa, panel boards) Location: (Zone 02, Ground Level) Closest Column Line: (E.6,5)				
ASI 128 drawing E1-3301 adds "ELRC-1-A-EMG" to the electrical room 01243 with numbered note 5 attached to it. Please confirm numbered note 3 should be attached to ELRC-1-A-EMG in lieu of numbered note 5. Please note circuit ELP-1-A-EMG/2 is assigned to DML-1-A-1.		ASI 128 drawing E1-3301 adds "ELRC-1-A-EMG" to the electrical room 01243 with numbered note 5 attached to it. Please confirm numbered note 3 should be attached to ELRC-1-A-EMG in lieu of numbered note 5. Please note circuit ELP-1-A-EMG/2 is assigned to DML-1-A-1.				
<b>T-2102</b>	<b>SCS - Widended Moment Frame Beam Ends</b>	<b>Closed</b>	<b>01</b>	<b>01/16/2015</b>	<b>01/26/2015</b>	<b>01/27/2015</b>
<b>From:</b> Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: 2/S1-3705, S1-3701,3702,3704,3705,3707		Contract Doc Ref: 2/S1-3705, S1-3701,3702,3704,3705,3707				





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	<div><div>Location: Ground Level Grid Line: 13,15,21,23 Add'l Doc Ref's: Attached.</div><div>Several concrete sections reference a widened concrete MFB, but their location in plan view do not show the widened beam. Reference section 2/SI-3705 as an example. The section is first cut at GL C/11, where the widened end is shown in plan. Section 2/SI-3705 is later cut at GL 13, 15, 21, and 23, where the widened end is not shown in plan.</div><div>Please confirm that the MFBs with widened ends occur when indicated the S 1-230# plans, and not when referenced by the concrete section. This solution would also apply to conflicts between plans and sections on Details: 9/SI-3701, 8/SI-3702, 9/SI-3702, I/SI-3704, 2/SI-3704, 2/SI-3705, 6/SI-3707.</div></div>					
T-2103	SCS - Ground Level Air Vent - Moment Frame Beam Conflict	Closed	01	01/16/2015	01/26/2015	01/26/2015
	<div>From: Webcor Construction LP</div> <div>Claude Titcher</div> <div>REQUEST: Contract Doc Ref: S1-2304, S1-2864 Location: Ground Level Grid Line: 14/G Add'l Doc Ref's: Attached.  Reference S1-2304 and AI-2864 for the Ground Level air vents. Cross referencing the two sheets results in a conflict between the openings, the TR girder running north/south, and the widened MFB. Find attached markup showing a partial plan view through the air vent slab opening at GL 14-G illustrating this conflict.  To resolve this conflict, Shimmick proposes to center the openings between the MFB CLs. This will result in 4.5" from edge of opening to face of beam, and 3' 9" from GL 14 to CL of opening. This proposed solution will apply at all GL intersections where this air vent opening conflict occurs.  Please confirm this solution is acceptable, and/or please</div>					
	<div></div> <div>ANSWER: Contract Doc Ref: S1-2304, S1-2864 Location: Ground Level Grid Line: 14/G Add'l Doc Ref's: Attached.  Reference S1-2304 and AI-2864 for the Ground Level air vents. Cross referencing the two sheets results in a conflict between the openings, the TR girder running north/south, and the widened MFB. Find attached markup showing a partial plan view through the air vent slab opening at GL 14-G illustrating this conflict.  To resolve this conflict, Shimmick proposes to center the openings between the MFB CLs. This will result in 4.5" from edge of opening to face of beam, and 3' 9" from GL 14 to CL of opening. This proposed solution will apply at all GL intersections where this air vent opening conflict occurs.  Please confirm this solution is acceptable, and/or</div>					





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	revise the slab edge drawings to reflect the moved openings.					please revise the slab edge drawings to reflect the moved openings.
<b>T-2104</b>	<b>SCS - Rebar Detail for MFB14 at G16.9 - Roadway Slab Area</b>	<b>Closed</b>	<b>01</b>	<b>01/16/2015</b>	<b>01/26/2015</b>	<b>01/27/2015</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b> Contract Doc Ref: 1/S1-3703, S1-2304 Location: Ground Level Grid Line: 16.9/19.1 Add'l Doc Ref's: Attached.  Please reference Detail 1/SI-3703, showing the rebar detail for the MFBs bordering the roadway slab at GL 16.9/19.1. The detail shows the bottom of the concrete beam below the bottom of steel. At MFB14 at G/16.9, the bottom of the concrete beam is above the bottom of steel. Please see attached SI-2304 mark up cross referencing contract elevations. The above elevation conflict removes the need for the 4' 90 degree embedment and the welded couplers/bars at the bottom flange shown in SI-3703. No detail is currently available for the above situation.  Please provide a new rebar detail applicable when the bottom of beam is above top of steel for MFB 14 at G/16.9. Please include an alternative for the 4' vertical embedment and the welded couplers/bars on bottom flange, both of which are not applicable in this case. Note that the new bottom bar end condition must correlate with the stiffener plates on the bottom of flange.						<b>ANSWER:</b> Contract Doc Ref: 1/S1-3703, S1-2304 Location: Ground Level Grid Line: 16.9/19.1 Add'l Doc Ref's: Attached.  Please reference Detail 1/SI-3703, showing the rebar detail for the MFBs bordering the roadway slab at GL 16.9/19.1. The detail shows the bottom of the concrete beam below the bottom of steel. At MFB14 at G/16.9, the bottom of the concrete beam is above the bottom of steel. Please see attached SI-2304 mark up cross referencing contract elevations. The above elevation conflict removes the need for the 4' 90 degree embedment and the welded couplers/bars at the bottom flange shown in SI-3703. No detail is currently available for the above situation.  Please provide a new rebar detail applicable when the bottom of beam is above top of steel for MFB 14 at G/16.9. Please include an alternative for the 4' vertical embedment and the welded couplers/bars on bottom flange, both of which are not applicable in this case. Note that the new bottom bar end condition must correlate with the stiffener plates on the bottom of flange.





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<b>T-2107</b>	<b>SCS - Chamfer Requirements for Ground Level Beams</b>	<b>Closed</b>	<b>01</b>	<b>01/16/2015</b>	<b>01/26/2015</b>	<b>01/27/2015</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Contract Doc Ref: 03 10 01 Location: Ground Level Grid Line: N/A Add'l Doc Ref's: Attached.  Please find attached Specification 03 10 01 sections 2.2.B.6 and 3.1.A.12, stating that Chamfers for Form Corners are "required for all exposed corners and edge of beam, walls and column forms".  Note that it is not industry standard to chamfer inside corners of concrete beams. Only outside corners are chamfered. See attached sketch.  Please confirm chamfers are only required on outside corners of concrete beams for the ground level.						<b>ANSWER:</b>  Contract Doc Ref: 03 10 01 Location: Ground Level Grid Line: N/A Add'l Doc Ref's: Attached.  Please find attached Specification 03 10 01 sections 2.2.B.6 and 3.1.A.12, stating that Chamfers for Form Corners are "required for all exposed corners and edge of beam, walls and column forms".  Note that it is not industry standard to chamfer inside corners of concrete beams. Only outside corners are chamfered. See attached sketch.  Please confirm chamfers are only required on outside corners of concrete beams for the ground level.
<b>T-2108A</b>	<b>SSS - ST603 AECS Approval Comment Clarifications GL27.1-28</b>	<b>Closed</b>	<b>CR</b>	<b>01/19/2015</b>	<b>01/29/2015</b>	<b>01/21/2015</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  Contract Doc Ref: S1-2406, 4/A1-7836 Location: Zone 4, Second Level Gridline: 27.1-28 Add'l Doc Ref's: CD RFI 744 SK1  See attached CD RFI 744 SK1:  2.) On shop drawing No. 7088 for beam A7088 in submittal TG0701-133, AAI noted "Exposed areas are AECS with IFRM-1 per 4/A1-7836. Clarify extent of prep and finish."  Reference to detail 4/A1-7836 is not helpful in determining exposed areas.  Please supply a sketch showing exact extents of paint and no paint zones on this beam to be primed for future IFRM.						<b>ANSWER:</b>  Contract Doc Ref: S1-2406, 4/A1-7836 Location: Zone 4, Second Level Gridline: 27.1-28 Add'l Doc Ref's: CD RFI 744 SK1  See attached CD RFI 744 SK1:  2.) On shop drawing No. 7088 for beam A7088 in submittal TG0701-133, AAI noted "Exposed areas are AECS with IFRM-1 per 4/A1-7836. Clarify extent of prep and finish."  Reference to detail 4/A1-7836 is not helpful in determining exposed areas.  Please supply a sketch showing exact extents of paint and no paint zones on this beam to be primed for future IFRM.



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<b>T-2108B</b>	<b>SSS - ST603 AESS Approval Comment Clarifications GL27.1-28</b>	<b>Closed</b>	<b>CR</b>	<b>01/19/2015</b>	<b>01/29/2015</b>	<b>01/26/2015</b>
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div><div><b>REQUEST:</b> Contract Doc Ref: S1-2406 Location: Zone 4, Second Level Gridline: 27.1-28 Add'l Doc Ref's: CD RFI 744 SK1  See attached CD RFI 744 SK1:  3.) In submittal TG0701-133, beam A7032 was not marked to be painted by AAI at approval.  Is it AESS/painted?  If yes, supply a sketch showing exact extents of paint and no paints zones on this beam to be primed for future IRFM.</div><div><b>ANSWER:</b> Contract Doc Ref: S1-2406 Location: Zone 4, Second Level Gridline: 27.1-28 Add'l Doc Ref's: CD RFI 744 SK1  See attached CD RFI 744 SK1:  3.) In submittal TG0701-133, beam A7032 was not marked to be painted by AAI at approval.  Is it AESS/painted?  If yes, supply a sketch showing exact extents of paint and no paints zones on this beam to be primed for future IRFM.</div></div>						
<b>T-2109</b>	<b>SSS - ST603 AESS Approval Comment Clarifications GL28</b>	<b>Closed</b>	<b>CR</b>	<b>01/19/2015</b>	<b>01/29/2015</b>	<b>01/21/2015</b>
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div><div><b>REQUEST:</b> Contract Doc Ref: S1-2406, 4/A1-7836 Location: Zone 4, Second Level Gridline: E/28 Add'l Doc Ref's: CD RFI 743 SK1 and TG0701-133  See attached CD RFI 743 SK1:  On shop drawing No. 2897 for beam A2987 in submittal TG0701-133, AAI noted "Exposed areas are AESS with IFRM-1 per 4/A1-7836. Clarify extent of prep and finish."  Reference to detail 4/A1-7836 is not helpful in determining exposed areas.  Please supply a sketch showing exact extents of paint and no paints zones on this beam to be primed for future IRFM.</div><div><b>ANSWER:</b> Contract Doc Ref: S1-2406, 4/A1-7836 Location: Zone 4, Second Level Gridline: E/28 Add'l Doc Ref's: CD RFI 743 SK1 and TG0701-133  See attached CD RFI 743 SK1:  On shop drawing No. 2897 for beam A2987 in submittal TG0701-133, AAI noted "Exposed areas are AESS with IFRM-1 per 4/A1-7836. Clarify extent of prep and finish."  Reference to detail 4/A1-7836 is not helpful in determining exposed areas.  Please supply a sketch showing exact extents of paint and no paints zones on this beam to be primed for future IRFM.</div></div>						
<b>T-2110</b>	<b>SCS - Layout for Roof Crane Concrete Pedestals</b>	<b>Open</b>	<b>01</b>	<b>01/19/2015</b>	<b>01/29/2015</b>	<b>01/19/2015</b>





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T-2112	SSS - Shop Splice on Transfer Girder GL20.1	Closed	CR	01/20/2015	01/20/2015	01/22/2015
From: Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  Please confirm it is acceptable to introduce a web shop splice 2'-6" from the north and south ends of transfer girder TR20.1. See attached for clarification.		<b>ANSWER:</b>  Please confirm it is acceptable to introduce a web shop splice 2'-6" from the north and south ends of transfer girder TR20.1. See attached for clarification.				
T-2113	BGP - Pouring Against Shared CDSM Wall - Zone 3	Closed	01	01/21/2015	01/31/2015	01/30/2015
From: Webcor Construction LP                      Claude Titcher						
<b>REQUEST:</b>  Contract Doc Ref: N/A Location: Zone 3 CDSM Wall Grid Line: A Add'l Doc Ref's: N/A  The Transbay Tower (Salesforce Tower) is performing excavation on the North side of the CDSM wall in Zone 3, such that the CDSM wall in Zone 3 is being shared by Transbay Project and Salesforce Tower Project.  Based on the current progress of construction, it is more than likely that TG06 will be pouring 3rd wall lift against CDSM Wall that are unsupported by soil (although braced) on the North side due to the ongoing excavation at the Tower project. See below; height of foundation wall pours under concern, hydrostatic pressures anticipated for subject pours.  - Height of 3rd WF at Z3 = Generally 11'-8", refer to TG0600-953 - Pmax= 720psf (see attached)  Please confirm that the slurry portion of the CDSM wall will provide sufficient structural integrity to withstand concrete pressure during TG06' 3rd Wall Lift Concrete Pour		<b>ANSWER:</b>  Contract Doc Ref: N/A Location: Zone 3 CDSM Wall Grid Line: A Add'l Doc Ref's: N/A  The Transbay Tower (Salesforce Tower) is performing excavation on the North side of the CDSM wall in Zone 3, such that the CDSM wall in Zone 3 is being shared by Transbay Project and Salesforce Tower Project.  Based on the current progress of construction, it is more than likely that TG06 will be pouring 3rd wall lift against CDSM Wall that are unsupported by soil (although braced) on the North side due to the ongoing excavation at the Tower project. See below; height of foundation wall pours under concern, hydrostatic pressures anticipated for subject pours.  - Height of 3rd WF at Z3 = Generally 11'-8", refer to TG0600-953 - Pmax= 720psf (see attached)  Please confirm that the slurry portion of the CDSM wall will provide sufficient structural integrity to withstand concrete pressure during TG06' 3rd Wall Lift Concrete Pour				
T-2114	BGP - Elimination of Construction Joint - Area 15 Foundation Walls - 2nd and 3rd I Closed		01	01/21/2015	01/31/2015	01/23/2015
From: Webcor Construction LP                      Claude Titcher						



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	<div>REQUEST:<div>Contract Doc Ref: N/A Location: W133, W132, W144,W143 Grid Line: 31-32 Add'l Doc Ref's: Attached</div><div>Please reference attached comprehensive layout drawings of Area 15 Foundation Walls.</div><div>Combined length of foundation walls 132 and 133 is 58'0" (North side). Combined length of foundation walls 143 and 144 is 58'-0" (South side).</div><div>Please confirm is acceptable to eliminate the construction joint location between the subject walls, on North and South sides, at the 2nd and 3rd lifts.</div></div>					<div>ANSWER:<div>Contract Doc Ref: N/A Location: W133, W132, W144,W143 Grid Line: 31-32 Add'l Doc Ref's: Attached</div><div>Please reference attached comprehensive layout drawings of Area 15 Foundation Walls.</div><div>Combined length of foundation walls 132 and 133 is 58'0" (North side). Combined length of foundation walls 143 and 144 is 58'-0" (South side).</div><div>Please confirm is acceptable to eliminate the construction joint location between the subject walls, on North and South sides, at the 2nd and 3rd lifts.</div></div>
T-2115	INT - Ramps and Railings at Prefabricated Buildings	Closed	CR	01/21/2015	01/21/2015	01/28/2015
	From: Webcor Construction LP      Tram Nguyen					
	<div>REQUEST:<div>Contract Doc Ref: Sheet A1-2502 (ASI 128 dated 12/16/14) Sheet SKA-4361 (A1-8168)</div><div>Location: Bus Deck Superintendent Station</div><div>Closest Column Line Intersection: N/A</div><div>Add'l Doc Ref's: T-2028 Response</div><div>Sheet A1-2502 at the Bus Deck Superintendent Station references A1-8168 for Prefabricated Booth details.</div><div>Sheet A1-8168 was provided in the T-2028 RFI Response and calls out ramps and "HD galv st railing with extensions and guide rails"at the Bus Deck Superintendent Station.</div><div>Specification Section 13 34 24 Prefabricated Guard Booth does not specify railing requirements. Specification 05 75 00 Architectural Metal Fabrications does not specifically call out railings at Prefabricated Buildings.</div></div>					<div>ANSWER:<div>Contract Doc Ref: Sheet A1-2502 (ASI 128 dated 12/16/14) Sheet SKA-4361 (A1-8168)</div><div>Location: Bus Deck Superintendent Station</div><div>Closest Column Line Intersection: N/A</div><div>Add'l Doc Ref's: T-2028 Response</div><div>Sheet A1-2502 at the Bus Deck Superintendent Station references A1-8168 for Prefabricated Booth details.</div><div>Sheet A1-8168 was provided in the T-2028 RFI Response and calls out ramps and "HD galv st railing with extensions and guide rails"at the Bus Deck Superintendent Station.</div><div>Specification Section 13 34 24 Prefabricated Guard Booth does not specify railing requirements.</div></div>



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T-2116	<b>INT - Painting of Walls and Ceilings</b>  From: Webcor Construction LP      Tram Nguyen  <b>REQUEST:</b> Contract Doc Ref: (ASI 128 dated 12/16/14) Sheet A1-9601 Sheet A1-9602  Location: N/A  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Paint finishes for walls and ceilings have been removed from spaces called out on Sheets A1-9601 and A1-9602.  The spaces still call for floor coverings/rubber base & ceilings to be furnished and installed as part of the contract.  Please confirm spaces shown not to receive paint on finish schedules are still to receive the remaining scheduled finishes (floor coverings/rubber base, ceilings, etc.) as part of the Phase 1 scope of work.	Open	CR	01/21/2015	01/31/2015	
	<b>ANSWER:</b> Contract Doc Ref: (ASI 128 dated 12/16/14) Sheet A1-9601 Sheet A1-9602  Location: N/A  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Paint finishes for walls and ceilings have been removed from spaces called out on Sheets A1-9601 and A1-9602.  The spaces still call for floor coverings/rubber base & ceilings to be furnished and installed as part of the contract.  Please confirm spaces shown not to receive paint on finish schedules are still to receive the remaining scheduled finishes (floor coverings/rubber base, ceilings, etc.) as part of the Phase 1 scope of work.					
T-2117	<b>MME - 4' High Checker Boards at Stair 403</b>  From: Webcor Construction LP      Tram Nguyen	Closed	CR	01/21/2015	01/31/2015	01/23/2015





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	<b>REQUEST:</b>  Contract Doc Ref: (ASI 128 dated 12/16/14) Sheet A1-9602 Sheet A1-7502  Location: Stair 403  Closest Column Line Intersection: N/A  Add'l Doc Ref's: ASI 128 Revision Narrative (dated 12/16/14) RFI P1-0208 Response  Per the Room Finish Schedule on Sheet A1-9602, Stair 403 requires a 4'-0" High Aluminum Checker Plate Base on all GB partitions.  Per Sheet A1-7502 Typical Stair Details, ASI 128 Revision Narrative, and RFI P1-0208 Response wall checker plates have been removed from all stairwells.  Please confirm that Aluminum Checker Plates have been removed from all stairwells and revise the drawings to match.					<b>ANSWER:</b>  Contract Doc Ref: (ASI 128 dated 12/16/14) Sheet A1-9602 Sheet A1-7502  Location: Stair 403  Closest Column Line Intersection: N/A  Add'l Doc Ref's: ASI 128 Revision Narrative (dated 12/16/14) RFI P1-0208 Response  Per the Room Finish Schedule on Sheet A1-9602, Stair 403 requires a 4'-0" High Aluminum Checker Plate Base on all GB partitions.  Per Sheet A1-7502 Typical Stair Details, ASI 128 Revision Narrative, and RFI P1-0208 Response wall checker plates have been removed from all stairwells.  Please confirm that Aluminum Checker Plates have been removed from all stairwells and revise the drawings to match.
<b>T-2118</b>	<b>INT - Acoustics Report Reference</b>	<b>Open</b>	<b>CR</b>	<b>01/21/2015</b>	<b>01/31/2015</b>	
	<b>From:</b> Webcor Construction LP      Tram Nguyen  <b>REQUEST:</b>  Contract Doc Ref: (ASI 128 dated 12/16/14) Detail 3/A-0020  Location: N/A  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Note #9 of Partition Notes on Detail 3/A-0020 refers to "Acoustics Report".  An "Acoustics Report" has not been provided with ASI 128.					<b>ANSWER:</b>  Contract Doc Ref: (ASI 128 dated 12/16/14) Detail 3/A-0020  Location: N/A  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Note #9 of Partition Notes on Detail 3/A-0020 refers to "Acoustics Report".  An "Acoustics Report" has not been provided with ASI 128.



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<b>T-2120A</b>	<b>SSS - TR28 Missing Information</b>	<b>Closed</b>	<b>CR</b>	<b>01/26/2015</b>	<b>02/05/2015</b>	<b>02/04/2015</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> Contract Doc: 1/S1-7015 Location: Zone 4, Ground Level Gridline: D.8/28 Add'l Doc Ref's: CD RFI 749 SK1  The attached CD RFI 749 SK1 has missing dimensions at GL D.8/28.  Please supply the missing dimensions.						<b>ANSWER:</b> Contract Doc: 1/S1-7015 Location: Zone 4, Ground Level Gridline: D.8/28 Add'l Doc Ref's: CD RFI 749 SK1  The attached CD RFI 749 SK1 has missing dimensions at GL D.8/28.  Please supply the missing dimensions.
<b>T-2120B</b>	<b>SSS - TR28 Missing Information</b>	<b>Closed</b>	<b>CR</b>	<b>01/26/2015</b>	<b>02/05/2015</b>	<b>02/04/2015</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> Contract Doc: 1/S1-7015, 2/S1-7663 Location: Zone 4, Ground Level Gridline: D.8/28 Add'l Doc Ref's: CD RFI 749 SK1  The connections for the posts per 2/S1-7663 will not work at the Transfer Girder with a 3 1/2" thick web.  Please supply a connection detail based on the actual condition at GL D.8.						<b>ANSWER:</b> Contract Doc: 1/S1-7015, 2/S1-7663 Location: Zone 4, Ground Level Gridline: D.8/28 Add'l Doc Ref's: CD RFI 749 SK1  The connections for the posts per 2/S1-7663 will not work at the Transfer Girder with a 3 1/2" thick web.  Please supply a connection detail based on the actual condition at GL D.8.
<b>T-2121</b>	<b>SSS - Conflicting Information at Roof Level GL28</b>	<b>Closed</b>	<b>CR</b>	<b>01/26/2015</b>	<b>02/05/2015</b>	<b>01/28/2015</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b> Contract Doc: A1-2906, S1-2606, S1-8008 Location: Zone 4, Roof Park Gridline: E/28 Add'l Doc Ref's: CD RFI 752 SK1  Drawing S1-2606 near GL E/28 references Detail 2 & 4/S1-8008 which indicates that the top of wall is at EL. 87'-4 ½.  Drawing A1-2906 at the same location shows that the top						<b>ANSWER:</b> Contract Doc: A1-2906, S1-2606, S1-8008 Location: Zone 4, Roof Park Gridline: E/28 Add'l Doc Ref's: CD RFI 752 SK1  Drawing S1-2606 near GL E/28 references Detail 2 & 4/S1-8008 which indicates that the top of wall is at EL. 87'-4 ½.  Drawing A1-2906 at the same location shows that the

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	<p>of wall is at EL. 87'-4"</p> <p>Please confirm which information is correct.</p>					<p>top of wall is at EL. 87'-4"</p> <p>Please confirm which information is correct.</p>
<b>T-2122</b>	<b>SSS - Wing Plate Detail at Girder Bottom Flange</b>	<b>Closed</b>	<b>CR</b>	<b>01/26/2015</b>	<b>02/05/2015</b>	<b>01/29/2015</b>
	<p><b>From:</b> Webcor Construction LP                      Stephanie Azzolino</p> <p><b>REQUEST:</b></p> <p>Contract Doc: S1-2252, S1-4350  Location: Zone 3, Ground Level  Gridline: B-D/23  Add'l Doc Ref's: Shop drawing 706BB from Submittal  TG0701-94, attached SK1</p> <p>Detail 3/S1-4350 shows a vertical CJP weld between the  3" thick wing plate and the bottom flange of the transfer  girder.</p> <p>We would like to eliminate the requirement for this short  length of vertical weld (approx. 1 5/8"). The length of the  horizontal weld would be increased to compensate as the  weld access hole can be reduced in width. Refer to the  attached SK1.</p> <p>Please confirm that it is acceptable to eliminate the  requirement for this short length of vertical weld.</p>					<p><b>ANSWER:</b></p> <p>Contract Doc: S1-2252, S1-4350  Location: Zone 3, Ground Level  Gridline: B-D/23  Add'l Doc Ref's: Shop drawing 706BB from Submittal  TG0701-94, attached SK1</p> <p>Detail 3/S1-4350 shows a vertical CJP weld between  the 3" thick wing plate and the bottom flange of the  transfer girder.</p> <p>We would like to eliminate the requirement for this  short length of vertical weld (approx. 1 5/8"). The  length of the horizontal weld would be increased to  compensate as the weld access hole can be reduced  in width. Refer to the attached SK1.</p> <p>Please confirm that it is acceptable to eliminate the  requirement for this short length of vertical weld.</p>



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<b>T-2123</b>	<b>SSS - TG to Perimeter Beam Connection at GL11</b>	<b>Closed</b>	<b>CR</b>	<b>01/27/2015</b>	<b>02/06/2015</b>	<b>02/10/2015</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc: S1-2303, S1-2304 Location: Zone 2, Ground Level Gridline: 10.1-12/C & G Add'l Doc Ref's: Shop Drawings from TG0701-073.1, SK RFI 1011 SK1-SK5		Contract Doc: S1-2303, S1-2304 Location: Zone 2, Ground Level Gridline: 10.1-12/C & G Add'l Doc Ref's: Shop Drawings from TG0701-073.1, SK RFI 1011 SK1-SK5				
The following condition occurs at 4 locations as can be seen in the attached SK1 to SK5.		The following condition occurs at 4 locations as can be seen in the attached SK1 to SK5.				
The transfer girder wing plate to perimeter beam connection at GL11 has several misaligned holes where the web extension plates are CJP welded to the BU beam. Misalignments range from 1/8" to 5/16".		The transfer girder wing plate to perimeter beam connection at GL11 has several misaligned holes where the web extension plates are CJP welded to the BU beam. Misalignments range from 1/8" to 5/16".				
Please confirm it is acceptable to ream 36 of the 90 holes in this slip critical connection. AISC 16.1-121, allows short slotted holes in any or all plies of slip critical connections.		Please confirm it is acceptable to ream 36 of the 90 holes in this slip critical connection. AISC 16.1-121, allows short slotted holes in any or all plies of slip critical connections.				
<b>T-2124</b>	<b>SSS - OCS Hanger Clarifications</b>	<b>Closed</b>	<b>CR</b>	<b>01/28/2015</b>	<b>02/07/2015</b>	<b>01/29/2015</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc: S1-9100 to S1-9102 Location: Zone 4 Gridline: 31.7-32/C.3 Add'l Doc Ref's: CD RFI 296.14 SK1		Contract Doc: S1-9100 to S1-9102 Location: Zone 4 Gridline: 31.7-32/C.3 Add'l Doc Ref's: CD RFI 296.14 SK1				
See the attached CD RFI 296.14 SK1 referencing detail 5/S1-9100:		See the attached CD RFI 296.14 SK1 referencing detail 5/S1-9100:				
1. Please supply (4) missing dimensions. 2. Confirm the underside (bottom) of 1/2" plate elevation.		1. Please supply (4) missing dimensions. 2. Confirm the underside (bottom) of 1/2" plate elevation.				
<b>T-2125</b>	<b>ELV- ELRC-B1-D-EMG Missing Location</b>	<b>Closed</b>	<b>CR</b>	<b>01/28/2015</b>	<b>02/07/2015</b>	<b>02/05/2015</b>
<b>From:</b> Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>		<b>ANSWER:</b>				



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	<p>Contract Doc Ref:[ASI 128/E1-2206,E1-3203,E1-5006/26 09 33] Location: Zone[06], Level [Concourse Level] Closest Column Line Intersection is [H,29] Add'l Doc's Ref's: N/A</p> <p>Per ASI 128, panel ELRC-B1-D-EMG was added to the riser diagram drawing E1-5006. The location for this panel is not shown on the lower concourse enlarged plans drawing. Please confirm ELRC-B1-D-EMG should be located in Electric Room B1644 (E1-3203/4).</p>					<p>Contract Doc Ref:[ASI 128/E1-2206,E1-3203,E1-5006/26 09 33] Location: Zone[06], Level [Concourse Level] Closest Column Line Intersection is [H,29] Add'l Doc's Ref's: N/A</p> <p>Per ASI 128, panel ELRC-B1-D-EMG was added to the riser diagram drawing E1-5006. The location for this panel is not shown on the lower concourse enlarged plans drawing. Please confirm ELRC-B1-D-EMG should be located in Electric Room B1644 (E1-3203/4).</p>
<b>T-2126</b>	<b>ELV-ELRC-3-B-EMG-1 Missing Electrical Room Location</b>	<b>Closed</b>	<b>CR</b>	<b>01/28/2015</b>	<b>02/07/2015</b>	<b>02/05/2015</b>
<b>From:</b> Webcor Construction LP      Aseem Goyal						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Contract Doc Ref:[ASI 128/E1-2504, E1-3501, E1-5005/26 09 33] Location: Zone[ 04], Level [Bus Deck ] Closest Column Line Intersection is [D.4,15] Add'l Doc's Ref's: N/A</p> <p>Per ASI 128, ELRC-3-B-EMG-1 was added to the riser diagram drawing E1-5005. The electrical room location for this panel is not shown on the bus deck enlarged plans. Should this panel be located in room Electrical Room #03480? Please confirm it is to be fed from ELP-3-B-EMG-1.</p>			<p>Contract Doc Ref:[ASI 128/E1-2504, E1-3501, E1-5005/26 09 33] Location: Zone[ 04], Level [Bus Deck ] Closest Column Line Intersection is [D.4,15] Add'l Doc's Ref's: N/A</p> <p>Per ASI 128, ELRC-3-B-EMG-1 was added to the riser diagram drawing E1-5005. The electrical room location for this panel is not shown on the bus deck enlarged plans. Should this panel be located in room Electrical Room #03480? Please confirm it is to be fed from ELP-3-B-EMG-1.</p>			



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>T-2127</b>	<b>ELV- ELRC-1-C-EMG Missing Electrical Room Location</b>	<b>Closed</b>	<b>CR</b>	<b>01/28/2015</b>	<b>02/07/2015</b>	<b>02/06/2015</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Aseem Goyal</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref:[ASI 128/E1-2304,E1-3301/3,E1-5006/26 09 33] Location: Zone[04 ], Level [Ground] Closest Column Line Intersection is [D,15] Add'l Doc's Ref's: N/A			Contract Doc Ref:[ASI 128/E1-2304,E1-3301/3,E1-5006/26 09 33] Location: Zone[04 ], Level [Ground] Closest Column Line Intersection is [D,15] Add'l Doc's Ref's: N/A			
Per ASI 128, ELRC-1-C-EMG was added to the riser diagram E1-5006. This panel is not shown on the ground level electrical enlarged plans. Please confirm this panel is located in ER #01421 and should be shown on E1-3301/3 with a numbered note attached to it instructing us to provide a 120V emergency power connection to ELP-1-C-EMG.			Per ASI 128, ELRC-1-C-EMG was added to the riser diagram E1-5006. This panel is not shown on the ground level electrical enlarged plans. Please confirm this panel is located in ER #01421 and should be shown on E1-3301/3 with a numbered note attached to it instructing us to provide a 120V emergency power connection to ELP-1-C-EMG.			



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T-2128	ELV - Missing Emergency Lighting Relay Cabinets	Open	CR	01/28/2015	02/07/2015	
From: Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref:[ASI 128/E1-5301 thru E1-5303/26 09 33/APA] Location: Zone[02-03], Level [Ground]; Zone [02,04], Level [Second];Zone[02,03,05,06], Level [Bus Deck]; Closest Column Line Intersection is [N/A] Add'l Doc's Ref's: N/A		Contract Doc Ref:[ASI 128/E1-5301 thru E1-5303/26 09 33/APA] Location: Zone[02-03], Level [Ground]; Zone [02,04], Level [Second];Zone[02,03,05,06], Level [Bus Deck]; Closest Column Line Intersection is [N/A] Add'l Doc's Ref's: N/A				
The following Emergency Lighting Relay Cabinets are shown on drawings E1-5301 through E1-5303 and were all added per ASI 0128:		The following Emergency Lighting Relay Cabinets are shown on drawings E1-5301 through E1-5303 and were all added per ASI 0128:				
1. ELRC-2-A-EMG 2. ELRC-1-A-EMG 3. ELRC-3-B-EMG-1 4. ELRC-3-B-EMG-2 5. ELRC-2-B-EMG 6. ELRC-1-B-EMG 7. ELRC-3-C-EMG 8. ELRC-1-C-EMG 9. ELRC-3-D-EMG 10. ELRC-B1-D-EMG		1. ELRC-2-A-EMG 2. ELRC-1-A-EMG 3. ELRC-3-B-EMG-1 4. ELRC-3-B-EMG-2 5. ELRC-2-B-EMG 6. ELRC-1-B-EMG 7. ELRC-3-C-EMG 8. ELRC-1-C-EMG 9. ELRC-3-D-EMG 10. ELRC-B1-D-EMG				
The schedules for ALL of the above listed Emergency Lighting Relay Cabinets are missing from specification section 26 09 33/APA - ASI 0128.Without these missing schedules the Emergency Lighting Relay Cabinets, added by ASI 0128 cannot be properly quantified.Please provide these missing schedules.		The schedules for ALL of the above listed Emergency Lighting Relay Cabinets are missing from specification section 26 09 33/APA - ASI 0128.Without these missing schedules the Emergency Lighting Relay Cabinets, added by ASI 0128 cannot be properly quantified.Please provide these missing schedules.				





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T-2129	ELT - Roof Park Restaurant Elevator in ASI 128	Closed	CR	01/28/2015	02/07/2015	02/03/2015
<b>From:</b> Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>  Location: Grid 5 Roof Park Ref: A1- 3450, A1-3451 & A1-6251 Add'l Doc Ref's : N/A  ASI # 128 shows a new, future two-stop elevator for the Roof Park Restaurant but specifications were not revised to include a new two-stop elevator. Please advise if: 1.) Is Webcor/Obayashi to include the new Roof Park Restaurant elevator within its ASI # 128 pricing? 2.)If so, are there new specifications, drawings and details with which to price the new elevator?		<b>ANSWER:</b>  Location: Grid 5 Roof Park Ref: A1- 3450, A1-3451 & A1-6251 Add'l Doc Ref's : N/A  ASI # 128 shows a new, future two-stop elevator for the Roof Park Restaurant but specifications were not revised to include a new two-stop elevator. Please advise if: 1.) Is Webcor/Obayashi to include the new Roof Park Restaurant elevator within its ASI # 128 pricing? 2.)If so, are there new specifications, drawings and details with which to price the new elevator?				
T-2130	ELT - Roof Park Restaurant Dumbwaiter in ASI 128	Closed	CR	01/28/2015	02/07/2015	02/03/2015
<b>From:</b> Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>  Subject: Roof Park Restaurant Dumbwaiter (ASI # 128) Location: Grid 5 Roof Park Add'l Doc : N/A  During Pre-Construction scope discussions regarding the Roof Park Restaurant, it was understood a dumbwaiter was to be included in ASI # 128. However, ASI # 128 Drawings and Specifications do not indicate a dumbwaiter requirement. Please advise if:  1.)Is Webcor/Obayashi to include the new Roof Park Restaurant dumbwaiter within its ASI # 128 pricing? 2.)If so, are there new specifications, drawings and details with which to price a dumbwaiter?		<b>ANSWER:</b>  Subject: Roof Park Restaurant Dumbwaiter (ASI # 128) Location: Grid 5 Roof Park Add'l Doc : N/A  During Pre-Construction scope discussions regarding the Roof Park Restaurant, it was understood a dumbwaiter was to be included in ASI # 128. However, ASI # 128 Drawings and Specifications do not indicate a dumbwaiter requirement. Please advise if:  1.)Is Webcor/Obayashi to include the new Roof Park Restaurant dumbwaiter within its ASI # 128 pricing? 2.)If so, are there new specifications, drawings and details with which to price a dumbwaiter?				
T-2131	TOP - Enclosed Space Requirements for Polish Concrete	Open	CR	01/28/2015	01/28/2015	02/06/2015
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>  Contract Doc Ref: (ASI 128 dated 12/16/14)		<b>ANSWER:</b>  Contract Doc Ref: (ASI 128 dated 12/16/14)				



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	Specification Section 03 35 43  Location: Bus Deck Level  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Specification Section 03 35 43 (1.4) C Polished Concrete Finishing requires that the space around the polished concrete be completely enclosed.  The Bus Deck Level Polished Concrete is an exterior space.  Please confirm areas around polished concrete does not have to be enclosed prior to installation.					
	Specification Section 03 35 43  Location: Bus Deck Level  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Specification Section 03 35 43 (1.4) C Polished Concrete Finishing requires that the space around the polished concrete be completely enclosed.  The Bus Deck Level Polished Concrete is an exterior space.  Please confirm areas around polished concrete does not have to be enclosed prior to installation.					
<b>T-2132</b>	<b>TOP - Precast Concrete Tactile Guide Specifications</b>	<b>Open</b>	<b>CR</b>	<b>01/28/2015</b>	<b>02/07/2015</b>	
	<b>From:</b> Webcor Construction LP      Tram Nguyen  <b>REQUEST:</b> Contract Doc Ref: (ASI 128 dated 12/16/14) Sheets A1-9562 through A1-9567  Location: Bus Deck Level  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Per Sheets A1-9562 through A1-9567, precast conc. tactile guides are required at the bus deck level pedestrian paving areas.  No specification has been provided for the precast conc. tactile guides.  Please provide a specification for the precast conc. tactile guide.					
	<b>ANSWER:</b> Contract Doc Ref: (ASI 128 dated 12/16/14) Sheets A1-9562 through A1-9567  Location: Bus Deck Level  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Per Sheets A1-9562 through A1-9567, precast conc. tactile guides are required at the bus deck level pedestrian paving areas.  No specification has been provided for the precast conc. tactile guides.  Please provide a specification for the precast conc. tactile guide.					



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<b>T-2133</b>	<b>TOP - Polished Concrete Layout and Color Scheme Coordination</b>	<b>Open</b>	<b>CR</b>	<b>01/28/2015</b>	<b>01/28/2015</b>	<b>02/10/2015</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43						Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43
Location: N/A						Location: N/A
Closest Column Line Intersection: N/A						Closest Column Line Intersection: N/A
Add'l Doc Ref's: N/A						Add'l Doc Ref's: N/A
Specification Section 03 35 43 (1.4) D.1 Polished Concrete Finishing requires bidders to coordinate layout and color scheme of the work with the TJPA Representative.						Specification Section 03 35 43 (1.4) D.1 Polished Concrete Finishing requires bidders to coordinate layout and color scheme of the work with the TJPA Representative.
No layout (beyond CJ and EJ locations) or color scheme has been provided.						No layout (beyond CJ and EJ locations) or color scheme has been provided.
Please provide any layout information required beyond the EJ/CJ layout (if any) and all polished concrete color schemes.						Please provide any layout information required beyond the EJ/CJ layout (if any) and all polished concrete color schemes.
<b>T-2134</b>	<b>TOP - Polished Concrete Reinforcement Clarification</b>	<b>Open</b>	<b>CR</b>	<b>01/28/2015</b>	<b>02/07/2015</b>	<b>02/10/2015</b>
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43 Sheet A1-9533						Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43 Sheet A1-9533
Location: N/A						Location: N/A
Closest Column Line Intersection: N/A						Closest Column Line Intersection: N/A
Add'l Doc Ref's: N/A						Add'l Doc Ref's: N/A
Specification Section 03 35 43 (2.3) K calls 6x6x9x9 galv. 12 ga. welded wire mesh reinforcement.						Specification Section 03 35 43 (2.3) K calls 6x6x9x9 galv. 12 ga. welded wire mesh reinforcement.
Topping Slab Note 4.0/A1-9533 requires #4 rebar at 16" o.c. each way.						Topping Slab Note 4.0/A1-9533 requires #4 rebar at 16" o.c. each way.
Please clarify the desired reinforcement of polished concrete topping slabs.						Please clarify the desired reinforcement of polished concrete topping slabs.



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T-2135	TOP - Artist Requirements for Polish Concrete Samples	Open	CR	01/28/2015	02/07/2015	02/10/2015
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43		Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43				
Location: N/A		Location: N/A				
Closest Column Line Intersection: N/A		Closest Column Line Intersection: N/A				
Add'l Doc Ref's: N/A		Add'l Doc Ref's: N/A				
Specification Section 03 35 43 (1.5) C Polished Concrete Finishing indicates that approved samples are to be used for the artist's samples.		Specification Section 03 35 43 (1.5) C Polished Concrete Finishing indicates that approved samples are to be used for the artist's samples.				
No artist requirements have been provided for this work.		No artist requirements have been provided for this work.				
Please confirm there are no artist requirements for this scope of work.		Please confirm there are no artist requirements for this scope of work.				
T-2136	TOP - Terrazzo References per Specification Section 03 35 43	Open	CR	01/28/2015	02/07/2015	02/10/2015
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:		ANSWER:				
Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43		Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43				
Location: N/A		Location: N/A				
Closest Column Line Intersection: N/A		Closest Column Line Intersection: N/A				
Add'l Doc Ref's: N/A		Add'l Doc Ref's: N/A				
Specification Section 03 35 43 (1.5) B, D, G, and H Polished Concrete Finishing reference terrazzo and/or work associated with terrazzo.		Specification Section 03 35 43 (1.5) B, D, G, and H Polished Concrete Finishing reference terrazzo and/or work associated with terrazzo.				
This is not applicable to polished concrete.		This is not applicable to polished concrete.				
Please revise the shop drawing requirements to conform with a polished concrete specification.		Please revise the shop drawing requirements to conform with a polished concrete specification.				
T-2137	TOP - FL Clarification for Concrete on Structural Steel Building	Open	CR	01/28/2015	02/07/2015	



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	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div><div>REQUEST: Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43  Location: N/A  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Specification Section 03 35 43 (1.8) B.5 Polished Concrete Finishing requires a floor levelness of FL 40.  Per ACI and structural concrete specifications, FL requirements are not applicable to concrete poured on a structural steel building.  Please confirm that no FL requirements need to be met by this scope of work.</div></div>					
						<div>ANSWER: Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43  Location: N/A  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Specification Section 03 35 43 (1.8) B.5 Polished Concrete Finishing requires a floor levelness of FL 40.  Per ACI and structural concrete specifications, FL requirements are not applicable to concrete poured on a structural steel building.  Please confirm that no FL requirements need to be met by this scope of work.</div>
T-2138	TOP - Field Condition Requirements for Polished Concrete	Open	CR	01/28/2015	02/07/2015	
	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div><div>REQUEST: Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43  Location: Bus Deck Level  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Specification Section 03 35 43 (1.10) A Polished Concrete Finishing requires that the polished concrete contractor maintain temperature above 50 deg F for 48 hours before and during polished concrete flooring installation.  This will require the pedestrian area of the Bus Deck Level be temporarily enclosed and temporary heaters brought in.  Webcor/Obayashi suggests revising the language to require environmental conditions meet the manufacturer's</div></div>					
						<div>ANSWER: Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43  Location: Bus Deck Level  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Specification Section 03 35 43 (1.10) A Polished Concrete Finishing requires that the polished concrete contractor maintain temperature above 50 deg F for 48 hours before and during polished concrete flooring installation.  This will require the pedestrian area of the Bus Deck Level be temporarily enclosed and temporary heaters brought in.  Webcor/Obayashi suggests revising the language to</div>



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	<p>written installation instructions.</p> <p>Please confirm that Specification Section 03 35 43 (1.10) A Polished Concrete Finishing is to be revised to mandate the environmental conditions meet the manufacture's written installation instructions, and not what is shown in ASI 128.</p>					<p>require environmental conditions meet the manufacturer's written installation instructions.</p> <p>Please confirm that Specification Section 03 35 43 (1.10) A Polished Concrete Finishing is to be revised to mandate the environmental conditions meet the manufacture's written installation instructions, and not what is shown in ASI 128.</p>
T-2139	TOP - Environmental Condition Requirements for Polished Concrete	Open	CR	01/29/2015	01/29/2015	
	<p>From: Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43</p> <p>Location: Bus Deck Level</p> <p>Closest Column Line Intersection: N/A</p> <p>Add'l Doc Ref's: N/A</p> <p>Specification Section 03 35 43 (3.3) A Polished Concrete Finishing requires a temperature range between 50 deg F and 90 deg F.</p> <p>To avoid conflicts with the manufacturer's written installation instructions, Webcor/Obayashi suggests revising the language to mandate environmental conditions meet the manufacturer's written installation instructions.</p> <p>Please confirm that Specification Section 03 35 43 (3.3) A Polished Concrete Finishing is to be revised to mandate the environmental conditions meet the manufacture's written installation instructions, and not what is shown in ASI 128.</p>					<p><b>ANSWER:</b></p> <p>Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43</p> <p>Location: Bus Deck Level</p> <p>Closest Column Line Intersection: N/A</p> <p>Add'l Doc Ref's: N/A</p> <p>Specification Section 03 35 43 (3.3) A Polished Concrete Finishing requires a temperature range between 50 deg F and 90 deg F.</p> <p>To avoid conflicts with the manufacturer's written installation instructions, Webcor/Obayashi suggests revising the language to mandate environmental conditions meet the manufacturer's written installation instructions.</p> <p>Please confirm that Specification Section 03 35 43 (3.3) A Polished Concrete Finishing is to be revised to mandate the environmental conditions meet the manufacture's written installation instructions, and not what is shown in ASI 128.</p>
T-2140	TOP - Airblast Criteria Requirements for Seismic Joints	Open	CR	01/29/2015	02/08/2015	
	<p>From: Webcor Construction LP                      Tram Nguyen</p> <p><b>REQUEST:</b></p>					<p><b>ANSWER:</b></p>



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	<p>Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43 Specification Section 07 09 13</p> <p>Location: Bus Deck Level</p> <p>Closest Column Line Intersection: N/A</p> <p>Add'l Doc Ref's: N/A</p> <p>Specification Section 07 09 13 (1.6) E.2 Seismic Joint Assemblies indicates that seismic joint assemblies specified in Specification Section 07 09 13 are subject to the loading requirements of Specification Section 08 05 13 Airblast Criteria for Glazing.</p> <p>Not all of the seismic joints identified within Specification Section 07 09 13 are integral to a glazing system.</p> <p>Please confirm that seismic joints specified in Specification Section 07 09 13 Seismic Joint Assemblies which are not integral to a glazing system are not subject to the requirements of Specification Section 08 05 13 Airblast Criteria for Glazing.</p>					
	<p>Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 03 35 43 Specification Section 07 09 13</p> <p>Location: Bus Deck Level</p> <p>Closest Column Line Intersection: N/A</p> <p>Add'l Doc Ref's: N/A</p> <p>Specification Section 07 09 13 (1.6) E.2 Seismic Joint Assemblies indicates that seismic joint assemblies specified in Specification Section 07 09 13 are subject to the loading requirements of Specification Section 08 05 13 Airblast Criteria for Glazing.</p> <p>Not all of the seismic joints identified within Specification Section 07 09 13 are integral to a glazing system.</p> <p>Please confirm that seismic joints specified in Specification Section 07 09 13 Seismic Joint Assemblies which are not integral to a glazing system are not subject to the requirements of Specification Section 08 05 13 Airblast Criteria for Glazing.</p>					
<b>T-2141</b>	<b>SST - Stair and Railing Mock-Up Location</b>	<b>Open</b>	<b>01</b>	<b>01/29/2015</b>	<b>02/08/2015</b>	
	<p><b>From:</b> Webcor Construction LP      Claude Titcher</p> <p><b>REQUEST:</b></p> <p>As discussed in Olson Steel's QA-QC Meeting of Mutual Understanding on 12/23/14 with Turner/WOJV, and again in Olson's Preparatory and Initial DFOV meetings on 1/5/15, it was agreed that the Stair and Tailing Mock-up (One complete floor to floor exit stair mock-up complete with landing and railing per 05 51 00, 1.8 Quality Control, Part C) will encompass the Stair #403 stringers and landings from the Train Platform EL.-20'-0" to EL -18'-7 1/8". Olson Steel is currently fabricating this mock-up, and as agreed with Turner/WOJV, the mock-up will be fully assembled in Olson's San Leandro shop for the Design Team to review. This RFI is being submitted for record to revise the location of the mock-up to Olson's fabrication shop in San Leandro, CA. Since the mock-up is an actual</p>					
						<p><b>ANSWER:</b></p> <p>As discussed in Olson Steel's QA-QC Meeting of Mutual Understanding on 12/23/14 with Turner/WOJV, and again in Olson's Preparatory and Initial DFOV meetings on 1/5/15, it was agreed that the Stair and Tailing Mock-up (One complete floor to floor exit stair mock-up complete with landing and railing per 05 51 00, 1.8 Quality Control, Part C) will encompass the Stair #403 stringers and landings from the Train Platform EL.-20'-0" to EL -18'-7 1/8". Olson Steel is currently fabricating this mock-up, and as agreed with Turner/WOJV, the mock-up will be fully assembled in Olson's San Leandro shop for the Design Team to review. This RFI is being submitted for record to revise the location of the mock-up to Olson's fabrication shop</p>



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T-2142	<b>SSS - Approval Comment Clarification at Beam Connection GL29-30</b>  From: Webcor Construction LP Stephanie Azzolino  <b>REQUEST:</b> Contract Doc: 1/S1-5012, S1-2306 Location: Zone 4, Ground Level Gridline: D.8 /29 - 30 Add'l Doc Ref's: CD RFI #753 SK1, TG0701-135  The attached CD RFI #753 SK1 references Shop drawing 7094 from submittal TG0701-135 where TT commented "Copying of bottom flange not acceptable".  Per detail 1/S1-5012, the beam has to be coped to fit the connection.  Supply a new detail if this connection is not acceptable.	Closed	CR	01/30/2015	01/30/2015	02/06/2015
	portion of Stair #402, a secondary review can take place if needed once the stair is installed onsite.  Please confirm this is acceptable.					in San Leandro, CA. Since the mock-up is an actual portion of Stair #402, a secondary review can take place if needed once the stair is installed onsite.  Please confirm this is acceptable.





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T-2143	TOP - Manhole Access Openings at Glass Building - Café (W-20)	Open	CR	01/29/2015	02/08/2015	
From: Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: (ASI 128 dated 12/16/14) Detail 1/S1-2650 Detail 1/A1-8641			Contract Doc Ref: (ASI 128 dated 12/16/14) Detail 1/S1-2650 Detail 1/A1-8641			
Location: Glass Building (W-20) on Roof Park Level			Location: Glass Building (W-20) on Roof Park Level			
Closest Column Line Intersection: N/A			Closest Column Line Intersection: N/A			
Add'l Doc Ref's: RFI Response P1-0341			Add'l Doc Ref's: RFI Response P1-0341			
Detail 1/S1-2650 calls depicts two manhole access openings and references architectural drawings for typical locations and dimensions.			Detail 1/S1-2650 calls depicts two manhole access openings and references architectural drawings for typical locations and dimensions.			
Detail 1/A1-8641 does not show manhole access openings.			Detail 1/A1-8641 does not show manhole access openings.			
RFI Response P1-0341 states, "The two 30" access manholes shown on A1-8630A are deleted..."			RFI Response P1-0341 states, "The two 30" access manholes shown on A1-8630A are deleted..."			
Please confirm the manhole access openings depicted in Detail 1/S1-2650 are deleted per RFI Response P1-0341 and revise the drawings to match.			Please confirm the manhole access openings depicted in Detail 1/S1-2650 are deleted per RFI Response P1-0341 and revise the drawings to match.			



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T-2144	TOP - Seismic Joint Cover Clarification	Closed	CR	01/29/2015	01/29/2015	02/02/2015
<b>From:</b> Webcor Construction LP                      Tram Nguyen						
<b>REQUEST:</b> Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 07 09 13 Specification Section 05 60 00  Location: N/A  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Specification Section 07 09 13 (2.2) A Seismic Joint Assemblies identifies Construction Specialties, MM Systems, Michael Rizza, Specialties Building Components as acceptable manufacturers for FJC1, and the associated cover plate.  Specification Section 05 60 00 (2.2) C Site Metal Fabrication calls out for the seismic joint cover manufacturer to be Specialty Design Group and makes no mention of the associated seismic joint assembly.  Please confirm that the seismic joint assembly, including the associated cover is to be furnished and installed as specified in Specification Section 07 09 13 Seismic Joint Assemblies, and not Specification Section 05 60 00 Site Metal Fabrication.			<b>ANSWER:</b> Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 07 09 13 Specification Section 05 60 00  Location: N/A  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Specification Section 07 09 13 (2.2) A Seismic Joint Assemblies identifies Construction Specialties, MM Systems, Michael Rizza, Specialties Building Components as acceptable manufacturers for FJC1, and the associated cover plate.  Specification Section 05 60 00 (2.2) C Site Metal Fabrication calls out for the seismic joint cover manufacturer to be Specialty Design Group and makes no mention of the associated seismic joint assembly.  Please confirm that the seismic joint assembly, including the associated cover is to be furnished and installed as specified in Specification Section 07 09 13 Seismic Joint Assemblies, and not Specification Section 05 60 00 Site Metal Fabrication.			



**ANSWER:**

Contract Doc Ref: (ASI 128 dated 12/16/14)  
Detail 1/A1-9585  
Sheets A1-9520 through A1-9537

Location: N/A

Closest Column Line Intersection: N/A

Add'l Doc Ref's:  
RFI Response P1-0457

Detail 1/A1-9585 calls out for the Grand Hall topping slab thickness to be 3-5/8", with radiant floor heating tubes and #4 rebar.

Notes on Sheets A1-9520 through A1-9537 confirm that reinforcement within topping slabs is to be #4 rebar at 16" o.c. each way.

Based upon the minimum clearances and material thicknesses (1.5" clr. for tubing, 3/4" nominal O.D. for tubing, 1" for rebar mat, 3/4" minimum clr. for rebar), the topping slab minimum thickness would be required to be 4" (NOTE: 4" does not include any room for allowable tolerances), which exceeds the thickness called out on Detail 1/A1-9585.

Please provide direction as to the desired topping slab thickness at the Grand Hall (NOTE: RFI P1-0457 previously changed the reinforcement to 6x6x4.5x4.5 within the Grand Hall).



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T-2146	TOP - Dowel Requirements at Polished Concrete Expansion Joints	Open	CR	01/29/2015	02/08/2015	
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:			ANSWER:			
Contract Doc Ref: (ASI 128 dated 12/16/14) Details 2 & 4/A1-9585 Specification Section 03 20 02			Contract Doc Ref: (ASI 128 dated 12/16/14) Details 2 & 4/A1-9585 Specification Section 03 20 02			
Location: N/A			Location: N/A			
Closest Column Line Intersection: N/A			Closest Column Line Intersection: N/A			
Add'l Doc Ref's: N/A			Add'l Doc Ref's: N/A			
Details 2 & 4/A1-9585 feature the expansion joints identified on the A1-95XX Series. Specification Section 03 20 02 (3.1) D.1 Concrete Reinforcement and Embedded Assemblies indicates rebar and WWF are not to be continuous between expansion joints.			Details 2 & 4/A1-9585 feature the expansion joints identified on the A1-95XX Series. Specification Section 03 20 02 (3.1) D.1 Concrete Reinforcement and Embedded Assemblies indicates rebar and WWF are not to be continuous between expansion joints.			
Details 2 & 4/A1-9585 do not call out for dowels at the expansion joints to prevent the slab edges from curling away from each other.			Details 2 & 4/A1-9585 do not call out for dowels at the expansion joints to prevent the slab edges from curling away from each other.			
Please confirm no dowels are required at the polished concrete topping slab expansion joints.			Please confirm no dowels are required at the polished concrete topping slab expansion joints.			



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T-2147	TOP - Permanent Topping Slab to Knock Out Details	Open	CR	01/30/2015	02/09/2015	02/05/2015
From: Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc Ref: (ASI 128 dated 12/16/14) Detail 1/A1-7572		Contract Doc Ref: (ASI 128 dated 12/16/14) Detail 1/A1-7572				
Location: N/A		Location: N/A				
Closest Column Line Intersection: N/A		Closest Column Line Intersection: N/A				
Add'l Doc Ref's: N/A		Add'l Doc Ref's: N/A				
Detail 1/A1-7572 appears to show a bolted connection between the edge of permanent terrazzo covered topping slab and the knock out panel topping slabs.		Detail 1/A1-7572 appears to show a bolted connection between the edge of permanent terrazzo covered topping slab and the knock out panel topping slabs.				
No detailed information is given on the assembly (angle size, bolt size, how often does the bolted connection occur, what is the solid grey color, is this a typical detail at all edges of all knock out panels, etc.).		No detailed information is given on the assembly (angle size, bolt size, how often does the bolted connection occur, what is the solid grey color, is this a typical detail at all edges of all knock out panels, etc.).				
Please provide the full detailing on the permanent topping slab to knock out slab condition.		Please provide the full detailing on the permanent topping slab to knock out slab condition.				





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<b>T-2149</b>	<b>TOP - Curb Requirements at Knock Out/Permanent Slab Joint</b>	<b>Closed</b>	<b>CR</b>	<b>01/30/2015</b>	<b>02/09/2015</b>	<b>02/05/2015</b>
<b>From:</b> Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>  Contract Doc Ref: (ASI 128 dated 12/16/14) Sheet S1-2305 Sheet A1-2865  Location: N/A  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Sheet S1-2305 and Sheet A1-2865 show 3 ea. structural deck knock out slabs between GL 20 and 22. Sheet A1-2305 shows topping slab over the same areas. At GL 21, the structural slab steps from 16.12' to 18.22'.  No concrete curbs are called out to be installed around the knock out slab/permanent slab joint as the slab transition from 16.12' to 18.22' (i.e. the foam below the topping slabs will be visible when the knock out slabs are removed).  Please confirm no curbs are required at the knock out slab/permanent slab joint.						<b>ANSWER:</b>  Contract Doc Ref: (ASI 128 dated 12/16/14) Sheet S1-2305 Sheet A1-2865  Location: N/A  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Sheet S1-2305 and Sheet A1-2865 show 3 ea. structural deck knock out slabs between GL 20 and 22. Sheet A1-2305 shows topping slab over the same areas. At GL 21, the structural slab steps from 16.12' to 18.22'.  No concrete curbs are called out to be installed around the knock out slab/permanent slab joint as the slab transition from 16.12' to 18.22' (i.e. the foam below the topping slabs will be visible when the knock out slabs are removed).  Please confirm no curbs are required at the knock out slab/permanent slab joint.
<b>T-2150</b>	<b>SSS - ASI 128 Slab Opening Note Clarification GL29-30</b>	<b>Closed</b>	<b>CR</b>	<b>01/30/2015</b>	<b>02/09/2015</b>	<b>02/06/2015</b>
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>  Contract Doc: S1-2306 Location: Zone 4, Ground Level Gridline: C.3-D.4/29-30 Add'l Doc Ref's: CD RFI 746 SK1  In the attached CD RFI 746 SK1, the hi-lited note shows a 5" Thick Slab.  This note does not agree with the typical S10 Slab in this area.  Please clarify the hi-lited note.						<b>ANSWER:</b>  Contract Doc: S1-2306 Location: Zone 4, Ground Level Gridline: C.3-D.4/29-30 Add'l Doc Ref's: CD RFI 746 SK1  In the attached CD RFI 746 SK1, the hi-lited note shows a 5" Thick Slab.  This note does not agree with the typical S10 Slab in this area.  Please clarify the hi-lited note.



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<b>T-2151</b>	<b>BGP - Lower Concourse Wall Thickness - D226/D227</b>	<b>Closed</b>	<b>01</b>	<b>01/30/2015</b>	<b>01/30/2015</b>	<b>02/04/2015</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Claude Titcher</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: N/A Location: Lower Concourse D226/D227 Grid Line: H Add'l Doc Ref's: RFI T-2056, Attached.			Contract Doc Ref: N/A Location: Lower Concourse D226/D227 Grid Line: H Add'l Doc Ref's: RFI T-2056, Attached.			
The response to RFI T-2056 updated lower concourse wall thicknesses, however lower concourse deck D225 has already been poured with couplers cast at original thickness.			The response to RFI T-2056 updated lower concourse wall thicknesses, however lower concourse deck D225 has already been poured with couplers cast at original thickness.			
Please confirm the clouded wall running along Gridline H shall be 9-5/8" thick CMU, all other walls to be installed per RFI T-2056. See attached.			Please confirm the clouded wall running along Gridline H shall be 9-5/8" thick CMU, all other walls to be installed per RFI T-2056. See attached.			
<b>T-2152</b>	<b>SSS - PE403 &amp; PE404 Post Location and Elevation Clarifications</b>	<b>Open</b>	<b>CR</b>	<b>01/30/2015</b>	<b>02/09/2015</b>	
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc: S1-2604, 1/S1-8006 Location: Zone 2, Roof Park Gridline: D-F/16-16.9 Add'l Doc Ref's: CD RFI 747 SK1			Contract Doc: S1-2604, 1/S1-8006 Location: Zone 2, Roof Park Gridline: D-F/16-16.9 Add'l Doc Ref's: CD RFI 747 SK1			
Please see the attached hi-lited detail 1/S1-8006 on drawing S1-2604 between GL D-F/16-16.9:			Please see the attached hi-lited detail 1/S1-8006 on drawing S1-2604 between GL D-F/16-16.9:			
1) Supply the post locations. 2) Confirm the top of wall (top of post) elevations are EL. 86'-8.			1) Supply the post locations. 2) Confirm the top of wall (top of post) elevations are EL. 86'-8.			
<b>T-2153A</b>	<b>SSS - PE201 Missing Information at Roof Level</b>	<b>Closed</b>	<b>CR</b>	<b>02/01/2015</b>	<b>02/11/2015</b>	<b>02/04/2015</b>
<div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc: 5/S1-7003 Location: Zone 1, Roof Level Gridline: E.6-F/1 Add'l Doc Ref's: CD RFI 748 SK1			Contract Doc: 5/S1-7003 Location: Zone 1, Roof Level Gridline: E.6-F/1 Add'l Doc Ref's: CD RFI 748 SK1			
1. The attached CD RFI 748 SK1 shows detail 5/S1-7003			1. The attached CD RFI 748 SK1 shows detail 5/S1-			





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	with missing dimensions labeled 1a thru 1i.					7003 with missing dimensions labeled 1a thru 1i.
	Please supply missing dimensions 1a thru 1i.					Please supply missing dimensions 1a thru 1i.
T-2153B	SSS - PE201 Missing Information at Roof Level	Closed	CR	02/01/2015	02/11/2015	02/10/2015
From: Webcor Construction LP                      Stephanie Azzolino						
REQUEST:						ANSWER:
Contract Doc: 5/S1-7003, 12/S1-5003						Contract Doc: 5/S1-7003, 12/S1-5003
Location: Zone 1, Roof Level						Location: Zone 1, Roof Level
Gridline: E.6-F/1						Gridline: E.6-F/1
Add'l Doc Ref's: CD RFI 748 SK1						Add'l Doc Ref's: CD RFI 748 SK1
2. The attached CD RFI 748 SK1 shows a slab opening in detail 5/S1-7003.						2. The attached CD RFI 748 SK1 shows a slab opening in detail 5/S1-7003.
Per detail 12/S1-5003, confirm these C8x11.5 may span the distance as shown or supply alternate framing around the slab opening.						Per detail 12/S1-5003, confirm these C8x11.5 may span the distance as shown or supply alternate framing around the slab opening.
T-2154A	SSS - Conflicting Dimensions at Light Column Roof Level Connections	Open	CR	02/01/2015	02/11/2015	
From: Webcor Construction LP                      Stephanie Azzolino						
REQUEST:						ANSWER:
Contract Doc: A/S1-6005						Contract Doc: A/S1-6005
Location: Zone 3, Roof Level						Location: Zone 3, Roof Level
Gridline: F.7/22						Gridline: F.7/22
Add'l Doc Ref's: CD RFI 750 SK1, RFI T-1595						Add'l Doc Ref's: CD RFI 750 SK1, RFI T-1595
1. The attached CD RFI 750 SK1 shows highlighted ring elevations that were changed per RFI T-1595 (SK 748. CD 550).						1. The attached CD RFI 750 SK1 shows highlighted ring elevations that were changed per RFI T-1595 (SK 748. CD 550).
The noted dimensions will change as shown with the change of the ring elevation.						The noted dimensions will change as shown with the change of the ring elevation.
Confirm the information in the noted RFI remains correct.						Confirm the information in the noted RFI remains correct.
T-2154B	SSS - Conflicting Dimensions at Light Column Roof Level Connections	Open	CR	02/01/2015	02/11/2015	





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T-2154D	SSS - Conflicting Dimensions at Light Column Roof Level Connections	Open	CR	02/01/2015	02/11/2015	
From: Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  Contract Doc: 4/S1-6007 Location: Roof Level Gridline: N/A Add'l Doc Ref's: CD RFI 750 SK3, RFI T-1596  4. The highlighted dimension in detail 4/S1-6007 of the attached CD RFI 750 SK3 do not match the dimension lines confirmed in RFI T-1596 (SK 749, CD 551) item 1.  Confirm the information in the noted RFI remains correct.						<b>ANSWER:</b>  Contract Doc: 4/S1-6007 Location: Roof Level Gridline: N/A Add'l Doc Ref's: CD RFI 750 SK3, RFI T-1596  4. The highlighted dimension in detail 4/S1-6007 of the attached CD RFI 750 SK3 do not match the dimension lines confirmed in RFI T-1596 (SK 749, CD 551) item 1.  Confirm the information in the noted RFI remains correct.
T-2155A	SSS - ST405 Missing Post Information	Open	CR	02/01/2015	02/11/2015	
From: Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  Contract Doc: 1/S1-7017, 1/S1-7600 Location: Zone 2, Ground Level Gridline: F/16 Add'l Doc Ref's: CD RFI 751 SK1  1. The attached CD RFI 751 SK1 for Stair 405 detail 1/S1-7017 at Ground Level shows (2) posts along GL16.  Confirm the noted (2) posts will be supported on top of the Transfer Girder per detail 1/S1-7600 with the stiffener below the flange on one side only below the posts.						<b>ANSWER:</b>  Contract Doc: 1/S1-7017, 1/S1-7600 Location: Zone 2, Ground Level Gridline: F/16 Add'l Doc Ref's: CD RFI 751 SK1  1. The attached CD RFI 751 SK1 for Stair 405 detail 1/S1-7017 at Ground Level shows (2) posts along GL16.  Confirm the noted (2) posts will be supported on top of the Transfer Girder per detail 1/S1-7600 with the stiffener below the flange on one side only below the posts.
T-2155B	SSS - ST405 Missing Post Information	Closed	CR	02/01/2015	02/11/2015	02/05/2015
From: Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>  Contract Doc: 1/S1-7017, 1/A1-7017 Location: Zone 2, Ground Level Gridline: F/16 Add'l Doc Ref's: CD RFI 751 SK1  2. The attached CD RFI 751 SK1 for Stair 405 detail 1/S1-						<b>ANSWER:</b>  Contract Doc: 1/S1-7017, 1/A1-7017 Location: Zone 2, Ground Level Gridline: F/16 Add'l Doc Ref's: CD RFI 751 SK1  2. The attached CD RFI 751 SK1 for Stair 405 detail



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7017	at Ground Level shows (2) posts near GL F.  The location for the noted (2) posts are not shown on 1/A1-7017.  Supply the missing dimension if they are required.  3. Depending on the response to item 2, the noted post may foul the column base plate as shown in the model view.  Supply a connection detail if the post is required.  4. Depending on the response to item 2, the noted post does not have a support.  Supply the support information if the post is required.					1/S1-7017 at Ground Level shows (2) posts near GL F.  The location for the noted (2) posts are not shown on 1/A1-7017.  Supply the missing dimension if they are required.  3. Depending on the response to item 2, the noted post may foul the column base plate as shown in the model view.  Supply a connection detail if the post is required.  4. Depending on the response to item 2, the noted post does not have a support.  Supply the support information if the post is required.
<b>T-2156</b>	<b>SSS - Extra Holes Drilled on TR16 Web</b>	<b>Closed</b>	<b>CR</b>	<b>02/02/2015</b>	<b>02/12/2015</b>	<b>02/10/2015</b>
<b>From:</b> Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc: S1-2304, S1-2305 Location: Zone 2, Ground Level Gridline: F.7/16 Add'l Doc Ref's: TG0701-78.1, Shop drawing No. A603AC		Contract Doc: S1-2304, S1-2305 Location: Zone 2, Ground Level Gridline: F.7/16 Add'l Doc Ref's: TG0701-78.1, Shop drawing No. A603AC				
Six additional holes were accidentally drilled in the web plate for transfer girder A603 at gridline 16.		Six additional holes were accidentally drilled in the web plate for transfer girder A603 at gridline 16.				
The holes are located at running dimensions of 28'-2 11/16" & 28' 9 1/4" just below the set of holes that are detailed, and have a 3" spacing consistent with the rest of the group.		The holes are located at running dimensions of 28'-2 11/16" & 28' 9 1/4" just below the set of holes that are detailed, and have a 3" spacing consistent with the rest of the group.				
Please confirm it is acceptable to: (a) Leave as is or, (b) Install 6 A325 type 1 bolts in the mis-drilled holes.		Please confirm it is acceptable to: (a) Leave as is or, (b) Install 6 A325 type 1 bolts in the mis-drilled holes.				



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<b>T-2157</b>	<b>MWP - Metal Wall Panel Mockup Locations</b>	<b>Pending</b>	<b>CR</b>	<b>02/02/2015</b>	<b>02/12/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture Jonathan Flaming						
<b>REQUEST:</b>  Specification 07 42 13 Section 1.9K  Specification calls for mockups to be sized at a minimum span of 2 panel joints in each direction (approximately 20 feet x 20 feet).  The documents do not state specifically where onsite (gridline, etc.) the mockups are to be located, and if they will be left in place once approved.  Please provide location for each of the required system mockups and confirm that approved mockups will be left in place.						<b>ANSWER:</b>  Specification 07 42 13 Section 1.9K  Specification calls for mockups to be sized at a minimum span of 2 panel joints in each direction (approximately 20 feet x 20 feet).  The documents do not state specifically where onsite (gridline, etc.) the mockups are to be located, and if they will be left in place once approved.  Please provide location for each of the required system mockups and confirm that approved mockups will be left in place.
<b>T-2158</b>	<b>ELV - Telecommunications Underground Pull Boxes</b>	<b>Closed</b>	<b>CR</b>	<b>02/02/2015</b>	<b>02/12/2015</b>	<b>02/05/2015</b>
<b>From:</b> Webcor Construction LP Aseem Goyal						
<b>REQUEST:</b>  Contract Doc Ref: [(ASI 128/TE1-2602 thru TE1-2607/27 05 33)] Location: [Zone 02 - 07], Level [Roof Park] Closest Column Line Intersection is [N/A] Add'l Doc Ref's: [N/A]  The roof park level floor plan drawings has a box symbol "U" along with an identifier. Per the legend TE-0010, this symbol is a underground pull box. On sheet TE1-8013, detail 1 is the only underground pull box reference. Please confirm all park level pull boxes are indeed the detail, as shown, on sheet TE1-8013.						<b>ANSWER:</b>  Contract Doc Ref: [(ASI 128/TE1-2602 thru TE1-2607/27 05 33)] Location: [Zone 02 - 07], Level [Roof Park] Closest Column Line Intersection is [N/A] Add'l Doc Ref's: [N/A]  The roof park level floor plan drawings has a box symbol "U" along with an identifier. Per the legend TE-0010, this symbol is a underground pull box. On sheet TE1-8013, detail 1 is the only underground pull box reference. Please confirm all park level pull boxes are indeed the detail, as shown, on sheet TE1-8013.
<b>T-2159</b>	<b>ELV - E1-4603 - Jet Fountain Lighting System</b>	<b>Closed</b>	<b>CR</b>	<b>02/02/2015</b>	<b>02/12/2015</b>	<b>02/04/2015</b>
<b>From:</b> Webcor Construction LP Aseem Goyal						
<b>REQUEST:</b>  Contract Doc Ref:[ASI 128/E1-0062, E1-4603/26 50 00] Location: Zone[03 ], Level [Park] Closest Column Line Intersection is [(C,9), (G,9)] Add'l Doc's Ref's: N/A						<b>ANSWER:</b>  Contract Doc Ref:[ASI 128/E1-0062, E1-4603/26 50 00] Location: Zone[03 ], Level [Park] Closest Column Line Intersection is [(C,9), (G,9)] Add'l Doc's Ref's: N/A



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
	<p>Per ASI 128 narrative summary change description for drawing E1-4603, it states "No content revisions to sheet, previously issued version included with new date (markup)".</p> <p>1. This creates an inconstructibility issue with the jet fountain lighting system as the +/-60 type E18 fixtures would remain NIC"[along with other electrical scope] per Add4 issued on 06/20/14.</p> <p>2. In addition to the E18 fixtures, E2 is also shown as "NIC". Should this drawing be updated to include fixtures label "NIC"?</p>					<p>Per ASI 128 narrative summary change description for drawing E1-4603, it states "No content revisions to sheet, previously issued version included with new date (markup)".</p> <p>1. This creates an inconstructibility issue with the jet fountain lighting system as the +/-60 type E18 fixtures would remain NIC"[along with other electrical scope] per Add4 issued on 06/20/14.</p> <p>2. In addition to the E18 fixtures, E2 is also shown as "NIC". Should this drawing be updated to include fixtures label "NIC"?</p>
<b>T-2160</b>	<b>EAW - Aluminum Plate Panel Color</b>	<b>Pending</b>	<b>CR</b>	<b>02/02/2015</b>	<b>02/12/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture Jonathan Flaming						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: Specification 08 44 27 section 2.7 A						Contract Doc Ref: Specification 08 44 27 section 2.7 A
Location: Exterior Awning						Location: Exterior Awning
Specification calls for Prefinished Duranur XL Three-Coat system (Basis of Design), color; Pearlescent White.						Specification calls for Prefinished Duranur XL Three-Coat system (Basis of Design), color; Pearlescent White.
No paint color code is provided in the specifications for Pearlescent White and Pearlescent White was not listed in the Duranar Color List.						No paint color code is provided in the specifications for Pearlescent White and Pearlescent White was not listed in the Duranar Color List.
Provide paint color code for the Pearlescent White specified in 08 44 27 section 2.7 A.						Provide paint color code for the Pearlescent White specified in 08 44 27 section 2.7 A.



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<b>T-2161</b>	<b>SSS - Light Column Pipe to Cast Node Joints</b>	<b>Open</b>	<b>CR</b>	<b>02/02/2015</b>	<b>02/12/2015</b>	
<b>From:</b> Webcor Construction LP      Stephanie Azzolino						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Contract Doc: S1-6005, S1-6006 Location: Light Column Gridline: N/A Add'l Doc Ref's: SK RFI 1015 SK1 & SK2		Contract Doc: S1-6005, S1-6006 Location: Light Column Gridline: N/A Add'l Doc Ref's: SK RFI 1015 SK1 & SK2				
Pipe for the light column structure is specified on structural contract drawings S1-6005 and S1-6006 as API 5L Grade X52. Pipe has been ordered and delivered in conformance with this specification.		Pipe for the light column structure is specified on structural contract drawings S1-6005 and S1-6006 as API 5L Grade X52. Pipe has been ordered and delivered in conformance with this specification.				
Detail 6/S1-6006 shows the intended fit between the vertical pipe legs and the cast nodes. The castings have a machined feature on each nozzle end that is intended to slide into the pipe ends and act as a backing ring. The corresponding weld between these two components is called out as a complete joint penetration weld in the same detail. In accordance with AWS D1.1 Section 5.22.1.1, the separation between faying surfaces of butt joints landing on a backing shall not exceed 1/16".		Detail 6/S1-6006 shows the intended fit between the vertical pipe legs and the cast nodes. The castings have a machined feature on each nozzle end that is intended to slide into the pipe ends and act as a backing ring. The corresponding weld between these two components is called out as a complete joint penetration weld in the same detail. In accordance with AWS D1.1 Section 5.22.1.1, the separation between faying surfaces of butt joints landing on a backing shall not exceed 1/16".				
The allowable tolerances for API 5L Grade X52 pipe do not require concentricity (between internal and external bores) or even a constant wall thickness that would be required by AWS D1.1 to provide a complete joint penetration weld. Further, there is no requirement in the contract documents for additional machining to the internal bores of the pipe ends as would be required by AISC Code of Standard Practice.		The allowable tolerances for API 5L Grade X52 pipe do not require concentricity (between internal and external bores) or even a constant wall thickness that would be required by AWS D1.1 to provide a complete joint penetration weld. Further, there is no requirement in the contract documents for additional machining to the internal bores of the pipe ends as would be required by AISC Code of Standard Practice.				
As a result our fabricator has encountered the following conditions:		As a result our fabricator has encountered the following conditions:				
1. Pipe does not fit over the machined backing ring at the cast node nozzle end as intended without additional grinding and/or machining		1. Pipe does not fit over the machined backing ring at the cast node nozzle end as intended without additional grinding and/or machining				
2. Pipe fits over cast node backing ring; however, pipe ID is not circular or concentric, resulting in excessive gaps that exceed fit tolerance requirements for a complete joint penetration weld. Further, the lack of concentricity may result in misalignment of the pipe and cast nodes in the vertical legs.		2. Pipe fits over cast node backing ring; however, pipe ID is not circular or concentric, resulting in excessive gaps that exceed fit tolerance requirements for a complete joint penetration weld. Further, the lack of concentricity may result in misalignment of the pipe and cast nodes in the vertical legs.				
3. Ring pipe segments will be made from different pipe sections. Pipe ends from such sections will have varying and unmatched IDs again inhibiting the installation of a		3. Ring pipe segments will be made from different pipe sections. Pipe ends from such sections will have				



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	<p>tight fitting backing ring required to complete a CJP.</p> <p>For demonstration please refer to attached SK RFI 1015 SK 1 and SK 2 which shows actual dimensions for the first pipe section cut by our fabricator (p16056) and corresponding measurements from cast nodes 702-01 and 702-02</p> <p>machined backing ring (see attached sketches). The sketches compare the measured pipe ID to the cast node backing ring OD. This piece does not fit against either of the cast nodes</p> <p>To resolve these issues we see the following options / solutions:</p> <p>1. Grind internal bores of pipe ends to allow fit against castings. This option would not permit the completion of complete joint penetration welds so the CJP weld designation requirement would have to be modified to a PJP by the design team. Gaps exceeding PJP allowances would have to be built up with weld metal. Please note; however, that this option does not ensure a correct alignment between the vertical pipe and cast nodes.</p> <p>2. Machine internal bores of pipe ends to ensure a tight fit against the castings and correct alignment between the pipe and castings. The actual machining work to achieve this would be minimal; however, building machining fixtures and set up time for each pipe end would be significant.</p> <p>3. For pipe to pipe welds in the ring sections, we suggest removing the CJP weld designation.</p> <p>Please advise or provide a solution as the above options have cost and schedule impacts.</p>					<p>varying and unmatched IDs again inhibiting the installation of a tight fitting backing ring required to complete a CJP.</p> <p>For demonstration please refer to attached SK RFI 1015 SK 1 and SK 2 which shows actual dimensions for the first pipe section cut by our fabricator (p16056) and corresponding measurements from cast nodes 702-01 and 702-02</p> <p>machined backing ring (see attached sketches). The sketches compare the measured pipe ID to the cast node backing ring OD. This piece does not fit against either of the cast nodes</p> <p>To resolve these issues we see the following options / solutions:</p> <p>1. Grind internal bores of pipe ends to allow fit against castings. This option would not permit the completion of complete joint penetration welds so the CJP weld designation requirement would have to be modified to a PJP by the design team. Gaps exceeding PJP allowances would have to be built up with weld metal. Please note; however, that this option does not ensure a correct alignment between the vertical pipe and cast nodes.</p> <p>2. Machine internal bores of pipe ends to ensure a tight fit against the castings and correct alignment between the pipe and castings. The actual machining work to achieve this would be minimal; however, building machining fixtures and set up time for each pipe end would be significant.</p> <p>3. For pipe to pipe welds in the ring sections, we suggest removing the CJP weld designation.</p> <p>Please advise or provide a solution as the above options have cost and schedule impacts.</p>





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T-2162	SSS - Light Column Cast Node Ring Joint Fit-up	Void	CR	02/02/2015	02/12/2015	
From: Webcor Construction LP                      Stephanie Azzolino						
REQUEST:		ANSWER:				
Contract Doc: S1-6005, S1-6006		Contract Doc: S1-6005, S1-6006				
Location: Light Column		Location: Light Column				
Gridline: N/A		Gridline: N/A				
Add'l Doc Ref's: AWS D1.1, attached photos 1 & 2		Add'l Doc Ref's: AWS D1.1, attached photos 1 & 2				
The cast node ring has been laid out, plumbed, aligned and leveled. The ring layout has been optimized to maintain circularity as well as vertical and horizontal alignment to the greatest extent possible.		The cast node ring has been laid out, plumbed, aligned and leveled. The ring layout has been optimized to maintain circularity as well as vertical and horizontal alignment to the greatest extent possible.				
The weld joint design being used for these welds is BC-P2-GF which allows for a root opening of zero and corresponding 'As Fit-Up' tolerance of +1/8", -1/16". Please find attached AWS D1.1 extract for this joint.		The weld joint design being used for these welds is BC-P2-GF which allows for a root opening of zero and corresponding 'As Fit-Up' tolerance of +1/8", -1/16". Please find attached AWS D1.1 extract for this joint.				
Following layout and fit using the primary criteria of maintaining circularity of the ring and both vertical and horizontal alignment of the ring geometry, some root openings exceed the allowable fit up tolerance (see attached Photos 1 and 2).		Following layout and fit using the primary criteria of maintaining circularity of the ring and both vertical and horizontal alignment of the ring geometry, some root openings exceed the allowable fit up tolerance (see attached Photos 1 and 2).				
Please confirm we can proceed by "buttering" the joints at these locations (closing gaps with weld metal).		Please confirm we can proceed by "buttering" the joints at these locations (closing gaps with weld metal).				



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T-2163	SSS - Transfer Girder Details	Open	CR	02/02/2015	02/12/2015	
From: Webcor Construction LP                      Stephanie Azzolino						
REQUEST:		ANSWER:				
Contract Doc: S1-4350 thru S1-4356		Contract Doc: S1-4350 thru S1-4356				
Location: N/A		Location: N/A				
Gridline: N/A		Gridline: N/A				
Add'l Doc Ref's: SK RFI 1018 SK1-4		Add'l Doc Ref's: SK RFI 1018 SK1-4				
Based on previous discussions regarding cracks and lamellar tearing experienced by one of our fabricators we propose making the following modifications to minimize reoccurrence:		Based on previous discussions regarding cracks and lamellar tearing experienced by one of our fabricators we propose making the following modifications to minimize reoccurrence:				
1. Revise weld access holes in transfer girder web at interface with intermediate flange. See attached SK1.		1. Revise weld access holes in transfer girder web at interface with intermediate flange. See attached SK1.				
2. Replace vertical stiffeners at transfer girder ends beneath castings to a single continuous end plate. The web and flanges will be CJP'd to the end plate. See attached SK2.		2. Replace vertical stiffeners at transfer girder ends beneath castings to a single continuous end plate. The web and flanges will be CJP'd to the end plate. See attached SK2.				
3. Replace top horz plates at back of cast nodes to a single continuous plate and also remove 3" length of vertical weld at flange edge. Access hole would be extended/enlarged as shown. See SK3.		3. Replace top horz plates at back of cast nodes to a single continuous plate and also remove 3" length of vertical weld at flange edge. Access hole would be extended/enlarged as shown. See SK3.				
4. Revise wing plate to bottom flange detail as shown in SK4.		4. Revise wing plate to bottom flange detail as shown in SK4.				
Note: All detail modifications proposed would be typical for transfer girders with ground level cast nodes only		Note: All detail modifications proposed would be typical for transfer girders with ground level cast nodes only				



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T-2164	INT - Painting of All Exposed Surfaces	Open	CR	02/03/2015	02/13/2015	02/04/2015
From: Webcor Construction LP      Tram Nguyen						
REQUEST:						ANSWER:
Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 09 91 00						Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 09 91 00
Location: N/A						Location: N/A
Closest Column Line Intersection: N/A						Closest Column Line Intersection: N/A
Add'l Doc Ref's: N/A						Add'l Doc Ref's: N/A
Specification Section 09 91 00 (1.1) A.3 Painting states, "Paint all exposed surfaces whether or not colors are designated, except where the natural finish of the material is obviously intended or specifically noted as a surface not to be painted. Where items or surfaces are not specifically mentioned, paint these the same as adjacent similar materials or areas."						Specification Section 09 91 00 (1.1) A.3 Painting states, "Paint all exposed surfaces whether or not colors are designated, except where the natural finish of the material is obviously intended or specifically noted as a surface not to be painted. Where items or surfaces are not specifically mentioned, paint these the same as adjacent similar materials or areas."
Specification Section 09 91 00 provided specifications for the painting of CMU, cement plaster, and concrete.						Specification Section 09 91 00 provided specifications for the painting of CMU, cement plaster, and concrete.
There are a significant number of surfaces which are unclear as to whether or not the natural finish is to be painted (eg. concrete walls in the Train Box/Lower Concourse) or are not specifically noted as not to be painted (finishes left blank on the Room Finish Schedule).						There are a significant number of surfaces which are unclear as to whether or not the natural finish is to be painted (eg. concrete walls in the Train Box/Lower Concourse) or are not specifically noted as not to be painted (finishes left blank on the Room Finish Schedule).
Please confirm only surfaces specifically noted as being painted are to receive paint, or provide a specific list of (i.e. W-18 cement plaster, items left blank on the Room Finish Schedule, conc. & CMU walls not identified as being painted in the Train Box and Lower Concourse) items not to be painted.						Please confirm only surfaces specifically noted as being painted are to receive paint, or provide a specific list of (i.e. W-18 cement plaster, items left blank on the Room Finish Schedule, conc. & CMU walls not identified as being painted in the Train Box and Lower Concourse) items not to be painted.



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T-2165	TER - Epoxy Terrazzo Unit Price for Samples	Open	CR	02/03/2015	02/13/2015	
From: Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						<b>ANSWER:</b>
Contract Doc Ref: (ASI 128 dated 12/16/14)						Contract Doc Ref: (ASI 128 dated 12/16/14)
Specification Section 09 66 23						Specification Section 09 66 23
Specification Section 01 10 20						Specification Section 01 10 20
Location: N/A						Location: N/A
Closest Column Line Intersection: N/A						Closest Column Line Intersection: N/A
Add'l Doc Ref's: N/A						Add'l Doc Ref's: N/A
Specification Section 09 66 23 (1.4) C.1 Epoxy Terrazzo Flooring calls out for a unit price for samples.						Specification Section 09 66 23 (1.4) C.1 Epoxy Terrazzo Flooring calls out for a unit price for samples.
Specification Section 01 10 20 Unit Prices and Allowances does not identify this as a desired unit price.						Specification Section 01 10 20 Unit Prices and Allowances does not identify this as a desired unit price.
Please confirm unit pricing is not required for Epoxy Terrazzo samples.						Please confirm unit pricing is not required for Epoxy Terrazzo samples.



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T-2166	TER - Electronic File for Divider Strip Layout	Open	CR	02/03/2015	02/03/2015	
From: Webcor Construction LP      Tram Nguyen						
<b>REQUEST:</b>						
Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 09 66 23						
Location: N/A						
Closest Column Line Intersection: N/A						
Add'l Doc Ref's: N/A						
Specification Section 09 66 23 (1.3) D Epoxy Terrazzo Flooring states, "An electronic file is included in this package for installer?s use in the layout of the divider strips and colors to be used in addition to hard copy layout drawings in contract documents."						
No electronic file has been officially issued.						
Please provide the electronic file referenced in Specification Section 09 66 23 Epoxy Terrazzo Flooring.						
<b>ANSWER:</b>						
Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 09 66 23						
Location: N/A						
Closest Column Line Intersection: N/A						
Add'l Doc Ref's: N/A						
Specification Section 09 66 23 (1.3) D Epoxy Terrazzo Flooring states, "An electronic file is included in this package for installer?s use in the layout of the divider strips and colors to be used in addition to hard copy layout drawings in contract documents."						
No electronic file has been officially issued.						
Please provide the electronic file referenced in Specification Section 09 66 23 Epoxy Terrazzo Flooring.						
T-2167	FSP - Required Spacing between Sprinkler Heads	Open	CR	02/03/2015	02/13/2015	
From: Webcor Construction LP      Aseem Goyal						
<b>REQUEST:</b>						
Contract Doc Ref: A1-4506 & A1-4507/ Spec 21 13 19 1.3/B/7 Location: Bus Deck Level (Between Grids 27 & 33.5) Closest Column Line : N/A Add'l Doc Ref's : N/A						
Per Spec 211319 1.3/B/7.a closely spaced sprinklers (6' on centers) are required between 27 & 33.5. A1-4507 shows these closely spaced sprinklers at less than 6' on center. This is less than the minimum spacing between sprinklers allowed per NFPA #13 unless baffles are placed between the heads to prevent cold soldering. Baffles are not shown on sheets A1-4506 & A1-4507. Please confirm the closely spaced sprinklers will be spaced at 6' on center per specifications?						
<b>ANSWER:</b>						
Contract Doc Ref: A1-4506 & A1-4507/ Spec 21 13 19 1.3/B/7 Location: Bus Deck Level (Between Grids 27 & 33.5) Closest Column Line : N/A Add'l Doc Ref's : N/A						
Per Spec 211319 1.3/B/7.a closely spaced sprinklers (6' on centers) are required between 27 & 33.5. A1- 4507 shows these closely spaced sprinklers at less than 6' on center. This is less than the minimum spacing between sprinklers allowed per NFPA #13 unless baffles are placed between the heads to prevent cold soldering. Baffles are not shown on sheets A1-4506 & A1-4507. Please confirm the closely spaced sprinklers will be spaced at 6' on center per specifications?						



Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
T-2168	SSS - Light Column Pipe to Cast Node Joints	Void	01	02/03/2015	02/13/2015	
From: Webcor Construction LP                      Andrew Kitchen						
REQUEST:		ANSWER:				
Contract Doc Ref: S1-6005, S1-6006 Location: GL 23 Add'l Doc Ref: AWS D1.1		Contract Doc Ref: S1-6005, S1-6006 Location: GL 23 Add'l Doc Ref: AWS D1.1				
Pipe for the light column structure is specified on structural contract drawings S1-6005 and S1-6006 as API 5L Grade X52. Pipe has been ordered and delivered in conformance with this specification. Detail 6/S1-6006 shows the intended fit between the vertical pipe legs and the cast nodes. The castings have a machined feature on each nozzle end that is intended to slide into the pipe ends and act as a backing ring. The corresponding weld between these two components is called out as a complete joint penetration weld in the same detail. In accordance with AWS D1.1 Section 5.22.1.1, the separation between faying surfaces of butt joints landing on a backing shall not exceed 1/16". The allowable tolerances for API 5L Grade X52 pipe do not require concentricity (between internal and external bores) or even a constant wall thickness that would be required by AWS D1.1 to provide a complete joint penetration weld. Further, there is no requirement in the contract documents for additional machining to the internal bores of the pipe ends as would be required by AISC Code of Standard Practice.		Pipe for the light column structure is specified on structural contract drawings S1-6005 and S1-6006 as API 5L Grade X52. Pipe has been ordered and delivered in conformance with this specification. Detail 6/S1-6006 shows the intended fit between the vertical pipe legs and the cast nodes. The castings have a machined feature on each nozzle end that is intended to slide into the pipe ends and act as a backing ring. The corresponding weld between these two components is called out as a complete joint penetration weld in the same detail. In accordance with AWS D1.1 Section 5.22.1.1, the separation between faying surfaces of butt joints landing on a backing shall not exceed 1/16". The allowable tolerances for API 5L Grade X52 pipe do not require concentricity (between internal and external bores) or even a constant wall thickness that would be required by AWS D1.1 to provide a complete joint penetration weld. Further, there is no requirement in the contract documents for additional machining to the internal bores of the pipe ends as would be required by AISC Code of Standard Practice.				
As a result our fabricator has encountered the following conditions; 1. Pipe does not fit over the machined backing ring at the cast node nozzle end as intended without additional grinding and/or machining 2. Pipe fits over cast node backing ring; however, pipe ID is not circular or concentric, resulting in excessive gaps that exceed fit tolerance requirements for a complete joint penetration weld. Further, the lack of concentricity may result in misalignment of the pipe and cast nodes in the vertical legs. 3. Ring pipe segments will be made from different pipe sections. Pipe ends from such sections will have varying and unmatched IDs again inhibiting the installation of a tight fitting backing ring required to complete a CJP.		As a result our fabricator has encountered the following conditions; 1. Pipe does not fit over the machined backing ring at the cast node nozzle end as intended without additional grinding and/or machining 2. Pipe fits over cast node backing ring; however, pipe ID is not circular or concentric, resulting in excessive gaps that exceed fit tolerance requirements for a complete joint penetration weld. Further, the lack of concentricity may result in misalignment of the pipe and cast nodes in the vertical legs. 3. Ring pipe segments will be made from different pipe sections. Pipe ends from such sections will have varying and unmatched IDs again inhibiting the installation of a tight fitting backing ring required to complete a CJP.				
For demonstration please refer to attached SK RFI 1015 SK 1 and SK 2 which shows actual dimensions for the first pipe section cut by our fabricator (p16056) and corresponding measurements from cast nodes 702-01 and 702-02 machined backing ring (see attached sketches).		For demonstration please refer to attached SK RFI 1015 SK 1 and SK 2 which shows actual dimensions				





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	<p>Add'l Doc Ref's: N/A</p> <p>Specification Section 09 66 23 (1.3) E.2 Epoxy Terrazzo Flooring requires, "Coordinate installation of bicycle wheel channels in terrazzo work."</p> <p>Bicycle wheel channels are not shown in the contract documents (Sheet A1-9645 and Sheet A1-9546, for example).</p> <p>Please confirm no bicycle wheel channels are required in Phase 1.</p>					
	<p>Add'l Doc Ref's: N/A</p> <p>Specification Section 09 66 23 (1.3) E.2 Epoxy Terrazzo Flooring requires, "Coordinate installation of bicycle wheel channels in terrazzo work."</p> <p>Bicycle wheel channels are not shown in the contract documents (Sheet A1-9645 and Sheet A1-9546, for example).</p> <p>Please confirm no bicycle wheel channels are required in Phase 1.</p>					
<b>T-2170</b>	<b>SSS - Light Column Cast Node Ring Joint Fit Up</b>	<b>Void</b>	<b>01</b>	<b>02/03/2015</b>	<b>02/13/2015</b>	
<b>From:</b> Webcor Construction LP      Andrew Kitchen						
<b>REQUEST:</b>						
Contract Doc Ref: S1-6005, S1-6006 Location: GL 23 Add'l Doc Ref: AWS D1.1						
The cast node ring has been laid out, plumbed, aligned and leveled. The ring layout has been optimized to maintain circularity as well as vertical and horizontal alignment to the greatest extent possible.						
The weld joint design being used for these welds is BC-P2-GF which allows for a root opening of zero and corresponding "As Fit-Up" tolerance of +1/8", -1/16". Please find attached AWS D1.1 extract for this joint.						
Following layout and fit using the primary criteria of maintaining circularity of the ring and both vertical and horizontal alignment of the ring geometry, some root openings exceed the allowable fit up tolerance (see attached Photos 1 and 2). Please confirm we can proceed by "buttering" the joints at these locations (closing gaps with weld metal).						
<b>ANSWER:</b>						
Contract Doc Ref: S1-6005, S1-6006 Location: GL 23 Add'l Doc Ref: AWS D1.1						
The cast node ring has been laid out, plumbed, aligned and leveled. The ring layout has been optimized to maintain circularity as well as vertical and horizontal alignment to the greatest extent possible.						
The weld joint design being used for these welds is BC-P2-GF which allows for a root opening of zero and corresponding "As Fit-Up" tolerance of +1/8", -1/16". Please find attached AWS D1.1 extract for this joint.						
Following layout and fit using the primary criteria of maintaining circularity of the ring and both vertical and horizontal alignment of the ring geometry, some root openings exceed the allowable fit up tolerance (see attached Photos 1 and 2). Please confirm we can proceed by "buttering" the joints at these locations (closing gaps with weld metal).						
<b>T-2171</b>	<b>TER - Enclosure Requirements per Specification Section 09 66 23</b>	<b>Open</b>	<b>CR</b>	<b>02/03/2015</b>	<b>02/13/2015</b>	
<b>From:</b> Webcor Construction LP      Tram Nguyen						





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	<div><div>REQUEST:</div><div>Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 09 66 23</div><div>Location: N/A</div><div>Closest Column Line Intersection: N/A</div><div>Add'l Doc Ref's: N/A</div><div>Specification Section 09 66 23 (1.3) C Epoxy Terrazzo Flooring requires any space receiving terrazzo flooring to be completely enclosed.</div><div>Some areas receiving terrazzo are open air spaces (Public Lobby 02249, for example) and will require temporary enclosure. (Note: If the "W" system at the Grand Hall is not in place at the time of the terrazzo installation, it will also require temporary enclosure.)</div><div>Please confirm environmental conditions required by the manufacturer's written installation are to be followed, and enclosure is not necessary unless required by the manufacturer.</div></div>					
	<div><div>ANSWER:</div><div>Contract Doc Ref: (ASI 128 dated 12/16/14) Specification Section 09 66 23</div><div>Location: N/A</div><div>Closest Column Line Intersection: N/A</div><div>Add'l Doc Ref's: N/A</div><div>Specification Section 09 66 23 (1.3) C Epoxy Terrazzo Flooring requires any space receiving terrazzo flooring to be completely enclosed.</div><div>Some areas receiving terrazzo are open air spaces (Public Lobby 02249, for example) and will require temporary enclosure. (Note: If the "W" system at the Grand Hall is not in place at the time of the terrazzo installation, it will also require temporary enclosure.)</div><div>Please confirm environmental conditions required by the manufacturer's written installation are to be followed, and enclosure is not necessary unless required by the manufacturer.</div></div>					
T-2172	SSS - Transfer Girder Details	Void	01	02/03/2015	02/13/2015	
	<div>From: Webcor Construction LP</div> <div>Andrew Kitchen</div> <div><div>REQUEST:</div><div>Contract Doc Ref: S1-4350 - S1-4356 Add'l Doc Ref: Attached sketch SK2 - SK4</div><div>Based on previous discussions regarding cracks and lamellar tearing experienced by one of our fabricators we propose making the following modifications to minimize reoccurrence; 1. Revise weld access holes in transfer girder web at interface with intermediate flange. See attached SK1 2. Replace vertical stiffeners at transfer girder ends beneath castings to a single continuous end plate. The web and flanges will be CJP'd to the end plate. See attached SK2 3. Replace top horizontal plates at back of cast nodes to a single continuous plate and also remove 3" length of vertical weld at flange edge. Access hole would be</div></div>					
	<div><div>ANSWER:</div><div>Contract Doc Ref: S1-4350 - S1-4356 Add'l Doc Ref: Attached sketch SK2 - SK4</div><div>Based on previous discussions regarding cracks and lamellar tearing experienced by one of our fabricators we propose making the following modifications to minimize reoccurrence; 1. Revise weld access holes in transfer girder web at interface with intermediate flange. See attached SK1 2. Replace vertical stiffeners at transfer girder ends beneath castings to a single continuous end plate. The web and flanges will be CJP'd to the end plate. See attached SK2 3. Replace top horizontal plates at back of cast nodes to a single continuous plate and also remove 3" length of vertical weld at flange edge. Access hole would be</div></div>					



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T-2173	<p>extended/enlarged as shown. See SK3 4. Revise wing plate to bottom flange detail as shown in SK4</p> <p>Note: All detail modifications proposed would be typical for transfer girders with ground level cast nodes only</p> <p>Please advise.</p> <p><b>RPL - Utility Corridor Details</b></p> <p><b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: ASI 128 - L1-1602 through L1-1607 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>L1-1602 through L1-1607 show an outline for the main pathway of the utility corridor and vault. However, there are no details for the main utility corridor or any parameters for branches from the main corridor.</p> <p>Please provide details for the utility corridor and all parameters for branching from the utility corridor to security pylons, etc. Include details and parameters for trenching, backfill, coverage, routing requirements, trench dimensions, additional pull boxes, etc.</p>	Open	CR	02/03/2015	02/13/2015	
	<p>extended/enlarged as shown. See SK3 4. Revise wing plate to bottom flange detail as shown in SK4</p> <p>Note: All detail modifications proposed would be typical for transfer girders with ground level cast nodes only</p> <p>Please advise.</p> <p><b>ANSWER:</b></p> <p>Contract Doc Ref: ASI 128 - L1-1602 through L1-1607 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A</p> <p>L1-1602 through L1-1607 show an outline for the main pathway of the utility corridor and vault. However, there are no details for the main utility corridor or any parameters for branches from the main corridor.</p> <p>Please provide details for the utility corridor and all parameters for branching from the utility corridor to security pylons, etc. Include details and parameters for trenching, backfill, coverage, routing requirements, trench dimensions, additional pull boxes, etc.</p>					



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<b>T-2174</b>	<b>RPL - Utility Corridor Distribution</b>	<b>Open</b>	<b>CR</b>	<b>02/03/2015</b>	<b>02/13/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - L1-1602 through L1-1607 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - L1-1602 through L1-1607 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
L1-1602 through L1-1607 show an outline for the main pathway of the utility corridor and vault. However, there are no details for the main utility corridor or any parameters for branches from the main corridor.			L1-1602 through L1-1607 show an outline for the main pathway of the utility corridor and vault. However, there are no details for the main utility corridor or any parameters for branches from the main corridor.			
Provide the method and parameters for distribution of branching from the main utility corridor. Is the design intent to daisy chain security pylons, home run each pylon, etc.? Include parameters outlining the design intent of branching distribution from the main utility corridor, additional pull boxes in landscaping, security requirements, etc.			Provide the method and parameters for distribution of branching from the main utility corridor. Is the design intent to daisy chain security pylons, home run each pylon, etc.? Include parameters outlining the design intent of branching distribution from the main utility corridor, additional pull boxes in landscaping, security requirements, etc.			
<b>T-2175</b>	<b>RPL - Utility Corridor Access Point Details</b>	<b>Open</b>	<b>CR</b>	<b>02/03/2015</b>	<b>02/13/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - L1-1602 through L1-1607, L1-7670, L1-7671, L1-7672 and L1-7673 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - L1-1602 through L1-1607, L1-7670, L1-7671, L1-7672 and L1-7673 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
L1-1602 through L1-1607 show an outline for the main pathway of the utility corridor and vault. However, there are no details for the main utility corridor or any parameters for branches from the main corridor.			L1-1602 through L1-1607 show an outline for the main pathway of the utility corridor and vault. However, there are no details for the main utility corridor or any parameters for branches from the main corridor.			
Distribution from the main utility corridor will likely require additional pull boxes/vaults which will need to be accessed. Confirm that Utility Vault details on L1-7670, L1-7671, L1-7672 and L1-7673 should be used for any additional access points required in the main utility corridor and utility corridor branches for distribution throughout the Roof Park. If not, provide details for additional pull boxes/vaults which will be required for distribution.			Distribution from the main utility corridor will likely require additional pull boxes/vaults which will need to be accessed. Confirm that Utility Vault details on L1-7670, L1-7671, L1-7672 and L1-7673 should be used for any additional access points required in the main utility corridor and utility corridor branches for distribution throughout the Roof Park. If not, provide details for additional pull boxes/vaults which will be required for distribution.			







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T-2179	FSP - Fire Storage Tank Capacity	Open	CR	02/04/2015	02/14/2015	
From: Webcor Construction LP                      Aseem Goyal						
<b>REQUEST:</b>						
Contract Doc Ref: P1-5040, 21 13 19-3 Location: Train Platform Level Closest Column Line Intersection: Between B,C.3 & 4,5 Add'l Doc Ref's: N/A						
P1-5040 calls out the Fire Water Storage Tank as a 45,000 Gallon Tank. The Tank should be sized for the worst case hazard classification of the building. Per Specification 211319-3, the Bus Deck Level is Classified as Extra Hazard Group 1 with a design density of .24 over 4,000 sq. ft. If sizing the tank for this density it should hold 131,400 gallons.						
4,000 x .24 = 960 gallons 960 gallons + 500 gallons hose allowance = 1,460 gallons 1,460 gal. x 90 minutes duration = 131,400 gallons						
Please confirm water storage tank sizing?						
<b>ANSWER:</b>						
Contract Doc Ref: P1-5040, 21 13 19-3 Location: Train Platform Level Closest Column Line Intersection: Between B,C.3 & 4,5 Add'l Doc Ref's: N/A						
P1-5040 calls out the Fire Water Storage Tank as a 45,000 Gallon Tank. The Tank should be sized for the worst case hazard classification of the building. Per Specification 211319-3, the Bus Deck Level is Classified as Extra Hazard Group 1 with a design density of .24 over 4,000 sq. ft. If sizing the tank for this density it should hold 131,400 gallons.						
4,000 x .24 = 960 gallons 960 gallons + 500 gallons hose allowance = 1,460 gallons 1,460 gal. x 90 minutes duration = 131,400 gallons						
Please confirm water storage tank sizing?						
T-2180	SSS - Shop Splice on TG 1.4 & 2	Open	CR	02/04/2015	02/14/2015	
From: Webcor Construction LP                      Stephanie Azzolino						
<b>REQUEST:</b>						
Contract Doc: S1-2302, S1-4300 Location: Zone 1, Ground Level Gridline: D.4-F.7/1.4-2 Contract Doc Ref's: TG0701-87.1, Shop Drawings No. 652AC, 653AB, 676AB						
See the attached SK RFI 1021 SK1-3.						
Please confirm it is acceptable to introduce shop web and flange splices as indicated on the attached girder segments on GL 1.4 & 2.						
<b>ANSWER:</b>						
Contract Doc: S1-2302, S1-4300 Location: Zone 1, Ground Level Gridline: D.4-F.7/1.4-2 Contract Doc Ref's: TG0701-87.1, Shop Drawings No. 652AC, 653AB, 676AB						
See the attached SK RFI 1021 SK1-3.						
Please confirm it is acceptable to introduce shop web and flange splices as indicated on the attached girder segments on GL 1.4 & 2.						



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<b>T-2181</b>	<b>SSS - Deletion of Lower Nuts on Anchor Bolts</b>	<b>Closed</b>	<b>CR</b>	<b>02/04/2015</b>	<b>02/14/2015</b>	<b>02/06/2015</b>
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div><div><b>REQUEST:</b> Contract Doc: 3,5,8/S1-5051 Location: Zone1-4, Lower Concourse Gridline: All Add'l Doc Ref's: N/A  Skanska requests permission to eliminate the lower nuts on the threadbar anchor bolts securing the columns at the lower concourse level.  Based upon discussions with the owner design team, it was determined that the anchor bolts are only intended to resist uplift. Skanska employs shims under the baseplate of the columns for support and leveling, therefore the lower nuts are unnecessary and of no need to us.  Please confirm this is acceptable.</div><div><b>ANSWER:</b> Contract Doc: 3,5,8/S1-5051 Location: Zone1-4, Lower Concourse Gridline: All Add'l Doc Ref's: N/A  Skanska requests permission to eliminate the lower nuts on the threadbar anchor bolts securing the columns at the lower concourse level.  Based upon discussions with the owner design team, it was determined that the anchor bolts are only intended to resist uplift. Skanska employs shims under the baseplate of the columns for support and leveling, therefore the lower nuts are unnecessary and of no need to us.  Please confirm this is acceptable.</div></div>						
<b>T-2182</b>	<b>SSS - Bowed Deck Due to Cambered Beams</b>	<b>Open</b>	<b>CR</b>	<b>02/04/2015</b>	<b>02/14/2015</b>	
<div><div><div><b>From:</b> Webcor Construction LP</div><div>Stephanie Azzolino</div></div><div><b>REQUEST:</b> Contract Doc: S1-2303, S1-2304 Location: Zone 2, Ground Level Gridline: C.3-D/11-12 Add'l Doc Ref's: TG0701-074.1, SK RFI 1022 SK1  At locations where deck panels span over heavily cambered beams, the panels bow due to the camber and do not bear on adjacent flat beams.  A typical example at GL11-12/D where the deck panels spans from A4814 over A4807 (with 1-1/2" camber) and do not bear on A4808. This is typical at several locations throughout the building. See attached SK1.  Please confirm it is acceptable to relief cut through the high flutes over the cambered beam to create bearing on adjacent beams for stud welding.</div><div><b>ANSWER:</b> Contract Doc: S1-2303, S1-2304 Location: Zone 2, Ground Level Gridline: C.3-D/11-12 Add'l Doc Ref's: TG0701-074.1, SK RFI 1022 SK1  At locations where deck panels span over heavily cambered beams, the panels bow due to the camber and do not bear on adjacent flat beams.  A typical example at GL11-12/D where the deck panels spans from A4814 over A4807 (with 1-1/2" camber) and do not bear on A4808. This is typical at several locations throughout the building. See attached SK1.  Please confirm it is acceptable to relief cut through the high flutes over the cambered beam to create bearing on adjacent beams for stud welding.</div></div>						
<b>T-2183</b>	<b>SSS - Step in Slab Closure at GL19.1</b>	<b>Closed</b>	<b>CR</b>	<b>02/04/2015</b>	<b>02/14/2015</b>	<b>02/09/2015</b>

T-2184







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	<p>The answer to RFI P1-0476 has not been incorporated into ASI 128. P1-0476 was answered before the 10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.</p> <p>Please incorporate the answer to P1-0476 into the specifications.</p>					<p>The answer to RFI P1-0476 has not been incorporated into ASI 128. P1-0476 was answered before the 10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.</p> <p>Please incorporate the answer to P1-0476 into the specifications.</p>
T-2188	<p><b>RPL - Incorporate P1-0477 Answer</b></p> <p><b>From:</b> Webcor/Obayashi Joint Venture Sihaya Roselle</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: ASI 128 - 32 15 10 1.3G, 32 14 40 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0477</p> <p>The answer to RFI P1-0477 has not been incorporated into ASI 128. P1-0477 was answered before the 10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.</p> <p>Please incorporate the answer to P1-0477 into the specifications.</p>	Open	CR	02/05/2015	02/15/2015	<p><b>ANSWER:</b></p> <p>Contract Doc Ref: ASI 128 - 32 15 10 1.3G, 32 14 40 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0477</p> <p>The answer to RFI P1-0477 has not been incorporated into ASI 128. P1-0477 was answered before the 10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.</p> <p>Please incorporate the answer to P1-0477 into the specifications.</p>
T-2189	<p><b>RPL - Incorporate P1-0478 Answer</b></p> <p><b>From:</b> Webcor/Obayashi Joint Venture Sihaya Roselle</p> <p><b>REQUEST:</b></p> <p>Contract Doc Ref: ASI 128 - 32 15 10 2.2.B Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0478</p> <p>The answer to RFI P1-0478 has not been incorporated into ASI 128. P1-0478 was answered before the 10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.</p> <p>Please incorporate the answer to P1-0478 into the specifications.</p>	Open	CR	02/05/2015	02/15/2015	<p><b>ANSWER:</b></p> <p>Contract Doc Ref: ASI 128 - 32 15 10 2.2.B Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0478</p> <p>The answer to RFI P1-0478 has not been incorporated into ASI 128. P1-0478 was answered before the 10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.</p> <p>Please incorporate the answer to P1-0478 into the specifications.</p>



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<b>T-2190</b>	<b>RPL - Incorporate P1-0480 Answer</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>  Contract Doc Ref: ASI 128 - 32 15 10 3.12.A.2 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0480  The answer to RFI P1-0480 has not been incorporated into ASI 128. P1-0480 was answered before the 10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.  Please incorporate the answer to P1-0480 into the specifications.						<b>ANSWER:</b>  Contract Doc Ref: ASI 128 - 32 15 10 3.12.A.2 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0480  The answer to RFI P1-0480 has not been incorporated into ASI 128. P1-0480 was answered before the 10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.  Please incorporate the answer to P1-0480 into the specifications.
<b>T-2191</b>	<b>RPL - Incorporate P1-0481 Answer</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>  Contract Doc Ref: ASI 128 - 32 15 10 3.12.B.2 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0481  The answer to RFI P1-0481 has not been incorporated into ASI 128. P1-0481 was answered before the 10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.  Please incorporate the answer to P1-0481 into the specifications.						<b>ANSWER:</b>  Contract Doc Ref: ASI 128 - 32 15 10 3.12.B.2 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0481  The answer to RFI P1-0481 has not been incorporated into ASI 128. P1-0481 was answered before the 10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.  Please incorporate the answer to P1-0481 into the specifications.
<b>T-2192</b>	<b>RPL - Incorporate P1-0493 Answer</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>  Contract Doc Ref: ASI 128 - 32 14 40 3.3.A.1 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0493  The answer to RFI P1-0493 has not been incorporated into ASI 128. P1-0493 was answered before the						<b>ANSWER:</b>  Contract Doc Ref: ASI 128 - 32 14 40 3.3.A.1 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0493  The answer to RFI P1-0493 has not been incorporated into ASI 128. P1-0493 was answered before the



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T-2193	10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.  Please incorporate the answer to P1-0493 into the specifications.	Open	CR	02/05/2015	02/15/2015	10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.  Please incorporate the answer to P1-0493 into the specifications.
	<b>RPL - Incorporate P1-0651 Answer</b>  <b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle  <b>REQUEST:</b> Contract Doc Ref: ASI 128 - 32 15 10, 2.1.A Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0651  The answer to RFI P1-0651 has not been incorporated into ASI 128 as stated it would be.  Please incorporate the answer to P1-0651 into the specifications.					<b>ANSWER:</b> Contract Doc Ref: ASI 128 - 32 15 10, 2.1.A Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0651  The answer to RFI P1-0651 has not been incorporated into ASI 128 as stated it would be.  Please incorporate the answer to P1-0651 into the specifications.
T-2194	10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.  Please incorporate the answer to P1-0493 into the specifications.	Open	CR	02/05/2015	02/15/2015	10/17/2014 cut off date and should be included per the ASI 128 Revision Narrative, section 1.1.A.  Please incorporate the answer to P1-0493 into the specifications.
	<b>RPL - Incorporate P1-0665 Answer</b>  <b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle  <b>REQUEST:</b> Contract Doc Ref: ASI 128 - 03 33 12, 1.4.A.4 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0665  The answer to RFI P1-0665 has not been incorporated into ASI 128 as stated it would be.  Please incorporate the answer to P1-0665 into the specifications.					<b>ANSWER:</b> Contract Doc Ref: ASI 128 - 03 33 12, 1.4.A.4 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0665  The answer to RFI P1-0665 has not been incorporated into ASI 128 as stated it would be.  Please incorporate the answer to P1-0665 into the specifications.



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<b>T-2195</b>	<b>RPL - Incorporate P1-0666 Answer</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>  Contract Doc Ref: ASI 128 - 03 33 12, 1.4.A.5 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0666  The answer to RFI P1-0666 has not been incorporated into ASI 128 as stated it would be.  Please incorporate the answer to P1-0666 into the specifications.						<b>ANSWER:</b>  Contract Doc Ref: ASI 128 - 03 33 12, 1.4.A.5 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: P1-0666  The answer to RFI P1-0666 has not been incorporated into ASI 128 as stated it would be.  Please incorporate the answer to P1-0666 into the specifications.
<b>T-2196</b>	<b>RPL - Site Precast Design Reference Sample</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>  Contract Doc Ref: ASI 128 - 03 45 00, 1.3.C Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  03 45 00, 1.3.C states that a Design Reference Sample of approved architectural precast concrete color, finish, and texture will be pre-approved by a TJPA Representative.  Confirm that this pre-approved design reference sample will be provided to the trade subcontractor performing this scope of work. Which precast elements will this design reference sample be provided for?						<b>ANSWER:</b>  Contract Doc Ref: ASI 128 - 03 45 00, 1.3.C Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  03 45 00, 1.3.C states that a Design Reference Sample of approved architectural precast concrete color, finish, and texture will be pre-approved by a TJPA Representative.  Confirm that this pre-approved design reference sample will be provided to the trade subcontractor performing this scope of work. Which precast elements will this design reference sample be provided for?
<b>T-2197</b>	<b>RPL - MRc5 Option 2 Language Required - CMU</b>	<b>Pending</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>  Contract Doc Ref: ASI 128 - 04 22 00, 2.3.A.1.a. Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  MRc5 Option 2 language is missing from 04 22 00,						<b>ANSWER:</b>  Contract Doc Ref: ASI 128 - 04 22 00, 2.3.A.1.a. Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  MRc5 Option 2 language is missing from 04 22 00,



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2.3.A.1.a.	Provide MRc5 Option 2 language for 04 22 00, 2.3.A.1.a.		2.3.A.1.a.			
			Provide MRc5 Option 2 language for 04 22 00, 2.3.A.1.a.			
<b>T-2198</b>	<b>FSP - Clarification on Fire Protection Zoning</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor Construction LP      Aseem Goyal						
<b>REQUEST:</b> Contract Doc Ref: P1-2003 & P1-2004 Location: Ground Level & 2nd Level Throughout Closest Column Line: N/A Add'l Doc Ref's: N/A  P1-2003 & P1-2004 show the Zone boundaries for the project. The Zoning plan for the 2nd Level shown on P1-2004 includes large areas of the building that protect the Ground Floor Level (See dark blue areas on attached P1-2003). In our experience, the Fire Department requires a sprinkler system be zoned from the floor level it protects. Please confirm that the areas of the 2nd level protecting the Ground Floor should be zoned from the Ground Floor.			<b>ANSWER:</b> Contract Doc Ref: P1-2003 & P1-2004 Location: Ground Level & 2nd Level Throughout Closest Column Line: N/A Add'l Doc Ref's: N/A  P1-2003 & P1-2004 show the Zone boundaries for the project. The Zoning plan for the 2nd Level shown on P1-2004 includes large areas of the building that protect the Ground Floor Level (See dark blue areas on attached P1-2003). In our experience, the Fire Department requires a sprinkler system be zoned from the floor level it protects. Please confirm that the areas of the 2nd level protecting the Ground Floor should be zoned from the Ground Floor.			
<b>T-2199</b>	<b>RPL - Circular Planter Field Mockup</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b> Contract Doc Ref: ASI 128 - 04 43 00, 1.5.B and 1.5.C Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  Stone quarry mockups are required for both Amphitheater Stone Slab Stairs and Circular Planters. Field mockup is required for the stone stair.  Confirm that a field mockup is not required for the Circular Planters. If a field mockup is required for Circular Planters, provide the area in the drawings to be mocked up in the field.			<b>ANSWER:</b> Contract Doc Ref: ASI 128 - 04 43 00, 1.5.B and 1.5.C Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  Stone quarry mockups are required for both Amphitheater Stone Slab Stairs and Circular Planters. Field mockup is required for the stone stair.  Confirm that a field mockup is not required for the Circular Planters. If a field mockup is required for Circular Planters, provide the area in the drawings to be mocked up in the field.			





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	Confirm that this requirement also applies to all plant rails and elliptical rails.					Confirm that this requirement also applies to all plant rails and elliptical rails.
<b>T-2203</b>	<b>RPL - Metal Fabrications Protective Covering</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - 05 60 00, 3.7.A Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - 05 60 00, 3.7.A Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
05 60 00, 3.7.A states to apply protective coverings to prevent damage of metal fabrications.			05 60 00, 3.7.A states to apply protective coverings to prevent damage of metal fabrications.			
Confirm that this protective covering is meant to be a temporary protective covering during construction.			Confirm that this protective covering is meant to be a temporary protective covering during construction.			
<b>T-2204</b>	<b>RPL - Bond Breaker Samples</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - 07 92 14, 1.4.C.2 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - 07 92 14, 1.4.C.2 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
Bond Breaker samples are required per 07 92 14, 1.4.C.2.			Bond Breaker samples are required per 07 92 14, 1.4.C.2.			
Provide the number of samples and any criteria required for the bond breaker samples.			Provide the number of samples and any criteria required for the bond breaker samples.			





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<b>T-2205</b>	<b>RPL - Seismic Joint Cover Blast Criteria</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - 08 05 13, 1.6.B. and 05 60 00, 2.3.M. Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - 08 05 13, 1.6.B. and 05 60 00, 2.3.M. Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
Blast criteria for seismic joint covers is referenced in 08 05 13, 1.6.B.			Blast criteria for seismic joint covers is referenced in 08 05 13, 1.6.B.			
Confirm that blast criteria and performance criteria outlined in 08 05 13, 1.6.B apply for the seismic joint cover in specification 05 60 00, 2.3.M.			Confirm that blast criteria and performance criteria outlined in 08 05 13, 1.6.B apply for the seismic joint cover in specification 05 60 00, 2.3.M.			
<b>T-2206</b>	<b>RPL - Park Level Seismic Joint Cover</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - 05 60 00, 2.3.M., 07 09 13 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - 05 60 00, 2.3.M., 07 09 13 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
Park Level Seismic Joint Cover is specified in 05 60 00, 2.3.M, separately from 07 09 13.			Park Level Seismic Joint Cover is specified in 05 60 00, 2.3.M, separately from 07 09 13.			
Confirm that the Roof Park Seismic Joint Cover specified in 05 60 00, 2.3.M. is provided separate to the product specified in 07 09 13.			Confirm that the Roof Park Seismic Joint Cover specified in 05 60 00, 2.3.M. is provided separate to the product specified in 07 09 13.			
<b>T-2207</b>	<b>RPL - Site Laminate Glass Graffiti-Resistance</b>	<b>Open</b>	<b>CR</b>	<b>02/05/2015</b>	<b>02/15/2015</b>	
<b>From:</b> Webcor/Obayashi Joint Venture      Sihaya Roselle						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - 08 81 00, 08 87 00 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - 08 81 00, 08 87 00 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
08 81 00 Site Laminate Glass Glazing does not reference 08 87 00 Graffiti-Resistant Glass Film.			08 81 00 Site Laminate Glass Glazing does not reference 08 87 00 Graffiti-Resistant Glass Film.			



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	Confirm that there are no requirements for graffiti-resistant glass film for 08 81 00.					Confirm that there are no requirements for graffiti-resistant glass film for 08 81 00.
T-2208	RPL - Mortar-Set Stone Paving Alternate	Open	CR	02/05/2015	02/15/2015	
	From: Webcor/Obayashi Joint Venture      Sihaya Roselle					
	REQUEST: Contract Doc Ref: ASI 128 - 32 14 41, 01 10 30/APE Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  Mortar-Set Stone Paving alternates have been removed per specification 01 10 30/APE. 32 14 41 shows deduct bid alternates in sections 2.2.A.2, 2.2.B.2, 2.2.C.2, 2.2.D.2, 2.3.A.2, 2.3.B.2, 2.3.C.2, 2.3.D.2.  Confirm that there is no deduct bid alternates for mortar-set stone paving per specification 01 10 30/APE. If so, remove deduct bid alternates shown in 32 14 41.					ANSWER: Contract Doc Ref: ASI 128 - 32 14 41, 01 10 30/APE Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  Mortar-Set Stone Paving alternates have been removed per specification 01 10 30/APE. 32 14 41 shows deduct bid alternates in sections 2.2.A.2, 2.2.B.2, 2.2.C.2, 2.2.D.2, 2.3.A.2, 2.3.B.2, 2.3.C.2, 2.3.D.2.  Confirm that there is no deduct bid alternates for mortar-set stone paving per specification 01 10 30/APE. If so, remove deduct bid alternates shown in 32 14 41.
T-2209	RPL - Weed Barrier Installation Requirements	Open	CR	02/05/2015	02/15/2015	
	From: Webcor/Obayashi Joint Venture      Sihaya Roselle					
	REQUEST: Contract Doc Ref: ASI 128 - 32 15 00, L1-9667 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  Specification 32 15 00 includes requirements for weed barrier. There are no installation requirements for weed barrier.  Provide installation requirements for weed barrier. Clarify if weed barrier is synonymous with root inhibitor called out in the drawings, for example on L1-9667.					ANSWER: Contract Doc Ref: ASI 128 - 32 15 00, L1-9667 Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A  Specification 32 15 00 includes requirements for weed barrier. There are no installation requirements for weed barrier.  Provide installation requirements for weed barrier. Clarify if weed barrier is synonymous with root inhibitor called out in the drawings, for example on L1-9667.



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T-2210	RPL - MRc5 Option 2 Language Required - Play Area Surfacing	Open	CR	02/05/2015	02/15/2015	
<div><div>From: Webcor/Obayashi Joint Venture</div><div>Sihaya Roselle</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: ASI 128 - 32 18 16, 1.4.B.2 and 2.1.B Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			Contract Doc Ref: ASI 128 - 32 18 16, 1.4.B.2 and 2.1.B Location: Roof Park Closest Column Line Intersection: N/A Add'l Doc Ref's: N/A			
MRc5 Option 2 language is missing from 32 18 16, 1.4.B.2 and 2.1.B			MRc5 Option 2 language is missing from 32 18 16, 1.4.B.2 and 2.1.B			
Provide MRc5 Option 2 language for 32 18 16, 1.4.B.2 and 2.1.B			Provide MRc5 Option 2 language for 32 18 16, 1.4.B.2 and 2.1.B			
T-2211	GFS - Glass Floor Vibration Requirements	Open	CR	02/06/2015	02/16/2015	
<div><div>From: Webcor/Obayashi Joint Venture</div><div>Jonathan Flaming</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: 08 88 53			Contract Doc Ref: 08 88 53			
Surfaces supporting functions in the Roof Park Level are subject to dynamic effects from Park users. The current glass floor specification does not seem to address any dynamic performance with respect to pedestrian comfort. There is no natural frequency, nor any kind of vibration criteria mentioned in the Specifications.			Surfaces supporting functions in the Roof Park Level are subject to dynamic effects from Park users. The current glass floor specification does not seem to address any dynamic performance with respect to pedestrian comfort. There is no natural frequency, nor any kind of vibration criteria mentioned in the Specifications.			
Please confirm there is no vibration criteria required for the structural design of the glass floor at Roof Park Level.			Please confirm there is no vibration criteria required for the structural design of the glass floor at Roof Park Level.			
T-2212	BGP - Concrete Ramps at Lower Concourse Step - GL9-11 N/S	Open	CR	02/06/2015	02/16/2015	
<div><div>From: Webcor Construction LP</div><div>Claude Titche</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Contract Doc Ref: A1-2203 Location: Lower Concourse Grid Line: 9-11 Add'l Doc Ref's: Attached			Contract Doc Ref: A1-2203 Location: Lower Concourse Grid Line: 9-11 Add'l Doc Ref's: Attached			
See attached drawings.			See attached drawings.			



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T-2213	<b>SCS - Concrete Beam B78 at GL 9 Conflict</b>  From: Webcor Construction LP                      Claude Titcher  <b>REQUEST:</b> Contract Doc Ref: S1-2303, 8/S1-3701 Location: Ground Level Grid Line: GL9-H Add'l Doc Ref's: Attached  Reference sheet SI-2303 and Section 8/S1-3701 for Concrete beam B78 located along GL 9 between GLG and H.  S1-2303 shows B78 terminating at the face of concrete beams B87 and B88. Section 8/S1-3701 shows B78 bearing on the steel transfer beam and extending past B87 and B88.  1. Please confirm whether B78 terminates or extends past B87 and B88.  2. If the beam extends past B87/88 per Section 8/S1-3701, please provide the distance to which B78 extends, from south side of B87/88 to end of beam.  3. If the beam terminates at B87 and B88, please revise Section 8/S1-3701 rebar details to match the new limits.	Open	01	02/09/2015	02/19/2015	Sheet A1-2203 shows a 6" thick concrete ramp at the North and South foundation walls between GL 9 and 11. These ramps are not shown on either slab edge or structural drawings.  Please provide drawings detailing slab edge, rebar size, rebar spacing, finish requirements, and knee wall width.
						Sheet A1-2203 shows a 6" thick concrete ramp at the North and South foundation walls between GL 9 and 11. These ramps are not shown on either slab edge or structural drawings.  Please provide drawings detailing slab edge, rebar size, rebar spacing, finish requirements, and knee wall width.
T-2214	<b>SCS - Shrinkage and Temperature Bars Lap Location at One-Way Slabs</b>	Open	CR	02/09/2015	02/19/2015	



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<div><div><div><b>From:</b> Webcor Construction LP</div><div><b>REQUEST:</b> Contract Doc Ref: S1-3500 Location: N/A Grid Line: N/A Add'l Doc Ref's: Attached  Please refer to the Shrinkage and Temperature Bars (S&amp;T Bars) in details 1&amp;2/S1-3500. There are no lap location instructions, i.e. to stagger or not to stagger, given for the S&amp;T Bars.  Please confirm that no stagger is required in the top/bottom S&amp;T bars. If a stagger is required, please provide a detail and/or a description of how the top/bottom S&amp;T Bars are to stagger.</div><div><b>ANSWER:</b> Contract Doc Ref: S1-3500 Location: N/A Grid Line: N/A Add'l Doc Ref's: Attached  Please refer to the Shrinkage and Temperature Bars (S&amp;T Bars) in details 1&amp;2/S1-3500. There are no lap location instructions, i.e. to stagger or not to stagger, given for the S&amp;T Bars.  Please confirm that no stagger is required in the top/bottom S&amp;T bars. If a stagger is required, please provide a detail and/or a description of how the top/bottom S&amp;T Bars are to stagger.</div></div></div>						
<b>T-2215</b>	<b>ELV - Low and Line Voltage Floor Box layout Relative to Finish Floor Pattern</b>	<b>Open</b>	<b>CR</b>	<b>02/10/2015</b>	<b>02/20/2015</b>	
<div><div><div><b>From:</b> Webcor Construction LP</div><div><b>REQUEST:</b> Ref: ASI # 128 Sheet A1-9545, E1-2305 &amp; TE1-2305 dated 12/16/14 Location: Grand Hall (Floor Finish - Terrazzo) Closest Column Line: N/A Add'l Doc Ref's : N/A  The TG10.4 package is currently in shop drawing production and will shortly begin ground floor detailing of the Grand Hall. Relative to the finish terrazzo floor design, are there any other spatial/dimensional requirements or floor box finish/trim requirements for Electrical, Telecommunication or Security floor boxes beyond that shown and/or specified in ASI # 128 Documents? Additionally, A1-9545 does not reflect all require floor box locations (see grid Line 19.4 on E1-2305 &amp; TE1-2305). Please provide updated A1-9545 that reflects all Low and Line voltage floor box locations required per ASI # 128.</div><div><b>ANSWER:</b> Ref: ASI # 128 Sheet A1-9545, E1-2305 &amp; TE1-2305 dated 12/16/14 Location: Grand Hall (Floor Finish - Terrazzo) Closest Column Line: N/A Add'l Doc Ref's : N/A  The TG10.4 package is currently in shop drawing production and will shortly begin ground floor detailing of the Grand Hall. Relative to the finish terrazzo floor design, are there any other spatial/dimensional requirements or floor box finish/trim requirements for Electrical, Telecommunication or Security floor boxes beyond that shown and/or specified in ASI # 128 Documents? Additionally, A1-9545 does not reflect all require floor box locations (see grid Line 19.4 on E1-2305 &amp; TE1-2305). Please provide updated A1-9545 that reflects all Low and Line voltage floor box locations required per ASI # 128.</div></div></div>						
<b>T-2216</b>	<b>ELV - Electrical Room 01324 and Steel Beam Conflict</b>	<b>Open</b>	<b>CR</b>	<b>02/10/2015</b>	<b>02/20/2015</b>	



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	<div><div>From: Webcor Construction LP</div><div>Aseem Goyal</div></div> <div>REQUEST: Contract Doc Ref: [ASI 0128/E1-2303,2/E1-3301, S1-3111] Location: Zone[03 ], Level [Ground ] Closest Column Line Intersection is [D,11] Add'l Doc's Ref's: SK# FEC0022  In the electrical room 01324(E1-2303), the west side of the room shows a steel beam in conflict with the electrical panels. Please confirm where to relocate these panels. See attached sketch SK# FEC0022 screen shot for additional information of the conflict. Please advise.</div> <div>ANSWER: Contract Doc Ref: [ASI 0128/E1-2303,2/E1-3301, S1-3111] Location: Zone[03 ], Level [Ground ] Closest Column Line Intersection is [D,11] Add'l Doc's Ref's: SK# FEC0022  In the electrical room 01324(E1-2303), the west side of the room shows a steel beam in conflict with the electrical panels. Please confirm where to relocate these panels. See attached sketch SK# FEC0022 screen shot for additional information of the conflict. Please advise.</div>					
T-2217	RWP - WPM-2 Assembly Requirements at Grand Hall	Open	CR	02/10/2015	02/20/2015	
	<div><div>From: Webcor Construction LP</div><div>Tram Nguyen</div></div> <div>REQUEST: Contract Doc Ref: (ASI 128 dated 12/16/14) Detail 1/A1-9585 Specification Section 07 14 13  Location: Grand Hall  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Per Detail 1/A1-9585, Hot Fluid-Applied Waterproofing (WPM-2) is to be installed over the structural slab within the Grand Gall.  Per Specification Section 07 14 13 Hot Fluid-Applied Waterproofing (WPM-2) is to be installed with protection board and drain board.  Detail 1/A1-9585 does not show protection board or drain board over the WPM-2.  Please confirm protection board and drain board are required as part of the WPM-2 assembly slab within the Grand Hall.</div> <div>ANSWER: Contract Doc Ref: (ASI 128 dated 12/16/14) Detail 1/A1-9585 Specification Section 07 14 13  Location: Grand Hall  Closest Column Line Intersection: N/A  Add'l Doc Ref's: N/A  Per Detail 1/A1-9585, Hot Fluid-Applied Waterproofing (WPM-2) is to be installed over the structural slab within the Grand Gall.  Per Specification Section 07 14 13 Hot Fluid-Applied Waterproofing (WPM-2) is to be installed with protection board and drain board.  Detail 1/A1-9585 does not show protection board or drain board over the WPM-2.  Please confirm protection board and drain board are required as part of the WPM-2 assembly slab within the Grand Hall.</div>					



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T-2218	RWP - Floor Drain Requirements at Grand Hall	Open	CR	02/10/2015	02/20/2015	
From: Webcor Construction LP                      Tram Nguyen						
REQUEST:			ANSWER:			
Contract Doc Ref: (ASI 128 dated 12/16/14)			Contract Doc Ref: (ASI 128 dated 12/16/14)			
Detail 1/P1-2305			Detail 1/P1-2305			
Detail 1/A1-9585			Detail 1/A1-9585			
Location: Grand Hall			Location: Grand Hall			
Closest Column Line Intersection: N/A			Closest Column Line Intersection: N/A			
Add'l Doc Ref's: N/A			Add'l Doc Ref's: N/A			
Per Detail 1/P1-2305, no floor drains are present under the terrazzo at the Grand Hall.			Per Detail 1/P1-2305, no floor drains are present under the terrazzo at the Grand Hall.			
Per Detail 1/A1-9585, Hot Fluid-Applied Waterproofing (WPM-2) is to be installed over the T.O. structural slab within the Grand Hall.			Per Detail 1/A1-9585, Hot Fluid-Applied Waterproofing (WPM-2) is to be installed over the T.O. structural slab within the Grand Hall.			
Per detail 1/A1-9585, Crack Suppression Membrane (WPM-13) is to be installed over the T.O. the topping slab.			Per detail 1/A1-9585, Crack Suppression Membrane (WPM-13) is to be installed over the T.O. the topping slab.			
Should a water intrusion occur between the structural slab and topping slab, the water may float the topping slab.			Should a water intrusion occur between the structural slab and topping slab, the water may float the topping slab.			
Please confirm waterproofing is to be installed at the top of structural slab and topping slab, and no floor drains are to be installed within the terrazzo at the Grand Hall.			Please confirm waterproofing is to be installed at the top of structural slab and topping slab, and no floor drains are to be installed within the terrazzo at the Grand Hall.			



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<b>T-267</b>	<b>BSE - DI Installation at First Street</b>	<b>Closed</b>	<b>01</b>	<b>11/29/2011</b>	<b>12/09/2011</b>	<b>12/13/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b> Reference RFI U-101, Sheet U-3021  The RFI response U-101 dated 02-28-2011 eliminates the CB #501 from the RUP contractor's scope of work. However there has been no replacement or adequate surface water control system neither suggested nor installed to replace the CB # 501.  BBII recommends that this catch basin # 501, be installed per the original design to control surface water. Please confirm it will installed.		<b>ANSWER:</b> Reference RFI U-101, Sheet U-3021  The RFI response U-101 dated 02-28-2011 eliminates the CB #501 from the RUP contractor's scope of work. However there has been no replacement or adequate surface water control system neither suggested nor installed to replace the CB # 501.  BBII recommends that this catch basin # 501, be installed per the original design to control surface water. Please confirm it will installed.				
<b>T-268</b>	<b>BSE - Rebar in Secondary Shafts</b>	<b>Closed</b>	<b>01</b>	<b>12/08/2011</b>	<b>12/18/2011</b>	<b>12/12/2011</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b> Reference GT-2201, Installation Sequence Note 5  Please confirm the reinforcement in the secondary shafts should be installed in the last buttress shaft of each row.		<b>ANSWER:</b> Reference GT-2201, Installation Sequence Note 5  Please confirm the reinforcement in the secondary shafts should be installed in the last buttress shaft of each row.				
<b>T-474.1</b>	<b>BGP - Waterproofing Micropile on Slope</b>	<b>Closed</b>	<b>01</b>	<b>05/02/2013</b>	<b>05/12/2013</b>	<b>05/03/2013</b>
<b>From:</b> Webcor Construction LP      Kody Cooper						
<b>REQUEST:</b> Please reference response to RFI# T-0474. The manufacturer and installer will not provide a waterproofing detail for the micropile located in the sloped sump pits. Please provide a waterproofing detail acceptable for the use under the conditions specified in RFI# T-0474.		<b>ANSWER:</b> Please reference response to RFI# T-0474. The manufacturer and installer will not provide a waterproofing detail for the micropile located in the sloped sump pits. Please provide a waterproofing detail acceptable for the use under the conditions specified in RFI# T-0474.				
<b>T-509</b>	<b>BGP - Orientation of Protection Board</b>	<b>Closed</b>	<b>01</b>	<b>04/23/2013</b>	<b>05/03/2013</b>	<b>04/26/2013</b>
<b>From:</b> Webcor Construction LP      Kody Cooper						
<b>REQUEST:</b> Reference Specification: 07 12 10 - 3.2.D		<b>ANSWER:</b> Reference Specification: 07 12 10 - 3.2.D				





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T-701	<p>SSS - Dimension Clarification Required</p> <p>From: Webcor Construction LP      Robert Kjome</p> <p><b>REQUEST:</b></p> <p>Reference Drawing: 1/S1-5131</p> <p>Please see attached blow up of Plan Sheet S1-5131 Detail 1 View D (Front View). Please provide the location for the center of the 8" radius.</p>	Closed	01	08/29/2013	09/08/2013	08/30/2013
	<p><b>ANSWER:</b></p> <p>Reference Drawing: 1/S1-5131</p> <p>Please see attached blow up of Plan Sheet S1-5131 Detail 1 View D (Front View). Please provide the location for the center of the 8" radius.</p>					







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T-980.1	SSS - Perimeter Girders at Ground Level	Closed	CR	12/30/2013	01/09/2014	01/13/2014
<div>From: Webcor Construction LPGregory Kemerer</div> <div>REQUEST:As a revision to parts 1 &amp; 2 of SK RFI 238 (T-0980), please refer to the following and CD RFI 162.1 SK1 &amp; SK2 attached which are modifications to the BU girder connection details on 3 &amp;7/S1-4350:  1.) Confirm it is acceptable to provide the beam web, flange, and plate assembly as indicated. The CJP indicates the plate to flange weld above the beam per 5/S1-4350 while the PJP indicates the proposed web to flange weld. The web to flange fillet welds per RFI # T-0704.1 will be applied beyond the shown CJP and PJP welds.  2.) Per the response to SK RFI 238 (T-0980), it is acceptable to stop the bottom flange plate short as shown, extend the web plate of the BU WT to the web plate of the BU beam. Please verify the proposed weld is acceptable. The web to flange fillet welds per RFI # T-0704.1 will be applied beyond the shown CJP welds.</div> <div>ANSWER:As a revision to parts 1 &amp; 2 of SK RFI 238 (T-0980), please refer to the following and CD RFI 162.1 SK1 &amp; SK2 attached which are modifications to the BU girder connection details on 3 &amp;7/S1-4350:  1.) Confirm it is acceptable to provide the beam web, flange, and plate assembly as indicated. The CJP indicates the plate to flange weld above the beam per 5/S1-4350 while the PJP indicates the proposed web to flange weld. The web to flange fillet welds per RFI # T-0704.1 will be applied beyond the shown CJP and PJP welds.  2.) Per the response to SK RFI 238 (T-0980), it is acceptable to stop the bottom flange plate short as shown, extend the web plate of the BU WT to the web plate of the BU beam. Please verify the proposed weld is acceptable. The web to flange fillet welds per RFI # T-0704.1 will be applied beyond the shown CJP welds.</div>						
T0860.1	BGP - Rebar barlocks for interior Walls in Area 3	Closed	CR	11/13/2013	11/23/2013	11/19/2013
<div>From: Webcor Construction LPMichael Spillane</div> <div>REQUEST:Further to the response to RFI-860, Please find attached information (see exhibit A) on the proposed class 2 barlocks which are intended to be used at the noted partition walls in Area 3 as outlined in original RFI T-0860 see exhibit B.  Due to the overall diameter of these Type-2 bar locks, please confirm that it is acceptable to have reduced concrete clear cover to the barlocks which will be approximately ¾". This reduced clear cover will only be applicable for the length of the barlocks itself, which at worst case is approximately 12".  Please confirm that this is acceptable</div> <div>ANSWER:Further to the response to RFI-860, Please find attached information (see exhibit A) on the proposed class 2 barlocks which are intended to be used at the noted partition walls in Area 3 as outlined in original RFI T-0860 see exhibit B.  Due to the overall diameter of these Type-2 bar locks, please confirm that it is acceptable to have reduced concrete clear cover to the barlocks which will be approximately ¾". This reduced clear cover will only be applicable for the length of the barlocks itself, which at worst case is approximately 12".  Please confirm that this is acceptable</div>						
TG03.00-0001	TG03 Question 0001 - E & O Insurance	Closed	06	08/04/2010	08/18/2010	08/24/2010

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<b>REQUEST:</b>						
Reference Exhibit A Render on page SL-006						
A. Provide design criteria, if any, for minimum clear distance between street bridge columns, trestle, and shoring wall in the train box trench. Render shows single line of columns on either side of the trestle approximately equal distance between shoring wall and trestle edge.						
B. Confirm that clear-spanning from shoring wall to trestle is not required.						
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/03/2010						
<b>ANSWER:</b>						
Reference Exhibit A Render on page SL-006						
A. Provide design criteria, if any, for minimum clear distance between street bridge columns, trestle, and shoring wall in the train box trench. Render shows single line of columns on either side of the trestle approximately equal distance between shoring wall and trestle edge.						
B. Confirm that clear-spanning from shoring wall to trestle is not required.						
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/03/2010						
TG03.00-0006	TG03 Question 0006 - Temporary Bridge	Closed	06	08/10/2010	08/17/2010	08/23/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>						
Reference specs.						
A. Spec. section 01 15 13, 1.2.A states that street bridges shall be designed to support a "Fully assembled Manitowoc 999 crane weighing 500,000 lbs traveling w/out a hook load". Page A3-1 of Exhibit A states that the Manitowoc 999 crane body and counterweight weighs approximately 250,000 lbs, and gives unclear informatoin regarding the boom weight, and critical swing angle. Please clarify if 500,000 lbs applies to street crossings or can the same criteria for the trestle be applied to the street crossing bridges with respect to crane loads only.						
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/03/2010						
<b>ANSWER:</b>						
Reference specs.						
A. Spec. section 01 15 13, 1.2.A states that street bridges shall be designed to support a "Fully assembled Manitowoc 999 crane weighing 500,000 lbs traveling w/out a hook load". Page A3-1 of Exhibit A states that the Manitowoc 999 crane body and counterweight weighs approximately 250,000 lbs, and gives unclear informatoin regarding the boom weight, and critical swing angle. Please clarify if 500,000 lbs applies to street crossings or can the same criteria for the trestle be applied to the street crossing bridges with respect to crane loads only.						
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/03/2010						
TG03.00-0007	TG03 Question 0007 - West End Train Box	Closed	06	08/10/2010	08/17/2010	08/12/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>						
<b>ANSWER:</b>						







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<b>TG03.00-0011</b>	<b>TG03 Question 0011 - Bid Bond Form</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/26/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: Spec sections 31 63 29, 31 56 13						Ref: Spec sections 31 63 29, 31 56 13
Question: Please confirm that the internal bracing is designed to adequately support the loading indicated on dwg GT-1110, and work is installed (and top buttress removed) in compliance with the specifications, that the design for the drilled shafts (31-63-29) and CDSM shoring wall (31-56-13) is adequate to prevent further movement of 301 Mission St. and trade subcontractor's professional liability would not extend to the owner's design.						Question: Please confirm that the internal bracing is designed to adequately support the loading indicated on dwg GT-1110, and work is installed (and top buttress removed) in compliance with the specifications, that the design for the drilled shafts (31-63-29) and CDSM shoring wall (31-56-13) is adequate to prevent further movement of 301 Mission St. and trade subcontractor's professional liability would not extend to the owner's design.
Submitted by: Charles Gardner Kiewit Infrastructure West Co. 8/3/10.						Submitted by: Charles Gardner Kiewit Infrastructure West Co. 8/3/10.
<b>TG03.00-0012</b>	<b>TG03 Question 0012 - Electronic Drawing</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: N/A						Ref: N/A
Question: Will the owner or general contractor please provide bidders with electronic copies of the contract drawings for the Transbay Joint Powers Authority Contract No. 08-04-CMGC-000? Specifically those drawings which pertain to Trade Package #TG03.						Question: Will the owner or general contractor please provide bidders with electronic copies of the contract drawings for the Transbay Joint Powers Authority Contract No. 08-04-CMGC-000? Specifically those drawings which pertain to Trade Package #TG03.
Submitted by: Kelly Wigton Shimmick Construction, 8/4/10						Submitted by: Kelly Wigton Shimmick Construction, 8/4/10
<b>TG03.00-0013</b>	<b>TG03 Question 0013 - Milestones Clarification</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>						<b>ANSWER:</b>
Ref: Exhibit A - Trade Subcontractor Package						Ref: Exhibit A - Trade Subcontractor Package
Question:						Question:



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	<p>Page 15 Milestones state, "All submittals are to be provided within 10 days of NTP #1." Please clarify the expectation ("All" submittals?), and how this milestone relates to Milestone NTP #2 Start Date.</p> <p>Submitted by: Charles Gardner Kiewit Infrastructure West Co. 8/4/10</p>					<p>Page 15 Milestones state, "All submittals are to be provided within 10 days of NTP #1." Please clarify the expectation ("All" submittals?), and how this milestone relates to Milestone NTP #2 Start Date.</p> <p>Submitted by: Charles Gardner Kiewit Infrastructure West Co. 8/4/10</p>
<b>TG03.00-0014</b>	<b>TG03 Question 0014 - Demolition</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/23/2010</b>
<p><b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Ref: Exhibit A - Trade Subcontractor Package, D-1001, D2200</p> <p>Question: Drawing D-1001 shows processed concrete rubble from demolition contract left within the existing basement to approximately existing ground elevation. Drawing D-2200 note 1 indicates depth and thickness may vary. For bidding purposes, please clarify: 1. That the amount of processed rubble will not exceed the sections as shown on D-1001, or ground elevation, and 2. That all material on-site by the demolition subcontractor will be certified free of all contaminants.</p> <p>Submitted by: Charles Gardner Kiewit Infrastructure West Co. 8/4/10</p>			<p>Ref: Exhibit A - Trade Subcontractor Package, D-1001, D2200</p> <p>Question: Drawing D-1001 shows processed concrete rubble from demolition contract left within the existing basement to approximately existing ground elevation. Drawing D-2200 note 1 indicates depth and thickness may vary. For bidding purposes, please clarify: 1. That the amount of processed rubble will not exceed the sections as shown on D-1001, or ground elevation, and 2. That all material on-site by the demolition subcontractor will be certified free of all contaminants.</p> <p>Submitted by: Charles Gardner Kiewit Infrastructure West Co. 8/4/10</p>			



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<b>TG03.00-0015</b>	<b>TG03 Question 0015 - Night Noise Permit</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/18/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Ref: 01 15 70 3.2.A.12  Question: Specification section 01 15 70 3.2.A.12 states, " Work is restricted during the holiday moratorium (day after Thanksgiving to January 1. inclusive, 24 hours a day, seven days per week as set forth in the Blue Book by the SFMTA. Blue Book allows work at night within the restriction zone , "...as long as the proper night noise permit is obtained." Please confirm that DPW issues the night noise permit, what are the parameters, and that it will be obtainable so that we may at least work night shifts during this period.  Submitted by: Charles Gardner Kiewit Infrastructure West Co. 8/4/10						<b>ANSWER:</b> Ref: 01 15 70 3.2.A.12  Question: Specification section 01 15 70 3.2.A.12 states, " Work is restricted during the holiday moratorium (day after Thanksgiving to January 1. inclusive, 24 hours a day, seven days per week as set forth in the Blue Book by the SFMTA. Blue Book allows work at night within the restriction zone , "...as long as the proper night noise permit is obtained." Please confirm that DPW issues the night noise permit, what are the parameters, and that it will be obtainable so that we may at least work night shifts during this period.  Submitted by: Charles Gardner Kiewit Infrastructure West Co. 8/4/10
<b>TG03.00-0016</b>	<b>TG03 Question 0016 - Professional Liability Insurance</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/18/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference Exhibit A, VI.2.B. and spec. 00 08 05, 1.3.B  Specification 08 05 1.3.B requires Professional Liability Insurance in the amount of \$10,000,000 each claim with a deductible not to exceed \$50,00 each claim. Exhibit A VI.2.b requires \$25,000 with a deductible not to exceed \$250,000.  Q. Can you clarify why the Trade Subcontractor would be held to an amount higher than the CM/GC?  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/04/2010						<b>ANSWER:</b> Reference Exhibit A, VI.2.B. and spec. 00 08 05, 1.3.B  Specification 08 05 1.3.B requires Professional Liability Insurance in the amount of \$10,000,000 each claim with a deductible not to exceed \$50,00 each claim. Exhibit A VI.2.b requires \$25,000 with a deductible not to exceed \$250,000.  Q. Can you clarify why the Trade Subcontractor would be held to an amount higher than the CM/GC?  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/04/2010
<b>TG03.00-0017</b>	<b>TG03 Question 0017 - Commercial Liability Insurance</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/18/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>						<b>ANSWER:</b>



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	Reference Exhibit A, BI.1.B and 00 08 05, 1.2.B  Specification 08 05, 1.2.B requires Commercial Liability Insurance in the amount of \$25,000,000 each occurrence. Exhibit A, VI.1.B requires \$100,000,000.  Q. Can you clarify why the Trade Subcontractor would be held to an amount higher than the CM/GC?  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/04/2010					
	Reference Exhibit A, BI.1.B and 00 08 05, 1.2.B  Specification 08 05, 1.2.B requires Commercial Liability Insurance in the amount of \$25,000,000 each occurrence. Exhibit A, VI.1.B requires \$100,000,000.  Q. Can you clarify why the Trade Subcontractor would be held to an amount higher than the CM/GC?  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/04/2010					
<b>TG03.00-0018</b>	<b>TG03 Question 0018 - Fees</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/18/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 00 08 13, 1.8  Muni Code 2.4 requires,  "Each applicant shall submit and maintain with the Department a bond, cash deposit, or other security acceptable to the Department securing the faithful performance of the obligations of the owner and its agent under any permit(s) to excavate and the compliance with all terms and conditions of this Article (the "deposit"). The deposit shall be in the sum of \$25,000 in favor of the "Department of Public Works, City and County of San Francisco."  Also there are Administration fees daily inspection fees and other "additional fees"  Please clarify which fees the Trade Subcontractor on this project will be required to make.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/05/2010						
<b>ANSWER:</b>  Reference specification 00 08 13, 1.8  Muni Code 2.4 requires,  "Each applicant shall submit and maintain with the Department a bond, cash deposit, or other security acceptable to the Department securing the faithful performance of the obligations of the owner and its agent under any permit(s) to excavate and the compliance with all terms and conditions of this Article (the "deposit"). The deposit shall be in the sum of \$25,000 in favor of the "Department of Public Works, City and County of San Francisco."  Also there are Administration fees daily inspection fees and other "additional fees"  Please clarify which fees the Trade Subcontractor on this project will be required to make.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/05/2010						



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<b>TG03.00-0019</b>	<b>TG03 Question 0019 - Wastewater Discharge Permit</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/10/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 31 23 19, 1.7.C  Spec section 31 23 19 1.7.C requires Contractor to obtain a wastewater discharge permit from the City of San Francisco. Who pays for the cost of is charging into the local municipal waste water collection system? Who pays for the analytical testing?  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/05/2010						<b>ANSWER:</b>  Reference specification 31 23 19, 1.7.C  Spec section 31 23 19 1.7.C requires Contractor to obtain a wastewater discharge permit from the City of San Francisco. Who pays for the cost of is charging into the local municipal waste water collection system? Who pays for the analytical testing?  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/05/2010
<b>TG03.00-0020</b>	<b>TG03 Question 0020 - Buy America Requirements</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 00 08 13/APA.17  Spec section 00 08 13/APA.17 Buy America Requirements provide , " ** ** This provision applies only to the following types of Agreements: construction agreements of any value; agreements for the acquisition of goods valued at more than \$100,000; and agreements for the acquisition of rolling stock valued at more than \$100,000. This requirement does not apply to lower tier Subcontracts. 00 08 13/APA 17 (b) further states that, "The Prime Contractor is responsible for ensuring that lower tier subcontractors are in compliance.  A) Please confirm "This requirement does not apply to lower tier Subcontracts"  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/05/2010						<b>ANSWER:</b>  Reference specification 00 08 13/APA.17  Spec section 00 08 13/APA.17 Buy America Requirements provide , " ** ** This provision applies only to the following types of Agreements: construction agreements of any value; agreements for the acquisition of goods valued at more than \$100,000; and agreements for the acquisition of rolling stock valued at more than \$100,000. This requirement does not apply to lower tier Subcontracts. 00 08 13/APA 17 (b) further states that, "The Prime Contractor is responsible for ensuring that lower tier subcontractors are in compliance.  A) Please confirm "This requirement does not apply to lower tier Subcontracts"  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/05/2010
<b>TG03.00-0021</b>	<b>TG03 Question 0021 - SBE Program</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>						<b>ANSWER:</b>



# 30100 - Transbay Transit Center Project



# Webcor/Obayashi Joint Venture

## PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

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<b>TG03.00-0023</b>	<b>TG03 Question 0023 - Geotechnical Reports</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference: N/A  In regards to the reference documents: under the Folder: Geotechnical Reports, Volume 3 from the "Final Geotechnical Data Reports" cannot be found. Volumes 1&2 were clearly uploaded. Will you please upload V3 - or let me know where to find the document on the FTP website? Thank you.  Submitted by Briana Harvey Malcolm Drilling Co. 08/06/2010						<b>ANSWER:</b> Reference: N/A  In regards to the reference documents: under the Folder: Geotechnical Reports, Volume 3 from the "Final Geotechnical Data Reports" cannot be found. Volumes 1&2 were clearly uploaded. Will you please upload V3 - or let me know where to find the document on the FTP website? Thank you.  Submitted by Briana Harvey Malcolm Drilling Co. 08/06/2010
<b>TG03.00-0024</b>	<b>TG03 Question 0024 - Ancillary Permits</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference Project Bidding Manual (20/44) and specification 01 14 10/APA 1  Please clarify the definition of "ancillary permits". Request specific differentiation of responsibilities between Contractor/Trade Subcontractor/ TJPA. Perhaps a new responsibility matrix with a column for the Trade Subcontractor would help.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/06/2010						<b>ANSWER:</b> Reference Project Bidding Manual (20/44) and specification 01 14 10/APA 1  Please clarify the definition of "ancillary permits". Request specific differentiation of responsibilities between Contractor/Trade Subcontractor/ TJPA. Perhaps a new responsibility matrix with a column for the Trade Subcontractor would help.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/06/2010
<b>TG03.00-0025</b>	<b>TG03 Question 0025 - Access Trestle</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/16/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference: SL-001  Concept drawing for the access trestle shows a width of 48' in Zone 4 and 32' wide everywhere else. Are there minimum width requirements for the access trestle? If so, please provide details. Are there maximum width constraints? If so, please provide details.						<b>ANSWER:</b> Reference: SL-001  Concept drawing for the access trestle shows a width of 48' in Zone 4 and 32' wide everywhere else. Are there minimum width requirements for the access trestle? If so, please provide details. Are there maximum width constraints? If so, please provide





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Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/06/2010			details.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/06/2010			
<b>TG03.00-0026</b>	<b>TG03 Question 0026 - Surveyor Insurance</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/16/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference Exhibit A, Section VI  Part 2A indicates the Trade Subcontractor must utilize surveyors who can provide \$25,000,000 of insurance. This will effctively eliminate many survey engineers from being able to bid on this work. The resultant effect will be higher bid costs. Please consider this and confirm what the insurance requirements are for land surveyors.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/06/2010			<b>ANSWER:</b>  Reference Exhibit A, Section VI  Part 2A indicates the Trade Subcontractor must utilize surveyors who can provide \$25,000,000 of insurance. This will effctively eliminate many survey engineers from being able to bid on this work. The resultant effect will be higher bid costs. Please consider this and confirm what the insurance requirements are for land surveyors.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/06/2010			
<b>TG03.00-0027</b>	<b>TG03 Question 0027 - Temporary Street Closures / Detours</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/13/2010</b>
<b>From:</b> Webcor Construction LP      Manuel Saldana						
<b>REQUEST:</b>  Reference Exhibit A, Section VI  Will temporary closures and/or temporary detours of First St, Fremont St, and Beale St be alowed so the contractor can perform activities such as 1) installation of CDSM elements, 2) demolition, 3) installation of temporary street elements? If not, how is the Owner proposing these work elements be performed?  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/09/2010			<b>ANSWER:</b>  Reference Exhibit A, Section VI  Will temporary closures and/or temporary detours of First St, Fremont St, and Beale St be alowed so the contractor can perform activities such as 1) installation of CDSM elements, 2) demolition, 3) installation of temporary street elements? If not, how is the Owner proposing these work elements be performed?  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/09/2010			



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<b>TG03.00-0028</b>	<b>TG03 Question 0028 - Trade Subcontractor DBE Participation</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 00 08 21, Section 1.2.B  Section 1.2B states "The DBE Availability Advisory Percentage is not an enforceable goal under the CalTrans mandated changes to the DBE program, and compliance with the advisory is not a condition of the contract" Please clarify what the Trade Subcontractors requirement is regarding DBE participation on this contract.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/09/2010		<b>ANSWER:</b>  Reference specification 00 08 21, Section 1.2.B  Section 1.2B states "The DBE Availability Advisory Percentage is not an enforceable goal under the CalTrans mandated changes to the DBE program, and compliance with the advisory is not a condition of the contract" Please clarify what the Trade Subcontractors requirement is regarding DBE participation on this contract.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/09/2010				
<b>TG03.00-0029</b>	<b>TG03 Question 0029 - Demolition Contract</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/18/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 00 00 35, section 1.2.A  Section states "The demolition contractor is responsible for removing and abating products containing asbestos, lead, or PCB ballast, or mercury containing lamps." Please confirm the reference to demolition contractor is specific to the Trade Subcontractor performing work under contract 08-08-DM-000, Existing Terminal and Ramps Demolition Contract.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/09/2010		<b>ANSWER:</b>  Reference specification 00 00 35, section 1.2.A  Section states "The demolition contractor is responsible for removing and abating products containing asbestos, lead, or PCB ballast, or mercury containing lamps." Please confirm the reference to demolition contractor is specific to the Trade Subcontractor performing work under contract 08-08-DM-000, Existing Terminal and Ramps Demolition Contract.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/09/2010				
<b>TG03.00-0030</b>	<b>TG03 Question 0030 - Trade Subcontractor Insurance</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/18/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 00 08 05  Section 00 08 05 contains specific insurance requirements. These requirements differ materially from		<b>ANSWER:</b>  Reference specification 00 08 05  Section 00 08 05 contains specific insurance requirements. These requirements differ materially				



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	<p>those contained in Exhibit A Section VI as well as section 16 of the proposed subcontract between Webcor / Obayashi and the Trade Subcontractor. Please clarify what the insurance requirements are for the Trade Subcontractor.</p> <p>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/09/2010</p>					
	<p>from those contained in Exhibit A Section VI as well as section 16 of the proposed subcontract between Webcor / Obayashi and the Trade Subcontractor. Please clarify what the insurance requirements are for the Trade Subcontractor.</p> <p>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/09/2010</p>					
<b>TG03.00-0031</b>	<b>TG03 Question 0031 - Contaminated Groundwater</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/19/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference specification 01 35 65, sections 1.7.G & 1.7.H.			Reference specification 01 35 65, sections 1.7.G & 1.7.H.			
Section 1.7H.2 describes construction of a "small-scale batch wastewater treatment system to remove dissolved contaminants" such as petroleum hydrocarbons, benzene, toluene, etc. Please verify that the treatment costs to handle contaminated groundwater will be paid as extra work by TJPA.			Section 1.7H.2 describes construction of a "small-scale batch wastewater treatment system to remove dissolved contaminants" such as petroleum hydrocarbons, benzene, toluene, etc. Please verify that the treatment costs to handle contaminated groundwater will be paid as extra work by TJPA.			
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/09/2010			Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/09/2010			



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<b>TG03.00-0032</b>	<b>TG03 Question 0032 - Extend Bid Date</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Exhibit A, II, "Key Dates for Bidding Process" of the Project Bidding Manual establishes the Bid Due Date as Sept. 14, 2010, 6 weeks from the date of bid package issuance.</p> <p>Six weeks is an insufficient amount of time to adequately prepare a \$200M estimate and bid. We therefore request that the Bid Due Date be extended an additional 8 weeks for the following reasons</p> <p>Design-Build The SBE package includes major deisgn-build elements. The extent of the design work related to shoring, bracing, trestle, cross-street bridging and dewatering that the BSE Trade Subcontractor will be responsible for performing became fully apparent to the prequalified contractors only when the bid package was issued. Developing these required Trade Contractor designs far enough to allow accurate pricing to begin will take time.</p> <p>Considering the magniute and location of the construction work, the risk accompanying the design is also extremely high. A memorandum of understanding that adequately addresses this risk must be negotiated with the Trade Subcontractor's Professional Engineer before design can even beign. This will take time.</p> <p>Similarly, the Trade Subcontractor will likely enlist the services of an independent Professional Engineer to act as a peer reviewer to check the work of the Trade Subcontractor's principal engineer. Reconciliation of any differences identified during this review will take time.</p> <p>Liquidated Damges Liquidated damages associated with not meeting the CM/GC's schedule for Substantial Completion are extraordinarily high. It appears to us that the BSE Trade Subcontractor's time for substantial completion, i.e. 1,825 days following Notice to Proceed with pre-construction services, is highly interconnected with the work of other trade subcontractors and also contingent on their performance. If this is the case, it becomes very difficult to accurately assess risk of exposure to liquidated damages. The Joint Venture will need time to clarify with the CM/GC the relationship between the other trade subcontractors' work and the BSE Trade Subcontractor's</p>			<p>Exhibit A, II, "Key Dates for Bidding Process" of the Project Bidding Manual establishes the Bid Due Date as Sept. 14, 2010, 6 weeks from the date of bid package issuance.</p> <p>Six weeks is an insufficient amount of time to adequately prepare a \$200M estimate and bid. We therefore request that the Bid Due Date be extended an additional 8 weeks for the following reasons</p> <p>Design-Build The SBE package includes major deisgn-build elements. The extent of the design work related to shoring, bracing, trestle, cross-street bridging and dewatering that the BSE Trade Subcontractor will be responsible for performing became fully apparent to the prequalified contractors only when the bid package was issued. Developing these required Trade Contractor designs far enough to allow accurate pricing to begin will take time.</p> <p>Considering the magniute and location of the construction work, the risk accompanying the design is also extremely high. A memorandum of understanding that adequately addresses this risk must be negotiated with the Trade Subcontractor's Professional Engineer before design can even beign. This will take time.</p> <p>Similarly, the Trade Subcontractor will likely enlist the services of an independent Professional Engineer to act as a peer reviewer to check the work of the Trade Subcontractor's principal engineer. Reconciliation of any differences identified during this review will take time.</p> <p>Liquidated Damges Liquidated damages associated with not meeting the CM/GC's schedule for Substantial Completion are extraordinarily high. It appears to us that the BSE Trade Subcontractor's time for substantial completion, i.e. 1,825 days following Notice to Proceed with pre-construction services, is highly interconnected with the work of other trade subcontractors and also contingent on their performance. If this is the case, it becomes very difficult to accurately assess risk of exposure to</p>			



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	<p>substantial completion date. Then we can adequately assess the risk resulting from this relationship, include it in our pricing and secure surety commitment.</p> <p>Small Business Program The Trade Subcontractor is required to achieve a minimum small business enterprise participation of 24% of its' total bid. Given the magnitude of the principal scopes of work required in the BSE package - shoring/bracing, excavation, drilling - most small businesses will neither be interested in participating nor qualified to do so. Time will be needed to identify a sufficient amount of reasonable scopes of work for small business participation and to work with interested small businesses, as necessary, prior to bid day to help them with insurance, bonding, scheduling, and performance issues.</p> <p>Given the circumstances outline above, Shimmick / Skanska / Traylor strongly urges the Transbay Joint Powers Authority and Webcor / Obayashi to postpone the bid date for the TG03 BSE Package until Nov. 9, 2010.</p> <p>Submitted by Rich Zito Shimmick / Skanska / Traylor, a Joint Venture 08/09/2010</p>					<p>liquidated damages. The Joint Venture will need time to clarify with the CM/GC the relationship between the other trade subcontractors' work and the BSE Trade Subcontractor's substantial completion date. Then we can adequately assess the risk resulting from this relationship, include it in our pricing and secure surety commitment.</p> <p>Small Business Program The Trade Subcontractor is required to achieve a minimum small business enterprise participation of 24% of its' total bid. Given the magnitude of the principal scopes of work required in the BSE package - shoring/bracing, excavation, drilling - most small businesses will neither be interested in participating nor qualified to do so. Time will be needed to identify a sufficient amount of reasonable scopes of work for small business participation and to work with interested small businesses, as necessary, prior to bid day to help them with insurance, bonding, scheduling, and performance issues.</p> <p>Given the circumstances outline above, Shimmick / Skanska / Traylor strongly urges the Transbay Joint Powers Authority and Webcor / Obayashi to postpone the bid date for the TG03 BSE Package until Nov. 9, 2010.</p> <p>Submitted by Rich Zito Shimmick / Skanska / Traylor, a Joint Venture 08/09/2010</p>
TG03.00-0033	TG03 Question 0033 - Staging Areas	Closed	06	08/10/2010	08/17/2010	08/18/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:						ANSWER:
Reference Project Bid Manual IV, A.3.b.						Reference Project Bid Manual IV, A.3.b.
Trade Subcontractor Requirements Q - Will Staging areas 9, 10, 12 etc. from the Existing Terminal Ramps & Demolition Plans be made available to the TG03 - BSE Trade Subcontractor?						Trade Subcontractor Requirements Q - Will Staging areas 9, 10, 12 etc. from the Existing Terminal Ramps & Demolition Plans be made available to the TG03 - BSE Trade Subcontractor?





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<b>TG03.00-0035</b>	<b>TG03 Question 0035 - Temporary Power</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Project Bid Manual IV.A. 17.a			Reference Project Bid Manual IV.A. 17.a			
Trade Subcontractor Requirements Q Please confirm that the Owner/TJPA will pay the cost of Temporary Power consumption.			Trade Subcontractor Requirements Q Please confirm that the Owner/TJPA will pay the cost of Temporary Power consumption.			
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/09/2010			Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/09/2010			
<b>TG03.00-0036</b>	<b>TG03 Question 0036 - Unit Prices</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/18/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference specification 01 10 20 Section 01 10 20 describes a schedule of unit prices.			Reference specification 01 10 20 Section 01 10 20 describes a schedule of unit prices.			
Trade Subcontractor Requirements Q - These items are not shown on the Schedule of Bid Prices found in Exhibit A. How is the contractor to communicate what his applicable bid prices are?			Trade Subcontractor Requirements Q - These items are not shown on the Schedule of Bid Prices found in Exhibit A. How is the contractor to communicate what his applicable bid prices are?			
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/10/2010			Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/10/2010			
<b>TG03.00-0037</b>	<b>TG03 Question 0037 - Dewatering</b>	<b>Closed</b>	<b>06</b>	<b>08/10/2010</b>	<b>08/17/2010</b>	<b>08/16/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference 31 23 19			Reference 31 23 19			
Trade Subcontractor Requirements Q - Section 31 23 19,Dewatering, is unclear regarding the duration that the Trade Subcontractor remains responsible for maintaining the dewatering system. Please provide details of how long the Trade Subcontractor is responsible for the system. Is the system to be turned over to a follow on Subcontractor? Is the dewatering system to be			Trade Subcontractor Requirements Q - Section 31 23 19,Dewatering, is unclear regarding the duration that the Trade Subcontractor remains responsible for maintaining the dewatering system. Please provide details of how long the Trade Subcontractor is responsible for the system. Is the system to be turned over to a follow on			



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	<div>removed by the Trade Subcontractor for the BSE package? If so, when?</div> <div>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/10/2010</div>					<div>Subcontractor? Is the dewatering system to be removed by the Trade Subcontractor for the BSE package? If so, when?</div> <div>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/10/2010</div>
TG03.00-0038	TG03 Question 0038 - Temporary Power	Closed	06	08/17/2010	08/31/2010	08/13/2010
<div>From: Webcor/Obayashi Joint Venture      Manuel Saldana</div> <div>REQUEST: Reference Exhibit A Attachment 2  Q - Logistics, drawing sheet SL-003 Skid Layout (5) has notation "NOT INCLUDED IN THIS SERVICE REQUEST" Q -Please confirm that the Owner/TJPA will be providing this Skid, typically per detail 4/SL-003  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/11/2010</div> <div>ANSWER: Reference Exhibit A Attachment 2  Q - Logistics, drawing sheet SL-003 Skid Layout (5) has notation "NOT INCLUDED IN THIS SERVICE REQUEST" Q -Please confirm that the Owner/TJPA will be providing this Skid, typically per detail 4/SL-003  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/11/2010</div>						
TG03.00-0039	TG03 Question 0039 - Access Trestle	Closed	06	08/11/2010	08/18/2010	08/13/2010
<div>From: Webcor/Obayashi Joint Venture      Manuel Saldana</div> <div>REQUEST: Reference Exhibit A - Attachment 3.1  Q - Please confirm access trestle shall be designed (similar to Temp Bridges)for a Manitowoc Crane 999 Series 2 which weigh's approximately 475,000 lbs  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/11/2010</div> <div>ANSWER: Reference Exhibit A - Attachment 3.1  Q - Please confirm access trestle shall be designed (similar to Temp Bridges)for a Manitowoc Crane 999 Series 2 which weigh's approximately 475,000 lbs  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/11/2010</div>						
TG03.00-0040	TG03 Question 0040 - Access Trestle	Closed	06	08/11/2010	08/17/2010	08/16/2010







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TG03.00-0042	TG03 Question 0042 - Dimensions	Closed	06	08/11/2010	08/17/2010	08/17/2010
<div><div><div>From: Webcor/Obayashi Joint Venture</div><div>Manuel Saldana</div></div><div><div>REQUEST:</div><div>Reference drawing sheet GT-2101</div><div>Q - Dimensions to the radius' center point for wall segment R2-1 do not correspond to the given radius. Please revise.</div><div>Submitted by Shad Gardner Balfour Beatty 08/11/2010</div></div><div><div>ANSWER:</div><div>Reference drawing sheet GT-2101</div><div>Q - Dimensions to the radius' center point for wall segment R2-1 do not correspond to the given radius. Please revise.</div><div>Submitted by Shad Gardner Balfour Beatty 08/11/2010</div></div></div>						
TG03.00-0043	TG03 Question 0043 - Liquidated Damages	Closed	06	08/11/2010	08/17/2010	08/13/2010
<div><div><div>From: Webcor/Obayashi Joint Venture</div><div>Manuel Saldana</div></div><div><div>REQUEST:</div><div>Reference Exhibit A Section 5 last paragraph references Specification 00 05 20 for responsibilities for liquidated damages.</div><div>Q - The liquidated damages described in 00 05 20 are based on a requirement for the completion of Trade package No. 1 in 1,825 calendar days beginning with Notice to Proceed with Pre-Construction Services. 1) Please provide the Notice to Proceed date for Pre-Construction Services. 2) Please advise how liquidated damages will be assessed for late completion of Zone 1, Zone 2, Zone 3, and Zone 4. Since any of these zones could potentially be late, it is not clear how the CM/GC will assess potential LD's.</div><div>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/11/2010</div></div><div><div>ANSWER:</div><div>Reference Exhibit A Section 5 last paragraph references Specification 00 05 20 for responsibilities for liquidated damages.</div><div>Q - The liquidated damages described in 00 05 20 are based on a requirement for the completion of Trade package No. 1 in 1,825 calendar days beginning with Notice to Proceed with Pre-Construction Services. 1) Please provide the Notice to Proceed date for Pre-Construction Services. 2) Please advise how liquidated damages will be assessed for late completion of Zone 1, Zone 2, Zone 3, and Zone 4. Since any of these zones could potentially be late, it is not clear how the CM/GC will assess potential LD's.</div><div>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/11/2010</div></div></div>						
TG03.00-0044	TG03 Question 0044 - Existing Utilities	Closed	06	08/11/2010	08/17/2010	08/18/2010
<div><div><div>From: Webcor/Obayashi Joint Venture</div><div>Manuel Saldana</div></div><div><div>REQUEST:</div><div>Reference drawing sheet D-2230</div><div>Q - Note #2 states that, unless otherwise specified, all utilities to be removed have been cut and capped. The</div></div><div><div>ANSWER:</div><div>Reference drawing sheet D-2230</div><div>Q - Note #2 states that, unless otherwise specified, all utilities to be removed have been cut and capped. The</div></div></div>						



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	<p>only remaining utilities shown to be cut and capped are the sewer systems at First, Fremont, and Beale. We interpret this note to mean that, except for the sewer systems discussed, there are no other active utilities in the work zone (including in First St, Fremont St, and Beale). 1) Please confirm there are no other active utilities that the Trade Subcontractor has to either cut/cap or maintain in place. 2) If there are other utilities that have to be cut and capped, please provide specific details. 3) If there are other utilities that have to be maintained in place, please provide specific details.</p> <p>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/11/2010</p>					
	<p>only remaining utilities shown to be cut and capped are the sewer systems at First, Fremont, and Beale. We interpret this note to mean that, except for the sewer systems discussed, there are no other active utilities in the work zone (including in First St, Fremont St, and Beale). 1) Please confirm there are no other active utilities that the Trade Subcontractor has to either cut/cap or maintain in place. 2) If there are other utilities that have to be cut and capped, please provide specific details. 3) If there are other utilities that have to be maintained in place, please provide specific details.</p> <p>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/11/2010</p>					
TG03.00-0045	TG03 Question 0045 - Escrow Documents	Closed	06	08/12/2010	08/18/2010	08/16/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:			ANSWER:			
Reference specification 00 02 12, 1.3.A			Reference specification 00 02 12, 1.3.A			
Q - Paragraph 1.3A states escrow documents are to be submitted within 3 working days after the date of bid opening. This contradicts Project Bidding Manual page 15 which states that escrow documents are to be submitted within 3 calendar days after the bid opening date. Please clarify.			Q - Paragraph 1.3A states escrow documents are to be submitted within 3 working days after the date of bid opening. This contradicts Project Bidding Manual page 15 which states that escrow documents are to be submitted within 3 calendar days after the bid opening date. Please clarify.			
Submitted by Kelly Turner Granite /CJA / NCC Joint Venture 08/11/2010			Submitted by Kelly Turner Granite /CJA / NCC Joint Venture 08/11/2010			



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<b>TG03.00-0046</b>	<b>TG03 Question 0046 - Construction Schedule</b>	<b>Closed</b>	<b>06</b>	<b>08/12/2010</b>	<b>08/18/2010</b>	<b>08/20/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference specification 01 13 10, 1.2.B			Reference specification 01 13 10, 1.2.B			
Q - Paragraph 1.2B states a construction schedule is to be submitted within 15 days after bid package Notice to Proceed. This contradicts Exhibit A Section 5 which states the schedule is to be submitted within 15 calendar days of award. Please clarify.			Q - Paragraph 1.2B states a construction schedule is to be submitted within 15 days after bid package Notice to Proceed. This contradicts Exhibit A Section 5 which states the schedule is to be submitted within 15 calendar days of award. Please clarify.			
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/11/2010			Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/11/2010			
<b>TG03.00-0047</b>	<b>TG03 Question 0047 - SBE Program</b>	<b>Closed</b>	<b>06</b>	<b>08/12/2010</b>	<b>08/18/2010</b>	<b>08/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Exhibit A, Part IV and specification 00 08 21, paragraph B, item D and paragraph 1.3, item B.			Reference Exhibit A, Part IV and specification 00 08 21, paragraph B, item D and paragraph 1.3, item B.			
Q - Exhibit A, Part IV., Scope of the Package and Bid Item Information, Paragraph B., General Work, Item D., SBE Program states "Trade Subcontractor shall obtain a minimum SBE participation of 24% of the total value of Trade Subcontractor's bid value." However, Section 00 08 21, Disadvantaged & Small Business Enterprise and Equal Employment Opportunity/Employment Nondiscrimination Requirements, Paragraph 1.3 Small Business Enterprise (SBE) Program Requirements, Item B. states "The TJPA has established an SBE Utilization Goal of 17% overall for this Contract."			Q - Exhibit A, Part IV., Scope of the Package and Bid Item Information, Paragraph B., General Work, Item D., SBE Program states "Trade Subcontractor shall obtain a minimum SBE participation of 24% of the total value of Trade Subcontractor's bid value." However, Section 00 08 21, Disadvantaged & Small Business Enterprise and Equal Employment Opportunity/Employment Nondiscrimination Requirements, Paragraph 1.3 Small Business Enterprise (SBE) Program Requirements, Item B. states "The TJPA has established an SBE Utilization Goal of 17% overall for this Contract."			
Are we correct in assuming that the SBE participation is to be 24% of our bid value for this contract (Contract No. 08-04-CMGC-000), but that the SBE Goal for the entire Transit Center Project is 17%?			Are we correct in assuming that the SBE participation is to be 24% of our bid value for this contract (Contract No. 08-04-CMGC-000), but that the SBE Goal for the entire Transit Center Project is 17%?			
Submitted by Gerald Brown Tutor-Salib Corporation 08/12/20101			Submitted by Gerald Brown Tutor-Salib Corporation 08/12/20101			
<b>TG03.00-0048</b>	<b>TG03 Question 0048 - Instruction to Bidders</b>	<b>Closed</b>	<b>06</b>	<b>08/12/2010</b>	<b>08/18/2010</b>	<b>08/18/2010</b>



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<hr/>						
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Instruction To Bidders, subparagraph D.			Reference Instruction To Bidders, subparagraph D.			
Q - Reference is made to Part III. Instruction to Bidders, Subparagraph D., Bidding Process and Procedures, Item 6. Statutory Bidding Requirements, Subitem b) Bidders Qualification Statement (1) which states that "Bidder shall list on the Bidder's Qualification Statement (BQS in Forms Section) its current contractor license number. . ." we can not find such a form. Please provide.			Q - Reference is made to Part III. Instruction to Bidders, Subparagraph D., Bidding Process and Procedures, Item 6. Statutory Bidding Requirements, Subitem b) Bidders Qualification Statement (1) which states that "Bidder shall list on the Bidder's Qualification Statement (BQS in Forms Section) its current contractor license number. . ." we can not find such a form. Please provide.			
Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/12/2010			Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/12/2010			
<hr/>						
<b>TG03.00-0049</b>	<b>TG03 Question 0050 - Bid Due Date</b>	<b>Closed</b>	<b>06</b>	<b>08/12/2010</b>	<b>08/18/2010</b>	<b>08/13/2010</b>
<hr/>						
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Part III Instruction to Bidders, Section V, Paragraph A, Item 3			Reference Part III Instruction to Bidders, Section V, Paragraph A, Item 3			
Q - Reference is made to Part III. Instruction to Bidders, Section V., Webcor/Obayashi Bidding Forms, Paragraph A., Bidding Checklist (BCL), Item 3. which states "Each Bidder shall submit with its Bid the following forms, properly completed and executed." Following this statement there are various forms listed including "Escrow Agreement for Security Deposits in Lieu of Retention (Section 00 06 30)." Since this "Escrow Agreement for Security Deposits in Lieu of Retention" form states that - "pursuant to the construction contract entered into between the TJPA and Contractor for Transbay Transit Center. . . in the amount of _____ dated _____", we request that this form be among those forms submitted by the successful Trade Subcontractor after the Notice of Intent to Award and not with the bid form. Please advise.			Q - Reference is made to Part III. Instruction to Bidders, Section V., Webcor/Obayashi Bidding Forms, Paragraph A., Bidding Checklist (BCL), Item 3. which states "Each Bidder shall submit with its Bid the following forms, properly completed and executed." Following this statement there are various forms listed including "Escrow Agreement for Security Deposits in Lieu of Retention (Section 00 06 30)." Since this "Escrow Agreement for Security Deposits in Lieu of Retention" form states that - "pursuant to the construction contract entered into between the TJPA and Contractor for Transbay Transit Center. . . in the amount of _____ dated _____", we request that this form be among those forms submitted by the successful Trade Subcontractor after the Notice of Intent to Award and not with the bid form. Please advise.			
Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/12/2010			Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/12/2010			



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<b>TG03.00-0050</b>	<b>TG03 Question 0050 - Bid Due Date</b>	<b>Closed</b>	<b>06</b>	<b>08/12/2010</b>	<b>08/18/2010</b>	<b>08/18/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Transbay Terminal Center Bid Package TG03 Shoring, Buttress and Excavation (Cont.)			Reference Transbay Terminal Center Bid Package TG03 Shoring, Buttress and Excavation (Cont.)			
<p>Q - We have started into our second week of intensive review of bid documents and drawings in which to grasp what is prescriptive work and what requires additional contractor design and scheduling in our bid proposal preparation. Having been involved in not only pre-bid contractor designed support-of-excavations but final design and construction of numerous deep supported excavations in urban environments in numerous cities in California, we look forward to working on this unique and challenging project. This project brings additional elements to be considered during design of support-of-excavation and traffic supported trestles that would not normally be considered in below street level construction. One, the width of the supported excavation at approximately one hundred and eighty L.F., will require at least two intermediate vertical piles to support the horizontal bracing levels. Second, the need to incorporate both longitudinal and transverse traffic trestle decking whose vertical support locations will add to the constraints and interference with the location of the horizontal bracing levels. Third, the phasing of the shoring and excavation will require various locations of bracing levels of the CDSM cut-off walls. Fourth, the tie-down anchors will have to be designed to meet project specifications. Fifth, the volume of information including as-builts and the myriad of geotechnical information provided will consume considerable time which our designers can ill afford not to digest the pertinent information. And lastly, the tremendous coordination and evaluation of the various key subcontractors scopes and proposals will be itself a considerable effort since various proprietary information will be provided to them for pricing Tutor-Saliba's in-house designs. For these various reasons, we strongly urge a postponement of at least four weeks to the current scheduled bid date of September 14th in which we can properly develop the most complete and competitive design concepts and pricing proposal to Webcor/Obayashi. It is very important that you evaluate this request in a timely manner in which we can allocate the limited time available to our engineering and estimating forces. Your timely written response is appreciated.</p>			<p>Q - We have started into our second week of intensive review of bid documents and drawings in which to grasp what is prescriptive work and what requires additional contractor design and scheduling in our bid proposal preparation. Having been involved in not only pre-bid contractor designed support-of-excavations but final design and construction of numerous deep supported excavations in urban environments in numerous cities in California, we look forward to working on this unique and challenging project. This project brings additional elements to be considered during design of support-of-excavation and traffic supported trestles that would not normally be considered in below street level construction. One, the width of the supported excavation at approximately one hundred and eighty L.F., will require at least two intermediate vertical piles to support the horizontal bracing levels. Second, the need to incorporate both longitudinal and transverse traffic trestle decking whose vertical support locations will add to the constraints and interference with the location of the horizontal bracing levels. Third, the phasing of the shoring and excavation will require various locations of bracing levels of the CDSM cut-off walls. Fourth, the tie-down anchors will have to be designed to meet project specifications. Fifth, the volume of information including as-builts and the myriad of geotechnical information provided will consume considerable time which our designers can ill afford not to digest the pertinent information. And lastly, the tremendous coordination and evaluation of the various key subcontractors scopes and proposals will be itself a considerable effort since various proprietary information will be provided to them for pricing Tutor-Saliba's in-house designs. For these various reasons, we strongly urge a postponement of at least four weeks to the current scheduled bid date of September 14th in which we can properly develop the most complete and competitive design concepts and pricing proposal to Webcor/Obayashi. It is very important that you evaluate this request in a timely manner in which we can allocate the limited time available to our engineering and estimating forces. Your timely written</p>			



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Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/12/2010			response is appreciated.  Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/12/2010			
<b>TG03.00-0051</b>	<b>TG03 Question 0051 - Elevations</b>	<b>Closed</b>	<b>06</b>	<b>08/13/2010</b>	<b>08/19/2010</b>	<b>08/19/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference drawing sheet GT-1100 and drawing tables 3, 4, 7 & 8  Q - The lateral earth pressure diagram and tables 3&4 have the top street at elevation +4, but tables 7&8 show elevation +6 Which is correct  Submitted by Shad Gardner Balfour Beatty 08/13/2010			<b>ANSWER:</b>  Reference drawing sheet GT-1100 and drawing tables 3, 4, 7 & 8  Q - The lateral earth pressure diagram and tables 3&4 have the top street at elevation +4, but tables 7&8 show elevation +6 Which is correct  Submitted by Shad Gardner Balfour Beatty 08/13/2010			
<b>TG03.00-0052</b>	<b>TG03 Question 0052 - Mud Slab</b>	<b>Closed</b>	<b>06</b>	<b>08/16/2010</b>	<b>08/22/2010</b>	<b>08/17/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference drawing sheets S1-3003 & A1-8711.  Q - Detail 2/S1-3003 indicates a 3" Mud slab (SAD). Detail 2/A1-8711 indicates a 4" Mud Slab w/ 6"X6" Wire Mesh. Please confirm that the Architectural detail governs, and that the BSE sope ends at the top of Mud Slab and WPM and up by others.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010			<b>ANSWER:</b>  Reference drawing sheets S1-3003 & A1-8711.  Q - Detail 2/S1-3003 indicates a 3" Mud slab (SAD). Detail 2/A1-8711 indicates a 4" Mud Slab w/ 6"X6" Wire Mesh. Please confirm that the Architectural detail governs, and that the BSE sope ends at the top of Mud Slab and WPM and up by others.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010			
<b>TG03.00-0053</b>	<b>TG03 Question 0053 - Internal Bracing</b>	<b>Closed</b>	<b>06</b>	<b>08/16/2010</b>	<b>08/22/2010</b>	<b>08/18/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						









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<b>TG03.00-0055</b>	<b>TG03 Question 0055 - Internal Bracing</b>	<b>Closed</b>	<b>06</b>	<b>08/16/2010</b>	<b>08/22/2010</b>	<b>08/26/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference drawing sheet GT-2101.			Reference drawing sheet GT-2101.			
Q - Is the use of tiebacks acceptable for support of this wall segment? Is the project planning on eventually taking the properties under which such tiebacks would be placed?			Q - Is the use of tiebacks acceptable for support of this wall segment? Is the project planning on eventually taking the properties under which such tiebacks would be placed?			
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010			Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010			
<b>TG03.00-0056</b>	<b>TG03 Question 0056 - Access Trestle Permanent Structure</b>	<b>Closed</b>	<b>06</b>	<b>08/16/2010</b>	<b>08/22/2010</b>	<b>08/20/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Exhibit A, Attachment 3 A.			Reference Exhibit A, Attachment 3 A.			
Q - We note that the access trestle is to be coordinated with permanent construction, not conflict with the permanent structure except for penetrations, and is to be at the same level of the temporary bridges at the connections. This would appear to require that the trestle deck be below the bottom of the ground floor permanent structure section, with built-up ramps to match the street bridges. We also note that the permanent ground level structure is lower at the street crossings than elsewhere. This would push the trestle deck further down, conflicting with the limits of placement of the top level bracing strut. No guidance is given regarding how the future Trade Subcontractor will want to use the trestle to construct the ground floor or the superstructure. Please clarify where the top of trestle deck is intended to be located, and whether it is acceptable to locate the trestle deck at ground floor level, such that it could be used for sequential construction of the ground floor and superstructure. Is there an upturned longitudinal beam down the middle of the street crossings? . Schedule A on S1-3201 only indicates a 30" slab.			Q - We note that the access trestle is to be coordinated with permanent construction, not conflict with the permanent structure except for penetrations, and is to be at the same level of the temporary bridges at the connections. This would appear to require that the trestle deck be below the bottom of the ground floor permanent structure section, with built-up ramps to match the street bridges. We also note that the permanent ground level structure is lower at the street crossings than elsewhere. This would push the trestle deck further down, conflicting with the limits of placement of the top level bracing strut. No guidance is given regarding how the future Trade Subcontractor will want to use the trestle to construct the ground floor or the superstructure. Please clarify where the top of trestle deck is intended to be located, and whether it is acceptable to locate the trestle deck at ground floor level, such that it could be used for sequential construction of the ground floor and superstructure. Is there an upturned longitudinal beam down the middle of the street crossings? . Schedule A on S1-3201 only indicates a 30" slab.			
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010			Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010			



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TG03.00-0057	TG03 Question 0057 - Access Trestle	Closed	06	08/16/2010	08/22/2010	08/19/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Reference Exhibit A, Attachment 3		Reference Exhibit A, Attachment 3				
Q - Crane load is indicated to be considered at any location on the access trestle. Only total weight is given, not the concentrated load that occurs when the crane reaches out on one side or the other to the maximum under load. Please confirm that the trestle is to accommodate the crane operating, not just standing or walking, at any location on the trestle.		Q - Crane load is indicated to be considered at any location on the access trestle. Only total weight is given, not the concentrated load that occurs when the crane reaches out on one side or the other to the maximum under load. Please confirm that the trestle is to accommodate the crane operating, not just standing or walking, at any location on the trestle.				
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010		Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010				
TG03.00-0058	TG03 Question 0058 - Internal Bracing	Closed	06	08/17/2010	08/31/2010	08/22/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Reference drawing sheet GT-1111 Legend. A.		Reference drawing sheet GT-1111 Legend. A.				
Q - Please help to clarify the strut and waler system stiffness requirements. Our initial interpretation and the associated analyses indicate that strut and waler sizes increase very significantly over what would be required by strength considerations alone. Please provide a sample calculation or procedure for determining stiffness for comparison with the values given in kip per foot, per foot of wall. B. Pre-loading will take out a portion of the axial shortening of the struts. We assume that it is appropriate to subtract out that deflection from the stiffness calculation. Please confirm.		Q - Please help to clarify the strut and waler system stiffness requirements. Our initial interpretation and the associated analyses indicate that strut and waler sizes increase very significantly over what would be required by strength considerations alone. Please provide a sample calculation or procedure for determining stiffness for comparison with the values given in kip per foot, per foot of wall. B. Pre-loading will take out a portion of the axial shortening of the struts. We assume that it is appropriate to subtract out that deflection from the stiffness calculation. Please confirm.				
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010		Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010				
TG03.00-0059	TG03 Question 0059 - Demolition	Closed	06	08/16/2010	08/22/2010	08/23/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				



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	Reference D-1076 (Existing Terminal Demo).  Q - Existing Terminal Demolition Drawing D-1076 indicates (E) Cantilever Wall for 301 Mission St Building (60 story Tower) to be relocated by others. Please confirm this will be completed prior to the TG03 Work in this area.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010					Reference D-1076 (Existing Terminal Demo).  Q - Existing Terminal Demolition Drawing D-1076 indicates (E) Cantilever Wall for 301 Mission St Building (60 story Tower) to be relocated by others. Please confirm this will be completed prior to the TG03 Work in this area.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/13/2010
<b>TG03.00-0060</b>	<b>TG03 Question 0060 - Milestones</b>	<b>Closed</b>	<b>06</b>	<b>08/16/2010</b>	<b>08/22/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference Exhibit A Section V  Q - Milestones indicates the Trade Subcontractor is to provide all submittals within 10 days of NTP #1. This contradicts innumerable sections of the specifications which provide specific and reasonable time frames for submittals. It is not reasonable to expect all submittals to be delivered within 10 days of NTP #1. Please provide clarification on the contract requirements for delivery of submittals.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/13/2010						<b>ANSWER:</b> Reference Exhibit A Section V  Q - Milestones indicates the Trade Subcontractor is to provide all submittals within 10 days of NTP #1. This contradicts innumerable sections of the specifications which provide specific and reasonable time frames for submittals. It is not reasonable to expect all submittals to be delivered within 10 days of NTP #1. Please provide clarification on the contract requirements for delivery of submittals.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/13/2010



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<b>TG03.00-0061</b>	<b>TG03 Question 0061 - Micropile</b>	<b>Closed</b>	<b>06</b>	<b>08/16/2010</b>	<b>08/22/2010</b>	<b>08/17/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 31 63 33, 2.1.A.2.  Q - Specification require micropile contractor to select installation means & methods to develop load capacity & performance required by project documents. Paragraph 2.1.A.2 dictates an installation method of grout pressure at least 145 psi. We request this sentence is removed since it appears to conflict with objective of contractor selected construction procedures.  Submitted by Rob Jameson Malcolm Drilling 08/13/2010		<b>ANSWER:</b>  Reference specification 31 63 33, 2.1.A.2.  Q - Specification require micropile contractor to select installation means & methods to develop load capacity & performance required by project documents. Paragraph 2.1.A.2 dictates an installation method of grout pressure at least 145 psi. We request this sentence is removed since it appears to conflict with objective of contractor selected construction procedures.  Submitted by Rob Jameson Malcolm Drilling 08/13/2010				
<b>TG03.00-0062</b>	<b>TG03 Question 0062 - Micropile</b>	<b>Closed</b>	<b>06</b>	<b>08/17/2010</b>	<b>08/31/2010</b>	<b>08/17/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 32 63 33, 3.1.B. & 3.7.B.  Q - Section 3.7.B requires temporary casing or other method of drill hole support in caving or unstable ground. By reference to Section 3.1.B & 3.7.B we understand that use of fluid containing bentonite, drilling mud or chemical stabilizers will not be permitted on the project.  Submitted by Rob Jameson Malcolm Drilling 08/13/2010		<b>ANSWER:</b>  Reference specification 32 63 33, 3.1.B. & 3.7.B.  Q - Section 3.7.B requires temporary casing or other method of drill hole support in caving or unstable ground. By reference to Section 3.1.B & 3.7.B we understand that use of fluid containing bentonite, drilling mud or chemical stabilizers will not be permitted on the project.  Submitted by Rob Jameson Malcolm Drilling 08/13/2010				
<b>TG03.00-0063</b>	<b>TG03 Question 0063 - Micropile</b>	<b>Closed</b>	<b>06</b>	<b>08/16/2010</b>	<b>08/22/2010</b>	<b>08/17/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 33 63 33, 3.2.C.K.2.  Q - Performance test acceptance criteria is defined in terms of parameter "T" - maximum test load. Parameter T is not defined in performance test schedule. By referral to		<b>ANSWER:</b>  Reference specification 33 63 33, 3.2.C.K.2.  Q - Performance test acceptance criteria is defined in terms of parameter "T" - maximum test load. Parameter T is not defined in performance test				



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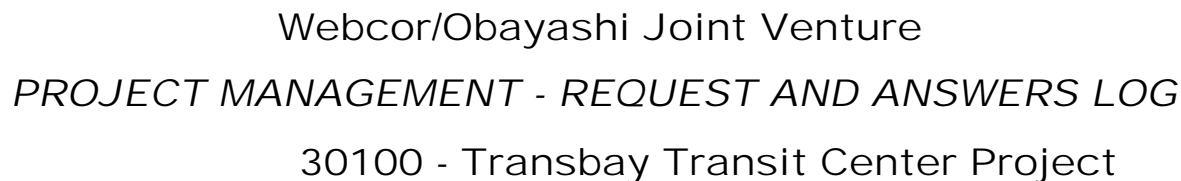
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	<p>proof test schedule we infer: T = 1.4 x 1.1 x Design Load Please confirm or provide definition of "T" which is applicable to performance test acceptance.</p> <p>Submitted by Rob Jameson Malcolm Drilling 08/13/2010</p>					<p>schedule. By referral to proof test schedule we infer: T = 1.4 x 1.1 x Design Load Please confirm or provide definition of "T" which is applicable to performance test acceptance.</p> <p>Submitted by Rob Jameson Malcolm Drilling 08/13/2010</p>
<b>TG03.00-0064</b>	<b>TG03 Question 0064 - Micropile</b>	<b>Closed</b>	<b>06</b>	<b>08/16/2010</b>	<b>08/22/2010</b>	<b>08/17/2010</b>
<p><b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Reference specification 34 63 33, 3.5.D. &amp; A.</p> <p>Q - Per Micropile allowable construction tolerance, centerline of piling shall not more than 3" from indicated location on drawings. We understand that centerline of reinforcing shall be not more than 0.5" from centerline of pile, I.E. tolerance is cumulative such that reinforcing cannot exceed 3.5" from plan centerline location. Please confirm.</p> <p>Submitted by Rob Jameson Malcolm Drilling 08/13/2010</p>			<p>Reference specification 34 63 33, 3.5.D. &amp; A.</p> <p>Q - Per Micropile allowable construction tolerance, centerline of piling shall not more than 3" from indicated location on drawings. We understand that centerline of reinforcing shall be not more than 0.5" from centerline of pile, I.E. tolerance is cumulative such that reinforcing cannot exceed 3.5" from plan centerline location. Please confirm.</p> <p>Submitted by Rob Jameson Malcolm Drilling 08/13/2010</p>			



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TG03.00-0065	TG03 Question 0065 - Bid Due Date	Closed	06	08/17/2010	08/24/2010	08/18/2010
<div><div><div>From: Webcor/Obayashi Joint Venture</div><div>Manuel Saldana</div></div><div>REQUEST: Our QBD 1 sent on 8/9/10 requested an 8-week extension to the bidding period and provided reasons for our request. We would like to reiterate our concern that it is not possible to provide an accurate design-build bid of this magnitude within the currently allocated 6-week period. (Bidder Name - hidden) requests that the CM/GC decide as soon as possible whether or not the bid period will be extended and notify all bidders of the decision. If SST does not receive notification of a bid extension by Friday, Aug. 20, it may determine that it cannot continue to participate in this procurement.  Submitted by Rich Zito Shimmick / Skanska / Traylor Joint Venture (SST) 08/17/2010</div><div>ANSWER: Our QBD 1 sent on 8/9/10 requested an 8-week extension to the bidding period and provided reasons for our request. We would like to reiterate our concern that it is not possible to provide an accurate design-build bid of this magnitude within the currently allocated 6-week period. (Bidder Name - hidden) requests that the CM/GC decide as soon as possible whether or not the bid period will be extended and notify all bidders of the decision. If SST does not receive notification of a bid extension by Friday, Aug. 20, it may determine that it cannot continue to participate in this procurement.  Submitted by Rich Zito Shimmick / Skanska / Traylor Joint Venture (SST) 08/17/2010</div></div>						
TG03.00-0066	TG03 Question 0066 - Temporary Power	Closed	06	08/18/2010	08/24/2010	08/23/2010
<div><div><div>From: Webcor/Obayashi Joint Venture</div><div>Manuel Saldana</div></div><div>REQUEST: Reference Proect Bidding Manual, IV.A.17(a)  Project Bidding Manual states, "Contractor will provide temporary power to distribution points in the Site Logistics plan (see Exhibit A) for the project. Subcontractor shall be responsible for all temporary power needs to complete their work beyond the distribution points provided by Contractor. Contractor will not provide temporary power for welding." A.Will the Owner (TJPA) pay for temporary power consumption, or does the Trade Subcontractor put thisin our Bid? B. Will the distribution points require separate metering for welding?  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/17/2010</div><div>ANSWER: Reference Proect Bidding Manual, IV.A.17(a)  Project Bidding Manual states, "Contractor will provide temporary power to distribution points in the Site Logistics plan (see Exhibit A) for the project. Subcontractor shall be responsible for all temporary power needs to complete their work beyond the distribution points provided by Contractor. Contractor will not provide temporary power for welding." A.Will the Owner (TJPA) pay for temporary power consumption, or does the Trade Subcontractor put thisin our Bid? B. Will the distribution points require separate metering for welding?  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/17/2010</div></div>						
TG03.00-0067	TG03 Question 0067 - Hazardous Waste	Closed	06	08/18/2010	09/01/2010	08/18/2010





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TG03.00-0069	TG03 Question 0069 - Permits	Closed	06	08/18/2010	08/24/2010	08/20/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Project Bidding Manual, IV.A.6 and specification 01 14 10.		Reference Project Bidding Manual, IV.A.6 and specification 01 14 10.				
Project Bidding Manual IV.4.6 a0 states "Trade Subcontractor shall obtain all required ancillary permits to complete their scope in a timely manner. Refer to Specifications Section 01 14 10 for project permits" Specification 01 14 10 does not distinguish between the Contractor and the Trade Subcontractor. Please clarify specifically which permits are considered "ancillary" and not reimbursed by TJPA. (I.e DPW Tree Planting / Removal, Rock Wheel? SFMTA Traffic Control Plan?, etc.)		Project Bidding Manual IV.4.6 a0 states "Trade Subcontractor shall obtain all required ancillary permits to complete their scope in a timely manner. Refer to Specifications Section 01 14 10 for project permits" Specification 01 14 10 does not distinguish between the Contractor and the Trade Subcontractor. Please clarify specifically which permits are considered "ancillary" and not reimbursed by TJPA. (I.e DPW Tree Planting / Removal, Rock Wheel? SFMTA Traffic Control Plan?, etc.)				
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/17/2010		Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/17/2010				
TG03.00-0070	TG03 Question 0070 - CDSM	Closed	06	08/18/2010	08/24/2010	08/19/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference drawing sheets GT2101, GT2101, GT2103, S1-2027, & S1-2030.		Reference drawing sheets GT2101, GT2101, GT2103, S1-2027, & S1-2030.				
The Drawings GT 2101, GT 2102, GT2103 show one CDSM Layout and Drawings S1 2022 thru S1 2027, S1 2030 show another CDSM Layout and the dimensioning do not match, the distance to CL of CDSM at A /1 and A / 35 do not match between the two different Layouts.		The Drawings GT 2101, GT 2102, GT2103 show one CDSM Layout and Drawings S1 2022 thru S1 2027, S1 2030 show another CDSM Layout and the dimensioning do not match, the distance to CL of CDSM at A /1 and A / 35 do not match between the two different Layouts.				
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/17/2010		Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/17/2010				





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TG03.00-0071	TG03 Question 0071 - As-Built Drawings	Closed	06	08/18/2010	08/24/2010	08/20/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 01 17 20.  Who is responsible for utility relocation as-built drawings, TG03 or TG04?  Submitted by Charles M. Gardner Kiewit Infrastrcutre West Co. 08/17/2010		<b>ANSWER:</b>  Reference specification 01 17 20.  Who is responsible for utility relocation as-built drawings, TG03 or TG04?  Submitted by Charles M. Gardner Kiewit Infrastrcutre West Co. 08/17/2010				
TG03.00-0072	TG03 Question 0072 - Electronic Files	Closed	06	08/18/2010	08/24/2010	08/18/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  1. Will the owner or general contractor please provide bidders with CADD digital files (preferably AutoCAD format) of the drawing provided to TG03 Trade Subcontractors?  2. Will the owner or general contractor please provide bidders with Primavera digital files for Exhibit I schedule contained in the bid package?  Submitted by Kelley Wigton Shimmick / Skanska / Traylor JV (SST) 08/17/2010		<b>ANSWER:</b>  1. Will the owner or general contractor please provide bidders with CADD digital files (preferably AutoCAD format) of the drawing provided to TG03 Trade Subcontractors?  2. Will the owner or general contractor please provide bidders with Primavera digital files for Exhibit I schedule contained in the bid package?  Submitted by Kelley Wigton Shimmick / Skanska / Traylor JV (SST) 08/17/2010				
TG03.00-0073	TG03 Question 0073 - Existing Piles and Pile Caps	Closed	06	08/18/2010	08/24/2010	08/19/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  After reviewing the reference documents we are not able to find details of the existing San Francisco Terminal pile caps and piles to be removed. Document No. 1 in the attached listing appears to be the one we need. Where can we get a copy of this document and any other pertinent as-built drawings for the existing terminal structure?  Submitted by Kelley Wigton		<b>ANSWER:</b>  After reviewing the reference documents we are not able to find details of the existing San Francisco Terminal pile caps and piles to be removed. Document No. 1 in the attached listing appears to be the one we need. Where can we get a copy of this document and any other pertinent as-built drawings for the existing terminal structure?  Submitted by Kelley Wigton				



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Shimmick / Skanska / Traylor JV (SST) 08/17/2010						Shimmick / Skanska / Traylor JV (SST) 08/17/2010
TG03.00-0074	TG03 Question 0074 - Tax Certificate	Closed	06	08/18/2010	08/24/2010	08/20/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST: Reference Project Bidding Manual, Section V, paragraph A (BCL), #3  Item # 3 requires each bidder to provide a specifically what certificate is required. Is this certificate issued by the State or by the Federal government? It is our understanding that General Partnerships are not usually registered with the State of California, therefore this document would presumably be a Federal certificate. Please advise.  Submitted by Chad Trabucco Shimmick / Skanska / Traylor JV (SST) 08/18/2010						ANSWER: Reference Project Bidding Manual, Section V, paragraph A (BCL), #3  Item # 3 requires each bidder to provide a specifically what certificate is required. Is this certificate issued by the State or by the Federal government? It is our understanding that General Partnerships are not usually registered with the State of California, therefore this document would presumably be a Federal certificate. Please advise.  Submitted by Chad Trabucco Shimmick / Skanska / Traylor JV (SST) 08/18/2010



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TG03.00-0075	TG03 Question 0075 - Temporary Bridge	Closed	06	08/19/2010	08/25/2010	08/25/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Reference specification 01 53 13, paragraph 1.3.A.1		Reference specification 01 53 13, paragraph 1.3.A.1				
Specification states "design shall include cross bridge travel including typical semi truck traffic and a fully assembled Manitowoc 999 crane weighing 500,000 lbs traveling from trestle to trestle without a hook load." Please verify it is the intent of the specifications that the 999 crane only travels across the temp street while moving from trestle to trestle and that the temp street design does not have to include the Manitowoc 999 loading at any other area of the temporary street.		Specification states "design shall include cross bridge travel including typical semi truck traffic and a fully assembled Manitowoc 999 crane weighing 500,000 lbs traveling from trestle to trestle without a hook load." Please verify it is the intent of the specifications that the 999 crane only travels across the temp street while moving from trestle to trestle and that the temp street design does not have to include the Manitowoc 999 loading at any other area of the temporary street.				
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 8/18/2010		Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 8/18/2010				
TG03.00-0076	TG03 Question 0076 - Access Trestle	Closed	06	08/19/2010	08/25/2010	08/19/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Reference Exhibit A		Reference Exhibit A				
Attachment 3, Access Trestle Criteria, states "The level of the access trestle shall be the same as the level of the Temporary Bridges at the connections." Attachment 3 also states "The layout for each member of the Access Trestle shall not conflict with the permanent structure." Tying the trestle to the cross streets will cause the trestle structure (which is approx 7' deep) to conflict with the concrete roof of the follow on structure. It is our understanding that the CM/GC understands this and will coordinate the removal of the access trestle and the temp streets such that the concrete roof can be constructed after these conflicting structures are removed. Pls confirm.		Attachment 3, Access Trestle Criteria, states "The level of the access trestle shall be the same as the level of the Temporary Bridges at the connections." Attachment 3 also states "The layout for each member of the Access Trestle shall not conflict with the permanent structure." Tying the trestle to the cross streets will cause the trestle structure (which is approx 7' deep) to conflict with the concrete roof of the follow on structure. It is our understanding that the CM/GC understands this and will coordinate the removal of the access trestle and the temp streets such that the concrete roof can be constructed after these conflicting structures are removed. Pls confirm.				
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/18/2010		Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/18/2010				



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TG03.00-0077	TG03 Question 0077 - Mat Slab Pile Sleeve	Closed	06	08/19/2010	08/25/2010	08/18/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST: Ref: 2/S1-3003  Slip Detail @ Trestle Pile/Mat connection shows a pipe sleeve over the trestle pipe pile to allow for vertical movement of the mat slab per Note 2. Not clear how that will be achieved since the detail shows the Mat slab with the mud slab directly bearing on the concrete encasement of the trestle pipe pile. Please clarify.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/18/2010		ANSWER: Ref: 2/S1-3003  Slip Detail @ Trestle Pile/Mat connection shows a pipe sleeve over the trestle pipe pile to allow for vertical movement of the mat slab per Note 2. Not clear how that will be achieved since the detail shows the Mat slab with the mud slab directly bearing on the concrete encasement of the trestle pipe pile. Please clarify.  Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/18/2010				
TG03.00-0078	TG03 Question 0078 - CSM	Closed	06	08/19/2010	08/25/2010	08/19/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST: Ref: CSM Width/GT-5101  Plan Sheet 35 of 105, GT-5101, detail 2 shows 3' diameter multi auger soil mixing or a cutter soil mixing system (CSM) with conflicting widths. The stated width is 3'-6", yet the schedule width equals 3'-0", which matches dimension of the multi auger system. However, a 30" wide CSM system and a 36" multi auger system provide the same minimum width. Please confirm the desired width of the CSM system, 30, 36, or 42 inches.  Submitted by Andres Melgoza Drill Tech & Shoring Inc 08/18/2010		ANSWER: Ref: CSM Width/GT-5101  Plan Sheet 35 of 105, GT-5101, detail 2 shows 3' diameter multi auger soil mixing or a cutter soil mixing system (CSM) with conflicting widths. The stated width is 3'-6", yet the schedule width equals 3'-0", which matches dimension of the multi auger system. However, a 30" wide CSM system and a 36" multi auger system provide the same minimum width. Please confirm the desired width of the CSM system, 30, 36, or 42 inches.  Submitted by Andres Melgoza Drill Tech & Shoring Inc 08/18/2010				
TG03.00-0079	TG03 Question 0079 - Insurance	Closed	06	08/19/2010	08/25/2010	08/23/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST: Reference General Requirements - Insurance		ANSWER: Reference General Requirements - Insurance				



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	<p>Says surveyor must carry professional liability of \$25 million/claim. What general liability would the surveyor need to carry? I assume it is not the \$100 million/occurrence as noted for the trade subcontractors. The insurance would cost the surveyor more than it would cost to survey the project.</p> <p>Submitted by Lyndi Love MVE 08/18/2010</p>					<p>Says surveyor must carry professional liability of \$25 million/claim. What general liability would the surveyor need to carry? I assume it is not the \$100 million/occurrence as noted for the trade subcontractors. The insurance would cost the surveyor more than it would cost to survey the project.</p> <p>Submitted by Lyndi Love MVE 08/18/2010</p>
<b>TG03.00-0080</b>	<b>TG03 Question 0080 - Schedule</b>	<b>Closed</b>	<b>06</b>	<b>08/19/2010</b>	<b>08/25/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Exhibit A, Section V						Reference Exhibit A, Section V
Reference NTP #6, 7, 8, 9, and 10. Please provide specific dates when the Trade Subcontractor will be required to perform the removal work associated with these NTP's. It is not possible to estimate costs for managing and maintaining this project without that specific information.						Reference NTP #6, 7, 8, 9, and 10. Please provide specific dates when the Trade Subcontractor will be required to perform the removal work associated with these NTP's. It is not possible to estimate costs for managing and maintaining this project without that specific information.
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/18/2010						Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/18/2010



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TG03.00-0081	TG03 Question 0081 - Police Officers	Closed	06	08/19/2010	08/25/2010	08/23/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference specification 01 15 70, 3.4.A		Reference specification 01 15 70, 3.4.A				
Section states "Contractor shall provide uniformed...police officers, as required by the TJPA representative..." It is our understanding that per the contract definitions, the Contractor is defined to be Webcor/Obayashi. Please confirm that Webcor/Obayashi will direct and pay the costs for the uniformed officers described herein. If it is the intent of the contract that the Trade Subcontractor direct and pay the costs for these officers, please provide specific guidelines on when these officers will be required. Simply stating "as required by TJPA" will result in exorbitant bid costs due to the lack of specific information provided. Suggest an allowance for this.		Section states "Contractor shall provide uniformed...police officers, as required by the TJPA representative..." It is our understanding that per the contract definitions, the Contractor is defined to be Webcor/Obayashi. Please confirm that Webcor/Obayashi will direct and pay the costs for the uniformed officers described herein. If it is the intent of the contract that the Trade Subcontractor direct and pay the costs for these officers, please provide specific guidelines on when these officers will be required. Simply stating "as required by TJPA" will result in exorbitant bid costs due to the lack of specific information provided. Suggest an allowance for this.				
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/18/2010		Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/18/2010				
TG03.00-0082	TG03 Question 0082 - Internal Bracing	Closed	06	08/19/2010	08/25/2010	08/25/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Ref: GT-1110		Ref: GT-1110				
Ref Note 3 "strut loads are working stress level." Regarding 301 Mission Buttress Case Table 3 & 7; is Table 7 loading cumulative, or must Table 3 & Table 7 be additive? Regardless of cumulative or additive do Table 3 & 7 loads represent "working stress level"?		Ref Note 3 "strut loads are working stress level." Regarding 301 Mission Buttress Case Table 3 & 7; is Table 7 loading cumulative, or must Table 3 & Table 7 be additive? Regardless of cumulative or additive do Table 3 & 7 loads represent "working stress level"?				
Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010		Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010				



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TG03.00-0083	TG03 Question 0083 - Dimensions	Closed	06	08/19/2010	08/25/2010	08/23/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Ref: GT-2101		Ref: GT-2101				
Reference drawing sheet GT-2101 Verify Shoring Wall Radius (594') at wall segment R2-1 and or dimensions to radius center line (170'-2 1/2" & 220'-9"). Radius & Center as identified do not work with layout as shown.		Reference drawing sheet GT-2101 Verify Shoring Wall Radius (594') at wall segment R2-1 and or dimensions to radius center line (170'-2 1/2" & 220'-9"). Radius & Center as identified do not work with layout as shown.				
Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010		Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010				
TG03.00-0084	TG03 Question 0084 - Dimensions	Closed	06	08/19/2010	08/25/2010	08/25/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Reference drawing sheet GT-2101		Reference drawing sheet GT-2101				
Ref Note #16 (RE: Wall Segment X1-1) 1. At what stage of excavation in zone #1 will wall X1-1 be removed? 2. Can tiebacks be used to support wall segment X1-1?		Ref Note #16 (RE: Wall Segment X1-1) 1. At what stage of excavation in zone #1 will wall X1-1 be removed? 2. Can tiebacks be used to support wall segment X1-1?				
Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010		Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010				



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<b>TG03.00-0085</b>	<b>TG03 Question 0085 - Cut Off Wall</b>	<b>Closed</b>	<b>06</b>	<b>08/19/2010</b>	<b>08/25/2010</b>	<b>08/20/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference drawing sheet GT-2103.			Reference drawing sheet GT-2103.			
Is cutoff wall between grids 33 & 34 required? Schedule appears to show excavation on both sides of this wall going down at the same time.			Is cutoff wall between grids 33 & 34 required? Schedule appears to show excavation on both sides of this wall going down at the same time.			
Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010			Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010			
<b>TG03.00-0086</b>	<b>TG03 Question 0086 - Wood Pile Removal</b>	<b>Closed</b>	<b>06</b>	<b>08/19/2010</b>	<b>08/25/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference drawing sheets GT-5000 & GT-5301.			Reference drawing sheets GT-5000 & GT-5301.			
1. Can wood piles be drilled out and material (Grout) replaced VRS extracted and grouted as shown.			1. Can wood piles be drilled out and material (Grout) replaced VRS extracted and grouted as shown.			
2. How does removal of wood piles and placement buttress piles work with regard to schedule.			2. How does removal of wood piles and placement buttress piles work with regard to schedule.			
Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010			Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010			





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<b>TG03.00-0087</b>	<b>TG03 Question 0087 - Dimensions</b>	<b>Closed</b>	<b>06</b>	<b>08/19/2010</b>	<b>08/25/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference drawing sheets S1-2030, S1-2029, & GT-2103  Drawings appear to have made contradiction regarding limits of contract VRS location of shoring wall at both SW and SE project corners.  Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010						<b>ANSWER:</b>  Reference drawing sheets S1-2030, S1-2029, & GT-2103  Drawings appear to have made contradiction regarding limits of contract VRS location of shoring wall at both SW and SE project corners.  Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010
<b>TG03.00-0088</b>	<b>TG03 Question 0088 - Train Platforms</b>	<b>Closed</b>	<b>06</b>	<b>08/19/2010</b>	<b>08/25/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Will train platforms be constructed prior to/after removal of wall & Trestle/Bridge vertical support removal?  Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010						<b>ANSWER:</b>  Will train platforms be constructed prior to/after removal of wall & Trestle/Bridge vertical support removal?  Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010
<b>TG03.00-0089</b>	<b>TG03 Question 0089 - Access Trestle</b>	<b>Closed</b>	<b>06</b>	<b>08/19/2010</b>	<b>08/25/2010</b>	<b>08/20/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 01 53 13, 1.3.A.2  Do deflection limits for temp bridges also apply to work trestle?  Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010						<b>ANSWER:</b>  Reference specification 01 53 13, 1.3.A.2  Do deflection limits for temp bridges also apply to work trestle?  Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010



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TG03.00-0090	TG03 Question 0090 - Internal Bracing	Closed	06	08/19/2010	08/25/2010	08/25/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST: Reference drawing sheet GT-1112  Vertical supports of shoring are shown in all stages up to stage 16. Vertical supports will still be required at stage 16 and and beyond to support work trestle & roadways.  Submitted by Gerald W. Brown Tutor-Salbia Corporation 08/19/2010		ANSWER: Reference drawing sheet GT-1112  Vertical supports of shoring are shown in all stages up to stage 16. Vertical supports will still be required at stage 16 and and beyond to support work trestle & roadways.  Submitted by Gerald W. Brown Tutor-Salbia Corporation 08/19/2010				
TG03.00-0091	TG03 Question 0091 - Mat Slab Pile Sleeve	Closed	06	08/19/2010	08/25/2010	08/23/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST: Reference drawing sheet 2/S1-3003  What is the intent of this detail, how does it work and at what stage of construction is it to be installed?  Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010		ANSWER: Reference drawing sheet 2/S1-3003  What is the intent of this detail, how does it work and at what stage of construction is it to be installed?  Submitted by Gerald W. Brown Tutor-Saliba Corporation 08/19/2010				
TG03.00-0092	TG03 Question 0092 - Insurance	Closed	06	08/19/2010	08/25/2010	08/23/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST: Reference Exhibit A, paragraph section VI  4.F. as written is commercially unavailable. The clause requires Trade Subcontractor to maintain professional liability coverage continuously throughout the the term of the Contract, and without lapse, for 10 years beyond the Contract Final Final Completion date. The maximum policy term commercially available is 10 years combined for the construction and extended reporting period. A more usualreporting period is 3 years. Please change the extended reporting period to 3 years, or revise the		ANSWER: Reference Exhibit A, paragraph section VI  4.F. as written is commercially unavailable. The clause requires Trade Subcontractor to maintain professional liability coverage continuously throughout the the term of the Contract, and without lapse, for 10 years beyond the Contract Final Final Completion date. The maximum policy term commercially available is 10 years combined for the construction and extended reporting period. A more usualreporting period is 3 years. Please change the extended				



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	<p>requirement to a total of 10 years. Carrying \$25,000,000 professional liability insurance for 10 years will addsignificant cost to the Project without corresponding benefit as the majority of the design performed by Trade Subcontractor is for temporary work rather than the permanent structure.</p> <p>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/19/2010</p>					<p>reporting period to 3 years, or revise the requirement to a total of 10 years. Carrying \$25,000,000 professional liability insurance for 10 years will addsignificant cost to the Project without corresponding benefit as the majority of the design performed by Trade Subcontractor is for temporary work rather than the permanent structure.</p> <p>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/19/2010</p>
TG03.00-0093	TG03 Question 0093 - Insurance	Closed	06	08/19/2010	08/25/2010	08/23/2010
<div><div>From: Webcor/Obayashi Joint Venture</div><div>Manuel Saldana</div></div>						
REQUEST:			ANSWER:			
<p>Reference Exhibit A, paragraph 1.B</p> <p>Section 1.B requires the Trade Subcontractor to maintain \$100,000,000 Commercial General Liability Insurance. Section 16.7 of the proposed subcontract between Webcor and the Trade Subcontractor requires that Sub-subcontractors carry the same amounts of coverage. Potential SBE sub-subcontractors will not be able to provide \$100,000,000 CGL. As a result, Trade Subcontractors will not be able to reach the 24% SBE Goal. It is highly likely that all of the Trade Subcontractors will offer 0% SBE participation as a result of section 16.7. Please advise if Webcor intends to modify section 16.7 and if so, how will it specifically be changed?</p> <p>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/19/2010</p>			<p>Reference Exhibit A, paragraph 1.B</p> <p>Section 1.B requires the Trade Subcontractor to maintain \$100,000,000 Commercial General Liability Insurance. Section 16.7 of the proposed subcontract between Webcor and the Trade Subcontractor requires that Sub-subcontractors carry the same amounts of coverage. Potential SBE sub-subcontractors will not be able to provide \$100,000,000 CGL. As a result, Trade Subcontractors will not be able to reach the 24% SBE Goal. It is highly likely that all of the Trade Subcontractors will offer 0% SBE participation as a result of section 16.7. Please advise if Webcor intends to modify section 16.7 and if so, how will it specifically be changed?</p> <p>Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/19/2010</p>			
TG03.00-0094	TG03 Question 0094 - Vibration Level	Closed	06	08/23/2010	08/30/2010	08/25/2010
<div><div>From: Webcor/Obayashi Joint Venture</div><div>Manuel Saldana</div></div>						
REQUEST:			ANSWER:			



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	Reference specification 00 35 65.  1.11.A. States "Limit or prohibit use of construction techniques that create high vibration levels. Do not drive piles"  1.11.C.3. States "Perform vibration intensive activities such as pile driving only on weekdays during daytime hours between 7 a.m. and 8 p.m."  These two sections contradict each other. Please confirm that pile driving, if desired, may be performed on this project.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/23/2010					
	Reference specification 00 35 65.  1.11.A. States "Limit or prohibit use of construction techniques that create high vibration levels. Do not drive piles"  1.11.C.3. States "Perform vibration intensive activities such as pile driving only on weekdays during daytime hours between 7 a.m. and 8 p.m."  These two sections contradict each other. Please confirm that pile driving, if desired, may be performed on this project.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/23/2010					
<b>TG03.00-0095</b>	<b>TG03 Question 0095 - Internal Bracing</b>	<b>Closed</b>	<b>06</b>	<b>08/23/2010</b>	<b>08/30/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 31 55 00.  31 55 00 Internal Bracing for Shoring Wall 1.5.N Please clarify if primary struts can be proof loaded prior to installation or if they must be proof loaded once in place and all connections made.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/23/2010						
<b>ANSWER:</b>  Reference specification 31 55 00.  31 55 00 Internal Bracing for Shoring Wall 1.5.N Please clarify if primary struts can be proof loaded prior to installation or if they must be proof loaded once in place and all connections made.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/23/2010						



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<b>TG03.00-0096</b>	<b>TG03 Question 0096 - Internal Bracing</b>	<b>Closed</b>	<b>06</b>	<b>08/23/2010</b>	<b>08/30/2010</b>	<b>08/26/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference drawing sheet GT-2101, note 16.  Note 16 on GT-2101 requires the internal bracing system to permit removal of wall segment X1-1 PRIOR to the completion of the excavation. When or at what stage of excavation will this wall be removed? Can tiebacks be used to support wall X1-1?  Submitted by Shad Gardner Balfour Beatty 08/23/2010						<b>ANSWER:</b>  Reference drawing sheet GT-2101, note 16.  Note 16 on GT-2101 requires the internal bracing system to permit removal of wall segment X1-1 PRIOR to the completion of the excavation. When or at what stage of excavation will this wall be removed? Can tiebacks be used to support wall X1-1?  Submitted by Shad Gardner Balfour Beatty 08/23/2010
<b>TG03.00-0097</b>	<b>TG03 Question 0097 - Internal Bracing</b>	<b>Closed</b>	<b>06</b>	<b>08/23/2010</b>	<b>08/30/2010</b>	<b>10/07/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference drawing sheet GT-1111.  GT-1111 requires a minimum stiffness of the internal bracing system which makes strut sizes dependent on the strut's length & spacing regardless of the load. This makes a very inefficient bracing system . Would the Authority consider providing an allowable design deflection criteria, in lieu of the stiffness requirement.  Submitted by Shad Gardner Balfour Beatty 08/23/2010						<b>ANSWER:</b>  Reference drawing sheet GT-1111.  GT-1111 requires a minimum stiffness of the internal bracing system which makes strut sizes dependent on the strut's length & spacing regardless of the load. This makes a very inefficient bracing system . Would the Authority consider providing an allowable design deflection criteria, in lieu of the stiffness requirement.  Submitted by Shad Gardner Balfour Beatty 08/23/2010
<b>TG03.00-0098</b>	<b>TG03 Question 0098 - Cut-Off Wall</b>	<b>Closed</b>	<b>06</b>	<b>08/23/2010</b>	<b>08/30/2010</b>	<b>08/25/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference GT drawing set.  The SCDSM cut-off walls are to be located by the contractor as needed. Do these walls serve any purpose for the final design? If not, can other earth retaining systems be used or could they be eliminated if they are						<b>ANSWER:</b>  Reference GT drawing set.  The SCDSM cut-off walls are to be located by the contractor as needed. Do these walls serve any purpose for the final design? If not, can other earth retaining systems be used or could they be eliminated



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	not needed by the contractor.					
	Submitted by Shad Gardner Balfour Beatty 08/23/2010					
						if they are not needed by the contractor.
						Submitted by Shad Gardner Balfour Beatty 08/23/2010
TG03.00-0099	TG03 Question 0099 - Dewatering	Closed	06	08/23/2010	08/30/2010	08/25/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:			ANSWER:			
Reference specification 31 23 19, paragraph 1.3.A, drawing sheet S1-2024, Note A Exhibit I, and Schedule (Dewatering).			Reference specification 31 23 19, paragraph 1.3.A, drawing sheet S1-2024, Note A Exhibit I, and Schedule (Dewatering).			
The above ref specification indicates TG03 dewatering system responsibility for duration of TG03 package. The referenced schedule shows dewatering thru March 2016. Note A DWS S1-2024 states dewatering maintained thru all dead load applications. Question: Does TG03 dewatering responsibility end Feb 2015 and remaining dewatering responsibility by subsequent contractors? (Pump Ownership/Pump/etc).			The above ref specification indicates TG03 dewatering system responsibility for duration of TG03 package. The referenced schedule shows dewatering thru March 2016. Note A DWS S1-2024 states dewatering maintained thru all dead load applications. Question: Does TG03 dewatering responsibility end Feb 2015 and remaining dewatering responsibility by subsequent contractors? (Pump Ownership/Pump/etc).			
Submitted by John Foote Balfour Beatty Infrastructure 08/23/2010			Submitted by John Foote Balfour Beatty Infrastructure 08/23/2010			





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	grade beams are removed under prior demolition contract on sheet D-2213.  Submitted by John Foote Balfour Beatty Infrastructure 08/23/2010					caps and grade beams are removed under prior demolition contract on sheet D-2213.  Submitted by John Foote Balfour Beatty Infrastructure 08/23/2010
<b>TG03.00-0103</b>	<b>TG03 Question 0103 - Monitoring</b>	<b>Closed</b>	<b>06</b>	<b>08/24/2010</b>	<b>08/31/2010</b>	<b>09/04/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 39 09 13  The plans show geotechnical instrumentation including inclinometers, MPBXs, piezometers and settlement points. Specification Section 39 09 13 states that "... Geotechnical instrumentation consists of inclinometers, settlement casings, settlement monitoring points, survey reference points, piezometers and multiple point borehole extensometers." The plans state that the geotechnical instruments for monitoring the TBT excavation and shoring work are to be drilled and installed by the TJPA representative. However, the specifications call for "...furnishing, installing, monitoring, reading, recording, maintaining, protecting ..... geotechnical instrumentation." The specifications go on to state that "...where shown on the drawings, the Contractor will procure and install the specified instrumentation." We find no notes on the plans calling for the Contractor to procure and install the specified instrumentation nor notes as to who is responsible for monitoring the shoring performance. The plans appear to be inconsistent with local practice in that it is customary in Northern California for the Contractor to furnish, install and monitor appropriate geotechnical instrumentation when the Contractor is responsible for constructing works involving deep excavations and shoring. We recognize the Internal Bracing Specification requires a monitoring program for struts, but is silent on exterior monitoring.  1. What is the Owner's intent in this regard?  2. Will the TJPA be responsible for the exterior monitoring						<b>ANSWER:</b>  Reference specification 39 09 13  The plans show geotechnical instrumentation including inclinometers, MPBXs, piezometers and settlement points. Specification Section 39 09 13 states that "... Geotechnical instrumentation consists of inclinometers, settlement casings, settlement monitoring points, survey reference points, piezometers and multiple point borehole extensometers." The plans state that the geotechnical instruments for monitoring the TBT excavation and shoring work are to be drilled and installed by the TJPA representative. However, the specifications call for "...furnishing, installing, monitoring, reading, recording, maintaining, protecting ..... geotechnical instrumentation." The specifications go on to state that "...where shown on the drawings, the Contractor will procure and install the specified instrumentation." We find no notes on the plans calling for the Contractor to procure and install the specified instrumentation nor notes as to who is responsible for monitoring the shoring performance. The plans appear to be inconsistent with local practice in that it is customary in Northern California for the Contractor to furnish, install and monitor appropriate geotechnical instrumentation when the Contractor is responsible for constructing works involving deep excavations and shoring. We recognize the Internal Bracing Specification requires a monitoring program for struts, but is silent on exterior monitoring.  1. What is the Owner's intent in this regard?







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08/24/2010				Granite / CJA / NCC Joint Venture 08/24/2010		
<b>TG03.00-0105</b>	<b>TG03 Question 0105 - Utilities</b>	<b>Closed</b>	<b>06</b>	<b>08/24/2010</b>	<b>08/31/2010</b>	<b>10/15/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Can we get a copy of Site Utilities Trade Packages:  Package TG04.7 Package TG04.1 Package TG04.3 Package TG04.4 Package TG04.6  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/24/2010				<b>ANSWER:</b> Can we get a copy of Site Utilities Trade Packages:  Package TG04.7 Package TG04.1 Package TG04.3 Package TG04.4 Package TG04.6  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/24/2010		
<b>TG03.00-0106</b>	<b>TG03 Question 0106 - Hazardous Material</b>	<b>Closed</b>	<b>06</b>	<b>08/24/2010</b>	<b>08/31/2010</b>	<b>08/25/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference Exhibit A  Please confirm that the "hazardous/High -PH" material that may result from the Perimeter Shoring Diaphragm wall, disposal costs will be included in the Shoring Wall Bid Items and not in the additive Class I and II Soil Disposal Premium.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/24/2010				<b>ANSWER:</b> Reference Exhibit A  Please confirm that the "hazardous/High -PH" material that may result from the Perimeter Shoring Diaphragm wall, disposal costs will be included in the Shoring Wall Bid Items and not in the additive Class I and II Soil Disposal Premium.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/24/2010		



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TG03.00-0107	TG03 Question 0107 - Internal Bracing	Closed	06	08/24/2010	08/31/2010	08/25/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 31 55 00.  In Spec Section 31 55 00 on Page 5 Sub-Section 1.5.B.3. it says that we are to include incidental loads defined by the Contractor (Webcor/Ob??). Can you Please define these loads now during the Bidding Process?  Submitted by Chrales M. Gardner Kiewit Infrastructure West Co. 08/24/2010						<b>ANSWER:</b>  Reference specification 31 55 00.  In Spec Section 31 55 00 on Page 5 Sub-Section 1.5.B.3. it says that we are to include incidental loads defined by the Contractor (Webcor/Ob??). Can you Please define these loads now during the Bidding Process?  Submitted by Chrales M. Gardner Kiewit Infrastructure West Co. 08/24/2010
TG03.00-0108	TG03 Question 0108 - Internal Bracing	Closed	06	08/24/2010	08/31/2010	08/27/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference drawing sheet GT-1111, Legend  (TG0300-0058) requested: A. Please help to clarify the strut and waler system stiffness requirements. Our initial interpretation and the associated analyses indicate that strut and waler sizes increase very significantly over what would be required by strength considerations alone. Please provide a sample calculation or procedure for determining stiffness for comparison with the values given in kip per foot, per foot of wall. B. Pre-loading will take out a portion of the axial shortening of the struts. We assume that it is appropriate to subtract out that deflection from the stiffness calculation. Please confirm.  Q/A Answers received this morning did not address this question, and can have a significant impacton the Contractor's design and potential for competetive underbidding of this project. We request your clarification on a priority basis as this may affect our decision to Bid this project.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/24/2010						<b>ANSWER:</b>  Reference drawing sheet GT-1111, Legend  (TG0300-0058) requested: A. Please help to clarify the strut and waler system stiffness requirements. Our initial interpretation and the associated analyses indicate that strut and waler sizes increase very significantly over what would be required by strength considerations alone. Please provide a sample calculation or procedure for determining stiffness for comparison with the values given in kip per foot, per foot of wall. B. Pre-loading will take out a portion of the axial shortening of the struts. We assume that it is appropriate to subtract out that deflection from the stiffness calculation. Please confirm.  Q/A Answers received this morning did not address this question, and can have a significant impacton the Contractor's design and potential for competetive underbidding of this project. We request your clarification on a priority basis as this may affect our decision to Bid this project.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/24/2010



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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
TG03.00-0109	TG03 Question 0109 - Utilities	Closed	06	08/25/2010	09/01/2010	08/27/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Response to question TG003-0044 states "Coordinate with and protect in place New active utilities (PG&E and Verizon) constructed by the private utilities that will be supported by the temporary bridge." Please advise where is the information regarding these new utilities so the Trade Subcontractor can review them prior to bid? How does the Trade Subcontractor obtain this information?		Response to question TG003-0044 states "Coordinate with and protect in place New active utilities (PG&E and Verizon) constructed by the private utilities that will be supported by the temporary bridge." Please advise where is the information regarding these new utilities so the Trade Subcontractor can review them prior to bid? How does the Trade Subcontractor obtain this information?				
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010		Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010				
TG03.00-0110	TG03 Question 0110 - Utilities	Closed	06	08/25/2010	09/01/2010	08/27/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Response to question TG003-0044 states "Protect in place New active sewers constructed as part of the Relocation of Utilities Project as shown on the plans." Please confirm that the project referenced is TG 04.5.1. As these are the only utility relocation plans available to the Trade Subcontractor, we need confirmation that these are the only drawings that must be reviewed prior to bid. If there are other plans the Trade Subcontractor must review in order to ascertain the impacts of new utility relocations, advise where they can be obtained.		Response to question TG003-0044 states "Protect in place New active sewers constructed as part of the Relocation of Utilities Project as shown on the plans." Please confirm that the project referenced is TG 04.5.1. As these are the only utility relocation plans available to the Trade Subcontractor, we need confirmation that these are the only drawings that must be reviewed prior to bid. If there are other plans the Trade Subcontractor must review in order to ascertain the impacts of new utility relocations, advise where they can be obtained.				
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010		Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010				
TG03.00-0111	TG03 Question 0111 - Schedule	Closed	06	08/25/2010	09/01/2010	08/25/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Reference BSE Concept Schedule.		Reference BSE Concept Schedule.				





# Webcor/Obayashi Joint Venture

## PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

### 30100 - Transbay Transit Center Project

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<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
<b>TG03.00-0113</b>	<b>TG03 Question 0113 - Schedule</b>	<b>Closed</b>	<b>06</b>	<b>08/25/2010</b>	<b>09/01/2010</b>	<b>08/25/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference BSE Concept Schedule.			Reference BSE Concept Schedule.			
Activity UT-201800, titled "Available: Start Phase 1 Cross Cross Shoring @ 1st Street". Please explain what this activity represents. What work is the Trade Subcontractor unable to perform before 15Jul11? Is this date still accurate? Please clarify.			Activity UT-201800, titled "Available: Start Phase 1 Cross Cross Shoring @ 1st Street". Please explain what this activity represents. What work is the Trade Subcontractor unable to perform before 15Jul11? Is this date still accurate? Please clarify.			
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010			Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010			
<b>TG03.00-0114</b>	<b>TG03 Question 0114 - Schedule</b>	<b>Closed</b>	<b>06</b>	<b>08/25/2010</b>	<b>09/01/2010</b>	<b>08/25/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference BSE Concept Schedule.			Reference BSE Concept Schedule.			
Activity UT-202400, titled "Franchise Utilities Phase 2 @ 1st". Please explain what specific work this activity represents.			Activity UT-202400, titled "Franchise Utilities Phase 2 @ 1st". Please explain what specific work this activity represents.			
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010			Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010			
<b>TG03.00-0115</b>	<b>TG03 Question 0115 - Schedule</b>	<b>Closed</b>	<b>06</b>	<b>08/25/2010</b>	<b>09/01/2010</b>	<b>08/25/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference BSE Concept Schedule.			Reference BSE Concept Schedule.			
Activity UT-200600, titled "Available: Start Shoring @ Zone 1 & 2 Minna"has a start date of 15Jul11. Please explain what this date means. Is the trade subcontractorto understand it cannot begin any zone 1 and 2 cdsm work (including pre-trenching) untilthis date? If so, is the date still accurate? Please clarify.			Activity UT-200600, titled "Available: Start Shoring @ Zone 1 & 2 Minna"has a start date of 15Jul11. Please explain what this date means. Is the trade subcontractorto understand it cannot begin any zone 1 and 2 cdsm work (including pre-trenching) untilthis date? If so, is the date still accurate? Please clarify.			



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30100 - Transbay Transit Center Project

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Job: 30100

<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
TG03.00-0118	TG03 Question 0118 - Schedule	Closed	06	08/25/2010	09/01/2010	08/25/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Reference BSE Concept Schedule.		Reference BSE Concept Schedule.				
Activity UT-201100, titled "Available: Start Shoring @ Zone 3 Natoma" has a start date of 18Mar11. Please explain what this date means. Is the trade subcontractor to understand it cannot begin any zone 3 cdsm work (including pre-trenching) until this date? If so, is the date still accurate? Please clarify.		Activity UT-201100, titled "Available: Start Shoring @ Zone 3 Natoma" has a start date of 18Mar11. Please explain what this date means. Is the trade subcontractor to understand it cannot begin any zone 3 cdsm work (including pre-trenching) until this date? If so, is the date still accurate? Please clarify.				
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010		Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010				
TG03.00-0119	TG03 Question 0119 - Shoring Wall	Closed	06	08/25/2010	09/01/2010	08/30/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Reference drawing sheet GT-1110.		Reference drawing sheet GT-1110.				
Drawing defines four different design cases for temporary shoring design. Please specify limits for each case relative to building column lines.		Drawing defines four different design cases for temporary shoring design. Please specify limits for each case relative to building column lines.				
Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010		Submitted by Kelly Turner Granite / CJA / NCC Joint Venture 08/24/2010				
TG03.00-0120	TG03 Question 0120 - Dewatering	Closed	06	08/25/2010	09/01/2010	08/25/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Reference specification 31 23 19, paragraph 1.1.A.		Reference specification 31 23 19, paragraph 1.1.A.				
Please refer to our previous inquiry regarding dewatering. (TG0300-0099) The bid form docs show 72 mo for maint. However, the above spec section allows for "transfer of ownership." Our concern for 72 mo has to do with issues related to bond limits/duration; definition of final completion; and retention release. Also, what is the		Please refer to our previous inquiry regarding dewatering. (TG0300-0099) The bid form docs show 72 mo for maint. However, the above spec section allows for "transfer of ownership." Our concern for 72 mo has to do with issues related to bond limits/duration; definition of final completion; and				





<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
	warranty period and when does it commence.  Submitted by John Foote Balfour Beatty Infrastructure 08/25/2010					retention release. Also, what is the warranty period and when does it commence.  Submitted by John Foote Balfour Beatty Infrastructure 08/25/2010
<b>TG03.00-0121</b>	<b>TG03 Question 0121 - Utilities</b>	<b>Closed</b>	<b>06</b>	<b>08/25/2010</b>	<b>09/01/2010</b>	<b>08/25/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Referene drawing sheet U-2009.  Per Transbay Transit Center Program Relocation of Utilities Project drawing sheet U-1121 (30 of 172) issued 8-6-10 there are 2 large vaults indicated on the SW corner of Minna and 1st Streets. According to the Demolition and Construction Sequence note 6 ¿after electric services are connected and existing electric ductbank is abandoned by PG&E, demolish as indicated existing electrical ductbank manholes, and contents to the limits shown¿ as well as all other utilities that run North and South on 1st Street between Minna and Natoma. Drawing sheet U-2009 (50 of 172) do not indicate these utilities in the Composite Utility Plan and Elevation. Please confirm as per Transbay Transit Center Program Butress/ Shoring/ Excavation drawing D-2230 detail 1 Remove Utilities that the utilities removal will be complete by the TG03 contract start date.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/25/2010						<b>ANSWER:</b> Referene drawing sheet U-2009.  Per Transbay Transit Center Program Relocation of Utilities Project drawing sheet U-1121 (30 of 172) issued 8-6-10 there are 2 large vaults indicated on the SW corner of Minna and 1st Streets. According to the Demolition and Construction Sequence note 6 ¿after electric services are connected and existing electric ductbank is abandoned by PG&E, demolish as indicated existing electrical ductbank manholes, and contents to the limits shown¿ as well as all other utilities that run North and South on 1st Street between Minna and Natoma. Drawing sheet U-2009 (50 of 172) do not indicate these utilities in the Composite Utility Plan and Elevation. Please confirm as per Transbay Transit Center Program Butress/ Shoring/ Excavation drawing D-2230 detail 1 Remove Utilities that the utilities removal will be complete by the TG03 contract start date.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/25/2010

<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
TG03.00-0122	TG03 Question 0122 - Logistics	Closed	06	08/25/2010	09/01/2010	08/25/2010
<div> <div> <b>From:</b> Webcor/Obayashi Joint Venture           Manuel Saldana         </div> <div> <b>REQUEST:</b>            Reference Project Bid Manual IV.A.12.a, 27.b, and Site Logistics Exhibit A.             These sections reference material/personnel hoists. Is the TG03 Contractor to provide access for the follow on trade subcontracts? Please provide specifications for size, type, and capacity, otherwise hoists will be designed to minimum requirements for this Trade Subcontractor to complete its work.             Submitted by Charles M. Gardner            Kiewit Infrastructure West Co.            08/25/2010         </div> <div> <b>ANSWER:</b>            Reference Project Bid Manual IV.A.12.a, 27.b, and Site Logistics Exhibit A.             These sections reference material/personnel hoists. Is the TG03 Contractor to provide access for the follow on trade subcontracts? Please provide specifications for size, type, and capacity, otherwise hoists will be designed to minimum requirements for this Trade Subcontractor to complete its work.             Submitted by Charles M. Gardner            Kiewit Infrastructure West Co.            08/25/2010         </div> </div>						





<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
<b>TG03.00-0124</b>	<b>TG03 Question 0124 - Warranties</b>	<b>Closed</b>	<b>06</b>	<b>08/25/2010</b>	<b>09/01/2010</b>	<b>09/08/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference specification 01 17 40.			Reference specification 01 17 40.			
1) Please confirm that the 2 year warranty for subsurface work applies to this Trade Subcontractor package			1) Please confirm that the 2 year warranty for subsurface work applies to this Trade Subcontractor package			
2) Request a copy of the Contractor's Builder's Risk insurance so we can review terms and conditions.			2) Request a copy of the Contractor's Builder's Risk insurance so we can review terms and conditions.			
3) Does the California Public Contract Code 7105 (Acts of God) statute apply in this Trade Subcontract?			3) Does the California Public Contract Code 7105 (Acts of God) statute apply in this Trade Subcontract?			
4) Will a Contractors Protective Professional Indemnity policy in the amounts specified in Exhibit A section VI be sufficient evidence of coverage to the Owner? 5) Request the general liability requirements be amended to more customary rated A-VII or higher			4) Will a Contractors Protective Professional Indemnity policy in the amounts specified in Exhibit A section VI be sufficient evidence of coverage to the Owner? 5) Request the general liability requirements be amended to more customary rated A-VII or higher			
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/25/2010			Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/25/2010			
<b>TG03.00-0125</b>	<b>TG03 Question 0125 - QBD</b>	<b>Closed</b>	<b>06</b>	<b>08/25/2010</b>	<b>09/01/2010</b>	<b>08/27/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Project Bidding Manual 37/44.			Reference Project Bidding Manual 37/44.			
Please refer to the attached spreadsheet for status of QBD's submitted, corresponding TG Question number and response dates. Currently, there are a number (18) QBD's unanswered that were submitted as of 8/20/10.			Please refer to the attached spreadsheet for status of QBD's submitted, corresponding TG Question number and response dates. Currently, there are a number (18) QBD's unanswered that were submitted as of 8/20/10.			
Request your review of this list and response to the unanswered QBD's as soon as possible.			Request your review of this list and response to the unanswered QBD's as soon as possible.			
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/25/2010			Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/25/2010			



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>TG03.00-0126</b>	<b>TG03 Question 0126 - Shoring Wall</b>	<b>Closed</b>	<b>06</b>	<b>08/27/2010</b>	<b>09/03/2010</b>	<b>09/02/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference drawing sheet GT-5102.			Reference drawing sheet GT-5102.			
Section 9/GT-5102 Indicates CLSM Shoring wall placed abutting the existing 301 Mission Wall. 301 Mission Drawings Sheet SH-31 Wall D section indicates a grade beam which may extend south past the wall line. This may interfere with the CLSM Shoring wall installation. Please provide more detail to confirm the coordinates of the existing building at 301 mission and the interface with the new CLSM wall, and confirm that the CLSM wall is sufficient to act as one side of shoring for the existing pile removal program.			Section 9/GT-5102 Indicates CLSM Shoring wall placed abutting the existing 301 Mission Wall. 301 Mission Drawings Sheet SH-31 Wall D section indicates a grade beam which may extend south past the wall line. This may interfere with the CLSM Shoring wall installation. Please provide more detail to confirm the coordinates of the existing building at 301 mission and the interface with the new CLSM wall, and confirm that the CLSM wall is sufficient to act as one side of shoring for the existing pile removal program.			
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/26/2010			Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 08/26/2010			
<b>TG03.00-0127</b>	<b>TG03 Question 0127 - Temporary Power</b>	<b>Closed</b>	<b>06</b>	<b>08/27/2010</b>	<b>09/03/2010</b>	<b>08/30/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference page 22 of 44, note #17.			Reference page 22 of 44, note #17.			
Project Bid Manual Temp Power Page 22 of 44 note #17; calls out for Temp Power per Site Logistics Plan in exhibit A. This is on sheet SL-003 (see attached) Exhibit A - Scope of Package (general work) Page 6 calls for Temp Power skids to be used for dewatering only Base Bid Item Scope Page 11 #18 Dewatering System calls out for power to be provided per attachment #2 which is the Site Logistics Plan in exhibit A drawing SL-003. In this paragraph it also says that there might be power available for our use in this scope of work. Do we need to provide the 4-skid units as shown on attachment #2 Site Logistics Plan? If so please electrical load and voltage requirements. Documents imply there is an existing temp power system for the dewatering? If so please provide information & how it is to be modified for this project.			Project Bid Manual Temp Power Page 22 of 44 note #17; calls out for Temp Power per Site Logistics Plan in exhibit A. This is on sheet SL-003 (see attached) Exhibit A -Scope of Package (general work) Page 6 calls for Temp Power skids to be used for dewatering only Base Bid Item Scope Page 11 #18 Dewatering System calls out for power to be provided per attachment #2 which is the Site Logistics Plan in exhibit A drawing SL-003. In this paragraph it also says that there might be power available for our use in this scope of work. Do we need to provide the 4-skid units as shown on attachment #2 Site Logistics Plan? If so please electrical load and voltage requirements. Documents imply there is an existing temp power system for the dewatering? If so please provide information & how it is to be modified for this project.			





<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>TG03.00-0129</b>	<b>TG03 Question 0129 - Temporary Lighting</b>	<b>Closed</b>	<b>06</b>	<b>08/27/2010</b>	<b>09/03/2010</b>	<b>08/30/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Exhibit A - Scope of Package (General Work page 6 state.			Reference Exhibit A - Scope of Package (General Work page 6 state.			
Exhibit A - Scope of Package (general work) Page 6 States 18 Temporary lighting Trade Subcontractor shall be responsible for installing and maintaining temporary lighting at the perimeter traffic/pedestrian barricades, at pedestrian walkways, and as required to provide code-minimum lighting at egress paths, as well as sufficient foot candle lighting levels to safety perform the work at all times, including within the excavation. At a minimum, Trade Subcontractor's lighting plan will include temporary poles at street level. In addition to supporting lighting, temporary poles shall include conduit for security cameras, power at the pole tops for security cameras, and mounting hardware for security cameras. Security cameras will be installed and maintained by others. Temporary lighting work item includes, but is not limited to, installing lighting poles, installing all hardware, switch boxes, breakers, conduits, and pulling strings among temporary power skids/generators/lighting poles and maintenance required for temporary lighting works. Trade Subcontractor's lighting plan will be a submittal requirement for the project.			Exhibit A - Scope of Package (general work) Page 6 States 18 Temporary lighting Trade Subcontractor shall be responsible for installing and maintaining temporary lighting at the perimeter traffic/pedestrian barricades, at pedestrian walkways, and as required to provide code-minimum lighting at egress paths, as well as sufficient foot candle lighting levels to safety perform the work at all times, including within the excavation. At a minimum, Trade Subcontractor's lighting plan will include temporary poles at street level. In addition to supporting lighting, temporary poles shall include conduit for security cameras, power at the pole tops for security cameras, and mounting hardware for security cameras. Security cameras will be installed and maintained by others. Temporary lighting work item includes, but is not limited to, installing lighting poles, installing all hardware, switch boxes, breakers, conduits, and pulling strings among temporary power skids/generators/lighting poles and maintenance required for temporary lighting works. Trade Subcontractor's lighting plan will be a submittal requirement for the project.			
Do we need to provide pricing for this scope of work? If so, are there drawings showing existing conditions and areas that require Temp Lighting, Street Lighting, and Pedestrian & Traffic Signal, size of Generator required, CCTV, and Electric Security requirements.			Do we need to provide pricing for this scope of work? If so, are there drawings showing existing conditions and areas that require Temp Lighting, Street Lighting, and Pedestrian & Traffic Signal, size of Generator required, CCTV, and Electric Security requirements.			



# Webcor/Obayashi Joint Venture

## PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

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<b>TG03.00-0130</b>	<b>TG03 Question 0130 - Temporary Power</b>	<b>Closed</b>	<b>06</b>	<b>08/27/2010</b>	<b>09/03/2010</b>	<b>08/30/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference Base Bid Items Scope #1.  Base Bid Item Scope #1 Mobilization refers you to Section 01 15 05 which calls out in Summary of Work #3, to install temporary construction power and wiring. What temp power is this referring to? (Lay-down area, office trailers, etc.)  Are we to provide pricing for this scope of work? If so, please define the scope of work required in SOW #3.						<b>ANSWER:</b>  Reference Base Bid Items Scope #1.  Base Bid Item Scope #1 Mobilization refers you to Section 01 15 05 which calls out in Summary of Work #3, to install temporary construction power and wiring. What temp power is this referring to? (Lay-down area, office trailers, etc.)  Are we to provide pricing for this scope of work? If so, please define the scope of work required in SOW #3.
<b>TG03.00-0131</b>	<b>TG03 Question 0131 - Temporary Lighting</b>	<b>Closed</b>	<b>06</b>	<b>08/27/2010</b>	<b>09/03/2010</b>	<b>08/30/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Page 14 - Exhibit A #23 5- 23 Bridge at First Street call for street lighting at pedestrian walkways and hanging/un-hanging of existing utilities Page 13 Exhibit #24 Bridge at First Street call for removal street lighting.  Do we need to provide temp lighting, or permanent lighting or both?  Is lighting required on the bridge and on the underside? If so, what are the lighting requirements?  Does the bridge work referenced here pertain to only the portion of the bridge that crosses over 1st street?						<b>ANSWER:</b>  Page 14 - Exhibit A #23 5- 23 Bridge at First Street call for street lighting at pedestrian walkways and hanging/un-hanging of existing utilities Page 13 Exhibit #24 Bridge at First Street call for removal street lighting.  Do we need to provide temp lighting, or permanent lighting or both?  Is lighting required on the bridge and on the underside? If so, what are the lighting requirements?  Does the bridge work referenced here pertain to only the portion of the bridge that crosses over 1st street?
<b>TG03.00-0132</b>	<b>TG03 Question 0132 - Schedule</b>	<b>Closed</b>	<b>06</b>	<b>08/27/2010</b>	<b>09/03/2010</b>	<b>08/30/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  CPM: The concept schedule provided in the Bid Documents does not provide sufficient details for Subcontractors to review risk and workforce requirements. We are requesting you to provide electronic Primavera files for the concept schedule, so we can apply sorts,						<b>ANSWER:</b>  CPM: The concept schedule provided in the Bid Documents does not provide sufficient details for Subcontractors to review risk and workforce requirements. We are requesting you to provide electronic Primavera files for the concept schedule, so





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PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

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	<div>review calendars, work weeks, restrictions, etc. Cost information can be terminated from these file, as we do not need that information.</div>					<div>we can apply sorts, review calendars, work weeks, restrictions, etc. Cost information can be terminated from these file, as we do not need that information.</div>
TG03.00-0133	TG03 Question 0133 - Insurance	Closed	06	08/31/2010	09/07/2010	08/31/2010
<div>From: Webcor/Obayashi Joint Venture      Manuel Saldana</div>						
REQUEST:			ANSWER:			
Reference specification 00 08 05 and Exhibit A IV.2.A.			Reference specification 00 08 05 and Exhibit A IV.2.A.			
Professional Liability Insurance limits cahnged to \$2,000,000 each occurrence in addendum #2 in spec section 00 08 05. Please confirm limits changed in Exhibit A VI.2.A to \$2,000,000 as well.			Professional Liability Insurance limits cahnged to \$2,000,000 each occurrence in addendum #2 in spec section 00 08 05. Please confirm limits changed in Exhibit A VI.2.A to \$2,000,000 as well.			
TG03.00-0134	TG03 Question 0134 - Temporary Bridge	Closed	06	08/31/2010	09/07/2010	09/07/2010
<div>From: Webcor/Obayashi Joint Venture      Manuel Saldana</div>						
REQUEST:			ANSWER:			
In regards to the temporary bridges at 1st, Fremont and Beale St. The contractor is to reference Spec. # 01 15 70-2 and # 01 53 13 -3.6. Section 01 15 70-2 states we are to provide three lanes at 11'. Section 01 53 13-3.6 calls for one 10' pedestrian path and three barriers assumed 1'-6" at the base. These dimensions add up to 47'-6".			In regards to the temporary bridges at 1st, Fremont and Beale St. The contractor is to reference Spec. # 01 15 70-2 and # 01 53 13 -3.6. Section 01 15 70-2 states we are to provide three lanes at 11'. Section 01 53 13-3.6 calls for one 10' pedestrian path and three barriers assumed 1'-6" at the base. These dimensions add up to 47'-6".			
Exhibit A Trade Subcontractor Bid Package Drawing SL-001 shows road widths of 36" at these locations.			Exhibit A Trade Subcontractor Bid Package Drawing SL-001 shows road widths of 36" at these locations.			
Please confirm total width to be 47'6"			Please confirm total width to be 47'6"			





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<div><div>REQUEST:</div><div>Reference Exhibit A (NTPs).</div><div>There are no set dates for NTP's 6-10 and they are contingent on follow on Trade Subcontractor schedules. All of the excavation is required to be completed in 2014. Are NTP's 6-10 going to be issued in a timely manner to ensure the subcontractor is not waiting idle?</div></div> <div><div>ANSWER:</div><div>Reference Exhibit A (NTPs).</div><div>There are no set dates for NTP's 6-10 and they are contingent on follow on Trade Subcontractor schedules. All of the excavation is required to be completed in 2014. Are NTP's 6-10 going to be issued in a timely manner to ensure the subcontractor is not waiting idle?</div></div>						
TG03.00-0139	TG03 Question 0139 - Access Trestle	Closed	06	09/01/2010	09/08/2010	09/07/2010
<div><div>From: Webcor/Obayashi Joint Venture</div><div>Manuel Saldana</div><div>REQUEST:</div><div>Reference Attachment 3. Please confirm that all horizontal members of the access trestle must be above the ground floor slab at all locations.</div></div> <div><div>ANSWER:</div><div>Reference Attachment 3. Please confirm that all horizontal members of the access trestle must be above the ground floor slab at all locations.</div></div>						
TG03.00-0140	TG03 Question 0140 - Business Tax Registration	Closed	06	09/01/2010	09/08/2010	09/13/2010
<div><div>From: Webcor/Obayashi Joint Venture</div><div>Manuel Saldana</div><div>REQUEST:</div><div>Reference is made to Part III. Instruction to Bidders, Subparagraph D., Bidding Process and Procedures, Item 6. Statutory Bidding Requirements, Subitem b) Tax Registration that was changed per Addendum No. 2 and states "Bidder shall list its current contractor license number on the Business Tax Registration Declaration (Section 00 04 54) its San Francisco business tax registration certificate number, as well as the current contractor license number and San Francisco business tax registration certificate number for each Subcontractor listed on the Subcontract list". This form was not changed per Addendum No. 2 and does not contain spaces for us to include this information. Is this form going to be revised or are we just to type this information anywhere on the form?</div></div> <div><div>ANSWER:</div><div>Reference is made to Part III. Instruction to Bidders, Subparagraph D., Bidding Process and Procedures, Item 6. Statutory Bidding Requirements, Subitem b) Tax Registration that was changed per Addendum No. 2 and states "Bidder shall list its current contractor license number on the Business Tax Registration Declaration (Section 00 04 54) its San Francisco business tax registration certificate number, as well as the current contractor license number and San Francisco business tax registration certificate number for each Subcontractor listed on the Subcontract list". This form was not changed per Addendum No. 2 and does not contain spaces for us to include this information. Is this form going to be revised or are we just to type this information anywhere on the form?</div></div>						
TG03.00-0141	TG03 Question 0141 - Bid Forms	Closed	06	09/01/2010	09/08/2010	09/07/2010

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<p><b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana</p> <p><b>REQUEST:</b></p> <p>Reference is made to the various forms that were revised per Addendum No. 2 (i.e. Acknowledgment of Receipt and Review - Project Bidding Manual; Bid Form and Schedule of Bid Prices; Bidding Checklist (BCL); Bid Bond Form; etc). All these form now have "FINAL FOR ADDENDUM" stamped across them. Is it your intent that we submit these forms as is or are you going to be providing us with a separate Bid Package of these forms without this reference stamped across them?</p>						<p><b>ANSWER:</b></p> <p>Reference is made to the various forms that were revised per Addendum No. 2 (i.e. Acknowledgment of Receipt and Review - Project Bidding Manual; Bid Form and Schedule of Bid Prices; Bidding Checklist (BCL); Bid Bond Form; etc). All these form now have "FINAL FOR ADDENDUM" stamped across them. Is it your intent that we submit these forms as is or are you going to be providing us with a separate Bid Package of these forms without this reference stamped across them?</p>
<b>TG03.00-0142</b>	<b>TG03 Question 0142 - Schedule</b>	<b>Closed</b>	<b>06</b>	<b>09/01/2010</b>	<b>09/08/2010</b>	<b>09/08/2010</b>
<p><b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana</p> <p><b>REQUEST:</b></p> <p>Owner response to question TG0300-0080 is incomplete. The BSE concept schedule does not contain any information concerning the removal of the access trestle nor the temporary streets. Further, the concept schedule provided shows no work activities beyond the construction of the lower concourse walls. Please provide specific information regarding the expected dates for these NTP's so the bidders can estimate the total costs for performing this work.</p>						<p><b>ANSWER:</b></p> <p>Owner response to question TG0300-0080 is incomplete. The BSE concept schedule does not contain any information concerning the removal of the access trestle nor the temporary streets. Further, the concept schedule provided shows no work activities beyond the construction of the lower concourse walls. Please provide specific information regarding the expected dates for these NTP's so the bidders can estimate the total costs for performing this work.</p>



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TG03.00-0143	TG03 Question 0143 - Long Form Subcontract	Closed	06	09/01/2010	09/08/2010	09/08/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Reference Exhibit B "Long Form Subcontract". Some of the terms and conditions that were provided in the Long Form Subcontract to be utilized as the written agreement between Webcor/Obayashi and the low bid Subcontractor are overly burdensome, unacceptable and potentially not in conformance with statues and regulations. Please confirm that mutually agreeable terms can be negotiated with Obayashi/Webcor prior to the bid date for the Project.		Reference Exhibit B "Long Form Subcontract". Some of the terms and conditions that were provided in the Long Form Subcontract to be utilized as the written agreement between Webcor/Obayashi and the low bid Subcontractor are overly burdensome, unacceptable and potentially not in conformance with statues and regulations. Please confirm that mutually agreeable terms can be negotiated with Obayashi/Webcor prior to the bid date for the Project.				
If the terms and conditions for the Subcontract are not negotiable, then we regret to inform you that we will not be able to supply a bid for this Project.		If the terms and conditions for the Subcontract are not negotiable, then we regret to inform you that we will not be able to supply a bid for this Project.				
TG03.00-0144	TG03 Question 0144 - CDSM	Closed	06	09/01/2010	09/08/2010	09/07/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
The CDSM wall requires no vertical interruptions in its drilling path, the utilities will have to wait until a portion of the CDSM wall is complete. Once a portion of CDSM wall is complete then relocation may happen and the utilities will have to be cored through the CDSM wall. Why is the owner relocating the utilities prior; and just not wait until a portion of CDSM is in before locating?		The CDSM wall requires no vertical interruptions in its drilling path, the utilities will have to wait until a portion of the CDSM wall is complete. Once a portion of CDSM wall is complete then relocation may happen and the utilities will have to be cored through the CDSM wall. Why is the owner relocating the utilities prior; and just not wait until a portion of CDSM is in before locating?				
TG03.00-0145	TG03 Question 0145 - Schedule	Closed	06	09/01/2010	09/08/2010	09/08/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:		ANSWER:				
Zone 2 NTP within 235 cd of NTP 1 Finish within 570 cd ----- From NTP1      805 cd		Zone 2 NTP within 235 cd of NTP 1 Finish within 570 cd ----- From NTP1      805 cd				
Zone 3 NTP within 265 cd of NTP 1		Zone 3 NTP within 265 cd of NTP 1				



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	Finish within 535 cd ----- From NTP1      800 cd  Is requirement to have zone 3 completed prior to zone 2 the owners true intent?					
	Finish within 535 cd ----- From NTP1      800 cd  Is requirement to have zone 3 completed prior to zone 2 the owners true intent?					
<b>TG03.00-0146</b>	<b>TG03 Question 0146 - Utilities</b>	<b>Closed</b>	<b>06</b>	<b>09/02/2010</b>	<b>09/09/2010</b>	<b>09/03/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference specification 02 41 01, 3.3.C.1.			Reference specification 02 41 01, 3.3.C.1.			
1. referenced Specifications states, "...Contractor shall remove and dispose of as the Contractor's property the San Francisco Fire Dept's (SSFD) Auxillary Water Supply System (AWSS) High Pressure Piping in accordance with (AWSS) standard plans and specifications..."			1. referenced Specifications states, "...Contractor shall remove and dispose of as the Contractor's property the San Francisco Fire Dept's (SSFD) Auxillary Water Supply System (AWSS) High Pressure Piping in accordance with (AWSS) standard plans and specifications..."			
Please identify which lines are the AWSS lines and if any abatement procedures will be required.			Please identify which lines are the AWSS lines and if any abatement procedures will be required.			
2 Please confirm the existing 16" HPG line indicated on Survey drawing sht. 4 of 10 will be relocated and/or abandoned prior to construction.			2 Please confirm the existing 16" HPG line indicated on Survey drawing sht. 4 of 10 will be relocated and/or abandoned prior to construction.			



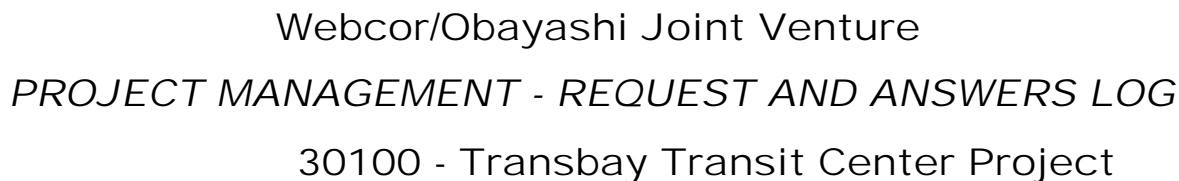
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<b>TG03.00-0147</b>	<b>TG03 Question 0147 - Traffic Routing</b>	<b>Closed</b>	<b>06</b>	<b>09/02/2010</b>	<b>09/09/2010</b>	<b>09/02/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 01 15 70.  Reference spec sections 2.1 E, 2.5 A, & 2.5 C. These sections state that Triton Barriers (or equal) must be used to separate traffic and pedestrians from construction areas, as well as traffic and pedestrians from each other. It also states that K-rail may not be substituted or used in conjunction with them. It is the contractors understanding that this means the entire perimeter of the job site will have to be barricaded off using Triton barriers and not K-rail. Please confirm.						<b>ANSWER:</b>  Reference specification 01 15 70.  Reference spec sections 2.1 E, 2.5 A, & 2.5 C. These sections state that Triton Barriers (or equal) must be used to separate traffic and pedestrians from construction areas, as well as traffic and pedestrians from each other. It also states that K-rail may not be substituted or used in conjunction with them. It is the contractors understanding that this means the entire perimeter of the job site will have to be barricaded off using Triton barriers and not K-rail. Please confirm.
<b>TG03.00-0148</b>	<b>TG03 Question 0148 - 301 Mission Wall</b>	<b>Closed</b>	<b>06</b>	<b>09/02/2010</b>	<b>09/09/2010</b>	<b>09/04/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference 301 Mission St. Drawings, drawing sheet GT-5102.  The Temporary Shoring and Earth Retention Drawings for 301 Mission St. show a grade beam on K line in Section 1 Drwg. SH-32 under the screening wall that is to be relocated by others, it appears that this grade beam carries through the parking structure as shown in Section 2 Drwg. SH-31 and Section 9 Drwg. GT-5102 of these bid documents. Please confirm this grade beam will be demolished prior to CDSM Shoring wall obstruction removal and Geotechnical Monitoring Instrumentation.  In Transbay Demolition Plans, drawing # D1060, and D1076, show the backfill material fill to first floor elevation in the area adjacent to 301 Mission Building. In BSE Plans, drawing # GT5000 shows the backfill material fill to about basement slab elevation. Please confirm which one is correct?						<b>ANSWER:</b>  Reference 301 Mission St. Drawings, drawing sheet GT-5102.  The Temporary Shoring and Earth Retention Drawings for 301 Mission St. show a grade beam on K line in Section 1 Drwg. SH-32 under the screening wall that is to be relocated by others, it appears that this grade beam carries through the parking structure as shown in Section 2 Drwg. SH-31 and Section 9 Drwg. GT-5102 of these bid documents. Please confirm this grade beam will be demolished prior to CDSM Shoring wall obstruction removal and Geotechnical Monitoring Instrumentation.  In Transbay Demolition Plans, drawing # D1060, and D1076, show the backfill material fill to first floor elevation in the area adjacent to 301 Mission Building. In BSE Plans, drawing # GT5000 shows the backfill material fill to about basement slab elevation. Please confirm which one is correct?
<b>TG03.00-0149</b>	<b>TG03 Question 0149 - Geotechnical Report</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/08/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						



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TG03.00-0151	<p><b>REQUEST:</b></p> <p>Reference plan sheet GT-5100, notes 11 and 12.</p> <p>On sheet D-5100 of the plans notes 11 and 12 reference "draft report results of the prototype test program installation of shoring walls using the cement deep soil mixing method" and "prototype test program and monitoring during construction of drilled shafts." How can I obtain these reports? Are they available online? Please send response to (e-mail address). Thank You.</p> <p>Submitted by Jesse Johnson Becho Inc 09/02/2010</p>	Closed	06	09/03/2010	09/10/2010	09/09/2010
	<p><b>ANSWER:</b></p> <p>Reference plan sheet GT-5100, notes 11 and 12.</p> <p>On sheet D-5100 of the plans notes 11 and 12 reference "draft report results of the prototype test program installation of shoring walls using the cement deep soil mixing method" and "prototype test program and monitoring during construction of drilled shafts." How can I obtain these reports? Are they available online? Please send response to (e-mail address). Thank You.</p> <p>Submitted by Jesse Johnson Becho Inc 09/02/2010</p>					
TG03.00-0151	<p><b>REQUEST:</b></p> <p>Drawings D-2210, D-2211, D-2212 and D-2213 are showing to remove existing pile caps and piles. But there are no details regarding which ones are timber and which ones are concrete. Please clarify.</p> <p>Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010</p>	Closed	06	09/03/2010	09/10/2010	09/09/2010
	<p><b>ANSWER:</b></p> <p>Drawings D-2210, D-2211, D-2212 and D-2213 are showing to remove existing pile caps and piles. But there are no details regarding which ones are timber and which ones are concrete. Please clarify.</p> <p>Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010</p>					





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<b>TG03.00-0152</b>	<b>TG03 Question 0152 - Demolition</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/08/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference documents for the (E) 80 Natoma Piles and Shoring wall don't match. Document #3 - 80 Natoma Installed Piles and Document #5 - 80 Natoma Foundation and Structure Plans show a difference of over 400 installed piles.			Reference documents for the (E) 80 Natoma Piles and Shoring wall don't match. Document #3 - 80 Natoma Installed Piles and Document #5 - 80 Natoma Foundation and Structure Plans show a difference of over 400 installed piles.			
Please confirm which document shows the correct number of installed piles for the (E) 80 Natoma structure.			Please confirm which document shows the correct number of installed piles for the (E) 80 Natoma structure.			
Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010			Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010			
<b>TG03.00-0153</b>	<b>TG03 Question 0153 - Pile Removal</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/07/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Drawing GT-2202 says that the existing piles in the buttress area have to be removed and it refers to Drawing GT-5301 for schematic diagrams of pile removal methods.			Drawing GT-2202 says that the existing piles in the buttress area have to be removed and it refers to Drawing GT-5301 for schematic diagrams of pile removal methods.			
Please clarify that only the piles in the buttress area have to be removed by one of the methods specified in GT-5301 and all the other piles can be removed during excavation as specified in Stage 4 of Drawing GT-5000.			Please clarify that only the piles in the buttress area have to be removed by one of the methods specified in GT-5301 and all the other piles can be removed during excavation as specified in Stage 4 of Drawing GT-5000.			
Submitted by Aparna Alla Shimmick / Skanska / Traylor JV 09/02/2010			Submitted by Aparna Alla Shimmick / Skanska / Traylor JV 09/02/2010			
<b>TG03.00-0154</b>	<b>TG03 Question 0154 - Buttress</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/07/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Installation sequence Notes 6 & 7 in Drawing GT-2201 states that Primary Shafts C/4, C/6, C/8 and Secondary Shafts C/5 and C/7 shall be filled with concrete from bottom of shaft to ground surface ( elevation +17.00 +/-			Installation sequence Notes 6 & 7 in Drawing GT-2201 states that Primary Shafts C/4, C/6, C/8 and Secondary Shafts C/5 and C/7 shall be filled with concrete from bottom of shaft to ground surface (			



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	<p>2.00) which contradicts with the detail 1 on GT-5201 and Detail 16 on GT-5202. Details on GT-5201 and GT-5202 show that the shafts get filled with concrete to subgrade elevatio(i.e.,bottom of excavation -45.00 +/- 2.00) Please clarify the top elevation of concrete in shafts.</p> <p>Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010</p>					
	<p>elevation +17.00 +/- 2.00) which contradicts with the detail 1 on GT-5201 and Detail 16 on GT-5202. Details on GT-5201 and GT-5202 show that the shafts get filled with concrete to subgrade elevatio(i.e.,bottom of excavation -45.00 +/- 2.00) Please clarify the top elevation of concrete in shafts.</p> <p>Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010</p>					
<b>TG03.00-0155</b>	<b>TG03 Question 0155 - Buttress</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/07/2010</b>
<p><b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Drawing GT-0000 and GT-2201 say that Secondary Shafts gets filled with Type "A" ( 6000 psi) concrete and Primary Shafts gets filled with Type "B" (2000 psi) Concrete.</p> <p>The legend for Primary and Secondary Shafts on GT-5201 contradicts with the above detail.</p> <p>Please clarify.</p> <p>Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010</p>			<p>Drawing GT-0000 and GT-2201 say that Secondary Shafts gets filled with Type "A" ( 6000 psi) concrete and Primary Shafts gets filled with Type "B" (2000 psi) Concrete.</p> <p>The legend for Primary and Secondary Shafts on GT-5201 contradicts with the above detail.</p> <p>Please clarify.</p> <p>Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010</p>			



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TG03.00-0156	TG03 Question 0156 - Buttress	Closed	06	09/03/2010	09/10/2010	09/07/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:  Drawing GT-5201 and GT-5202 shows that the shafts gets extended to Working Platform. If so, the shaft above the subgrade elevation shows it getting filled with 300 PSI CLSM, but per drawing GT-2201 it calls for Type "A" in the primary shafts and Type ¿B¿ in the secondary shafts up to ground surface?  Please clarify.  Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010		ANSWER:  Drawing GT-5201 and GT-5202 shows that the shafts gets extended to Working Platform. If so, the shaft above the subgrade elevation shows it getting filled with 300 PSI CLSM, but per drawing GT-2201 it calls for Type "A" in the primary shafts and Type ¿B¿ in the secondary shafts up to ground surface?  Please clarify.  Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010				
TG03.00-0157	TG03 Question 0157 - Shoring Wall	Closed	06	09/03/2010	09/10/2010	09/07/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:  On dwgs GT-2101, 2102, 2103 calls for sectional details for CDSM wall which gives the details about pre-trenching. As per the Specification 31 56 13, the contractor shall construct a trench along the entire alignment of the shoring wall& cut-off walls. But for walls X2-1, J/12.3 -13, A/19-25, A/25-26, A/26-30, A/30-33.5, A/33.5-35, J/25-27, J/33.5-35, 35-1&cut-off walls do not have any pre-trenching details shown. Can the contractor assume that the walls with no pre-trenching details do not require any pre-trenching?  Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010		ANSWER:  On dwgs GT-2101, 2102, 2103 calls for sectional details for CDSM wall which gives the details about pre-trenching. As per the Specification 31 56 13, the contractor shall construct a trench along the entire alignment of the shoring wall& cut-off walls. But for walls X2-1, J/12.3 -13, A/19-25, A/25-26, A/26-30, A/30-33.5, A/33.5-35, J/25-27, J/33.5-35, 35-1&cut-off walls do not have any pre-trenching details shown. Can the contractor assume that the walls with no pre-trenching details do not require any pre-trenching?  Submitted by Aparna Alla Shimmick / Skanska / Traylor JV (SST) 09/02/2010				
TG03.00-0158	TG03 Question 0158 - Specific Project Requirements	Closed	06	09/03/2010	09/10/2010	09/07/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:  Specification section 00 08 13 1.6 B states that the Contractor shall comply with Ordinance #175 91, Article 21 of the SF Municipal Code restricting the use of potable water for soil compaction and dust control activities. Does this specification also apply to water being used for drilled		ANSWER:  Specification section 00 08 13 1.6 B states that the Contractor shall comply with Ordinance #175 91, Article 21 of the SF Municipal Code restricting the use of potable water for soil compaction and dust control activities. Does this specification also apply to water				



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	shaft excavation?					
	Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010					
	being used for drilled shaft excavation?					
	Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010					
TG03.00-0159	TG03 Question 0159 - Temporary Bridge	Closed	06	09/03/2010	09/10/2010	09/13/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:			ANSWER:			
Schedule A on S1-3201 identifies top of structure elevations. When these elevations are cross referenced against the elevations of the cross streets the temporary bridges that tie into it will be several feet above grade, unless there is a provision for a concrete "leave out" Drawing A 5206 shows First Street Elevation at 14.94. The top of structure at this zone is from 12.79' to 13.47', this allows less than 2' for the temporary bridge installation. Depending on the final Temp. bridge design and clearance necessary to construct box structure below the bridge deck may be as much as 6' above the city street. Is it the owners intent to ramp up on the city street to the temp bridge elevation? If so what is the max grade allowed for the approach ramp?			Schedule A on S1-3201 identifies top of structure elevations. When these elevations are cross referenced against the elevations of the cross streets the temporary bridges that tie into it will be several feet above grade, unless there is a provision for a concrete "leave out" Drawing A 5206 shows First Street Elevation at 14.94. The top of structure at this zone is from 12.79' to 13.47', this allows less than 2' for the temporary bridge installation. Depending on the final Temp. bridge design and clearance necessary to construct box structure below the bridge deck may be as much as 6' above the city street. Is it the owners intent to ramp up on the city street to the temp bridge elevation? If so what is the max grade allowed for the approach ramp?			
Also, please comment on the intent for side sloping, access for business, support of fill, etc. This condition applies to Beale street as well.			Also, please comment on the intent for side sloping, access for business, support of fill, etc. This condition applies to Beale street as well.			
Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010			Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010			



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<b>TG03.00-0160</b>	<b>TG03 Question 0160 - Schedule</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/08/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Response to TG0300-0080 indicates , "Refer to Exhibit I - BSE Concept Schedule" BSE Concept schedule zones conclude with "Verticl Walls (2nd Lift) to Ground Level" with it's successor being "(Finish) Below Grade Structure Zone 'X'"  Is it the Concept for this activity to include the Top Deck of the Below Grade Structure? Do the removal activities described in NTP #7-10 commence after completion of the Finish Below Grade Structure activities?  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010		<b>ANSWER:</b>  Response to TG0300-0080 indicates , "Refer to Exhibit I - BSE Concept Schedule" BSE Concept schedule zones conclude with "Verticl Walls (2nd Lift) to Ground Level" with it's successor being "(Finish) Below Grade Structure Zone 'X'"  Is it the Concept for this activity to include the Top Deck of the Below Grade Structure? Do the removal activities described in NTP #7-10 commence after completion of the Finish Below Grade Structure activities?  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010				
<b>TG03.00-0161</b>	<b>TG03 Question 0161 - Water Discharge</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/07/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification 01 14 10/APA-4  Addendum No. 2 states that TJPA will reimburse the Contractor for costs associated with the State Water Resources Control.  Please confirm that the TJPA will reimburse the Contractor for costs associated with the preparation of the Stormwater Pollution Prevention Plan as required by the General Permit for Stormwater Discharges.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010		<b>ANSWER:</b>  Reference specification 01 14 10/APA-4  Addendum No. 2 states that TJPA will reimburse the Contractor for costs associated with the State Water Resources Control.  Please confirm that the TJPA will reimburse the Contractor for costs associated with the preparation of the Stormwater Pollution Prevention Plan as required by the General Permit for Stormwater Discharges.  Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010				
<b>TG03.00-0162</b>	<b>TG03 Question 0162 - Site Area</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/08/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Spec Section 01 14 19 - Restriction to Use of Site Areas, lists the location of adjacent site areas and when they are		<b>ANSWER:</b>  Spec Section 01 14 19 - Restriction to Use of Site Areas, lists the location of adjacent site areas and				



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	<p>available to the Trade/Subcontractor. This section does not address the area labeled in the drawings as the MUNI Hump or the area directly west of Zone 1. Is the trade/subcontractor to assume that both of these areas are not considered adjacent site areas, but areas acquired with the respective zones? Do these areas become available to the Trade/Subcontractor at NTP of Zones 1 and 3?</p> <p>Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010</p>					
	<p>when they are available to the Trade/Subcontractor. This section does not address the area labeled in the drawings as the MUNI Hump or the area directly west of Zone 1. Is the trade/subcontractor to assume that both of these areas are not considered adjacent site areas, but areas acquired with the respective zones? Do these areas become available to the Trade/Subcontractor at NTP of Zones 1 and 3?</p> <p>Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010</p>					
<b>TG03.00-0163</b>	<b>TG03 Question 0163 - Temporary Bridge</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/08/2010</b>
<p><b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana</p>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>Attachment 3, Access Trestle Criteria, of Exhibit A includes a section titled, Minimum Radius of Corner, which states, "Additional spaces at all inner corners of the Access Trestle shall be added for helping Truck/trailer/Crane turn." Temporary Bridges, 01 53 13 1.3 A. 5 establishes the gate requirements with, "Gates providing twenty-four feet (24') of clear unobstructed access shall be provided through all barrier systems at the center of the bridge."</p> <p>Please confirm the intent confirm to add additional space for turning radius to the trestle at the bridges intersections, when the access is restricted by the clear opening of the gates.</p> <p>Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010</p>			<p>Attachment 3, Access Trestle Criteria, of Exhibit A includes a section titled, Minimum Radius of Corner, which states, "Additional spaces at all inner corners of the Access Trestle shall be added for helping Truck/trailer/Crane turn." Temporary Bridges, 01 53 13 1.3 A. 5 establishes the gate requirements with, "Gates providing twenty-four feet (24') of clear unobstructed access shall be provided through all barrier systems at the center of the bridge."</p> <p>Please confirm the intent confirm to add additional space for turning radius to the trestle at the bridges intersections, when the access is restricted by the clear opening of the gates.</p> <p>Submitted by Charles M. Gardner Kiewit Infrastructure West Co. 09/02/2010</p>			







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09/03/2010				09/03/2010		
<b>TG03.00-0169</b>	<b>TG03 Question 0169 - Demolition</b>	<b>Closed</b>	<b>CR</b>	<b>09/07/2010</b>	<b>09/13/2010</b>	<b>09/15/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference reference documents 80 Natoma St., Installed Piles.  In the reference document, only a select number of piles show Pile Top and Pile Tip Elevations in Table 1 (these piles are highlighted in yellow on the drawing). For the piles where no information is given, please provide pile lengths, pile top elevations and pile tip elevations.  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010						
			<b>ANSWER:</b>  Reference reference documents 80 Natoma St., Installed Piles.  In the reference document, only a select number of piles show Pile Top and Pile Tip Elevations in Table 1 (these piles are highlighted in yellow on the drawing). For the piles where no information is given, please provide pile lengths, pile top elevations and pile tip elevations.  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010			
<b>TG03.00-0170</b>	<b>Can tiebacks be used for temporary bracing at the transverse end walls (Lines 1 ar</b>	<b>Closed</b>	<b>CR</b>	<b>09/07/2010</b>	<b>09/13/2010</b>	<b>09/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Can tiebacks be used for temporary bracing at the transverse end walls (Lines 1 and 35)? If so, please indicate any requirements or limitations associated with their use.  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010						
			<b>ANSWER:</b>  Can tiebacks be used for temporary bracing at the transverse end walls (Lines 1 and 35)? If so, please indicate any requirements or limitations associated with their use.  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010			



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<b>TG03.00-0171</b>	<b>TG03 Question 0171 - Internal Bracing</b>	<b>Closed</b>	<b>CR</b>	<b>09/07/2010</b>	<b>09/13/2010</b>	<b>09/17/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  As noted in Question TG0300-0058, preloading the struts will increase the effective stiffness of the bracing system (particularly by pre-compressing the struts).  (1) Can this factor be considered when evaluating the average stiffness tributary to a given strut per the note in the lower right-hand corner of GT-1111?  (2) Can preload values higher than those specified in Tables 1 through 4 on GT-1110 be used to increase the effective stiffness of the bracing system?  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010						
						<b>ANSWER:</b>  As noted in Question TG0300-0058, preloading the struts will increase the effective stiffness of the bracing system (particularly by pre-compressing the struts).  (1) Can this factor be considered when evaluating the average stiffness tributary to a given strut per the note in the lower right-hand corner of GT-1111?  (2) Can preload values higher than those specified in Tables 1 through 4 on GT-1110 be used to increase the effective stiffness of the bracing system?  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010
<b>TG03.00-0172</b>	<b>TG03 Question 0172 - Schedule</b>	<b>Closed</b>	<b>CR</b>	<b>09/07/2010</b>	<b>09/13/2010</b>	<b>09/09/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference Exhibit A, Section V. In reference to the concept schedule (Exhibit I), it is stated that although the schedule activities should not be assumed to be a complete or binding work plan. . . it is mandatory that each Completion Date be met so as not to impact follow-on Trade Subcontractors or the Critical Path of the Project. Are the Excavation Finish Dates for each of the Zones the mandatory Completion Dates referred to above?  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010						
						<b>ANSWER:</b>  Reference Exhibit A, Section V. In reference to the concept schedule (Exhibit I), it is stated that although the schedule activities should not be assumed to be a complete or binding work plan. . . it is mandatory that each Completion Date be met so as not to impact follow-on Trade Subcontractors or the Critical Path of the Project. Are the Excavation Finish Dates for each of the Zones the mandatory Completion Dates referred to above?  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010
<b>TG03.00-0173</b>	<b>TG03 Question 0173 - Demolition</b>	<b>Closed</b>	<b>CR</b>	<b>09/07/2010</b>	<b>09/13/2010</b>	<b>09/27/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference Exhibit A, Section IV.C.14 (p. 10).  Exhibit A states: Existing temporary shoring wall installed						
						<b>ANSWER:</b>  Reference Exhibit A, Section IV.C.14 (p. 10).  Exhibit A states: Existing temporary shoring wall



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	<p>by Existing Terminal and Ramps Demolition Contractor at the eastside of Fremont Street shall be removed and disposed in accordance with the Contract Documents.¿</p> <p>However, the Terminal Demolition Plans include notes stating that ¿. . . Contractor to furnish and install shoring and bracing as necessary to ensure no adverse impacts to adjacent roadways and building.¿ These notes seem to apply to the entire perimeter of the existing building and not just to the eastside of Fremont St.</p> <p>Please clarify if the TG03 Contract includes removal of the previously installed shoring and bracing along the eastside of Fremont Street only, or also around the entire perimeter of the existing terminal structure.</p> <p>Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010</p>					
	<p>installed by Existing Terminal and Ramps Demolition Contractor at the eastside of Fremont Street shall be removed and disposed in accordance with the Contract Documents.¿</p> <p>However, the Terminal Demolition Plans include notes stating that ¿. . . Contractor to furnish and install shoring and bracing as necessary to ensure no adverse impacts to adjacent roadways and building.¿ These notes seem to apply to the entire perimeter of the existing building and not just to the eastside of Fremont St.</p> <p>Please clarify if the TG03 Contract includes removal of the previously installed shoring and bracing along the eastside of Fremont Street only, or also around the entire perimeter of the existing terminal structure.</p> <p>Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010</p>					
TG03.00-0174	TG03 Question 0174 - Shoring Wall	Closed	CR	09/07/2010	09/13/2010	09/13/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:			ANSWER:			
Will it be permissible to shed bracing loads from the transverse end walls (near Lines 1 and 35) into the longitudinal CDSM walls (Lines A and J)? If this is acceptable, please indicate if there are any limitations or restrictions on the design assumptions regarding the amount of load that can be shed over a given length of wall.			Will it be permissible to shed bracing loads from the transverse end walls (near Lines 1 and 35) into the longitudinal CDSM walls (Lines A and J)? If this is acceptable, please indicate if there are any limitations or restrictions on the design assumptions regarding the amount of load that can be shed over a given length of wall.			





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<b>TG03.00-0177</b>	<b>TG03 Question 0177 - Internal Bracing</b>	<b>Closed</b>	<b>CR</b>	<b>09/07/2010</b>	<b>09/13/2010</b>	<b>09/16/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference drawing sheet GT-2101.  Regarding previous question TG0300-0084, part 1 which was unanswered: At what stage of excavation in Zone 1 will wall X1-1 be removed?  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010						<b>ANSWER:</b>  Reference drawing sheet GT-2101.  Regarding previous question TG0300-0084, part 1 which was unanswered: At what stage of excavation in Zone 1 will wall X1-1 be removed?  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010
<b>TG03.00-0178</b>	<b>TG03 Question 0178 - Micropile</b>	<b>Closed</b>	<b>CR</b>	<b>09/07/2010</b>	<b>09/13/2010</b>	<b>09/08/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference drawing sheet S1-3003.  Detail 1 indicates that the micropile design shall be by the Contractor, or in this case, by the micropile subcontractor.  (1) Is the micropile subcontractor responsible for designing the micropile anchorage in the concrete base slab?  (2) Is the micropile subcontractor responsible for furnishing and installing micropile anchorage reinforcing steel?  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010						<b>ANSWER:</b>  Reference drawing sheet S1-3003.  Detail 1 indicates that the micropile design shall be by the Contractor, or in this case, by the micropile subcontractor.  (1) Is the micropile subcontractor responsible for designing the micropile anchorage in the concrete base slab?  (2) Is the micropile subcontractor responsible for furnishing and installing micropile anchorage reinforcing steel?  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010
<b>TG03.00-0179</b>	<b>TG03 Question 0179 - Shoring Wall</b>	<b>Closed</b>	<b>CR</b>	<b>09/07/2010</b>	<b>09/13/2010</b>	<b>09/08/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  CDSM wall layout sheet GT-2101 shows Wall Segment X2-1 on the south side of the building between grid lines 11 and 13. The CDSM Shoring Wall Schedule (16/GT-5101) does not list this wall segment. Please clarify.						<b>ANSWER:</b>  CDSM wall layout sheet GT-2101 shows Wall Segment X2-1 on the south side of the building between grid lines 11 and 13. The CDSM Shoring Wall Schedule (16/GT-5101) does not list this wall



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TG03.00-0180	Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010	Closed	CR	09/07/2010	09/13/2010	10/21/2010
	TG03 Question 0180 - Buy America  From: Webcor/Obayashi Joint Venture      Manuel Saldana  <b>REQUEST:</b> Reference specification 00 08 13/APA, paragraph 17.  Please clarify the following questions regarding the Buy America requirements as they relate to the SBE Trade Subcontract:  (1) Can manufactured steel products such as wide flange sections, pipes, H piles, plate, etc. used in the SBE Trade Subcontract for temporary bracing, trestle and temporary cross street bridge construction be manufactured by foreign sources?  (2) Can the W sections used in CDSM shoring wall be manufactured by foreign sources?  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010					
TG03.00-0180	segment. Please clarify.  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010	Closed	CR	09/07/2010	09/13/2010	10/21/2010
	<b>ANSWER:</b> Reference specification 00 08 13/APA, paragraph 17.  Please clarify the following questions regarding the Buy America requirements as they relate to the SBE Trade Subcontract:  (1) Can manufactured steel products such as wide flange sections, pipes, H piles, plate, etc. used in the SBE Trade Subcontract for temporary bracing, trestle and temporary cross street bridge construction be manufactured by foreign sources?  (2) Can the W sections used in CDSM shoring wall be manufactured by foreign sources?  Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010					
TG03.00-0181	Submitted by Rich Zito Shimmick / Skanska / Traylor JV (SST) 09/07/2010	Closed	CR	09/07/2010	09/13/2010	09/21/2010
	TG03 Question 0181 - Internal Bracing  From: Webcor/Obayashi Joint Venture      Manuel Saldana  <b>REQUEST:</b> Sheet GT-1110 shows numerical values for horizontal strut loads. GT-1110 also shows a design profile. Are we to use the numerical values shown or are we to calculate loads based upon the design profile?  Shimmick / Skanska / Traylor JV (SST) 09/07/2010					
TG03.00-0181	Sheet GT-1110 shows numerical values for horizontal strut loads. GT-1110 also shows a design profile. Are we to use the numerical values shown or are we to calculate loads based upon the design profile?  Shimmick / Skanska / Traylor JV (SST) 09/07/2010	Closed	CR	09/07/2010	09/13/2010	09/21/2010
	<b>ANSWER:</b> Sheet GT-1110 shows numerical values for horizontal strut loads. GT-1110 also shows a design profile. Are we to use the numerical values shown or are we to calculate loads based upon the design profile?  Shimmick / Skanska / Traylor JV (SST) 09/07/2010					

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<b>TG03.00-0185</b>	<b>TG03 Question 0185 - Hazardous Material</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2010</b>	<b>09/15/2010</b>	<b>09/14/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  1. As per the Site Mitigation plan by Treadwell & Rollo, the extent of hazardous of material information is available for the proposed project location except in the CDSM wall segment X1-1 & R2-1 areas.  Please provide the related hazardous material information for the above mentioned areas.						<b>ANSWER:</b>  1. As per the Site Mitigation plan by Treadwell & Rollo, the extent of hazardous of material information is available for the proposed project location except in the CDSM wall segment X1-1 & R2-1 areas.  Please provide the related hazardous material information for the above mentioned areas.
<b>TG03.00-0186</b>	<b>TG03 Question 0186 - Traffic Routing</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2010</b>	<b>09/15/2010</b>	<b>09/14/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  The traffic lane requirements as per the specification 01 15 70 says that contractor needs to maintain 3 lanes of 11' at all the times on the First Street from Mission to Folsom and also at the intersection of Fremont and Natoma.  With the above restrictions, safe operations for CDSM walls by using big equipment may not be obtained. Is it allowable for contractor to perform the work with half the street closed?						<b>ANSWER:</b>  The traffic lane requirements as per the specification 01 15 70 says that contractor needs to maintain 3 lanes of 11' at all the times on the First Street from Mission to Folsom and also at the intersection of Fremont and Natoma.  With the above restrictions, safe operations for CDSM walls by using big equipment may not be obtained. Is it allowable for contractor to perform the work with half the street closed?
<b>TG03.00-0187</b>	<b>TG03 Question 0187 - Shoring Wall Traffic Routing</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2010</b>	<b>09/15/2010</b>	<b>09/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Position of CDSM walls with reference to sidewalks is not clear on the provided GT drawings. Please provide the distances between sidewalks and CDSM walls to see the possibility of maintaining traffic lanes as specified in specifications 01 15 70.						<b>ANSWER:</b>  Position of CDSM walls with reference to sidewalks is not clear on the provided GT drawings. Please provide the distances between sidewalks and CDSM walls to see the possibility of maintaining traffic lanes as specified in specifications 01 15 70.
<b>TG03.00-0188</b>	<b>TG03 Question 0188 - SBE Program</b>	<b>Closed</b>	<b>CR</b>	<b>09/09/2010</b>	<b>09/15/2010</b>	<b>09/10/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						





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TG03.00-0191	TG03 Question 0191 - Shoring Wall	Closed	CR	09/09/2010	09/15/2010	09/13/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:						ANSWER:
Reference specification 31 56 13.						Reference specification 31 56 13.
Can the Trade Subcontractor rely on the CDSM wall being impervious? If the CDSM wall leaks will it be the basis for a change order?						Can the Trade Subcontractor rely on the CDSM wall being impervious? If the CDSM wall leaks will it be the basis for a change order?
TG03.00-0192	TG03 Question 0192 - Buttress	Closed	CR	09/09/2010	09/15/2010	09/14/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:						ANSWER:
Reference drawing sheet GT-5202.						Reference drawing sheet GT-5202.
This plan calls out "(4) 4 in. diameter PVC or steel pipe sleeves...tied to the reinforcement steel cage" We assume that these are the access tubes for the required Cross Hole Sonic Logging Test. Usual access tube size is only 2 inches in diameter. Please confirm that you specifically require 4 in. or that regular 2 inches in diameter access tubes (PVC or steel) can be used instead.						This plan calls out "(4) 4 in. diameter PVC or steel pipe sleeves...tied to the reinforcement steel cage" We assume that these are the access tubes for the required Cross Hole Sonic Logging Test. Usual access tube size is only 2 inches in diameter. Please confirm that you specifically require 4 in. or that regular 2 inches in diameter access tubes (PVC or steel) can be used instead.
Question B: Can reinforcement spiral be replaced by regular ring hoops?						Question B: Can reinforcement spiral be replaced by regular ring hoops?
TG03.00-0193	TG03 Question 0193 - Site Maintenance	Closed	CR	09/13/2010	09/19/2010	09/13/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:						ANSWER:
Reference Project Bidding Manual, page 34 paragraph 6.						Reference Project Bidding Manual, page 34 paragraph 6.
The Project Bidding Manual states: "Trade Subcontractor shall include in the Bid two man-hours of cleanup for every forty man-hours of work. This Labor, provided by Trade Subcontractor, will be used as part of a Composite Project efforts to maintain a clean work area. The actual clean-up hours used versus the number of hours owed (tracked through certified payroll) will be reconciled. Contractor has option to deduct this from Trade Subcontractor's scope of Work incrementally or in its entirety and execute the						The Project Bidding Manual states: "Trade Subcontractor shall include in the Bid two man-hours of cleanup for every forty man-hours of work. This Labor, provided by Trade Subcontractor, will be used as part of a Composite Project efforts to maintain a clean work area. The actual clean-up hours used versus the number of hours owed (tracked through certified payroll) will be reconciled. Contractor has



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	Composite Project clean-up."					
	Is this necessary for the entire TG03 package or just the portion that we are coordinating trestle removal? And/or re-shoring?			option to deduct this from Trade Subcontractor's scope of Work incrementally or in its entirety and execute the Composite Project clean-up."		
				Is this necessary for the entire TG03 package or just the portion that we are coordinating trestle removal? And/or re-shoring?		
<b>TG03.00-0194</b>	<b>TG03 Question 0194 - Temporary Power</b>	<b>Closed</b>	<b>CR</b>	<b>09/13/2010</b>	<b>09/19/2010</b>	<b>09/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference drawing sheet SL-003  Please confirm PG&E will provide the transformers for the skids as shown on the Site Logistics Temporary Power Plan SL-003.				<b>ANSWER:</b> Reference drawing sheet SL-003  Please confirm PG&E will provide the transformers for the skids as shown on the Site Logistics Temporary Power Plan SL-003.		
<b>TG03.00-0195</b>	<b>TG03 Question 0195 - Schedule</b>	<b>Closed</b>	<b>CR</b>	<b>09/13/2010</b>	<b>09/19/2010</b>	<b>09/15/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Exhibit A.V Construction Schedule and Milestones:  1) NTP #03, #04, and #05 are indicated to be issued , "no later than" 175, 235, and 265 calander days (respectively) for each of Zones 1,2, and 3. What is the "no earlier than" dates for these milestones? Theoretically TJPA could issue NTP #03-#05 immediately after NTP#02 which would stack the work. Request earliest start date to realistically plan our work.  2) What are the durations of Milestones NTP#06 through #7?  3) How are Milestones NTP #06 through #10 tied to the critical path?				<b>ANSWER:</b> Exhibit A.V Construction Schedule and Milestones:  1) NTP #03, #04, and #05 are indicated to be issued , "no later than" 175, 235, and 265 calander days (respectively) for each of Zones 1,2, and 3. What is the "no earlier than" dates for these milestones? Theoretically TJPA could issue NTP #03-#05 immediately after NTP#02 which would stack the work. Request earliest start date to realistically plan our work.  2) What are the durations of Milestones NTP#06 through #7?  3) How are Milestones NTP #06 through #10 tied to the critical path?		



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<b>TG03.00-0196</b>	<b>TG03 Question 0196 - Access Trestle</b>	<b>Closed</b>	<b>CR</b>	<b>09/13/2010</b>	<b>09/19/2010</b>	<b>09/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The Scope of Work for the Removal and Disposal of Access Trestle as described in Exhibit A IV.C.22 indicates, "The Structural Steel Trade Subcontractor shall remove/dispose the Access Trestle above the Lower Concourse slab,¿"			The Scope of Work for the Removal and Disposal of Access Trestle as described in Exhibit A IV.C.22 indicates, "The Structural Steel Trade Subcontractor shall remove/dispose the Access Trestle above the Lower Concourse slab,¿"			
Please confirm that the Access Trestle remains the property of the BSE Trade Subcontractor, and will be disposed at a location (within the Bay Area) of their choice.			Please confirm that the Access Trestle remains the property of the BSE Trade Subcontractor, and will be disposed at a location (within the Bay Area) of their choice.			
<b>TG03.00-0197</b>	<b>TG03 Question 0197 - Geotechnical Report</b>	<b>Closed</b>	<b>CR</b>	<b>09/13/2010</b>	<b>09/19/2010</b>	<b>09/16/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The Final Geotechnical Data Report prepared by Arup North America Ltd. Article 6.1.2.2 indicates,"A more detailed description of stratigraphy and information on the characterization of the majormajor soil strata are presented in the Geotechnical Recommendations report."			The Final Geotechnical Data Report prepared by Arup North America Ltd. Article 6.1.2.2 indicates,"A more detailed description of stratigraphy and information on the characterization of the majormajor soil strata are presented in the Geotechnical Recommendations report."			
Is this report available? If so, how may we access this?			Is this report available? If so, how may we access this?			



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<b>TG03.00-0198</b>	<b>TG03 Question 0198 - Site Area</b>	<b>Closed</b>	<b>CR</b>	<b>09/13/2010</b>	<b>09/19/2010</b>	<b>09/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference Exhibit A drawings SL-001 & SL-002.  Dwg. SL-001 shows the area bounded by First, Fremont, Minna and Mission Sts. as an "Emergency Gathering Point" Dwg. SL-002 shows outbound trucks exiting this area. The "Staging Parcels" sketch in Section 01 14 19 does not show this area. Is this area available for staging/laydown use by the SBE Subcontractor? If so, are there any restrictions on its use?						<b>ANSWER:</b>  Reference Exhibit A drawings SL-001 & SL-002.  Dwg. SL-001 shows the area bounded by First, Fremont, Minna and Mission Sts. as an "Emergency Gathering Point" Dwg. SL-002 shows outbound trucks exiting this area. The "Staging Parcels" sketch in Section 01 14 19 does not show this area. Is this area available for staging/laydown use by the SBE Subcontractor? If so, are there any restrictions on its use?
<b>TG03.00-0199</b>	<b>TG03 Question 0199 - Retention</b>	<b>Closed</b>	<b>CR</b>	<b>09/13/2010</b>	<b>09/19/2010</b>	<b>09/14/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference Answers to Pre-Bid Meeting Questions.  The answer to Question 13 says that retention is withheld for the duration of each subcontractor's scope of work rather than the entire project. Most of the SBE Subcontractor's scope of work will be complete with the placing of the rat slabs. However, the contract will continue until the SBE Subcontractor's responsibility for shoring/bracing removal and trestle/bridge removal is complete. The schedule for this work is contingent on other Trade Subcontractors and the completion date for the entire scope of the SBE Trade Subcontract Package is unknown. Will Webcor/Obayashi release retention attributable to all work completed up until the placing of the rat slabs when the placing of the rat slabs is complete?						<b>ANSWER:</b>  Reference Answers to Pre-Bid Meeting Questions.  The answer to Question 13 says that retention is withheld for the duration of each subcontractor's scope of work rather than the entire project. Most of the SBE Subcontractor's scope of work will be complete with the placing of the rat slabs. However, the contract will continue until the SBE Subcontractor's responsibility for shoring/bracing removal and trestle/bridge removal is complete. The schedule for this work is contingent on other Trade Subcontractors and the completion date for the entire scope of the SBE Trade Subcontract Package is unknown. Will Webcor/Obayashi release retention attributable to all work completed up until the placing of the rat slabs when the placing of the rat slabs is complete?
<b>TG03.00-0200</b>	<b>TG03 Question 0200 - Temporary Lighting</b>	<b>Closed</b>	<b>CR</b>	<b>09/13/2010</b>	<b>09/19/2010</b>	<b>09/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference Exhibit A, Section IV.B.A.18, "Temporary Lighting"						<b>ANSWER:</b>  Reference Exhibit A, Section IV.B.A.18, "Temporary Lighting"



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	Is the SBE Trade Subcontractor responsible for maintaining temporary lighting until the rat slabs are placed or until the end of the SBE Trade Subcontract?					
TG03.00-0201	TG03 Question 0201 - Tax Certificate	Closed	CR	09/13/2010	09/19/2010	09/16/2010
From: Webcor/Obayashi Joint Venture      Manuel Saldana						
REQUEST:			ANSWER:			
Reference is made to Part V, Webcor/Obayashi Bidding Forms, Item A. Bidding Check List, Subitem 3. Current Business Tax Registration Certificate. In Addendum 2 you specifically deleted the requirement for us to submit our "Current San Francisco Business License Certificate". You also changed "Current Business Tax Certificate" to "Current Business Tax Registration Certificate". We have various city Business Tax Registration Certificates. Is it your intent for us to only submit our current Business Tax Registration Certificate for "San Francisco".			Reference is made to Part V, Webcor/Obayashi Bidding Forms, Item A. Bidding Check List, Subitem 3. Current Business Tax Registration Certificate. In Addendum 2 you specifically deleted the requirement for us to submit our "Current San Francisco Business License Certificate". You also changed "Current Business Tax Certificate" to "Current Business Tax Registration Certificate". We have various city Business Tax Registration Certificates. Is it your intent for us to only submit our current Business Tax Registration Certificate for "San Francisco".			



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<b>TG03.00-0202</b>	<b>TG03 Question 0202 - Bid Due Date</b>	<b>Closed</b>	<b>CR</b>	<b>09/13/2010</b>	<b>09/19/2010</b>	<b>09/13/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Our QBD sent on 9/02/10 expressed our serious concern with the SBE Trade Package schedule, liquidated damages and other contract terms. We requested that Webcor/Obayashi meet with the SBE Trade Package bidders, as soon as possible, in order to clarify and resolve the major issues of concern. We also requested that the bid due date be postponed by 6 weeks.			Our QBD sent on 9/02/10 expressed our serious concern with the SBE Trade Package schedule, liquidated damages and other contract terms. We requested that Webcor/Obayashi meet with the SBE Trade Package bidders, as soon as possible, in order to clarify and resolve the major issues of concern. We also requested that the bid due date be postponed by 6 weeks.			
We have not yet received a response to our 9/02/10 QBD. We cannot continue to expend resources on this bid without Webcor/Obayashi acknowledging our concerns and committing to act timely to help resolve them.			We have not yet received a response to our 9/02/10 QBD. We cannot continue to expend resources on this bid without Webcor/Obayashi acknowledging our concerns and committing to act timely to help resolve them.			
We ask that Webcor/Obayashi provide us with a response to our 9/02/10 QBD by Friday 9/17/10, otherwise we will have to suspend our estimating effort on Bid Package TG03.			We ask that Webcor/Obayashi provide us with a response to our 9/02/10 QBD by Friday 9/17/10, otherwise we will have to suspend our estimating effort on Bid Package TG03.			





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<b>TG03.00-0203</b>	<b>TG03 Question 0203 - Regulatory Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>09/15/2010</b>	<b>09/21/2010</b>	<b>09/15/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
1. Reference section 1 of the Long Form Subcontract (last sub-paragraph); also reference paragraph 2.05 of section 00 05 20 of the Agreement and section 01 14 10 Regulatory Requirements: which require compliance with applicable federal laws and guidelines. Several other specifications (particularly, those relating to health and safety) specifically list specific provisions of the Code of Federal Regulations that the Contractor (and therefore, where relevant, the Trade Subcontractor) must comply with. Provide a list of all applicable federal laws and guidelines (other than those specific provisions of the Code of Federal Regulations that are already included in the General Conditions and other bid documents) that the Trade Subcontractor must comply with on this project. In particular, provide a list of all Federal Acquisition Regulations that apply to Trade Subcontractor's obligations on this project.			1. Reference section 1 of the Long Form Subcontract (last sub-paragraph); also reference paragraph 2.05 of section 00 05 20 of the Agreement and section 01 14 10 Regulatory Requirements: which require compliance with applicable federal laws and guidelines. Several other specifications (particularly, those relating to health and safety) specifically list specific provisions of the Code of Federal Regulations that the Contractor (and therefore, where relevant, the Trade Subcontractor) must comply with. Provide a list of all applicable federal laws and guidelines (other than those specific provisions of the Code of Federal Regulations that are already included in the General Conditions and other bid documents) that the Trade Subcontractor must comply with on this project. In particular, provide a list of all Federal Acquisition Regulations that apply to Trade Subcontractor's obligations on this project.			
<b>TG03.00-0204</b>	<b>TG03 Question 0204 - Payment</b>	<b>Closed</b>	<b>CR</b>	<b>09/15/2010</b>	<b>09/21/2010</b>	<b>09/15/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference 4.1 of the Long Form Subcontract: The second paragraph of this provision states that the CM/GC has no obligation to pay the subcontractor until TJPA approves the CM/GC's application for payment and TJPA actually pays the CM/GC. We request that you correct this provision to comply with the California public policy against pay-if-paid provisions.			Reference 4.1 of the Long Form Subcontract: The second paragraph of this provision states that the CM/GC has no obligation to pay the subcontractor until TJPA approves the CM/GC's application for payment and TJPA actually pays the CM/GC. We request that you correct this provision to comply with the California public policy against pay-if-paid provisions.			
<b>TG04.5.1-0001</b>	<b>TG0451 Question 0001 - SBE Program</b>	<b>Closed</b>	<b>06</b>	<b>08/18/2010</b>	<b>08/25/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference RFQ, p6			Reference RFQ, p6			
Is this project only open to SBA's for bidding? Is this set			Is this project only open to SBA's for bidding? Is this			



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	aside for only SBA bidders?  Submitted by Heather Kay KJ Woods Construction Inc. 08/16/2010					set aside for only SBA bidders?  Submitted by Heather Kay KJ Woods Construction Inc. 08/16/2010
<b>TG04.5.1-0002</b>	<b>TG0451 Question 0002 - SBE Program</b>	<b>Closed</b>	<b>06</b>	<b>08/18/2010</b>	<b>08/25/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  W.A. Rasic is not an SBE. However, we fully intend to conduct a comprehensive GFE to increase overall SBE participation (1st, 2nd tier subcontractors, vendors, etc.) Can W.A. Rasic bid on this project direct to the J.V.?  Submitted by John Solis W.A. Rasic Construction 08/16/2010						<b>ANSWER:</b>  W.A. Rasic is not an SBE. However, we fully intend to conduct a comprehensive GFE to increase overall SBE participation (1st, 2nd tier subcontractors, vendors, etc.) Can W.A. Rasic bid on this project direct to the J.V.?  Submitted by John Solis W.A. Rasic Construction 08/16/2010
<b>TG04.5.1-0003</b>	<b>TG0451 Question 0003 - SBE Program</b>	<b>Closed</b>	<b>06</b>	<b>08/19/2010</b>	<b>08/26/2010</b>	<b>08/23/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>  Reference specification section IV, paragraph #1  We are not a SBE or DBE are we excluded from bidding on this project TG04.5.1.  Submitted by Tom Cornett Underground Construction Co., Inc. 08/16/2010						<b>ANSWER:</b>  Reference specification section IV, paragraph #1  We are not a SBE or DBE are we excluded from bidding on this project TG04.5.1.  Submitted by Tom Cornett Underground Construction Co., Inc. 08/16/2010

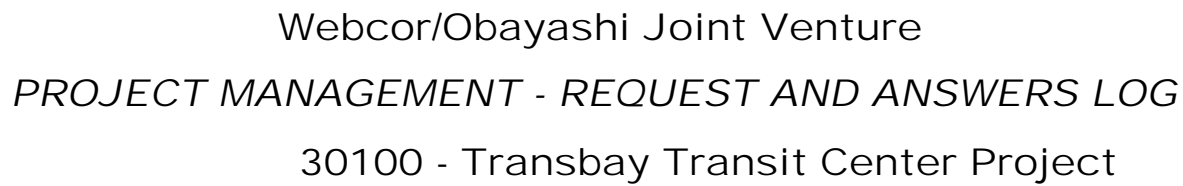


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TG04.5.1-0004	TG0451 Question 0004 - Liquidated Damages	Closed	06	08/31/2010	09/07/2010	09/08/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference specification 00 05 20-11, 4.02.  This section requires \$50,000 liquidated damages per day if project is not substantially complete more than 90 days. This is for the entire project and not TG04.5.1. Please Clarify. Page 11 of Bid manual and forms specifices 4 Milestones without and LD's.		<b>ANSWER:</b> Reference specification 00 05 20-11, 4.02.  This section requires \$50,000 liquidated damages per day if project is not substantially complete more than 90 days. This is for the entire project and not TG04.5.1. Please Clarify. Page 11 of Bid manual and forms specifices 4 Milestones without and LD's.				
TG04.5.1-0005	TG0451 Question 0005 - Project Staffing Requirements	Closed	06	08/31/2010	09/07/2010	09/01/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference Exhibit H Safety, page 7.  If we have 61 employees work on site, we need to have 2 full time designated safety persons (DSP) on site. Is this correct? Is this in addition to the requirements of 00 70 00-68 paragraph 12.01.B?		<b>ANSWER:</b> Reference Exhibit H Safety, page 7.  If we have 61 employees work on site, we need to have 2 full time designated safety persons (DSP) on site. Is this correct? Is this in addition to the requirements of 00 70 00-68 paragraph 12.01.B?				
TG04.5.1-0006	TG0451 Question 0006 - Bid Bond	Closed	06	08/31/2010	09/07/2010	09/03/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference Bidding Manual, page 12 paragraph 4.C., document 00 04 30-1.  Project Bidding manual page 12 of 44 requires bid security made to Webcor/Obayashi JV. Document 00 04 30 Appears to be between TJPA and JV. Please clarify.		<b>ANSWER:</b> Reference Bidding Manual, page 12 paragraph 4.C., document 00 04 30-1.  Project Bidding manual page 12 of 44 requires bid security made to Webcor/Obayashi JV. Document 00 04 30 Appears to be between TJPA and JV. Please clarify.				
TG04.5.1-0007	TG0451 Question 0007 - BIM & CPM	Closed	06	08/31/2010	09/07/2010	09/01/2010
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference specificatoin 01 13 10-2, paragraph 13.A		<b>ANSWER:</b> Reference specificatoin 01 13 10-2, paragraph 13.A				



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TG04.5.1-0008	<p>Contract Requires primavera P6 or compatible format. Is Microsoft Project Acceptable?</p> <p><b>TG0451 Question 0008 - Length Of Warranty</b></p> <p><b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana</p> <p><b>REQUEST:</b></p> <p>Reference Exhibit "B" Warranty, 2nd paragraph.</p> <p>Does warranty period extend to after completion of the entire project or only after TG04.5.1 is completed? The warranty form says indicates after "Filing Notice of Completion on all improvements". See attached which can be more than 4 years.</p>	Closed	06	08/31/2010	09/07/2010	09/08/2010
TG04.5.1-0009	<p>Contract Requires primavera P6 or compatible format. Is Microsoft Project Acceptable?</p> <p><b>TG0451 Question 0009 - Length Of Warranty</b></p> <p><b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana</p> <p><b>REQUEST:</b></p> <p>Reference specification 01 17 40-2, paragraph 1.5.A.</p> <p>Exhibit "B" is blank for period of warranty. 01-17-40-2 requires 2 years after substantial completion of TG04.5.1. Draft Subcontract agreement seems to indicate warranty after completion of the entire project. Please Clarify.</p>	Closed	06	08/31/2010	09/07/2010	09/08/2010



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<b>TG04.5.1-0010</b>	<b>TG0451 Question 0010 - Maintenance Bond</b>	<b>Closed</b>	<b>06</b>	<b>08/31/2010</b>	<b>09/07/2010</b>	<b>09/03/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference specification 00 08 13/APA, page 15, paragraph 6.B.			Reference specification 00 08 13/APA, page 15, paragraph 6.B.			
Is Maintenance Bond Required?			Is Maintenance Bond Required?			
<b>TG04.5.1-0011</b>	<b>TG0451 Question 0011 - Insurance Requirements</b>	<b>Closed</b>	<b>06</b>	<b>08/31/2010</b>	<b>09/07/2010</b>	<b>09/03/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference specification 00 08 05, paragraph 1.2.			Reference specification 00 08 05, paragraph 1.2.			
Please confirm that \$25,000,000 limit on GL is not for TG04.5.1 contract. This question was reponded to during pre-qualificiation process (Question #1).			Please confirm that \$25,000,000 limit on GL is not for TG04.5.1 contract. This question was reponded to during pre-qualificiation process (Question #1).			
<b>TG04.5.1-0012</b>	<b>TG0451 Question 0012 - Mobilizations</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/03/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference specification 01 15 05 & Bid Form Exhibit A.			Reference specification 01 15 05 & Bid Form Exhibit A.			
This section is about mobilization cost and how it is release, however there is no mobilization line item on bid form (exhibit A). Should bid form be revised to included mobilization?			This section is about mobilization cost and how it is release, however there is no mobilization line item on bid form (exhibit A). Should bid form be revised to included mobilization?			
<b>TG04.5.1-0013</b>	<b>TG0451 Question 0013 - Personnel Requirements</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/03/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference specification 01 14 00, paragraph 1.8.B.			Reference specification 01 14 00, paragraph 1.8.B.			
This section requires a full time graduate licensed engineer or architect to be on site as CQC manager. Is this position filled with CMGC or Trade Contractor for			This section requires a full time graduate licensed engineer or architect to be on site as CQC manager. Is this position filled with CMGC or Trade Contractor for			



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TG04.5.1.						
			for TG04.5.1.			
<b>TG04.5.1-0014</b>	<b>TG0451 Question 0014 - Fall Protection</b>	<b>Closed</b>	<b>06</b>	<b>09/03/2010</b>	<b>09/10/2010</b>	<b>09/03/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Manuel Saldana						
<b>REQUEST:</b> Reference specification 01 15 45-5, paragraph 1.6.C.  Fall protection is required for all trenches 5 feet or deeper. Does this requirement include lifeline harness, lanyard, tie down, etc>?						
			<b>ANSWER:</b> Reference specification 01 15 45-5, paragraph 1.6.C.  Fall protection is required for all trenches 5 feet or deeper. Does this requirement include lifeline harness, lanyard, tie down, etc>?			
<b>TG043-0001</b>	<b>Site Survey</b>	<b>Closed</b>	<b>03</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/02/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Do we provide all survey for our work?  W/O to provide answer.						
			<b>ANSWER:</b> Do we provide all survey for our work?  W/O to provide answer.			
<b>TG043-0002</b>	<b>SBE Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> This project is indicated as to be 100% SBE. Please confirm that this means that all of our subcontractors (and sub tier subcontractors if any) and truckers must therefore be SBE companies. Please also confirm that not all suppliers must be SBE. It will be impossible to obtain quotations for asphalt, VCP, aggregate import materials etc. from SBE companies.						
			<b>ANSWER:</b> This project is indicated as to be 100% SBE. Please confirm that this means that all of our subcontractors (and sub tier subcontractors if any) and truckers must therefore be SBE companies. Please also confirm that not all suppliers must be SBE. It will be impossible to obtain quotations for asphalt, VCP, aggregate import materials etc. from SBE companies.			



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TG043-0003	Lead in AWSS Pipes	Closed	CR	11/02/2010	11/16/2010	11/03/2010
From: Webcor Construction LP      David Hungerford						
REQUEST: Are AWSS pipes to be demolished? If so, how is the hazardous lead in the joints to be handled?						ANSWER: Are AWSS pipes to be demolished? If so, how is the hazardous lead in the joints to be handled?
TG043-0004	Permit Reimbursables	Closed	CR	11/02/2010	11/16/2010	11/04/2010
From: Webcor Construction LP      David Hungerford						
REQUEST: Will we be reimbursed for all excavation, street space, meter, and other permits?						ANSWER: Will we be reimbursed for all excavation, street space, meter, and other permits?
TG043-0005	Phase II Drawings	Closed	CR	11/02/2010	11/16/2010	11/02/2010
From: Webcor Construction LP      David Hungerford						
REQUEST: Do we include anything indicated in the Phase II drawings?  W/O to provide answer.						ANSWER: Do we include anything indicated in the Phase II drawings?  W/O to provide answer.
TG043-0006	OCIP Requirements	Closed	CR	11/02/2010	11/16/2010	11/02/2010
From: Webcor Construction LP      David Hungerford						
REQUEST: Is there (or will there be) an OCIP on this project? If so, what insurances will this OCIP include?						ANSWER: Is there (or will there be) an OCIP on this project? If so, what insurances will this OCIP include?



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TG043-0007	Bid Form Clarification	Closed	CR	11/02/2010	11/16/2010	11/02/2010
From: Webcor Construction LP      David Hungerford						
REQUEST:		ANSWER:				
The bid form is far too complex. We can possibly understand the need for the data indicated by the bid form from the subcontractor awarded the project, but we can see no need for each and every bidder to fill out every space in a ten page bid form. In addition the bid form requires us to fill in the quantities. As every subcontractor will have different quantities, this seems somewhat odd. A typical SF PUC style bid form, with quantities provided, would result in bids that could be confidently compared with each other- ¿Apples to Apples¿, not¿ Oranges to Apples¿. We request that such a bid form be provided. If the bid form is not simplified greatly, we will not be able to bid this project, since it will take more time to complete the form than actually estimate the project.		The bid form is far too complex. We can possibly understand the need for the data indicated by the bid form from the subcontractor awarded the project, but we can see no need for each and every bidder to fill out every space in a ten page bid form. In addition the bid form requires us to fill in the quantities. As every subcontractor will have different quantities, this seems somewhat odd. A typical SF PUC style bid form, with quantities provided, would result in bids that could be confidently compared with each other- ¿Apples to Apples¿, not¿ Oranges to Apples¿. We request that such a bid form be provided. If the bid form is not simplified greatly, we will not be able to bid this project, since it will take more time to complete the form than actually estimate the project.				
W/O to provide answer.		W/O to provide answer.				
TG043-0008	Bid Date Extension	Closed	CR	11/02/2010	11/16/2010	11/02/2010
From: Webcor Construction LP      David Hungerford						
REQUEST:		ANSWER:				
Can the bid date please be delayed?		Can the bid date please be delayed?				
TG043-0009	Exhibit I Schedule and Exhibit A.V	Closed	CR	11/02/2010	11/16/2010	11/04/2010
From: Webcor Construction LP      David Hungerford						
REQUEST:		ANSWER:				
Exhibit I, Schedule, appears to show the physical work occurring between 1/27/11 and 3/31/11, approximately 68 calendar days (CD). This is approximately 48 working days. In this amount of time we are to install the sewer, install the water, get SFWD to do the tie ins (which often takes 2 weeks) and then do all the demolition and restoration. Please confirm that this is your intent. Also note that Exhibit A.V appears to require (under milestones) that the water work be completed within 80 CD of NTP-thus it seems that the water will have to be completed prior to the sewer. Please confirm this is your		Exhibit I, Schedule, appears to show the physical work occurring between 1/27/11 and 3/31/11, approximately 68 calendar days (CD). This is approximately 48 working days. In this amount of time we are to install the sewer, install the water, get SFWD to do the tie ins (which often takes 2 weeks) and then do all the demolition and restoration. Please confirm that this is your intent. Also note that Exhibit A.V appears to require (under milestones) that the water work be completed within 80 CD of NTP-thus it seems that the water will have to be completed prior to the sewer.				





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	intent.					Please confirm this is your intent.
	W/O to provide answer.					W/O to provide answer.
TG043-0010	Bid Package Drawing Clarification	Closed	CR	11/02/2010	11/16/2010	11/02/2010
From: Webcor Construction LP David Hungerford						
REQUEST:						ANSWER:
Exhibit A.IV, Clarification of Bid Package. It states that The issued for construction drawings and the Field Order revisions are for construction of Bid Package TG04.5.1 ONLY. However the Delta 1- Field Order-revisions state that the revisions (are) for TG04.3 and TG 04.4. So if they are for construction of Bid Package TG04.5.1 ONLY, why are they revisions for TG04.3 and TG04.4? Are you trying to say-Use all the drawings as provided, and then when we give you a contract we will reissue the drawings, without changes, other than stating that they are now issued for construction? Please clarify exactly what you mean.						Exhibit A.IV, Clarification of Bid Package. It states that The issued for construction drawings and the Field Order revisions are for construction of Bid Package TG04.5.1 ONLY. However the Delta 1- Field Order- revisions state that the revisions (are) for TG04.3 and TG 04.4. So if they are for construction of Bid Package TG04.5.1 ONLY, why are they revisions for TG04.3 and TG04.4? Are you trying to say-Use all the drawings as provided, and then when we give you a contract we will reissue the drawings, without changes, other than stating that they are now issued for construction? Please clarify exactly what you mean.
W/O to provide answer.						W/O to provide answer.
TG043-0011	Investigative Trench Drawing	Closed	CR	11/02/2010	11/16/2010	11/02/2010
From: Webcor Construction LP David Hungerford						
REQUEST:						ANSWER:
Exhibit A.IV (page 9), Number 11 states to include two investigation trenches as shown on sheets U-1007 & U-1008 dated 8-27-10. This is not the date of the drawings provided. Please clarify.						Exhibit A.IV (page 9), Number 11 states to include two investigation trenches as shown on sheets U-1007 & U-1008 dated 8-27-10. This is not the date of the drawings provided. Please clarify.
W/O to provide answer.						W/O to provide answer.



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<b>TG043-0012</b>	<b>Demolition of Existing Electrical, Gas and Telecom</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/02/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Sheet U1110 and all the other demo sheets. On sheet U1110 note 5 does not specifically state that the gas is to be demolished by PG&E, note 7 does not say duct bank to be removed by AT&T (other notes are similar), but the notes also do not specifically state that ¿Trade contractor is to demolish¿ or something similar. Exhibit A.IV.D.2 (page 7), third bullet, states: ¿Unless noted otherwise on the drawings, Electrical, Gas & Telecommunication lines will be abandon and all feeders removed by the respective utility owners prior to demolition by this contract.¿ Thus there is an ambiguity. Are we, or are we not to include demolition of the Electrical, Gas & Telecommunication lines? In either case, who removes the boxes, vaults etc.? Please clarify. Exactly what is required at all locations on the plans?  W/O to provide answer.						
						<b>ANSWER:</b>  Sheet U1110 and all the other demo sheets. On sheet U1110 note 5 does not specifically state that the gas is to be demolished by PG&E, note 7 does not say duct bank to be removed by AT&T (other notes are similar), but the notes also do not specifically state that ¿Trade contractor is to demolish¿ or something similar. Exhibit A.IV.D.2 (page 7), third bullet, states: ¿Unless noted otherwise on the drawings, Electrical, Gas & Telecommunication lines will be abandon and all feeders removed by the respective utility owners prior to demolition by this contract.¿ Thus there is an ambiguity. Are we, or are we not to include demolition of the Electrical, Gas & Telecommunication lines? In either case, who removes the boxes, vaults etc.? Please clarify. Exactly what is required at all locations on the plans?  W/O to provide answer.
<b>TG043-0013</b>	<b>Demolition of Utilities</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Referenced: Sheet U1110  Are the utilities to the east of the wall, (and on U1111 to the west of the wall) to be demolished by others? Please confirm.						
						<b>ANSWER:</b>  Referenced: Sheet U1110  Are the utilities to the east of the wall, (and on U1111 to the west of the wall) to be demolished by others? Please confirm.
<b>TG043-0014</b>	<b>Temporary Tie In</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Who is to perform Exhibit A.IV.D.4 (page 8), 4th bullet and any other, temporary tie in? Please provide drawing showing exactly what is required for the temporary tie in(s). Include line, grade, size etc.  W/O to provide answer.						
						<b>ANSWER:</b>  Who is to perform Exhibit A.IV.D.4 (page 8), 4th bullet and any other, temporary tie in? Please provide drawing showing exactly what is required for the temporary tie in(s). Include line, grade, size etc.  W/O to provide answer.



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<b>TG043-0015</b>	<b>Tie In Sequence</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP                      David Hungerford						
<b>REQUEST:</b>  Exhibit A.IV.D.4 (Page 8), 7th bullet. This states that "final tie into buildings will be performed after complete commissioning of the water systems at 1st, Howard, & Natoma Streets east of First Street. As this work will not be under our control, how can we be sure we can complete this work in the time allowed for in bid Exhibit A? Please discuss and clarify.  W/O to provide answer.		<b>ANSWER:</b>  Exhibit A.IV.D.4 (Page 8), 7th bullet. This states that "final tie into buildings will be performed after complete commissioning of the water systems at 1st, Howard, & Natoma Streets east of First Street. As this work will not be under our control, how can we be sure we can complete this work in the time allowed for in bid Exhibit A? Please discuss and clarify.  W/O to provide answer.				
<b>TG043-0016</b>	<b>Liquidated Damages</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP                      David Hungerford						
<b>REQUEST:</b>  Are there liquidated damages associated with this subcontract?		<b>ANSWER:</b>  Are there liquidated damages associated with this subcontract?				
<b>TG043-0017</b>	<b>Open Trenches</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP                      David Hungerford						
<b>REQUEST:</b>  Exhibit A.IV.D.11 (page 9) discusses the two investigation trenches to be excavated (and backfilled) in this scope of work. For these trenches, do we open up end to end and plate? If so, how long will they be kept open, who will move and replace the plates etc.? Or, can we trench and backfill concurrently, with representatives observing and taking requested measurements, thus limiting the amount of open trench?		<b>ANSWER:</b>  Exhibit A.IV.D.11 (page 9) discusses the two investigation trenches to be excavated (and backfilled) in this scope of work. For these trenches, do we open up end to end and plate? If so, how long will they be kept open, who will move and replace the plates etc.? Or, can we trench and backfill concurrently, with representatives observing and taking requested measurements, thus limiting the amount of open trench?				
<b>TG043-0018</b>	<b>Mark Up Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP                      David Hungerford						
<b>REQUEST:</b>  Specs. 000700.6.06.C.2 states that markup on labor is direct cost plus Caltrans surcharge plus 15%, or is it direct		<b>ANSWER:</b>  Specs. 000700.6.06.C.2 states that markup on labor is direct cost plus Caltrans surcharge plus 15%, or is it				



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TG043-0019	cost including bonds and insurance plus 15%? As the current Caltrans surcharge is approximately 11% and our liability insurance and WC insurance total 16.21% and adding the approximately 6.20% FICA, 1.45% medicare, 1.5% SF payroll tax, 7.1% unemployment & training taxes equals approximately 32.5% which exceeds the sum of the 11% surcharge and 15% markup, this is a very important point. As currently written, we would do any extra work at a loss. Note that Caltrans uses the surcharge plus 33% for labor, which allows a profit. Please explain why we should bid a project that guarantees that all extra work would be performed at a loss, or change or clarify the specification.	Closed	CR	11/02/2010	11/16/2010	11/04/2010
	direct cost including bonds and insurance plus 15%? As the current Caltrans surcharge is approximately 11% and our liability insurance and WC insurance total 16.21% and adding the approximately 6.20% FICA, 1.45% medicare, 1.5% SF payroll tax, 7.1% unemployment & training taxes equals approximately 32.5% which exceeds the sum of the 11% surcharge and 15% markup, this is a very important point. As currently written, we would do any extra work at a loss. Note that Caltrans uses the surcharge plus 33% for labor, which allows a profit. Please explain why we should bid a project that guarantees that all extra work would be performed at a loss, or change or clarify the specification.					
TG043-0020	Testing Payment Responsibilities	Closed	CR	11/02/2010	11/16/2010	11/03/2010
	Utility Crossing Rate Schedule					
TG043-0019	From: Webcor Construction LP David Hungerford	Closed	CR	11/02/2010	11/16/2010	11/04/2010
	REQUEST: Specs. 000700.8.02. Do we have to pay for any testing such as compaction testing, concrete testing, water quality testing, soils analytical etc.?					
TG043-0020	From: Webcor Construction LP David Hungerford	Closed	CR	11/02/2010	11/16/2010	11/03/2010
	REQUEST: Specs . 000810/APB contains Cost of Utility Crossing Schedules dated January 2004. These are obviously not the current rates. Will we be paid for support and work around of non-governmental and SFWD facilities? If so, will the rates be the rates in effect when the work will be done (2011) or not?					
TG043-0020	From: Webcor Construction LP David Hungerford	Closed	CR	11/02/2010	11/16/2010	11/03/2010
	REQUEST: Specs . 000810/APB contains Cost of Utility Crossing Schedules dated January 2004. These are obviously not the current rates. Will we be paid for support and work around of non-governmental and SFWD facilities? If so, will the rates be the rates in effect when the work will be done (2011) or not?					
TG043-0020	From: Webcor Construction LP David Hungerford	Closed	CR	11/02/2010	11/16/2010	11/03/2010
	REQUEST: Specs . 000810/APB contains Cost of Utility Crossing Schedules dated January 2004. These are obviously not the current rates. Will we be paid for support and work around of non-governmental and SFWD facilities? If so, will the rates be the rates in effect when the work will be done (2011) or not?					
TG043-0020	From: Webcor Construction LP David Hungerford	Closed	CR	11/02/2010	11/16/2010	11/03/2010
	REQUEST: Specs . 000810/APB contains Cost of Utility Crossing Schedules dated January 2004. These are obviously not the current rates. Will we be paid for support and work around of non-governmental and SFWD facilities? If so, will the rates be the rates in effect when the work will be done (2011) or not?					



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<b>TG043-0021</b>	<b>Fire Hydrant Use</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 000813.1.6.  Will we be allowed to use hydrant water?						<b>ANSWER:</b> Specs. 000813.1.6.  Will we be allowed to use hydrant water?
<b>TG043-0022</b>	<b>Excavation - Public Notice</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 000813.1.8.B.  Who does the excavation permit public notifications?						<b>ANSWER:</b> Specs. 000813.1.8.B.  Who does the excavation permit public notifications?
<b>TG043-0023</b>	<b>Waste Management Plan</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/02/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 000815.  Is Webcor/Obayashi's Solid Waste Management plan available? What do we need to do to comply with your plan?  W/O to provide answer.						<b>ANSWER:</b> Specs. 000815.  Is Webcor/Obayashi's Solid Waste Management plan available? What do we need to do to comply with your plan?  W/O to provide answer.
<b>TG043-0024</b>	<b>Unit Prices for Class 1&amp;2 Disposal</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/02/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 011020.  Do you already have unit prices for Class 1 & 2 disposal? Do we have to match these?  W/O to provide answer.						<b>ANSWER:</b> Specs. 011020.  Do you already have unit prices for Class 1 & 2 disposal? Do we have to match these?  W/O to provide answer.



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<b>TG043-0025</b>	<b>Groundwater Discharge</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> For groundwater discharge does the allowance cover all our costs including fees, testing and analysis, metering etc.?					<b>ANSWER:</b> For groundwater discharge does the allowance cover all our costs including fees, testing and analysis, metering etc.?	
<b>TG043-0026</b>	<b>Class 1 - Contaminated Soil</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/03/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 011020.  What is the definition of Class 1 Contaminated soil? Is it Federal Class 1 RCRA or California Class 1 Non-RCRA or what exactly?					<b>ANSWER:</b> Specs. 011020.  What is the definition of Class 1 Contaminated soil? Is it Federal Class 1 RCRA or California Class 1 Non-RCRA or what exactly?	
<b>TG043-0027</b>	<b>Class 2 - Contaminated Soil</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/03/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 011020.  What is the exact definition of Class 2 Contaminated soil?					<b>ANSWER:</b> Specs. 011020.  What is the exact definition of Class 2 Contaminated soil?	
<b>TG043-0028</b>	<b>HASP</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/02/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> SMP plan page 8.  Is your HASP available?  W/O to provide answer.					<b>ANSWER:</b> SMP plan page 8.  Is your HASP available?  W/O to provide answer.	
<b>TG043-0029</b>	<b>Traffic Control Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>					<b>ANSWER:</b>	



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	<p>Specs. 011570.1.2.D requires that the contractor or subcontractor to have a C-31 license to do the traffic control. We have an A license. Does this mean we cannot do our own traffic control, but must instead hire a subcontractor? Or, are you providing traffic control including plans, signs etc.? If we are allowed to provide our own traffic control (or if we must hire a subcontractor) does the three man traffic control need to be dedicated solely to performing traffic control work?</p>					<p>Specs. 011570.1.2.D requires that the contractor or subcontractor to have a C-31 license to do the traffic control. We have an A license. Does this mean we cannot do our own traffic control, but must instead hire a subcontractor? Or, are you providing traffic control including plans, signs etc.? If we are allowed to provide our own traffic control (or if we must hire a subcontractor) does the three man traffic control need to be dedicated solely to performing traffic control work?</p>
<b>TG043-0030</b>	<b>Changeable Message Sign Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
	<p><b>From:</b> Webcor Construction LP      David Hungerford</p> <p><b>REQUEST:</b></p> <p>Specs. 011570.2.4.A.</p> <p>Do we need to include changeable message signs?</p>					<p><b>ANSWER:</b></p> <p>Specs. 011570.2.4.A.</p> <p>Do we need to include changeable message signs?</p>
<b>TG043-0031</b>	<b>K-rail requirements</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
	<p><b>From:</b> Webcor Construction LP      David Hungerford</p> <p><b>REQUEST:</b></p> <p>Specs. 011570.2.5.</p> <p>Do we need to provide K rail?</p>					<p><b>ANSWER:</b></p> <p>Specs. 011570.2.5.</p> <p>Do we need to provide K rail?</p>
<b>TG043-0032</b>	<b>Temp. Tape and Markers</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
	<p><b>From:</b> Webcor Construction LP      David Hungerford</p> <p><b>REQUEST:</b></p> <p>Specs. 011570.2.7.</p> <p>Do we need to provide temporary tape and markers?</p>					<p><b>ANSWER:</b></p> <p>Specs. 011570.2.7.</p> <p>Do we need to provide temporary tape and markers?</p>
<b>TG043-0033</b>	<b>Traffic Loop Repair</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
	<p><b>From:</b> Webcor Construction LP      David Hungerford</p>					







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<b>TG043-0037</b>	<b>Trench Plate installation method</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/02/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Specs. 013565.1.5.B.  This section requires that plates be flush with street or sidewalk. Does this mean all plates must be in cut-in depressions in the street, and not placed on the street with cutback ramps as is normal procedure in SF? Please confirm.						<b>ANSWER:</b>  Specs. 013565.1.5.B.  This section requires that plates be flush with street or sidewalk. Does this mean all plates must be in cut-in depressions in the street, and not placed on the street with cutback ramps as is normal procedure in SF? Please confirm.
<b>TG043-0038</b>	<b>Depth of Bedding above Pipe</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/03/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Specs. 312310.1.8.A.  This section states "pipe zone shall include...from bottom of pipe...to a horizontal level above the top, as specified below." Could not find anywhere "below" where information regarding the depth of the bedding above the top of the pipe was provided. Please provide.						<b>ANSWER:</b>  Specs. 312310.1.8.A.  This section states "pipe zone shall include...from bottom of pipe...to a horizontal level above the top, as specified below." Could not find anywhere "below" where information regarding the depth of the bedding above the top of the pipe was provided. Please provide.
<b>TG043-0039</b>	<b>Pavement Mill and Fill Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/03/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Specs. 321217.3.4.A.  This section states that ¿Contractor shall mill and fill AC pavement as directed by TJPA representative.¿ Prior to bid we must be told exactly which areas are to be milled & filled. If you cannot provide exact and complete parameters (such as a drawing showing the exact limits of the mill and fill required), we will have to assume that absolutely no mill and fill is required. We cannot be expected to know what the TJPA representative will require.						<b>ANSWER:</b>  Specs. 321217.3.4.A.  This section states that ¿Contractor shall mill and fill AC pavement as directed by TJPA representative.¿ Prior to bid we must be told exactly which areas are to be milled & filled. If you cannot provide exact and complete parameters (such as a drawing showing the exact limits of the mill and fill required), we will have to assume that absolutely no mill and fill is required. We cannot be expected to know what the TJPA representative will require.



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<b>TG043-0040</b>	<b>Permanent Pavement Restoration Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/03/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 321724.  Typically in SF all permanent pavement restoration is provided by SF at no cost. Are we to provide permanent restoration of pavement markings or will SF provide at no cost?						<b>ANSWER:</b> Specs. 321724.  Typically in SF all permanent pavement restoration is provided by SF at no cost. Are we to provide permanent restoration of pavement markings or will SF provide at no cost?
<b>TG043-0041</b>	<b>Fire Hydrant Procurement Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs 331100.1.1.B & 331100.3.3.C&D.  There is ambiguity regarding fire hydrants. It appears SFFD installs fire hydrants. Do they supply the hydrants? If not, do we have to pay for the hydrants and valves etc.? If we have to pay, EXACTLY what must we obtain and EXACTLY what is the cost? If we are required to purchase something from one source only, we need to be provided with information regarding what we are buying and what it will cost.						<b>ANSWER:</b> Specs 331100.1.1.B & 331100.3.3.C&D.  There is ambiguity regarding fire hydrants. It appears SFFD installs fire hydrants. Do they supply the hydrants? If not, do we have to pay for the hydrants and valves etc.? If we have to pay, EXACTLY what must we obtain and EXACTLY what is the cost? If we are required to purchase something from one source only, we need to be provided with information regarding what we are buying and what it will cost.
<b>TG043-0042</b>	<b>SFWD Temp. Connections</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/03/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs 331100.1.1.C.  Does SFWD perform temporary connections that are indicated by the bid documents?						<b>ANSWER:</b> Specs 331100.1.1.C.  Does SFWD perform temporary connections that are indicated by the bid documents?
<b>TG043-0043</b>	<b>SFWD Material Transportation</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 331100.1.3.B.  This requires that we move SFFD & SFWD material.						<b>ANSWER:</b> Specs. 331100.1.3.B.  This requires that we move SFFD & SFWD material.



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	<p>What material is that? Perhaps we will understand the fire hydrant situation if previous question, above is answered, but what material is SFWD providing that we have to move? For connections, SFWD has always transported their own material. Please specify exactly what material we need to transport.</p>					<p>What material is that? Perhaps we will understand the fire hydrant situation if previous question, above is answered, but what material is SFWD providing that we have to move? For connections, SFWD has always transported their own material. Please specify exactly what material we need to transport.</p>
<b>TG043-0044</b>	<b>Water Dist. Piping &amp; Valves Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 331100.3.2.E.  See items 2 and 5. What do they mean? This does not make any sense as written. Please provide rewritten specification that can be understood.			<b>ANSWER:</b> Specs. 331100.3.2.E.  See items 2 and 5. What do they mean? This does not make any sense as written. Please provide rewritten specification that can be understood.			
<b>TG043-0045</b>	<b>Side Sewer Replacement Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 333110.1.1.A.4.  This section says ζto replace existing side sewers and/or culverts which are to remain in place as per plans.ζ This could be interpreted to mean that all side sewers and culverts on all portions of the project are to be replaced in their entirety. Is that the intent? If so, please indicate more obviously.			<b>ANSWER:</b> Specs. 333110.1.1.A.4.  This section says ζto replace existing side sewers and/or culverts which are to remain in place as per plans.ζ This could be interpreted to mean that all side sewers and culverts on all portions of the project are to be replaced in their entirety. Is that the intent? If so, please indicate more obviously.			
<b>TG043-0046</b>	<b>ACWS and Planning Limits</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/03/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Specs. 333110.1.1.A.9.  As stated above, please provide exact limits of planning and ACWS.			<b>ANSWER:</b> Specs. 333110.1.1.A.9.  As stated above, please provide exact limits of planning and ACWS.			



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<b>TG043-0047</b>	<b>Catch Basins and Traps</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Reference: Specs 333110.1.1. A.5 & 11.  Please provide exact limits and count of catch basins requiring cleaning and installation of traps and caps.						<b>ANSWER:</b> Reference: Specs 333110.1.1. A.5 & 11.  Please provide exact limits and count of catch basins requiring cleaning and installation of traps and caps.
<b>TG043-0048</b>	<b>Spigot Type</b>	<b>Closed</b>	<b>CR</b>	<b>11/02/2010</b>	<b>11/16/2010</b>	<b>11/04/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> 333110.2.1.B.  Define ¿spigot¿ type. Is this Bell x Spigot or Spigot x Spigot (Band seal) pipe?						<b>ANSWER:</b> 333110.2.1.B.  Define ¿spigot¿ type. Is this Bell x Spigot or Spigot x Spigot (Band seal) pipe?
<b>TG05.02-0001</b>	<b>Inclusion of Engineering Enterprise in Bid</b>	<b>Closed</b>	<b>CR</b>	<b>02/11/2011</b>	<b>02/21/2011</b>	<b>02/14/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Sieera Electric wants to include the Engineering Enterprise for this bid. The Engineering Enterprise (at the advise of their insurance carrier) carries a \$75K deductible not a \$50K deductible. The Engineering Enterprise is registered as an SBE with the State of California. They have been in business for 36 years and never have had a claim filed against them. Their Insurance Company will not allow the deductible changed for any singular project. Is there a way the deductible difference can be waived?						<b>ANSWER:</b> Sieera Electric wants to include the Engineering Enterprise for this bid. The Engineering Enterprise (at the advise of their insurance carrier) carries a \$75K deductible not a \$50K deductible. The Engineering Enterprise is registered as an SBE with the State of California. They have been in business for 36 years and never have had a claim filed against them. Their Insurance Company will not allow the deductible changed for any singular project. Is there a way the deductible difference can be waived?
<b>TG05.02-0002</b>	<b>Amount for Liquidated Damages</b>	<b>Closed</b>	<b>CR</b>	<b>02/11/2011</b>	<b>02/21/2011</b>	<b>02/14/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> An actual Dollar amount is not specified for the LD's. What is the Dollar amount in the LD's and will it be based on per calendar day.						<b>ANSWER:</b> An actual Dollar amount is not specified for the LD's. What is the Dollar amount in the LD's and will it be based on per calendar day.



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<b>TG05.04-0001</b>	<b>Insurance Requirements</b>	<b>Closed</b>	<b>CR</b>	<b>02/10/2011</b>	<b>02/20/2011</b>	<b>02/03/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Does a submitting JV need its own insurance meeting the requirements stated in the RFP, or is the specified coverage being met by the partner firms making up the JV sufficient for the General Contract/Selection Panel?						<b>ANSWER:</b> Does a submitting JV need its own insurance meeting the requirements stated in the RFP, or is the specified coverage being met by the partner firms making up the JV sufficient for the General Contract/Selection Panel?
<b>TG05.04-0002</b>	<b>Definition of a Joint Venture</b>	<b>Closed</b>	<b>CR</b>	<b>02/10/2011</b>	<b>02/20/2011</b>	<b>02/14/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> What is the Agency's Definition of a Joint Venture and what agreements need to be in place if firms want to submit their proposal as a Joint Venture?						<b>ANSWER:</b> What is the Agency's Definition of a Joint Venture and what agreements need to be in place if firms want to submit their proposal as a Joint Venture?
<b>TG05.04-0003</b>	<b>Temporary Pavement Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>02/10/2011</b>	<b>02/20/2011</b>	<b>02/14/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> In the Traffic Control Specification 01570-2F # 10 "TEMPORARY PAVEMENT" is Temporary Pavement in regards to traffic controls limited to the following: A. Pothole Patching; B. "Cutback or Coldpatch" at bridging and plating, handicap ramps, and sidewalk repairs; C. Misc Roadway Maintenance; D. Does not include Roadway Grinding or Hot Asphalt Application.						<b>ANSWER:</b> In the Traffic Control Specification 01570-2F # 10 "TEMPORARY PAVEMENT" is Temporary Pavement in regards to traffic controls limited to the following: A. Pothole Patching; B. "Cutback or Coldpatch" at bridging and plating, handicap ramps, and sidewalk repairs; C. Misc Roadway Maintenance; D. Does not include Roadway Grinding or Hot Asphalt Application.
<b>TG05.2R-0001</b>	<b>Bass Electric - Switch Board AIC Rating</b>	<b>Closed</b>	<b>CR</b>	<b>03/29/2011</b>	<b>03/29/2011</b>	<b>03/29/2011</b>
<b>From:</b> Webcor Construction LP      Daniel Foudy						
<b>REQUEST:</b> Please provide AIC rating for the (5) five 2500 Amp temp switch boards.						<b>ANSWER:</b> Please provide AIC rating for the (5) five 2500 Amp temp switch boards.



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<b>TG05.4-0004</b>	<b>Team Leader Preference</b>	<b>Closed</b>	<b>CR</b>	<b>02/10/2011</b>	<b>02/20/2011</b>	<b>02/10/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Will there be a preference for teams led by a Contractor versus a Professional Services Company.		<b>ANSWER:</b>  Will there be a preference for teams led by a Contractor versus a Professional Services Company.				
<b>TG05.4-0005</b>	<b>CityBuild/First Source Referral Program Certificate</b>	<b>Closed</b>	<b>CR</b>	<b>02/10/2011</b>	<b>02/20/2011</b>	<b>02/10/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  The proposal checklist (attachment 2) includes "CityBuild/FirstSource Referral Program Certificate" but section 00 04 57 includes no Certification form. What should submitters included in their proposal to satisfy this checklist requirement?		<b>ANSWER:</b>  The proposal checklist (attachment 2) includes "CityBuild/FirstSource Referral Program Certificate" but section 00 04 57 includes no Certification form. What should submitters included in their proposal to satisfy this checklist requirement?				
<b>TG05.4-0006</b>	<b>Warning Sign Clarification</b>	<b>Closed</b>	<b>CR</b>	<b>02/10/2011</b>	<b>02/20/2011</b>	<b>02/14/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  In reference to section 01-15-50-6, is the GC committed to the specs laid out for the changeable warning signs or can an alternate sign model be used, so long as it meets/exceeds the capabilities of the model specified?		<b>ANSWER:</b>  In reference to section 01-15-50-6, is the GC committed to the specs laid out for the changeable warning signs or can an alternate sign model be used, so long as it meets/exceeds the capabilities of the model specified?				
<b>TG05.4-0007</b>	<b>Subcontractor List</b>	<b>Closed</b>	<b>CR</b>	<b>02/10/2011</b>	<b>02/20/2011</b>	<b>02/10/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  The proposal checklist states the submissions must include the item "Subcontractor List (SL)" but there is no "subcontractor list" in the package - does the checklist actually refer to the "Subcontracting Request (SR)" included on page 45 of the proposal manual?		<b>ANSWER:</b>  The proposal checklist states the submissions must include the item "Subcontractor List (SL)" but there is no "subcontractor list" in the package - does the checklist actually refer to the "Subcontracting Request (SR)" included on page 45 of the proposal manual?				
<b>TG05.4-0008</b>	<b>Traffic Control Plan Budget</b>	<b>Closed</b>	<b>CR</b>	<b>02/10/2011</b>	<b>02/20/2011</b>	<b>02/10/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>		<b>ANSWER:</b>				



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Has an overall budget for the Traffic Control Plan development and execution been establish or is that TBD?						
Has an overall budget for the Traffic Control Plan development and execution been establish or is that TBD?						
<b>TG05.4-0009</b>	<b>Non-Discrimination in Contracts and Benefits</b>	<b>Closed</b>	<b>CR</b>	<b>02/10/2011</b>	<b>02/20/2011</b>	<b>02/10/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> The proposal checklist in Attachment 2 specifies "NonDiscrimination in Contracts and Benefits (section 00 04 70)" but the only mention of this section is in the Specifications' table of contents where the title and information are struck through/crossed out. What do the submitters need to include in proposals to satisfy the checklist requirement?						
<b>ANSWER:</b> The proposal checklist in Attachment 2 specifies "NonDiscrimination in Contracts and Benefits (section 00 04 70)" but the only mention of this section is in the Specifications' table of contents where the title and information are struck through/crossed out. What do the submitters need to include in proposals to satisfy the checklist requirement?						
<hr/>						
<b>TG08.1-A001</b>	<b>Blast Loading</b>	<b>Closed</b>	<b>CR</b>	<b>11/30/2010</b>	<b>12/14/2010</b>	<b>12/06/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b> Spec Section/Dwg Sheet: 08 44 26 - 1.5.B.f  Is "Blast Loading" criteria applicable to all systems? It is shown in all of the specifications, but is only used by Schlaich Bergman in the design of W-3 (CW-2).						
<b>ANSWER:</b> Spec Section/Dwg Sheet: 08 44 26 - 1.5.B.f  Is "Blast Loading" criteria applicable to all systems? It is shown in all of the specifications, but is only used by Schlaich Bergman in the design of W-3 (CW-2).						



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TG08.1-A002	Spec Clarification	Closed	CR	11/30/2010	12/14/2010	12/06/2010
From: Webcor Construction LP                      David Hungerford						
REQUEST:  Spec Section/Dwg Sheet: 08 44 03; S1-6000; 08 44 36; S1-6001  In drawing S1-6000, specification 08 44 03 is mentioned for three wall types (W-1, W-3 & W-8). According to the table of contents, specification 08 44 03 does not exist. On drawing S1-6001, refers to spec 08 44 36 for the skylight (W-10). This spec section refers to W-8. Please clarify the applicable spec section.						ANSWER:  Spec Section/Dwg Sheet: 08 44 03; S1-6000; 08 44 36; S1-6001  In drawing S1-6000, specification 08 44 03 is mentioned for three wall types (W-1, W-3 & W-8). According to the table of contents, specification 08 44 03 does not exist. On drawing S1-6001, refers to spec 08 44 36 for the skylight (W-10). This spec section refers to W-8. Please clarify the applicable spec section.
TG08.1-A003	Wall Type Nomenclature	Closed	CR	11/30/2010	12/14/2010	12/06/2010
From: Webcor Construction LP                      David Hungerford						
REQUEST:  The drawings reference "CW" wall types and the IFB and specifications reference "wall types". Please confirm the following correlation: W-1 = RSC-1 & RSC-2; W-3 = C-1; W-4 = C-2; W-8 = CW-3; W-10 = SL-1. Please clarify the applicable specification section.						ANSWER:  The drawings reference "CW" wall types and the IFB and specifications reference "wall types". Please confirm the following correlation: W-1 = RSC-1 & RSC-2; W-3 = C-1; W-4 = C-2; W-8 = CW-3; W-10 = SL-1. Please clarify the applicable specification section.
TG08.1-A004	Glass Specification	Closed	CR	11/30/2010	12/14/2010	12/06/2010
From: Webcor Construction LP                      David Hungerford						
REQUEST:  Spec Section/Dwg Sheet: 08 44 26-25; 2.2-A-1  Please clarify the glass specification; According to spec 08 44 26-25, para 2.2-A-1, "heat strengthened" according to spec 08 44 26-25, para 2.2-A-1-a, "full tempered float glass". Which should it be?						ANSWER:  Spec Section/Dwg Sheet: 08 44 26-25; 2.2-A-1  Please clarify the glass specification; According to spec 08 44 26-25, para 2.2-A-1, "heat strengthened" according to spec 08 44 26-25, para 2.2-A-1-a, "full tempered float glass". Which should it be?
TG08.1-A005	Glass Spec - Frit	Closed	CR	11/30/2010	12/14/2010	12/06/2010
From: Webcor Construction LP                      David Hungerford						
REQUEST:  Spec Section/Dwg Sheet: 08 44 26-25 / A-8021 & A1-8140						ANSWER:  Spec Section/Dwg Sheet: 08 44 26-25 / A-8021 & A1-8140





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	No frit is required according to spec 08 44 26-25, para 2.2-A-1-a Glass with frit is shown on dwgs. A-8021 / A1-8140. Should there be glass with frit or not?					No frit is required according to spec 08 44 26-25, para 2.2-A-1-a Glass with frit is shown on dwgs. A-8021 / A1-8140. Should there be glass with frit or not?
TG08.1-A006	Corner Supported Glazing Assemb (W-1)	Closed	CR	11/30/2010	12/14/2010	12/06/2010
From: Webcor Construction LP David Hungerford						
REQUEST: Spec Section/Dwg Sheet: 08 44 26-2  3. Joints: Ventilation shall be provided through the awning through the 3/4" gaps between each of the glass panels, according to spec 08 44 26-2, para 1.2-A-5. [More]"¿designed using rainscreen system with 2 layers of defense with rainscreen seal and continuous air seal system." - spec 08 44 26-8, para 1.5-D. Interpret that the joints shall be opened, not sealed. If the joints will not be sealed, all items specifying the sealing system in spec 08 44 26, will not be applied to W-1? Please confirm the interpretation.						ANSWER: Spec Section/Dwg Sheet: 08 44 26-2  3. Joints: Ventilation shall be provided through the awning through the 3/4" gaps between each of the glass panels, according to spec 08 44 26-2, para 1.2-A-5. [More]"¿designed using rainscreen system with 2 layers of defense with rainscreen seal and continuous air seal system." - spec 08 44 26-8, para 1.5-D. Interpret that the joints shall be opened, not sealed. If the joints will not be sealed, all items specifying the sealing system in spec 08 44 26, will not be applied to W-1? Please confirm the interpretation.
TG08.1-A007	Cable supported glazed curtain wall (W-3) - Steel	Closed	CR	11/30/2010	12/14/2010	11/30/2010
From: Webcor Construction LP David Hungerford						
REQUEST: Spec Section/Dwg Sheet: 08 44 33-2 / A1-8100; A1-8201  1. Steel spec: "with stainless steel Tee sections consisting of steel plates and a double row of stainless steel cables" - spec 08 44 33-2, para. 1.2-A-1.; [note] "PTD. Galv steel horizontal girder" and "S.S. clip screwed to welded T-section" per dwg. A1-8100; [note] "painted galv. steel horizontal girder" per dwg A1-8201. Interpret that painted galv. steel horizontal T-section and stainless steel cables and cable clamps are required. Is that correct?						ANSWER: Spec Section/Dwg Sheet: 08 44 33-2 / A1-8100; A1-8201  1. Steel spec: "with stainless steel Tee sections consisting of steel plates and a double row of stainless steel cables" - spec 08 44 33-2, para. 1.2-A-1.; [note] "PTD. Galv steel horizontal girder" and "S.S. clip screwed to welded T-section" per dwg. A1-8100; [note] "painted galv. steel horizontal girder" per dwg A1-8201. Interpret that painted galv. steel horizontal T-section and stainless steel cables and cable clamps are required. Is that correct?
TG08.1-A008	Cable supported glazed curtain wall (W-3) - Glass	Closed	CR	11/30/2010	12/14/2010	12/06/2010



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<b>From:</b> Webcor Construction LP	David Hungerford					
<b>REQUEST:</b>			<b>ANSWER:</b>			
Spec Section/Dwg Sheet: 08 44 33-27			Spec Section/Dwg Sheet: 08 44 33-27			
2. Glass spec: "heat strengthened" glass per spec 08 44 33-27, para 2.2-C; "fully tempered float glass" per spec 08 44 33-27, para 2,2-C-1 & C-2. Should the glass be "heat strengthened" or "fully tempered"?			2. Glass spec: "heat strengthened" glass per spec 08 44 33-27, para 2.2-C; "fully tempered float glass" per spec 08 44 33-27, para 2,2-C-1 & C-2. Should the glass be "heat strengthened" or "fully tempered"?			
<b>TG08.1-A009</b>	<b>Cable supported glazed curtain wall (W-3) - Glass Type GL-1B</b>	<b>Closed</b>	<b>CR</b>	<b>11/30/2010</b>	<b>12/14/2010</b>	<b>12/06/2010</b>
<b>From:</b> Webcor Construction LP	David Hungerford					
<b>REQUEST:</b>			<b>ANSWER:</b>			
Spec Section/Dwg Sheet: 08 44 33-27			Spec Section/Dwg Sheet: 08 44 33-27			
3. Glass spec: GL-1B are spandrel panels according to spec 08 44 33-27, para 2.2-C-2. Cannot locate GL-1B in the drawings. Please advise on location of type GL-1B.			3. Glass spec: GL-1B are spandrel panels according to spec 08 44 33-27, para 2.2-C-2. Cannot locate GL-1B in the drawings. Please advise on location of type GL-1B.			
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<b>TG08.1-A010</b>	<b>Cable supported glazed curtain wall (W-3) - Fall Protection system</b>	<b>Closed</b>	<b>CR</b>	<b>11/30/2010</b>	<b>12/14/2010</b>	<b>11/30/2010</b>
<b>From:</b> Webcor Construction LP	David Hungerford					
<b>REQUEST:</b>			<b>ANSWER:</b>			
Spec Section/Dwg Sheet: 08 44 33.7			Spec Section/Dwg Sheet: 08 44 33.7			
4. Fall protection system: per spec 08 44 33-7, para 1.5-A-3, should the fall protection system be included in the W-3 package? Please advise of the locations if the fall protection package is required in this assembly.			4. Fall protection system: per spec 08 44 33-7, para 1.5-A-3, should the fall protection system be included in the W-3 package? Please advise of the locations if the fall protection package is required in this assembly.			
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<b>TG08.1-A011</b>	<b>Cable supported glazed curtain wall (W-3) - Firestopping</b>	<b>Closed</b>	<b>CR</b>	<b>11/30/2010</b>	<b>12/14/2010</b>	<b>12/06/2010</b>
<b>From:</b> Webcor Construction LP	David Hungerford					
<b>REQUEST:</b>			<b>ANSWER:</b>			
Spec Section/Dwg Sheet: 08 44 33-28			Spec Section/Dwg Sheet: 08 44 33-28			
5. Fire stopping: per spec 08 44 33-28, para 2.4-C, is there any fire stopping required for package W-3? If so, please advise on the location of the fire stopping.			5. Fire stopping: per spec 08 44 33-28, para 2.4-C, is there any fire stopping required for package W-3? If so, please advise on the location of the fire stopping.			



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<b>TG08.1-A012</b>	<b>Cable supported glazed curtain wall (W-3) - Documents</b>	<b>Closed</b>	<b>CR</b>	<b>11/30/2010</b>	<b>12/14/2010</b>	<b>12/07/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Spec Section/Dwg Sheet: 08 44 33-20  6. Missing documents - spec section 01 35 73 is mentioned in 08 44 33-20, para 1.8-E; spec section 05 12 13 is mentioned in spec 08 44 33-29, para 2.5-B. [Please provide if they are required.]						<b>ANSWER:</b>  Spec Section/Dwg Sheet: 08 44 33-20  6. Missing documents - spec section 01 35 73 is mentioned in 08 44 33-20, para 1.8-E; spec section 05 12 13 is mentioned in spec 08 44 33-29, para 2.5-B. [Please provide if they are required.]
<b>TG08.1-A013</b>	<b>Steel-framed glazed curtain wall (W-8) - Glass</b>	<b>Closed</b>	<b>CR</b>	<b>11/30/2010</b>	<b>12/14/2010</b>	<b>12/07/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Spec Section/Dwg Sheet: 08 44 36-26  1. Glass spec: "heat strengthened" glass accoring to spec 08 44 36-26, para 2.2-A; "full tempered float glass" according to spec 08 44 36-26, para 2.2-A-1 & A-2; "laminated float glass" according to spec 08 44 36-26, para 2.2-A-3 & A-4. Should glass types GL-1 & GL-1A be "heat strengthened" or "fully tempered float glass"? Should glass types GL-2 & GL-2A be "heat strengthened" or "float glass"?						<b>ANSWER:</b>  Spec Section/Dwg Sheet: 08 44 36-26  1. Glass spec: "heat strengthened" glass according to spec 08 44 36-26, para 2.2-A; "full tempered float glass" according to spec 08 44 36-26, para 2.2-A-1 & A-2; "laminated float glass" according to spec 08 44 36-26, para 2.2-A-3 & A-4. Should glass types GL-1 & GL-1A be "heat strengthened" or "fully tempered float glass"? Should glass types GL-2 & GL-2A be "heat strengthened" or "float glass"?
<b>TG08.1-A014</b>	<b>Steel-framed glazed curtain wall (W-8) - Glass Types GL-2 &amp; GL-2A</b>	<b>Closed</b>	<b>CR</b>	<b>11/30/2010</b>	<b>12/14/2010</b>	<b>12/06/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Spec Section/Dwg Sheet: 08 44 36-26  2. Glass spec: according to 08 44 36-26, para 2.2-A-3 & A-4, there are glass types GL-2 & GL-2A. Please advise on the location(s) of GL-2 & GL-2A.						<b>ANSWER:</b>  Spec Section/Dwg Sheet: 08 44 36-26  2. Glass spec: according to 08 44 36-26, para 2.2-A-3 & A-4, there are glass types GL-2 & GL-2A. Please advise on the location(s) of GL-2 & GL-2A.
<b>TG08.1-A015</b>	<b>Steel-framed glazed curtain wall (W-8) - Glass Frit</b>	<b>Closed</b>	<b>CR</b>	<b>11/30/2010</b>	<b>12/14/2010</b>	<b>12/07/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Spec Section/Dwg Sheet: 08 44 36-26 / A1-8351  3. Glass spec: According to spec 08 44 36-26, para 2.2-						<b>ANSWER:</b>  Spec Section/Dwg Sheet: 08 44 36-26 / A1-8351  3. Glass spec: According to spec 08 44 36-26, para



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	A-1, no frit is required for GL-1. Glass with frit is shown on dwg. A1-8351. Should this glass be provided with frit or not?					
TG08.1-A016	Steel-framed glazed curtain wall (W-8) - Removable sections	Closed	CR	11/30/2010	12/14/2010	12/07/2010
From: Webcor Construction LP                      David Hungerford						
REQUEST:						
Spec Section/Dwg Sheet: 08 44 36-26						
4. Removable sections; please clarify and advise on the location(s) of the "removable section of curtain wall" per spec 08 44 36-2 & 36-3, para 1.2-A-12 and parap 1.2-B-12.						



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<b>TG08.1-A019</b>	<b>Metal-framed skylights (W-10) - Glass</b>	<b>Closed</b>	<b>CR</b>	<b>11/30/2010</b>	<b>12/14/2010</b>	<b>12/06/2010</b>
<b>From:</b> Webcor Construction LP                      David Hungerford						
<b>REQUEST:</b>  Spec Section/Dwg Sheet: 08 63 03-25  2. Glass spec: per spec 08 63 03-25, para 2.2-A, shown to be "heat strengthened" glass; per spec 08 63 03-25, para 2.2-A-1, shown as "fully tempered float glass". Should the glass be "heat strengthened" or "fully tempered"?						<b>ANSWER:</b>  Spec Section/Dwg Sheet: 08 63 03-25  2. Glass spec: per spec 08 63 03-25, para 2.2-A, shown to be "heat strengthened" glass; per spec 08 63 03-25, para 2.2-A-1, shown as "fully tempered float glass". Should the glass be "heat strengthened" or "fully tempered"?
<b>TG08.1-A021</b>	<b>Steel (W-10) - AESS type</b>	<b>Closed</b>	<b>CR</b>	<b>12/01/2010</b>	<b>12/15/2010</b>	<b>12/07/2010</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b>  It is specified Architecturally exposed structural steel, but drawing A1-8401 indicates stainless steel for system W-10. What material shall be used for the grid shell of the skylights?						<b>ANSWER:</b>  It is specified Architecturally exposed structural steel, but drawing A1-8401 indicates stainless steel for system W-10. What material shall be used for the grid shell of the skylights?
<b>TG08.1-A022</b>	<b>Blast Loading Reqs</b>	<b>Closed</b>	<b>CR</b>	<b>12/01/2010</b>	<b>12/15/2010</b>	<b>12/07/2010</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b>  The blast requirement is mentioned in all above listed specifications but the "Transbay transit Center Basis of Design Report" from Schlaich Pergermann and Partner LP dated October 18, 2010, indicates blast load only for the Grand Hall façade (w-3)? Which glazing systems have a bomb blast requirement?						<b>ANSWER:</b>  The blast requirement is mentioned in all above listed specifications but the "Transbay transit Center Basis of Design Report" from Schlaich Pergermann and Partner LP dated October 18, 2010, indicates blast load only for the Grand Hall façade (w-3)? Which glazing systems have a bomb blast requirement?
<b>TG19.01-0001</b>	<b>TG19.1 Questions 0001 - (E) Lighting</b>	<b>Closed</b>	<b>CR</b>	<b>10/11/2010</b>	<b>10/25/2010</b>	<b>10/13/2010</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b>  Ref C-2000  There are currently (E) light fixtures in the (5) planter boxes. Are we required to stub up electrical conduits for the future installation of light fixtures in the planter boxes?						<b>ANSWER:</b>  Ref C-2000  There are currently (E) light fixtures in the (5) planter boxes. Are we required to stub up electrical conduits for the future installation of light fixtures in the planter boxes?



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<b>TG19.1-0002</b>	<b>TG19.1 Questions 0002 - Stone</b>	<b>Closed</b>	<b>CR</b>	<b>10/11/2010</b>	<b>10/25/2010</b>	<b>10/13/2010</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b> Ref S-0002 Note 6-B  Please provide "Architect Approved" stone support system, anchors, and accessories for the stone walls.		<b>ANSWER:</b> Ref S-0002 Note 6-B  Please provide "Architect Approved" stone support system, anchors, and accessories for the stone walls.				
<b>TG19.1-0003</b>	<b>TG19.1 Questions 0003 - Stone</b>	<b>Closed</b>	<b>CR</b>	<b>10/11/2010</b>	<b>10/25/2010</b>	<b>10/13/2010</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b> Ref S-0002 Note 6-A  Please provide manufacturer & specs for the stone panels.		<b>ANSWER:</b> Ref S-0002 Note 6-A  Please provide manufacturer & specs for the stone panels.				
<b>TG19.1-0004</b>	<b>TG19.1 Questions 0004 - (N) Lighting</b>	<b>Closed</b>	<b>CR</b>	<b>10/11/2010</b>	<b>10/25/2010</b>	<b>10/13/2010</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b> Ref C-2000 Note 10  Please provide the locations where the (N) in ground lights will be reconnected to. Electrician needs to know where the (N) in-ground lights will receive power.		<b>ANSWER:</b> Ref C-2000 Note 10  Please provide the locations where the (N) in ground lights will be reconnected to. Electrician needs to know where the (N) in-ground lights will receive power.				
<b>TG19.1-0005</b>	<b>TG19.1 Questions 0005 - (E) Lighting</b>	<b>Closed</b>	<b>CR</b>	<b>10/11/2010</b>	<b>10/25/2010</b>	<b>10/13/2010</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b> Ref C-2000 Note 10  Please provide specifications for the (E) in-ground lights. Require specs in order to match manufacture, size, and model type.		<b>ANSWER:</b> Ref C-2000 Note 10  Please provide specifications for the (E) in-ground lights. Require specs in order to match manufacture, size, and model type.				
<b>TG19.1-0006</b>	<b>(E) Lighting</b>	<b>Closed</b>	<b>CR</b>	<b>10/15/2010</b>	<b>10/29/2010</b>	<b>10/13/2010</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						





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<b>TG19.1-0009</b>	<b>Joint Sealant</b>	<b>Closed</b>	<b>CR</b>	<b>10/15/2010</b>	<b>10/29/2010</b>	<b>10/15/2010</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b> A-B/A-6000  In details A&B, the drawings show a 1/2" joint between the aluminum composite panel and the epoxy set stone panel with no sealant. The (e) wall joint is caulked with sealant. Should this joint be sealed?						<b>ANSWER:</b> A-B/A-6000  In details A&B, the drawings show a 1/2" joint between the aluminum composite panel and the epoxy set stone panel with no sealant. The (e) wall joint is caulked with sealant. Should this joint be sealed?
<b>TG19.1-0010</b>	<b>Painting</b>	<b>Closed</b>	<b>CR</b>	<b>10/15/2010</b>	<b>10/29/2010</b>	<b>10/15/2010</b>
<b>From:</b> Webcor Construction LP                      Joanne Filipas						
<b>REQUEST:</b> A&D/A-6000  The drawings call for the wall on the transbay terminal side to receive 16ga galv. Coated G90 rolled steel panels. The (e) wall looks to be plaster w/expansion joints. Are there any specific requirements for joints etc. on the (n) wall. Does the (n) wall get painted?						<b>ANSWER:</b> A&D/A-6000  The drawings call for the wall on the transbay terminal side to receive 16ga galv. Coated G90 rolled steel panels. The (e) wall looks to be plaster w/expansion joints. Are there any specific requirements for joints etc. on the (n) wall. Does the (n) wall get painted?
<b>TG19.1-0011</b>	<b>TG19.1 Questions 0011 - Concrete Curbs</b>	<b>Closed</b>	<b>CR</b>	<b>10/15/2010</b>	<b>11/18/2010</b>	<b>10/15/2010</b>
<b>From:</b> Webcor/Obayashi Joint Venture                      Manuel Saldana						
<b>REQUEST:</b> C-2000 & C-5000 Plan view on sheet sht C-2000 calls for (4) new 9" x 12" concrete curbs. Detail B-C-5000 shows the curbs in section. Do these curbs get placed on top of the (E) pavers of do they go down on top of the 4" topping slab w/pavers around the (N) curb? Are there any rebar or epoxy anchorage requirements?						<b>ANSWER:</b> C-2000 & C-5000 Plan view on sheet sht C-2000 calls for (4) new 9" x 12" concrete curbs. Detail B-C-5000 shows the curbs in section. Do these curbs get placed on top of the (E) pavers of do they go down on top of the 4" topping slab w/pavers around the (N) curb? Are there any rebar or epoxy anchorage requirements?
<b>TG4.2R-0001</b>	<b>AWSS Experience Requirement</b>	<b>Closed</b>	<b>03</b>	<b>01/24/2011</b>	<b>02/03/2011</b>	<b>01/28/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b> Reference BOE Specifications Section 02723, Part 3  There was discussion at the Pre-Bid Meeting that an						<b>ANSWER:</b> Reference BOE Specifications Section 02723, Part 3  There was discussion at the Pre-Bid Meeting that an













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	<p>Field verified measurements and layout for the location of the structural steel does not coordinate with the stucco inset locations as shown on detail C/S-5000. In addition framing around the perimeter of the wall (aluminum panel locations) had to be modified due to assembly and installation methods. (See attached pictures and sketches. This RFI addresses three framing issues. All issues have been discussed in the weekly 301 Mission Wall subcontractor meeting with URS, Turner, Transworld, TJPA and Webcor-Obayashi.</p> <p>1.) In two of the four stucco slot locations, field conditions show that a portion of the base plate conflicts with the stucco slot. This base plate encroaches into the stucco panel per dimensions shown on the attached sketch. Please advise.</p> <p>2.) The structural steel had been relocated to CL of the wall (per RFI T-0098) and therefore studs around the steel per B/A-6000 could not be set per plan. Transworld has installed hat channel metal framing to the face of the structural steel tube using fasteners into the structural steel as per RFI T-0106 as well as modified the boxed framing per attached sketches around the perimeter of the wall. Sizes of metal framing were used to align with adjacent framing per plan. This work is currently installed, please confirm framing modifications per attached marked up details are acceptable.</p> <p>3.) Blocking at the top of the wall at the north side (between the framing and 8"x 8" tube steel) was not installed, as there was no room between the framing and steel. Framing was attached directly to the tube steel. See attached.</p> <p>Please confirm that the framing modifications in item 2 and 3 are acceptable and provide direction at the base plate conflict per item 1.</p>					
	<p>Field verified measurements and layout for the location of the structural steel does not coordinate with the stucco inset locations as shown on detail C/S-5000. In addition framing around the perimeter of the wall (aluminum panel locations) had to be modified due to assembly and installation methods. (See attached pictures and sketches. This RFI addresses three framing issues. All issues have been discussed in the weekly 301 Mission Wall subcontractor meeting with URS, Turner, Transworld, TJPA and Webcor-Obayashi.</p> <p>1.) In two of the four stucco slot locations, field conditions show that a portion of the base plate conflicts with the stucco slot. This base plate encroaches into the stucco panel per dimensions shown on the attached sketch. Please advise.</p> <p>2.) The structural steel had been relocated to CL of the wall (per RFI T-0098) and therefore studs around the steel per B/A-6000 could not be set per plan. Transworld has installed hat channel metal framing to the face of the structural steel tube using fasteners into the structural steel as per RFI T-0106 as well as modified the boxed framing per attached sketches around the perimeter of the wall. Sizes of metal framing were used to align with adjacent framing per plan. This work is currently installed, please confirm framing modifications per attached marked up details are acceptable.</p> <p>3.) Blocking at the top of the wall at the north side (between the framing and 8"x 8" tube steel) was not installed, as there was no room between the framing and steel. Framing was attached directly to the tube steel. See attached.</p> <p>Please confirm that the framing modifications in item 2 and 3 are acceptable and provide direction at the base plate conflict per item 1.</p>					



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<b>TRANSWORLD 028</b>	<b>Install the sleeves for light fixtures</b>	<b>Closed</b>	<b>CR</b>	<b>04/14/2011</b>	<b>04/24/2011</b>	<b>04/14/2011</b>
<b>From:</b> Transworld Construction, Inc. Erik Liu						
<b>REQUEST:</b> Per W/O Field directions, TCI was required to install 1-1/2" sleeves for future light fixtures at new concrete footing below the asphalt paver. Please confirm if this is acceptable.					<b>ANSWER:</b> Per W/O Field directions, TCI was required to install 1-1/2" sleeves for future light fixtures at new concrete footing below the asphalt paver. Please confirm if this is acceptable.	
<b>TRANSWORLD 029</b>	<b>Extra HSS Steel Column needed</b>	<b>Closed</b>	<b>CR</b>	<b>04/13/2011</b>	<b>04/23/2011</b>	<b>04/13/2011</b>
<b>From:</b> Transworld Construction, Inc. Erik Liu						
<b>REQUEST:</b> Reference: S-4000  On sheet S-4000, it is indicated that the tube steel should be maintained 8" clear on both sides where the utility vault is located. The two (2) steel tube at the east end wall is more than 5' apart. Please clarify that an additional tube steel is needed?					<b>ANSWER:</b> Reference: S-4000  On sheet S-4000, it is indicated that the tube steel should be maintained 8" clear on both sides where the utility vault is located. The two (2) steel tube at the east end wall is more than 5' apart. Please clarify that an additional tube steel is needed?	
<b>TRANSWORLD 031</b>	<b>Stone and Aluminum Panel layout sketch</b>	<b>Closed</b>	<b>CR</b>	<b>06/08/2011</b>	<b>04/19/2011</b>	<b>04/19/2011</b>
<b>From:</b> Webcor Construction LP David Hungerford						
<b>REQUEST:</b> Please confirm the attached aluminum and stone tile layout is acceptable.					<b>ANSWER:</b> Please confirm the attached aluminum and stone tile layout is acceptable.	
<b>TRANSWORLD 038</b>	<b>Concrete mix design for concrete repair work</b>	<b>Closed</b>	<b>CR</b>	<b>06/08/2011</b>	<b>06/18/2011</b>	<b>06/08/2011</b>
<b>From:</b> Webcor Construction LP David Hungerford						
<b>REQUEST:</b> This is to respond to submittal title TA1010.S-5000.A01 Rapid Set for Concrete Repair ( TCI #31) Please identify a product or a custom mix design that would meet these specifications. In our past practice we have successfully used the suggested grout product as a concrete patch. In our investigations with suppliers and other contractors, it seems that the general conclusion is that using a grout product (such as the one proposed) would be					<b>ANSWER:</b> This is to respond to submittal title TA1010.S-5000.A01 Rapid Set for Concrete Repair ( TCI #31) Please identify a product or a custom mix design that would meet these specifications. In our past practice we have successfully used the suggested grout product as a concrete patch. In our investigations with suppliers and other contractors, it seems that the general conclusion is that using a grout product (such as the one proposed) would be	



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	<p>the appropriate product for this application and condition. The proposed grout seems to offer greater strength and structural performance than the original concrete that has since been removed.</p> <p>Is there another product that you could identify that would achieve these specifications? Since we are not the project designers, we can only suggest those products that would generally be used and accepted in our standard of practice It was based on this standard of practice that we submitted the RapidSet grout product.</p>					<p>the appropriate product for this application and condition. The proposed grout seems to offer greater strength and structural performance than the original concrete that has since been removed.</p> <p>Is there another product that you could identify that would achieve these specifications? Since we are not the project designers, we can only suggest those products that would generally be used and accepted in our standard of practice It was based on this standard of practice that we submitted the RapidSet grout product.</p>
TRANSWORLD 039	301 Mission Wall - New concrete curb detail	Closed	CR	06/13/2011	06/30/2011	06/13/2011
From: Webcor Construction LP      David Hungerford						
REQUEST:			ANSWER:			
Please provide detail for the new concrete curb			Please provide detail for the new concrete curb			
U-0001	First Street Electrical or Telecom Trench	Closed	CR	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP      Joanne Filipas						
REQUEST:			ANSWER:			
Ref U-2016, U-2020 and Attached			Ref U-2016, U-2020 and Attached			
Sheet U-2016 calls out a 9-6", 1-4" E by PG&E. Sheet U-2020 calls out the same trench as AT&T's. The section shows it as a AT&T's. Please confirm this trench is AT&T's.			Sheet U-2016 calls out a 9-6", 1-4" E by PG&E. Sheet U-2020 calls out the same trench as AT&T's. The section shows it as a AT&T's. Please confirm this trench is AT&T's.			



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U-0002	Conflict with Electrical and Water Pipe Station 5.50	Closed	03	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST:		ANSWER:				
Ref U-3408 and attached.		Ref U-3408 and attached.				
During the review of the model, we have found that a conflict exists between the joint trench electrical conduits and water pipes. Please advise.		During the review of the model, we have found that a conflict exists between the joint trench electrical conduits and water pipes. Please advise.				
U-0003	Conflict Between Electrical trench and telecom conduit near station 1.50	Closed	03	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST:		ANSWER:				
Ref U-2007, and attached		Ref U-2007, and attached				
During our review of the model, we have found a conflict between the electrical joint trench and telecom conduit near station 1.50 on Minna Street. Please advise.		During our review of the model, we have found a conflict between the electrical joint trench and telecom conduit near station 1.50 on Minna Street. Please advise.				
U-0004	Telecom and Water Conflict Station 3.25	Closed	03	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST:		ANSWER:				
Ref U-2007 and attached.		Ref U-2007 and attached.				
During our review of the model, we have found that the water latereal running north on Minna street is in conflict with telecom conduits in the joint trench. Please advise.		During our review of the model, we have found that the water latereal running north on Minna street is in conflict with telecom conduits in the joint trench. Please advise.				
U-0005	Water, Telecom and Electrical Conflict at Station 5.50	Closed	03	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST:		ANSWER:				
Ref U-2008, U-2030 and attached.		Ref U-2008, U-2030 and attached.				
During our review of the model, we have found that the water system running in the east/west direction along Minna Street at station 5.50 is in conflict at three locations with the Electrical/Telecom joint trench. Please advise.		During our review of the model, we have found that the water system running in the east/west direction along Minna Street at station 5.50 is in conflict at three locations with the Electrical/Telecom joint trench.				





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				Please advise.		
U-0006	Gas and Electrical Conduit Conflict	Closed	03	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST:				ANSWER:		
Ref U-2008, U-2030 and attached.				Ref U-2008, U-2030 and attached.		
A conflict exists between the 4" HPG and electrical conduits near station 6.45. Please advise.				A conflict exists between the 4" HPG and electrical conduits near station 6.45. Please advise.		
U-0007	Water and Electrical Conduit Conflict at Station 6.50	Closed	03	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST:				ANSWER:		
Ref U-2030 and attached.				Ref U-2030 and attached.		
The water line running east/west along Minna street is in conflict with an Electrical trench at station 6.45. Please advise.				The water line running east/west along Minna street is in conflict with an Electrical trench at station 6.45. Please advise.		
U-0008	Gas and Water Conflict at Station 7.30	Closed	03	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST:				ANSWER:		
Ref U-2009 and attached.				Ref U-2009 and attached.		
A conflict exists between the HPG and water line at station 7.30 along Minna Street. Please advise.				A conflict exists between the HPG and water line at station 7.30 along Minna Street. Please advise.		
U-0009	Joint Trench and Sewer Conflict on First Street at Station 9.25	Closed	CR	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST:				ANSWER:		
Ref U-2009 and attached				Ref U-2009 and attached		
The sewer line running in the north south direction at				The sewer line running in the north south direction at		



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	station 9.25 along First street is in conflict with the electrical joint trench. Please advise.				station 9.25 along First street is in conflict with the electrical joint trench. Please advise.	
U-0010	Electrical Line Transition In Joint Trench from Minna to Shaw Alley	Closed	03	10/25/2010	11/08/2010	11/05/2010
	From: Webcor Construction LP                      Joanne Filipas					
	REQUEST: Ref U-3408, Q/U-3410, P/U-3410 attached.  Section Q/U-3410 shows a 5" and 2" electrical line on the north side of the joint trench. Section P/U-3410 shows the same 5" and 2" electrical lines on the west side of the joint trench as it turns north on Shaw Alley. Is the intent for these electrical lines to cross within the joint trench? Please advise.				ANSWER: Ref U-3408, Q/U-3410, P/U-3410 attached.  Section Q/U-3410 shows a 5" and 2" electrical line on the north side of the joint trench. Section P/U-3410 shows the same 5" and 2" electrical lines on the west side of the joint trench as it turns north on Shaw Alley. Is the intent for these electrical lines to cross within the joint trench? Please advise.	
U-0011	Manhole #203 Elevation Conflict	Closed	03	10/25/2010	11/08/2010	11/05/2010
	From: Webcor Construction LP                      Joanne Filipas					
	REQUEST: Ref U3031, U3007 and attached.  Detail B/U-3031 shows the elavtion of manhole #203 at 21.75 however U-3007 calls out an elevation of 22.0. Please confirm what the elavation of Manhole #203 is.				ANSWER: Ref U3031, U3007 and attached.  Detail B/U-3031 shows the elavtion of manhole #203 at 21.75 however U-3007 calls out an elevation of 22.0. Please confirm what the elavation of Manhole #203 is.	
U-0012	Electrical/Telecom Conflicts between Plan and Section	Closed	03	10/25/2010	11/08/2010	11/05/2010
	From: Webcor Construction LP                      Joanne Filipas					
	REQUEST: Ref U-1108. U4000, H/4001 and attached.  1. Section H/U-4001 shows the (E)(6)4"E(D) just north of the (E)T(NR) however the plans show it north of the (E) SS. Please advise.				ANSWER: Ref U-1108. U4000, H/4001 and attached.  1. Section H/U-4001 shows the (E)(6)4"E(D) just north of the (E)T(NR) however the plans show it north of the (E) SS. Please advise.	



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U-0013	Water Connections at Howard	Closed	03	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST: Ref I-3120, U-3116, U-3112		ANSWER: Ref I-3120, U-3116, U-3112				
There is a discrepancy in the elevations called out for the 12" water line connections at Howard. The First and Howard connection shows the elevation at 13 on U-3120 and no elevation is provided on Howard. If we were to scale, the elevation should be at 14. Please provide the connection elevation.		There is a discrepancy in the elevations called out for the 12" water line connections at Howard. The First and Howard connection shows the elevation at 13 on U-3120 and no elevation is provided on Howard. If we were to scale, the elevation should be at 14. Please provide the connection elevation.				
U-0014	Size of Gas Line on First Street	Closed	03	10/25/2010	11/08/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST: Ref U-2003, U-2021 and attached.		ANSWER: Ref U-2003, U-2021 and attached.				
The HPG line on U-2003 is 4". The same gas line on U-2021 is shown as 2". What size is the gas line?		The HPG line on U-2003 is 4". The same gas line on U-2021 is shown as 2". What size is the gas line?				
U-0015	LEED Requirements for RUP work	Closed	03	10/26/2010	11/09/2010	11/05/2010
From: Webcor Construction LP                      Joanne Filipas						
REQUEST: RE: Specification 01-81-13 1.1.3B		ANSWER: RE: Specification 01-81-13 1.1.3B				
The specification section referenced provides a drawing which outlines the "LEED Project Limit". On this drawing, the limit line is drawn on Minna Street and Natoma Street and incorporates First Street, Fremont Street and Beale Street where they cross the new building. Is it the intent of this specification section that the RUP work in the areas enclosed are to be incorporated into the LEED program?		The specification section referenced provides a drawing which outlines the "LEED Project Limit". On this drawing, the limit line is drawn on Minna Street and Natoma Street and incorporates First Street, Fremont Street and Beale Street where they cross the new building. Is it the intent of this specification section that the RUP work in the areas enclosed are to be incorporated into the LEED program?				
U-0016	Street Light Relocation	Closed	03	11/02/2010	11/16/2010	11/17/2010
From: Webcor Construction LP                      Jeffrey Negley						
REQUEST: Plan/Drawing Reference: U-3201		ANSWER: Plan/Drawing Reference: U-3201				



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U-0017	<b>JT Conflict with Basement @ Rickenbacker Rest.</b>  <b>From:</b> Webcor Construction LP                      Jeffrey Negley  <b>REQUEST:</b>  Reference sheet U-3407. PG&E has been potholing on the south east corner of Minna @ 2nd St. for a new gas line over the past number of days. We have observed in their potholes that a basement structure for the Rickenbacker Restaurant (123 2nd St.) extends out beyond the property line and under the sidewalk, along both Minna and 2nd Street. The basement appears to extend almost up to the roadway curb on 2nd Street and to face of curb or beyond on Minna. The joint trench at its current alignment (on Plan Sheet U-3407) along the south east corner of 2nd & Minna will be in conflict with this basement structure. Please review and advise.	Closed	03	11/09/2010	11/23/2010	01/12/2011
						<b>ANSWER:</b>  Reference sheet U-3407. PG&E has been potholing on the south east corner of Minna @ 2nd St. for a new gas line over the past number of days. We have observed in their potholes that a basement structure for the Rickenbacker Restaurant (123 2nd St.) extends out beyond the property line and under the sidewalk, along both Minna and 2nd Street. The basement appears to extend almost up to the roadway curb on 2nd Street and to face of curb or beyond on Minna. The joint trench at its current alignment (on Plan Sheet U-3407) along the south east corner of 2nd & Minna will be in conflict with this basement structure. Please review and advise.



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U-0018	AWSS caps requirement	Closed	03	11/10/2010	11/10/2010	11/24/2010
From: Webcor Construction LP Jeffrey Negley						
REQUEST:		ANSWER:				
Please refer to sheets MA-5, MA-8, U-1120, U-1121, see attached.		Please refer to sheets MA-5, MA-8, U-1120, U-1121, see attached.				
Please confirm that the AWSS caps shown on sheets MA-5 and MA-8 are required prior to the installation of the new PG&E ductbank (sheet U-2021) on the East side of First St.		Please confirm that the AWSS caps shown on sheets MA-5 and MA-8 are required prior to the installation of the new PG&E ductbank (sheet U-2021) on the East side of First St.				
U-0018.1	AWSS Removal Work on First Street - Scope Clarification	Closed	03	11/22/2010	11/24/2010	11/24/2010
From: Webcor Construction LP Jeffrey Negley						
REQUEST:		ANSWER:				
The First Street AWSS cap issue has created a two part question. RFI #U-0018 will remain open to track the sequence of installation regarding installation of the AWSS cap and PG&E trench.		The First Street AWSS cap issue has created a two part question. RFI #U-0018 will remain open to track the sequence of installation regarding installation of the AWSS cap and PG&E trench.				
RFI #U-0018.1 addresses scope. Refer to sheets MA-5, MA-8, U-1120, U-1121, and Guy Hollins email attached.		RFI #U-0018.1 addresses scope. Refer to sheets MA-5, MA-8, U-1120, U-1121, and Guy Hollins email attached.				
Per conversations between Guy Hollins, Eric Zagol and Michael Smith (mechanical engineer with DPW Bureau of Engineering), please clarify the work involved to install the two AWSS caps on First & Howard and First & Mission St. Also produce a list of material required to complete the work. Provide drawing/ sketch if necessary to clarify scope of work.		Per conversations between Guy Hollins, Eric Zagol and Michael Smith (mechanical engineer with DPW Bureau of Engineering), please clarify the work involved to install the two AWSS caps on First & Howard and First & Mission St. Also produce a list of material required to complete the work. Provide drawing/ sketch if necessary to clarify scope of work.				
U-0019	Street Light Location	Closed	03	11/10/2010	11/12/2010	12/02/2010
From: Webcor Construction LP Jeffrey Negley						
REQUEST:		ANSWER:				
Please provide layout for the Street Lights shown to be relocated on sheets U-3201 and U-3202.		Please provide layout for the Street Lights shown to be relocated on sheets U-3201 and U-3202.				



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U-0019.1	Light Pole at Station 4+12.03: Reroute existing conduit	Closed	03	12/21/2010	12/31/2010	02/02/2011
From: Webcor Construction LP                      David Hungerford						
<b>REQUEST:</b>  Reference: RFI #U-0019, attached picture and sheet U-3201  The streetlight at station 4+12.03 was laid out per the response to RFI #U-0019. When the new location was potholed, a number of existing utilities were discovered. Per inspection with BLHP on 12/20/2010, inspector Robert Kawano requests to re-route existing conduits in the new light pole ftg. location at STN. 4+12.03. Utilities seem to be privately owned by 555 Mission St.. Please advise.						<b>ANSWER:</b>  Reference: RFI #U-0019, attached picture and sheet U-3201  The streetlight at station 4+12.03 was laid out per the response to RFI #U-0019. When the new location was potholed, a number of existing utilities were discovered. Per inspection with BLHP on 12/20/2010, inspector Robert Kawano requests to re-route existing conduits in the new light pole ftg. location at STN. 4+12.03. Utilities seem to be privately owned by 555 Mission St.. Please advise.
U-0019.2	Light Pole at Station 4+12.03: Reroute existing conduit	Closed	03	12/21/2010	12/31/2010	02/02/2011
From: Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>  Question from RFI #U-0019.1 ----- Reference: RFI #U-0019, attached picture and sheet U-3201  The streetlight at station 4+12.03 was laid out per the response to RFI #U-0019. When the new location was potholed, a number of existing utilities were discovered. Per inspection with BLHP on 12/20/2010, inspector Robert Kawano requests to re-route existing conduits in the new light pole ftg. location at STN. 4+12.03. Utilities seem to be privately owned by 555 Mission St.. Please advise.						<b>ANSWER:</b>  Question from RFI #U-0019.1 ----- Reference: RFI #U-0019, attached picture and sheet U-3201  The streetlight at station 4+12.03 was laid out per the response to RFI #U-0019. When the new location was potholed, a number of existing utilities were discovered. Per inspection with BLHP on 12/20/2010, inspector Robert Kawano requests to re-route existing conduits in the new light pole ftg. location at STN. 4+12.03. Utilities seem to be privately owned by 555 Mission St.. Please advise.
U-0020	Street Lighting Relocation Plan for Minna	Closed	03	11/15/2010	11/29/2010	11/18/2010
From: Webcor Construction LP                      Jeffrey Negley						
<b>REQUEST:</b>  Reference: Plan/Drawing Reference: U-3201  We have been informally advised that the Design Engineer and BLHP are considering a revised installation plan for the street lights on Minna. This would include the installation of temporary overhead power lines to feed the relocated street light poles, until such time as the new						<b>ANSWER:</b>  Reference: Plan/Drawing Reference: U-3201  We have been informally advised that the Design Engineer and BLHP are considering a revised installation plan for the street lights on Minna. This would include the installation of temporary overhead power lines to feed the relocated street light poles,



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	<p>lights are powered from underground by Trinet.</p> <p>Here is a sequence as Trinet understands it. Trinet would install the new light pole foundations on the north side of Minna and then relocate the light poles from the south side, per plans. BLHP would then install overhead cable, extending from a pole on 2nd St., to provide power for the lights. During installation of the new foundations, Trinet would install underground conduit from the pole to an adjacent splice box, and then later extend the underground conduit from the splice box to the PG&amp;E power source, as depicted on the plans.</p> <p>Please clarify the street lighting relocation plan currently under consideration. Also, if the BLHP plan to feed the lights temporarily from overhead, will any changes be required to the foundation and light pole installation plan to accommodate an overhead power feed?</p> <p>Please review and advise.</p>					<p>until such time as the new lights are powered from underground by Trinet.</p> <p>Here is a sequence as Trinet understands it. Trinet would install the new light pole foundations on the north side of Minna and then relocate the light poles from the south side, per plans. BLHP would then install overhead cable, extending from a pole on 2nd St., to provide power for the lights. During installation of the new foundations, Trinet would install underground conduit from the pole to an adjacent splice box, and then later extend the underground conduit from the splice box to the PG&amp;E power source, as depicted on the plans.</p> <p>Please clarify the street lighting relocation plan currently under consideration. Also, if the BLHP plan to feed the lights temporarily from overhead, will any changes be required to the foundation and light pole installation plan to accommodate an overhead power feed?</p> <p>Please review and advise.</p>
U-0021	M.H. #501 and existing utilities	Closed	03	11/17/2010	11/22/2010	12/02/2010
	<p>From: Webcor Construction LP                      Jeffrey Negley</p> <p><b>REQUEST:</b></p> <p>Reference Plan/Drawing: U-3021</p> <p>During potholing activities in 1st St. where Manhole #501 is to be installed Trinet has encountered a number of existing utilities which occupy the same intended space for Manhole #501. Please see the attached sketch for locations and clarifications of these utilities.</p> <p>Some of these utilities, particularly UT Group #2 and UT Group #5 (reference sketch) are intended to be disconnected by PG&amp;E by November 24th. Please confirm.</p> <p>UT Group #1, which appears to be owned by ATT is noted on the drawings as to be disconnected and demolished.</p>					<p><b>ANSWER:</b></p> <p>Reference Plan/Drawing: U-3021</p> <p>During potholing activities in 1st St. where Manhole #501 is to be installed Trinet has encountered a number of existing utilities which occupy the same intended space for Manhole #501. Please see the attached sketch for locations and clarifications of these utilities.</p> <p>Some of these utilities, particularly UT Group #2 and UT Group #5 (reference sketch) are intended to be disconnected by PG&amp;E by November 24th. Please confirm.</p> <p>UT Group #1, which appears to be owned by ATT is</p>



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U-0022	<p>Please advise as to when this utility is scheduled to be disconnected.</p> <p>UT Groups #3 and #4 are unidentified and were not included in the USA markings for this area. In order to construct M.H. #501 per the contract drawings these utilities must be removed or relocated. Please advise as to the ownership of these utilities and provide direction on how to proceed.</p> <p>Note: due to construction, we are requesting that this RFI be answered by 11/22/10 if possible.</p> <p><b>SFWD crossings at Minna St. and 1st</b></p> <p><b>From:</b> Webcor Construction LP                      Jeffrey Negley</p> <p><b>REQUEST:</b></p> <p>Reference Plan/Drawing: U-1002 and attached PDF.</p> <p>Current USA markings have identified (2) SFWD laterals which are not indicated on the drawings. These are located at approximately STA 8+59 and 9+06 and extend from the main in Minna St., North toward the building of 100 1st. ST. These laterals need to be identified and recorded in order to properly document and construct both the new water line and the new joint trench.</p> <p>Any additional work associated with these utilities may result in a cost or schedule impact. Please review and provide direction on how we should proceed.</p>	Closed	03	11/17/2010	12/01/2010	12/03/2010
	<p><b>ANSWER:</b></p> <p>Reference Plan/Drawing: U-1002 and attached PDF.</p> <p>Current USA markings have identified (2) SFWD laterals which are not indicated on the drawings. These are located at approximately STA 8+59 and 9+06 and extend from the main in Minna St., North toward the building of 100 1st. ST. These laterals need to be identified and recorded in order to properly document and construct both the new water line and the new joint trench.</p> <p>Any additional work associated with these utilities may result in a cost or schedule impact. Please review and provide direction on how we should proceed.</p>					









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U-0027	Unidentified Facility in First St Invest Trench - 18'-7 from Curb	Closed	03	12/03/2010	12/06/2010	12/07/2010
From: Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)  See attached plan and section through the investigative trench on the east side of First St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 18'-7" from face of curb, on the attachment and advise if it needs to be cut and capped.						<b>ANSWER:</b>  Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)  See attached plan and section through the investigative trench on the east side of First St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 18'-7" from face of curb, on the attachment and advise if it needs to be cut and capped.
U-0028	Unidentified Facility in First St Invest Trench - 14'-7 from Curb	Closed	03	12/03/2010	12/06/2010	12/07/2010
From: Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)  See attached plan and section through the investigative trench on the east side of First St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 14'-7" from face of curb, on the attachment and advise if it needs to be cut and capped.						<b>ANSWER:</b>  Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)  See attached plan and section through the investigative trench on the east side of First St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 14'-7" from face of curb, on the attachment and advise if it needs to be cut and capped.
U-0029	Unidentified Facility in First St Invest Trench - 13'-4" from Curb	Closed	03	12/03/2010	12/06/2010	12/07/2010
From: Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)  See attached plan and section through the investigative trench on the east side of First St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 13'-4" from face of curb, on the attachment and advise if it needs to be cut and capped.						<b>ANSWER:</b>  Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)  See attached plan and section through the investigative trench on the east side of First St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 13'-4" from face of curb, on the attachment and advise if it needs to be cut and capped.



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<b>U-0030</b>	<b>Unidentified Facility in First St Invest Trench - 9'-10" from Curb</b>	<b>Closed</b>	<b>03</b>	<b>12/03/2010</b>	<b>12/06/2010</b>	<b>12/10/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)		Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)				
See attached plan and section through the investigative trench on the east side of First St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 9'-10" from face of curb, on the attachment and advise if it needs to be cut and capped.		See attached plan and section through the investigative trench on the east side of First St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 9'-10" from face of curb, on the attachment and advise if it needs to be cut and capped.				
<b>U-0031</b>	<b>Unidentified Facility in First St Invest Trench - 7'-2" from Curb</b>	<b>Closed</b>	<b>03</b>	<b>12/03/2010</b>	<b>12/06/2010</b>	<b>12/07/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)		Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)				
See attached plan and section through the investigative trench on the east side of First St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 7'-2" from face of curb, on the attachment and advise if it needs to be cut and capped.		See attached plan and section through the investigative trench on the east side of First St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 7'-2" from face of curb, on the attachment and advise if it needs to be cut and capped.				
<b>U-0031.1</b>	<b>24in Concrete Wall in First St. Invest Trench - 7ft 2in from FOC</b>	<b>Closed</b>	<b>03</b>	<b>12/23/2010</b>	<b>01/02/2011</b>	<b>12/29/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Sheet U-1007, attached section and plan sketches, and attached documentation of notifications to USA North		Reference: Sheet U-1007, attached section and plan sketches, and attached documentation of notifications to USA North				
See the highlighted wall on attached plan and section through the investigative trench on the East side of First St.from Stn. 10+00 to 9+70. Per note 4 on sheet U-1007 Trinet requests direction regarding the unidentified 24" concrete wall found 7'-2" from the East face of curb and 10" cover that was encountered but not indicated on the contract plans.		See the highlighted wall on attached plan and section through the investigative trench on the East side of First St.from Stn. 10+00 to 9+70. Per note 4 on sheet U-1007 Trinet requests direction regarding the unidentified 24" concrete wall found 7'-2" from the East face of curb and 10" cover that was encountered but not indicated on the contract plans.				





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U-0032.1	Unidentified 18" Concrete Wall in First St Invest Trench - 3ft-2in from Curb	Closed	03	12/23/2010	01/02/2011	12/29/2010
From: Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: Sheet U-1007, attached section and plan sketches, and attached documentation of notifications to USA North  See the highlighted item on attached plan and section through the investigative trench on the East side of First St.from Station 10+00 to 9+70. Per note 4 on sheet U-1007, Trinet requests direction for the demolition of the 18" concrete wall found 3'-2" from the East face of curb and 17.5" covered that was encountered but not indicated on the contract plans.  Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/27/10.		<b>ANSWER:</b>  Reference: Sheet U-1007, attached section and plan sketches, and attached documentation of notifications to USA North  See the highlighted item on attached plan and section through the investigative trench on the East side of First St.from Station 10+00 to 9+70. Per note 4 on sheet U-1007, Trinet requests direction for the demolition of the 18" concrete wall found 3'-2" from the East face of curb and 17.5" covered that was encountered but not indicated on the contract plans.  Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/27/10.				
U-0033	Unidentified Facility in First St Invest Trench - 5'-8" from Curb	Closed	03	12/03/2010	12/06/2010	12/07/2010
From: Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)  See attached plan and section through the investigative trench on First St. at Minna St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 5'-8" from face of curb, on the attachment and advise if it needs to be cut and capped.		<b>ANSWER:</b>  Reference: Sheet U-1002 (dated 2010-10-01 - RUP Field Order)  See attached plan and section through the investigative trench on First St. at Minna St.. During Trinet's investigation, an unidentified utility/facility was encountered in the trench. Please identify the highlighted utility, located 5'-8" from face of curb, on the attachment and advise if it needs to be cut and capped.				
U-0033.1	Unidentified 2in Pipe in First St Invest Trench - 5ft-8in from Curb	Closed	03	12/23/2010	01/02/2011	12/29/2010
From: Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>  Reference: Sheet U-1007, attached section and plan sketches, and attached documentation of notifications to USA North  See attached plan and section through the investigative		<b>ANSWER:</b>  Reference: Sheet U-1007, attached section and plan sketches, and attached documentation of notifications to USA North  See attached plan and section through the				



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U-0034	<p>trench on the East side of First St.from Station 10+00 to 9+70. Per note 4 on sheet U-1007, Trinet requests direction for demolition of the unidentified 2" pipe found 5'-8" from the East face of curb and 15" covered that was encountered but not indicated on the contract plans.</p> <p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/27/10.</p> <p>Station 9+10 New Hydrant Conflict with Sidewalk Basement</p> <p>From: Webcor Construction LP                      David Hungerford</p> <p><b>REQUEST:</b></p> <p>Reference: Sheet U-3109 (dated 2010-09-29)</p> <p>During Trinet's potholing for the Joint trench along the North side of Minna St, a basemenet for building "100 First St." was revealed. The basement wall is located just behind the face of curb and extends to more than 8 feet below finish grade. The extent of the basement is unknown, but assumed to run the length of the "100 First St" property. The basement structure is in conflict with the proposed new fire hydrant installation at Station 9+10.</p> <p>Please provide layout for the fire hydrant.</p>	Closed	03	12/09/2010	12/20/2010	12/13/2010
						<p><b>ANSWER:</b></p> <p>Reference: Sheet U-3109 (dated 2010-09-29)</p> <p>During Trinet's potholing for the Joint trench along the North side of Minna St, a basemenet for building "100 First St." was revealed. The basement wall is located just behind the face of curb and extends to more than 8 feet below finish grade. The extent of the basement is unknown, but assumed to run the length of the "100 First St" property. The basement structure is in conflict with the proposed new fire hydrant installation at Station 9+10.</p> <p>Please provide layout for the fire hydrant.</p>









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<b>U-0037</b>	<b>Unidentified 2in Facility Encountered in Minna St. - 7in from FOC</b>	<b>Closed</b>	<b>03</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>12/30/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: Sheet U-1007, attached sketch of section from Trinet RFI 16 and Documentation of notification to USA North						Reference: Sheet U-1007, attached sketch of section from Trinet RFI 16 and Documentation of notification to USA North
See the attached section through the investigative trench at station 2 + 29.68 on Minna St. Per note 4 on sheet U - 1007, Trinet "hereby requests that Webcor "notify TJPB" of the unidentified 2" steel line found 7" from south face of curb and 2'-2" to cover. Per the same note, Trinet requests "direction on the demolition" of this line.						See the attached section through the investigative trench at station 2 + 29.68 on Minna St. Per note 4 on sheet U - 1007, Trinet "hereby requests that Webcor "notify TJPB" of the unidentified 2" steel line found 7" from south face of curb and 2'-2" to cover. Per the same note, Trinet requests "direction on the demolition" of this line.
Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.						Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.
<b>U-0038</b>	<b>Unidentified 4" Facility Encountered in Minna St. - 7ft 4in from FOC</b>	<b>Closed</b>	<b>03</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>12/16/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: Sheet U-1007, attached sketch of section from Trinet RFI 17 and documentation of notifications to USA North						Reference: Sheet U-1007, attached sketch of section from Trinet RFI 17 and documentation of notifications to USA North
See the attached section through the investigative trench at station 2 + 29.68 on Minna St. Per note 4 on sheet U - 1007, Trinet "hereby requests that Webcor "notify TJPB" of the unidentified 4" steel line found 7'-4" from north face of curb and 2'-11" to cover. Per the same note, Trinet requests "direction on the demolition" of this line.						See the attached section through the investigative trench at station 2 + 29.68 on Minna St. Per note 4 on sheet U - 1007, Trinet "hereby requests that Webcor "notify TJPB" of the unidentified 4" steel line found 7'-4" from north face of curb and 2'-11" to cover. Per the same note, Trinet requests "direction on the demolition" of this line.
Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.						Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.







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<b>U-0043</b>	<b>Fire Hydrant at St. 5+70 on Minna</b>	<b>Closed</b>	<b>03</b>	<b>12/13/2010</b>	<b>12/23/2010</b>	<b>12/14/2010</b>
<b>From:</b> Webcor Construction LP                      Mario Saldana Sr.						
<b>REQUEST:</b>						<b>ANSWER:</b>
INFORMATION NEEDED See the attached picture of the proposed fire hydrant location as indicated by drawings on Minna St. at Stn. 5+70. This location is in conflict with an existing driveway apron not shown on drawing # U-2008. Eric Zagol from AECOM is aware and has seen this issue in the field. NOTE - Due to the 8" water line currently being installed, the location for the "T" section oinstall could be as early as Tuesday the 14th. Please provide direction by 12-14-10 if possible.						INFORMATION NEEDED See the attached picture of the proposed fire hydrant location as indicated by drawings on Minna St. at Stn. 5+70. This location is in conflict with an existing driveway apron not shown on drawing # U-2008. Eric Zagol from AECOM is aware and has seen this issue in the field. NOTE - Due to the 8" water line currently being installed, the location for the "T" section oinstall could be as early as Tuesday the 14th. Please provide direction by 12-14-10 if possible.
We propose to move the fire hydrant location 6½ West to Stn. 5+64. Please advise.						We propose to move the fire hydrant location 6½ West to Stn. 5+64. Please advise.
<b>U-0044</b>	<b>Unidentified 4ft x 6.5ft Wall Encountered in Minna St. - 1ft from FOC</b>	<b>Closed</b>	<b>03</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>12/20/2010</b>
<b>From:</b> Webcor Construction LP                      David Hungerford						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference: Sheet U-1007, attached sketch of section from Trinet RFI 22 and documentation of notifications to USA North  See the attached section through the investigative trench at station 2 + 29.68 on Minna St. Per note 4 on sheet U-1007, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 4' x 6.5' wall (bottom was not found) at 1' from north face of curb and 18" to cover that Trinet encountered in the east wall of the trench. Per the same note, Trinet requests "direction on the demolition" of this structure.  Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.						Reference: Sheet U-1007, attached sketch of section from Trinet RFI 22 and documentation of notifications to USA North  See the attached section through the investigative trench at station 2 + 29.68 on Minna St. Per note 4 on sheet U-1007, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 4' x 6.5' wall (bottom was not found) at 1' from north face of curb and 18" to cover that Trinet encountered in the east wall of the trench. Per the same note, Trinet requests "direction on the demolition" of this structure.  Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.
<b>U-0045</b>	<b>Unidentified Concrete Wall Encountered in Minna St. - in line with FOC</b>	<b>Closed</b>	<b>03</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>12/29/2010</b>
<b>From:</b> Webcor Construction LP                      David Hungerford						
<b>REQUEST:</b>						<b>ANSWER:</b>





<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>U-0047</b>	<b>Unidentified 3in Pipe Encountered in Fremont St. - 5ft-8in from FOC</b>	<b>Closed</b>	<b>03</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>12/30/2010</b>
<div><div><b>From:</b> Webcor Construction LP      David Hungerford</div><div><b>REQUEST:</b> Reference: Sheet U-1008, attached sketch of section from Trinet RFI 25 and documentation of notifications to USA North  See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 3"steel pipe at 5'-8" from the east face of curb and 4'-3" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.  Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</div><div><b>ANSWER:</b> Reference: Sheet U-1008, attached sketch of section from Trinet RFI 25 and documentation of notifications to USA North  See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 3"steel pipe at 5'-8" from the east face of curb and 4'-3" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.  Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</div></div>						
<b>U-0048</b>	<b>Unidentified 3in Pipe Encountered in Fremont St. - 6ft-10in from FOC</b>	<b>Closed</b>	<b>03</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>12/30/2010</b>
<div><div><b>From:</b> Webcor Construction LP      David Hungerford</div><div><b>REQUEST:</b> Reference: Sheet U-1008, attached sketch of section from Trinet RFI 26 and documentation of notifications to USA North  See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 3" steel pipe at 6'-10" from the east face of curb and 18" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.  Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</div><div><b>ANSWER:</b> Reference: Sheet U-1008, attached sketch of section from Trinet RFI 26 and documentation of notifications to USA North  See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 3" steel pipe at 6'-10" from the east face of curb and 18" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.  Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</div></div>						
<b>U-0049</b>	<b>Unidentified 1in Pipe Encountered in Fremont St. - 6ft-10in from FOC</b>	<b>Closed</b>	<b>03</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>12/30/2010</b>





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<hr/>						
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>						
Reference: Sheet U-1008, attached sketch of section from Trinet RFI 27 and documentation of notifications to USA North						
See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 1" steel pipe at 6'-10" from the east face of curb and 4'-3" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.						
Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.						
<b>U-0050</b>	<b>Lower Sewer Laterals on Minna</b>	<b>Closed</b>	<b>03</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>01/11/2011</b>
<b>From:</b> Webcor Construction LP      Mario Saldana Sr.						
<b>REQUEST:</b>						
Reference: Sheets U-3007 & 3008, and Trinet RFI 41						
Two of the active sewer service laterals potholed on Minna St. are lower than the new sewer main and will not drain. The details of each issue are as follows: 1. Station 5+05 - Service for #2 Shaw Alley Top of pipe grade @ FOC for the 6" VCP sewer lateral is 11.37 . The invert elevation is approximately 10.8. The invert elevation of the new 24" sewer main @ Station 5+05 is approximately 11.4  2. Station 2+10 - Service for Anchor & Hope Restaurant Top of pipe grade @ FOC for the 6" VCP sewer lateral is 13.51. The invert elevation is approximately 12.94. The invert of the new 18" VCP sewer main @ Station 2+10 is approximately 13.4.  Please review these issues and advise. An expedited response is requested by 12/16/10.						
<b>ANSWER:</b>						
Reference: Sheets U-3007 & 3008, and Trinet RFI 41						
Two of the active sewer service laterals potholed on Minna St. are lower than the new sewer main and will not drain. The details of each issue are as follows: 1. Station 5+05 - Service for #2 Shaw Alley Top of pipe grade @ FOC for the 6" VCP sewer lateral is 11.37 . The invert elevation is approximately 10.8. The invert elevation of the new 24" sewer main @ Station 5+05 is approximately 11.4  2. Station 2+10 - Service for Anchor & Hope Restaurant Top of pipe grade @ FOC for the 6" VCP sewer lateral is 13.51. The invert elevation is approximately 12.94. The invert of the new 18" VCP sewer main @ Station 2+10 is approximately 13.4.  Please review these issues and advise. An expedited response is requested by 12/16/10.						









<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
U-0055	Unidentified 10in Pipe Encountered in Fremont St. - 14ft 3in from FOC	Closed	03	12/15/2010	12/25/2010	12/20/2010
<div><div><p><b>From:</b> Webcor Construction LP      David Hungerford</p><p><b>REQUEST:</b></p><p>Reference: Sheet U-1008, attached sketch of section from Trinet RFI 34 and documentation of notifications to USA North</p><p>See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 10" steel pipe at 14'-3" from the west face of curb and 2'-11" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.</p><p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</p></div><div><p><b>ANSWER:</b></p><p>Reference: Sheet U-1008, attached sketch of section from Trinet RFI 34 and documentation of notifications to USA North</p><p>See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 10" steel pipe at 14'-3" from the west face of curb and 2'-11" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.</p><p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</p></div></div>						
U-0056	Unidentified 4in Pipe Encountered in Fremont St. - 12ft 3in from FOC	Closed	03	12/15/2010	12/25/2010	12/29/2010
<div><div><p><b>From:</b> Webcor Construction LP      David Hungerford</p><p><b>REQUEST:</b></p><p>Reference: Sheet U-1008, attached sketch of section from Trinet RFI 35 and documentation of notifications to USA North</p><p>See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 4" steel pipe at 12'-3" from the west face of curb and 2' to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.</p><p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</p></div><div><p><b>ANSWER:</b></p><p>Reference: Sheet U-1008, attached sketch of section from Trinet RFI 35 and documentation of notifications to USA North</p><p>See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 4" steel pipe at 12'-3" from the west face of curb and 2' to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.</p><p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</p></div></div>						
U-0057	Unidentified 2.5in Pipes Encountered in Fremont St. - 4ft 10in from FOC	Closed	03	12/15/2010	12/25/2010	12/30/2010



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<div><div><p><b>From:</b> Webcor Construction LP      David Hungerford</p><p><b>REQUEST:</b></p><p>Reference: Sheet U-1008, attached sketch of section from Trinet RFI 36 and documentation of notifications to USA North</p><p>See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified pair of 2.5" steel pipes at 4'-10" from the west face of curb and 21" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.</p><p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</p></div><div><p><b>ANSWER:</b></p><p>Reference: Sheet U-1008, attached sketch of section from Trinet RFI 36 and documentation of notifications to USA North</p><p>See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified pair of 2.5" steel pipes at 4'-10" from the west face of curb and 21" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.</p><p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</p></div></div>						
<b>U-0058</b>	<b>Unidentified 4in Pipe Encountered in Fremont St. - 2ft from FOC</b>	<b>Closed</b>	<b>03</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>12/29/2010</b>
<div><div><p><b>From:</b> Webcor Construction LP      David Hungerford</p><p><b>REQUEST:</b></p><p>Reference: Sheet U-1008, attached sketch of section from Trinet RFI 37 and documentation of notifications to USA North</p><p>See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 4" steel pipe at 2' from the west face of curb and 15" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.</p><p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</p></div><div><p><b>ANSWER:</b></p><p>Reference: Sheet U-1008, attached sketch of section from Trinet RFI 37 and documentation of notifications to USA North</p><p>See the attached section through the investigative trench at station 4+40 on Fremont St. Per note 4 on sheet U-1008, Trinet hereby requests that Webcor "notify TJPA" of the unidentified 4" steel pipe at 2' from the west face of curb and 15" to cover that Trinet encountered in their trenching which was not indicated on the contract plan. Per the same note, Trinet requests "direction on the demolition" of this line.</p><p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/16/10.</p></div></div>						
<b>U-0059</b>	<b>Unidentified 6in Pipe Encountered in Fremont St. - in line with FOC</b>	<b>Closed</b>	<b>03</b>	<b>12/15/2010</b>	<b>12/25/2010</b>	<b>01/03/2011</b>





<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
U-0062	<p><b>REQUEST:</b></p> <p>Reference: Sheet U-3407</p> <p>Please provide drawing for the 8" water line and vertical / hydrant installation on Minna St. (reference RFI U-0017 response) west of Station 1+02. Please provide A.S.A.P. as field construction should be at this point by Tuesday pm.</p>	Closed	03	12/22/2010	01/01/2011	01/03/2011
	<p><b>ANSWER:</b></p> <p>Reference: Sheet U-3407</p> <p>Please provide drawing for the 8" water line and vertical / hydrant installation on Minna St. (reference RFI U-0017 response) west of Station 1+02. Please provide A.S.A.P. as field construction should be at this point by Tuesday pm.</p>					
U-0062	<p><b>Unidentified 8in Pipe Encountered in Fremont St. - 8ft 3in from FOC</b></p> <p><b>From:</b> Webcor Construction LP                      David Hungerford</p> <p><b>REQUEST:</b></p> <p>Reference: Sheet U-1008 (dated 2010.09.29) and attached sketch from Trinet</p> <p>See attached section through the investigative trench at Station 4+40 on Fremont St. Per note 4, on sheet U-1008 Trinet requests direction on an unidentified 8" steel pipe found 8'-3" from the East face of curb and 4'-4" to cover that was encountered but not indicated on the contract documents.</p> <p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this pipe by 12/27/10.</p>	Closed	03	12/22/2010	01/01/2011	01/03/2011
	<p><b>ANSWER:</b></p> <p>Reference: Sheet U-1008 (dated 2010.09.29) and attached sketch from Trinet</p> <p>See attached section through the investigative trench at Station 4+40 on Fremont St. Per note 4, on sheet U-1008 Trinet requests direction on an unidentified 8" steel pipe found 8'-3" from the East face of curb and 4'-4" to cover that was encountered but not indicated on the contract documents.</p> <p>Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this pipe by 12/27/10.</p>					



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>U-0063</b>	<b>Unmarked service lateral on Minna St. at Station 3+08</b>	<b>Closed</b>	<b>03</b>	<b>12/22/2010</b>	<b>01/01/2011</b>	<b>12/27/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Sheet U-3107 (dated 2010.09.29)			Reference: Sheet U-3107 (dated 2010.09.29)			
During excavation for the 8" water main along Minna Street, Trinet encountered a 1" Polyethylene service lateral at station 3+08, that extended into the vacant lot on the south side of the street. The service was broken during construction and Trinet has temporarily capped it. The utility was not shown on any utility plans. There is also no new service lateral, or reconnection of an existing, depicted on the new water main drawings at or adjacent to this location. Please advise on what should be done with the service. The repair is only temporary and a permanent reconnection will need to be performed by the SFWD if the service is to be maintained active. If the service is to be de-activated, then Trinet recommends that it be shut off at the connection to the old main.			During excavation for the 8" water main along Minna Street, Trinet encountered a 1" Polyethylene service lateral at station 3+08, that extended into the vacant lot on the south side of the street. The service was broken during construction and Trinet has temporarily capped it. The utility was not shown on any utility plans. There is also no new service lateral, or reconnection of an existing, depicted on the new water main drawings at or adjacent to this location. Please advise on what should be done with the service. The repair is only temporary and a permanent reconnection will need to be performed by the SFWD if the service is to be maintained active. If the service is to be de-activated, then Trinet recommends that it be shut off at the connection to the old main.			
<b>U-0064</b>	<b>Unidentified Facility in First St. Invest Trench - from Stn. 9+70 to 9+59.5</b>	<b>Closed</b>	<b>03</b>	<b>12/22/2010</b>	<b>01/01/2011</b>	<b>01/03/2011</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference: Sheet U-1007 and attached sketch of areas plan view			Reference: Sheet U-1007 and attached sketch of areas plan view			
See attached, plan views of the investigative trench on the East side of First St, West of the concrete MUNI median, from Stn. 9+70 to 9+59.5. Per note 4 on sheet U -1007, Trinet requests direction on the 4" Cardboard Pipe found 2'-0" West of the concrete MUNI median face of curb and 3'-6" to cover that was encountered but not indicated on the plans.			See attached, plan views of the investigative trench on the East side of First St, West of the concrete MUNI median, from Stn. 9+70 to 9+59.5. Per note 4 on sheet U -1007, Trinet requests direction on the 4" Cardboard Pipe found 2'-0" West of the concrete MUNI median face of curb and 3'-6" to cover that was encountered but not indicated on the plans.			
Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/23/10.			Trinet has this plated but would like to backfill the trench as soon as possible. An expedited response is requested with official direction on how to proceed with this facility by 12/23/10.			











# Webcor/Obayashi Joint Venture

## PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

### 30100 - Transbay Transit Center Project

Page: 2095 of 2218  
Date: 02/11/2015  
Time: 07:01 AM  
Job: 30100

<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>U-0068</b>	<b>Minna St Water Main Conflict w Abandoned Sewer MH</b>	<b>Closed</b>	<b>03</b>	<b>12/23/2010</b>	<b>01/02/2011</b>	<b>12/27/2010</b>
<b>From:</b> Webcor Construction LP      David Hungerford						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached drawings adn photos		See attached drawings adn photos				
During the water main installation on Minna St, Trinet encountered what appears to be an old abandoned sewer manhole in the trench line at station 1+15. the structure was not indicated on the drawings and was not discovered untill the pavement asphalt was removed. The sewer manhole is directly in conflict with the alignment of the new water main. The installation of the watermin cannot proceed furhter untill the manhole is demolished and/ or abandoned.		During the water main installation on Minna St, Trinet encountered what appears to be an old abandoned sewer manhole in the trench line at station 1+15. the structure was not indicated on the drawings and was not discovered untill the pavement asphalt was removed. The sewer manhole is directly in conflict with the alignment of the new water main. The installation of the watermin cannot proceed furhter untill the manhole is demolished and/ or abandoned.				
Per a field walk with Eric Zagol on 12/23/10, the existing MH was confirmed abandoned. Please confirm/ advise the top of the MH will be demolished to allow the installation of the waterline, and the MH will be backfilled with CDF.		Per a field walk with Eric Zagol on 12/23/10, the existing MH was confirmed abandoned. Please confirm/ advise the top of the MH will be demolished to allow the installation of the waterline, and the MH will be backfilled with CDF.				
****Please provide direction by 12/28/10.		****Please provide direction by 12/28/10.				
<b>U-0069</b>	<b>Street Light CCTV Camera-East Side of Fremont St. @ Stn. 5+45</b>	<b>Closed</b>	<b>03</b>	<b>01/05/2011</b>	<b>01/15/2011</b>	<b>01/14/2011</b>
<b>From:</b> Webcor Construction LP      Richard Buellesbach						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet U-3302 and Trinet RFI 62		Reference Sheet U-3302 and Trinet RFI 62				
During removal of the light pole arm on the east side of Fremont St. @ Stn. 5+45, Trinet observed that there is a CCTV camera and associated wiring on the light pole. Please advise of the plan for removal of CCTV camera.		During removal of the light pole arm on the east side of Fremont St. @ Stn. 5+45, Trinet observed that there is a CCTV camera and associated wiring on the light pole. Please advise of the plan for removal of CCTV camera.				
<b>U-0070</b>	<b>Subsurface Structures in Conflict with Minna St. AT&amp;T Vault</b>	<b>Closed</b>	<b>03</b>	<b>01/10/2011</b>	<b>01/20/2011</b>	<b>01/12/2011</b>
<b>From:</b> Webcor Construction LP      Jason Dunne						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet U-2008 and Trinet RFI 63		Reference Sheet U-2008 and Trinet RFI 63				
During our potholing on Minna St. for the proposed AT&T vault in the sidewalk (Stn. 3+72), we encountered an existing subsurface foundation and slurry shoring wall. The		During our potholing on Minna St. for the proposed AT&T vault in the sidewalk (Stn. 3+72), we encountered an existing subsurface foundation and				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
U-0071	<p>Existing fittings at tie in location for Minna St. 8 in. Water Main (Stn. 9+30)</p> <p><b>From:</b> Webcor Construction LP      Richard Buellesbach</p> <p><b>REQUEST:</b></p> <p>Reference Sheet U-3109 and Trinet RFI 64</p> <p>Due to the presence of existing fittings installed in the existing 8 inch water main at our tie in location (Stn. 9+30) at First St. and Minna St. for the new 8 inch water main on Minna St., SFWD inspector Dan Helmnik has requested to extend the limits of the tie in excavation beyond the locations of the existing fittings. This is beyond what would normally be required for a tie in of this nature. Existing conditions were reviewed in the field by W/O, Turner, SFWD, Eric Zagol from Aecom, and Trinet personnel.</p> <p>Please advise. An expedited response is requested.</p>	Closed	03	01/10/2011	01/20/2011	01/12/2011
						<p><b>ANSWER:</b></p> <p>Reference Sheet U-3109 and Trinet RFI 64</p> <p>Due to the presence of existing fittings installed in the existing 8 inch water main at our tie in location (Stn. 9+30) at First St. and Minna St. for the new 8 inch water main on Minna St., SFWD inspector Dan Helmnik has requested to extend the limits of the tie in excavation beyond the locations of the existing fittings. This is beyond what would normally be required for a tie in of this nature. Existing conditions were reviewed in the field by W/O, Turner, SFWD, Eric Zagol from Aecom, and Trinet personnel.</p> <p>Please advise. An expedited response is requested.</p>



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
U-0072	Fremont St traffic Signal Pole to be removed and salvaged - has Muni Cable attached	Closed	03	01/10/2011	01/20/2011	01/18/2011
From: Webcor Construction LP		David Hungerford				
REQUEST:			ANSWER:			
Reference Sheet U-3302 and Trinet RFI 65			Reference Sheet U-3302 and Trinet RFI 65			
Per contract, Trinet is required to remove and salvage the existing light pole indicated in the attached drawing. Through observation in the field, the existing light pole has a MUNI cable attached which runs to the intersection of Fremont St. and Mission St.. Based on these findings, should the light pole be removed as indicated? Mario Saldana from W/O was present when this item was observed and issue has been discussed with Eric Zagol from AECOM.			Per contract, Trinet is required to remove and salvage the existing light pole indicated in the attached drawing. Through observation in the field, the existing light pole has a MUNI cable attached which runs to the intersection of Fremont St. and Mission St.. Based on these findings, should the light pole be removed as indicated? Mario Saldana from W/O was present when this item was observed and issue has been discussed with Eric Zagol from AECOM.			
Please advise. An expedited response is requested by 01/12/2011.			Please advise. An expedited response is requested by 01/12/2011.			
U-0073	Fremont St. Light Pole and Muni Cables to be protected - indicated light pole has r	Closed	03	01/10/2011	01/20/2011	01/10/2011
From: Webcor Construction LP		David Hungerford				
REQUEST:			ANSWER:			
Reference Sheet U-3302 Traffic Signal E and Trinet RFI 66			Reference Sheet U-3302 Traffic Signal E and Trinet RFI 66			
As indicated on the plans, Trinet is required to "Remove and Salvage Traffic Signal Equipment. Protect Pole and Muni Cables in Place." Conditions were reviewed in the field and there is no Muni cable attached to the (E) light pole.			As indicated on the plans, Trinet is required to "Remove and Salvage Traffic Signal Equipment. Protect Pole and Muni Cables in Place." Conditions were reviewed in the field and there is no Muni cable attached to the (E) light pole.			
Mario Saldana from W/O has observed there is a CCTV cable attached to the pole not mentioned in Trinet RFI 66 and requests clarification on ownership and status of the CCTV line. This issue has been discussed with Eric Zagol from AECOM.			Mario Saldana from W/O has observed there is a CCTV cable attached to the pole not mentioned in Trinet RFI 66 and requests clarification on ownership and status of the CCTV line. This issue has been discussed with Eric Zagol from AECOM.			
Please advise. An expedited response is requested by 01/12/2011.			Please advise. An expedited response is requested by 01/12/2011.			
U-0074	Unidentified 9in Concrete Wall in First St Invest Trench - 10ft-5in west of Conc. Mu	Closed	03	01/10/2011	01/20/2011	01/25/2011
From: Webcor Construction LP		Jason Dunne				
REQUEST:			ANSWER:			



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Reference Sheet U-1007 Traffic Signal E and Trinet RFI 051	See attached, plan views of the investigative trench on the east side of First St., west of the concrete Muni median, from Stn. 9+70 to 9+59.5. Per note 4 of sheet U-1007, Trinet requests that Webcor "notify TJPA" of the unidentified 9" concrete wall at 10ft-5in west of the concrete Muni median face of curb and 3ft-6in cover that Trinet encountered "not indicated on plans". Per same note, Trinet requests "direction on the demolition" of this structure. Trinet has plated but would like to backfill the trench as soon as possible. Please advise.					
Reference Sheet U-1007 Traffic Signal E and Trinet RFI 051	See attached, plan views of the investigative trench on the east side of First St., west of the concrete Muni median, from Stn. 9+70 to 9+59.5. Per note 4 of sheet U-1007, Trinet requests that Webcor "notify TJPA" of the unidentified 9" concrete wall at 10ft-5in west of the concrete Muni median face of curb and 3ft-6in cover that Trinet encountered "not indicated on plans". Per same note, Trinet requests "direction on the demolition" of this structure. Trinet has plated but would like to backfill the trench as soon as possible. Please advise.					
<b>U-0075</b>	<b>Water Main Connection at 2nd St and Minna St - expose new line for SFWD</b>	<b>Closed</b>	<b>03</b>	<b>01/11/2011</b>	<b>01/21/2011</b>	<b>01/12/2011</b>
<b>From:</b> Webcor Construction LP Mario Saldana						
<b>REQUEST:</b>						
Reference Sheet U-3107 and attached photos						
At the intersection of 2nd St and Minna St, there is an existing 2in gas line running directly on top and next to the existing 8in main to be tied into. SFWD cannot make the Tee connection due to the bells of the fittings with the 2in gas line so close.						
The end of the new line installed by Trinet will need to be exposed about 2ft for SFWD to move the end of the line by 1ft east so that SFWD can make the connection without moving the gas line. This will require extra work for Trinet to expose the new line for SFWD. Eric Zangol from AECOM and Dan Helminiak from SFWD were present during the discussion of this issue.						
Please provide direction as soon as possible as this will impact the chlorination and tie-in schedule.						
<b>ANSWER:</b>						
Reference Sheet U-3107 and attached photos						
At the intersection of 2nd St and Minna St, there is an existing 2in gas line running directly on top and next to the existing 8in main to be tied into. SFWD cannot make the Tee connection due to the bells of the fittings with the 2in gas line so close.						
The end of the new line installed by Trinet will need to be exposed about 2ft for SFWD to move the end of the line by 1ft east so that SFWD can make the connection without moving the gas line. This will require extra work for Trinet to expose the new line for SFWD. Eric Zangol from AECOM and Dan Helminiak from SFWD were present during the discussion of this issue.						
Please provide direction as soon as possible as this will impact the chlorination and tie-in schedule.						
<b>U-0076</b>	<b>Water Main Connection at 2nd St and Minna St - demo/excavate per SFWD</b>	<b>Closed</b>	<b>03</b>	<b>01/11/2011</b>	<b>01/21/2011</b>	<b>01/14/2011</b>
<b>From:</b> Webcor Construction LP Mario Saldana						





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	<b>REQUEST:</b>  Reference Sheet U-2008  The existing 4" water service found at Stn. 5+37 has been confirmed abandoned by SFWD personnel through on site investigations. Since the service is determined to be inactive, Trinet intends to not provide service from the new main for this 4" service as discussed in the field, with Eric Zagol from AECOM, Mario Saldana from W/O, Dan Helminick from SFWD and Robert Friend from Trinet. In addition, Dan Helminick from SFWD requested to have the service tee installed in the new 8" main which was to provide service for this 4" lateral removed and straight pipe installed. Please confirm if this is acceptable.  The 6" water service lateral found at Stn. 5+30 has been confirmed as an active fire service to 2 Shaw Alley by SFWD personnel through on site investigations. Trinet intends to provide service from the new water main for this 6" service as discussed in the field with Eric Zangol from AECOM, Mario Saldana from W/O, Dan Helminiak from SFWD and Robert Friend from Trinet.  An expedited response is requested.					
	<b>ANSWER:</b>  Reference Sheet U-2008  The existing 4" water service found at Stn. 5+37 has been confirmed abandoned by SFWD personnel through on site investigations. Since the service is determined to be inactive, Trinet intends to not provide service from the new main for this 4" service as discussed in the field, with Eric Zagol from AECOM, Mario Saldana from W/O, Dan Helminick from SFWD and Robert Friend from Trinet. In addition, Dan Helminick from SFWD requested to have the service tee installed in the new 8" main which was to provide service for this 4" lateral removed and straight pipe installed. Please confirm if this is acceptable.  The 6" water service lateral found at Stn. 5+30 has been confirmed as an active fire service to 2 Shaw Alley by SFWD personnel through on site investigations. Trinet intends to provide service from the new water main for this 6" service as discussed in the field with Eric Zangol from AECOM, Mario Saldana from W/O, Dan Helminiak from SFWD and Robert Friend from Trinet.  An expedited response is requested.					
<b>U-0079</b>	<b>Fremont St Temp Water Line Installed over AT&amp;T Duct</b>	<b>Closed</b>	<b>03</b>	<b>01/17/2011</b>	<b>01/27/2011</b>	<b>01/19/2011</b>
	<b>From:</b> Webcor Construction LP      Nhi Tran					
	<b>REQUEST:</b>  Reference Sheet U-3123 and attached detail  During Trinet's installation of the temporary water line in Fremont St., Trinet encountered an existing AT&T duct that was in direct conflict with the temporary water line. Trinet was directed by Eugene Chu of SFWD/SFPUC to run the temporary water line over the existing AT&T duct using 45 degree bends. This resulted in less cover for the piping than what is required by the Water Department. Due to the lack of cover, Trinet was directed to install a 1/2in steel plate beneath the concrete base along the trench as depicted in the attached detail. The plate was approximately 2ft wide by 6ft long and extended to the limits of the installed 45 degree bends.					
	<b>ANSWER:</b>  Reference Sheet U-3123 and attached detail  During Trinet's installation of the temporary water line in Fremont St., Trinet encountered an existing AT&T duct that was in direct conflict with the temporary water line. Trinet was directed by Eugene Chu of SFWD/SFPUC to run the temporary water line over the existing AT&T duct using 45 degree bends. This resulted in less cover for the piping than what is required by the Water Department. Due to the lack of cover, Trinet was directed to install a 1/2in steel plate beneath the concrete base along the trench as depicted in the attached detail. The plate was approximately 2ft wide by 6ft long and extended to the					





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	Please provide confirmation that this is acceptable.				limits of the installed 45 degree bends. Please provide confirmation that this is acceptable.	
<b>U-0080</b>	<b>Proposed Design Change for MH #501</b>	<b>Closed</b>	<b>03</b>	<b>01/17/2011</b>	<b>01/27/2011</b>	<b>01/28/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>					<b>ANSWER:</b>	
Reference Sheet U-2021 and attached drawings					Reference Sheet U-2021 and attached drawings	
Trinet proposes to change the design of sewer manhole #501 from a Modified Box Manhole (per SF Standard Plan #87,184) to a Precast Concrete Manhole (per SF Standard Plan #87,181 - see attached drawing). The proposal includes the installation of a temporary 24" PVC pipe stub, extending south from the manhole and connected to the brick sewer per SF Standard Plan #87,197.					Trinet proposes to change the design of sewer manhole #501 from a Modified Box Manhole (per SF Standard Plan #87,184) to a Precast Concrete Manhole (per SF Standard Plan #87,181 - see attached drawing). The proposal includes the installation of a temporary 24" PVC pipe stub, extending south from the manhole and connected to the brick sewer per SF Standard Plan #87,197.	
The proposed manhole design will facilitate construction around the many utilities identified in the excavation - see RFI # U-0021 (Trinet RFI 04). It is also the preferred manhole design for 24in pipe per the SF Standard Drawings, especially since the brick sewer on the south side will later be abandoned and plugged (in the manhole) by the owner. This plan will also facilitate the later abandonment of the outlet to the south, as the owner will just have to plug the 24in outlet pipe and not a 3x5 brick sewer.					The proposed manhole design will facilitate construction around the many utilities identified in the excavation - see RFI # U-0021 (Trinet RFI 04). It is also the preferred manhole design for 24in pipe per the SF Standard Drawings, especially since the brick sewer on the south side will later be abandoned and plugged (in the manhole) by the owner. This plan will also facilitate the later abandonment of the outlet to the south, as the owner will just have to plug the 24in outlet pipe and not a 3x5 brick sewer.	
Please consider. An expedited response is requested.					Please consider. An expedited response is requested.	







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	<b>From:</b> Webcor Construction LP      Nhi Tran					
	<b>REQUEST:</b> Reference Specifications Section 33 31 10, 1.4.E  Please clarify if TJPA or DPW is going to complete the inspection of the pipe as described in the referenced specification section.					<b>ANSWER:</b> Reference Specifications Section 33 31 10, 1.4.E  Please clarify if TJPA or DPW is going to complete the inspection of the pipe as described in the referenced specification section.
<b>U-0083</b>	<b>Water Main Alignment on Howard at Beale</b>	<b>Closed</b>	<b>03</b>	<b>01/19/2011</b>	<b>01/29/2011</b>	<b>01/20/2011</b>
	<b>From:</b> Webcor Construction LP      Nhi Tran					
	<b>REQUEST:</b> Reference Sheet U-3118  Potholes on Beale Street at Sta 14+00, Sta 14+90 and Sta 16+25 reveal a 6in steel line that is unmarked and not shown on contract drawings. The line is 18ft south of the Howard St centerline. This is the proposed alignment for the new 12in water main. The pothole at Sta 14+00 also reveals a 3in steel conduit which is 16ft south of the Howard St centerline. Also there is a 6ft x 6ft wooden telecom duct bank that runs east to west on Howard Street at 15ft south of the Howard Street centerline. This location offers the closest window for the new 12in water line to the original alignment shown in the contract drawings.  This would require the removal of the wooden duct bank and the removal of the abandoned manhole shown on U-3118 (Sta 14+96 ĳ 15ft from Howard St centerline)  Please confirm the alignment of the new 12in water main.					<b>ANSWER:</b> Reference Sheet U-3118  Potholes on Beale Street at Sta 14+00, Sta 14+90 and Sta 16+25 reveal a 6in steel line that is unmarked and not shown on contract drawings. The line is 18ft south of the Howard St centerline. This is the proposed alignment for the new 12in water main. The pothole at Sta 14+00 also reveals a 3in steel conduit which is 16ft south of the Howard St centerline. Also there is a 6ft x 6ft wooden telecom duct bank that runs east to west on Howard Street at 15ft south of the Howard Street centerline. This location offers the closest window for the new 12in water line to the original alignment shown in the contract drawings.  This would require the removal of the wooden duct bank and the removal of the abandoned manhole shown on U-3118 (Sta 14+96 ĳ 15ft from Howard St centerline)  Please confirm the alignment of the new 12in water main.



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U-0083.1	Water Main Alignment on Howard at Beale	Closed	03	01/24/2011	02/03/2011	01/25/2011
From: Webcor Construction LP                      Nhi Tran						
REQUEST:		ANSWER:				
M Squared has confirmed that the wooden duct bank is a 6inch x 6 inch wooden duct bank and is abandoned.		M Squared has confirmed that the wooden duct bank is a 6inch x 6 inch wooden duct bank and is abandoned.				
Please direct M Squared on how to proceed.		Please direct M Squared on how to proceed.				
*****		*****				
Question from U-0083:		Question from U-0083:				
Reference Sheet U-3118		Reference Sheet U-3118				
Potholes on Beale Street at Sta 14+00, Sta 14+90 and Sta 16+25 reveal a 6in steel line that is unmarked and not shown on contract drawings. The line is 18ft south of the Howard St centerline. This is the proposed alignment for the new 12in water main. The pothole at Sta 14+00 also reveals a 3in steel conduit which is 16ft south of the Howard St centerline. Also there is a 6in x 6in wooden telecom duct bank that runs east to west on Howard Street at 15ft south of the Howard Street centerline. This location offers the closest window for the new 12in water line to the original alignment shown in the contract drawings.		Potholes on Beale Street at Sta 14+00, Sta 14+90 and Sta 16+25 reveal a 6in steel line that is unmarked and not shown on contract drawings. The line is 18ft south of the Howard St centerline. This is the proposed alignment for the new 12in water main. The pothole at Sta 14+00 also reveals a 3in steel conduit which is 16ft south of the Howard St centerline. Also there is a 6in x 6in wooden telecom duct bank that runs east to west on Howard Street at 15ft south of the Howard Street centerline. This location offers the closest window for the new 12in water line to the original alignment shown in the contract drawings.				
This would require the removal of the wooden duct bank and the removal of the abandoned manhole shown on U-3118 (Sta 14+96 15ft from Howard St centerline)		This would require the removal of the wooden duct bank and the removal of the abandoned manhole shown on U-3118 (Sta 14+96 15ft from Howard St centerline)				
Please confirm the alignment of the new 12in water main.		Please confirm the alignment of the new 12in water main.				



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U-0084	Water Main Alignment on Beale Street	Closed	03	01/21/2011	01/31/2011	01/25/2011
From: Webcor Construction LP Nhi Tran						
REQUEST:		ANSWER:				
Reference Sheet U-3124		Reference Sheet U-3124				
M Squared potholed at Sta 1+10 on Beale Street. We discovered that the 10in High pressure water line is 9ft-5in from the FOC. The existing 12in water line is 14ft-8in from the FOC. The 10in High Pressure line is closer to the FOC that shown on contract drawings. This now means that there is a larger window between the 10in high pressure water and the existing 12in water main.		M Squared potholed at Sta 1+10 on Beale Street. We discovered that the 10in High pressure water line is 9ft-5in from the FOC. The existing 12in water line is 14ft-8in from the FOC. The 10in High Pressure line is closer to the FOC that shown on contract drawings. This now means that there is a larger window between the 10in high pressure water and the existing 12in water main.				
M Squared would like to install the new 12in water line at 12ft-3in from center line of the pipe to the FOC. This would mean the new 12in water line would be outside the parking strip and the parking strip would stay in tact. SFWD would also prefer it outside the parking strip for maintenance purposes.		M Squared would like to install the new 12in water line at 12ft-3in from center line of the pipe to the FOC. This would mean the new 12in water line would be outside the parking strip and the parking strip would stay in tact. SFWD would also prefer it outside the parking strip for maintenance purposes.				
Please confirm that it is acceptable to install the new 12in water line at 12ft-3in from FOC, going from Sta 0+60 to Sta 1+90.		Please confirm that it is acceptable to install the new 12in water line at 12ft-3in from FOC, going from Sta 0+60 to Sta 1+90.				
U-0084.1	Water Main Alignment on Beale Street	Closed	03	02/18/2011	02/28/2011	02/24/2011
From: Webcor Construction LP Nhi Tran						
REQUEST:		ANSWER:				
Reference Sheet U-3124 and RFI #U-0084		Reference Sheet U-3124 and RFI #U-0084				
In response to the Engineer's questions, M Square has noted the following: - Yes, the dimensions provided are to centerline of the pipe - Depth to centerline of existing 10-inch AWSS is 72-inches		In response to the Engineer's questions, M Square has noted the following: - Yes, the dimensions provided are to centerline of the pipe - Depth to centerline of existing 10-inch AWSS is 72-inches				
U-0085	AT&T Duct Bank on Beale at STA 6+00	Closed	03	01/21/2011	01/31/2011	01/27/2011
From: Webcor Construction LP Nhi Tran						



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	<div><div>REQUEST:</div><div>Reference Sheet U-3125 and attached sketch</div><div>The existing 4no. 4in AT&amp;T lines on Beale Street at Sta 6+10 are not as shown on the contract drawings. See attached sketch. Contract drawings show the conduit crossing M Squared's trench for 6 or 7 feet, however the duct bank is in the trench for 37 feet due to the alignment and width of the duct bank. The conduits are covered with a 2 foot wide concrete cap and appear in the trench for the new 12in water main at Sta 6+12 before leaving the trench at Sta 5+75. M Squared cannot lay the pipe on top of the concrete cap as the pipe will not have the required coverage. Due to this M Squared is unable to install the new 12in water as shown. Juan with AT&amp;T advised that M Squared remove the concrete cap from the conduits to allow for excavation of this portion of trench. With the cap removed it is more likely that the pipe will have the necessary minimum coverage.  Please confirm that this is how M Squared is to proceed. An expedited reponse is requested.</div></div>					
	<div><div>ANSWER:</div><div>Reference Sheet U-3125 and attached sketch</div><div>The existing 4no. 4in AT&amp;T lines on Beale Street at Sta 6+10 are not as shown on the contract drawings. See attached sketch. Contract drawings show the conduit crossing M Squared's trench for 6 or 7 feet, however the duct bank is in the trench for 37 feet due to the alignment and width of the duct bank. The conduits are covered with a 2 foot wide concrete cap and appear in the trench for the new 12in water main at Sta 6+12 before leaving the trench at Sta 5+75. M Squared cannot lay the pipe on top of the concrete cap as the pipe will not have the required coverage. Due to this M Squared is unable to install the new 12in water as shown. Juan with AT&amp;T advised that M Squared remove the concrete cap from the conduits to allow for excavation of this portion of trench. With the cap removed it is more likely that the pipe will have the necessary minimum coverage.  Please confirm that this is how M Squared is to proceed. An expedited reponse is requested.</div></div>					
U-0086	Concrete Slab & Rail Ties at Howard STA 13+60	Closed	03	01/24/2011	02/03/2011	01/25/2011
	From: Webcor Construction LP      Nhi Tran					
	<div><div>REQUEST:</div><div>Reference Sheet U-3117 and attached sketch</div><div>M Squared potholed at Howard Sta 13+60. The pothole revealed a 15in thick concrete slab which is in conflict with the proposed alignment of the new 12in water line. M Squared broke out a cross section of the slab and found nothing in it. There was also nothing underneath the slab for 5.5 feet. The southern edge of the slab is 4 feet north of the Howard Street center line. M Squared also discovered 6inch x 8inch x 4foot-6inch wooden rail ties.  If M Squared has to remove the concrete slab to install the water line at the alignment shown there is a danger that the MFS (fiber optic) conduits will be damaged as these conduits sit on top of the slab.</div></div>					
	<div><div>ANSWER:</div><div>Reference Sheet U-3117 and attached sketch</div><div>M Squared potholed at Howard Sta 13+60. The pothole revealed a 15in thick concrete slab which is in conflict with the proposed alignment of the new 12in water line. M Squared broke out a cross section of the slab and found nothing in it. There was also nothing underneath the slab for 5.5 feet. The southern edge of the slab is 4 feet north of the Howard Street center line. M Squared also discovered 6inch x 8inch x 4foot-6inch wooden rail ties.  If M Squared has to remove the concrete slab to install the water line at the alignment shown there is a danger that the MFS (fiber optic) conduits will be</div></div>					



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	<p>Breaking off an 18in section of the concrete slab and also a section of the rail ties would allow M Squared to excavate and install the new water pipe, while keeping away from the MFS conduits and not damaging them. However this will be time consuming.</p> <p>An alternative option is to move the trench for the new 12in water pipe 18in south and just remove the wooden rail ties (as shown in sketch).</p> <p>Mario S. from W/O and Eric Z. from AECOM were present during the discussion of this issue with M Squared in the field.</p> <p>Please direct M Squared on how to proceed with the water line installation. An expedited response is requested</p>					<p>damaged as these conduits sit on top of the slab.</p> <p>Breaking off an 18in section of the concrete slab and also a section of the rail ties would allow M Squared to excavate and install the new water pipe, while keeping away from the MFS conduits and not damaging them. However this will be time consuming.</p> <p>An alternative option is to move the trench for the new 12in water pipe 18in south and just remove the wooden rail ties (as shown in sketch).</p> <p>Mario S. from W/O and Eric Z. from AECOM were present during the discussion of this issue with M Squared in the field.</p> <p>Please direct M Squared on how to proceed with the water line installation. An expedited response is requested</p>
<b>U-0086.1</b>	<b>Concrete Slab &amp; Rail Ties at Howard STA 13+60</b>	<b>Closed</b>	<b>03</b>	<b>02/03/2011</b>	<b>02/14/2011</b>	<b>02/04/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
<p>As discussed at the meeting on Friday, 01/28/2011 between Noel (M2), Eric (AECOM) and Mario (Webcor) - due to existing utilities and the presence of the concrete slab and rail ties found in the additional potholing that was requested (Ref. Response to RFI U-0086), the new 12in water main is to be installed 5ft from the northern FOC on Howard Street Sta 12+60 to Sta 9+50.</p> <p>Please confirm.</p>						<p>As discussed at the meeting on Friday, 01/28/2011 between Noel (M2), Eric (AECOM) and Mario (Webcor) - due to existing utilities and the presence of the concrete slab and rail ties found in the additional potholing that was requested (Ref. Response to RFI U-0086), the new 12in water main is to be installed 5ft from the northern FOC on Howard Street Sta 12+60 to Sta 9+50.</p> <p>Please confirm.</p>

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<b>U-0088</b>	<b>Minna St 18in Sewer Conflict with PG&amp;E MH#1355 at STA 1+77</b>	<b>Closed</b>	<b>03</b>	<b>01/28/2011</b>	<b>02/07/2011</b>	<b>03/24/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Reference Sheet U-2007 and attached drawings			Reference Sheet U-2007 and attached drawings			
During layout for the installation of the new 18in Sewer Main on Minna St., Trinet observed that the alignment of the 18in Sewer Main is in conflict with existing PG&E MH #1355 at STA 1+77.50, which is to remain in place. The center line of the new sewer main is 0.10ft north of the outside edge of the manhole wall, as depicted in the attached drawing. The north side wall of the manhole is constructed on top of the existing 3ft x 5ft brick sewer. The brick sewer structure extends approximately 16in into the vault along its entire length. The brick sewer therefore cannot be demolished without undermining the north wall of the electric vault. Eric Z. of AECOM was notified of this issue via phone call on 01/21/2011.			During layout for the installation of the new 18in Sewer Main on Minna St., Trinet observed that the alignment of the 18in Sewer Main is in conflict with existing PG&E MH #1355 at STA 1+77.50, which is to remain in place. The center line of the new sewer main is 0.10ft north of the outside edge of the manhole wall, as depicted in the attached drawing. The north side wall of the manhole is constructed on top of the existing 3ft x 5ft brick sewer. The brick sewer structure extends approximately 16in into the vault along its entire length. The brick sewer therefore cannot be demolished without undermining the north wall of the electric vault. Eric Z. of AECOM was notified of this issue via phone call on 01/21/2011.			
Please advise: 1. How should Trinet proceed with the installation of the new 18in VCP Sewer at this location? 2. How should Trinet proceed with the demolition of the existing 3ft x 5ft brick sewer?			Please advise: 1. How should Trinet proceed with the installation of the new 18in VCP Sewer at this location? 2. How should Trinet proceed with the demolition of the existing 3ft x 5ft brick sewer?			
<b>U-0089</b>	<b>TJPA/DPW Inspection of Materials</b>	<b>Closed</b>	<b>CR</b>	<b>01/31/2011</b>	<b>02/10/2011</b>	<b>02/02/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Bob Garcia						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Ref. response to RFI U-0082, specs 331100, 011600:			Ref. response to RFI U-0082, specs 331100, 011600:			
In response to RFI U-0082 stated "TJPA/DPW intends to inspect the material deliveries of each subcontractor..."			In response to RFI U-0082 stated "TJPA/DPW intends to inspect the material deliveries of each subcontractor..."			
Does the TJPA/DPW or Turner have an established material inspection protocol in place to allow W/O and the trade subcontractors to verify and document that the materials have been inspected by TJPA/DPW or Turner per the above referenced specifications?			Does the TJPA/DPW or Turner have an established material inspection protocol in place to allow W/O and the trade subcontractors to verify and document that the materials have been inspected by TJPA/DPW or Turner per the above referenced specifications?			





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<b>U-0090</b>	<b>46 Minna St 6in Fire Service Connection</b>	<b>Closed</b>	<b>03</b>	<b>02/01/2011</b>	<b>02/11/2011</b>	<b>02/03/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet U-3108 and attached sketch and photos		Reference Sheet U-3108 and attached sketch and photos				
The original plan for connection of the 6in Fire Service Lateral @ 46 Minna St. was to leave the existing 6in gate valve (which is located at FOC) in place and connect the new 6in fire line to the downstream side of the old valve (See attached photo and sketch). This plan was proposed by SFWD inspectors, Tom Farhnam and Dan Helminiak, at a field meeting on 12/28/10. On Friday 1/28/11 the SFWD, plumbers when taking measurements for the tie-in, proposed a different plan. They want to extend the new 6in fire line beyond the curb and into the basement, and connect to the homeowners fire line inside the basement (under the sidewalk).		The original plan for connection of the 6in Fire Service Lateral @ 46 Minna St. was to leave the existing 6in gate valve (which is located at FOC) in place and connect the new 6in fire line to the downstream side of the old valve (See attached photo and sketch). This plan was proposed by SFWD inspectors, Tom Farhnam and Dan Helminiak, at a field meeting on 12/28/10. On Friday 1/28/11 the SFWD, plumbers when taking measurements for the tie-in, proposed a different plan. They want to extend the new 6in fire line beyond the curb and into the basement, and connect to the homeowners fire line inside the basement (under the sidewalk).				
Note: This will require coordination with building owner to put a hole through their foundation. Layout and a detail would need to be provided for the wall penetration, as well a detail to plug the hole where the existing water line is entering the basement.		Note: This will require coordination with building owner to put a hole through their foundation. Layout and a detail would need to be provided for the wall penetration, as well a detail to plug the hole where the existing water line is entering the basement.				
Please provide direction on how to proceed.		Please provide direction on how to proceed.				
<b>U-0091</b>	<b>SSMH #301 Located in Crosswalk at Natoma STA 0+81.72</b>	<b>Closed</b>	<b>03</b>	<b>02/01/2011</b>	<b>02/11/2011</b>	<b>02/24/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet U-3010		Reference Sheet U-3010				
SSMH #301 is shown to be located in the crosswalk at Sta 0+81.72.		SSMH #301 is shown to be located in the crosswalk at Sta 0+81.72.				
Please confirm that it is to be located in the pedestrian crosswalk.		Please confirm that it is to be located in the pedestrian crosswalk.				
<b>U-0092</b>	<b>AWSS Schedule Restrictions</b>	<b>Closed</b>	<b>CR</b>	<b>02/02/2011</b>	<b>02/12/2011</b>	<b>02/10/2011</b>
<b>From:</b> Webcor Construction LP      Richard Buellbach						
<b>REQUEST:</b>		<b>ANSWER:</b>				



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	<p>Webcor/Obayashi has received Bid Addendum #1 for the TG04.2R bid. As part of this addendum, note number 8 under "General Notes" on sheet U-0008 is deleted. This note had previously placed a constraint on the AWSS construction schedule that the Mission Street work must be complete prior to cutting both the Beale Street and the 1st Street lines. It was acceptable to abandon one or the other prior to the Mission Street work but not both.</p> <p>Based on the deletion of this note, it is our understanding that there is no schedule constraint on any of the AWSS system modifications other than the cutting &amp; capping procedures at 1st Street and Beale Street which are required for construction of the TTC Building. Please confirm.</p>					<p>Webcor/Obayashi has received Bid Addendum #1 for the TG04.2R bid. As part of this addendum, note number 8 under "General Notes" on sheet U-0008 is deleted. This note had previously placed a constraint on the AWSS construction schedule that the Mission Street work must be complete prior to cutting both the Beale Street and the 1st Street lines. It was acceptable to abandon one or the other prior to the Mission Street work but not both.</p> <p>Based on the deletion of this note, it is our understanding that there is no schedule constraint on any of the AWSS system modifications other than the cutting &amp; capping procedures at 1st Street and Beale Street which are required for construction of the TTC Building. Please confirm.</p>
<b>U-0093</b>	<b>46 Minna 6in FS Water &amp; 1in Copper Water Service Lateral at STA 5+17 Tie-In</b>	<b>Closed</b>	<b>03</b>	<b>02/03/2011</b>	<b>02/13/2011</b>	<b>02/07/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						<b>ANSWER:</b>
Reference Sheet U-3108, attached sketches, and material information sheets						Reference Sheet U-3108, attached sketches, and material information sheets
At 11:30am on 2/2/2011, Michelle Smith (Turner), Eric Zagol (AECOM), Guy Hollins (TJPA), Rick Bowling (46 Minna Property Manager), Dan Helminiak (SFWD Inspector), SFWD water department crew, Robert Friend (Trinet), Jason Dunne (Webcor Obayashi), and Mario Saldana (Webcor Obayashi) met to discuss the 6in Fire Service Lateral and 1in Water Service Lateral for the 46 Minna building.						At 11:30am on 2/2/2011, Michelle Smith (Turner), Eric Zagol (AECOM), Guy Hollins (TJPA), Rick Bowling (46 Minna Property Manager), Dan Helminiak (SFWD Inspector), SFWD water department crew, Robert Friend (Trinet), Jason Dunne (Webcor Obayashi), and Mario Saldana (Webcor Obayashi) met to discuss the 6in Fire Service Lateral and 1in Water Service Lateral for the 46 Minna building.
SFWD has proposed the new tie-in pipe configuration. 1. New 6in Fire Service Lateral Tie-in at 46 Minna St (See Attachment A) - Old existing fire service lateral is to be cut out of the existing water main up to the gate valve as shown in the sketch, and replaced with straight pipe. A new 10in hole is to be core drilled into the existing basement wall 22in east of the existing service lateral to incorporate the new 6in fire service lateral. SFWD will run the new 6in fire service lateral through the hole and Trinet is to provide Link Seals (see attached material information sheets) to seal the						SFWD has proposed the new tie-in pipe configuration. 1. New 6in Fire Service Lateral Tie-in at 46 Minna St (See Attachment A) - Old existing fire service lateral is to be cut out of the existing water main up to the gate valve as shown in the sketch, and replaced with straight pipe. A new 10in hole is to be core drilled into the existing basement wall 22in east of the existing service lateral to incorporate the new 6in fire service lateral. SFWD will run the new 6in fire service lateral through the hole and Trinet is to provide Link Seals (see attached





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<b>U-0094</b>	<b>Joint Trench Alignment Conflict With (E) Steam MH at Minna St. STA 0+85</b>	<b>Closed</b>	<b>03</b>	<b>02/03/2011</b>	<b>02/13/2011</b>	<b>02/04/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b> Reference Sheet U-3107 revised 12/27/10  The revised drawings show the Joint Trench alignment crossing through an existing old steam MH (Sta 0+85). The vault is a very large structure and extends to the north face of the curb of Minna St. Trinet believes that this vault is an abandoned structure.  Trinet requests direction for abandonment and/or demolition of this structure.		<b>ANSWER:</b> Reference Sheet U-3107 revised 12/27/10  The revised drawings show the Joint Trench alignment crossing through an existing old steam MH (Sta 0+85). The vault is a very large structure and extends to the north face of the curb of Minna St. Trinet believes that this vault is an abandoned structure.  Trinet requests direction for abandonment and/or demolition of this structure.				
<b>U-0095</b>	<b>Utility Company Contacts</b>	<b>Closed</b>	<b>03</b>	<b>02/03/2011</b>	<b>02/13/2011</b>	<b>02/04/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b> Reference Sheet U-0002 General Notes - Existing Utilities  Sheet U-0002 - EXISTING UTILITIES lists several phone numbers for contacting various utility companies in the city. M Squared has tried to contact most of these numbers and each one has had either no answer or is currently not in service.  M Squared requests a list of active phone numbers for the utility companies listed. An expedited response is necessary due to utility conflicts.		<b>ANSWER:</b> Reference Sheet U-0002 General Notes - Existing Utilities  Sheet U-0002 - EXISTING UTILITIES lists several phone numbers for contacting various utility companies in the city. M Squared has tried to contact most of these numbers and each one has had either no answer or is currently not in service.  M Squared requests a list of active phone numbers for the utility companies listed. An expedited response is necessary due to utility conflicts.				
<b>U-0096</b>	<b>PG&amp;E Conflict with Sewer Installation at Natoma STA 9+50</b>	<b>Closed</b>	<b>03</b>	<b>02/09/2011</b>	<b>02/19/2011</b>	<b>02/14/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b> Reference Sheet U-3012 and attached drawing  On 02/07/2011, M Squared encountered what appeared to be a live PG&E duct bank during their sewer installation excavation on Natoma Street STA 9+50. Due to this conflict, M Squared was unable to continue excavating for the sewer (See attachment). On 02/09/2011, M Squared's Superintendant met with a PG&E Representative and PG&E Representative confirmed that the duct bank is live		<b>ANSWER:</b> Reference Sheet U-3012 and attached drawing  On 02/07/2011, M Squared encountered what appeared to be a live PG&E duct bank during their sewer installation excavation on Natoma Street STA 9+50. Due to this conflict, M Squared was unable to continue excavating for the sewer (See attachment). On 02/09/2011, M Squared's Superintendant met with a PG&E Representative and PG&E Representative				



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	<p>and is not due to be decommissioned for at least 3 months.</p> <p>In order for M Squared to continue with the sewer installation, M Squared is proposing to:</p> <ul style="list-style-type: none"><li>- install MH #305 and begin installing pipe west of MH #305</li><li>- perform a temporary connection from MH#305 to the existing 3' x 5' brick sewer</li></ul> <p>M Squared can then perform the remainder of the work once PG&amp;E has decommissioned the duct bank.</p> <p>M Squared estimates that the additional cost to perform the temporary tie-in would be approximately \$4,500.</p> <p>Please confirm how you would like M Squared to proceed. M Squared requests an expedited response as they are currently stopped work and awaiting a response.</p>					
	<p>confirmed that the duct bank is live and is not due to be decommissioned for at least 3 months.</p> <p>In order for M Squared to continue with the sewer installation, M Squared is proposing to:</p> <ul style="list-style-type: none"><li>- install MH #305 and begin installing pipe west of MH #305</li><li>- perform a temporary connection from MH#305 to the existing 3' x 5' brick sewer</li></ul> <p>M Squared can then perform the remainder of the work once PG&amp;E has decommissioned the duct bank.</p> <p>M Squared estimates that the additional cost to perform the temporary tie-in would be approximately \$4,500.</p> <p>Please confirm how you would like M Squared to proceed. M Squared requests an expedited response as they are currently stopped work and awaiting a response.</p>					
U-0096.1	PGE Conflict with Sewer on Natoma at First Workaround	Closed	03	02/15/2011	02/25/2011	02/18/2011
	From: Webcor Construction LP                      Nhi Tran					
	REQUEST:					
	Reference U-3012 and attached sketch					
	Per response to RFI#U-0096, M Squared has provided the attached connection detail.					
	Please confirm if it is acceptable to proceed					
	ANSWER:					
	Reference U-3012 and attached sketch					
	Per response to RFI#U-0096, M Squared has provided the attached connection detail.					
	Please confirm if it is acceptable to proceed					



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U-0097	PG&E Conflict with Sewer Instll on Natoma at First	Closed	03	02/10/2011	02/20/2011	02/14/2011
From: Webcor Construction LP Nhi Tran						
REQUEST:		ANSWER:				
Reference Sheet U-3012		Reference Sheet U-3012				
Following on from M Squared's RFI #U-0096, M Squared has confirmed in the field that there is a grade conflict between the proposed sewer and the existing electrical duct bank on Natoma between STA 9+30 to 9+50. The conflict is between the bottom of the electrical duct bank and the top of the new 24" sewer pipe.		Following on from M Squared's RFI #U-0096, M Squared has confirmed in the field that there is a grade conflict between the proposed sewer and the existing electrical duct bank on Natoma between STA 9+30 to 9+50. The conflict is between the bottom of the electrical duct bank and the top of the new 24" sewer pipe.				
The elevation of bottom of electrical duct bank is 11.5' The top of the 24" VCP sewer is 11.82'		The elevation of bottom of electrical duct bank is 11.5' The top of the 24" VCP sewer is 11.82'				
M Squared has also confirmed with PG&E that 3 of the 4 concrete encased conduits are occupied, 2 being occupied by 12KV lines. The duct bank is to be abandoned in the future but PG&E was unable to provide a schedule for this work.		M Squared has also confirmed with PG&E that 3 of the 4 concrete encased conduits are occupied, 2 being occupied by 12KV lines. The duct bank is to be abandoned in the future but PG&E was unable to provide a schedule for this work.				
Please advise M Squared on how to proceed.		Please advise M Squared on how to proceed.				
U-0098	Potholing at Blackrock	Closed	03	02/10/2011	02/20/2011	02/10/2011
From: Webcor Construction LP Nhi Tran						
REQUEST:		ANSWER:				
M Squared is planning to pothole next week at Howard STA 9+40, First St STA 1+50 and First St STA 2+10 to confirm the alignment and depths of the new 12" water main on First St. from Howard to Natoma.		M Squared is planning to pothole next week at Howard STA 9+40, First St STA 1+50 and First St STA 2+10 to confirm the alignment and depths of the new 12" water main on First St. from Howard to Natoma.				
Guy Hollins from TJPA has advised M Squared that Blackrock is requesting additional potholing in the off-hours to determine locations of AT&T facilities in the area.		Guy Hollins from TJPA has advised M Squared that Blackrock is requesting additional potholing in the off-hours to determine locations of AT&T facilities in the area.				
Please provide M Squared information regarding the locations of the additional potholes requested, including the requested depths and sizes.		Please provide M Squared information regarding the locations of the additional potholes requested, including the requested depths and sizes.				
U-0099	Returned Submittal Comments	Closed	03	02/16/2011	02/26/2011	03/11/2011
From: Webcor Construction LP David Hungerford						







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U-0102	<p>Reference Sheet U-3021, attached sketch, and USA ticket</p> <p>During excavation for CB#501, Trinet encountered what appears to be a PG&amp;E vault (shown in plans as EMH 7712), PG&amp;E Duct (Shown in plans as 1- 2" &amp; 4-6" EP), 2- 2" steel conduits (not shown in plans), and a concrete shoring wall (not shown in plans).</p> <p>- The 2-2" steel pipe is in conflict with Trinet's installation of CB#501, and will need to be relocated or abandoned to facilitate the installation of the catch basin. Trinet has done their due diligence (2nd and 3rd No Response follow ups) and these lines were not marked by the owner through USA (attached). Trinet requests direction on the relocation/abandonment of these utilities.</p> <p>- Trinet proposes to move CB#501 two-feet north to avoid the conflict with the existing EMH 7712. Please advise if this is acceptable.</p> <p><b>First St. CB#206 in Conflict with (E) Subsurface Conc. Structure / Duct Bank</b></p> <p><b>From:</b> Webcor Construction LP                      Nhi Tran</p> <p><b>REQUEST:</b></p> <p>Reference Sheet U-3009 and attached sketch and photo</p> <p>During Trinet's excavation for replacement of CB#206 on the northwest corner of First St. and Minna St. (at STA 9+31), they encountered a concrete subsurface structure or concrete encased duct bank not indicated on the contract drawings. The existing catch basin is approximately 30in deep and is constructed on top of the existing concrete structure/duct bank (see attached drawing).</p> <p>Trinet requests direction on the demolition of the existing catch basin and the installation of the new catch basin CB#206.</p>	Closed				
	<p>Reference Sheet U-3021, attached sketch, and USA ticket</p> <p>During excavation for CB#501, Trinet encountered what appears to be a PG&amp;E vault (shown in plans as EMH 7712), PG&amp;E Duct (Shown in plans as 1- 2" &amp; 4- 6" EP), 2-2" steel conduits (not shown in plans), and a concrete shoring wall (not shown in plans).</p> <p>- The 2-2" steel pipe is in conflict with Trinet's installation of CB#501, and will need to be relocated or abandoned to facilitate the installation of the catch basin. Trinet has done their due diligence (2nd and 3rd No Response follow ups) and these lines were not marked by the owner through USA (attached). Trinet requests direction on the relocation/abandonment of these utilities.</p> <p>- Trinet proposes to move CB#501 two-feet north to avoid the conflict with the existing EMH 7712. Please advise if this is acceptable.</p>					
	<p><b>ANSWER:</b></p> <p>Reference Sheet U-3009 and attached sketch and photo</p> <p>During Trinet's excavation for replacement of CB#206 on the northwest corner of First St. and Minna St. (at STA 9+31), they encountered a concrete subsurface structure or concrete encased duct bank not indicated on the contract drawings. The existing catch basin is approximately 30in deep and is constructed on top of the existing concrete structure/duct bank (see attached drawing).</p> <p>Trinet requests direction on the demolition of the existing catch basin and the installation of the new catch basin CB#206.</p>					





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<b>U-0102.1</b>	<b>Catch Basin #206 redesign</b>	<b>Closed</b>	<b>03</b>	<b>04/01/2011</b>	<b>04/11/2011</b>	<b>04/13/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please clarify the following items relating to the re-design of CB#206:		Please clarify the following items relating to the re-design of CB#206:				
1) The only specification section addressing mortar coating is in 33 31 10 Paragraph 2.1.I, which specifies a "Wet Spray Mortar" application. This process would be cost prohibitive for coating only one catch basin. Trinet proposes the use of "SikaTop 123 Plus" mortar - product data sheets are attached. Please advise if this product is acceptable or specify an alternate material.		1) The only specification section addressing mortar coating is in 33 31 10 Paragraph 2.1.I, which specifies a "Wet Spray Mortar" application. This process would be cost prohibitive for coating only one catch basin. Trinet proposes the use of "SikaTop 123 Plus" mortar - product data sheets are attached. Please advise if this product is acceptable or specify an alternate material.				
2) The RFI response directs Trinet to use ductile iron pipe for culvert runs with less than 3' of cover. If 22.5% DI bends are required to construct the culverts Trinet would prefer to use Mechanical Joint Fittings. Please advise if these are acceptable.		2) The RFI response directs Trinet to use ductile iron pipe for culvert runs with less than 3' of cover. If 22.5% DI bends are required to construct the culverts Trinet would prefer to use Mechanical Joint Fittings. Please advise if these are acceptable.				
<b>U-0103</b>	<b>Natoma St. 4in Water Line Conflict with MH#306</b>	<b>Closed</b>	<b>03</b>	<b>02/24/2011</b>	<b>03/07/2011</b>	<b>02/24/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet U-1113 and U-3113		Reference Sheet U-1113 and U-3113				
A 4-inch water line runs from east to west on the south side of Natoma from Sta 9+40 to Sta 10+95. At Sta 10+95, the 4-in water line 90degrees into the building at 400 Howard St. This building however, appears to be fed from the existing 8-inch line on 1st St between Howard and Natoma.		A 4-inch water line runs from east to west on the south side of Natoma from Sta 9+40 to Sta 10+95. At Sta 10+95, the 4-in water line 90degrees into the building at 400 Howard St. This building however, appears to be fed from the existing 8-inch line on 1st St between Howard and Natoma.				
Is this 4-inch water lateral at Sta 10+95 on Natoma already abandoned? If not, can M Squared abandon it? It is currently in conflict with the proposed location of MH#306, and is also in conflict with the excavation and shoring for the new 30-inch sewer along Natoma (TG04.1).		Is this 4-inch water lateral at Sta 10+95 on Natoma already abandoned? If not, can M Squared abandon it? It is currently in conflict with the proposed location of MH#306, and is also in conflict with the excavation and shoring for the new 30-inch sewer along Natoma (TG04.1).				
<b>U-0104</b>	<b>Natoma St. Temporary Sewer Connections at Sta 9+25 and Sta 7+20</b>	<b>Closed</b>	<b>03</b>	<b>02/24/2011</b>	<b>03/06/2011</b>	<b>03/01/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						



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	<p><b>REQUEST:</b></p> <p>Reference Sheets U-1112, U-1120, U-3012, and RFI#U-0096</p> <p>In order for M Squared to install the new water main on Natoma Street between Sta 6+40 to Sta 10+00, the existing 3'x5' sewer must first be demolished. The 3'x5' sewer cannot be demolished until the new 24-inch VCP has been installed and connected to the existing sewer on First Street at Sta 9+59. Per sheets U-1112 and U-1120, the new 24-inch sewer is to be constructed after the demolition of the PG&amp;E ducts. However, demolition of the PG&amp;E ducts cannot be completed because PG&amp;E has not completed their relocation work</p> <p>Per RFI#U-0096 (M Squared RFI #009), as confirmed by PG&amp;E in the field on 02/09/2011, there is a live PG&amp;E duct bank in conflict with MH#305 and the new 24-inch VCP between MH#305 and MH#306, and not due to be decommissioned for at least three months.</p> <p>M Squared proposes to install a 12-inch HDPE pipe from Sta 9+25 to Sta 9+59, and perform a temporary connection to the existing 3'x5' sewer on First Street. Surveys carried out on the electric duct bank at Sta 9+30 on 02/08/11 shows that the bottom of the Duct Bank is approx. 10.8, meaning a 12-inch pipe will fit. In addition, M Squared proposes to perform a temporary connection (also 12-inch HDPE) at Sta 7+20 from the new MH#303 to the existing 3'x5' sewer. This would allow M Squared to demolish the 3'x5' sewer from Sta 7+02 to Sta 9+59, and allow M Squared to install the water from Sta 6+40 to Sta 10+00.</p> <p>M Squared estimates the cost for both of these connections is \$20,000.</p> <p>An expedited response is required to avoid impact to the installation of the water line</p>					
	<p><b>ANSWER:</b></p> <p>Reference Sheets U-1112, U-1120, U-3012, and RFI#U-0096</p> <p>In order for M Squared to install the new water main on Natoma Street between Sta 6+40 to Sta 10+00, the existing 3'x5' sewer must first be demolished. The 3'x5' sewer cannot be demolished until the new 24-inch VCP has been installed and connected to the existing sewer on First Street at Sta 9+59. Per sheets U-1112 and U-1120, the new 24-inch sewer is to be constructed after the demolition of the PG&amp;E ducts. However, demolition of the PG&amp;E ducts cannot be completed because PG&amp;E has not completed their relocation work</p> <p>Per RFI#U-0096 (M Squared RFI #009), as confirmed by PG&amp;E in the field on 02/09/2011, there is a live PG&amp;E duct bank in conflict with MH#305 and the new 24-inch VCP between MH#305 and MH#306, and not due to be decommissioned for at least three months.</p> <p>M Squared proposes to install a 12-inch HDPE pipe from Sta 9+25 to Sta 9+59, and perform a temporary connection to the existing 3'x5' sewer on First Street. Surveys carried out on the electric duct bank at Sta 9+30 on 02/08/11 shows that the bottom of the Duct Bank is approx. 10.8, meaning a 12-inch pipe will fit. In addition, M Squared proposes to perform a temporary connection (also 12-inch HDPE) at Sta 7+20 from the new MH#303 to the existing 3'x5' sewer. This would allow M Squared to demolish the 3'x5' sewer from Sta 7+02 to Sta 9+59, and allow M Squared to install the water from Sta 6+40 to Sta 10+00.</p> <p>M Squared estimates the cost for both of these connections is \$20,000.</p> <p>An expedited response is required to avoid impact to the installation of the water line</p>					
U-0105	Natoma St Duct Bank Conflict at Sta 12+92	Closed	03	02/24/2011	03/06/2011	03/01/2011
From: Webcor Construction LP      Nhi Tran						
	<p><b>REQUEST:</b></p>					
	<p><b>ANSWER:</b></p>					





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	<p>run is unaffected by this change. Trinet will adjust rebar as required to maintain the required spacing and clearances.</p> <p>Please confirm if the adjustment of MH#502 is acceptable.</p>					<p>new 24-inch VCP run is unaffected by this change. Trinet will adjust rebar as required to maintain the required spacing and clearances.</p> <p>Please confirm if the adjustment of MH#502 is acceptable.</p>
<b>U-0107</b>	<b>AWSS Cap Permit Requirements</b>	<b>Closed</b>	<b>03</b>	<b>02/25/2011</b>	<b>03/07/2011</b>	<b>02/28/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b> <p>W/O would like to confirm that other than any standard permits required for any excavation in the city of San Francisco, there is no additional permit required by any city agency in order to perform work on the AWSS caps.</p>						<b>ANSWER:</b> <p>W/O would like to confirm that other than any standard permits required for any excavation in the city of San Francisco, there is no additional permit required by any city agency in order to perform work on the AWSS caps.</p>
<b>U-0108</b>	<b>FH Relocation on Beale St</b>	<b>Closed</b>	<b>03</b>	<b>02/25/2011</b>	<b>03/07/2011</b>	<b>02/28/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b> <p>Reference sheet U-3124 and attached photo</p> <p>See the photo attached. The proposed location for the FH on Beale St at ~Sta 2+20 is in between a driveway for a parking garage and a driveway for a loading dock. Per discussions with Eric Zagol, please confirm the FH is to be relocated to the East side of Beale St as highlighted by the green line on the attached drawing.</p> <p>Please advise.</p>						<b>ANSWER:</b> <p>Reference sheet U-3124 and attached photo</p> <p>See the photo attached. The proposed location for the FH on Beale St at ~Sta 2+20 is in between a driveway for a parking garage and a driveway for a loading dock. Per discussions with Eric Zagol, please confirm the FH is to be relocated to the East side of Beale St as highlighted by the green line on the attached drawing.</p> <p>Please advise.</p>





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U-0111	Minna St. Joint Trench Conflict with (E) 8" elbow and thrust block	Closed	CR	03/04/2011	03/14/2011	03/09/2011
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>						
Reference drawing sheet U-3409 and attached sketch.						
During our excavation for the joint trench on the east end of Minna St. (STA 9+29) Trinet encountered the (E) 8" water main in Trinet's trench line, approximately 1 foot from our termination point. The existing alignment is different from what is shown in the contract drawings. The drawings do not show the water line crossing the joint trench. The alignment and grade of the water main changed in Trinet's excavation to avoid the adjacent catch basin. A 22.5 degree elbow is located in the center of the joint trench excavation. The elbow is rolled up to accommodate the grade change and there is a thrust block under the footing. Trinet does not believe that it would be safe to excavate under the water main for Trinet's duct bank without having the line shutoff. Extending the PG&E ducts to FOC will also place the connection point for PG&E's extension of the duct bank directly under the water main fittings and elbows. There is adequate clearance to install the 4" gas line above the water main and extend it out to FOC per contract. The top of the water main is 49" below FG at the south side of the joint trench, at the location of the ags line.						
Trinet proposes to terminate the concrete encased duct bank approximately 5 ft. back from FOC. This would allow adequate room for Trinet to mandrel the ducts after the joint trench is installed without undermining the water main. PG&E could then extend their duct bank under the water main to connect to Trinet's water main. Please advise.						
<b>ANSWER:</b>						
Reference drawing sheet U-3409 and attached sketch.						
During our excavation for the joint trench on the east end of Minna St. (STA 9+29) Trinet encountered the (E) 8" water main in Trinet's trench line, approximately 1 foot from our termination point. The existing alignment is different from what is shown in the contract drawings. The drawings do not show the water line crossing the joint trench. The alignment and grade of the water main changed in Trinet's excavation to avoid the adjacent catch basin. A 22.5 degree elbow is located in the center of the joint trench excavation. The elbow is rolled up to accommodate the grade change and there is a thrust block under the footing. Trinet does not believe that it would be safe to excavate under the water main for Trinet's duct bank without having the line shutoff. Extending the PG&E ducts to FOC will also place the connection point for PG&E's extension of the duct bank directly under the water main fittings and elbows. There is adequate clearance to install the 4" gas line above the water main and extend it out to FOC per contract. The top of the water main is 49" below FG at the south side of the joint trench, at the location of the ags line.						
Trinet proposes to terminate the concrete encased duct bank approximately 5 ft. back from FOC. This would allow adequate room for Trinet to mandrel the ducts after the joint trench is installed without undermining the water main. PG&E could then extend their duct bank under the water main to connect to Trinet's water main. Please advise.						
U-0111.1	Minna St Joint Trench Conflict @ Existing Water Line Elbow	Closed	03	04/18/2011	04/28/2011	04/21/2011
<b>From:</b> Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b>						
Please find the attached as built drawing of the Joint Trench @ the intersection of Minna St. and First St. where the (E) 8" W main elbow was encountered.						
<b>ANSWER:</b>						
Please find the attached as built drawing of the Joint Trench @ the intersection of Minna St. and First St. where the (E) 8" W main elbow was encountered.						



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>U-0111.2</b>	<b>Minna St Joint Trench Conflict @ Existing Water Line Elbow</b>	<b>Closed</b>	<b>03</b>	<b>04/25/2011</b>	<b>05/05/2011</b>	<b>04/28/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>  Eric Zagol 4/20/2011: Please provide the information requested in RFI U-0111 response or confirm that the existing water line referenced in RFI U-0111 is mechanically restrained.  Answer: The waterline is mechanically restrained.						<b>ANSWER:</b>  Eric Zagol 4/20/2011: Please provide the information requested in RFI U-0111 response or confirm that the existing water line referenced in RFI U-0111 is mechanically restrained.  Answer: The waterline is mechanically restrained.
<b>U-0112</b>	<b>Minna St. Joint Trench, AT&amp;T Vault and Conduit Configuration</b>	<b>Closed</b>	<b>03</b>	<b>03/08/2011</b>	<b>03/18/2011</b>	<b>03/15/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Sheet U-3408  At the 02/03/2011 Joint Trench Pre-Construction meeting and field walk through, the AT&T inspector expressed concern with the configuration of the AT&T ducts connecting to the AT&T vault at Sta 3+71. The AT&T inspector was specifically concerned with the east side of the vault where all eight 4-inch ducts are shown entering the vault on the one side (north side) of the center line.  Trinet would like AT&T to review the duct configuration connection to the vault as depicted in the contract drawings and provide a revised drawing if they wish to make a change.						<b>ANSWER:</b>  Reference Sheet U-3408  At the 02/03/2011 Joint Trench Pre-Construction meeting and field walk through, the AT&T inspector expressed concern with the configuration of the AT&T ducts connecting to the AT&T vault at Sta 3+71. The AT&T inspector was specifically concerned with the east side of the vault where all eight 4-inch ducts are shown entering the vault on the one side (north side) of the center line.  Trinet would like AT&T to review the duct configuration connection to the vault as depicted in the contract drawings and provide a revised drawing if they wish to make a change.
<b>U-0113</b>	<b>AWSS Cap on First St. at Howard</b>	<b>Closed</b>	<b>03</b>	<b>03/08/2011</b>	<b>03/18/2011</b>	<b>03/10/2011</b>
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>  Reference Drawing No. AWSS MA-5  On 03/08/2011, M Squared excavated and exposed the existing AWSS line and gate valve on First St. at Howard. Upon inspection of the existing gate valve, it appears that the gate valve does not have lugs on it. This means that M Squared cannot tie back the proposed 10-inch AWSS cap on the AWSS line.  Please advise on how you would like M Squared to						<b>ANSWER:</b>  Reference Drawing No. AWSS MA-5  On 03/08/2011, M Squared excavated and exposed the existing AWSS line and gate valve on First St. at Howard. Upon inspection of the existing gate valve, it appears that the gate valve does not have lugs on it. This means that M Squared cannot tie back the proposed 10-inch AWSS cap on the AWSS line.  Please advise on how you would like M Squared to





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U-0113.1	<b>AWSS Strong Backs</b>  From: Webcor Construction LP                      Nhi Tran  <b>REQUEST:</b> Reference RFI #U-0113  On 3/16/2011, M Squared met with Dan Helminiak from SFWD and Michael Smith from BOE to proceed with the AWSS Cap work at First & Howard. As directed in the response to RFI#U-0013, M Squared installed the strong back provided to them. After the strong back was installed, Dan H. and Michael S. determined that the strong backs would not work due to the diameter of the existing valve bell.  M Squared requests direction on how to proceed.	Closed	03	03/17/2011	03/27/2011	03/22/2011
	proceed with the cap installation. An expedited response is requested.					proceed with the cap installation. An expedited response is requested.
	<b>ANSWER:</b> Reference RFI #U-0113  On 3/16/2011, M Squared met with Dan Helminiak from SFWD and Michael Smith from BOE to proceed with the AWSS Cap work at First & Howard. As directed in the response to RFI#U-0013, M Squared installed the strong back provided to them. After the strong back was installed, Dan H. and Michael S. determined that the strong backs would not work due to the diameter of the existing valve bell.  M Squared requests direction on how to proceed.					





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U-0114	PG&E Abandonment Schedule for Natoma St. at Second St.	Closed	03	03/09/2011	03/19/2011	05/07/2011
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet U-1110 and U-2010		Reference Sheet U-1110 and U-2010				
On 03/04/2011, M Squared met with a PG&E representative on site at Natoma and 2nd Street. The PG&E representative confirmed that none of their utilities had been abandoned in the area, and that the PG&E representative would be unable to provide a schedule for this abandonment.		On 03/04/2011, M Squared met with a PG&E representative on site at Natoma and 2nd Street. The PG&E representative confirmed that none of their utilities had been abandoned in the area, and that the PG&E representative would be unable to provide a schedule for this abandonment.				
Per note 2 on sheet U-1110, the services for 77 Natoma and 83 Natoma were to be terminated by Feb 2011. To date, this work does not appear to be completed. In PG&E's letter to the TJPA regarding their schedule, there is no reference to work on Natoma Street at 2nd St.		Per note 2 on sheet U-1110, the services for 77 Natoma and 83 Natoma were to be terminated by Feb 2011. To date, this work does not appear to be completed. In PG&E's letter to the TJPA regarding their schedule, there is no reference to work on Natoma Street at 2nd St.				
M Squared is unable to proceed with their sewer and water utility installation on Natoma St. west of shoring wall until PG&E has completed abandonment of their existing utilities.		M Squared is unable to proceed with their sewer and water utility installation on Natoma St. west of shoring wall until PG&E has completed abandonment of their existing utilities.				
Please provide M Squared with an updated schedule for all PG&E's termination/abandonment work at 2nd and Natoma St.		Please provide M Squared with an updated schedule for all PG&E's termination/abandonment work at 2nd and Natoma St.				
U-0115	AWSS Cap Work Sequence on First St	Closed	03	03/07/2011	03/17/2011	03/15/2011
<b>From:</b> Webcor Construction LP      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Refer to Sheets MA-5, MA-8		Refer to Sheets MA-5, MA-8				
There are two caps that are required to be installed in order to shutdown the AWSS service on First St between Mission to Howard St. Per the construction schedule, both caps were supposed to be worked on simultaneously. Please confirm per a conversation in the field on 03/07/2011 with inspectors Michael Smith (SFDPW) and Dan Helminak (DPW), only one AWSS cap can be installed at a time.		There are two caps that are required to be installed in order to shutdown the AWSS service on First St between Mission to Howard St. Per the construction schedule, both caps were supposed to be worked on simultaneously. Please confirm per a conversation in the field on 03/07/2011 with inspectors Michael Smith (SFDPW) and Dan Helminak (DPW), only one AWSS cap can be installed at a time.				



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<b>U-0116</b>	<b>Abandoned 6" Fire Water Service Thru 100 First St Basement Wall</b>	<b>Closed</b>	<b>03</b>	<b>03/18/2011</b>	<b>03/28/2011</b>	<b>03/21/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Refer to sheets U-1109 and U-3109		Refer to sheets U-1109 and U-3109				
An abandoned existing 6" fire water service lateral was discovered while demolishing the old 8" water main running down Minna St. The 6" fire water service lateral was not shown on the plans and there were no existing water valve covers to indicate the existence of this line. The abandoned lateral penetrates the foundation wall entering the basement to 100 First St at Station 7+36.		An abandoned existing 6" fire water service lateral was discovered while demolishing the old 8" water main running down Minna St. The 6" fire water service lateral was not shown on the plans and there were no existing water valve covers to indicate the existence of this line. The abandoned lateral penetrates the foundation wall entering the basement to 100 First St at Station 7+36.				
Please provide direction for plugging the void that will be left after 100 First St management removes the 6" water lateral pipe. A roughly 1ft x ft x 1ft deep square opening will remain after the fire water lateral pipe is removed.		Please provide direction for plugging the void that will be left after 100 First St management removes the 6" water lateral pipe. A roughly 1ft x ft x 1ft deep square opening will remain after the fire water lateral pipe is removed.				
<b>U-0117</b>	<b>Natoma St. Future Hydrant Location at Sta 11+79</b>	<b>Closed</b>	<b>03</b>	<b>03/21/2011</b>	<b>03/31/2011</b>	<b>03/24/2011</b>
<b>From:</b> Webcor Construction LP                      Nhi Tran						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference Sheet U-3113		Reference Sheet U-3113				
Sheet U-3113 shows an 8in x 8in x 6in tee in the new 8-inch water main on Natoma at Sta 11+79. The note on the drawing makes reference to it being used as a future location for a fire hydrant. Sta 11+79 is in front of a loading dock and parking garage on Natoma Street.		Sheet U-3113 shows an 8in x 8in x 6in tee in the new 8-inch water main on Natoma at Sta 11+79. The note on the drawing makes reference to it being used as a future location for a fire hydrant. Sta 11+79 is in front of a loading dock and parking garage on Natoma Street.				
Please confirm that it is intended for M Squared to install the tee in the water main line at this location.		Please confirm that it is intended for M Squared to install the tee in the water main line at this location.				
<b>U-0118</b>	<b>Minna Street Joint Trench, PG&amp;E Duct Routing and Termination Points</b>	<b>Closed</b>	<b>03</b>	<b>03/24/2011</b>	<b>04/03/2011</b>	<b>04/06/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture                      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please provide a routing drawing or written clarification of the routing for the PG&E Duct stub-outs in the Minna St. Joint Trench, between First St. and Second St. It is not		Please provide a routing drawing or written clarification of the routing for the PG&E Duct stub-outs in the Minna St. Joint Trench, between First St.				



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	clear from the plans in all cases where all the ducts extending from stub-outs terminate. Please expedite.				and Second St. It is not clear from the plans in all cases where all the ducts extending from stub-outs terminate. Please expedite.	
U-0119	Minna St. JT_ AT&T Reconfiguration and impact on (E) trees	Closed	03	03/25/2011	04/04/2011	03/30/2011
<hr/>						
From: Webcor Construction LP		Colin Azevedo				
REQUEST:			ANSWER:			
The revised drawings for the Joint Trench alignment dated 3/16/2011 show the reconfigured AT&T ducts running through an existing tree well on the east side of the AT&T vault at Stn. 3+71. RFI U-0112 (Minna St, Joint Trench, AT&T Vault and Conduit Configuration) also shows the reconfigured AT&T ducts running through an existing tree well on the east side of the vault. This conduit layout in consistent with discussions with the AT&T inspector in the field was reflected in the shop drawings. The revised drawings do not address relocation and/or removal of the impacted trees and the related irrigation changes. Please review and advise.			The revised drawings for the Joint Trench alignment dated 3/16/2011 show the reconfigured AT&T ducts running through an existing tree well on the east side of the AT&T vault at Stn. 3+71. RFI U-0112 (Minna St, Joint Trench, AT&T Vault and Conduit Configuration) also shows the reconfigured AT&T ducts running through an existing tree well on the east side of the vault. This conduit layout in consistent with discussions with the AT&T inspector in the field was reflected in the shop drawings. The revised drawings do not address relocation and/or removal of the impacted trees and the related irrigation changes. Please review and advise.			
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U-0120	MH601 Locatio	Closed	03	03/28/2011	04/07/2011	04/05/2011
<hr/>						
From: Webcor Construction LP		Colin Azevedo				
REQUEST:			ANSWER:			
Sheet U-3022 shows MH601 @ Sta 0+70 on Fremont Street. This location is also in the middle of the crosswalk on Fremont Street. USA markings show the existing traffic signal conduits crossing thru the center of the manhole. By moving the manhole approx 8¿ north the conflict with the traffic signal conduits would be avoided and it would also avoid having a manhole cover in a crosswalk. Please advise on how you would like to proceed.			Sheet U-3022 shows MH601 @ Sta 0+70 on Fremont Street. This location is also in the middle of the crosswalk on Fremont Street. USA markings show the existing traffic signal conduits crossing thru the center of the manhole. By moving the manhole approx 8¿ north the conflict with the traffic signal conduits would be avoided and it would also avoid having a manhole cover in a crosswalk. Please advise on how you would like to proceed.			
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U-0121	AWSS Caps at Beale Street	Closed	03	03/31/2011	04/10/2011	04/06/2011
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From: Webcor Construction LP		Colin Azevedo				





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U-0122	M Squared Submittals for TG04 Bid Packages	Closed	CR	04/01/2011	04/11/2011	04/11/2011
<div><div><div>From: Webcor Construction LP</div><div>Colin Azevedo</div></div><div><div>REQUEST:</div><div>Please confirm the following:</div><div>Per previous discussions it has been agreed between the TJPA, AECOM, Turner Webcor/Obayashi and M Squared that material submittals approved for use by M Squared in individual bid packages will be considered acceptable for all bid packages M Squared is working on (TG04.1, TG04.3, TG04.4, &amp; TG04.6).</div><div>These submittal include:</div><div>TG0434-002 - Excavation &amp; Backfill Samples TG0434-003 - Excavation &amp; Backfill Test Reports TG0434-004 - Excavation &amp; Backfill Compaction &amp; Warning Tape TG0434-005 - Shoring Plan TG0434-006 - Backfill Material TG0434-007 - Water Utilities Distribution Piping &amp; Valves TG0434-010 - Asphalt Mix Design TG0434-013 - Noise Mitigation Plan TG0434-015 - CQC Plan TG0434-016 - Health and Safety Plan and MSDS TG0434-017 - SWPPP TG0434-018 - Debris Management Plan TG0434-025 - Cast in Place Concrete TG0434-030 - Labor Rates TG0404-001 - Sewer Package TG0404-002 - Filter Fabric TG0404-003 - Concrete Forming TG0404-004 - Precast Concrete TG0404-005 - Precast Concrete Catch Basin Base</div></div></div> <div><div>ANSWER:</div><div>Please confirm the following:</div><div>Per previous discussions it has been agreed between the TJPA, AECOM, Turner Webcor/Obayashi and M Squared that material submittals approved for use by M Squared in individual bid packages will be considered acceptable for all bid packages M Squared is working on (TG04.1, TG04.3, TG04.4, &amp; TG04.6).</div><div>These submittal include:</div><div>TG0434-002 - Excavation &amp; Backfill Samples TG0434-003 - Excavation &amp; Backfill Test Reports TG0434-004 - Excavation &amp; Backfill Compaction &amp; Warning Tape TG0434-005 - Shoring Plan TG0434-006 - Backfill Material TG0434-007 - Water Utilities Distribution Piping &amp; Valves TG0434-010 - Asphalt Mix Design TG0434-013 - Noise Mitigation Plan TG0434-015 - CQC Plan TG0434-016 - Health and Safety Plan and MSDS TG0434-017 - SWPPP TG0434-018 - Debris Management Plan TG0434-025 - Cast in Place Concrete TG0434-030 - Labor Rates TG0404-001 - Sewer Package TG0404-002 - Filter Fabric TG0404-003 - Concrete Forming TG0404-004 - Precast Concrete TG0404-005 - Precast Concrete Catch Basin Base</div></div>						



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
<b>U-0123</b>	<b>Unknown Fire Service @ 85 Natoma</b>	<b>Closed</b>	<b>03</b>	<b>04/04/2011</b>	<b>04/14/2011</b>	<b>04/05/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> While Excavating to install the water line on Natoma from the shoring wall to 2nd Street M Squared encountered an existing fire service going to 85 Natoma. This service is not shown on the drawings and is not in the specifications as one of the connections to be made to the new line. (See attached) Please advise on how to proceed.						<b>ANSWER:</b> While Excavating to install the water line on Natoma from the shoring wall to 2nd Street M Squared encountered an existing fire service going to 85 Natoma. This service is not shown on the drawings and is not in the specifications as one of the connections to be made to the new line. (See attached) Please advise on how to proceed.
<b>U-0123.1</b>	<b>Fire Service @ 85 Natoma</b>	<b>Closed</b>	<b>03</b>	<b>04/11/2011</b>	<b>04/21/2011</b>	<b>04/18/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> Please note that on RFI #U-0123 the location of the fire service was incorrectly drawn. The fire service is actually located around Sta 2+35.  M Squared potholed at Sta 2+35 and discovered a 4" ductile iron pipe which is believe to be the active fire service for 85 Natoma Street.  Please advise.						<b>ANSWER:</b> Please note that on RFI #U-0123 the location of the fire service was incorrectly drawn. The fire service is actually located around Sta 2+35.  M Squared potholed at Sta 2+35 and discovered a 4" ductile iron pipe which is believe to be the active fire service for 85 Natoma Street.  Please advise.
<b>U-0124</b>	<b>Conflict Between New 24" Sewer and existing AWSS Line on Beale</b>	<b>Closed</b>	<b>03</b>	<b>04/07/2011</b>	<b>04/17/2011</b>	<b>04/28/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> M Squared has confirmed that the 14" AWSS Line shown on sheet U-3024 is in conflict with the proposed 24" VCP on Beale Street. The AWSS line is shown on the plan view but not on the elevation view on sheet U-3024. M Squared also shot the elevation of the existing sewer manhole. The elevation is 4.60, and not 4.70 as shown on the plans. The invert of the 14" AWSS is 6.2. (See attached) Please advise.						<b>ANSWER:</b> M Squared has confirmed that the 14" AWSS Line shown on sheet U-3024 is in conflict with the proposed 24" VCP on Beale Street. The AWSS line is shown on the plan view but not on the elevation view on sheet U-3024. M Squared also shot the elevation of the existing sewer manhole. The elevation is 4.60, and not 4.70 as shown on the plans. The invert of the 14" AWSS is 6.2. (See attached) Please advise.



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U-0124.1	<b>Conflict Between 24" Sewer and AWSS Line on Beale</b>  From: Webcor Construction LP                      Colin Azevedo  <b>REQUEST:</b>  Per the response to RFI#U-0124 a design to relocate the AWSS line @ Howard and Beale is forthcoming. Please advise the status of this design.	Closed	03	07/07/2011	07/17/2011	03/27/2012
		<b>ANSWER:</b>  Per the response to RFI#U-0124 a design to relocate the AWSS line @ Howard and Beale is forthcoming. Please advise the status of this design.				
U-0125	<b>Precast Catch Basin Bases</b>  From: Webcor Construction LP                      Colin Azevedo  <b>REQUEST:</b>  In lieu of a cast in place base per CCSF DPW Standards, M Squared would like to propose the use of a precast catch basin. The catch basin barrel is attached to the precast base and it comes as one single unit. Before installing the precast catch basin base with barrel, M Squared will place a minimum 6" compacted level layer of crushed rock as the sub base. The proposed material specifications are attached. Please confirm if this method is acceptable.	Closed	03	04/08/2011	04/18/2011	04/13/2011
		<b>ANSWER:</b>  In lieu of a cast in place base per CCSF DPW Standards, M Squared would like to propose the use of a precast catch basin. The catch basin barrel is attached to the precast base and it comes as one single unit. Before installing the precast catch basin base with barrel, M Squared will place a minimum 6" compacted level layer of crushed rock as the sub base. The proposed material specifications are attached. Please confirm if this method is acceptable.				
U-0126	<b>Existing Brick Man Hole @ Second and Natoma In Conflict With Joint Trench</b>  From: Webcor Construction LP                      Colin Azevedo  <b>REQUEST:</b>  While potholing the Second St. Joint Trench crossing Trinet encountered an existing brick sewer man hole which is in conflict with the joint trench alignment. The manhole is not shown on the plans and had been paved over. The manhole also appears to have been previously abandoned. See the attached sketch and photograph detailing the location of the manhole.  Please advise on how to proceed.	Closed	03	04/11/2011	04/11/2011	04/13/2011
		<b>ANSWER:</b>  While potholing the Second St. Joint Trench crossing Trinet encountered an existing brick sewer man hole which is in conflict with the joint trench alignment. The manhole is not shown on the plans and had been paved over. The manhole also appears to have been previously abandoned. See the attached sketch and photograph detailing the location of the manhole.  Please advise on how to proceed.				
U-0127	<b>Minna Street Sewer Manhole #201 in Crosswalk</b>  From: Webcor Construction LP                      Colin Azevedo  <b>REQUEST:</b>	Closed	03	04/11/2011	04/21/2011	04/13/2011
		<b>ANSWER:</b>				





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<b>U-0128</b>	<p>Plan Sheet U-3007 shows MH#201 to be installed in the center of the crosswalk @ Minna and Second Street. The City of San Francisco typically avoids locating manholes in crosswalks, whenever possible, for ADA considerations. Please advise if MH#201 should be installed outside of the crosswalk.</p> <p><b>AWSS Conflict with Sewer on Fremont</b></p> <p><b>From:</b> Webcor Construction LP      Colin Azevedo</p> <p><b>REQUEST:</b></p> <p>A pothole at Sta 0+52 has confirmed that the existing AWSS line is in direct conflict with the proposed sewer on Fremont Street. The drawings show a 4" HPW line at invert elevation 13.0. Measurements taken in the pothole reveal a 14" HPW line at invert elevation 8.4. At this elevation the HPW line is in direct conflict with the proposed VCP sewer. Please advise.</p>	<b>Closed</b>	<b>03</b>	<b>04/11/2011</b>	<b>04/21/2011</b>	<b>04/19/2011</b>
	<p>Plan Sheet U-3007 shows MH#201 to be installed in the center of the crosswalk @ Minna and Second Street. The City of San Francisco typically avoids locating manholes in crosswalks, whenever possible, for ADA considerations. Please advise if MH#201 should be installed outside of the crosswalk.</p> <p><b>ANSWER:</b></p> <p>A pothole at Sta 0+52 has confirmed that the existing AWSS line is in direct conflict with the proposed sewer on Fremont Street. The drawings show a 4" HPW line at invert elevation 13.0. Measurements taken in the pothole reveal a 14" HPW line at invert elevation 8.4. At this elevation the HPW line is in direct conflict with the proposed VCP sewer. Please advise.</p>					
<b>U-0128.1</b>	<p><b>AWSS Conflict with Sewer on Fremont</b></p> <p><b>From:</b> Webcor Construction LP      Colin Azevedo</p> <p><b>REQUEST:</b></p> <p>M Squared has confirmed the invert elevation for the existing manhole at station 0+29.5 Fremont St. is EL 6.4 as shown on U-3022.</p> <p>Please advise.</p>	<b>Closed</b>	<b>03</b>	<b>04/11/2011</b>	<b>04/21/2011</b>	<b>04/26/2011</b>
	<p><b>ANSWER:</b></p> <p>M Squared has confirmed the invert elevation for the existing manhole at station 0+29.5 Fremont St. is EL 6.4 as shown on U-3022.</p> <p>Please advise.</p>					
<b>U-0128.2</b>	<p><b>AWSS Conflict with Sewer on Fremont</b></p> <p><b>From:</b> Webcor Construction LP      Colin Azevedo</p> <p><b>REQUEST:</b></p> <p>Per the response to RFI#U-0128.1 a design to relocate the AWSS line @ Howard and Fremont is forthcoming. Please advise the status of this design.</p>	<b>Closed</b>	<b>03</b>	<b>07/07/2011</b>	<b>07/17/2011</b>	<b>03/27/2012</b>
	<p><b>ANSWER:</b></p> <p>Per the response to RFI#U-0128.1 a design to relocate the AWSS line @ Howard and Fremont is forthcoming. Please advise the status of this design.</p>					





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<b>U-0129</b>	<b>Sewer Conflicts @ Second and Natoma</b>	<b>Closed</b>	<b>03</b>	<b>04/13/2011</b>	<b>04/25/2011</b>	<b>04/28/2011</b>
<b>From:</b> Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b> M Squared is unable to excavate/shore/install the 18" VCP from the existing manhole at Sta 0+45 to MH#301 at Sta 0+81 as shown on sheet U-3010. While excavating for the sewer installation M Squared encountered several unknown utilities which were unmarked and not shown on the contract drawings. Also, some of the known utilities are at different locations and elevations than indicated on the drawings. Due to the quantity and proximity of these utilities it is not possible to excavate and shore between MH#301 and the existing MH at Sta 0+45. Additionally PGE have yet to relocate their gas and electric utilities out of the area of the proposed MH#301. See attached drawings illustrating M Squared's pothole findings. Please advise on how to proceed.		<b>ANSWER:</b> M Squared is unable to excavate/shore/install the 18" VCP from the existing manhole at Sta 0+45 to MH#301 at Sta 0+81 as shown on sheet U-3010. While excavating for the sewer installation M Squared encountered several unknown utilities which were unmarked and not shown on the contract drawings. Also, some of the known utilities are at different locations and elevations than indicated on the drawings. Due to the quantity and proximity of these utilities it is not possible to excavate and shore between MH#301 and the existing MH at Sta 0+45. Additionally PGE have yet to relocate their gas and electric utilities out of the area of the proposed MH#301. See attached drawings illustrating M Squared's pothole findings. Please advise on how to proceed.				
<b>U-0129.1</b>	<b>Sewer Conflicts @ Second and Natoma</b>	<b>Closed</b>	<b>03</b>	<b>05/02/2011</b>	<b>05/12/2011</b>	<b>06/03/2011</b>
<b>From:</b> Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b> Per response to RFI#U-0129 Webcor/Obayashi, M Squared and AECOM met on 4/29/2011 and discussed why the sewer line between MH#301 and the existing manhole at Sta 0+45 could not be installed with normal means and methods. M Squared remove the plates from their investigative pot hole trench on 5/2/2011 for AECOM to further review and understand the existing conflicts.  Please provide AECOM's findings from these meetings and provide direction on how to proceed with the sewer installation in this location.		<b>ANSWER:</b> Per response to RFI#U-0129 Webcor/Obayashi, M Squared and AECOM met on 4/29/2011 and discussed why the sewer line between MH#301 and the existing manhole at Sta 0+45 could not be installed with normal means and methods. M Squared remove the plates from their investigative pot hole trench on 5/2/2011 for AECOM to further review and understand the existing conflicts.  Please provide AECOM's findings from these meetings and provide direction on how to proceed with the sewer installation in this location.				
<b>U-0130</b>	<b>Sewer Removal On First Street</b>	<b>Closed</b>	<b>03</b>	<b>04/15/2011</b>	<b>04/25/2011</b>	<b>04/21/2011</b>
<b>From:</b> Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b> During the weekly Utility Relocation OAC meeting on 04/12/2011 Eric Zagol with AECOM informed		<b>ANSWER:</b> During the weekly Utility Relocation OAC meeting on 04/12/2011 Eric Zagol with AECOM informed				



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U-0131	<b>Minna St PG&amp;E Duct Bank Termination Points</b>  From: Webcor Construction LP                      Colin Azevedo  <b>REQUEST:</b>  PG&E has confirmed Trinet is to terminate the PG&E duct back 3' outside the east and west walls of manhole 1319. Please confirm that the termination points of the PG&E duct bank as described will fulfill Trinet's scope of work and the future completion of the duct bank will be performed by PG&E.  Please note terminating the duct bank 3' outside the west wall of MH 1319 will leave the end of the ducts directly under the 24" high pressure water main. This may create an issue with future access for complete the duct bank by PG&E.  Please advise.	Closed	03	04/19/2011	04/29/2011	04/22/2011
U-0132	<b>Minna St Sewer Pressure Test</b>  From: Webcor Construction LP                      Colin Azevedo  <b>REQUEST:</b>  The SFDPW inspector Jason Chin has advised Trinet that he will be requesting a pressure test of the newly installed 18" and 24" VCP sewer main. The contract specification and drawings to do not specify any form of testing for the sewer mains.  Please advise if pressure testing of the sewer main will be required.	Closed	03	04/20/2011	04/30/2011	04/27/2011
U-0132.1	<b>Sewer Main Pressure Test</b>	Closed	03	05/07/2011	05/17/2011	05/11/2011





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U-0133.1	Minna St Joint Trench Configuration and Alignment, Sta 2+24	Closed	03	04/26/2011	05/10/2011	05/02/2011
From: Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b>  During the installation of the AT&T ducts between Sta 2+24 and 1+62 the AT&T inspector, Juan, instructed Trinet to remove two bends from the duct bank. AECOM was contacted and approved the layout in the field prior to Trinet proceeding. Attached is the revised AT&T duct routing required by the inspector.  Please confirm the revised joint trench alignment is acceptable.						<b>ANSWER:</b>  During the installation of the AT&T ducts between Sta 2+24 and 1+62 the AT&T inspector, Juan, instructed Trinet to remove two bends from the duct bank. AECOM was contacted and approved the layout in the field prior to Trinet proceeding. Attached is the revised AT&T duct routing required by the inspector.  Please confirm the revised joint trench alignment is acceptable.
U-0134	Water Department Tie In Conflict at Howard and Beale	Closed	03	04/26/2011	05/06/2011	05/02/2011
From: Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b>  The SF Water Department has determined they are unable to perform the water tie in at the south west corner of Howard and Beale because of a conflict with the existing sewer sludge force main. M Squared has pothole the line and confirmed it is the existing 10" concrete encased sewer sludge force main.  Please advise.						<b>ANSWER:</b>  The SF Water Department has determined they are unable to perform the water tie in at the south west corner of Howard and Beale because of a conflict with the existing sewer sludge force main. M Squared has pothole the line and confirmed it is the existing 10" concrete encased sewer sludge force main.  Please advise.
U-0135	4" Water Service @ 1st and Natoma	Closed	03	04/27/2011	05/07/2011	05/05/2011
From: Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b>  While excavating for the 6" service connection to the new water line on First Street at Sta2+25 M Squared located an additional 4" ductile iron service that is connected to the existing water main. This 4" line is not shown in the contract documents.  SFWD records show this to be a live service and would like for this to be tied into the new main.  There is now no point of connection on the new water line to receive this 4" service.  Please advise.						<b>ANSWER:</b>  While excavating for the 6" service connection to the new water line on First Street at Sta2+25 M Squared located an additional 4" ductile iron service that is connected to the existing water main. This 4" line is not shown in the contract documents.  SFWD records show this to be a live service and would like for this to be tied into the new main.  There is now no point of connection on the new water line to receive this 4" service.  Please advise.



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U-0135.1	4" Water Service at First and Natoma	Closed	03	05/09/2011	05/19/2011	05/10/2011
From: Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b>						<b>ANSWER:</b>
In response to RFI #U-0135, see attached piping plan, as requested in RFI response.						In response to RFI #U-0135, see attached piping plan, as requested in RFI response.
Once approved M Squared will coordinate with SFWD to perform the work.						Once approved M Squared will coordinate with SFWD to perform the work.
**An expedited response is required as this is holding up all other water work on Natoma Street**						**An expedited response is required as this is holding up all other water work on Natoma Street**
U-0136	Existing Water Bypass @ Howard and Fremont	Closed	03	05/03/2011	05/13/2011	05/05/2011
From: Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b>						<b>ANSWER:</b>
While planning for the water tie in at Howard and Beale the Water Department discovered that there is an existing bypass line that will connect the existing water system (which is to be abandoned) to the new water system. This bypass is not shown on the plans. The Water department has requested that the existing bypass be excavated and plated so it can be cut and capped while they have the line shut down for the tie in on the new system at Howard and Beale the night of 05/04/2011.						While planning for the water tie in at Howard and Beale the Water Department discovered that there is an existing bypass line that will connect the existing water system (which is to be abandoned) to the new water system. This bypass is not shown on the plans. The Water department has requested that the existing bypass be excavated and plated so it can be cut and capped while they have the line shut down for the tie in on the new system at Howard and Beale the night of 05/04/2011.
Please advise.						Please advise.
U-0137	Verizon Ductbank conflict w/MH 701	Closed	03	05/03/2011	05/13/2011	05/10/2011
From: Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b>						<b>ANSWER:</b>
M Squared's sewer potholing on Beale (Sta 0+30) has indicated a conflict between an existing Verizon duct bank and MH# 701 on Howard Street. See attached drawing. The ductbank is approximately 18" wide x 18" deep. It is 2'4" to the top and it is slurry encased. Verizon underground locators have confirmed that this is live and serves Charles Schwabb building south of Howard on Beale Street. Please advise.						M Squared's sewer potholing on Beale (Sta 0+30) has indicated a conflict between an existing Verizon duct bank and MH# 701 on Howard Street. See attached drawing. The ductbank is approximately 18" wide x 18" deep. It is 2'4" to the top and it is slurry encased. Verizon underground locators have confirmed that this is live and serves Charles Schwabb building south of Howard on Beale Street. Please advise.



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<b>U-0138</b>	<b>Temporary Telecom Pole Layout in Lot N and N'</b>	<b>Closed</b>	<b>03</b>	<b>05/09/2011</b>	<b>05/19/2011</b>	<b>05/10/2011</b>
<b>From:</b> Webcor Construction LP      Joanne Filipas						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference attached layout and submittal package#TG0406-014:		Reference attached layout and submittal package#TG0406-014:				
Due to the future use of lot N and N' prime, the temporary telecom poles must be relocated. The attached sketch indicates the proposed layout of these poles which has been coordinated with AECOM. Submittal Package#TG0406-014 has been submitted for formal approval of the pole locations.		Due to the future use of lot N and N' prime, the temporary telecom poles must be relocated. The attached sketch indicates the proposed layout of these poles which has been coordinated with AECOM. Submittal Package#TG0406-014 has been submitted for formal approval of the pole locations.				
Please confirm relocating the poles is acceptable.		Please confirm relocating the poles is acceptable.				
<b>U-0139</b>	<b>Existing Water Line on Beale in Conflict with New Sewer</b>	<b>Closed</b>	<b>03</b>	<b>05/09/2011</b>	<b>05/09/2011</b>	<b>05/10/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Today while trying to execute the USAR for the existing 12" water line on Beale Dan Helminiak with SFWD informed Webcor/Obayashi and M Squared that the existing water line will remain active until the water tie in at First and Natoma is completed and the existing 8" is capped at First and Howard as shown on sheet U-3116.		Today while trying to execute the USAR for the existing 12" water line on Beale Dan Helminiak with SFWD informed Webcor/Obayashi and M Squared that the existing water line will remain active until the water tie in at First and Natoma is completed and the existing 8" is capped at First and Howard as shown on sheet U-3116.				
The water tie in and capping of the existing line on First Street is currently being delayed by separate issues and it is unclear when this work will be completed.		The water tie in and capping of the existing line on First Street is currently being delayed by separate issues and it is unclear when this work will be completed.				
Dan Helminiak suggested that the existing 8" water line running down Howard could be capped by the water department at one of the existing tees which would allow the decommissioning of the existing line on Beale.		Dan Helminiak suggested that the existing 8" water line running down Howard could be capped by the water department at one of the existing tees which would allow the decommissioning of the existing line on Beale.				
Please advise.		Please advise.				
<b>U-0139.1</b>	<b>Cap (E) Water on Howard @ Beale</b>	<b>Closed</b>	<b>03</b>	<b>05/16/2011</b>	<b>05/26/2011</b>	<b>05/24/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				



Number	Subject	Status	Choice Required	Date Created	Date Required	Date Answered
	<p>-New 12" water main along Howard between First and Main is active.</p> <p>-New 12" water main along Beale Street North of Howard is active.</p> <p>-New 12" water main along Beale Street South of Mission is active.</p> <p>Per U-1124 Demolition and Construction Sequence order, Beale Street sewer is to commence after existing water main on Beale is abandoned.</p> <p>- The old water line on Howard Streets and Beale Streets is currently not active because the valves on the line at First and Howard are currently shutdown. Dan from the water department has expressed his concern that anyone can just open these valves and fill the old line along Howard Street. He is also concerned that the valve is not 100% closed and that the SFWD cannot get a complete shutdown on the old line. This means when M Squared removes the old water line on Beale Street in order to install the new sewer, it is possible that there will be a constant flow of water in the old line.</p> <p>The suggestion from Dan is to cap the old water line on Howard Street so that When M Squared removes the old line on Beale Street there will be no possibility of water flow. A cap on the line at Howard would also confirm for definite that the old line on Howard and Beale Street is "abandoned".</p> <p>Please provide direction for capping the existing water line on Howard so the sewer installation on Beale can proceed.</p>					
U-0140	<p><b>Proposed Changes by BLHP to S/L Conduit Run @ 2nd &amp; Minna</b></p> <p>From: Webcor Construction LP                      Colin Azevedo</p> <p><b>REQUEST:</b></p> <p>During a field meeting on 5/10/2011 with Eric Zagol, AECOM and Robert Kawano, BLHP to discuss the alignment of the conduit run from 2nd St to the relocated S/L pole @ Stn 2+89, Robert Kawano asked that a splice box be installed in the sidewalk downstream from the connection point to PG&amp;E's manhole. The box would</p>	Closed	03	05/11/2011	05/21/2011	05/20/2011
	<p>-New 12" water main along Howard between First and Main is active.</p> <p>-New 12" water main along Beale Street North of Howard is active.</p> <p>-New 12" water main along Beale Street South of Mission is active.</p> <p>Per U-1124 Demolition and Construction Sequence order, Beale Street sewer is to commence after existing water main on Beale is abandoned.</p> <p>- The old water line on Howard Streets and Beale Streets is currently not active because the valves on the line at First and Howard are currently shutdown. Dan from the water department has expressed his concern that anyone can just open these valves and fill the old line along Howard Street. He is also concerned that the valve is not 100% closed and that the SFWD cannot get a complete shutdown on the old line. This means when M Squared removes the old water line on Beale Street in order to install the new sewer, it is possible that there will be a constant flow of water in the old line.</p> <p>The suggestion from Dan is to cap the old water line on Howard Street so that When M Squared removes the old line on Beale Street there will be no possibility of water flow. A cap on the line at Howard would also confirm for definite that the old line on Howard and Beale Street is "abandoned".</p> <p>Please provide direction for capping the existing water line on Howard so the sewer installation on Beale can proceed.</p>					
	<p><b>ANSWER:</b></p> <p>During a field meeting on 5/10/2011 with Eric Zagol, AECOM and Robert Kawano, BLHP to discuss the alignment of the conduit run from 2nd St to the relocated S/L pole @ Stn 2+89, Robert Kawano asked that a splice box be installed in the sidewalk downstream from the connection point to PG&amp;E's</p>					









<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>U-0142</b>	<b>Concrete Specifications for Sidewalk Replacement @ 555 Mission</b>	<b>Closed</b>	<b>03</b>	<b>05/16/2011</b>	<b>05/26/2011</b>	<b>05/18/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>  The sidewalk concrete @ 555 Mission (on Minna) is not the typical San Francisco sidewalk mix design. It is a colored concrete with what appears to be a sandblasted finish. Please provide the concrete specifications for repair and/or replacement of the sidewalk in this area.						<b>ANSWER:</b>  The sidewalk concrete @ 555 Mission (on Minna) is not the typical San Francisco sidewalk mix design. It is a colored concrete with what appears to be a sandblasted finish. Please provide the concrete specifications for repair and/or replacement of the sidewalk in this area.
<b>U-0143</b>	<b>Demolition of PG&amp;E Duct Bank Alongside (N) 18" Sewer Main on Minna</b>	<b>Closed</b>	<b>03</b>	<b>05/16/2011</b>	<b>05/26/2011</b>	<b>05/20/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>  During excavation and shoring for installation of the 18" Sewer main along Minna St., between the (E) electrical vault @ Stn 1+80 (demolished) and (N) manhole # 201, Trinet was unable to save the entire length of the existing PG&E duct bank (currently abandoned), which runs along the south side of the sewer trench. Between stations Stn 0+95 and 1+25 (approx.) the duct bank had veered into the sewer trench and had to be demolished - see attached sketch. Please review and advise.						<b>ANSWER:</b>  During excavation and shoring for installation of the 18" Sewer main along Minna St., between the (E) electrical vault @ Stn 1+80 (demolished) and (N) manhole # 201, Trinet was unable to save the entire length of the existing PG&E duct bank (currently abandoned), which runs along the south side of the sewer trench. Between stations Stn 0+95 and 1+25 (approx.) the duct bank had veered into the sewer trench and had to be demolished - see attached sketch. Please review and advise.
<b>U-0143.1</b>	<b>(E) PG&amp;E Duct Bank from EMH #1320 to Demolished EMH #1355</b>	<b>Closed</b>	<b>03</b>	<b>06/14/2011</b>	<b>06/24/2011</b>	<b>06/14/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>  After further investigation of the existing PG&E duct bank between EMH #1320 and demolished EMH # 1355 (@ Anchor & Hope), Trinet found that there is only one unobstructed conduit between the two manholes. The unobstructed conduit is the one that already had a pull rope in place. Trinet had demolished a section of this conduit during excavation for sewer MH # 201 because it was in conflict with the shoring. Trinet replaced the damaged section (approx. 8 LF) on Saturday 6/1, and reconnected the pull rope in the conduit run. A sketch of the conduit run, depicting the section replaced, is attached. Please review and advise if one 4" conduit will be adequate from EMH #1320 to the west end of demolished EMH #1355.						<b>ANSWER:</b>  After further investigation of the existing PG&E duct bank between EMH #1320 and demolished EMH # 1355 (@ Anchor & Hope), Trinet found that there is only one unobstructed conduit between the two manholes. The unobstructed conduit is the one that already had a pull rope in place. Trinet had demolished a section of this conduit during excavation for sewer MH # 201 because it was in conflict with the shoring. Trinet replaced the damaged section (approx. 8 LF) on Saturday 6/1, and reconnected the pull rope in the conduit run. A sketch of the conduit run, depicting the section replaced, is attached. Please review and advise if one 4" conduit will be adequate from EMH #1320 to the west end of demolished EMH #1355.



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>U-0144</b>	<b>PGE Vault conflict with 24" VCP on Beale</b>	<b>Closed</b>	<b>CR</b>	<b>05/17/2011</b>	<b>05/27/2011</b>	<b>05/20/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> PG&E confirmed the location of the inside of the east wall of PG&E manhole 1702 at Howard and Beale Street. Allowing for a 12" thick wall, the vault will be in conflict with the proposed alignment of the future 24" VCP, even with moving the alignment 1' further east as directed in RFI U-0124. The conflict could be avoided by moving the alignment another 6" further east. However this will cause a conflict between manhole #701 and the existing 14" AWSS. Additionally the Verizon duct bank conflict increases(RFI#U-0137). Please advise.						<b>ANSWER:</b> PG&E confirmed the location of the inside of the east wall of PG&E manhole 1702 at Howard and Beale Street. Allowing for a 12" thick wall, the vault will be in conflict with the proposed alignment of the future 24" VCP, even with moving the alignment 1' further east as directed in RFI U-0124. The conflict could be avoided by moving the alignment another 6" further east. However this will cause a conflict between manhole #701 and the existing 14" AWSS. Additionally the Verizon duct bank conflict increases(RFI#U-0137). Please advise.
<b>U-0144.1</b>	<b>PG&amp;E Vault conflict with 24" VCP on Beale</b>	<b>Closed</b>	<b>CR</b>	<b>06/30/2011</b>	<b>07/10/2011</b>	<b>07/01/2011</b>
<b>From:</b> Webcor Construction LP      Jonathan Flaming						
<b>REQUEST:</b> In response to RFI U-0144, please note that M Squared confirms the following:  2-10inch VCP and future 24inch VCP will clear existing AWSS Valve at Sta 0+70.						<b>ANSWER:</b> In response to RFI U-0144, please note that M Squared confirms the following:  2-10inch VCP and future 24inch VCP will clear existing AWSS Valve at Sta 0+70.
<b>U-0145</b>	<b>Sludge Main Conflicts with Existing Utilities</b>	<b>Closed</b>	<b>03</b>	<b>05/17/2011</b>	<b>05/27/2011</b>	<b>05/18/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> Please see attached pothole results for the new sludge main on Mission Street. Due to the quantity and location of existing utilities, and utility vaults/manholes it will not be possible to install the new 12" sludge main on Mission Street as shown on the contract drawings.  Please advise.						<b>ANSWER:</b> Please see attached pothole results for the new sludge main on Mission Street. Due to the quantity and location of existing utilities, and utility vaults/manholes it will not be possible to install the new 12" sludge main on Mission Street as shown on the contract drawings.  Please advise.
<b>U-0145.1</b>	<b>Sludge Main Conflicts with existing utilities</b>	<b>Closed</b>	<b>03</b>	<b>05/18/2011</b>	<b>05/28/2011</b>	<b>06/07/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						







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<b>U-0149</b>	<b>MH#701 Conflicts with existing utilities</b>	<b>Closed</b>	<b>03</b>	<b>05/27/2011</b>	<b>06/06/2011</b>	<b>06/09/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>			<b>ANSWER:</b>			
The 14" AWSS line west of MH#701 was found to be constructed thru the roof of the existing 3x5 sewer. Several bends were used in the AWSS line construction and these bends included lugs and tie rods. As a result of the presence of these tie rods and fittings we can now not move MH#701 any further west. To install the new 24" VCP in a straight line (perpendicular to MH wall), and in order to get by the existing PGE MH we will have to pour the pipe wall and 2" of the internal diameter of the pipe into the west wall of MH 701. Please advise on how to proceed.			The 14" AWSS line west of MH#701 was found to be constructed thru the roof of the existing 3x5 sewer. Several bends were used in the AWSS line construction and these bends included lugs and tie rods. As a result of the presence of these tie rods and fittings we can now not move MH#701 any further west. To install the new 24" VCP in a straight line (perpendicular to MH wall), and in order to get by the existing PGE MH we will have to pour the pipe wall and 2" of the internal diameter of the pipe into the west wall of MH 701. Please advise on how to proceed.			
<b>U-0149.1</b>	<b>MH#701 Conflicts with existing utilities</b>	<b>Closed</b>	<b>CR</b>	<b>06/30/2011</b>	<b>07/10/2011</b>	<b>07/01/2011</b>
<b>From:</b> Webcor Construction LP      Jonathan Flaming						
<b>REQUEST:</b>			<b>ANSWER:</b>			
In response to RFI U-0149, please note the following:  M Squared confirms that 6inch deflection of the VCP will allow the 24inch pipe to be clear of the manhole wall.			In response to RFI U-0149, please note the following:  M Squared confirms that 6inch deflection of the VCP will allow the 24inch pipe to be clear of the manhole wall.			
<b>U-0150</b>	<b>Proposed Correction to Field Condition Report 40C</b>	<b>Closed</b>	<b>03</b>	<b>05/31/2011</b>	<b>06/10/2011</b>	<b>06/01/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please see the attached detail from Trinet Construction Inc for their proposed solution to mitigate the incorrect installation of CB203 identified in Field Condition Report 40C.  Please advise if the proposed solution is acceptable.			Please see the attached detail from Trinet Construction Inc for their proposed solution to mitigate the incorrect installation of CB203 identified in Field Condition Report 40C.  Please advise if the proposed solution is acceptable.			
<b>U-0151</b>	<b>Additional Sewer Lateral Connection for 100 1st Street</b>	<b>Closed</b>	<b>03</b>	<b>06/02/2011</b>	<b>06/12/2011</b>	<b>06/08/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>			<b>ANSWER:</b>			



<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
U-0151.1	<b>Additional Sewer Lateral Connection</b>  From: Webcor Construction LP Jonathan Flaming  <b>REQUEST:</b>  This is a follow-up to the request by the Engineer in his response to W/O RFI #U-0151 (Trinet RFI #097) for additional information relating to the 2nd sewer lateral connection for the 100 1st St building. Trinet also clarifies the issue of the existing 4" trap on the line, which was raised in the original RFI.  The sewer lateral is located @ Stn. 7+09 and the invert elevation of the 4" cast iron sewer lateral pipe at face-of-curb is 14.6'. The elevation for the top of the new concrete encased ductbank @ Stn 7+09 is 13.85'. The sewer lateral was therefore not in conflict with the new joint trench utilities.  With regards to the existing 4" trap on the line, Trinet checked with the SF Plumbing department which advised that a 4" cast iron trap was adequate for a 4" sewer lateral. The existing trap was therefore in compliance with the SF plumbing code. Trinet advised Jason Chin of this in the field and he agreed that the trap did not need to be replaced.  The 4" cast iron vent pipe for the trap did not extend to street level but was capped-off approximately 18" below grade. Per field discussions with Jason Chin, Trinet	Closed	CR	06/29/2011	07/09/2011	07/05/2011
	<b>ANSWER:</b>  This is a follow-up to the request by the Engineer in his response to W/O RFI #U-0151 (Trinet RFI #097) for additional information relating to the 2nd sewer lateral connection for the 100 1st St building. Trinet also clarifies the issue of the existing 4" trap on the line, which was raised in the original RFI.  The sewer lateral is located @ Stn. 7+09 and the invert elevation of the 4" cast iron sewer lateral pipe at face-of-curb is 14.6'. The elevation for the top of the new concrete encased ductbank @ Stn 7+09 is 13.85'. The sewer lateral was therefore not in conflict with the new joint trench utilities.  With regards to the existing 4" trap on the line, Trinet checked with the SF Plumbing department which advised that a 4" cast iron trap was adequate for a 4" sewer lateral. The existing trap was therefore in compliance with the SF plumbing code. Trinet advised Jason Chin of this in the field and he agreed that the trap did not need to be replaced.  The 4" cast iron vent pipe for the trap did not extend to street level but was capped-off approximately 18" below grade. Per field discussions with Jason Chin,					



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U-0154	Electrical Service for Street Lights on Natoma	Closed	03	06/08/2011	06/18/2011	09/01/2011
From: Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>  Per Sheet U-1120 the electrical service feeding the street lights on Natoma is to be demolished, see attached. This conduit has been exposed through the investigative trenching process on First, confirmed dead and remove. As a result the existing street lights on Natoma are without power. There are no details provided in the plans for reestablishing power to these street lights now that the demo is complete.  Please advise.						
						<b>ANSWER:</b>  Per Sheet U-1120 the electrical service feeding the street lights on Natoma is to be demolished, see attached. This conduit has been exposed through the investigative trenching process on First, confirmed dead and remove. As a result the existing street lights on Natoma are without power. There are no details provided in the plans for reestablishing power to these street lights now that the demo is complete.  Please advise.
U-0155	AWSS Cast In Place Concrete Testing	Closed	CR	06/20/2011	06/30/2011	06/28/2011
From: Webcor Construction LP      Jonathan Flaming						
<b>REQUEST:</b>  The AWSS Specification section 03300-2, Cast-In-Place Concrete 1.5 C (Quality Assurance) states that the concrete testing will be performed by an agency employed by the TJPA.  However, 03300-10, 3.9 B (Field Quality Control) states that the concrete testing will be performed by the City Testing and Inspection Agency.  Please advise who will be performing the cast in place concrete testing.						
						<b>ANSWER:</b>  The AWSS Specification section 03300-2, Cast-In-Place Concrete 1.5 C (Quality Assurance) states that the concrete testing will be performed by an agency employed by the TJPA.  However, 03300-10, 3.9 B (Field Quality Control) states that the concrete testing will be performed by the City Testing and Inspection Agency.  Please advise who will be performing the cast in place concrete testing.
U-0156	Sink Hole under road base at MH#701	Closed	CR	06/21/2011	07/01/2011	06/22/2011
From: Webcor Construction LP      Jonathan Flaming						
<b>REQUEST:</b>  While excavating for MH#701 M Squared discovered what appears to be a large void under the street base adjacent to the west wall of the MH#701. We estimate the void to be approximately 3' wide and 12' long. This may be a hazard as the street base may collapse at some point in the future.  Please advise how you would like to proceed.						
						<b>ANSWER:</b>  While excavating for MH#701 M Squared discovered what appears to be a large void under the street base adjacent to the west wall of the MH#701. We estimate the void to be approximately 3' wide and 12' long. This may be a hazard as the street base may collapse at some point in the future.  Please advise how you would like to proceed.







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<b>U-0159</b>	<b>Unknown Concrete Structure In Conflict with Sludge Line on Mission</b>	<b>Closed</b>	<b>03</b>	<b>07/28/2011</b>	<b>08/07/2011</b>	<b>08/16/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> While potholing at the locations shown on the attached drawing M Squared discovered what appears to be a concrete wall under the parking strip. M Squared excavated both potholes 7' deep and at that depth the wall appeared to be continuing deeper. This concrete structure is in direct conflict with the proposed location of the new sludge main on Mission Street. The concrete curb on the north side of Mission St also extends 7' deep.  See attached pothole findings.  Please advise on how you would like to proceed.		<b>ANSWER:</b> While potholing at the locations shown on the attached drawing M Squared discovered what appears to be a concrete wall under the parking strip. M Squared excavated both potholes 7' deep and at that depth the wall appeared to be continuing deeper. This concrete structure is in direct conflict with the proposed location of the new sludge main on Mission Street. The concrete curb on the north side of Mission St also extends 7' deep.  See attached pothole findings.  Please advise on how you would like to proceed.				
<b>U-0159.1</b>	<b>Conflict with Sludge Line Conflict on Mission</b>	<b>Closed</b>	<b>03</b>	<b>08/26/2011</b>	<b>09/05/2011</b>	<b>09/13/2011</b>
<b>From:</b> Webcor Construction LP      Jacob Giannandrea						
<b>REQUEST:</b> In response to RFI U-159. See attached pothole findings from remaining potholes on Mission street. Also included is pothole data for Sta 17+28 and Sta 17+50.		<b>ANSWER:</b> In response to RFI U-159. See attached pothole findings from remaining potholes on Mission street. Also included is pothole data for Sta 17+28 and Sta 17+50.				
<b>U-0159.2</b>	<b>Unknown Concrete Structure Sludge Line Conflict</b>	<b>Closed</b>	<b>03</b>	<b>09/15/2011</b>	<b>09/15/2011</b>	<b>09/21/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> In response to RFI U-159.1 There is not adequate space between the face of curb and the unknown concrete structure in order for a welder to be able to weld the bells of each piece of pipe. Please advise on how to proceed.		<b>ANSWER:</b> In response to RFI U-159.1 There is not adequate space between the face of curb and the unknown concrete structure in order for a welder to be able to weld the bells of each piece of pipe. Please advise on how to proceed.				
<b>U-0160</b>	<b>Location of Existing Sludge Force Main on Beale Street</b>	<b>Closed</b>	<b>03</b>	<b>07/29/2011</b>	<b>08/08/2011</b>	<b>08/02/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> M Squared has potholed for the sludge line on Mission		<b>ANSWER:</b> M Squared has potholed for the sludge line on Mission				



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U-0160.1	<p>Location of FM on Beale Street</p> <p>From: Webcor Construction LP Jonathan Flaming</p> <p><b>REQUEST:</b></p> <p>Per response to RFI U-0160 M Squared continued its potholing at Sta 7+08 on Beale Street. M Squared potholed 7' long x 4' wide and 8' deep and M Squared was still unable to determine the location of the existing FM.</p> <p>See attached pothole findings.</p> <p>Please advise how M Squared should proceed.</p>	Closed	CR	08/05/2011	08/05/2011	08/09/2011
U-0160.2	<p>Location of FM on Beale Street</p> <p>From: Webcor Construction LP Jonathan Flaming</p> <p><b>REQUEST:</b></p> <p>M Squared potholed the location of the existing FM to the limits in the drawing provided in the response to RFI U-0160.1. M Squared located the FM within this pothole.</p> <p>See attached pothole findings.</p> <p>Please direct M Squared how to proceed.</p>	Closed	CR	08/11/2011	08/21/2011	08/24/2011



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<b>U-0161</b>	<b>Unknown Concrete Structure in Investigative Trench</b>	<b>Closed</b>	<b>03</b>	<b>07/29/2011</b>	<b>08/08/2011</b>	<b>08/01/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> <p>M Squared discovered an obstruction in the Beale Street investigative trench on station 2+55 approximately 25' west of centerline. The obstruction appears to be a 2'-3' thick concrete wall starting directly below the street base and extending down to an unknown depth. M Squared began demoing the obstruction yesterday believing it was part of a concrete encased PG&amp;E trench. It is now known it is not part of any duct package. Please advise on how you would like to proceed.</p>						<b>ANSWER:</b> <p>M Squared discovered an obstruction in the Beale Street investigative trench on station 2+55 approximately 25' west of centerline. The obstruction appears to be a 2'-3' thick concrete wall starting directly below the street base and extending down to an unknown depth. M Squared began demoing the obstruction yesterday believing it was part of a concrete encased PG&amp;E trench. It is now known it is not part of any duct package. Please advise on how you would like to proceed.</p>
<b>U-0162</b>	<b>Manhole #602 Orientation</b>	<b>Closed</b>	<b>CR</b>	<b>08/03/2011</b>	<b>08/13/2011</b>	<b>08/09/2011</b>
<b>From:</b> Webcor Construction LP      Jonathan Flaming						
<b>REQUEST:</b> <p>The PG&amp;E manhole at Station 2+55 is actually further south than is shown on the drawings. As a result of this the new water main on Natoma Street was installed in a different alignment than shown on the drawings. In order to excavate and shore for the new Manhole #602, without damaging the new water main M Squared will have to install the manhole at a different alignment than what is shown on the plans. M Squared will maintain the correct internal manhole dimensions per DPW standard drawings.  Please confirm this is acceptable.</p>						<b>ANSWER:</b> <p>The PG&amp;E manhole at Station 2+55 is actually further south than is shown on the drawings. As a result of this the new water main on Natoma Street was installed in a different alignment than shown on the drawings. In order to excavate and shore for the new Manhole #602, without damaging the new water main M Squared will have to install the manhole at a different alignment than what is shown on the plans. M Squared will maintain the correct internal manhole dimensions per DPW standard drawings.  Please confirm this is acceptable.</p>
<b>U-0163</b>	<b>Utilities Demolition Plan</b>	<b>Closed</b>	<b>CR</b>	<b>08/04/2011</b>	<b>08/14/2011</b>	<b>08/24/2011</b>
<b>From:</b> Webcor Construction LP      Jonathan Flaming						
<b>REQUEST:</b> <p>The submittal TG04.4 - UG1020-024100B01 Utilities Demolition Plan was returned to M Squared marked "Revise &amp; Resubmit". The review note was: Please provide demo and sequencing plan per specification 02 41 00 Part 1.3A.  M Squared is unable to acquire the necessary utility abandonment schedules from the utility companies concerned.</p>						<b>ANSWER:</b> <p>The submittal TG04.4 - UG1020-024100B01 Utilities Demolition Plan was returned to M Squared marked "Revise &amp; Resubmit". The review note was: Please provide demo and sequencing plan per specification 02 41 00 Part 1.3A.  M Squared is unable to acquire the necessary utility abandonment schedules from the utility companies concerned.</p>



<b><u>Number</u></b>	<b><u>Subject</u></b>	<b><u>Status</u></b>	<b><u>Choice Required</u></b>	<b><u>Date Created</u></b>	<b><u>Date Required</u></b>	<b><u>Date Answered</u></b>
U-0164	<b>Beale Investigative Trench Limits</b>  From: Webcor Construction LP Jonathan Flaming  <b>REQUEST:</b>  Sheet U-1008 shows the limits of the investigative trench on Beale Street (south of Mission St) to be 56' in total. 41.1' from center going west and 14.9' from center going east. By going 14.9' from center with the eastern portion of the investigative trench M Squared will not encompass the existing water line and the existing AWSS line as they are outside the limits of the 14.9'.  Please direct M Squared how to proceed.	Closed	CR	08/09/2011	08/19/2011	08/10/2011
U-0165	<b>Sewer Lateral to 92 Natoma</b>  From: Webcor Construction LP Jonathan Flaming  <b>REQUEST:</b>  While installing the new sewer on Natoma Street from 2nd to the shoring wall M Squared noticed that the sewer lateral to 92 Natoma is a new VCP lateral and has been installed in the last 12 months. The contract drawings show M Squared replacing all sewer laterals on Natoma from 2nd to the shoring wall, however this lateral appears like it does not require replacing. Jason Chin (BCM) has been made aware of this issue.  Please confirm it is acceptable to leave this lateral in place and perform permanent connection to the new 24" VCP main.	Closed	CR	08/09/2011	08/19/2011	08/10/2011
U-0166	<b>Broken Culvert Pipe Encountered in Utility Demolition Trench on Fremont St.</b>	Closed	CR	08/19/2011	08/29/2011	08/24/2011





<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
<b>U-0169</b>	<b>CB#703 Location</b>	<b>Closed</b>	<b>03</b>	<b>09/01/2011</b>	<b>09/01/2011</b>	<b>09/07/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>  See attached photo showing conflict with location of new CB#703 and unknown underground concrete structures. They appear to be the same structures discovered in the investigative trenches on Beale Street.  Please confirm that it is acceptable to put the new CB in the same location as the existing CB which has been removed.		<b>ANSWER:</b>  See attached photo showing conflict with location of new CB#703 and unknown underground concrete structures. They appear to be the same structures discovered in the investigative trenches on Beale Street.  Please confirm that it is acceptable to put the new CB in the same location as the existing CB which has been removed.				
<b>U-0169.1</b>	<b>CB#703 Location</b>	<b>Closed</b>	<b>03</b>	<b>11/15/2011</b>	<b>11/25/2011</b>	<b>11/23/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>  - CB#703 was constructed in the location of the existing catch basin. - See attached profile with culvert elevations. Culvert was installed deeper as several utilities were lower than shown on the drawings. - Per M Squared's response to comments made in the RFI #U-0181, one of the duct banks shown on the drawings could not be located and was not as shown on the drawings. The alignment of the other duct bank is also different than what is shown on the drawings. (See attached) The depth of this duct bank at the point where M Squared capped it (3' south of the unknown concrete structure) was 6' 8" to the top. Its location/alignment beyond that point are unknown.		<b>ANSWER:</b>  - CB#703 was constructed in the location of the existing catch basin. - See attached profile with culvert elevations. Culvert was installed deeper as several utilities were lower than shown on the drawings. - Per M Squared's response to comments made in the RFI #U-0181, one of the duct banks shown on the drawings could not be located and was not as shown on the drawings. The alignment of the other duct bank is also different than what is shown on the drawings. (See attached) The depth of this duct bank at the point where M Squared capped it (3' south of the unknown concrete structure) was 6' 8" to the top. Its location/alignment beyond that point are unknown.				
<b>U-0170</b>	<b>Duct bank Demo on Natoma</b>	<b>Closed</b>	<b>03</b>	<b>09/15/2011</b>	<b>09/25/2011</b>	<b>09/23/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>  M Squared has determined in the field that the duct bank highlighted which is to be demolished, is in fact underneath the curb and gutter. In order to demolish it per the plans M Squared will have to remove the curb and gutter and possibly a portion of sidewalk. See attached.  Please confirm whether you would like the duct bank removed and repour the curb and gutter after demo, or		<b>ANSWER:</b>  M Squared has determined in the field that the duct bank highlighted which is to be demolished, is in fact underneath the curb and gutter. In order to demolish it per the plans M Squared will have to remove the curb and gutter and possibly a portion of sidewalk. See attached.  Please confirm whether you would like the duct bank				





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U-0170.1	<b>Duct Bank Demo on Natoma</b>  From: Webcor Construction LP      Colin Azevedo  <b>REQUEST:</b> In response to RFI #U-0170, see attached photos. Approx 20' of curb and gutter to be repaired. Sidewalk remained undamaged and does not require repair. Please advise if M Squared is to repair this portion of curb and gutter.	Closed	03	09/21/2011	10/01/2011	10/05/2011
U-0170.2	<b>Duct bank Demo on Natoma</b>  From: Webcor Construction LP      Colin Azevedo  <b>REQUEST:</b> M Squard has reviewed their photo logs and were unable to locate any photos showing the ducktbank running under the curb and gutter. M Squared will proceed with providing a credit per CR U-027.	Closed	03	11/18/2011	11/28/2011	12/01/2011
U-0171	<b>AWSS Ductile Iron Pipe</b>  From: Webcor Construction LP      Colin Azevedo  <b>REQUEST:</b> Please confirm that it is acceptable to use non-gauged ductile iron pipe for the AWSS system.	Closed	03	09/15/2011	09/25/2011	09/19/2011
U-0172	<b>City Furnished Gate Valves</b>  From: Webcor Construction LP      Colin Azevedo  <b>REQUEST:</b> Specifications direct the contractor to provide a clear distance between the pipe flanges that consists of the gate valves laying length plus ½" not including the thickness of	Closed	03	09/20/2011	09/30/2011	10/05/2011





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U-0173	<p>the gaskets to be installed. In order to do this M Squared will need the dimensions of all City furnished gate valves. Please provide cut sheets for all valves provided by SFWD for this project.</p> <p><b>Valve control panel pick-up</b></p> <p><b>From:</b> Webcor Construction LP                      Colin Azevedo</p> <p><b>REQUEST:</b></p> <p>M Squared's supplier, Control Systems West, have been coordinating with SFWD regarding which of the City's panels will be used for the TG04.2 project. Tom Reid with SFWD has designated 3 panels to be used for this project. These panels are to be picked up at SFWD, transported to Control Systems West for testing, programming etc and then returned to the job for use at 3 of the valve locations. As the panels have been selected M Squared would like to begin the process of getting the panels to their supplier so they can begin the work.</p> <p>Please provide the name and contact information for the person with whom M Squared can coordinate the pick up of the 3 units.</p>	Closed	03	09/24/2011	10/04/2011	10/05/2011
						<p><b>ANSWER:</b></p> <p>M Squared's supplier, Control Systems West, have been coordinating with SFWD regarding which of the City's panels will be used for the TG04.2 project. Tom Reid with SFWD has designated 3 panels to be used for this project. These panels are to be picked up at SFWD, transported to Control Systems West for testing, programming etc and then returned to the job for use at 3 of the valve locations. As the panels have been selected M Squared would like to begin the process of getting the panels to their supplier so they can begin the work.</p> <p>Please provide the name and contact information for the person with whom M Squared can coordinate the pick up of the 3 units.</p>



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<b>U-0174</b>	<b>AWSS Antenna location at Location 1</b>	<b>Closed</b>	<b>03</b>	<b>09/27/2011</b>	<b>10/07/2011</b>	<b>10/11/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
On drawing MA-20 regarding location 1 the antenna is shown to be mounted on a street light. However, on drawing MA-29 the same antenna is shown to be mounted on the enclosure. Early conversations between Dick Borders (Control Systems West) and Kenny Chin (DPW) confirm that mounting the antenna on the enclosure is the preferred option.  Please confirm the antenna mounting location.		On drawing MA-20 regarding location 1 the antenna is shown to be mounted on a street light. However, on drawing MA-29 the same antenna is shown to be mounted on the enclosure. Early conversations between Dick Borders (Control Systems West) and Kenny Chin (DPW) confirm that mounting the antenna on the enclosure is the preferred option.  Please confirm the antenna mounting location.				
<b>U-0175</b>	<b>Sludge line layout</b>	<b>Closed</b>	<b>03</b>	<b>09/27/2011</b>	<b>10/07/2011</b>	<b>11/08/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
The 12" sludge line cannot be installed along Mission Street as shown on the revised drawings due to the elevation and location of existing utilities and other unknown subsurface obstacles. Please see attached pothole information. Please advise how you would like to proceed.		The 12" sludge line cannot be installed along Mission Street as shown on the revised drawings due to the elevation and location of existing utilities and other unknown subsurface obstacles. Please see attached pothole information. Please advise how you would like to proceed.				
<b>U-0176</b>	<b>AWSS Conflict @ Location 7</b>	<b>Closed</b>	<b>03</b>	<b>09/28/2011</b>	<b>09/28/2011</b>	<b>10/17/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Due to the location of existing utilities it will not be possible to install the AWSS valve vault at the location shown on sheet MA 18 of the AWSS drawings. See attached pothole drawings from 09/26/11 and 09/27/11. Please advise how you would like to proceed.		Due to the location of existing utilities it will not be possible to install the AWSS valve vault at the location shown on sheet MA 18 of the AWSS drawings. See attached pothole drawings from 09/26/11 and 09/27/11. Please advise how you would like to proceed.				
<b>U-0176.1</b>	<b>AWSS Conflicts at Location #7</b>	<b>Closed</b>	<b>03</b>	<b>11/18/2011</b>	<b>11/28/2011</b>	<b>11/21/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Per the response to RFI #U-0176 a field meeting was attended by Michael Smith and M Squared.		Per the response to RFI #U-0176 a field meeting was attended by Michael Smith and M Squared.				



<b><i>Number</i></b>	<b><i>Subject</i></b>	<b><i>Status</i></b>	<b><i>Choice Required</i></b>	<b><i>Date Created</i></b>	<b><i>Date Required</i></b>	<b><i>Date Answered</i></b>
	<p>M Squared received direction to perform additional potholes further west of First St on Howard St. Please see attached pothole findings. Please advise how you would like to proceed.</p>					<p>M Squared received direction to perform additional potholes further west of First St on Howard St. Please see attached pothole findings. Please advise how you would like to proceed.</p>
<b>U-0176.2</b>	<b>AWSS Conflicts @ Location 7</b>	<b>Closed</b>	<b>03</b>	<b>01/18/2012</b>	<b>01/28/2012</b>	<b>02/16/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>						<b>ANSWER:</b>
<p>Per response to RFI#U-0176.1 M Squared performed additional potholing at Location 7.</p> <p>Please see the attached pothole findings.</p> <p>Please advise how you would like to proceed.</p> <p>Note: The 4" Unknown Utility was confirmed to be an abandoned PG&amp;E gas main. On 1/10/12 PG&amp;E drilled the line and confirmed it to be abandoned.</p>						<p>Per response to RFI#U-0176.1 M Squared performed additional potholing at Location 7.</p> <p>Please see the attached pothole findings.</p> <p>Please advise how you would like to proceed.</p> <p>Note: The 4" Unknown Utility was confirmed to be an abandoned PG&amp;E gas main. On 1/10/12 PG&amp;E drilled the line and confirmed it to be abandoned.</p>
<b>U-0177</b>	<b>Ductbank Demo on Fremont St</b>	<b>Closed</b>	<b>03</b>	<b>10/04/2011</b>	<b>10/14/2011</b>	<b>10/10/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>						<b>ANSWER:</b>
<p>See attached sketch.</p> <p>The duct bank shown on Fremont Street to be demolished is in fact underneath the curb and gutter and portion of the sidewalk on Fremont St.</p> <p>In order for M Squared to remove this duct bank it will require us to close the west sidewalk on Fremont St, demo and remove the sidewalk, remove the ductbank and then replace the sidewalk.</p> <p>Currently the east sidewalk is closed also due to BBI activity.</p> <p>Please advise how you would like to proceed.</p>						<p>See attached sketch.</p> <p>The duct bank shown on Fremont Street to be demolished is in fact underneath the curb and gutter and portion of the sidewalk on Fremont St.</p> <p>In order for M Squared to remove this duct bank it will require us to close the west sidewalk on Fremont St, demo and remove the sidewalk, remove the ductbank and then replace the sidewalk.</p> <p>Currently the east sidewalk is closed also due to BBI activity.</p> <p>Please advise how you would like to proceed.</p>

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U-0178	Sludge line layout on Mission between Beale and Main	Closed	03	10/04/2011	10/04/2011	11/08/2011
<div> <div> <b>From:</b> Webcor Construction LP           Colin Azevedo         </div> <div> <b>REQUEST:</b>            Continued potholing on Mission Street between Beale and Main has revealed additional grade conflicts on the proposed alignment for the new 12" steel sludge line. Some of the utilities are not as shown on the drawings nor marked in the field by USAN. See attached sketches.             Please advise if M Squared is to continue potholing on Mission Street as it may be necessary to excavate the entire length of the trench between Beale and Main to locate and map all conflicts.         </div> <div> <b>ANSWER:</b>            Continued potholing on Mission Street between Beale and Main has revealed additional grade conflicts on the proposed alignment for the new 12" steel sludge line. Some of the utilities are not as shown on the drawings nor marked in the field by USAN. See attached sketches.             Please advise if M Squared is to continue potholing on Mission Street as it may be necessary to excavate the entire length of the trench between Beale and Main to locate and map all conflicts.         </div> </div>						
U-0179	AWSS Main line conflicts at Location 7	Closed	03	10/05/2011	10/15/2011	11/21/2011
<div> <div> <b>From:</b> Webcor Construction LP           Colin Azevedo         </div> <div> <b>REQUEST:</b>            Some of the existing utilities are not shown on the drawings and have been installed on top of the existing 12" AWSS line. Due to the proximity and volume of these utilities it is not possible to even hand excavate down to the existing AWSS line to verify its location and depth. Please see attached pothole information. Please advise.         </div> <div> <b>ANSWER:</b>            Some of the existing utilities are not shown on the drawings and have been installed on top of the existing 12" AWSS line. Due to the proximity and volume of these utilities it is not possible to even hand excavate down to the existing AWSS line to verify its location and depth. Please see attached pothole information. Please advise.         </div> </div>						
U-0180	Conflict with CB 305	Closed	CR	10/10/2011	10/20/2011	10/17/2011
<div> <div> <b>From:</b> Webcor Construction LP           Colin Azevedo         </div> <div> <b>REQUEST:</b>            While excavating to install CB305 M Squared encountered a large unknown concrete structure. The concrete structure is in conflict with CB305. CB305 cannot be installed as planned. See attached photo.             Tsu-Ling with AECOM and Alberto with SFDPW reviewed the situation in the field and agreed the solution was to salvage the existing CB where CB 305 was to be installed. This work was performed on 10/7/2011 under the inspection of SFDPW.         </div> <div> <b>ANSWER:</b>            While excavating to install CB305 M Squared encountered a large unknown concrete structure. The concrete structure is in conflict with CB305. CB305 cannot be installed as planned. See attached photo.             Tsu-Ling with AECOM and Alberto with SFDPW reviewed the situation in the field and agreed the solution was to salvage the existing CB where CB 305 was to be installed. This work was performed on 10/7/2011 under the inspection of SFDPW.         </div> </div>						



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	Please confirm.				Please confirm.	
<b>U-0181</b>	<b>Unknown subsurface structure on Beale</b>	<b>Closed</b>	<b>03</b>	<b>10/13/2011</b>	<b>10/23/2011</b>	<b>10/24/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> During M Squared's demo work on the West side of Beale Street at Sta 4+70 they uncovered an unknown subsurface structure. This structure appears to be an abandoned vault that has been filled with concrete. Please see attached photo. M Squared ceased work on the removal of the six 6" electric duct banks 6' south of this structure. If they are to continue with the removal of this abandoned duct bank per sheet U-1125 of the contract drawings they will be forced to remove the subsurface structure. Please advise.					<b>ANSWER:</b> During M Squared's demo work on the West side of Beale Street at Sta 4+70 they uncovered an unknown subsurface structure. This structure appears to be an abandoned vault that has been filled with concrete. Please see attached photo. M Squared ceased work on the removal of the six 6" electric duct banks 6' south of this structure. If they are to continue with the removal of this abandoned duct bank per sheet U-1125 of the contract drawings they will be forced to remove the subsurface structure. Please advise.	
<b>U-0181.1</b>	<b>Unknown subsurface structure at 301 Mission</b>	<b>Closed</b>	<b>03</b>	<b>11/18/2011</b>	<b>11/28/2011</b>	<b>11/23/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> See attached information as requested in response to RFI #U-0181.					<b>ANSWER:</b> See attached information as requested in response to RFI #U-0181.	
<b>U-0182</b>	<b>AWSS Conflict with AT&amp;T Vault at Location 2</b>	<b>Closed</b>	<b>03</b>	<b>10/24/2011</b>	<b>11/03/2011</b>	<b>11/21/2011</b>
<b>From:</b> Webcor/Obayashi Joint Venture      Jason Dunne						
<b>REQUEST:</b> On the north east side of the Mission Street and 2nd intersection the existing AWSS line is running through the floor of the AT&T vault. The removal of the existing 12" pipe and installation of the new 16" AWSS pipe will require the floor vault to be demolished and re-poured.  Please provide a detail for this work or a new alignment for the AWSS line so as to avoid this vault.					<b>ANSWER:</b> On the north east side of the Mission Street and 2nd intersection the existing AWSS line is running through the floor of the AT&T vault. The removal of the existing 12" pipe and installation of the new 16" AWSS pipe will require the floor vault to be demolished and re-poured.  Please provide a detail for this work or a new alignment for the AWSS line so as to avoid this vault.	



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<b>U-0182.1</b>	<b>AWSS Conflict with AT&amp;T Vault at Location 2</b>	<b>Closed</b>	<b>03</b>	<b>03/28/2012</b>	<b>04/07/2012</b>	<b>05/16/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>  The sketch provided in response to RFI U-0182 does not provide adequate information to perform additional potholing. Please provide additional information.					<b>ANSWER:</b>  The sketch provided in response to RFI U-0182 does not provide adequate information to perform additional potholing. Please provide additional information.	
<b>U-0182.2</b>	<b>AWSS - Conflict with AT&amp;T Vault at Location 2</b>	<b>Closed</b>	<b>CR</b>	<b>07/31/2012</b>	<b>07/31/2012</b>	<b>08/14/2012</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Per the response to U-0182.1, M Squared potholed the locations shown. See attached pothole data. <ul style="list-style-type: none"><li>- The pothole 24' north of Mission appears to have a substructure underneath PGE duct banks.</li><li>- The pothole 12' north of Mission St had several utilities in them that have since been confirmed abandoned.</li></ul>					<b>ANSWER:</b>  Per the response to U-0182.1, M Squared potholed the locations shown. See attached pothole data. <ul style="list-style-type: none"><li>- The pothole 24' north of Mission appears to have a substructure underneath PGE duct banks.</li><li>- The pothole 12' north of Mission St had several utilities in them that have since been confirmed abandoned.</li></ul>	
<b>U-0182.3</b>	<b>AWSS - Design Route at 2nd Street Intersection</b>	<b>Closed</b>	<b>CR</b>	<b>02/06/2013</b>	<b>02/16/2013</b>	<b>02/28/2013</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Per the response to RFI U-0182.2, M Squared has field verified a new alignment for the 16" AWSS at 2nd & Mission St. (See attached drawing).  Due to several PG&E conflicts this is the only available route capable of accepting a 16" pipe; M Squared is unable to locate an alignment per the sketch attached to the response to RFI U-0182.2. By proceeding with this alignment M Squared will again return the AWSS pipe through the structure of an AT&T vault and a PG&E Vault. It does not appear from our field work that there are other options for a workaround.  Based on information M Squared currently have attained from the trenching; restraining each joint, per the original contract will require the following: <ul style="list-style-type: none"><li>- 4 additional 16-inch 45deg bends</li><li>- 2 additional 16-inch 90deg bends</li><li>- 1 additional 16-inch bell collar</li><li>- 15 additional stop collars</li></ul>					<b>ANSWER:</b>  Per the response to RFI U-0182.2, M Squared has field verified a new alignment for the 16" AWSS at 2nd & Mission St. (See attached drawing).  Due to several PG&E conflicts this is the only available route capable of accepting a 16" pipe; M Squared is unable to locate an alignment per the sketch attached to the response to RFI U-0182.2. By proceeding with this alignment M Squared will again return the AWSS pipe through the structure of an AT&T vault and a PG&E Vault. It does not appear from our field work that there are other options for a workaround.  Based on information M Squared currently have attained from the trenching; restraining each joint, per the original contract will require the following: <ul style="list-style-type: none"><li>- 4 additional 16-inch 45deg bends</li></ul>	



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	<p>- 4 additional kickers/thrust blocks.</p> <p>Please confirm the proposed route and additional fittings and restraints are acceptable.</p>					<p>- 2 additional 16-inch 90deg bends - 1 additional 16-inch bell collar - 15 additional stop collars - 4 additional kickers/thrust blocks.</p> <p>Please confirm the proposed route and additional fittings and restraints are acceptable.</p>
U-0182.4	<b>AWSS - Final Design Route and Additional Fittings List at 2nd Street Intersection</b>	Closed	CR	03/14/2013	03/24/2013	03/21/2013
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>						<b>ANSWER:</b>
Refer to drawing MA-3 and MA-13						Refer to drawing MA-3 and MA-13
Please refer to previous RFI 182 series for history.						Please refer to previous RFI 182 series for history.
As M Squared must connect to an existing 16" line at 2nd & Mission Street, M Squared performed additional trenching which has now opened up the possibility of a different and more straight forward alignment for 2nd Street piping.						As M Squared must connect to an existing 16" line at 2nd & Mission Street, M Squared performed additional trenching which has now opened up the possibility of a different and more straight forward alignment for 2nd Street piping.
This new alignment shall replace the alignment sent in the previous RFI-0182.3.						This new alignment shall replace the alignment sent in the previous RFI-0182.3.
1. Please confirm the new alignment shown in the attached M Squared sketch SK-008.3 is acceptable. 2. Please confirm where the 16" to 12" reducer is to be located. The location of this reducer will decide whether M Squared will need to purchase two (2) more 16" 45-deg elbows or 12" 45-deg elbows.						1. Please confirm the new alignment shown in the attached M Squared sketch SK-008.3 is acceptable. 2. Please confirm where the 16" to 12" reducer is to be located. The location of this reducer will decide whether M Squared will need to purchase two (2) more 16" 45-deg elbows or 12" 45-deg elbows.



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<b>U-0183</b>	<b>AWSS Valve Vault Conflict at Location 1</b>	<b>Closed</b>	<b>03</b>	<b>10/24/2011</b>	<b>11/03/2011</b>	<b>10/26/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
The proposed valve vault at location 1 cannot be installed as per the plans due to utility conflicts encountered during potholing. See attached pothole info. These utilities are not shown on the contract drawings. Please advise.		The proposed valve vault at location 1 cannot be installed as per the plans due to utility conflicts encountered during potholing. See attached pothole info. These utilities are not shown on the contract drawings. Please advise.				
<b>U-0183.1</b>	<b>AWSS Valve Vault Conflict at Location 1</b>	<b>Closed</b>	<b>03</b>	<b>11/16/2011</b>	<b>11/26/2011</b>	<b>11/18/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Per the response to RFI#U-0183 a site visit was held with SFDPW and SFWD on 11/2/2011 to review the conflicts at location 1. Please provide direction based on this meeting.		Per the response to RFI#U-0183 a site visit was held with SFDPW and SFWD on 11/2/2011 to review the conflicts at location 1. Please provide direction based on this meeting.				
<b>U-0183.2</b>	<b>AWSS Valve Vault Location 1</b>	<b>Closed</b>	<b>CR</b>	<b>12/02/2011</b>	<b>12/12/2011</b>	<b>12/15/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please see the attached letter regarding the response to RFI#U-0183.1.		Please see the attached letter regarding the response to RFI#U-0183.1.				
Please provide direction.		Please provide direction.				
<b>U-0183.3</b>	<b>Valve Vault Conflict at Location 1</b>	<b>Closed</b>	<b>03</b>	<b>01/23/2012</b>	<b>02/02/2012</b>	<b>02/08/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Per the response to RFI #U-0183.2, M Squared Construction performed further potholing on the valve vault location on Market Street. Please see attached findings of these potholes. Please advise on how you would like M Squared to proceed with the vault construction/installation.		Per the response to RFI #U-0183.2, M Squared Construction performed further potholing on the valve vault location on Market Street. Please see attached findings of these potholes. Please advise on how you would like M Squared to proceed with the vault construction/installation.				
<b>U-0183.4</b>	<b>AWSS - Valve Vault Conflict at Location 1 for Trade Package</b>	<b>Closed</b>	<b>03</b>	<b>07/05/2012</b>	<b>07/15/2012</b>	<b>07/19/2012</b>









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	the enclosure per the specifications.				install drain rock beneath the enclosure per the specifications.	
<b>U-0187.1</b>	<b>Conflicts with Controller Cabinet Foundation and Battery Enclosure at Location # Closed</b>		<b>CR</b>	<b>12/02/2011</b>	<b>12/12/2011</b>	<b>12/15/2011</b>
<b>From:</b> Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b> Please see the attached letter regarding the response to RFI#U-0187.  Please provide direction.					<b>ANSWER:</b> Please see the attached letter regarding the response to RFI#U-0187.  Please provide direction.	
<b>U-0187.2</b>	<b>Conflicts with Controller Cabinet and Battery @ Location 1</b>	<b>Closed</b>	<b>03</b>	<b>01/23/2012</b>	<b>02/02/2012</b>	<b>03/21/2012</b>
<b>From:</b> Webcor Construction LP                      Colin Azevedo						
<b>REQUEST:</b> In response to RFI # U-0187.1 (Revised Response to RFI# U-0187 ON 12/14/11) - See attached pothole data from additional potholing at this location. - During initial discussions with MCI/Verizon M Squared informed them of the intent to install units on their utility. They requested a letter from the owner highlighting the intent. Please confirm if it is acceptable to install a unit on their utility. Please provide direction on the locations of the battery vault and controller cabinet taking into consideration all current utilities in place.					<b>ANSWER:</b> In response to RFI # U-0187.1 (Revised Response to RFI# U-0187 ON 12/14/11) - See attached pothole data from additional potholing at this location. - During initial discussions with MCI/Verizon M Squared informed them of the intent to install units on their utility. They requested a letter from the owner highlighting the intent. Please confirm if it is acceptable to install a unit on their utility. Please provide direction on the locations of the battery vault and controller cabinet taking into consideration all current utilities in place.	



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<b>U-0188</b>	<b>Control Stations on AWSS Drawings</b>	<b>Closed</b>	<b>03</b>	<b>11/18/2011</b>	<b>11/28/2011</b>	<b>11/21/2011</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> At present M Squared has set up control points along Mission Street. These stations were based on a continuation of survey points used on Mission Street for the TG04.6-Sludge Line Project. The City designed AWSS Drawings do not have these stations on them. Please provide an updated set of AWSS Drawings with the project stations marked on them so it will allow M Squared to accurately document field conditions and as built the necessary information.						<b>ANSWER:</b> At present M Squared has set up control points along Mission Street. These stations were based on a continuation of survey points used on Mission Street for the TG04.6-Sludge Line Project. The City designed AWSS Drawings do not have these stations on them. Please provide an updated set of AWSS Drawings with the project stations marked on them so it will allow M Squared to accurately document field conditions and as built the necessary information.
<b>U-0189</b>	<b>First &amp; Howard Utility Conflicts, Location 7 Complete Pothole Data</b>	<b>Closed</b>	<b>03</b>	<b>12/02/2011</b>	<b>12/12/2011</b>	<b>07/03/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> While potholes #2 & #3 have been addressed in a previous RFI (RFI#U-0176), other potholes carried out in Location 7 exposed various utilities that are not shown on the contract documents. Other utilities were not in the locations indicated on the contract documents.  See attached pothole data from potholes #1 through #11 at location 7.  Please clarify if the utilities will be removed, protected in place or relocated.						<b>ANSWER:</b> While potholes #2 & #3 have been addressed in a previous RFI (RFI#U-0176), other potholes carried out in Location 7 exposed various utilities that are not shown on the contract documents. Other utilities were not in the locations indicated on the contract documents.  See attached pothole data from potholes #1 through #11 at location 7.  Please clarify if the utilities will be removed, protected in place or relocated.
<b>U-0190</b>	<b>Fire Hydrant Location on Mission @ First</b>	<b>Closed</b>	<b>03</b>	<b>01/10/2012</b>	<b>01/20/2012</b>	<b>01/19/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b> While potholing for the new Hydrant and associated piping in the sidewalk on Mission Street (see attached), M Squared's crews damaged the roof of the basement to Portico Restaurant, 88 First Street (see attached photos). This basement structure was not noted on the plans and is a differing site condition.  The roof of the basement will now need to be repaired. Please provide direction and repair details for this work.						<b>ANSWER:</b> While potholing for the new Hydrant and associated piping in the sidewalk on Mission Street (see attached), M Squared's crews damaged the roof of the basement to Portico Restaurant, 88 First Street (see attached photos). This basement structure was not noted on the plans and is a differing site condition.  The roof of the basement will now need to be repaired. Please provide direction and repair details for this

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<p><b>From:</b> Webcor Construction LP                      Colin Azevedo</p> <p><b>REQUEST:</b></p> <p>In order for the controller enclosures for the motorized gate valves at Location #1, #2 and #7 to be operational a power source will need to be provided at each enclosure location.</p> <p>Please confirm that the owner has applied to PG&amp;E for the power sources at these locations and advise on the status of these connections.</p>						<p><b>ANSWER:</b></p> <p>In order for the controller enclosures for the motorized gate valves at Location #1, #2 and #7 to be operational a power source will need to be provided at each enclosure location.</p> <p>Please confirm that the owner has applied to PG&amp;E for the power sources at these locations and advise on the status of these connections.</p>
<b>U-0191.1</b>	<b>Power Source at Location #1, #2 &amp; #7</b>	<b>Closed</b>	<b>03</b>	<b>03/21/2012</b>	<b>03/31/2012</b>	<b>05/01/2012</b>
<p><b>From:</b> Webcor Construction LP                      Colin Azevedo</p> <p><b>REQUEST:</b></p> <p>Recent meeting on the AWSS project resulted in the response to RFI#U-0191 being revised to include a procedure to be followed once the controller cabinets were ready to accept power. However, what was sent in the revised response was a new scope of work followed by the mentioned procedure.</p> <p>The contract drawings show M Squared's work beginning at pull boxes and going to the controllers. M Squared's interpretation of the drawings sent in the revised response to RFI#U-0191 is the scope of work that goes from the pull boxes to PG&amp;E manholes. This is unclear because the PG&amp;E drawings are not comparable with the contract drawings.</p> <p>Please clarify the intent and scope of the PG&amp;E drawings. Please clarify how the PG&amp;E drawings correlate with the contract drawings.</p>						<p><b>ANSWER:</b></p> <p>Recent meeting on the AWSS project resulted in the response to RFI#U-0191 being revised to include a procedure to be followed once the controller cabinets were ready to accept power. However, what was sent in the revised response was a new scope of work followed by the mentioned procedure.</p> <p>The contract drawings show M Squared's work beginning at pull boxes and going to the controllers. M Squared's interpretation of the drawings sent in the revised response to RFI#U-0191 is the scope of work that goes from the pull boxes to PG&amp;E manholes. This is unclear because the PG&amp;E drawings are not comparable with the contract drawings.</p> <p>Please clarify the intent and scope of the PG&amp;E drawings. Please clarify how the PG&amp;E drawings correlate with the contract drawings.</p>



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U-0191.2	Amperes Interrupting Capacity (AIC) at AWSS Location #1 (Market St.)	Closed	CR	05/23/2012	06/02/2012	06/21/2012
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to RFI U0191.1 and the attached drawings MA-1, MA-29 and MA-31.  1. As per response to RFI U-0191.1, the SFDPW-Bureau of Engineering sketches and letter for the AIC only addresses the motorized gate valve number 21 at Location #7. As new power service will be required at gate valve number 2, Location 1, please provide an AIC letter for this location.  2. Please provide a conformed drawing of the the PG&E clarification sketches provided in RFI U-0191.1 by revising the drawing sheet MA-29 and MA-31, respectively. It is unclear from the PG&E sketches whether the scope from the original contract drawings (MA-29 and MA-31) have changed.						<b>ANSWER:</b>  Please refer to RFI U0191.1 and the attached drawings MA-1, MA-29 and MA-31.  1. As per response to RFI U-0191.1, the SFDPW-Bureau of Engineering sketches and letter for the AIC only addresses the motorized gate valve number 21 at Location #7. As new power service will be required at gate valve number 2, Location 1, please provide an AIC letter for this location.  2. Please provide a conformed drawing of the the PG&E clarification sketches provided in RFI U-0191.1 by revising the drawing sheet MA-29 and MA-31, respectively. It is unclear from the PG&E sketches whether the scope from the original contract drawings (MA-29 and MA-31) have changed.
U-0191.3	Amperes Interrupting Capacity (AIC) at AWSS Location #1 (Market St.)	Closed	CR	06/28/2012	07/08/2012	07/16/2012
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  The response to RFI #U-0191.2 does not answer the question posed in the RFI.  As mentioned in the previous RFI there appears to be a difference in the PG&E drawings provided in the original response and the contract drawings.  See attached M Squared's interpretation of these PG&E drawings. Please confirm if this interpretation is correct.						<b>ANSWER:</b>  The response to RFI #U-0191.2 does not answer the question posed in the RFI.  As mentioned in the previous RFI there appears to be a difference in the PG&E drawings provided in the original response and the contract drawings.  See attached M Squared's interpretation of these PG&E drawings. Please confirm if this interpretation is correct.
U-0192	AWSS Strong Backs	Closed	03	01/18/2012	01/28/2012	02/08/2012
From: Webcor Construction LP Colin Azevedo						
<b>REQUEST:</b>  Current project drawings show that this project requires two (2) 14" Strong Backs and two (2) 10" Strong Backs to be used at different locations. Olympic Foundry does not produce strong backs and were unable to include them in the order to M Squared. M						<b>ANSWER:</b>  Current project drawings show that this project requires two (2) 14" Strong Backs and two (2) 10" Strong Backs to be used at different locations. Olympic Foundry does not produce strong backs and were unable to include them in the order to M



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U-0193	<b>2nd to 1st St - Various Conflicts</b>  <b>From:</b> Webcor Construction LP      Colin Azevedo  <b>REQUEST:</b> See attached sheet which details the conditions discovered in the potholing operations between 2nd Street and 1st Street. Please use Submittal TG04.2-024.1 for reference. Please provide direction on how to proceed at each location.	Closed	CR	03/08/2012	03/18/2012	03/21/2012
U-0194	<b>AWSS Strong Back Dimensions</b>  <b>From:</b> Webcor Construction LP      Colin Azevedo  <b>REQUEST:</b> On the detail for the strong backs on the San Francisco Standard AWSS Plans M Squared has discovered an error in the dimensions for the 14" strong back. Dimension C (outside diameter) is smaller than dimension B (inside diameter). See attached.  M Squared believes the OD should be 27.37". Please confirm.	Closed	03	03/13/2012	03/23/2012	03/21/2012





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<b>U-0195</b>	<b>Parking Sensors on Mission</b>	<b>Closed</b>	<b>03</b>	<b>03/13/2012</b>	<b>03/23/2012</b>	<b>04/16/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>			<b>ANSWER:</b>			
M Squared has discovered that either SF Park or MUNI have installed what appear to be sensors in the street surface along Mission Street. See photo attached.			M Squared has discovered that either SF Park or MUNI have installed what appear to be sensors in the street surface along Mission Street. See photo attached.			
They existing between Fremont and Beale in particular.			They existing between Fremont and Beale in particular.			
As the AWSS line is installed along Mission St from 2nd to Main these sensors will be in conflict. Please confirm these sensors will be removed prior to trenching.			As the AWSS line is installed along Mission St from 2nd to Main these sensors will be in conflict. Please confirm these sensors will be removed prior to trenching.			
<b>U-0196</b>	<b>AWSS Pipe Bedding Material</b>	<b>Closed</b>	<b>03</b>	<b>04/02/2012</b>	<b>04/12/2012</b>	<b>04/09/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Section 02225-2 2.2 specifies that the bedding material for the new AWSS piping shall be crushed rock, however section 02723-18 2.12 contradicts this by specifying the bedding shall be pea gravel. Please clarify.			Section 02225-2 2.2 specifies that the bedding material for the new AWSS piping shall be crushed rock, however section 02723-18 2.12 contradicts this by specifying the bedding shall be pea gravel. Please clarify.			
<b>U-0197</b>	<b>AWSS/PG&amp;E Phase 2 Duct Conflict</b>	<b>Closed</b>	<b>03</b>	<b>04/05/2012</b>	<b>04/16/2012</b>	<b>04/16/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>			<b>ANSWER:</b>			
See attached photo. M Squared discovered a conflict on 4/4/12 at 11.10am while excavating to remove the existing AWSS Main at Howard and First.			See attached photo. M Squared discovered a conflict on 4/4/12 at 11.10am while excavating to remove the existing AWSS Main at Howard and First.			
PGE's new Phase 2 duct package is sitting directly on top of the existing AWSS main at First and Howard intersection. The top and sides of the duct bank are encased in concrete however the PVC conduits are not encased on the bottom and the PVC Conduits are currently touching the AWSS Main at this location.			PGE's new Phase 2 duct package is sitting directly on top of the existing AWSS main at First and Howard intersection. The top and sides of the duct bank are encased in concrete however the PVC conduits are not encased on the bottom and the PVC Conduits are currently touching the AWSS Main at this location.			
As a result M Squared is unable to remove the existing AWSS main from this point east.			As a result M Squared is unable to remove the existing AWSS main from this point east.			



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	<p>Please advise on how you would like to proceed.</p>					<p>Please advise on how you would like to proceed.</p>
U-0197.1	AWSS/PG&E Phase 2 Duct Conflict Location 7	Closed	03	04/16/2012	04/26/2012	04/17/2012
<p><b>From:</b> Webcor Construction LP                      Colin Azevedo</p>						
<p><b>REQUEST:</b></p> <p>The Phase 2 PG&amp;E plans only provide minimum depths and clearances. It appears the Phase 2 ducts were installed in accordance with the minimum depth requirement but not the minimum clearance requirement. Please confirm this with PG&amp;E.</p> <p>Regardless, the AWSS main can not be reinstalled per plan and maintain minimum clearance required in the AWSS specification. Please advise how M Squared is to proceed.</p>			<p><b>ANSWER:</b></p> <p>The Phase 2 PG&amp;E plans only provide minimum depths and clearances. It appears the Phase 2 ducts were installed in accordance with the minimum depth requirement but not the minimum clearance requirement. Please confirm this with PG&amp;E.</p> <p>Regardless, the AWSS main can not be reinstalled per plan and maintain minimum clearance required in the AWSS specification. Please advise how M Squared is to proceed.</p>			
U-0197.2	AWSS-PG&E Phase 2 Duct Conflict	Closed	03	04/23/2012	05/03/2012	05/02/2012
<p><b>From:</b> Webcor Construction LP                      Colin Azevedo</p>						
<p><b>REQUEST:</b></p> <p>Through detailed analysis and discussions with PG&amp;E during the weekly AWSS coordination meetings it has been determined that it would be infeasible to relocate the PG&amp;E duct bank as requested in option one in the response to RFI#U-0197.1.</p> <p>Please provide details for realigning the AWSS main referenced in option two in the response to RFI#U-0197.1.</p>			<p><b>ANSWER:</b></p> <p>Through detailed analysis and discussions with PG&amp;E during the weekly AWSS coordination meetings it has been determined that it would be infeasible to relocate the PG&amp;E duct bank as requested in option one in the response to RFI#U-0197.1.</p> <p>Please provide details for realigning the AWSS main referenced in option two in the response to RFI#U-0197.1.</p>			



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<b>U-0198</b>	<b>Vault Drainage</b>	<b>Closed</b>	<b>03</b>	<b>04/09/2012</b>	<b>04/09/2012</b>	<b>04/16/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
1. On sheet MA-26 the 1" discharge piping inside the manhole is labeled as stainless steel in the detail drawings but is described as type K copper tube in the manhole construction note #7. Please confirm what type of material is required. 2. Spec Section 02728-23 Paragraph E. calls for the use of ball float valves as shown on the construction drawings. However the float valves are not shown on the drawings. Please confirm if these ball float valves are required.		1. On sheet MA-26 the 1" discharge piping inside the manhole is labeled as stainless steel in the detail drawings but is described as type K copper tube in the manhole construction note #7. Please confirm what type of material is required. 2. Spec Section 02728-23 Paragraph E. calls for the use of ball float valves as shown on the construction drawings. However the float valves are not shown on the drawings. Please confirm if these ball float valves are required.				
<b>U-0199</b>	<b>PG&amp;E Vault Conflict with North East Tie In @ Location 7</b>	<b>Closed</b>	<b>03</b>	<b>04/16/2012</b>	<b>04/26/2012</b>	<b>04/23/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Today while setting up to remove and cast the new lead joint at the North East tie in at location 7 it was discovered that the existing PG&E vault adjacent to the tie in is too close and E. Mitchell would not be able to properly caulk the lead joint. Please advise how M Squared is to proceed.		Today while setting up to remove and cast the new lead joint at the North East tie in at location 7 it was discovered that the existing PG&E vault adjacent to the tie in is too close and E. Mitchell would not be able to properly caulk the lead joint. Please advise how M Squared is to proceed.				
<b>U-0200</b>	<b>AT&amp;T Vault Conflict at Location 7</b>	<b>Closed</b>	<b>03</b>	<b>04/16/2012</b>	<b>04/26/2012</b>	<b>04/23/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
It has been discovered that the AT&T vault near the North West tie in of Location 7 is in conflict with the new AWSS pipe and tie rods to be installed at this location.  Please advise how M Squared is to proceed.		It has been discovered that the AT&T vault near the North West tie in of Location 7 is in conflict with the new AWSS pipe and tie rods to be installed at this location.  Please advise how M Squared is to proceed.				
<b>U-0200.1</b>	<b>AT&amp;T Vault Conflict at Location 7</b>	<b>Closed</b>	<b>03</b>	<b>04/24/2012</b>	<b>05/04/2012</b>	<b>04/24/2012</b>
<b>From:</b> Webcor Construction LP      Colin Azevedo						
<b>REQUEST:</b>		<b>ANSWER:</b>				
The response to RFI#U-0200 did not properly document the coordination efforts and course of action. Please		The response to RFI#U-0200 did not properly document the coordination efforts and course of				



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	provide a revised response.					action. Please provide a revised response.
	See attached email chain for additional information.					See attached email chain for additional information.
<b>U-0201</b>	<b>AWSS - Countersunk Bolts in 14-Inch Ductile Iron Pipe Strong Back Plate</b>	<b>Closed</b>	<b>CR</b>	<b>05/04/2012</b>	<b>05/14/2012</b>	<b>05/08/2012</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> Please reference attached excerpt from the AWSS STANDARD DRAWING III, drawing No. AWSS 3.  The sizing chart for 14" diameter pipe require the use of Strong Back Type B. The Type B Strong Back configuration requires the use of a countersunk bolt and nut to adjoin connecting DI pipe. The countersunk bolts are a special order product and will have to be fabricated specifically for each piece.  Please confirm it is acceptable to use the typical 316 Stainless Steel bolt and nut without the countersink, similar to what is used and shown in Type A for all 14" diameter DI pipe.						<b>ANSWER:</b> Please reference attached excerpt from the AWSS STANDARD DRAWING III, drawing No. AWSS 3.  The sizing chart for 14" diameter pipe require the use of Strong Back Type B. The Type B Strong Back configuration requires the use of a countersunk bolt and nut to adjoin connecting DI pipe. The countersunk bolts are a special order product and will have to be fabricated specifically for each piece.  Please confirm it is acceptable to use the typical 316 Stainless Steel bolt and nut without the countersink, similar to what is used and shown in Type A for all 14" diameter DI pipe.

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U-0204	SLUDGE LINE - Compaction Method for Trade Package TG04.66	Closed	CR	06/22/2012	07/02/2012	06/22/2012
<div> <div> <b>From:</b> Webcor Construction LP           Jackson Tukuafu         </div> </div>						
<b>REQUEST:</b> <p>Specification section 33 34 10 (3.1, C<sub>2</sub>]7) forbids the use of flooding or jetting in order to gain the necessary levels of compaction in the HDPE pipe trench. However due to the amount of utilities and duct packages in the trenches it will not be possible to gain the necessary levels of compaction under and around these utilities by utilizing the methods referenced in the specifications. By not gaining the necessary compaction around utilities it is possible that voids will occur over time causing the utility to be come unsupported and the street surface to sink.</p> <p>M Squared is requesting the use of jetting (as described in Section 703.08 of the City and County of San Francisco Standard Specifications) as a method to gain the necessary levels of compaction for the AWSS trenches.</p> <p>Jetting has previously been utilized as a successful method of gaining compaction levels on several other Transit Center Utility Relocation packages (see RFI0203).</p> <p>Please confirm that this proposed method is acceptable for use on this trade package. If not, please provide an alternative method for gaining the necessary compaction.</p>			<b>ANSWER:</b> <p>Specification section 33 34 10 (3.1, C<sub>2</sub>]7) forbids the use of flooding or jetting in order to gain the necessary levels of compaction in the HDPE pipe trench. However due to the amount of utilities and duct packages in the trenches it will not be possible to gain the necessary levels of compaction under and around these utilities by utilizing the methods referenced in the specifications. By not gaining the necessary compaction around utilities it is possible that voids will occur over time causing the utility to be come unsupported and the street surface to sink.</p> <p>M Squared is requesting the use of jetting (as described in Section 703.08 of the City and County of San Francisco Standard Specifications) as a method to gain the necessary levels of compaction for the AWSS trenches.</p> <p>Jetting has previously been utilized as a successful method of gaining compaction levels on several other Transit Center Utility Relocation packages (see RFI0203).</p> <p>Please confirm that this proposed method is acceptable for use on this trade package. If not, please provide an alternative method for gaining the necessary compaction.</p>			









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<div><div><b>REQUEST:</b><p>While excavating to expose the existing AWSS Main on Market Street M Squared's crew discovered that a portion of the existing cast iron main had already been abandoned in place. They then discovered a ductile iron main that is running parallel to the cast iron pipe.</p><p>The ductile iron main is the portion of pipe that is live and this is the line we should now be connecting to in order to proceed with the work. See attached photos. Please note that additional costs will be incurred, as a result of this unforeseen condition.</p><p>Please advise on how M Squared is to proceed.</p></div><div><b>ANSWER:</b><p>While excavating to expose the existing AWSS Main on Market Street M Squared's crew discovered that a portion of the existing cast iron main had already been abandoned in place. They then discovered a ductile iron main that is running parallel to the cast iron pipe.</p><p>The ductile iron main is the portion of pipe that is live and this is the line we should now be connecting to in order to proceed with the work. See attached photos. Please note that additional costs will be incurred, as a result of this unforeseen condition.</p><p>Please advise on how M Squared is to proceed.</p></div></div>						
U-0208	AWSS - Clearance Issues with Domestic Water Line on Market Street	Closed	CR	07/10/2012	07/20/2012	07/11/2012
<div><div><b>From:</b> Webcor Construction LP      Jackson Tukuafu</div><div><b>REQUEST:</b><p>While excavating west of the gate valve vault location on Market Street M Squard's crew discovered an 8-inch cast iron water line sitting on top of the existing AWSS main to be removed. This 8-inch line also appears to be leaking slightly.</p><p>1. As a result of this line M Squared is unable to install the new AWSS with the necessary clearances. Aside from the clearance issues M Squared can no longer install the 14-inch reducer where it is required. M Squared will be able to relocate the reducer which will then require a longer spool piece.</p><p>Please advise how M Squared is to proceed.</p><p>2. This 8-inch line also has three concrete kickers on the pipe that make it impossible to install the pipe and fittings at this vault location. Please confirm that it is acceptable to remove these kickers temporarily, as they are already restrained with tie rods, for construction purposes. The kickers can be reinstalled once the work in this location has been completed.</p></div><div><b>ANSWER:</b><p>While excavating west of the gate valve vault location on Market Street M Squard's crew discovered an 8-inch cast iron water line sitting on top of the existing AWSS main to be removed. This 8-inch line also appears to be leaking slightly.</p><p>1. As a result of this line M Squared is unable to install the new AWSS with the necessary clearances. Aside from the clearance issues M Squared can no longer install the 14-inch reducer where it is required. M Squared will be able to relocate the reducer which will then require a longer spool piece.</p><p>Please advise how M Squared is to proceed.</p><p>2. This 8-inch line also has three concrete kickers on the pipe that make it impossible to install the pipe and fittings at this vault location. Please confirm that it is acceptable to remove these kickers temporarily, as they are already restrained with tie rods, for construction purposes. The kickers can be reinstalled once the work in this location has been completed.</p></div></div>						
U-0208.01	AWSS - Clearance Issues with Domestic Water Line on Market Street	Closed	CR	07/24/2012	08/03/2012	08/03/2012



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U-0210	AWSS - 12" Water Conflict at 1st and Mission Street	Closed	CR	07/26/2012	08/05/2012	08/10/2012
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> <p>While performing the preliminary excavation across 1st and Mission street Intersection, M Squared's crew exposed a 12" water line that is running on top of the AWSS line for approx half of the intersection. Due to other utilities being present we are unable to excavate down to the AWSS main.</p> <p>M Squared met with SFWD crews on site and they have confirmed that the line is active, despite them agreeing with M Squared that the line sounded very hollow (an indication that it may be dead)</p> <p>M Squared believes that despite the presence of many unknown utilities they will still be able to remove and replace the existing AWSS main if this 12" water line can be abandoned or relocated.</p> <p>Please advise on how M Squared is to proceed.</p>		<b>ANSWER:</b> <p>While performing the preliminary excavation across 1st and Mission street Intersection, M Squared's crew exposed a 12" water line that is running on top of the AWSS line for approx half of the intersection. Due to other utilities being present we are unable to excavate down to the AWSS main.</p> <p>M Squared met with SFWD crews on site and they have confirmed that the line is active, despite them agreeing with M Squared that the line sounded very hollow (an indication that it may be dead)</p> <p>M Squared believes that despite the presence of many unknown utilities they will still be able to remove and replace the existing AWSS main if this 12" water line can be abandoned or relocated.</p> <p>Please advise on how M Squared is to proceed.</p>				
U-0211	AWSS - Valve Vault at Sta 9+05	Closed	CR	08/06/2012	08/16/2012	08/14/2012
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> <p>Please refer to that attached photo and schematic of current condition.</p> <p>M Squared has identified the space at Sta 9+05 as the only viable location for the gate valve in that area. However several utilities remain in conflict with this location:</p> <ul style="list-style-type: none"><li>- The MCI lines are plastic and the correspondence has already began with MCI to move these lines 2' south during AWSS Main installation</li><li>- The 3 x 2" Steel Electrical lines have been confirmed active by PGE representatives</li><li>- All remaining lines are unknown.</li></ul> <p>Please advise on how you would like M Squared to proceed.</p>		<b>ANSWER:</b> <p>Please refer to that attached photo and schematic of current condition.</p> <p>M Squared has identified the space at Sta 9+05 as the only viable location for the gate valve in that area. However several utilities remain in conflict with this location:</p> <ul style="list-style-type: none"><li>- The MCI lines are plastic and the correspondence has already began with MCI to move these lines 2' south during AWSS Main installation</li><li>- The 3 x 2" Steel Electrical lines have been confirmed active by PGE representatives</li><li>- All remaining lines are unknown.</li></ul> <p>Please advise on how you would like M Squared to proceed.</p>				
U-0212	AWSS - Various Conflicts - Sta 9+12 to PG&E Vault	Closed	CR	08/07/2012	08/17/2012	08/30/2012



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<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  While performing preliminary trenching across 1st & Mission Street Intersection, M Squared's crew discovered many unknown and unmarked utilities. See attached photos.  The presence of these unknown utilities will greatly impact the ability to install shoring and install full pieces of pipe. Please Identify the utilities in this section and determine which can be removed in order for M Squared to proceed.						
			<b>ANSWER:</b>  While performing preliminary trenching across 1st & Mission Street Intersection, M Squared's crew discovered many unknown and unmarked utilities. See attached photos.  The presence of these unknown utilities will greatly impact the ability to install shoring and install full pieces of pipe. Please Identify the utilities in this section and determine which can be removed in order for M Squared to proceed.			
<b>U-0213</b>	<b>AWSS - Antenna At Location #7</b>	<b>Closed</b>	<b>CR</b>	<b>09/11/2012</b>	<b>09/21/2012</b>	<b>09/12/2012</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Sheet MA - 22 of the contract drawings shows the antenna for location #7 being mounted on the existing street light pole.  Sheet MA - 31 shows that the antenna is mounted on an antenna pole in the sidewalk.  Please clarify where the antenna pole is to be located.						
			<b>ANSWER:</b>  Sheet MA - 22 of the contract drawings shows the antenna for location #7 being mounted on the existing street light pole.  Sheet MA - 31 shows that the antenna is mounted on an antenna pole in the sidewalk.  Please clarify where the antenna pole is to be located.			
<hr/>						
<b>U-0213.01</b>	<b>AWSS - Antenna at Location #7</b>	<b>Closed</b>	<b>CR</b>	<b>09/13/2012</b>	<b>09/23/2012</b>	<b>09/20/2012</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to RFI U-0213 and SFDPW drawing File No. 87,208 and 87,212.  As no detail for the antenna pole foundation is provided in the contract documents, please advise if the standard detail for San Francisco Light Poles is an acceptable foundation of the antenna pole indicated on drawing MA-31.						
			<b>ANSWER:</b>  Please refer to RFI U-0213 and SFDPW drawing File No. 87,208 and 87,212.  As no detail for the antenna pole foundation is provided in the contract documents, please advise if the standard detail for San Francisco Light Poles is an acceptable foundation of the antenna pole indicated on drawing MA-31.			
<hr/>						
<b>U-0214</b>	<b>SLUDGE LINE - Air Release Valve at Sta 17+25</b>	<b>Closed</b>	<b>CR</b>	<b>09/28/2012</b>	<b>10/08/2012</b>	<b>11/09/2012</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						





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U-0215	AWSS - Hetch Hetchy Duct Bank Conflict	Closed	CR	09/28/2012	10/08/2012	10/12/2012
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  At Sta 2+40 on Mission St (Anthony St intersection) the existing AWSS Main runs through a Hetch Hetchy duct bank. There are several concrete encased ducts on top of the AWSS Main and several concrete encased ducts under the AWSS main.  On Friday 28th September, M Squared met with MUNI Underground Services and they have requested that the AWSS be abandon 1-ft on each side of the duct bank and install the new AWSS Main over or under this Hetch Hetchy duct bank.  Please advise how you would like M Squared to proceed with this conflict.		<b>ANSWER:</b>  At Sta 2+40 on Mission St (Anthony St intersection) the existing AWSS Main runs through a Hetch Hetchy duct bank. There are several concrete encased ducts on top of the AWSS Main and several concrete encased ducts under the AWSS main.  On Friday 28th September, M Squared met with MUNI Underground Services and they have requested that the AWSS be abandon 1-ft on each side of the duct bank and install the new AWSS Main over or under this Hetch Hetchy duct bank.  Please advise how you would like M Squared to proceed with this conflict.				
U-0216	AWSS - Gate Valve at Station 1+09	Closed	CR	10/04/2012	10/14/2012	10/15/2012
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached drawing MA-13.  Due to the location of existing utilities M Squared is unable to install the gate valve at Sta 0+90, as shown on sheet MA-13. Please confirm it is acceptable for M Squared to install the valve at Sta 1+90. M Squared has confirmed there are no conflicts at Sta 1+90.		<b>ANSWER:</b>  Please refer to attached drawing MA-13.  Due to the location of existing utilities M Squared is unable to install the gate valve at Sta 0+90, as shown on sheet MA-13. Please confirm it is acceptable for M Squared to install the valve at Sta 1+90. M Squared has confirmed there are no conflicts at Sta 1+90.				
U-0217	AWSS - 16" Gate Valve at Sta 5+00	Closed	03	10/12/2012	10/22/2012	10/15/2012
From: Webcor Construction LP Robert Kjome						
<b>REQUEST:</b>  Drawing Reference: MA-14  Please confirm that the 16" gate valve at Sta 5+00 can be deleted and is not required.		<b>ANSWER:</b>  Drawing Reference: MA-14  Please confirm that the 16" gate valve at Sta 5+00 can be deleted and is not required.				
U-0218	AWSS - PG&E Duct Bank Conflict at Sta.6+05 to Sta. 6+25	Closed	CR	11/06/2012	11/06/2012	11/15/2012
From: Webcor Construction LP Jackson Tukuafu						



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	<div><div>REQUEST:</div><div>Between Sta 6+05 and Sta 6+25 there is a PGE duct bank sitting on top of the AWSS pipe; as a result, M Squared is unable to install the new AWSS main at this location. See attached photo. The pipe cannot be lowered due to the AWSS penetrating PGE Vault #1302. In order for M Squared to be able to install the new AWSS main through PGE vault #1302, the PGE duct bank needs to be raised up.</div><div>Please advise.</div></div>					
	<div><div>ANSWER:</div><div>Between Sta 6+05 and Sta 6+25 there is a PGE duct bank sitting on top of the AWSS pipe; as a result, M Squared is unable to install the new AWSS main at this location. See attached photo. The pipe cannot be lowered due to the AWSS penetrating PGE Vault #1302. In order for M Squared to be able to install the new AWSS main through PGE vault #1302, the PGE duct bank needs to be raised up.</div><div>Please advise.</div></div>					
U-0219	AWSS - PG&E Vault #1313 Conflict with 4x4 Support Post	Closed	CR	11/06/2012	11/16/2012	11/29/2012
	From: Webcor Construction LP Jackson Tukuafu					
	<div><div>REQUEST:</div><div>On 10/26, PGE completed work on Vault #1313 on Mission Street. The existing AWSS pipe has been removed and M Squared is ready to install the new AWSS Main per the attached sketch (current condition).</div><div>In order for M Squared to install the AWSS pipe, the five 4"x4" supports installed by ARB crews require removal. As a result, a portion of the vault wall will overhanging the pipe, with no support.</div><div>Please advise if this is acceptable.</div></div>					
	<div><div>ANSWER:</div><div>On 10/26, PGE completed work on Vault #1313 on Mission Street. The existing AWSS pipe has been removed and M Squared is ready to install the new AWSS Main per the attached sketch (current condition).</div><div>In order for M Squared to install the AWSS pipe, the five 4"x4" supports installed by ARB crews require removal. As a result, a portion of the vault wall will overhanging the pipe, with no support.</div><div>Please advise if this is acceptable.</div></div>					





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U-0220	AWSS - MultiQuip Sump Pump	Closed	CR	01/23/2013	02/02/2013	01/29/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:		ANSWER:				
Please refer to the attached excerpt from spec section 02728 AWSS Motorized Gate Valve Equipment and product data for the MultiQuip Sump Pump: ST2037.		Please refer to the attached excerpt from spec section 02728 AWSS Motorized Gate Valve Equipment and product data for the MultiQuip Sump Pump: ST2037.				
As per coordination between Aidan Foley and Michael Smith, please confirm the attached MultiQuip Sump Pump: ST2037 is an acceptable alternate to the specified manufacturer Flygt, Model 2610 in specification section 02728- 2.13,A.		As per coordination between Aidan Foley and Michael Smith, please confirm the attached MultiQuip Sump Pump: ST2037 is an acceptable alternate to the specified manufacturer Flygt, Model 2610 in specification section 02728- 2.13,A.				
Please note the MultiQuip Sump Pump: ST2037 is being submitted for approval in WOJV submittal package TG04.2-031 - AWSS - MultiQuip Sump Pump.		Please note the MultiQuip Sump Pump: ST2037 is being submitted for approval in WOJV submittal package TG04.2-031 - AWSS - MultiQuip Sump Pump.				
U-0221	AWSS - Pipe Joints in Utility Vaults	Closed	01	01/31/2013	02/10/2013	02/06/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Per recent field direction provided by the City inspector to M Squared Construction, where possible no joints are permitted inside utility vaults (i.e PGE, ATT)		Per recent field direction provided by the City inspector to M Squared Construction, where possible no joints are permitted inside utility vaults (i.e PGE, ATT)				
This will require an additional restraint joint at each vault location.		This will require an additional restraint joint at each vault location.				
Please confirm this is the intent.		Please confirm this is the intent.				
U-0222	AWSS - Flanged Spools for Hydrants	Closed	01	01/31/2013	02/10/2013	02/06/2013
From: Webcor Construction LP Robert Kjome						
REQUEST:		ANSWER:				
Reference Drawings: MA-14 & MA-15		Reference Drawings: MA-14 & MA-15				
Hydrant at Sta 6+30 Contract drawings show the 45deg bend being connected directly to the rolled down tee. However the hydrant lateral is much lower than the main and it will not be possible to connect them directly together.		Hydrant at Sta 6+30 Contract drawings show the 45deg bend being connected directly to the rolled down tee. However the hydrant lateral is much lower than the main and it will not be possible to connect them directly together.				



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	<p>Hydrant at Sta 9+00 Due to the changes per RFI U-190 M Squared are to install the new fire hydrant in the same location as the existing, in the breezeway. As a result the new hydrant lateral will be higher than the newly installed main (the grade of the main being dictated by various utility conflicts).</p> <p>Please confirm M squared's suggested mediation is how M squared is to proceed</p>					
U-0223	<p><b>AWSS - Electrical Sevice at 2nd Street Intersection</b></p> <p>From: Webcor Construction LP Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>The contract drawings show M Squared replacing the existing 10" gate valve on Mission at 2nd St with a new 16" gate valve. Due to a PG&amp;E conflict M Squared will have to now move the valve location north onto 2nd Street.</p> <p>In moving the vault M Squared will now have to relocate the existing electrical service to the new vault location. M Squared will need the service disconnected so that all existing electrical cable and conduits can be removed.</p> <p>Once the new vault has been constructed M Squared can reestablish the service to the new vault location. These were previously thought to be street lighting conduits as mentioned in RFI U-182.2 and will need to be removed for the construction of the new vault anyway.</p> <p>The service is currently the responsibility of the SFPUC and PG&amp;E have indicated that any impact to the service needs to be handled by the SFPUC and not M Squared.</p> <p>Please advise on how to proceed</p>	Closed	01	02/06/2013	02/16/2013	05/20/2013
	<p><b>ANSWER:</b></p> <p>The contract drawings show M Squared replacing the existing 10" gate valve on Mission at 2nd St with a new 16" gate valve. Due to a PG&amp;E conflict M Squared will have to now move the valve location north onto 2nd Street.</p> <p>In moving the vault M Squared will now have to relocate the existing electrical service to the new vault location. M Squared will need the service disconnected so that all existing electrical cable and conduits can be removed.</p> <p>Once the new vault has been constructed M Squared can reestablish the service to the new vault location. These were previously thought to be street lighting conduits as mentioned in RFI U-182.2 and will need to be removed for the construction of the new vault anyway.</p> <p>The service is currently the responsibility of the SFPUC and PG&amp;E have indicated that any impact to the service needs to be handled by the SFPUC and not M Squared.</p> <p>Please advise on how to proceed</p>					



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U-0223.1	AWSS - Electrical Service at 2nd Street	Closed	01	07/17/2013	07/27/2013	07/19/2013
From: M Squared Construction, Inc. Aidan Foley						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Attached Drawing		Reference: Attached Drawing				
Per the response to RFI U-0223 a new electrical service, a new PGE meter pedestal, and a new drain line was to be installed at 2nd and Mission. However the drawing provided in the response showed the old AWSS vault location. See attached drawing prepared by M Squared showing the new vault location.		Per the response to RFI U-0223 a new electrical service, a new PGE meter pedestal, and a new drain line was to be installed at 2nd and Mission. However the drawing provided in the response showed the old AWSS vault location. See attached drawing prepared by M Squared showing the new vault location.				
We have established conduit routes for both the new electrical service to PGE vault #1316 and also the drain line to the catch basin.		We have established conduit routes for both the new electrical service to PGE vault #1316 and also the drain line to the catch basin.				
Please confrim that this acceptable.		Please confrim that this acceptable.				







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U-0226	RFI#U-0226 - AWSS - PG&E Duct Bank at 1st Intersection	Closed	CR	03/11/2013	03/21/2013	03/15/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:  Refer to drawing U-1002, U-2003, MA-15.  M Squared is unable to trench to the connection point as shown on the attached M Squared sketch SK-047.1 and SK-047.2 on First Street due to the presence of two (2) PG&E duct banks in the trench. One duct bank is deeper than the other and is sitting directly on top of the AWSS Pipe that is required to be removed.  Please provide direction to how M Squared will proceed.		ANSWER:  Refer to drawing U-1002, U-2003, MA-15.  M Squared is unable to trench to the connection point as shown on the attached M Squared sketch SK-047.1 and SK-047.2 on First Street due to the presence of two (2) PG&E duct banks in the trench. One duct bank is deeper than the other and is sitting directly on top of the AWSS Pipe that is required to be removed.  Please provide direction to how M Squared will proceed.				
U-0226.1	AWSS - TCG Duct Bank at 1st Street Intersection	Closed	CR	06/25/2013	07/05/2013	07/08/2013
From: Webcor Construction LP Jackson Tukuafu						
REQUEST:  Please refer to response for RFI U-0226.  As per response to RFI U-0226, M Squared is directed to "Remove the existing gate valve and connect to the existing pipe. Install an I-beam behind the 16" Tee...as an alternative restraint system" in order to avoid two PG&E duct banks in conflict with the AWSS. As a result of the I-Beam being installed at this location, a TCG duct bank would need to be moved 2-feet west.  TCG has determined that the duct bank would take several months to re-locate their duct bank. Therefore, TCG has opted to avoid the conflict by pursuing the suggested 22deg bends as an offset from the 16" tee in RFI U-0226.  Please confirm additional restraints are not required at the 22deg bends or provide all necessary restraints required to for the 10" line at this location.		ANSWER:  Please refer to response for RFI U-0226.  As per response to RFI U-0226, M Squared is directed to "Remove the existing gate valve and connect to the existing pipe. Install an I-beam behind the 16" Tee...as an alternative restraint system" in order to avoid two PG&E duct banks in conflict with the AWSS. As a result of the I-Beam being installed at this location, a TCG duct bank would need to be moved 2-feet west.  TCG has determined that the duct bank would take several months to re-locate their duct bank. Therefore, TCG has opted to avoid the conflict by pursuing the suggested 22deg bends as an offset from the 16" tee in RFI U-0226.  Please confirm additional restraints are not required at the 22deg bends or provide all necessary restraints required to for the 10" line at this location.				



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U-0227	AWSS - 2nd Street AWSS Gate Valve Vault	Closed	CR	04/16/2013	04/26/2013	04/22/2013
<div><div>From: Webcor Construction LP</div><div>Jackson Tukuafu</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Refer to drawing MA-13, MA-3, MA-10			Refer to drawing MA-13, MA-3, MA-10			
Due to the presence of several PGE duct banks and the steam line that runs along 2nd Street M Squared feels it will be significantly difficult to modify a precast valve vault to fit into the area designated for the vault. As a result, M Squared proposes to construct a cast in place valve vault as has previously installed and approved on Market Street.			Due to the presence of several PGE duct banks and the steam line that runs along 2nd Street M Squared feels it will be significantly difficult to modify a precast valve vault to fit into the area designated for the vault. As a result, M Squared proposes to construct a cast in place valve vault as has previously installed and approved on Market Street.			
1. Please confirm it is acceptable to install a cast in place vault at this location.			1. Please confirm it is acceptable to install a cast in place vault at this location.			
2. Please advise if rebar detail attached is acceptable for use.			2. Please advise if rebar detail attached is acceptable for use.			









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U-0230	<b>REQUEST:</b>  Reference: Attached Photo  Please confrim that the new 16" AWSS is acceptable to be in the position shown as there is not the required clearance with the PGE vault #1329	Closed	01	07/18/2013	07/28/2013	07/19/2013
	<b>ANSWER:</b>  Reference: Attached Photo  Please confrim that the new 16" AWSS is acceptable to be in the position shown as there is not the required clearance with the PGE vault #1329					
U-0230	<b>AWSS - AWSS Vault at 2nd Street</b>  <b>From:</b> Webcor Construction LP                      Jackson Tukuafu	Closed	01	07/18/2013	07/28/2013	07/19/2013
	<b>REQUEST:</b>  Due to the grade of the 16" AWSS gate valve, combined with the valve actuators the roof of the AWSS valve vault at 2nd Street will not be under the surface of the street. Previous AWSS valve vaults have 2" AC/8" concrete street base on top of the roof of the vault.  If M Squared installs the vault roof and then covers it with 2" AC then there is a danger that future contractors will saw cut through the roof of the vault while cutting out their trenches.  Our suggestion is to pour the vault roof to the same grade as the current street surface on 2nd Street. There does not appear to be any room for adjustment here and we are unaware of any other options in this case.  Please confirm it is acceptable to construct the vault roof in this manner, with a concrete broom finish.					
U-0230		Closed	01	07/18/2013	07/28/2013	07/19/2013
	<b>ANSWER:</b>  Due to the grade of the 16" AWSS gate valve, combined with the valve actuators the roof of the AWSS valve vault at 2nd Street will not be under the surface of the street. Previous AWSS valve vaults have 2" AC/8" concrete street base on top of the roof of the vault.  If M Squared installs the vault roof and then covers it with 2" AC then there is a danger that future contractors will saw cut through the roof of the vault while cutting out their trenches.  Our suggestion is to pour the vault roof to the same grade as the current street surface on 2nd Street. There does not appear to be any room for adjustment here and we are unaware of any other options in this case.  Please confirm it is acceptable to construct the vault roof in this manner, with a concrete broom finish.					



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U-0231	AWSS - Concrete Sampling for Kickers	Closed	01	07/25/2013	08/04/2013	08/02/2013
<div><div><b>From:</b> Webcor Construction LP</div><div>Jackson Tukuafu</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
<p>The contract specifications require concrete sampling of all cast in place concrete on the AWSS project. However in the pre-construction QC meeting the City confirmed that the SFWD Inspector - Dan Helminiak is permitted to inspect all concrete thrust blocks.</p> <p>Due to the small size of the thrust blocks it is not practical for concrete samples to be provided to an inspection agency.</p> <p>Please confirm that per the agreement SFWD inspector can inspect all concrete used in the AWSS thrust blocks and that no concrete sampling is required.</p>			<p>The contract specifications require concrete sampling of all cast in place concrete on the AWSS project. However in the pre-construction QC meeting the City confirmed that the SFWD Inspector - Dan Helminiak is permitted to inspect all concrete thrust blocks.</p> <p>Due to the small size of the thrust blocks it is not practical for concrete samples to be provided to an inspection agency.</p> <p>Please confirm that per the agreement SFWD inspector can inspect all concrete used in the AWSS thrust blocks and that no concrete sampling is required.</p>			

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U-0233	AWSS - 16" GV @ sta 9+00	Closed	01	08/14/2013	08/24/2013	08/14/2013
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Reference: Attached Photos		Reference: Attached Photos				
Technically the 2 operating nuts should be on the same side. That way you can operate the nut on both the valve and the by pass from the 24"x24" valve cover in the street. Now that the nut on the valve is facing a different way there is no possibility that you can access both nuts from the valve cover.		Technically the 2 operating nuts should be on the same side. That way you can operate the nut on both the valve and the by pass from the 24"x24" valve cover in the street. Now that the nut on the valve is facing a different way there is no possibility that you can access both nuts from the valve cover.				
U-0234	AWSS - Valve Vault Wiring Clarification	Closed	CR	10/17/2013	10/27/2013	11/06/2013
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please refer to drawing sheets MA-29, MA-30 and MA-31.		Please refer to drawing sheets MA-29, MA-30 and MA-31.				
The above referenced drawings show conduit and conductors required for vault wiring. The sheet note number 1on these drawings refer to Limitorque drawings. These Limitorque drawings show additional (54 #14 gauge) conductors in each of the three locations.		The above referenced drawings show conduit and conductors required for vault wiring. The sheet note number 1on these drawings refer to Limitorque drawings. These Limitorque drawings show additional (54 #14 gauge) conductors in each of the three locations.				
Please clarify the total number of conductors and corresponding conduits..		Please clarify the total number of conductors and corresponding conduits..				
U-0235	RUP - Missing Fittings at Main Street Intersection per Drawing MA-17	Closed	CR	12/10/2013	12/20/2013	12/23/2013
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Please refer to drawing MA-17 and specification section 00 70 00, 1.05 - B4.		Please refer to drawing MA-17 and specification section 00 70 00, 1.05 - B4.				
Per the General Conditions, 00 70 00,1.05 B4 , the parts list takes precedence over the drawing details. The attached excerpt from drawing MA-17 identifies fittings that are not shown on the material list. Please confirm the following fittings are required to complete the AWSS new install:		Per the General Conditions, 00 70 00,1.05 B4 , the parts list takes precedence over the drawing details. The attached excerpt from drawing MA-17 identifies fittings that are not shown on the material list. Please confirm the following fittings are required to complete the AWSS new install:				



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1. Three (3) 14" stop collar. 2. A 14" bell collar						1. Three (3) 14" stop collar. 2. A 14" bell collar
<b>U-0236</b>	<b>RUP - AWSS Pipe Configuration at PG&amp;E Vault #1722</b>	<b>Closed</b>	<b>CR</b>	<b>12/10/2013</b>	<b>12/20/2013</b>	<b>12/19/2013</b>
<b>From:</b> Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b> See attached sketches.  Due to the proximity of PGE's Vault #1722 to the new AWSS line M Squared believes the following changes are needed to keep all pipe joints and fittings outside the limits of existing utility vaults.  M Squared proposes installing the 14" x 12" reducer further east, until we are outside the limits of the vault. The pipe between the new cross piece and the new reducer will be 14-inch pipe, rather than the 12-inch shown on the plans. M Squared will be able to eliminate the need for the 12-inch sleeve here and tie 12-inch pipe into the existing main from the reducer. All joints will be restrained using stop collars.  Please confirm that this configuration is preferred to in lieu having fittings and joints within the limits of the PGE Vault. Please advise.						<b>ANSWER:</b> See attached sketches.  Due to the proximity of PGE's Vault #1722 to the new AWSS line M Squared believes the following changes are needed to keep all pipe joints and fittings outside the limits of existing utility vaults.  M Squared proposes installing the 14" x 12" reducer further east, until we are outside the limits of the vault. The pipe between the new cross piece and the new reducer will be 14-inch pipe, rather than the 12-inch shown on the plans. M Squared will be able to eliminate the need for the 12-inch sleeve here and tie 12-inch pipe into the existing main from the reducer. All joints will be restrained using stop collars.  Please confirm that this configuration is preferred to in lieu having fittings and joints within the limits of the PGE Vault. Please advise.



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U-0237	RUP - Location of Valve Vault at Main Street Phase	Closed	CR	12/13/2013	12/23/2013	12/19/2013
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to drawing MA-17.  Due to the location of several utilities it is not possible to install the gate valve and valve vault at Sta 19+85 as shown on sheet MA-17. The closest possible location with adequate space for a concrete vault is at Sta 19+50. See attached M Squared sketch SK-TG04.2-059.  Please confirm that this location is acceptable as the location for the gate valve and the valve vault. If this is an acceptable location, please clarify if 2 joints west of the new valve location are required to be restrained.		<b>ANSWER:</b>  Please refer to drawing MA-17.  Due to the location of several utilities it is not possible to install the gate valve and valve vault at Sta 19+85 as shown on sheet MA-17. The closest possible location with adequate space for a concrete vault is at Sta 19+50. See attached M Squared sketch SK-TG04.2-059.  Please confirm that this location is acceptable as the location for the gate valve and the valve vault. If this is an acceptable location, please clarify if 2 joints west of the new valve location are required to be restrained.				
U-0238	RUP - Catch Basin at Sta. 18+75	Closed	CR	12/17/2013	12/27/2013	12/23/2013
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  The existing catch basin at Sta 18+75, mid-block between Main and Beale on Mission Street is 3.5-inches higher than the surrounding concrete and asphalt. The catch basin itself is only 16-inches deep and does not appear to be active on account of the grate being higher than the surrounding areas.  In order for this catch basin to be utilized the grate would need to be dropped approx 5inches, leaving a catch basin less than a foot deep.  Please advise what steps are required to be taken before M Squared restores the concrete bus lane.		<b>ANSWER:</b>  The existing catch basin at Sta 18+75, mid-block between Main and Beale on Mission Street is 3.5-inches higher than the surrounding concrete and asphalt. The catch basin itself is only 16-inches deep and does not appear to be active on account of the grate being higher than the surrounding areas.  In order for this catch basin to be utilized the grate would need to be dropped approx 5inches, leaving a catch basin less than a foot deep.  Please advise what steps are required to be taken before M Squared restores the concrete bus lane.				
U-0238.1	AWSS - Abandoned Catch Basin at Sta. 18+75	Closed	CR	01/07/2014	01/17/2014	02/04/2014
From: Webcor Construction LP Jackson Tukuafu						
<b>REQUEST:</b>  The existing catch basin at Sta 18+75, mid block between Main and Beale on Mission Street is 3.5-inches higher		<b>ANSWER:</b>  The existing catch basin at Sta 18+75, mid block between Main and Beale on Mission Street is 3.5-				



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	<p>than the surrounding concrete and asphalt. The catch basin itself is only 16inches deep and does not appear to be active on account of the grate being higher than the surrounding areas.</p> <p>In order for this catch basin to be utilized the grate would need to be dropped approx. 5-inches, leaving a catch basin less than a foot deep. It is not possible to install a standard SF catch basin is this location and therefore M Squared suggest abandoning this CB. Alternatively, please provide grades for the restoration of the concrete bus lane to create necessary slopes to this catch basin.</p> <p>Please advise.</p>					<p>inches higher than the surrounding concrete and asphalt. The catch basin itself is only 16inches deep and does not appear to be active on account of the grate being higher than the surrounding areas.</p> <p>In order for this catch basin to be utilized the grate would need to be dropped approx. 5-inches, leaving a catch basin less than a foot deep. It is not possible to install a standard SF catch basin is this location and therefore M Squared suggest abandoning this CB. Alternatively, please provide grades for the restoration of the concrete bus lane to create necessary slopes to this catch basin.</p> <p>Please advise.</p>
U-0239	AWSS - The Use of Sand Slurry Backfill at Mission and Main Street Phase	Closed	CR	01/16/2014	01/26/2014	02/04/2014
	<p>From: Webcor Construction LP Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Please refer to drawing MA-17.</p> <p>Due to the high number of utility duct banks on Main Street &amp; Mission Street Intersection M Squared's crews effectively tunneled under sections of the street in order to install the new AWSS main. Where possible M Squared removed the street base and excavated as much as we could. The remainder of the trench was tunneled under duct banks and under the street, with the street base remaining in place.</p> <p>Now that all the AWSS main has been installed M Squared will shortly be faced with backfilling this intersection. There is currently only a small portion of trench that can be backfilled using conventional methods i.e ram compactor etc. The remainder of the trench will also not be suitable for backfilling using the jetting method as there will be no way to compact the area directly underneath the street base.</p> <p>M Squared is requesting permission to create several small holes (approx. 6" dia) in the street base, between some of the utilities and backfill the AWSS trench using a sand cement slurry backfill. See attached submittal</p>					<p><b>ANSWER:</b></p> <p>Please refer to drawing MA-17.</p> <p>Due to the high number of utility duct banks on Main Street &amp; Mission Street Intersection M Squared's crews effectively tunneled under sections of the street in order to install the new AWSS main. Where possible M Squared removed the street base and excavated as much as we could. The remainder of the trench was tunneled under duct banks and under the street, with the street base remaining in place.</p> <p>Now that all the AWSS main has been installed M Squared will shortly be faced with backfilling this intersection. There is currently only a small portion of trench that can be backfilled using conventional methods i.e ram compactor etc. The remainder of the trench will also not be suitable for backfilling using the jetting method as there will be no way to compact the area directly underneath the street base.</p> <p>M Squared is requesting permission to create several small holes (approx. 6" dia) in the street base, between some of the utilities and backfill the AWSS trench</p>





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	<p>sheets. This mix design is effectively sand and water, with just a minor amount of cement included to allow the sand to reach 95% compaction.</p> <p>If this is not acceptable please provide direction.</p>					<p>using a sand cement slurry backfill. See attached submittal sheets. This mix design is effectively sand and water, with just a minor amount of cement included to allow the sand to reach 95% compaction.</p> <p>If this is not acceptable please provide direction.</p>
U-0240	<b>AWSS - Concrete Mix and Slump at Parking Strip Placement</b>	Closed	CR	01/16/2014	01/26/2014	03/05/2014
	<p><b>From:</b> Webcor Construction LP                      Jackson Tukuafu</p> <p><b>REQUEST:</b></p> <p>Prior to replacing the 6ft wide parking strip on Mission Street at 1st Street it became apparent to us that the articulated semi trucks, buses and other larger vehicles would be unable to a make the turn from southbound 1st Street onto Mission Street if the 6ft wide parking strip was barricaded in order to let the concrete set.</p> <p>City standards call for the concrete to be poured with a 4inch slump, and that no traffic drive on the concrete for a period of 10 days. M Squared made the decision to add 2% calcium to the concrete mix to speed up the concrete setting process. M Squared used the same mix design as is used for the street base:</p> <ul style="list-style-type: none"><li>- Bode Concrete Mix Design 604 - sidewalk, curb and gutter and parking strip.</li><li>- Bode Concrete Mix Design 604CC - Street base</li></ul> <p>The only difference between the 2 concrete designs is the added 2% calcium.</p> <p>The concrete was poured with an 8inch slump in order to allow the crew enough time to satisfactorily finish the concrete to the required surface. M Squared acknowledge that this is out of spec, however the concrete still reached over 4000psi, when specs required only 3000psi. M Squared believes that this will be required in the future on other portions of Mission Street on account of Mission St. being MUNI's busiest route. SFMTA have asked that M Squared minimize lane closures where possible.</p> <p>Please confirm that this 8inch slump is acceptable on</p>				<p><b>ANSWER:</b></p> <p>Prior to replacing the 6ft wide parking strip on Mission Street at 1st Street it became apparent to us that the articulated semi trucks, buses and other larger vehicles would be unable to a make the turn from southbound 1st Street onto Mission Street if the 6ft wide parking strip was barricaded in order to let the concrete set.</p> <p>City standards call for the concrete to be poured with a 4inch slump, and that no traffic drive on the concrete for a period of 10 days. M Squared made the decision to add 2% calcium to the concrete mix to speed up the concrete setting process. M Squared used the same mix design as is used for the street base:</p> <ul style="list-style-type: none"><li>- Bode Concrete Mix Design 604 - sidewalk, curb and gutter and parking strip.</li><li>- Bode Concrete Mix Design 604CC - Street base</li></ul> <p>The only difference between the 2 concrete designs is the added 2% calcium.</p> <p>The concrete was poured with an 8inch slump in order to allow the crew enough time to satisfactorily finish the concrete to the required surface. M Squared acknowledge that this is out of spec, however the concrete still reached over 4000psi, when specs required only 3000psi. M Squared believes that this will be required in the future on other portions of Mission Street on account of Mission St. being</p>	



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	Bode Mix 604CC (attached).					
						MUNI's busiest route. SFMTA have asked that M Squared minimize lane closures where possible.
						Please confirm that this 8inch slump is acceptable on Bode Mix 604CC (attached).
U-0241	AWSS - Proposed Fire Hydrant Re-Location on Mission and Fremont Street	Closed	CR	01/21/2014	01/31/2014	02/13/2014
	From: Webcor Construction LP Jackson Tukuafu					
	REQUEST: As per AWSS Coordination Meeting on 01/17/2014, the location of this fire hydrant near the intersection of Fremont and Mission is potentially going to need to be moved to accommodate the new sidewalk expansion. The purpose of the RFI is for Michael Smith with SFWD to analyze, consider and direct the feasibility of relocating the fire hydrant further north.  If the hydrant is to be located somewhere other than is shown on the drawing please provide a detail for this work as additional fittings may need to be ordered.					ANSWER: As per AWSS Coordination Meeting on 01/17/2014, the location of this fire hydrant near the intersection of Fremont and Mission is potentially going to need to be moved to accommodate the new sidewalk expansion. The purpose of the RFI is for Michael Smith with SFWD to analyze, consider and direct the feasibility of relocating the fire hydrant further north.  If the hydrant is to be located somewhere other than is shown on the drawing please provide a detail for this work as additional fittings may need to be ordered.
U-0242	AWSS - Hydrant Lateral Connection Conflict at Sta. 17+20	Closed	CR	01/21/2014	01/31/2014	
	From: Webcor Construction LP Jackson Tukuafu					
	REQUEST: Sheet MA-16 shows that the hydrant tee is to be rolled down 45-degrees in order to tie in to the lateral piping. However, when M Squared excavated this section, they discovered that the hydrant lateral piping is shallow and the main is approx. 2-feet deeper.  In order to install the piping, the hydrant tee will need to be rolled UP 45 degrees. In additon to this M Squared will need to order a customized 8-inch DIP flanged spool to join the tee to the flanged 45-degree elbow.  Please confirm it is acceptable to proceed with the customized flange or provide direction.					ANSWER: Sheet MA-16 shows that the hydrant tee is to be rolled down 45-degrees in order to tie in to the lateral piping. However, when M Squared excavated this section, they discovered that the hydrant lateral piping is shallow and the main is approx. 2-feet deeper.  In order to install the piping, the hydrant tee will need to be rolled UP 45 degrees. In additon to this M Squared will need to order a customized 8-inch DIP flanged spool to join the tee to the flanged 45-degree elbow.  Please confirm it is acceptable to proceed with the customized flange or provide direction.



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U-0243	AWSS - Culvert at North West Corner of Beale and Mission Street	Closed	CR	02/04/2014	02/14/2014	02/13/2014
<div><div><b>From:</b> Webcor Construction LP</div><div>Jackson Tukuafu</div></div>						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please refer to attached drawing MA-16 and attached photos.			Please refer to attached drawing MA-16 and attached photos.			
After trenching to remove the AWSS, M Squared exposed the 12-inch VCP sewer culvert on the NW corner of Beale & Mission street spanning from the catch basin to the sewer main. The joint was open by 4-inches and was covered in cardboard and duct tape. There was also a reverse flow on the pipe by 15 degrees.			After trenching to remove the AWSS, M Squared exposed the 12-inch VCP sewer culvert on the NW corner of Beale & Mission street spanning from the catch basin to the sewer main. The joint was open by 4-inches and was covered in cardboard and duct tape. There was also a reverse flow on the pipe by 15 degrees.			
See attached photos and please advise.			See attached photos and please advise.			





<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
U-0246	AWSS - Gate Valve Vault Locations at 1st Street and Fremont Street	Closed	CR	03/27/2014	04/06/2014	04/01/2014
<div> <div> <b>From:</b> Webcor Construction LP           Jackson Tukuafu         </div> <div> <b>REQUEST:</b>            Please refer to drawing MA-15, MA-16 and attached M Squared sketch SK-TG04.2-070.             First Street:             1. M Squared is able to install the gate valve and gate valve vault at the west side of 1st Street on Mission Street without impacting the new sidewalk expansion. The valve vault will be in the street and outside any sidewalk limits. The vault can be constructed once the street light conduit has been relocated. This work has already been coordinated with the City Street Lighting Division. Please advise on when the City will remove the conflict to install the gate valve/vault.             Fremont Street (east side):             2. See attached sketch. Due to the amount and location of existing utilities M Squared will be unable to install a gate valve vault of any kind at this location. It is also looking highly unlikely that M Squared will be able to install any gate valve at this location due to the utilities in the area. Please advise as to how M Squared will proceed.         </div> <div> <b>ANSWER:</b>            Please refer to drawing MA-15, MA-16 and attached M Squared sketch SK-TG04.2-070.             First Street:             1. M Squared is able to install the gate valve and gate valve vault at the west side of 1st Street on Mission Street without impacting the new sidewalk expansion. The valve vault will be in the street and outside any sidewalk limits. The vault can be constructed once the street light conduit has been relocated. This work has already been coordinated with the City Street Lighting Division. Please advise on when the City will remove the conflict to install the gate valve/vault.             Fremont Street (east side):             2. See attached sketch. Due to the amount and location of existing utilities M Squared will be unable to install a gate valve vault of any kind at this location. It is also looking highly unlikely that M Squared will be able to install any gate valve at this location due to the utilities in the area. Please advise as to how M Squared will proceed.         </div> </div>						



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
U-0247	<b>AWSS - PG&amp;E Utility Conflicts at Fremont Intersection</b>	Closed	CR	03/27/2014	04/06/2014	04/18/2014
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>			<b>ANSWER:</b>			
Please refer to drawing MA-16 and attached M Squared sketch SK-TG04.2-071.			Please refer to drawing MA-16 and attached M Squared sketch SK-TG04.2-071.			
There are 3 duct banks exiting out of PG&E Vault #1669:			There are 3 duct banks exiting out of PG&E Vault #1669:			
<ol style="list-style-type: none"><li>1. PG&amp;E Duct bank #1 runs from 1st Street down Mission to Fremont Street and has been in our trench all that time. It has not encroached on the AWSS Main alignment and there is enough clearance from AWSS main most of that time. It has impacted excavation but should not pose too much of a problem for pipe installation.</li><li>2. PG&amp;E Duct bank #2 sits on top of the existing 12" AWSS Main and runs directly on top of the AWSS Main for a large portion of the intersection. Due to this duct bank we are unable to excavate down to the AWSS Main, unable to remove pipe and will be unable to install new 16-inch AWSS.</li><li>3. PG&amp;E Duct bank #3 runs underneath the existing 12" AWSS main, it is concrete encased and part of the concrete encasement encroaches onto the existing 12" AWSS Main. This duct bank appears to leave our trench after 10ft or so.</li></ol>			<ol style="list-style-type: none"><li>1. PG&amp;E Duct bank #1 runs from 1st Street down Mission to Fremont Street and has been in our trench all that time. It has not encroached on the AWSS Main alignment and there is enough clearance from AWSS main most of that time. It has impacted excavation but should not pose too much of a problem for pipe installation.</li><li>2. PG&amp;E Duct bank #2 sits on top of the existing 12" AWSS Main and runs directly on top of the AWSS Main for a large portion of the intersection. Due to this duct bank we are unable to excavate down to the AWSS Main, unable to remove pipe and will be unable to install new 16-inch AWSS.</li><li>3. PG&amp;E Duct bank #3 runs underneath the existing 12" AWSS main, it is concrete encased and part of the concrete encasement encroaches onto the existing 12" AWSS Main. This duct bank appears to leave our trench after 10ft or so.</li></ol>			
Where duct bank #2 and duct bank #3 cross each other there is only 11-inch between the two duct banks. Not enough space to install a 16inch pipe with tie rods and stop collars. The AWSS is pinched between these 2 duct banks for approx. 10ft with one duct bank on top of the 12-inch main and one touching the main from underneath. While all duct banks impede us it seems there is a higher chance of making this work with the removal of duct bank #2.			Where duct bank #2 and duct bank #3 cross each other there is only 11-inch between the two duct banks. Not enough space to install a 16inch pipe with tie rods and stop collars. The AWSS is pinched between these 2 duct banks for approx. 10ft with one duct bank on top of the 12-inch main and one touching the main from underneath. While all duct banks impede us it seems there is a higher chance of making this work with the removal of duct bank #2.			
Please provide direction on how to proceed or advise when PG&E will begin to remove their duct banks.			Please provide direction on how to proceed or advise when PG&E will begin to remove their duct banks.			



<b>Number</b>	<b>Subject</b>	<b>Status</b>	<b>Choice Required</b>	<b>Date Created</b>	<b>Date Required</b>	<b>Date Answered</b>
<b>U-0248</b>	<b>AWSS - Restraints at the Intersection of Mission and Fremont St.</b>	<b>Open</b>	<b>CR</b>	<b>06/04/2014</b>	<b>06/14/2014</b>	
<b>From:</b> Webcor Construction LP      Lynn Kowallis						
<b>REQUEST:</b>						
Please refer to drawing MA-16 and M Squared Sketch-TG04.2-072.1 and TG04.2-072.2.						
Question #1 - Does every joint between the new GV west of Fremont Street, and the new 16"x10" tee in Fremont Street intersection need to be restrained with bell & stop collars? This distance is approx. 50ft. Question #2 - Currently there does not appear to be enough space to install the new 16" pipe WITH bell & stop collars through the utilities in the Fremont St intersection. There does appear to be space to install 16" pipe without the stop/bell collars on.						
- Our suggestion would be to install the 16"x10" tee and restrain that tee on one joint heading east. Install piping across the Fremont St intersection without stop/bell collars, delete the 16" GV at that location (we believe we will not be able to fit one anyway), and tie off the last piece of pipe coming into the hydrant tee east of Fremont Street.						
Please clarify if this is acceptable.						
<b>ANSWER:</b>						
Please refer to drawing MA-16 and M Squared Sketch-TG04.2-072.1 and TG04.2-072.2.						
Question #1 - Does every joint between the new GV west of Fremont Street, and the new 16"x10" tee in Fremont Street intersection need to be restrained with bell & stop collars? This distance is approx. 50ft. Question #2 - Currently there does not appear to be enough space to install the new 16" pipe WITH bell & stop collars through the utilities in the Fremont St intersection. There does appear to be space to install 16" pipe without the stop/bell collars on.						
- Our suggestion would be to install the 16"x10" tee and restrain that tee on one joint heading east. Install piping across the Fremont St intersection without stop/bell collars, delete the 16" GV at that location (we believe we will not be able to fit one anyway), and tie off the last piece of pipe coming into the hydrant tee east of Fremont Street.						
Please clarify if this is acceptable.						
<b>U-0249</b>	<b>AWSS - Hydrant Location at Fremont St.</b>	<b>Closed</b>	<b>CR</b>	<b>06/23/2014</b>	<b>07/03/2014</b>	<b>07/10/2014</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>						
Please refer to drawing MA-15 and attached M2 sketch SK-TG04.2-073.						
1. Please confirm that it is acceptable to install the new hydrant at Fremont St. (Location #4, STA 12+50), 31" from face of concrete (FOC) at approx. STA. 7+15 on drawing MA-16. The revised location is a result of various conflict in the vicinity that prevent the hydrant from being installed as shown on MA-15. 2. Due to the 10" main being relatively deep, additional fittings will be required to install the hydrant at the revised location (STA 7+15). Please confirm it is acceptable to install the hydrant as shown in the attached sketch SK-TG04.2-073.						
<b>ANSWER:</b>						
Please refer to drawing MA-15 and attached M2 sketch SK-TG04.2-073.						
1. Please confirm that it is acceptable to install the new hydrant at Fremont St. (Location #4, STA 12+50), 31" from face of concrete (FOC) at approx. STA. 7+15 on drawing MA-16. The revised location is a result of various conflict in the vicinity that prevent the hydrant from being installed as shown on MA-15. 2. Due to the 10" main being relatively deep, additional fittings will be required to install the hydrant at the revised location (STA 7+15). Please confirm it is acceptable to install the hydrant as shown in the attached sketch SK-TG04.2-073.						





# Webcor/Obayashi Joint Venture

## PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

### 30100 - Transbay Transit Center Project

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Job: 30100

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<b>U-0249.1</b>	<b>AWSS - Hydrant on Fremont Street</b>	<b>Closed</b>	<b>01</b>	<b>09/10/2014</b>	<b>09/20/2014</b>	<b>09/11/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  Per response to RFI U-0249, M Squared last night investigated the use of a hydrant riser. It appears that a 24" hydrant riser will be possible in this situation.  Please confirm the use of the 24" riser.		<b>ANSWER:</b>  Per response to RFI U-0249, M Squared last night investigated the use of a hydrant riser. It appears that a 24" hydrant riser will be possible in this situation.  Please confirm the use of the 24" riser.				
<b>U-0250</b>	<b>AWSS - Hydrant Lateral Connection at STA 13+75 (MA-16)</b>	<b>Closed</b>	<b>CR</b>	<b>06/23/2014</b>	<b>07/03/2014</b>	<b>07/08/2014</b>
<b>From:</b> Webcor Construction LP      Jackson Tukuafu						
<b>REQUEST:</b>  Please refer to attached excerpt drawing MA-16 and M2 sketch SK-TG04.2-074.  The piping on this lateral is partially encased in the 3x5 sewer and appears to be ductile iron from 5 ft north of the 3x5 sewer heading south to the hydrant location.  Please confirm that it is acceptable to weld lugs on the existing ductile and restrain new ductile iron pipe from this point to the new 16" AWSS Main. Note additional fittings are required as shown in the attached sketch. (8inch 45deg elbows also required and additional bell and stop collars)		<b>ANSWER:</b>  Please refer to attached excerpt drawing MA-16 and M2 sketch SK-TG04.2-074.  The piping on this lateral is partially encased in the 3x5 sewer and appears to be ductile iron from 5 ft north of the 3x5 sewer heading south to the hydrant location.  Please confirm that it is acceptable to weld lugs on the existing ductile and restrain new ductile iron pipe from this point to the new 16" AWSS Main. Note additional fittings are required as shown in the attached sketch. (8inch 45deg elbows also required and additional bell and stop collars)				
<b>U-0251</b>	<b>AWSS - Valve Vault Conflict at Fremont Street</b>	<b>Closed</b>	<b>01</b>	<b>09/16/2014</b>	<b>09/26/2014</b>	<b>09/18/2014</b>
<b>From:</b> Webcor Construction LP      Claude Titcher						
<b>REQUEST:</b>  See attached photos.  The location for the gate valve on Mission Street, east of Fremont Street is in conflict with the piping on the hydrant lateral. The valve cannot be placed any further west due to the street light and traffic signal conduits. Moving it further west would also put the vault within the new curb return that is planned as part of the previously discussed sidewalk expansions project on Mission Street.		<b>ANSWER:</b>  See attached photos.  The location for the gate valve on Mission Street, east of Fremont Street is in conflict with the piping on the hydrant lateral. The valve cannot be placed any further west due to the street light and traffic signal conduits. Moving it further west would also put the vault within the new curb return that is planned as part of the previously discussed sidewalk expansions project on Mission Street.				



Webcor/Obayashi Joint Venture

PROJECT MANAGEMENT - REQUEST AND ANSWERS LOG

30100 - Transbay Transit Center Project

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<i><b>Number</b></i>	<i><b>Subject</b></i>	<i><b>Status</b></i>	<i><b>Choice Required</b></i>	<i><b>Date Created</b></i>	<i><b>Date Required</b></i>	<i><b>Date Answered</b></i>
U-0252	AWSS - 10inch Lead Joint on Fremont St	Closed	01	10/03/2014	10/13/2014	10/06/2014
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
See attached sketch.		See attached sketch.				
The existing lead joint (outside contract scope) is leaking. It appears that the pressure of the string back pulling on the lead has caused the lead to leak.		The existing lead joint (outside contract scope) is leaking. It appears that the pressure of the string back pulling on the lead has caused the lead to leak.				
Our crews have excavated further north and exposed more of the existing main to allow our plumber to repack the lead joint. In doing so we discovered ears on the existing pipe approx. 3ft north of the lead joint.		Our crews have excavated further north and exposed more of the existing main to allow our plumber to repack the lead joint. In doing so we discovered ears on the existing pipe approx. 3ft north of the lead joint.				
Our suggestion is to repack the lead joint and tie back the adaptor to the ears on the existing pipe. In eliminating the strong back we will be eliminating the pressure on the repacked joint.		Our suggestion is to repack the lead joint and tie back the adaptor to the ears on the existing pipe. In eliminating the strong back we will be eliminating the pressure on the repacked joint.				
Please confirm you are in agreement with our suggestion.		Please confirm you are in agreement with our suggestion.				
U-0253	AWSS - Valve Vault West of Fremont St	Closed	CR	10/10/2014	10/20/2014	10/14/2014
<b>From:</b> Webcor Construction LP                      Claude Titché						
<b>REQUEST:</b>		<b>ANSWER:</b>				
Following on from [previous discussion and previous correspondence, M Squared would like to clarify the issue with the gate valve vault on Mission St west of Fremont St.		Following on from [previous discussion and previous correspondence, M Squared would like to clarify the issue with the gate valve vault on Mission St west of Fremont St.				
Due to the presence of 2 duct banks and a gas main, no vault will be possible at this gate valve location. M Squared proposes to perform the same modifications as was performed west of Beale St.		Due to the presence of 2 duct banks and a gas main, no vault will be possible at this gate valve location. M Squared proposes to perform the same modifications as was performed west of Beale St.				
See attached		See attached				
Please confirm that this is how we are to proceed.		Please confirm that this is how we are to proceed.				
U-182.5	Tie Back Requirements on 2nd Street	Closed	01	06/21/2013	07/01/2013	06/27/2013
<b>From:</b> Webcor Construction LP                      Jackson Tukuafu						



# 30100 - Transbay Transit Center Project



# 30100 - Transbay Transit Center Project



<u>Number</u>	<u>Subject</u>	<u>Status</u>	<u>Choice Required</u>	<u>Date Created</u>	<u>Date Required</u>	<u>Date Answered</u>
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*END OF REPORT*

Report Parameters

Project:	30100	Status Class:	
Sent To:		Run Date:	02/11/2015
Restrict Value of:	C	Run Time:	07:01 AM
From Date:	01/01/2010	Operator:	JNELSON
To Date:	02/11/2015	Report Code:	WBPM3012
Status:			



**Transbay Transit Center – San Francisco, CA**

**Noise and Vibration Mitigation Management Plan**

Webcor/Obayashi

**September 07, 2012**

## **GENERAL:**

The Webcor/Obayashi (W/O or CM/GC) Noise and Vibration Mitigation Management policy that will be implemented on the Transbay Transportation Center Project will be an overall project policy, with each Trade Subcontractor contributing their specific plan as they come on board to the project. The primary function of this plan is to comply with Specification Section 00 08 13, 00 08 13/APB, the San Francisco Noise Control Ordinance, regulations and requirements and section 01 35 65, Specific Project mitigation measures and monitoring requirements as applicable to the various phases of work.

When required by the specifications, W/O will ensure its Trade Subcontractors comply with this plan as well as the San Francisco Noise Control Ordinance.

To expedite the project or minimize impacts, W/O will ensure that its Trade Subcontractors apply for written waivers of some of the noise requirements by application to the TJPA in accordance with Section 00 08 13 Specific Project Requirements when required by the specifications or contract. Written waivers shall be uploaded to Constructware by CM/GC. It is anticipated that some Work may require multiple shifts or for other reasons need to be performed outside of typical weekday daytime construction hours. Trade Subcontractors shall minimize construction activities during evening, nighttime, weekend, and holiday periods and shall obtain specific permits before performing construction in noise sensitive areas during these periods.

Night noise permits requests shall be submitted to the TJPA at least 7 days in advance of work. Noise permit request shall include:

1. Name of person in charge of work and phone number
2. Hours to be worked
3. Narrative of scope of work including necessity of doing work at night, maps, and truck routes
4. List of noise/vibration/light making equipment including make and model
5. Mitigation and monitoring methods being used

W/O will ensure that its Trade Subcontractors provide noise inspections and testing of equipment to ensure that all equipment onsite is in good condition and effectively muffled per manufacturer's recommendation. If inspection or testing documents are requested by the TJPA, or any of its representatives, W/O will require its Trade Subcontractors to provide requested documentation in a timely manner. Trade Subcontractors shall provide inspection and testing documents to CM/GC prior to start of work and as the equipment is replaced. CM/GC shall upload documents to a file location within Constructware.

W/O will ensure that its Trade Subcontractors minimize use of vehicle backup alarms and demonstrate how backup alarms will be minimized by using mitigation measures such as designing the construction site with a circular flow pattern that minimizes backing up of trucks and other heavy equipment. Trade Subcontractors shall submit quarterly reports of measures to reduce back up alarms. W/O shall upload these reports to a specific location within Constructware.

W/O will ensure that all its Trade Subcontractors' equipment onsite is equipped with broadband back-up alarms that will automatically adjust based on the ambient noise during nighttime hours (between 8 p.m. and 7 a.m.) when ambient noise is low. If safety considerations and applicable regulations will not allow use of broadband back-up alarms, Contractor shall request an exemption in writing to the TJPA



Representative including the applicable safety regulations (Cal/OSHA, OSHA). Trade Subcontractors shall comply with the TJPA's request for broadband back-up alarms for all work between 8 p.m. and 7 a.m. If requested by the TJPA or its representative, Trade Subcontractors shall provide W/O with equipment specifications showing broadband back-up alarms for submission via Constructware.

Through W/O's requirement of the submittals outlined in this noise and vibration plan, W/O will verify Trade Subcontractors' construction operations are performed in such a manner to minimize noise.

W/O will verify that its Trade Subcontractors perform noise monitoring to demonstrate compliance with noise limits and endeavor to minimize construction activities during off hours except for those required and deemed acceptable per the Contract Documents. Trade Subcontractors shall submit monthly monitoring reports to W/O for submission via Constructware.

W/O will verify Trade Subcontractors haul routes to ensure that they minimize noise intrusion into residential areas, and control noise during nighttime hours.

W/O will require all Trade Subcontractors to use procedures and equipment, when it would be effective, that produce lower noise levels than normal when required by the specifications or contract. W/O will require the Trade Subcontractor to submit manufacturer special noise control kit information. If none is available, then the Trade Subcontractor needs to submit a statement of this. Upon receipt and review of the information, W/O and the Trade Subcontractor will identify the events when the noise control measures should be used based on the specifications.

W/O will require all Trade Subcontractors plans to include use of temporary barriers near noisy activities as required by the specifications or contract. Such barriers shall be located close enough to the noise source to achieve noise attenuation. As necessary and when it is shown it would be effective, Trade Subcontractors shall construct shed-like structures or complete buildings to contain the noise from nighttime activities.

W/O shall require haul route map, plan and storage location to be part of Trade Subcontractor's plan and included within its submittal.

#### VIBRATION CONTROL

Vibration limits are based upon the Federal Transit Administration's Planning and Environment Transit Noise and Vibration Impact Assessment guidelines. W/O will require all Trade Subcontractors' to limit or prohibit use of construction techniques that create high vibration levels when it affects adjacent properties.

If construction techniques that create high vibration levels are used, W/O will require all Trade Subcontractors' to comply with the following additional restrictions:

1. Provide advance notice to TJPA of any vibration intensive activities. Perform vibration intensive activities only during daytime hours between 7 a.m. and 8 p.m. unless otherwise allowed by special permit or variance, as required by the specifications or contract. Perform vibration monitoring during vibration intensive activities during daytime hours between 7 a.m. and 8 p.m. unless otherwise allowed by special permit or variance, as required by the specifications or contract. Recorded data should be part of the Trade Subcontractor Daily report. A summary shall be submitted monthly and uploaded to Constructware.



2. Investigate alternative construction methods and practices to reduce the impacts if present and implement alternative methods and practices as reasonable.
3. Provide a plan to measure vibration levels including but not limited to measurement locations, times and metrics. Plan shall also include contingency plan if operations exceed the limits. This plan shall be uploaded into Constructware by W/O.
4. Limit or prohibit use of construction techniques that create high vibration levels.

Trade Subcontractors shall be responsible for providing technical information, as required by the specifications, in their plan. Trade Subcontractor's plan shall be submitted via Constructware for Record Only.



**Transbay Transit Center – San Francisco, CA**

**Air Quality Plan**

Webcor/Obayashi

January 16, 2012

## **GENERAL PLAN:**

The Webcor/Obayashi (W/O) Air Quality Plan that will be implemented on the Transbay Transit Center Project will be an overall policy with each subcontractor contributing their specific plan as they come on board to the project. The primary function of this plan is to comply with the Bay Area Air Quality Management District regulations and requirements.

W/O will require its Trade Subcontractors to establish a plan that complies with all requirements set for in specification sections 00 08 13, and 01 35 65 prior to starting Work onsite. W/O shall check and verify trade subcontractor's compliance with air quality requirements on a daily basis. Any non-compliant trade subcontractors will receive both verbal and written notice through Safe Site One (W/O internal program). Additional, W/O will require trade subcontractors to demonstrate they are actively monitoring air quality by providing checklists or documentation on each Trade Subcontractors daily report. W/O shall verify its Trade Subcontractors Air Quality plan includes the following but not necessary limited to:

1. Specific measures to minimize impacts to sensitive receptors associated with exposure to respirable nuisance dust (PM10) and achieve a goal of No Visible Emissions.
2. W/O shall verify Trade Subcontractors comply with City Dust Control Order (DPW Order No. 171,378. Water active construction areas at least twice daily to control dust using non-potable water in accordance with San Francisco Ordinance 175-91
3. Identify specific measures to minimize dust generation; to reduce health risks to workers and the public.
4. Mist the immediate excavation area with a water spray to prevent airborne dust particles. Perform continuous water spraying during dust-generating activities. Mist or spray in such a way as to prevent puddling or generation of runoff, which could potentially reach storm drains or catch basins.
5. Minimize the amount of excavated material or demolished debris stored at the Site. Remove excavated material and demolished debris, with the exception of hazardous materials or suspected hazardous materials, from the Site no later than the end of each workday. If hazardous materials or suspected hazardous materials are stored on site, store such materials in accordance with all applicable California Environmental Protection Agency regulations, including providing storage in proper containers and protection from exposure to the elements. Remove such materials from the Site as soon as possible for disposal or recycling in accordance with applicable laws and regulations.
6. Wet all exposed soil surfaces at least 3 times daily during dry weather or more frequently if dust is blowing or if required by the TJPA. Immediately wet sweep serpentine residuals from the street.
7. Keep the Site and adjacent areas clean and perform wet sweeping at the end of each shift. Sweep daily (with water sweepers) all paved access roads, parking areas and staging areas at construction sites. Sweep streets daily (with water sweepers) if visible soil material is carried onto adjacent public streets.
8. Load haul trucks carrying excavated material so that the material does not extend above the walls or back of the truck bed. Wet before covering and tightly cover the surface of each load before the haul truck leaves the loading area. Cover trucks hauling soil, sand, and other loose materials or require trucks to maintain at least 2 feet of freeboard
9. Clean up spillage on City streets, whether directly or indirectly caused by Contractor's operations.

10. Minimize use of on-site diesel construction equipment, particularly unnecessary idling. Shut off construction equipment to reduce idling when not in direct use. Where feasible, replace diesel equipment with electrically powered machinery.
11. Retain receipts of ultra-low sulphur fuel (ULSF) purchase and equipment tuning and repair and make these available to the TJPA Representative or to the Federal Transit Administration (FTA) designee upon request.
12. Locate diesel engines, motors, or equipment as far away as possible from existing residential areas.
13. Properly tune and maintain diesel power equipment. To manufacturer's specification and frequency.
14. Suspend grading operations during first and second stage smog alerts, and during high winds (i.e., winds greater than 25 miles per hour).
15. Upon completion of the construction phase, buildings with visible signs of dirt and debris from the construction site shall be power-washed and/or painted (provided that permission is obtained from the property owner to access and wash the property with no fee charged by the owner). Trade Contractor shall request CMGC to contact Singer and Associates to notify property owners for access. If permission from property owners for access is not granted, Trade Contractor is not responsible for power-washing or painting.
16. Pave, apply water three times daily, or apply (non-toxic) soil stabilizers on all unpaved access roads, parking areas, and staging areas at construction sites.
17. If applicable, replant vegetation in disturbed areas as quickly as possible.

W/O will verify Trade Subcontractors comply with the requirements of the Bay Area Air Quality Management District (BAAQMD) Regulation 6 (for particulate matter and visible emissions), Regulation 7 "Odorous Substances," Regulation 11 "Hazardous Pollutants," and the California Health and Safety Code Division 26 "Air Resource", Chapter 3 "Emission Limitations," Section 41700 "Prohibited Conduct," and related regulations. Trade Subcontractors shall notify the BAAQMD 10 working days prior to commencing demolition or hazardous materials abatement work.

1. Such notification shall include the names and addresses of operations and persons responsible; description and location of the structure to be demolished or altered including size, age and prior use, and the approximate amount of friable asbestos; scheduled starting and completion dates of demolition or abatement; nature of planned work and methods to be employed; procedures to be employed to meet BAAQMD requirements; and the name and location of the disposal site.
2. The BAAQMD randomly inspects removal operations and will respond to any complaints received. Contractor shall cooperate with and facilitate all BAAQMD authorized inspections.\
3. Notifications shall be documented and provided to CM/GC for submission to the TJPA via ConstructWare.

Trade Subcontractors shall be responsible for providing technical information, as required by the specifications, in their plan. All trade subcontractors plans shall be submitted for Record Only via ConstructWare.



**Transbay Transit Center – San Francisco, CA**

**Waste Management and Construction Debris Plan Revision 6**

Webcor/Obayashi  
November 7, 2013



## **GENERAL PLAN:**

Webcor/Obayashi Joint Venture (Webcor/Obayashi) understands that the building contractor plays a critical role in the management of jobsite produced construction waste. Webcor/Obayashi has adopted a waste reduction and recycling policy that will be implemented on the Transbay Transportation Center Project. This policy will be an overall policy with each subcontractor contributing their specific plan as they come on board to the project.

The primary goal of the plan is to divert as much construction generated debris & unused material from landfills as possible. At a minimum, Webcor/Obayashi and its trade subcontractors will divert 75% of the waste generated on the construction project from landfills. Trade subcontractors Construction Waste Management Plan shall be prepared and submitted in compliance with the Owner's LEED project requirements and the requirements of the City and County of San Francisco.

The Trade Subcontractors are required to comply with Specification Sections 00 08 15, 01 74

00, and 01 81 13 as well as any or all of the procedures listed below. If a conflict in percentages exists between this section and Section 01 81 13, General LEED Building Design and Construction Requirements, the most stringent section shall govern.

- Use of approved debris haulers with documented recycling levels.
- Source separated debris boxes will be provided onsite for mixed debris and recyclable items such as lumber and wood related products, dirt, concrete and asphalt, cardboard & metals.
- Trade Subcontractors are required to handle and dispose of any generated hazardous waste.
- Requesting Trade Subcontractors and vendors to utilize reusable packaging when possible.
- Trade Subcontractor shall provide a Construction Waste Management Plan.

All Trade Subcontractors shall develop their own Waste Management and Construction Debris Plan that complies with the Contract Documents and this plan. Trade Subcontractors shall submit this plan in accordance with the specifications and it shall become part of Webcor/Obayashi's overall project plan. All technical requirements defined in the contract documents shall be fulfilled by Trade Subcontractors and submitted to the Construction Management Oversight (CMO) For Record Only through ConstructWare

Webcor/Obayashi will ensure the Trade Subcontractors are effectively implementing the procedures and are in compliance with Specifications.

Webcor/Obayashi will verify that after Award of Contract and before commencement of the Work at the site, the Trade Subcontractor conducts a Reuse/Recycle Assessment as part of their Solid Waste Management Plan (SWMP): Trade Subcontractor's assessment shall

estimate the types and quantities of materials for the Project that are anticipated to be feasible for source separation for recycling or reuse, either onsite or offsite, and note the procedures intended for a recycling, reuse, and salvage program. Documentation of the trade subcontractor's plan shall consist of the following:

- Trade subcontractor and vendor waste management strategies.
- Trade subcontractor required to provide a monthly summary of the total waste material with backup documentation (weight tickets) if processed offsite.
- The amount recycled (in tons), material types, recycling procedures, and processing facility locations to which materials were diverted if processed offsite.

Trade Subcontractor's Construction Waste Management Plan shall also include estimated wastes, disposal, and handling with the following:

A. List of materials that comprise source separated materials include, but are not limited to:

- Concrete, Wood, Mud, Mixed Aggregates, Yard waste, Metals, and Cardboard.
- Yard waste is not included in our overall diversion rate calculation on the template or corresponding spreadsheet per the requirements from the LEED BD&C v3.0 Reference Guide.

B. List of materials that comprise Miscellaneous Construction Debris include, but are not limited to:

- Wood, Scrap Metal, Drywall, Plastics, Film Plastics, Wire, Cable, Glass.
- The total quantity estimated, inception to completion Disposal.
- Total Project Generation, Diversion + Disposal.
- Project Diversion Rate.

Webcor/Obayashi will verify that Construction and Demolition Waste; Non- hazardous solid resources resulting from Trade Subcontractor's construction, remodeling, repair, and demolition operations for the Project are properly transferred to a C&D Recycling Facility. The C&D Recycling Facility shall be a facility that receives only C&D (construction and demolition) material. Trade Subcontractors shall provide Webcor/Obayashi a summary sheet, including all receipts for transport materials each month with the progress billing if any materials are processed offsite.

Webcor/Obayashi will verify that of the inevitable waste generated, Trade Subcontractor's reuse, salvage, or recycle as many of the waste materials as economically feasible.

Webcor/Obayashi will participate/attend a meeting with Trade Subcontractor, the TJPA Representative and representatives of the City's Solid Waste Management and recycling programs prior to commencement of work. Webcor/Obayashi will

ensure all Trade Subcontractors are made aware of the LEED requirements for C&D diversion before being allowed to work on the site.

Webcor/Obayashi will verify that Trade Subcontractors submit a Monthly Disposal and Recycling Summary Report; quantifying the construction and demolition waste generated and recycled, reused or disposed of at Class 3 Landfill. Contractor shall also send a copy of this report to the TJPA Representative and the SWMP to the City Government Recycling Coordinator. The Comprehensive Disposal and Recycling Summary Report shall be submitted quantifying the construction and demolition waste generated and recycled, reused or disposed of at Class 3 Landfill, on a monthly basis. This report is a condition of progress payment and failure to submit this information shall render the Applications for Payment incomplete. The Trade Subcontractors/trades are also responsible for contracting with a regional facility to haul any hazardous materials from the site. The Trade Subcontractor shall calculate the C&D diversion rate for both LEED requirements (excluding yard waste) and the requirements set by the City (including yard waste) for all materials processed offsite. The W/O LEED representative will screen every C&D Submittal and review Trade Subcontractor and lower-tier subcontractors C&D Plans for clarity, completeness, and compliance with City/LEED requirements.

Webcor/Obayashi will verify that Trade Subcontractors develop and implement procedures for source separation to the greatest extent feasible.

Webcor/Obayashi will verify the Trade Subcontractors plans develop and implement procedures for transporting commingled (mixed) construction and demolition waste that cannot be feasibly source-separated if the intent is to process it offsite instead of using debris boxes provided onsite.

Webcor/Obayashi will verify the Trade Subcontractors plans develop and implement procedures for Salvage and Reuse.

Webcor/Obayashi will verify the Trade Subcontractors plans develop and implement practices for this project that will reduce waste at the source.

Webcor/Obayashi will verify the Trade Subcontractors plans develop and implement procedures for materials that are recycled and/or reused onsite

Webcor/Obayashi will verify that Trade Subcontractors participate in reuse programs by reviewing each Trade Subcontractors Monthly Disposal report for any material processed offsite. For such reuse programs, Trade Subcontractor shall refer to the City's construction and demolition recycling program.

Webcor/Obayashi shall review the environmental goals of this Project with all Trade Subcontractors during the preconstruction meeting. Webcor/Obayashi shall make a proactive effort to increase awareness of these goals among the job site workers. Webcor/Obayashi will make a proactive effort to increase awareness of these goals among the site workers by requiring that each Subcontractor take Click Safety training prior to stepping on the jobsite. As part of this Click Safety training, there is a module dedicated to teaching and reviewing the

Exhibit P



LEED requirements of the project during construction activity.

Webcor/Obayashi will verify that Trade Subcontractors are using registered transporters and registered facilities. Only registered transporters can remove mixed construction and demolition debris from the construction site, and they must take this material to a registered facility. NOTE: A Registered facility: is any facility that accepts mixed construction and demolition debris for processing and recycling must be registered with the City and County of San Francisco and must demonstrate an overall minimum recycling rate of 65% for mixed construction and demolition debris. A registered facility must have applied for and received a registration from the San Francisco Department of the Environment. Webcor/Obayashi will ensure that Waste Management Companies that service San Francisco and retained by the Trade Subcontractors are registered transporters and meet the City/LEED requirements. Trade Subcontractors shall refer to SFEnvironment.org for the City's most current list of registered transporters.

Webcor/Obayashi will verify that Trade Subcontractors are implementing the following:

1. Eliminate the procurement of unneeded supplies.
2. Reduce waste by printing and copying double-sided.
3. Submit all submittals, reports, and forms in electronic format (PDF) unless otherwise noted.
4. Fully participate in available and required recycling and composting programs.
5. Purchase products made with recycled content such as paper and recycled aggregate.

Webcor/Obayashi will verify that Trade Subcontractors shall submit:

1. Construction and Demolition Debris Management Plan.
2. Construction and Demolition Debris Recovery Monthly Summary Report and supporting documentation for any materials processed offsite.
3. Construction and Demolition Debris Recovery Final Report for all materials processed offsite.

Trade Subcontractor's plan shall comply with specification section 02 41 00. All Trade Subcontractors will remove and dispose of all waste materials from the site for off-site disposal in compliance with all applicable laws, ordinances, rules, and regulations. Webcor/Obayashi and all Trade Subcontractors will work with the TJPA representative so that the representative may characterize the waste materials as required by law to the extent required by Webcor/Obayashi's selected disposal facilities.

Trade Subcontractor's plan shall comply with specification section 01 15 00. Trade Subcontractor's shall perform work in a manner to minimize generation of dust, dirt, rubbish, and other debris, to prevent dust and debris from interfering with the progress of the work, and to keep dust and debris from accumulating at the work site or adjacent areas. Trade Subcontractor's shall remove debris and rubbish from the site on a daily basis.

Trade Subcontractor's plan shall comply with specification section 01 13 50, by preventing the mixing of hazardous and non-hazardous materials.

Trade Subcontractor's shall be required to provide technical information, as required by the specifications including compliance with the City and County of San Francisco Ordinance 27-Exhibit P

Construction Waste Management Plan

06, in their plan which will be submitted For Record Only to the CMO.



## Exhibit Q

# APPRENTICESHIP PROGRAM



<b>Trade Subcontractor Name</b>	
---------------------------------	--

### CRAFTS EXPECTED TO BE EMPLOYED BY TRADE SUBCONTRACTOR

[illegible]

**CRAFTS EXPECTED TO EMPLOYED BY SUBCONTRACTORS OF THE TRADE SUBCONTRACTOR**

## SUBCONTRACTOR #1

<b>Subcontractor Name</b>	
---------------------------	--

[illegible]

## SUBCONTRACTOR #2

Subcontractor Name	
--------------------	--

[illegible]

**WEBCOR/OBAYASHI JOINT VENTURE - TRADE SUBCONTRACTOR'S APPRENTICESHIP REQUIREMENTS**  
**SUBCONTRACTOR #3**

[illegible]

## SUBCONTRACTOR #4

[illegible]

## SUBCONTRACTOR #5

[illegible]

## SUBCONTRACTOR #6

[illegible]

**WEBCOR/OBAYASHI JOINT VENTURE - TRADE SUBCONTRACTOR'S APPRENTICESHIP REQUIREMENTS**  
**SUBCONTRACTOR #7**

[illegible]

### SUBCONTRACTOR #8

[illegible]

## SUBCONTRACTOR #9

[illegible]

## SUBCONTRACTOR #10

[illegible]

**WEBCOR/OBAYASHI JOINT VENTURE - TRADE SUBCONTRACTOR'S APPRENTICESHIP REQUIREMENTS**  
**SUBCONTRACTOR #11**

[illegible]

## SUBCONTRACTOR #12

[illegible]

### SUBCONTRACTOR #13

[illegible]

## SUBCONTRACTOR #14

[illegible]

**WEBCOR/OBAYASHI JOINT VENTURE - TRADE SUBCONTRACTOR'S APPRENTICESHIP REQUIREMENTS**  
**SUBCONTRACTOR #15**

[illegible]

## SUBCONTRACTOR #16

[illegible]

## SUBCONTRACTOR #17

[illegible]

### SUBCONTRACTOR #18

[illegible]



**WEBCOR/OBAYASHI JOINT VENTURE - TRADE SUBCONTRACTOR'S APPRENTICESHIP REQUIREMENTS**  
**SUBCONTRACTOR #19**

[illegible]

### SUBCONTRACTOR #20

[illegible]



# MONTHLY

## TRADE SUBCONTRACTOR AFFIDAVIT

TRADE PACKAGE NO.: \_\_\_\_\_

I, \_\_\_\_\_ declare under penalty of perjury that:

1. I am the \_\_\_\_\_ of \_\_\_\_\_ and I am responsible  
(Owner, Officer, Partner) (Company)  
for the payment of persons employed by \_\_\_\_\_ who performed work on  
(Company)  
the \_\_\_\_\_, in the classification(s) of \_\_\_\_\_  
(Project)  
\_\_\_\_\_.

2. \_\_\_\_\_ The apprenticeship committee(s) either denied or failed to respond to our request for the  
dispatch of apprentices, and therefore all workers were classified as journeymen for the  
following crafts: \_\_\_\_\_  
\_\_\_\_\_

Or

During the previous monthly period \_\_\_\_\_  
(month)

The required number of apprentices by craft listed and initialed below have been employed  
according to the minimum and/or maximum requirements as required by the regulating  
documents for the previous period. (Attach backup demonstrating compliance for period  
referenced above)

CRAFT	IN COMPLIANCE (Y/N)	BACKUP ATTACHED (Y/N)

Or

**WEBCOR/OBAYASHI JOINT VENTURE - TRADE SUBCONTRACTOR'S APPRENTICESHIP REQUIREMENTS**

**Provide a plan to satisfy this requirement by the end of the project without exceeding the maximum number of apprentices on a daily basis.**

**This document must be submitted and approved, with backup if required, prior to submittal and subsequent approval of the next billing period's progress billing.**

**Executed this \_\_\_\_\_ day of \_\_\_\_\_ 201\_\_\_\_, in \_\_\_\_\_, CA.**

\_\_\_\_\_  
**(Signature)**



# FINAL

## TRADE SUBCONTRACTOR AFFIDAVIT

TRADE PACKAGE NO.: \_\_\_\_\_

I, \_\_\_\_\_ declare under penalty of perjury that:

1. I am the \_\_\_\_\_ of \_\_\_\_\_ and I am responsible  
(Owner, Officer, Partner) (Company)  
for the payment of persons employed by \_\_\_\_\_ who performed work on  
(Company)  
the \_\_\_\_\_, in the classification(s) of \_\_\_\_\_  
(Project)  
\_\_\_\_\_.

2. During the payroll periods commencing on \_\_\_\_\_ and ending  
\_\_\_\_\_, all persons employed by my company on this project have been  
paid the specified general prevailing rate of per diem wages for the specified craft or  
classification pursuant to Labor Code §§ 1771 and 1813.<sup>1</sup>

3. \_\_\_\_\_ The apprenticeship committee(s) either denied or failed to respond to our request for the  
dispatch of apprentices, and therefore all workers were classified as journeymen.

Or

The required number of apprentices by craft listed and initialed below have been employed  
according to the minimum and/or maximum requirements as required by the regulating  
documents.

CRAFT	IN COMPLIANCE (Y/N)

Executed this \_\_\_\_\_ day of \_\_\_\_\_ 201\_\_\_\_, in \_\_\_\_\_, CA.

## WEBCOR/OBAYASHI JOINT VENTURE - TRADE SUBCONTRACTOR'S APPRENTICESHIP REQUIREMENTS

**This document must be submitted and approved prior to final retention payment.**

---

**(Signature)**

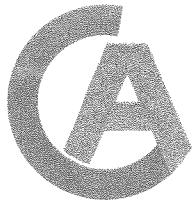
<sup>1</sup> Except for public works projects of one thousand dollars (\$1,000) or less, not less than the general prevailing rate of per diem wages for work of a similar character in the locality in which the public work is performed, and not less than the general prevailing rate of per diem wages for holiday and overtime work fixed as provided in this chapter, shall be paid to all workers employed on public works.

This section is applicable only to work performed under contract, and is not applicable to work carried out by a public agency with its own forces. This section is applicable to contracts let for maintenance work.



# Exhibit R

## Survey Information



# CHAUDHARY & ASSOCIATES, INC.

ENGINEERS  
SURVEYORS  
INSPECTORS

851 NAPA VALLEY CORPORATE WAY ■ SUITE G ■ NAPA, CALIFORNIA 94558-7551  
PHONE: 707.255.2729 ■ FAX: 707.255.5021 ■ WWW.CHAUDHARY.COM

December 27, 2011  
#11-03-014

Mr. Rick Buellesbach  
Senior Project Manager - Transbay Transit Center  
Webcor/Obayashi Joint Venture  
175 Beale Street  
San Francisco, CA 94105

Re: Transbay Transit Center Quality Control Surveys  
Subject: December 2011 Control Verification Survey Results

Dear Mr. Buellesbach:

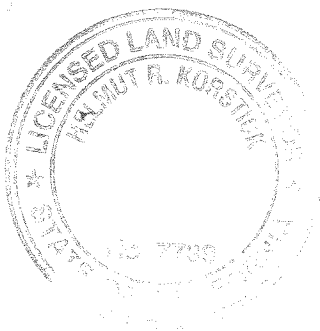
The field work for subject surveys was conducted by Chaudhary & Associates December 5 - 8, 2011. The surveys included verification of Chaudhary & Associates control (as shown on the Survey Control Plan dated 11-10-2011), with the exception of control point 217 which was destroyed sometime between the November 2011 and December 2011 control verification surveys.

Horizontal control values for point numbers 54, 208, 209, 213, 101, 105, 215, and 227 were constrained in this control network horizontal adjustment. The elevation values remain unchanged from the November 2011 surveys. The table below shows both the 11-10-2011 and the 12-21-2011 values for the remaining control points. Because data values can be impacted by environmental factors (temperature and humidity), seismic activity, and the various combinations of back sight and foresight data available on any given day, only the values which differ by 0.01' or more are adjusted and shown on the following table (and updated on the 12/2011 control map to be sent to you tomorrow). Field note copies and Star Net Reports have been mailed to you.

## Horizontal Values

Point #	November 10, 2011		December 2011		Description
	Northing	Easting	Northing	Easting	
79	2115835.42	6013588.51	2115835.43	6013588.49	Fnd Mag+Shnr on TC
205	2115091.66	6013145.43	2115091.66	6013145.42	Mag Nail
221	2115642.30	6013753.17	2115642.32	6013753.18	Fnd Scribed-X KCA #4
223	2115654.49	6014255.95	2115654.48	6014255.95	Fnd Scribed-x KCA 9605
224	2115924.30	6013990.82	2115924.30	6013990.81	Cut-X
225	2115838.99	6014083.47	2115838.98	6014083.47	Fnd Scribed-X KCA 9761
229	2115259.63	6013325.88	2115259.62	6013325.87	Mag+Wshr

Please feel free to call me at (707) 255-2729 any questions or comments.



Sincerely,  
**CHAUDHARY & ASSOCIATES, INC.**  
A California Corporation

*Helmut R. Korstick*  
Helmut R. Korstick, PLS 7739  
Project Surveyor

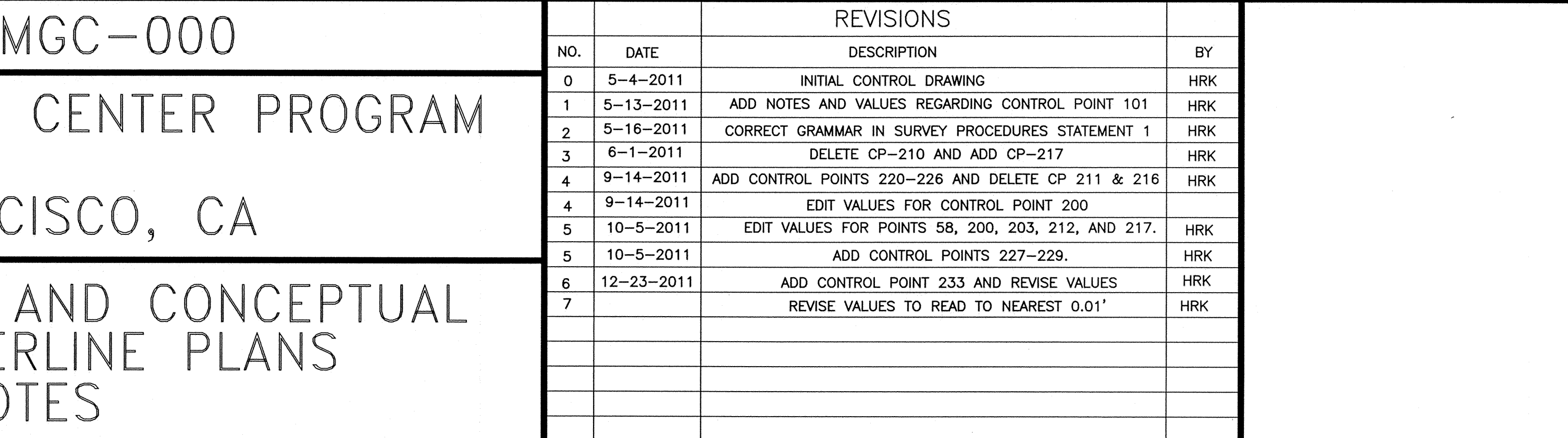





10

TRANSBAY JOINT POWERS AUTHORITY

- 
- CHAUDHARY  
& ASSOCIATES, INC.**  
ENGINEERS SURVEYORS INSPECTORS  
681 MAPA VALLEY CORPORATE WAY, SUITE G  
DALLAS, TEXAS 75248-4600  
(214) 343-8888  
FAX (214) 343-8889  
E-MAIL: CHAUDHARY@AOL.COM



PREPARED BY PERSONAL ARVIN K. CHAUDHARY	DRAWING NO. 08
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- 1) SEE ARCHITECTURAL PLANS FOR GRID LAYOUT AND DIMENSIONS.
- 2) NAD 83 REFERS TO THE CALIFORNIA STATE PLANE COORDINATE SYSTEM. HORIZONTAL CONTROL FOR THIS SURVEY IS BASED UPON NAD83, CA ZONE 3, EPOCH 1991.35, GRID 03.
- 3) UNITS ARE FEET AND REPRESENT GROUND (NOT GRID) DISTANCES.
- 4) THE FIELD DATA IS BASED UPON NAVD 84.
- 5) THE VERTICAL DATUM IS BASED BETWEEN APRIL 22 AND MAY 13, 2011 BY CGA FIELD CREW.

DESIGNED BY: H. KORSTICK	CHECKED BY:
DRAWN BY: H. KORSTICK	DATE: DECEMBER 23, 2011
SCALE: 1"=10'	REVISION

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SHEET NUMBER		
1 OF 1		



## EXHIBIT “S”



### Transbay Transit Center – San Francisco, CA

#### Traffic Control Plan

Webcor/Obayashi

WO-TCP0001

**REVISION 2**

**8/22/2012**

## **GENERAL**

The Webcor/Obayashi Joint Venture (W/O) Traffic Control Plan that will be implemented on the Transbay Transportation Center Project is an overall project policy, with each trade subcontractor contributing their specific plan as they come on board to the project. The primary function of this plan is to provide a framework to insure compliance with Specification Section 01 15 70. To assist in this effort, W/O has enlisted the services of a traffic control consultant (TCC) – Sandis Engineering. Award of this contract between Sandis Engineering and W/O was based on a competitive request for proposal (RFP) process referred to as TG05.4.

TCC is responsible for participating in all aspects of traffic control planning and implementation including, but not limited to:

- Traffic control design oversight;
- Coordination between trade subcontractor traffic control designs;
- Interface with City of San Francisco and other agencies as necessary;
- Participate in coordination efforts of the TJPA Representative;
- Oversight of implementation of approved traffic plans;
- Provide daily reports regarding status of traffic control measures;
- On call traffic control services as requested.

## **TRAFFIC PLAN REVIEW AND COORDINATION**

TCC shall prepare a detailed “as built” traffic plan for approximately four blocks in all directions from the jobsite. This map will be based on SFMTA maps and will be augmented as appropriate per field review of existing conditions. This map will include all striping, signage, curb lines, curb cuts, curb painting, buildings and any other feature of the street layout and traffic control. Beyond the four block distance, the map will include street layout and striping configuration.

Once a trade subcontractor is under contract, W/O shall provide the trade subcontractor with the as-built plan in CADD format. The trade subcontractor will then be required to use this base map for preparation of all their traffic control plans. A summary of the below criteria can be found in the attached Traffic Control Plan Preparation Packet.

The trade subcontractor is required to prepare and submit a complete traffic plan consistent with requirements of the project specification and all requirements per the City of San Francisco. The submittal must be made in a timely fashion to allow for the review timeframe prescribed in the specifications plus an additional four weeks for review by the TCC.

Upon receipt of the submittal from trade subcontractor, W/O will forward it to the TCC for review. The plan will be reviewed for adherence to specifications and for compatibility with previously submitted plans. Comments will be returned to the trade subcontractor who will make modifications as is appropriate.

When the trade subcontractor’s traffic control plan is reviewed and coordinated with the TCC, it will be submitted to the TJPA Representative for approval. Submittal will be in compliance with Specification Section 01 15 70, paragraph 1.4B.

Upon approval by the TJPA Representative and SFMTA, the TCC will update the baseline traffic

control plan as appropriate. The baseline plan will be updated only when a change to the traffic pattern will be in place for three or more months. If the traffic control plan will be in place for less than three months, the plan will be superimposed over the base map for coordination but the baseline drawing will not be modified.

#### **FIELD IMPLEMENTATION**

It is intended that the TCC will maintain a regular, but not full time, presence on site. Similar to the traffic control design review, their scope of work is to review the trade subcontractor's adherence to city standards, project specifications and approved traffic control plans.

TCC review and assistance in field coordination includes but is not necessarily limited to:

- Perform site review of traffic control;
- Note traffic control deficiencies;
- Coordinate correction of site deficiencies with W/O and trade subcontractor;
- Provide daily report of traffic control observations and corrective measures;
- Attend site meetings as necessary to review short term Special Traffic Permit and coordinate between subcontractors and SFMTA;
- Miscellaneous coordination with SFMTA as necessary;
- Review of pedestrian protection as it relates to vehicle traffic;
- Provide traffic control devices and personnel as required to augment traffic control efforts;
- Confirm proper training of subcontractor flagging personnel;
- Provide continuous oversight of traffic control for major construction operations as determined by CM/GC.

#### **TASKS NOT CURRENTLY ANTICIPATED BY TCC**

Training of flaggers for the trade subcontractors although it is an option should it become apparent that subcontractor employees need additional training.

Coordination of the 10b police officers between subcontractors will be the responsibility of the CMO.

Pedestrian control unless it is specifically impacted by vehicle traffic.

## **TRANSBAY TRANSIT CENTER – TRAFFIC CONTROL PLAN PREPARATION PACKET**

### ***Overview***

The purpose of this packet is to provide the contractor with the information necessary to prepare a Traffic Control Plan (TCP) for their work in accordance with the requirements of the Project Specifications and the City and County of San Francisco (CCSF). It includes procedures, timing, a base map, plan sheet template and examples for use when preparing and submitting Traffic Control Plans (TCPs) for review and approval. The documents included in the TCP Packet are described below.

### ***Flow Diagram***

The flow diagram included within the TCP packet identifies the specific components and required time intervals for TCP submittal, review and approval. Please note time requirements for Plan review and approval. No work will be allowed without an approved plan. It is the contractor's responsibility to anticipate and allow for required lead times.

### ***Base Map File***

The AutoCAD drawing of the Base Map file included in this packet represents the City of San Francisco street layout as of the date indicated on the Base Map file title block. ALL proposed TCPs shall be created using this Base Map file as a starting point. It is crucial that proposed TCPs be provided on the same coordinate system as the Base Map file so multiple approved TCPs can be overlain in a composite exhibit. TCPs prepared using a different base or plan template will be rejected.

### ***TCP Standards***

#### ***Design Standards***

The Traffic Control Plans shall be prepared and submitted in accordance with the following documents:

1. Transbay Transit Center Project Specification Section 011570 – Traffic Routing Work, dated September 23, 2010. A copy of this specification is included in the TCP Packet.
2. City and County of San Francisco Regulations for Working in San Francisco Streets (Bluebook), 7<sup>th</sup> Edition dated October 2006. Refer to the following link for a copy of this document: <http://www.sfmta.com/bluebook>

#### ***CAD Standards***

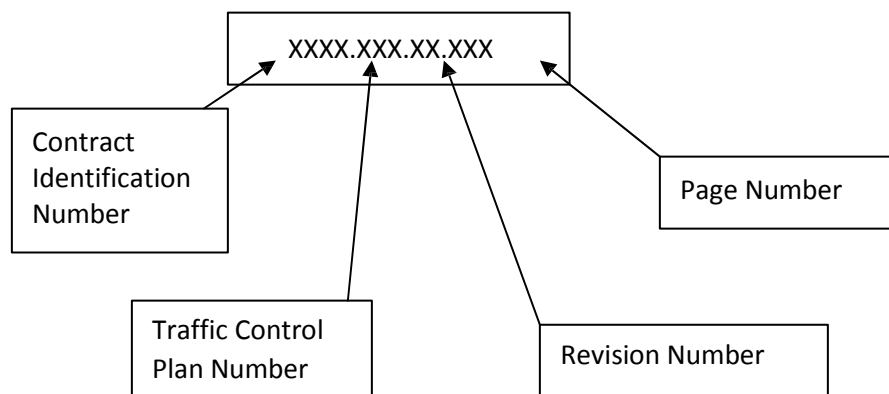
The sheet TCP-001 provides a template with title block, symbols, and specific details pertaining to the presentation and setup of drawings to be used when preparing a TCP. The CAD standards identified under the Vendor Submittal Instructions, including layering configuration, title block, and symbols, shall be referenced and followed when creating all TCP AutoCAD drawings. The contractor shall include additional signs in the form of blocks, notes, and details as needed.

### ***TCP Samples***

There are three sample Traffic Control Plans included in this packet. These samples provide an example of how the TCPs shall be set up and configured.

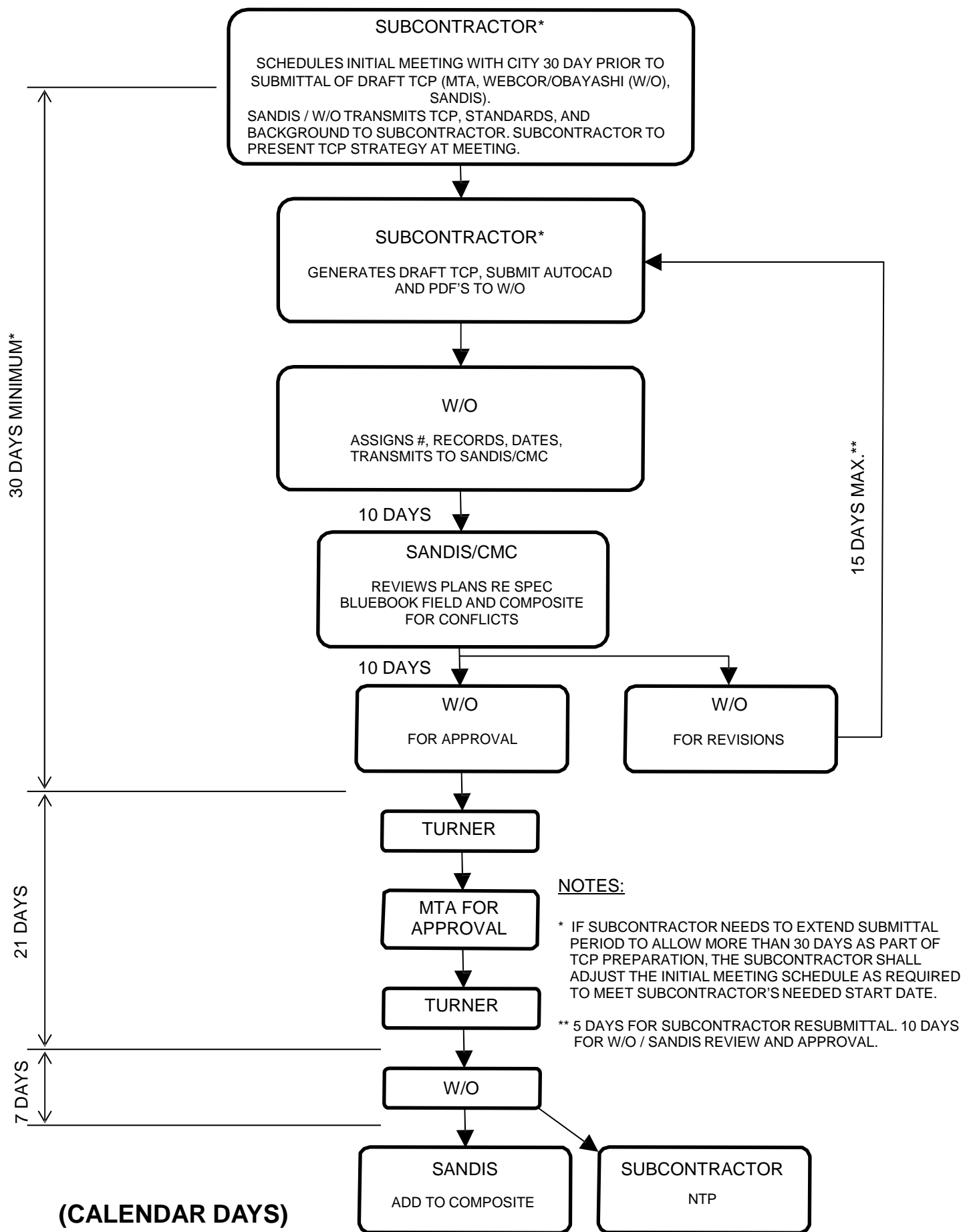
### ***TCP Submittals***

All proposed Traffic Control Plans shall be submitted at 1"=80' scale on 22"x34" sheet size in both pdf and AutoCAD 2007 formats. They are to be submitted electronically to Webcor-Obayashi's trade package project manager. An important item to be included on all TCP sheets is the submittal tracking number. The tracking number consists of four segments separated by a period. The first segment is the 4-digit contract identification number, the second segment the 3-digit TCP number (provided by Webcor), the third segment is the 2-digit revision number, and the fourth the 3-digit page number. Refer to the Submittal Tracking Number Diagram below for additional direction.



Submittal Tracking Number Diagram

# TRAFFIC CONTROL PLAN SUBMITTAL REVIEW AND APPROVAL PROCESS



VENDOR

SEAL

PROJECT 1  
PROJECT 2  
PROJECT 3

PROJECT

XXXX.XXX.XX.XXX

WEBCOR SUBMITTAL No.

No.	REVISION	DATE
X	-----	XX/XX/XX

SCALE: 1"=80'  
DATE: XX/XX/XX

TRAFFIC CONTROL  
STANDARDS

TCP-001

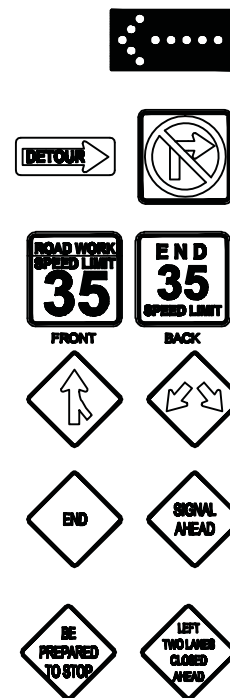
SHEET

## VENDOR SUBMITTAL INSTRUCTIONS

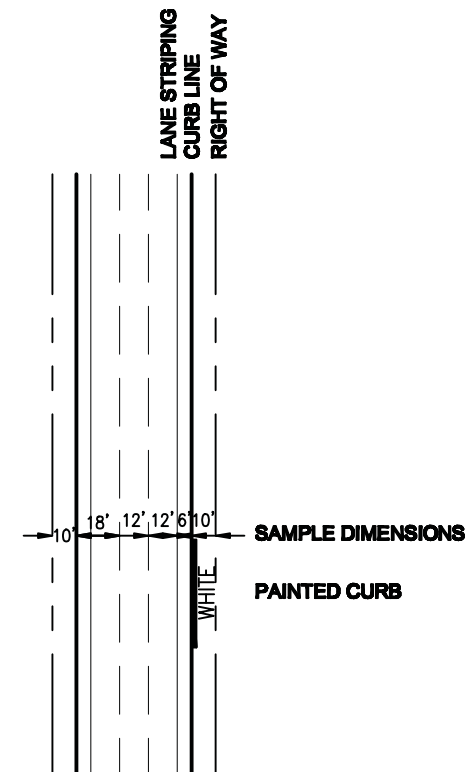
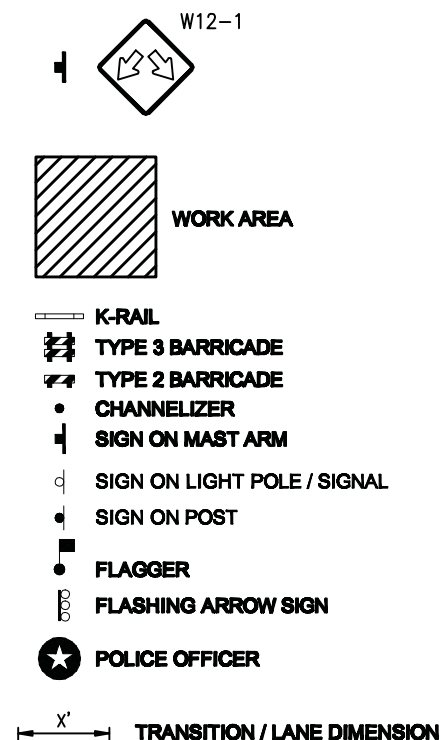
TRAFFIC CONTROL PLANS SHALL BE SUBMITTED AS FOLLOWS:

- 1) FIVE (5) HARD COPIES
- 2) ELECTRONIC COPY IN PDF AND AUTOCAD 2007 FORMATS
- 3) 11"x17" SHEET SIZE
- 4) 1"=80' SCALE
- 5) SHEET NUMBERING "TCP-###"
- 6) ELECTRONIC FORMAT PER TEMPLATE PROVIDED: SINGLE CAD FILE CONTAINING MULTIPLE LAYOUT TABS WITH A SINGLE TCP PER TAB. THE TCP SHALL BE DRAFTED IN MODEL SPACE ON TOP OF THE STREET BASE FILE WITH NOTES/LEGEND IN PAPER SPACE. MODEL SPACE SHALL BE DRAFTED AS FOLLOWS:
  - a) EACH TCP PAGE SHALL CONSIST OF FIVE LAYERS WITH A PREFIX FOR THAT PAGE NUMBER. FOR EXAMPLE, PAGE 001 WOULD CONTAIN THE FOLLOWING LAYERS:

001-TCP-DIM  
001-TCP-NOTES  
001-TCP-SIGN  
001-TCP-SIGNTEXT  
001-TCP-STRIPELINE  
001-TCP-WORKAREA
  - b) ALL SYMBOLS, BLOCKS AND DIMENSIONS SHALL MATCH THOSE ON THIS SHEET IN SIZE, COLOR, AND LAYER. CREATE NEW BLOCKS USING SIMILAR COLOR AND SIZE FOR SIGNS/DEVICES NOT SHOWN HERE.
  - c) INSERT STANDARD TITLE BLOCK PER SHEET. USE ATTRIBUTE EDITOR TO FILL IN TITLE BLOCK WITH APPLICABLE INFORMATION.
  - d) TABLES, NOTES, AND LEGENDS SHALL BE IN PAPER SPACE PER SHEET ON LAYER XXX-TCP-GENERAL, WHERE XXX IS THE PAGE NUMBER
  - e) STANDARD TEXT STYLES, SIZES, DIM STYLES PER TEMPLATE
  - f) STANDARD LAYER COLORS AND NAMES; AND CTB/PEN SETTINGS PER TEMPLATE
  - g) FREEZE LAYERS IN VIEWPORTS AS NECESSARY TO ONLY SHOW THOSE NEEDED FOR THAT INDIVIDUAL SHEET.
- 7) REFER TO PROVIDED SAMPLE TCP PLAN FOR AN EXAMPLE OF THE FORMAT BEING IMPLEMENTED.



NOTE:  
SAMPLE TEXT FOR  
FREESTANDING NOTES.



### APPROVAL TURNER

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TO W/O WITH \_\_\_\_\_  
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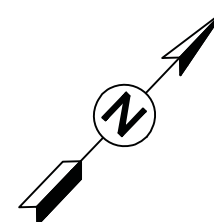
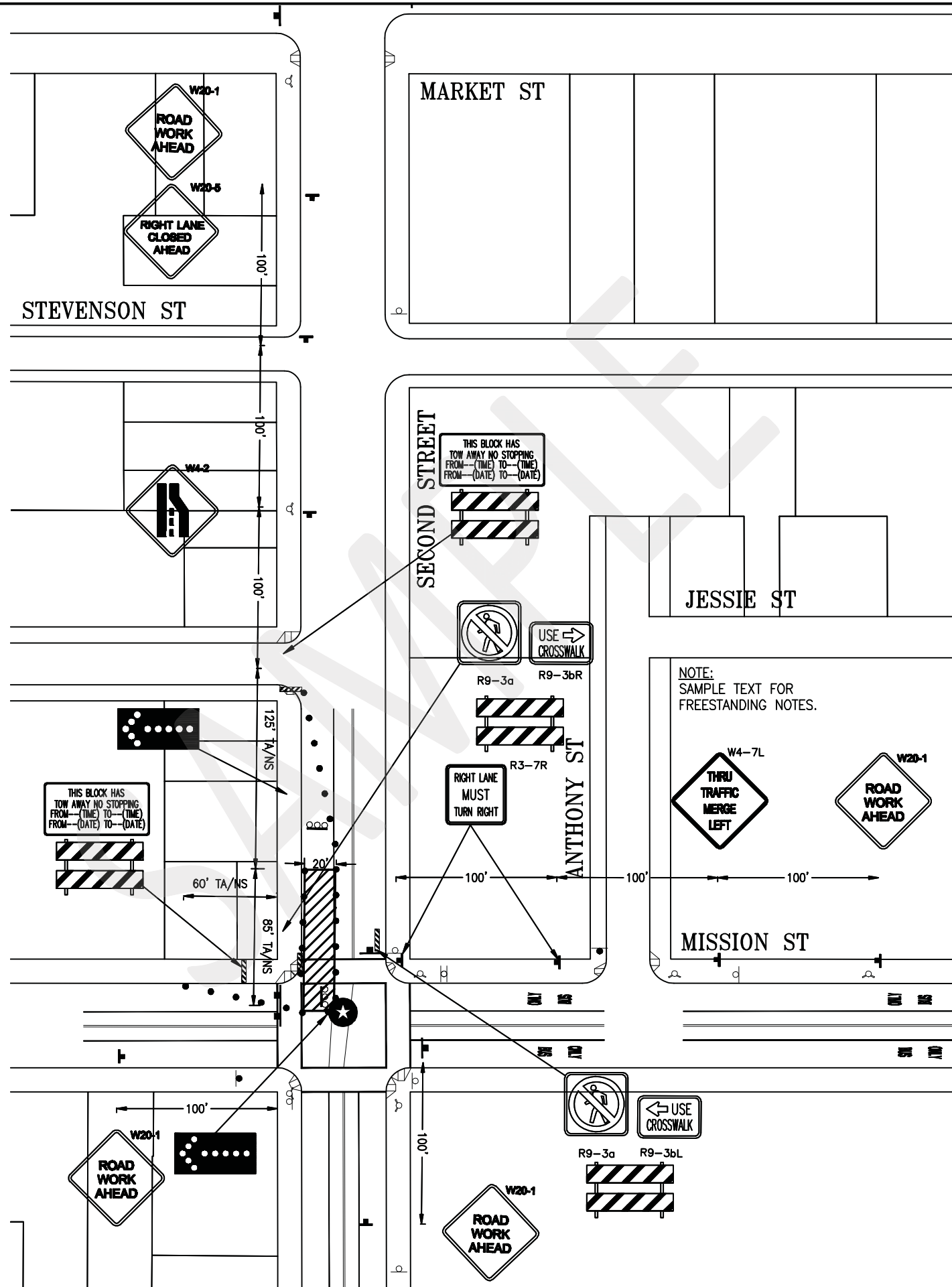
LEGEND

- TYPE II BARRICADE
- TYPE III BARRICADE
- CHANNELIZING DEVICE
- TRAFFIC CONE WITH CLIP ON SIGN
- SIGN
- ARROW PANEL (FLASHING ARROW)
- FLAGGER
- POLICE OFFICER
- WORK ZONE (ACTIVITY AREA) LIMITS

TABLE I				
MINIMUM TAPER LENGTH FOR WIDTH OF OFFSET = 12 FT (3.6m)				
APPROACH SPEED MPH (km/h)	MERGING L FT (m)	SHIFTING L/2 FT (m)	SHOULDER L/2 FT (m)	DOWN STREAM FT (m)
20 (30)	80 (24)	40 (10)	27 (7)	100 (30)
25 (40)	125 (37)	63 (19)	42 (12)	100 (30)
30 (50)	180 (55)	90 (28)	60 (18)	100 (30)
35 (60)	245 (84)	123 (42)	82 (25)	100 (30)
40 (70)	320 (115)	160 (79)	107 (33)	100 (30)
45 (80)	540 (180)	270 (80)	180 (60)	100 (30)
50 (80)	600 (203)	300 (101)	200 (66)	100 (30)
55 (100)	680 (225)	330 (113)	220 (78)	100 (30)
60 (110)	720 (245)	360 (124)	240 (85)	100 (30)
65	780	380	260	100
70	840	420	280	100

APPROACH SPEED (MPH)	MAXIMUM TAPER (FT)	TANGENT (FT)	CONFLICT (FT)
20	21	42	10
25	26	53	13
30	32	65	16
35	37	74	18
40	42	84	20
45	48	95	23
50	55	108	25
OVER 50	74	148	35

NOTES:  
1) WORK HOURS: 9:00 AM TO 3:00 PM



VENDOR

SEAL

PROJECT

PROJECT

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WEBCOR SUBMITTAL No.

No.	REVISION	DATE
X	-----	XX/XX/XX

SCALE: 1"=80'  
DATE: XX/XX/XX

TRAFFIC CONTROL  
STANDARDS

TCP-002

SHEET

APPROVAL  
TURNER

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2ND REVIEW \_\_\_\_\_  
APPROVAL \_\_\_\_\_



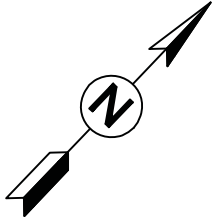
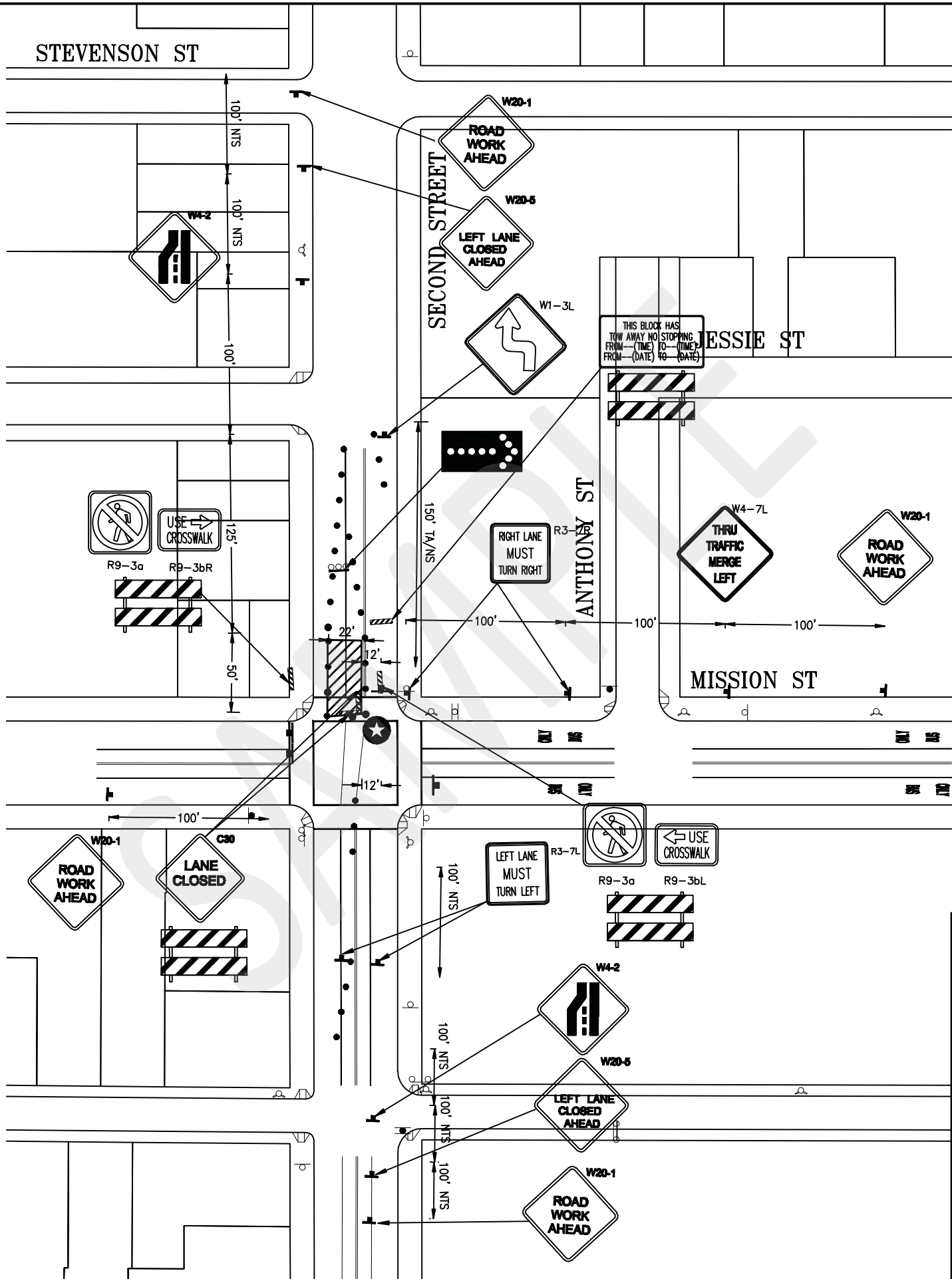
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25 (40)	125 (37)	63 (19)	42 (12)	100 (30)
30 (50)	180 (55)	90 (28)	60 (18)	100 (30)
35 (60)	245 (84)	123 (42)	82 (25)	100 (30)
40 (70)	320 (105)	160 (79)	107 (33)	100 (30)
45 (80)	540 (180)	270 (80)	180 (60)	100 (30)
50 (80)	600 (203)	300 (101)	200 (66)	100 (30)
55 (100)	680 (225)	330 (113)	220 (78)	100 (30)
60 (110)	720 (245)	360 (124)	240 (85)	100 (30)
65	780	380	280	100
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APPROACH SPEED (MPH)	MAXIMUM TAPER (FT)	TANGENT (FT)	CONFLICT (FT)
20	21	42	10
25	26	53	13
30	32	65	16
35	37	74	18
40	42	84	20
45	48	95	23
50	55	108	25
OVER 50	74	148	35

NOTES:  
1) WORK HOURS: 9:00 AM TO 3:00 PM



VENDOR

SEAL

PROJECT

PROJECT

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WEBCOR SUBMITTAL No.

No.	REVISION	DATE
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TRAFFIC CONTROL STANDARDS

TCP-003

SHEET

APPROVAL  
TURNER

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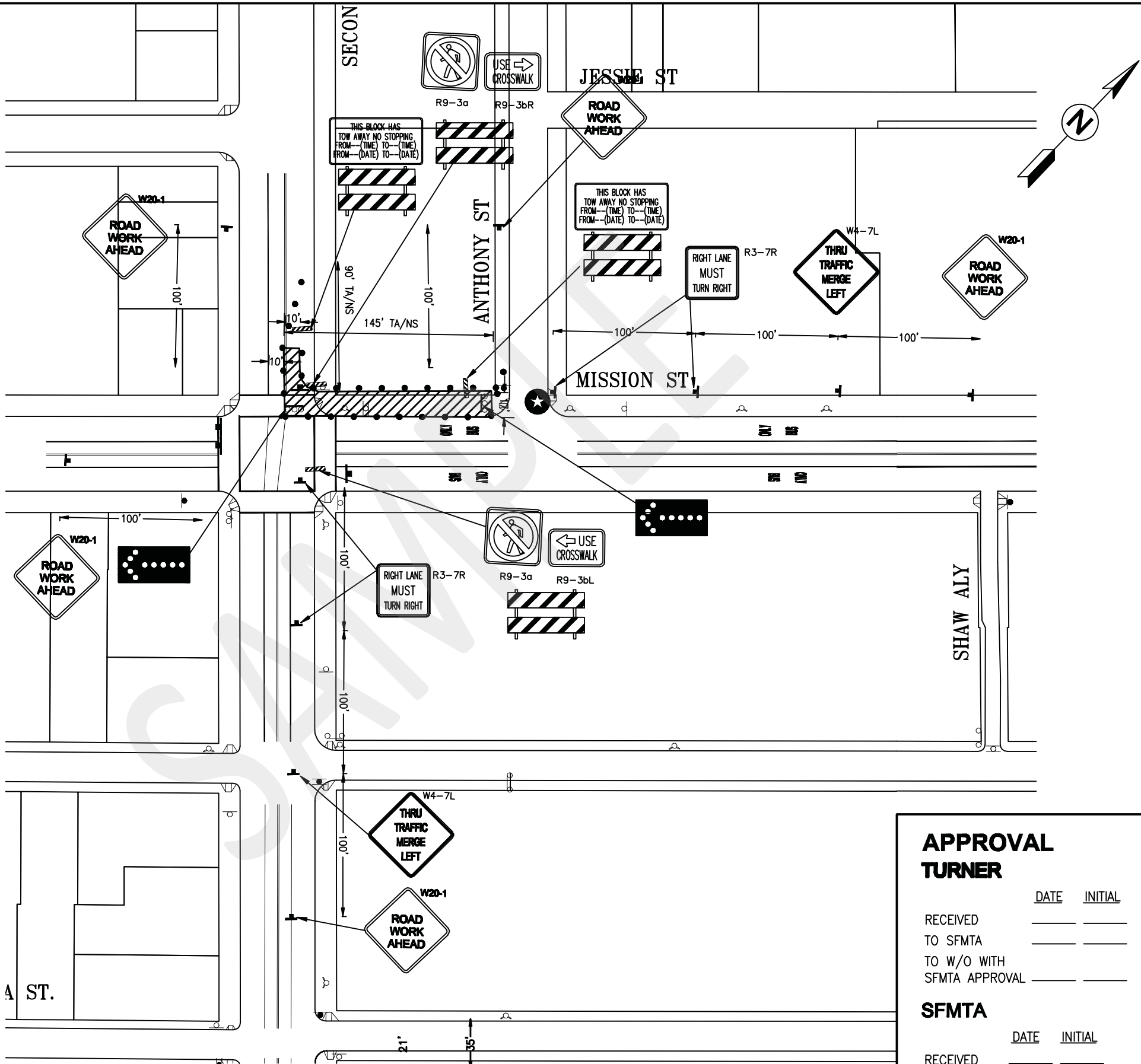
LEGEND

- TYPE II BARRICADE
- TYPE III BARRICADE
- CHANNELIZING DEVICE
- TRAFFIC CONE WITH CLIP ON SIGN
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TABLE L				
MINIMUM TAPER LENGTH FOR WIDTH OF OFFSET = 12 FT (3.6m)				
APPROACH SPEED MPH (km/h)	MERGING L FT (m)	SHIFTING L/2 FT (m)	SHOULDER L/2 FT (m)	DOWN STREAM FT (m)
20 (30)	80 (21)	40 (10)	27 (7)	100 (30)
25 (40)	125 (37)	63 (19)	42 (12)	100 (30)
30 (50)	180 (55)	90 (28)	60 (18)	100 (30)
35 (60)	245 (84)	123 (42)	82 (25)	100 (30)
40 (70)	320 (100)	160 (50)	107 (33)	100 (30)
45 (80)	400 (120)	200 (60)	136 (42)	100 (30)
50 (90)	500 (150)	250 (75)	175 (53)	100 (30)
55 (100)	600 (180)	300 (90)	214 (65)	100 (30)
60 (110)	720 (216)	360 (108)	257 (78)	100 (30)
65	780	390	280	100
70	840	420	280	100

APPROACH SPEED (MPH)	TAPER (FT)	TANGENT (FT)	CONFLICT (FT)
20	21	42	10
25	28	56	13
30	32	64	16
35	37	74	19
40	42	84	23
45	48	96	28
50	55	110	33
OVER 50	74	148	50

NOTES:  
1) WORK HOURS: 9:00 AM TO 3:00 PM



APPROVAL  
TURNER

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2ND REVIEW \_\_\_\_\_  
APPROVAL \_\_\_\_\_



VENDOR

SEAL

PROJECT

PROJECT

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WEBCOR SUBMITTAL No.

No.	REVISION	DATE
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SCALE: 1"=80'  
DATE: 08/10/11

TRAFFIC CONTROL  
STANDARDS

TCP-004

SHEET

## **Exhibit U      Submittal Schedule**

Trade Subcontractor's Schedule submission shall include a full submittal schedule per Specification Section 01 13 00 1.4 – Submittal Schedule.

1. All submittals are to be submitted to Webcor/Obayashi Joint Venture within 60 days of Award.
2. The Submittal Schedule shall contain additional data fields to indicate: 1) the duration in work days for procurement of the item starting from the date that the submittal is approved until the item is available for construction, and 2) the Activity ID of the earliest construction activity for which the item will be required (the submittal/procurement item's successor).
3. The Trade Subcontractor should use the attached data format, Submittal Schedule Excel Template, for the submission of Submittal Schedule as Microsoft Excel File. Contact Webcor/Obayashi Joint Venture to obtain the blank excel file of the Submittal Schedule.
4. The Trade Subcontractor shall show critical submittals in the Exhibit I Construction Schedule in addition to providing the comprehensive submittal schedule required herein. Critical submittals are those submittals considered vital to the timely progression of the project schedule. These items may include, but are not limited to, engineering submissions; long lead items; items required within the first 25% of Subcontractor's performance period; and items that are required for construction or installation of a task with less than 20 working days of total float in the overall project schedule. The last group of items may not be determined until after acceptance of the Trade Subcontractor Construction Schedule submission and its full incorporation into the project schedule. Therefore, the Subcontractor may be required to add items to its Primavera schedule file subsequent to approval of its Construction Schedule submission.

PMFSM_COMP_CODE	PMFSM_PROJ_CODE	PMFSM_SMT_ID	PMFSM_SMT_NAME	PMFSM_PKG_CODE	PMFSM_REC_FROM_PARTN_ABBREV	PMFSM_REC_FROM_CONTACT_COD	PMFSM_RET_BY_PARTN_ABBREV	PMFSM_RET_BY_CONTACT_COD	PMFSM_SENT_TO_PARTN_ABBREV	PMFSM_SENT_TO_CONTACT_COD	PMFSM_FWD_TO_PARTN_ABBREV	PMFSM_FWD_TO_CONTACT_COD
Always 30	Project #	Submittal #	Submittal Name		Received From Partner Abbreviation(Sub)	From Contact Code(Sub)	Return By Partner Abbreviation (Architect)	Returned By Contact Code(Architect)	Sent To Partner Abbreviation (Architect)	Sent to Contact Code (Architect)	Forward To Partner Abbreviation(Sub)	Forward to Contact Code(Sub)
	30	30100	Test Submittal	TG####001	ADERH023	BOBBROZ	TURNER		TURNER			

PMPSM_REQUIRED_START_DATE	DATE_CD=NON-RRRR	PMPSM_REQUIRED_END_DATE	DATE_CD=	PMPSM_CLV_VALUE_CODE1	PMPSM_CLV_VALUE_CODE2	PMPSM_CLV_VALUE_CODE3	PMPSM_CLV_VALUE_CODE4	PMPSM_CLV_VALUE_CODE5	PMPSM_CLV_VALUE_CODE6	PMPSM_SMT_STATUS_CODE
DO NOT USE		DO NOT USE		LEED MR 1 (See Sheet 2)	Credit Specific Data	LEED MR 2 (See Sheet 2)	Credit Specific Data	LEED EQ (See Sheet 2)	Credit Specific Data	Use PEND PEND

PMFSM ACTIVITY START DATE	DATE DD-MON-RRRR	PMFSM DATE CHANGE CODE	PMFSM COPIES NUM	PMFSM LEAD TIME STAGE1	PMFSM LEAD TIME STAGE1	PMFSM LEAD TIME STAGE4	PMFSM LEAD TIME STAGE3	PMFSM LEAD TIME STAGE2	PMFSM LEAD TIME STAGE1	PMFSM SPEC SEC CODE	PMFSM SORT ORDER NUMBER	PMFSM CLOSED DATE	DATE DD-MON-RRRR
		DO NOT USE	Number of Copies	Lead time Delivery	Lead Time Fabrication	Lead Time Float	Lead Time Arch Review	Lead Time Webcor Review	Lead Time From Sub to Web	Spec Section	DO NOT USE	DO NOT USE	
			6	5	8	5	21	5	5				

PMF5M_TYPE_CODE	PMF5M_ITEM_SUBSEC_CODE	PMF5M_PROC_FLG	Schedule Activity ID
Type Code(See Sheet 2)	Spec Sub Section	Procurement Flag Choose Yes or No	

# Code List for Submittal Schedule Data

1 / 4

Submittal Types	
3DCORD	3D Coordination
ASBUILT	As Built Drawings
ATTIC	Attic Stock
BRUSH	Brushouts
CALC	Calculations
CERT	Certificates
CLOSE	Close Out
COMM	Commissioning
DRAW	Shop Drawings
LEED	LEED Documentation
METHODS	Methods
MOCK	Mock Up
MSDS	MSDS Documentation
PRIME	Prime Level
PROD	Product Data
QUAL	Qualifications
SAMPLE	Samples
SCHED	Schedules
SURVEY	Survey
TEST	Test Reports



**LEED MR1**

Code	
NC MR 3	Material Reuse
NC MR 4	Recycled Content
NC MR 5	Regional Materials
NC MR 6	Rapidly Renewable Materials
NC MR 7	Certified Wood
CS MR 3	Material Reuse
CS MR 4	Recycled Content
CS MR 5	Regional Materials
CS MR 6	Certified Wood
CI MR 3	Resource Reuse
CI MR 4	Recycled Content
CI MR 5	Regional Materials
CI MR 6	Rapidly Renewable Materials
CI MR 7	Certified Wood

**LEED MR2**

Code	
NC MR 3	Material Reuse
NC MR 4	Recycled Content
NC MR 5	Regional Materials
NC MR 6	Rapidly Renewable Materials
NC MR 7	Certified Wood
CS MR 3	Material Reuse
CS MR 4	Recycled Content
CS MR 5	Regional Materials
CS MR 6	Certified Wood
CI MR 3	Resource Reuse
CI MR 4	Recycled Content
CI MR 5	Regional Materials
CI MR 6	Rapidly Renewable Materials
CI MR 7	Certified Wood

**LEED EQ**

Code	
NC EQ 4.1	Low-Emitting Materials: Adhesives & Sealants
NC EQ 4.2	Low-Emitting Materials: Paints & Coatings
NC EQ 4.3	Low-Emitting Materials: Carpet Systems
NC EQ 4.4	Low-Emitting Materials: Composite Wood & Agrifiber Products
CS EQ 4.1	Low-Emitting Materials: Adhesives & Sealants
CS EQ 4.2	Low-Emitting Materials: Paints & Coatings
CS EQ 4.3	Low-Emitting Materials: Carpet Systems
CS EQ 4.4	Low-Emitting Materials: Composite Wood & Agrifiber Products
CI EQ 4.1	Low-Emitting Materials: Adhesives & Sealants
CI EQ 4.2	Low-Emitting Materials: Paints & Coatings
CI EQ 4.3	Low-Emitting Materials: Carpet Systems
CI EQ 4.4	Low-Emitting Materials: Composite Wood & Agrifiber Products
CI EQ 4.5	Low-Emitting Materials: Systems Furniture & Seating



# QUALITY COMMISSIONING PROCEDURES AND GUIDELINES

## Exterior Skin and Waterproofing Systems

### EXHIBIT "W"

*The information, processes, techniques, material and other matters contained in the Quality Commissioning Procedures and Guidelines are proprietary, confidential, and unique to WEBCOR/OBAYASHI.*

***The Quality Commissioning Procedures and Guidelines shall only be used for WEBCOR/OBAYASHI only.***

Any other use without the expressed written consent from an Officer of WEBCOR/OBAYASHI is prohibited. Any unauthorized use could give rise to liability under the California Civil Code Sections 3426 et seq. involving Uniform Secrets Act, the California Business and Professions Code Sections 17200 et seq. involving Unfair Competition and 17500 et seq. involving Unfair Practices, the common law of unfair competition and interference with contractual relations and prospective advantage.

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## QUALITY COMMISSIONING PROCEDURES AND GUIDELINES

### Exterior Skin and Waterproofing Systems

- ✓ Roofs
- ✓ Decks
- ✓ Windows
- ✓ Curtain Walls
- ✓ Exterior Wall Systems (Precast, Stucco, EIFS, GFRC)
- ✓ Water Shedding Systems
- ✓ Flashings
- ✓ Expansion Joints
- ✓ Caulking, Sealants
- ✓ Primary and Secondary Water Barrier Systems
- ✓ Above & Below Grade Waterproofing
- ✓ General Waterproofing Systems

#### 1.0 Purpose

The purpose of this procedure and guideline is to set forth a commissioning process, which will ensure that the building's exterior envelop and waterproofing systems perform and function in conformity with design intent and to provide a means of verifying the implementation of these systems based on the project specifications, design and applicable industry standards.

#### 2.0 Definition of Commissioning

The term "Commission" refers to a Quality Assurance process by which the building's exterior envelop and waterproofing systems (i.e., below and above-grade waterproofing, decks, roofs, caulking, plaster, precast concrete and GFRC, curtain-wall, flashing, expansion joints, etc.) are provided, installed and tested in order to verify the systems perform in accordance with the contract documents and the design intent.

Commissioning entails the development of a clear and complete process that verifies the systems design and operational intent. It also is to verify that the exterior envelop and waterproofing systems and its components are installed according to the contract documents, manufacturer's recommendations and published industry standards and that the system receives adequate installation and performance inspections by the installing contractor.

The process must include verifying and documenting the installation steps, phases, and system performance with respect to the design intent and the contract documents. Commissioning is a team effort that requires cooperation by all parties to succeed.

#### 3.0 Description of the Commissioning Process

Commissioning is a **"systematic"** process for achieving, validating and documenting the performance of building systems as so that it meets the design intent and requirements.

The process extends through all phases from design to occupancy, and extending through the warranty period. Numerous checks and inspections shall be performed at each stage of the process to ensure that established procedures are followed. The process also includes training of facility operational personnel to ensure continued efficient use of the exterior envelop and waterproofing systems as originally designed and installed.

This guideline provides a uniform, integrated and consistent approach for the commissioning of all waterproofing systems as well as assisting in insuring product and design compatibility. Since many building waterproofing systems are integrated, a deficiency in one system or component may result in sub-optimal performance and failure among others.

#### 4.0 Commissioning Plan

Commissioning is a “**Quality Process**” for validating the system and component design performance.

The reports from the commissioning process are not just test reports, but reports that document design, installation, inspections, and particular tests and or evaluation procedures. The commissioning plan is continually updated to reflect changes in program and design of the waterproofing system(s). Commissioning reports shall document and record the results of the commissioning process.

Each Trade Subcontractor’s specific commissioning plan must be neatly organized in a consistent manner that reflects the nature of the building systems and their performance. The commissioning plan shall include schedules, requirements and procedures.

Trade Subcontractor(s) shall be responsible for the timely and efficient completion of all commissioning in accordance with the Subcontract Agreement.

**At no time shall any work be permitted to commence without a WEBCOR/OBAYASHI’ approved Trade Subcontractor Waterproofing Commissioning Program.**

Failure to do so may require Trade Subcontractor to assume all related costs and expenses in accordance with the Subcontract Agreement.

In addition, Trade Subcontractor may also be required to assume all related cost should WEBCOR/OBAYASHI find it necessary to develop, manage and or perform any Trade Subcontractor commissioning work.

#### 5.0 Objectives

The fundamental objectives of the commissioning process are:

- 5.1 Create a procedure to verify and provide documentation that the waterproofing performance of the facility meet the design requirements.

- 5.2 Enhance communication by documenting data and decisions throughout all phases of the project.
- 5.3 Validate and report that the performance of waterproofing systems meets design intent.
- 5.4 Provide a means of Quality Control and Quality Assurance (QA/QC) throughout all phases of the waterproofing system(s) installation, inspection, and testing process.

## **6.0 Contractors Normally Participating in the Commissioning Process**

- ✓ Waterproofing Consultant
- ✓ Architect
- ✓ Structural Engineer
- ✓ Mechanical
- ✓ Plumbing
- ✓ Electrical
- ✓ Fire Sprinkler
- ✓ Glass Systems
- ✓ Caulking
- ✓ Brick, Tile, Precast, GFRC, and Stone
- ✓ Fountains and Ponds
- ✓ Swimming Pools & Spas
- ✓ Roofing
- ✓ Insulation
- ✓ Flashing & Sheetmetal
- ✓ Waterproofing Contractors
- ✓ Concrete (If waterproofing admixtures are included by design)
- ✓ Stucco, EIFS, DEFS systems
- ✓ Elastomeric Painting
- ✓ Rough Carpentry (Wood cladding)
- ✓ Architectural Metal Cladding
- ✓ Expansion Joint Systems
- ✓ Water Tanks
- ✓ Special Systems or Components

## **7.0 Commissioning Team**

The commissioning team members may consist of the following:

- ✓ WEBCOR/OBAYASHI - Project Team as required



- ✓ Owner - Designated representative of the owner, building operator/engineer, and/or the owner's construction management firm
- ✓ Engineers - Architect and Designers
- ✓ Waterproofing Contractor
- ✓ Waterproofing Consultant
- ✓ Flashing / Sheet Metal Contractor
- ✓ Exterior Skin Contractor
- ✓ Roof Contractor
- ✓ Glass and Curtain Contractor
- ✓ Caulking and Sealants Contractor
- ✓ Commissioning Agent (CA)
- ✓ Mechanical Contractor
- ✓ Plumbing Contractor
- ✓ Fire Sprinkler Contractor
- ✓ Electrical Contractor
- ✓ Testing Contractor
- ✓ Other as necessary

## 8.0 Meetings

Regularly scheduled commissioning meetings of **the entire team** shall be conducted for site coordination, communicating issues of concern, resolving conflicts, reporting on system process and status, identifying urgent work and all deficiencies.

Commissioning meetings are critical to the **Quality** of the commissioning process as well as timely completion of the project.

## **9.0 Trade Subcontractor Performance Requirements**

- 9.1 Designation of the primary person who will be responsible, accountable, and act as the main contact person for all commissioning communications. Provide organizational chart indicating personnel who will be involved in the project. The chart should indicate factory, office, and on-site field personnel.
- 9.2 Review of drawings and specifications for completeness, appropriateness of details, and acceptance by Trade Subcontractor thereof.
- 9.3 Review WEBCOR/OBAYASHI standard details.
- 9.4 Preparing and submitting documentation of Trade Subcontractor's respective materials and systems to be integrated into the overall Commissioning Plan.
- 9.5 Submitting information on the intended commissioning protocol used on materials, and the integration into the system as a whole.
- 9.6 Provide a presentation of the commissioning process to WEBCOR/OBAYASHI, the Owner and or the owner's representatives. Demonstration shall indicate compliance with the Trade Subcontractor Commissioning requirements as outlined in this document.
- 9.7 Submitting shop drawings detailing waterproofing system layout as outlined in the contract documents. Shop drawings shall reflect all conditions present in the building, including but not limited to the following:
  - a. Conditions where different materials meet (i.e. windows to plaster or stone to plaster).
  - b. Corner conditions.
  - c. Conditions where vertical planes meet horizontal planes (i.e. soffits and sills).
  - d. Expansion joints and control joints.
  - e. Flashing.
  - f. Penetrations (i.e. Z-ducts, electrical outlets, louvers).
  - g. Conditions typically utilized by Trade Subcontractor's common practices.

Shop drawings shall include installation drawings indicating the planned sequence of installation of all components.
- 9.8 Providing means and method for preliminary testing of the exterior envelop and waterproofing systems with manufacturer's representative present as required:
  - a. Caulking: Include complete coordination with the caulking manufacturer's representative to assure compatibility of the caulking system with the

surrounding substrate and finishes. Trade Subcontractor shall submit caulking samples including manufacturer's specifications for materials, color, cleaning procedure, required primers, proper backer rod, installation procedures, testing requirements and results. Testing of caulking samples between all combinations of materials shall be performed by qualified testing agencies in direct accordance with A.S.T.M. Standard Test Method C794 (75), including seven (7) day immersion. A letter from the Caulking Manufacturer shall be submitted approving all testing procedures, the installation procedure and the use of the specified materials for the intended application. Any materials installed without such approval that may be in conflict with the approved procedures or of unacceptable color and appearance will be removed and replaced at the Trade Subcontractor's expense.

- b. Windows and Sliding Glass Doors: Assemblies shall be field tested in accordance with American Architectural Manufacturers Association (AAMA) 502-02 Voluntary Specification for Field Testing of Windows and Sliding Glass Doors using Test Methods A and B, testing a minimum of 1% of the products for air leakage resistance and water penetration resistance as specified for various stages of the product installation.
- 9.9 Reviewing all required testing under the witnessing of WEBCOR/OBAYASHI, Building Owner, and or the Owners representatives.
  - 9.10 Correcting all system deficiencies at Trade Subcontractor expense.
  - 9.11 Obtaining all required permits, code required inspections and final certifications.
  - 9.12 Preparing complete as-built record drawings made from an original set that has been marked up throughout the duration of the project. Drawings must indicate all work as it was actually installed showing change order revisions, field changes required to meet the working conditions, and any other items that will affect or reflected in the operation and maintenance of the facility.
  - 9.13 Obtaining all manufacturer's warranties and guarantees.
  - 9.14 Organizing the O&M manuals, if any, from suppliers and manufacturers.
  - 9.15 Performing any specified training for the facility's operational staff.

## 10.0 Information Management

The management and continued organization of the commissioning information shall be the sole responsibility of the Trade Subcontractor.

WEBCOR/OBAYASHI and the Trade Subcontractor shall mutually agree on the location where all the commissioning information and documentation shall be stored.

The Trade Subcontractor shall make every effort to continually update and manage the information throughout the commissioning process. WEBCOR/OBAYASHI and the Building Owner may review the commissioning information provided by the Trade Subcontractor at any time for updates, accuracy and completeness.

WEBCOR/OBAYASHI may elect to withhold or make appropriate adjustments to the Trade Subcontractor's monthly progress billing in the event the commissioning information or performance requirements as described in the Waterproofing Quality Commissioning Procedures & Guidelines are not being performed, managed and updated by the Trade Subcontractor.

## 11.0 Trade Subcontractor Commissioning Submittal Requirement

Each Trade Subcontractor has a responsibility to WEBCOR/OBAYASHI and the Building Owner to comply with the terms of the contract and to verify that the design intent of the waterproofing systems for the project is achieved.

Each Trade Subcontractor is required to provide two completed commissioning manuals containing the information outlined in Section 19 - Commissioning Binder Tab Index of this guideline. Each proposed formatted "3-ring" binder containing all information, including blank forms shall be provided to WEBCOR/OBAYASHI and the Owner for "**review and comment**" before the commissioning process begins, or by an agreed upon date.

WEBCOR/OBAYASHI, the Owner and the owner representative shall review the information and return it to the Trade Subcontractor within **two-week** time with all comments.

Each Trade Subcontractor shall make all required changes as agreed, to the commissioning manuals and resubmit them to WEBCOR/OBAYASHI within **two-weeks**.

Each Trade Subcontractor shall schedule and provide a formal demonstration of their commissioning process to WEBCOR/OBAYASHI, the Owner and the Owners representative after all required changes to the manuals have been satisfactory completed. Demonstration shall indicate compliance with the Trade Subcontractor Waterproofing Commissioning requirements as outlined in this document.

Each Commissioning Manual **shall be neatly organized** using appropriate tabs, dividers, table of content, index, etc. as required for easy referencing. Refer to Section 19 Commissioning Binder Tab Index for a standard binder organization. All Commissioning Manual(s) **must be user friendly**.

## 12.0 Commissioning Binder Tab Index

- Tab 1. Project design criteria specifications** – Provide information that describes the overall design criteria and performance requirements for the waterproofing system(s).
- Tab 2. Manufacture products and components** – Provide complete submittal list of all components that shall be contractually provided and installed.
- Tab 3. Manufacture installation instructions** – Provide manufacture documentation insuring that the system and components installation complies with all Manufacture requirements to maintain performance and guarantee obligations.
- Tab 4. Manufacture details** – Provide manufacture details or published industry standards for penetrations and terminations interfacing with other installed systems.
- Tab 5. Design transition review** – Provide design review comments and concerns on transition interfaces to other s or other compatibility issues.
- Tab 6. Quality Assurance / Quality Control Program** – Provide QAQC program with complete field inspections and checklists.
- Tab 7. Documentation** – Trade Subcontractor shall maintain a separate field binder documenting the QAQC inspections and field-testing for all installed work.
- Tab 8. Field mock-up and testing** – Provide information on mock-up or field performance tests that shall be preformed for all installed system(s). Provide manufacture recommendations or published testing standards used. If no performance testing is preformed, Trade Subcontractor shall provide documentation on how each system is performing in accordance to the documented design intent and contract warranty requirements.
- Tab 9. Schedule** – Provide schedule for, shop drawing devolvment, submittals fabrication, delivery and installation.
- Tab 10. Agency and factory test reports** – Provide all factory, agency, and field performance-testing reports on installed systems.
- Tab 11. Factory and Trade Subcontractor guarantee information** – Provide warranty responsibilities and durations for all systems and components installed.
- Tab 12. Owner Training** – Provide (O&M) and training for all required service and maintenance requirements as it extends throughout each system to maintain warranty. Include owner sign-off sheets verifying training.

- Tab 13. Attic Stock** – Provide list of spare material that shall be supplied by Trade Subcontractor to owner – Paint, applied materials, gaskets, handles, glazing, or patching products.
- Tab 14. As-Built Drawings** – Provide completed set of drawing and details accurately reflecting all installed and completed work.
- Tab 15. Material Safety Data Sheets** – Provide all Material and Data Safety Sheets (MSDS).

### 13.0 Identifying the Defects

It is the intent of the commissioning process to avoid defects in waterproofing systems. A standard of care exhibited during the commissioning process should anticipate potential defects and determine appropriate solutions prior to the installation of these systems. In the event that defects do occur, proper defect identification will help determine the repair needed and assist in selecting the appropriate method and materials.

It is important to acknowledge which factors have caused deficiencies in the waterproofing system and its components, and how a deficiency in one system may influence or amplify another. Careful and thorough defect identification is critical to obtain long-lasting, quality repairs. It is critical and necessary to eliminate the cause of the defect and not solely treat the symptom.

Each Trade Subcontractor shall be responsible for determining the cause and origin of various problems as it pertains to their contractual scope of work. Failure to do so may require Trade Subcontractor to assume all related costs and expenses for damages, repairs performed by others, testing, special inspections, and consultant fees.

### 14.0 Applicable Industry Standards

Unless the Contract Documents include more stringent requirements, applicable published construction industry standards shall be utilized. Where compliance with two or more standards is specified for quality or quantity levels, comply with the most stringent requirement.

Where sections of the specifications require that a product, material, installation, or test complies with a specified industry standard, the Trade Subcontractor shall obtain copies directly from the publication(s) source and include the information in the submitted commissioning information.

Each Trade Subcontractor engaged in construction on the project must be familiar with published industry standards applicable to their construction activity.

### 15.0 Schedules

An initial schedule shall be developed by the Trade Subcontractor identifying dates, times, and durations for shop drawings, approval of submittals, material fabrication, product delivery, acceptance, installation, testing and completion.

The schedule shall also include any commissioning task that shall be performed on waterproofing systems that may involve or affect other related building systems.

Each Trade Subcontractor shall update schedules, daily, weekly, monthly, or as required to keep WEBCOR/OBAYASHI and the Owner informed of the activities performed. This schedule will indicate appropriate milestones during the installation to allow WEBCOR/OBAYASHI and or the Owner the ability to observe and witness system installations prior to being covered up by subsequent s. The schedule will indicate milestone dates for Trade Subcontractor inspection and testing.

#### **16.0 Execution of Inspections and Checklists**

Trade Subcontractor and or vendors shall schedule initial inspections and checklist review with the commissioning team. The inspections and reviews shall be directed, executed, and documented by the Trade Subcontractor or vendor.

To document the process, the Trade Subcontractor performing the task shall provide and complete all documentation forms and checklists. (See attached sample checklist)

#### **17.0 Field Inspections**

One of the most important commissioning activities for waterproofing systems is field inspections. The field inspection process shall serve as a method and means of documenting the installation process as well as indicate variations between contractual design and construction.

Each Trade Subcontractor shall identify in detail the scope of their field inspections, and the types of field procedures that will be required to obtain the necessary information to provide a complete waterproofing quality control evaluation at the completion of the job.

#### **18.0 Field Witnessing of Trade Subcontractor's Quality Control**

WEBCOR/OBAYASHI, the Owner, consultants and the Architect reserve the right to witness the waterproofing system installation at any time. Spot checks shall be conducted on a random basis. If inconsistencies are discovered in quality, performance, or if commissioning information differs from those submitted, the Trade Subcontractor may be required to completely remove and remedy all conditions where the inconsistencies occurred at no additional cost or impact to the schedule.

Witnessing shall include all or part of, but not limited to the following:

- 14.1 Mock ups
- 14.2 Waterproofing component and system installation
- 14.3 System inspection and checks
- 14.4 Performance tests

## 14.5 Special Inspections

## 19.0 Documentation

Trade Subcontractor shall maintain a separate field binder documenting quality control inspections and field-testing for all installed work. Documentation shall include dates, quality control field checklist, reports with inspected locations defined by grid lines and elevations. Provide a dated photo log, documenting inspected areas and general sequence of installed work for the duration of the project.

## 20.0 Testing and Methods

The objective of field-testing is to correlate paths of moisture infiltration and to observe the source of damages. Moisture entering a building during extreme weather may be obvious, but the most reliable method to discover the infiltrating path is to recreate the leakage condition in a controlled manner. Testing also allows verification of the theory for the cause of leakage.

As all system and component tests are unique to some degree, there may not be one standard or method for testing that can be applied to all. There are several methods, standards, governing requirements, and manufacture recommendations, etc., which should be applied.

There are three types of acceptable testing methods that can be used during the investigation. All of which must be approved by WEBCOR/OBAYASHI. These testing categories include:

- ✓ Non-Destructive Testing
- ✓ Destructive Testing
- ✓ Laboratory Testing

### 20.1 Non-Destructive Testing

Non-destructive testing uses a variety of non-invasive tools. This type of testing causes little or no damage or interference to the building envelope. The various methods of non-destructive testing include:

- a. *Rilem Tube* - This calibrated device is adhered to exterior masonry walls to determine the porosity and condition of brick masonry units, mortar joints, head joints, and embedment joints.
- b. *Water Spray Rack (ASTM E1105)* - This test simulates a wind-driven rain condition on a facility. It can assist in determining the specific cause and origin of moisture infiltration when it is used to test independent components of the envelope. Spraying water over a large area in an uncontrolled fashion will not reveal specific causes of water infiltration.
- c. *Hose Spray Test (AAMA 501.2)* - This test method also simulates wind-driven rain in small segmented areas using a standard garden hose in which a calibrated nozzle is attached with a pressure gauge. The spray is



directed at a specific joint, crack, or defect to reveal potential moisture intrusion.

- d. *Differential Pressure Test (ASTM E1105)* - A pressure chamber is constructed on the interior of the facility at a specific location to test moisture driven through an assembly or component. The assembly or component is subjected to a negative force while simultaneously a spray rack is directed at the assembly to draw the moisture into the facility to simulate a negative pressure under a wind-driven rain condition.
- e. *Infra-Red Thermography* - Infra-red Thermography photographs the building exterior to determine the locations of wet components. Components, such as insulation and sheathing, etc., will act as heat sinks if they contain high levels of moisture. During the day, moist and dry components absorb heat. At night, the moist areas release the heat much slower than the dry areas. By reading the heat signature, Infrared Thermography will help expose the problem areas. Small test cuts may be required to verify moisture areas.
- f. *Soundings (ASTM D4580)* - There are different ways to perform sounding tests including the hammer tap test. In this test, a 16 oz. hammer is tapped against concrete for sound. A hollow sound indicates areas where the concrete has separated from the reinforcing steel, typically due to exfoliation or corrosion of the steel. Another method of sounding is to chain drag a heavy 15 ft. link chain along a concrete surface to listen for hollow sounds, indicating defective concrete. This method can cover larger areas effectively and is commonly used on parking garages and loading docks.
- g. *Pachometer Survey* - This test uses a magnetic device used to locate embedded steel reinforcement and help determine the concrete cover over the reinforcement. Generally, the Pachometer is fairly accurate when measuring ¼ inch to 3-inch thick concrete cover and when reinforcing placement is not too congested.
- h. *Poly-sheet Tape-down* - This test determines the presence of moisture coming through a concrete surface, typically a slab-on-grade type of assembly where the typical problem is tile or membrane separation from the floor. A 2' x 2' section of polyethylene is sealed to the concrete with duct tape and removed 24 hours later. If there is moisture beneath the polyethylene, it is a good indication that there is a vapor drive through the concrete section.
- i. *Glass-Slide Epoxy or Crack-o-meter* - This device is sealed in place over a crack and periodically checked to determine if any movement has occurred. If movement has occurred, the glass will crack or the meter will record movement.
- j. *Optical Illuminated Boroscope* - A boroscope is inserted into a 5/8-in. diameter pilot hole through an exterior wall system and allows the cavity walls of brick veneer, stud wall backup of exterior insulated finish systems (EIFS), or other types of constructions to be observed without large-scale destructive testing.

- k. *Smoke/Dust Tracer* - The smoke/dust tracer helps to find air infiltration. It is moved across the interior face of a window to observe the smoke and dust particles coming through the assembly.
- l. *Moisture Meter* - A Delmhorst meter is a digital device that detects the presence of moisture in various building components. This test is typically accompanied by a gravimetric analysis (oven drying of samples), which is used to confirm the results of the Delmhorst meter.
- m. *Flashlight and mirror* - These simple tools can be very useful to detect problem areas. Placing the mirror into the plenum or behind difficult-to-access areas with the flashlight will allow observation of concealed conditions.

## 20.2 Destructive Testing

When the main objective is to determine the existing composition and configuration of concealed assembly conditions, destructive testing may be warranted. The most common methods of destructive testing are test cuts and borings.

Any type of destructive testing must be reviewed and approved by WEBCOR/OBAYASHI.

- a. *Roof Testing* - Test cuts in the roof assembly may be necessary to determine the condition of the underlying insulation and substrate. Cutting into the system may help verify whether roofing problems are causing corrosion of the steel deck, or a spalled and cracked concrete deck, etc. Test cuts may also expose the as-built configurations of the flashing components at roof-to-wall locations, curb locations, etc. This information is critical to the appropriate remedial design and/or repairs.
- b. *Exterior Wall/Skin Testing* - Test cuts on exterior walls may be required to identify the origin of moisture infiltration. For masonry walls, it is most effective to make test cuts at window heads and sills, and at any through-wall flashing locations that may be suspected of allowing moisture intrusion. Masonry test cuts may expose defective through-wall flashing that is allowing moisture intrusion. Test cuts may also help determine the underlying conditions of the steel components in wall systems, including wall ties, reinforcing steel, sub-steel columns, etc.

## 20.3 Laboratory Testing

Destructive testing is also used to obtain samples for lab analysis. Samples of sealants, coatings, painted finishes, roofing materials, etc. can be sent to a laboratory to determine the presence of lead or asbestos. Samples of masonry or concrete can also be tested to help identify causes of moisture/air infiltration (descriptions of these analyses follow).

Laboratory testing may help obtain a better understanding of existing material types, presence of contaminants, and the possibility of hazardous components.

This type of testing can also provide valuable information concerning proper surface preparation, material selection, and implementation of repairs. The following laboratory tests are some of the more useful when performing building envelope evaluations:

- a. *Gravimetric Analysis* - This test will determine moisture content. After weighing and recording the in-situ existing sample, completely dry the sample in an oven and re-weigh it. The weight difference indicates moisture content and is particularly useful for insulating materials. Testing moisture contents of samples is critical to verify results from non-destructive moisture scans.
- b. *Petrography* - Petrography determines the “make-up” of concrete. This test will indicate the size and type of aggregate, air/void ratio, type of cement, and general mix design data of the concrete. Most materials testing lab can perform this test.
- c. *Air Entrainment* - Provides an indication of the existing concrete’s durability and freeze-thaw resistance. Air entrainment is generally indicated by petrography.
- d. *Presence of Carbonization* - Accomplished by spraying a solution of phenothelene on the concrete substrate and recording the depth of the solution’s color change. This will indicate to what depth carbon dioxide has progressed into the concrete. Carbon dioxide will degrade the cement matrix of the concrete and lower the pH level of it. The layer surrounding the reinforcement is then destroyed, allowing corrosion of the reinforcing steel. Corrosion by carbonization usually occurs over a broad area.
- e. *Chloride Ion Content* - Chlorides from marine atmospheres or mists from road salts entering the concrete substrate, and salts originally introduced to the concrete via admixtures or aggregates can promote accelerated corrosion of reinforcing steel, usually at concentrated or specific locations. The chlorides are not consumed in the corrosion process but rather act as catalysts in the process. The corrosion will progress along the reinforcing bars causing concrete de-bonding, cracking, and spalling.
- f. *Reinforcement Placement, Depth, Quantity, and Type* - This information may be established with the use of a Pachometer or similar electronic metal detector. It is useful in determining required steel replacement and structural capacities during engineering analysis phases.

## 21.0 Engineering Analysis

Using information obtained from the field, laboratory results, and collected data, a comprehensive engineering analysis may be required. The engineering analysis should include an assessment of field and laboratory data, structural analysis as well as the following:

- ✓ Thermal Analysis
- ✓ Drainage Analysis

- ✓ Vapor Drive Analysis
- ✓ Fire Rating Requirements
- ✓ Cost Estimations

## **22.0 Deficiencies and Non-Conformance**

The Trade Subcontractor shall identify and list any outstanding deficiencies or procedures that were not completed successfully during any final testing. Documented deficiencies shall be submitted to WEBCOR/OBAYASHI within 48 hours of each test completion.

The Trade Subcontractor shall also provide in writing, the corrective action for each deficiency as required within 48 hours. The installing Trade Subcontractor and or vendor shall correct all outstanding issues or deficiencies in the materials or the installation of the materials and provide the commissioning team with dates and times for the required corrections and any re-testing.

## **23.0 Remedial Work**

General considerations for the repair of defects and replacement of components should include the following:

- 23.1 Determine the effect, if any; the repairs have on the structure, surroundings, and operations of the building.
- 23.2 Ensure proper preparation of surfaces to be repaired and provide chemical and mechanical bonds for new materials.
- 23.3 Material selection should include an understanding of performance limitations and should rely on the products past acceptable performance. Material selections should include consideration of the following:
  - ✓ Compatibility
  - ✓ Maintenance
  - ✓ Life cycle

## **24.0 Project Commissioning Closeout**

WEBCOR/OBAYASHI, the Owner, and/or the Owner's representative shall determine when the Trade Subcontractor commissioning process has been satisfactorily completed and when to submit the final report information and all other documentation to Webcor.

As part of the project turnover, the quality of all work will be reviewed to determine whether it is within specific and manufacturers' guidelines, industry standards, and code compliance.

WEBCOR/OBAYASHI, the Owner, and/or the Owner's representative consultant must be completely satisfied that the commissioning procedures have been performed accurately and professionally.

In the event the commissioning information or performance requirements outlined in the Waterproofing Quality Commissioning Procedure & Guidelines have not been met, WEBCOR/OBAYASHI may elect to withhold or make appropriate adjustments to the Trade Subcontractor's final billing.